

W.B. Howland Truss Co.
P.O. Box 700
Live Oak, FL 32064
(386)362-1235
(386)362-7124 (fax)

ROOF PITCH: 7/12
CLG PITCH: 4/12
OVERHANG: 1'6"
LOADING: 40
WIND LOAD: 130
EXPOSURE: C
FBC 2020 RESIDENTIAL
EXT. WALLS: 2x4 FRAMING
REVISED: 2/10/23

NOTES:

- INTERIOR GARAGE WALL USED AS INTERIOR BEARING POINTS.
- ALL VALLEYS FRAMED WITH TRUSS DESIGN.
- ALL GABLE END TRUSSES HAVE A DROPPED TOP CHORD FOR 2x4 OUTLOOKERS.

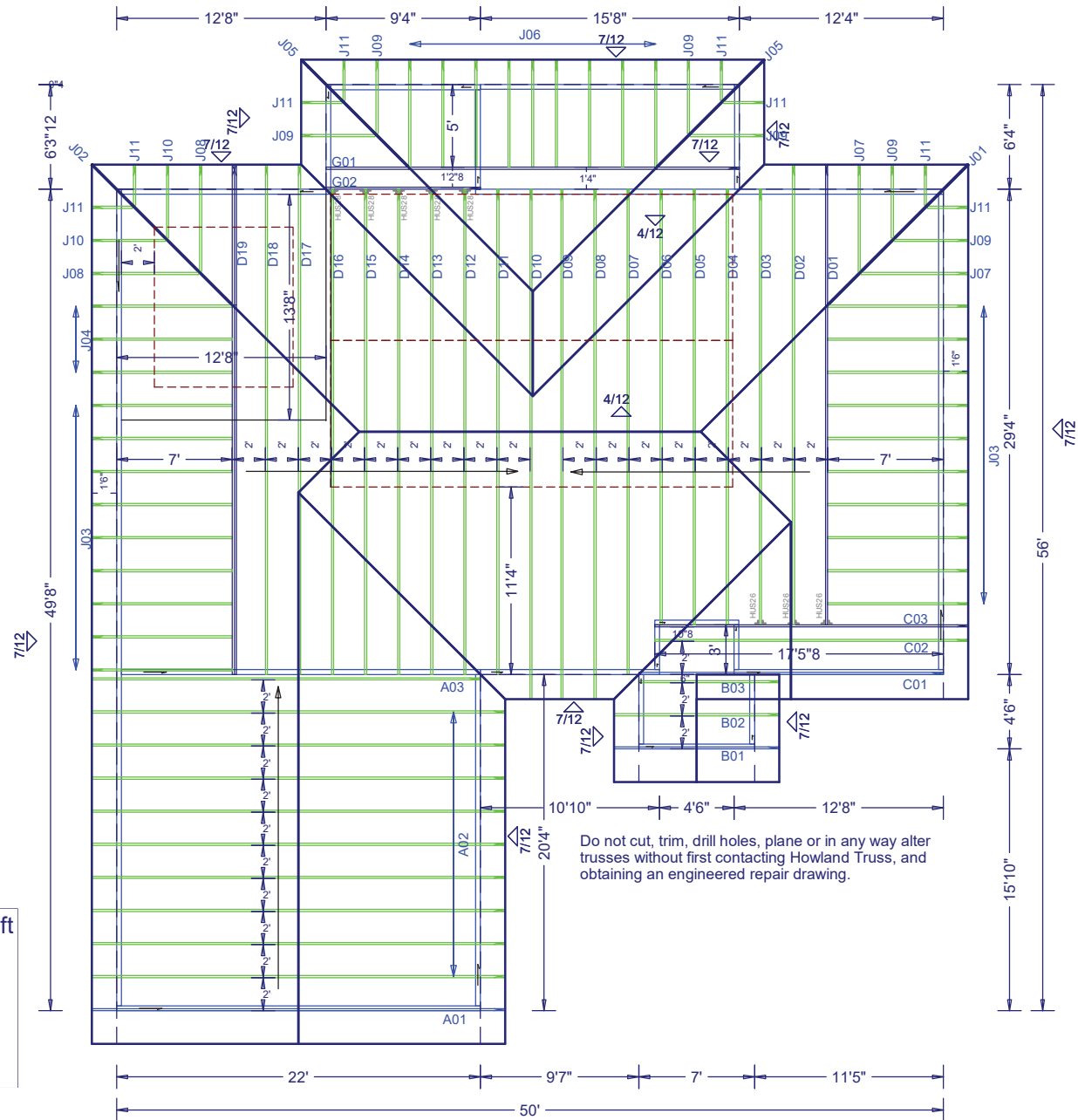
TRUSS TO TRUSS CONNECTIONS:

3 - HUS26
5 - HUS28

HUS26:
D01 TO C03
D02 TO C03
D03 TO C03

HUS28:
D12 TO G02
D13 TO G02
D14 TO G02
D15 TO G02
D16 TO G02

Roof Plane Sheathing Area = 2859 sq. ft
Gable Sheathing Area = 155 sq. ft
Total Sheathing Area = 3014 sq. ft
Fascia Material = 231 linear ft
Valley Flashing Material = 78 linear ft
Ridge Cap Material = 78 linear ft
Hip Ridge Material = 106 linear ft



JOB #: 23-8926

Job Name: Sunset 7
Customer: BRADLEY FRANKS
Designer: Chris
ADDRESS: 143 NW Germium Ct
SALESMAN: DB
: <Not Found>

JOB NO:
23-8926

PAGE NO:
1 OF 1

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COA #0 278

Florida Certificate of Product Approval #FL 1999
02/10/2023



Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com



Site Information:		Page 1:
Customer:	W. B. Howland Company, Inc.	Job Number: 23-8926
Job Description:	Sunset 7	
Address:	143 NW Germium Ct, Lake City, FL 32055	

Job Engineering Criteria:	
Design Code:	FBC 7th Ed. 2020 Res.
Wind Standard:	ASCE 7-16
Wind Speed (mph):	130
Building Type:	Closed
	Intell/VIEW Version: 21.01.01A through 21.02.01
	JRef #: 1XN32150003
	Design Loading (psf): 40.00

This package contains general notes pages, 41 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	041.23.0913.09137	A01	2	041.23.0912.21088	A02
3	041.23.0912.19222	A03	4	041.23.0913.12993	B01
5	041.23.0912.21341	B02	6	041.23.0912.20768	B03
7	041.23.0912.21648	C01	8	041.23.0912.22217	C02
9	041.23.0912.20297	C03	10	041.23.0912.22043	D01
11	041.23.0912.21281	D02	12	041.23.0912.19322	D03
13	041.23.0912.19924	D04	14	041.23.0912.19333	D05
15	041.23.0912.20682	D06	16	041.23.0912.22420	D07
17	041.23.0912.20164	D08	18	041.23.0912.20625	D09
19	041.23.0912.19446	D10	20	041.23.0912.22028	D11
21	041.23.0912.20380	D12	22	041.23.0912.19088	D13
23	041.23.0912.19498	D14	24	041.23.0912.20654	D15
25	041.23.0912.21666	D16	26	041.23.0912.20046	D17
27	041.23.0912.22164	D18	28	041.23.0912.21350	D19
29	041.23.0912.19090	G01	30	041.23.0912.20396	G02
31	041.23.0912.19176	J01	32	041.23.0912.21795	J02
33	041.23.0912.20622	J03	34	041.23.0912.20964	J04
35	041.23.0912.20107	J05	36	041.23.0912.22298	J06
37	041.23.0912.21529	J07	38	041.23.0912.20824	J08
39	041.23.0912.21512	J09	40	041.23.0912.22465	J10
41	041.23.0912.21614	J11	42	A14015ENC160118	
43	BRCLBSUB0119		44	CNNAILSP1014	
45	DEFLCAMB1014		46	GBLETTIN0118	
47	S14015ENC160118				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TC DL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
lc = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TC DL = Top Chord standard design Dead Load in pounds per square foot.
TC LL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

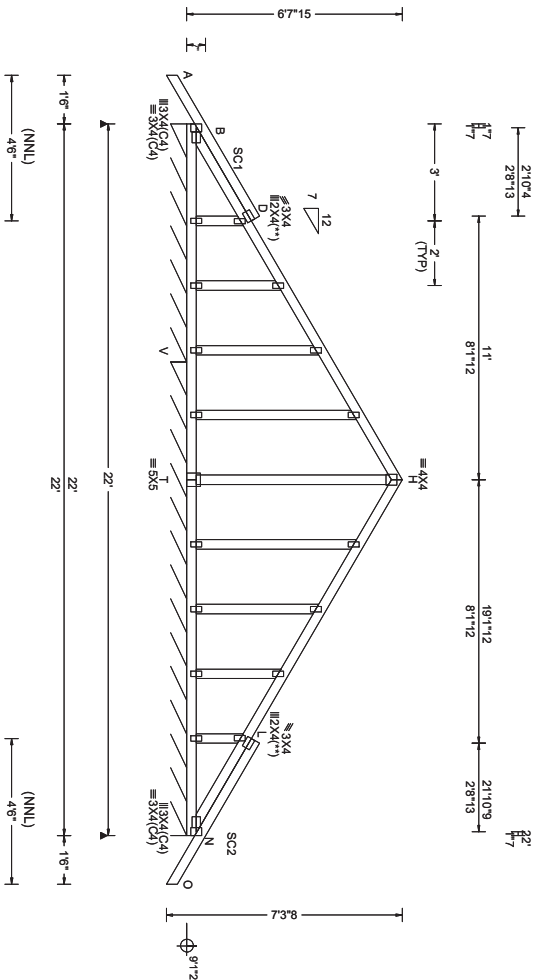
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SECON: 450705	GABL	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T16
FROM: CDM		Qty: 1	Sunset 7	DwnNo: 041.23.0913.09137	SSB / WHK	02/10/2023
			Truss Label: A01			



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.002 B 999 180 HORZ(LL): 0.002 K - - HORZ(TL): 0.003 K - - Creep Factor: 2.0 Max TC CSI: 0.207 Max BC CSI: 0.031 Max Web CSI: 0.087	Loc R+ / R- / Rh / Rw / U / RL B* 103 /- /- /67 /23 /28 V* 87 /- /- /52 /12 /- Wind reactions based on MWFRS B Big Wld = 88.3 Min Req = - V Big Wld = 175 Min Req = - Bearings B & V are a rigid surface. Members not listed have forces less than 375#
		VIEW Ver: 21.02.01.1214.12		

Additional Notes

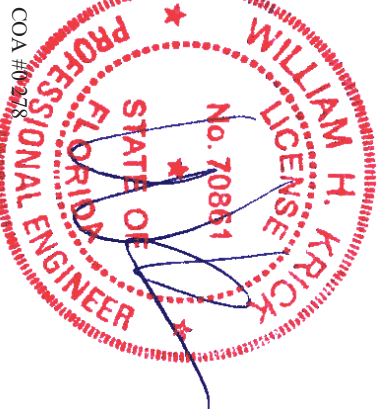
Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes
All plates are 2X4 except as noted.
(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

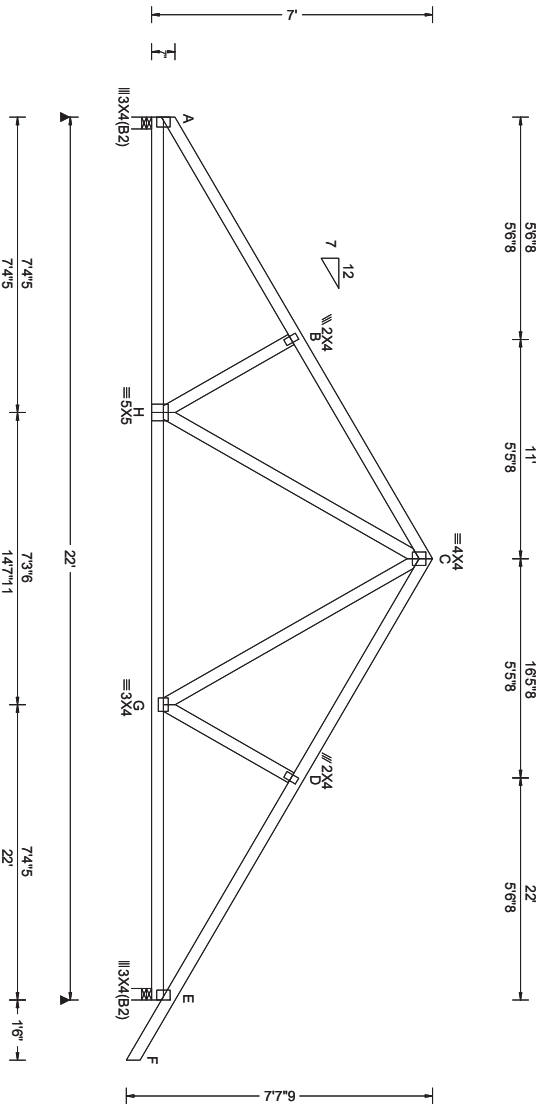
Additional Notes
See DWGS A14015ENC160118 & GBLLETTIN0118 for gable wind bracing and other requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.
The overall height of this truss excluding overhang is 6'-7-15".



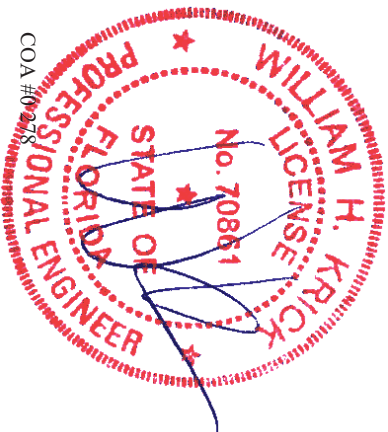
Florida State of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.
Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANS/ITP 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITP 1 Sec. 2.
For more information see these web sites: Alpine: alpineitw.com, TPI: tpinst.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

SECTN: 409467 /	COMN	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T31
FROM: CDM		Qty: 1	Sunset 7	DwnDn: 04/1.23.0912.19222		
			Truss Label: A03	/ WHK	02/10/2023	



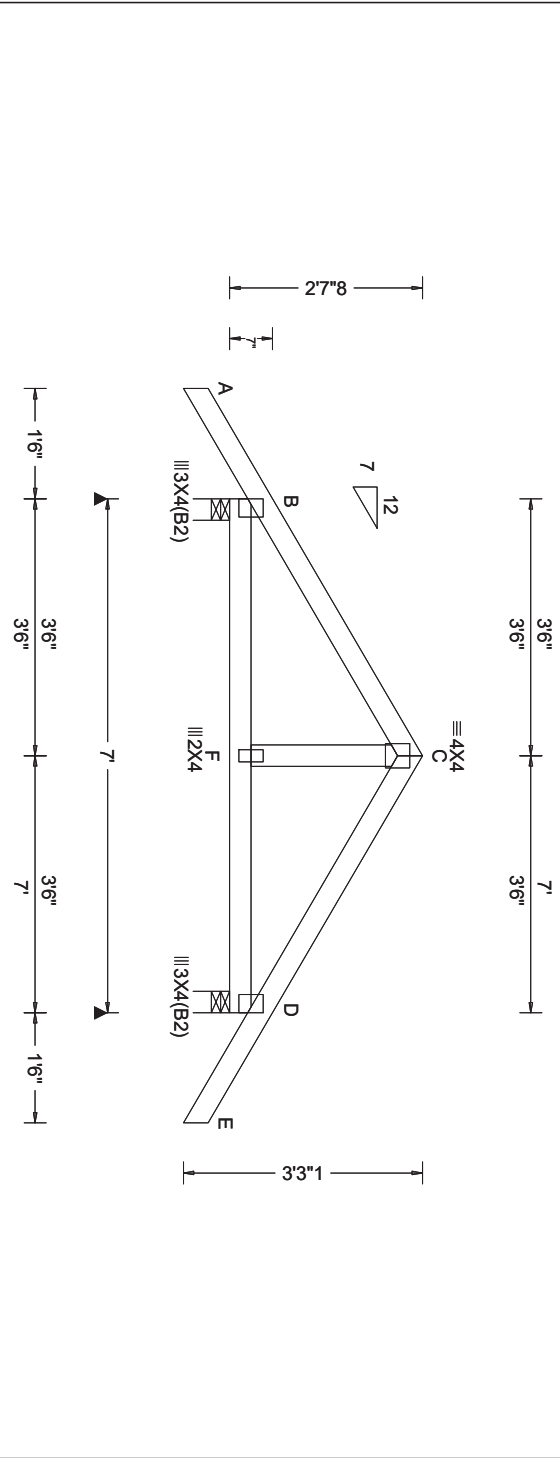
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(LL): 0.038 G 999 240 VERT(CL): 0.077 G 999 180 HORZ(LL): 0.019 E - - HORZ(TL): 0.039 E - - Creep Factor: 2.0 Max TC CSI: 0.187 Max BC CSI: 0.210 Max Web CSI: 0.059	Loc R+ /R- /Rh /Rw /U /RL A 911 /- /- /524 /10 /193 E 1020 /- /- /610 /17 /- Wind reactions based on MWFRS A Big Wld = 3.5 Min Req = 1.5 E Big Wld = 3.5 Min Req = 1.5 Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Wind		VIEW Ver: 21.01.01A.0521.20	A - B 281 -1371 C - D 312 -1194 B - C 318 -1206 D - E 275 -1359 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - H 1105 -142 G - E 1090 -135 H - G 754 -4 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. H - C 444 -100 C - G 428 -90



COA #00278
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 Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANS/ITP 11 or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITP 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com, TPI: tpist.org, SBCEA: sbceacomponents.com, ICC: iccsafe.org, AWC: awc.org

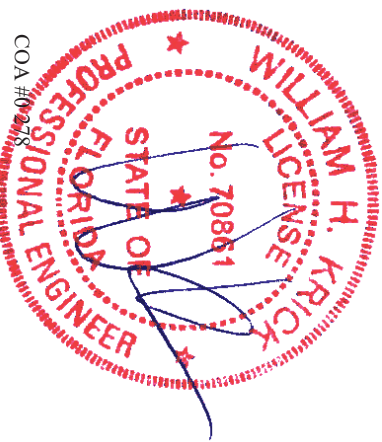
SECN: 409469 / FROM: CDM	COMN	Ply: 1 Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: B02	Cust: R 215 JRef: 1XN32150003 T54 / DwnNo: 041.23.0912.21341 / YK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P in PSF)	Def/CSI Criteria	▲Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.004 F 999 240 VERT(CL): 0.007 F 999 180 HORZ(L): 0.002 D - - HORZ(TL): 0.004 D - - Creep Factor: 2.0 Max TC CSI: 0.087 Max BC CSI: 0.043 Max Web CSI: 0.013	Loc R+ /R- /Rh /Rw /U /RL B 393 -/- /- /258 /72 /97 D 393 -/- /- /258 /72 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.5 D Big Wld = 3.5 Min Req = 1.5 Bearings B & D are a rigid surface. Members not listed have forces less than 375#
Lumber	Wind Criteria	VIEW Ver: 21.01.01A.0521.20		

Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP M-31;

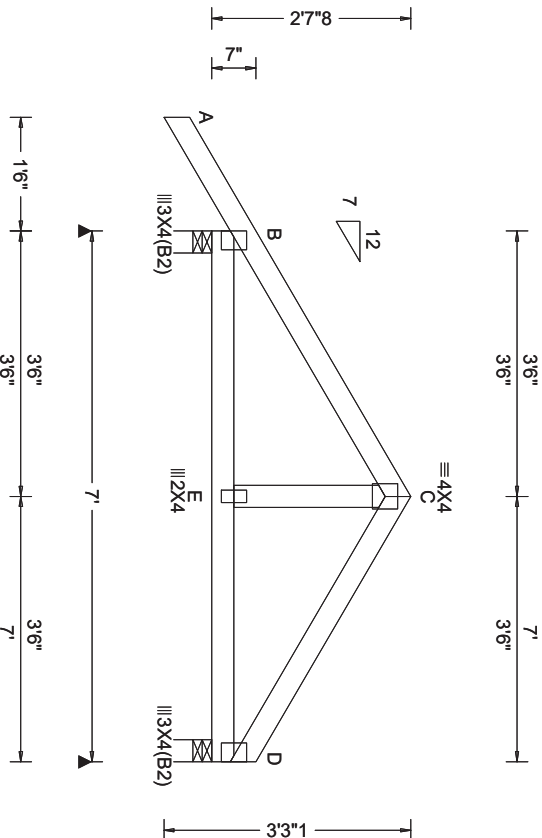
Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.



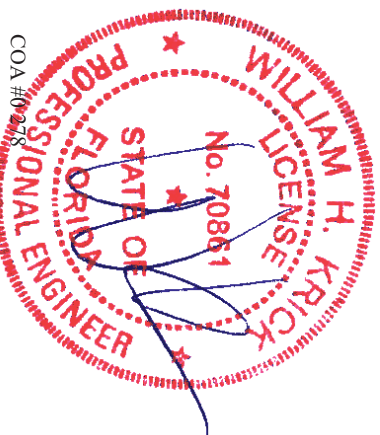
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SECTN: 409470 /	COMN	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T1 /
FROM: CDM		Qty: 1	Sunset 7			
			Truss Label: B03		DwnNo: 041.23.0912.20768	/ YK 02/10/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(L): 0.003 E 999 240 VERT(CL): 0.006 E 999 180 HORZ(L): 0.001 D - - HORZ(T/L): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.077 Max BC CSI: 0.048 Max Web CSI: 0.013	Loc R+ / R- / Rh / Rw / U / RL B 404 /- /- /258 /75 /80 D 279 /- /- /165 /42 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.5 D Big Wld = 3.5 Min Req = 1.5 Bearings B & D are a rigid surface. Members not listed have forces less than 375#
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;			
Wind	Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types.			
			VIEW Ver: 21.01.01A.0521.20	



Florida State Seal of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

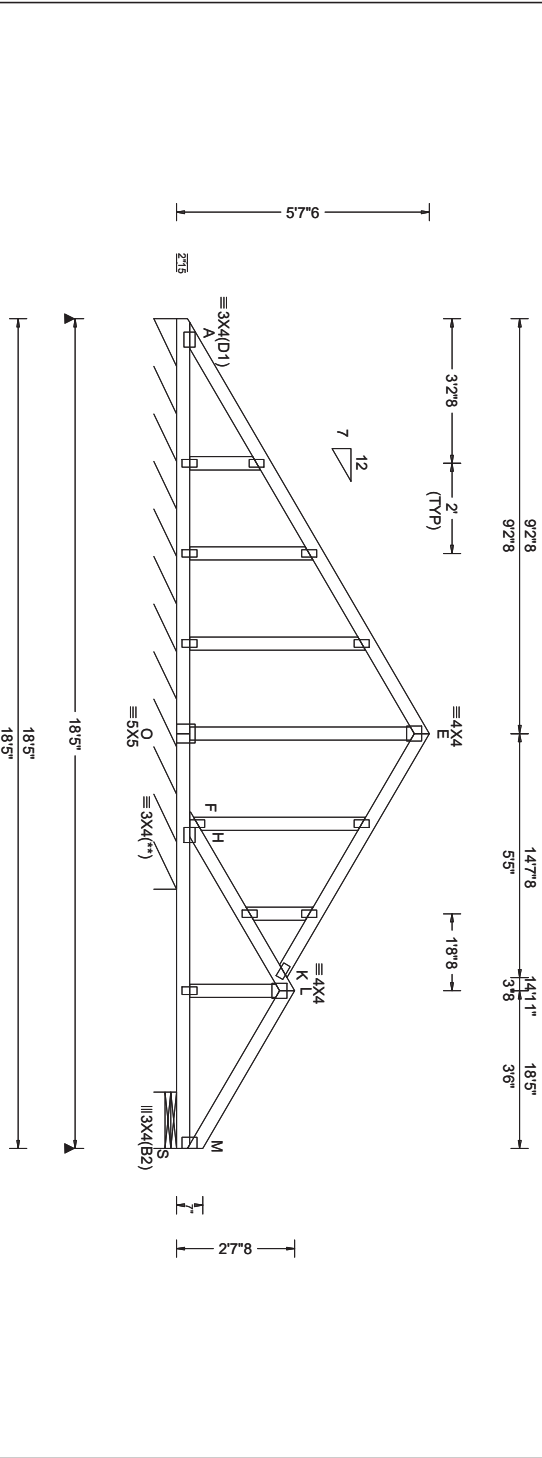
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details. Unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com, TPI: tpist.org, SBCEA: sbceacomponents.com, ICC: iccsafe.org, AWC: awc.org

SECTN: 409471 /	GABL	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T3 /
FROM: CDM		Qty: 1	Sunset 7		DwnNo: 041.23.0912.21648	
			Truss Label: C01		/ YK	02/10/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.003 I 999 240 VERT(CL): 0.007 I 999 180 HORZ(L): 0.002 J - - HORZ(TL): 0.003 J - - Creep Factor: 2.0 Max TC CSI: 0.047 Max BC CSI: 0.029 Max Web CSI: 0.042	Loc R+ /R- /Rh /Rw /U /RL A* 99 /- /- /54 /16 /11 S 267 /- /- /162 /37 /- Wind reactions based on MWFRS A Big Wld = 151 Min Req = - S Big Wld = 15.0 Min Req = 1.5 Bearings A & S are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. F - K 337 - 383
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;		VIEW Ver: 21.01.01A.0521.20	

Plating Notes

All plates are 2X4 except as noted.
 (***) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

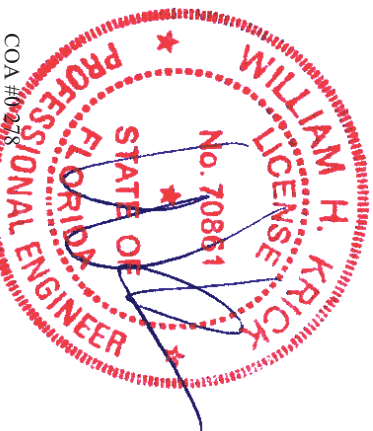
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS S14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.



COA #0218
 Florida Seal of Product Approval #FL 1999

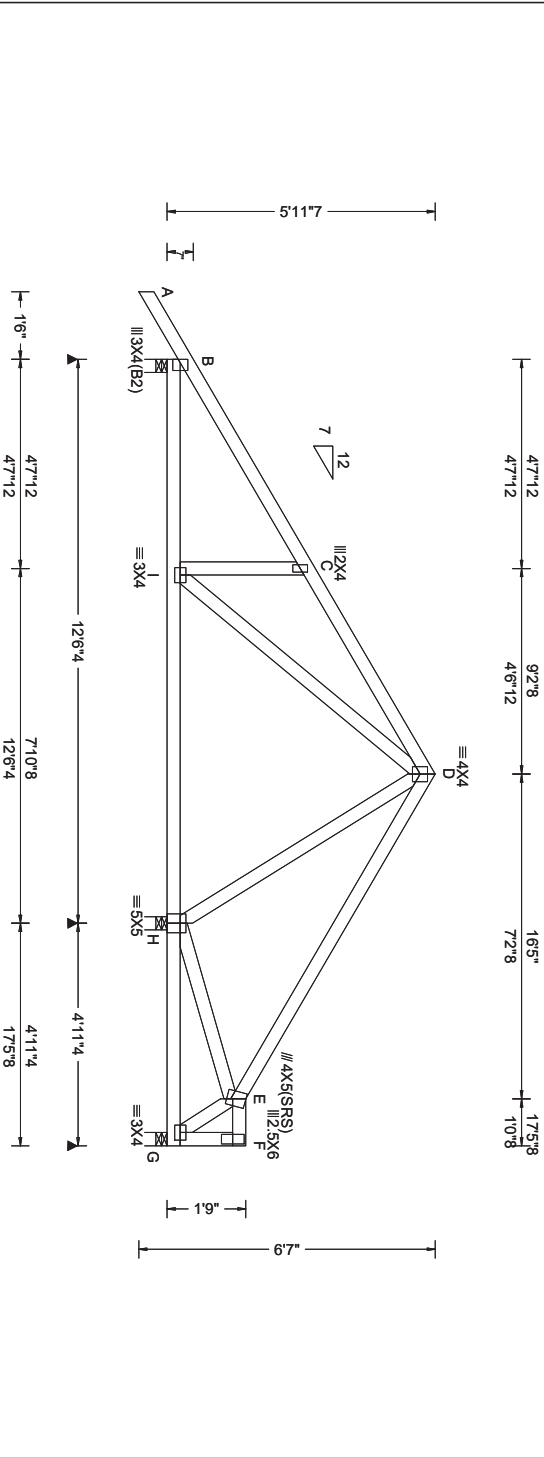
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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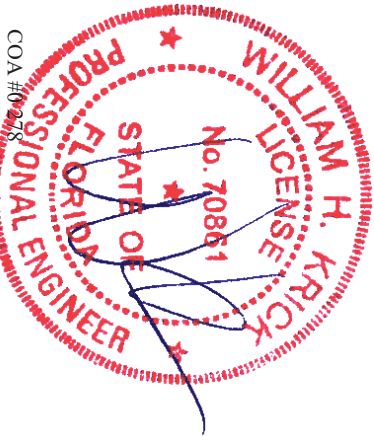
For more information see these web sites: Alpine: alpineitw.com, TPI: tpist.org, SBCEA: sbceacomponents.com, ICC: iccsafe.org, AWC: awc.org

SECTN: 409472 / FROM: CDM	COMN Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: C02	Cust R 215 JRef: 1XN32150003 T2 / DwnNo: 041.23.0912.222217 / YK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	DeficSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.012 C 999 240 VERT(CL): 0.025 C 999 180 HORZ(L): 0.004 C - - HORZ(TL): 0.007 C - - Creep Factor: 2.0 Max TC CSI: 0.290 Max BC CSI: 0.148 Max Web CSI: 0.303	Loc R+ /R- /Rh /Rw /U /RL B 575 /- /- /360 /104 /155 H 912 /- /- /521 /140 /- G 123 /-35 /- /51 /40 /- Wind reactions based on MWFRS B Big Wid = 3.5 Min Req = 1.5 H Big Wid = 3.5 Min Req = 1.5 G Big Wid = 3.5 Min Req = 1.5 Bearings B, H, & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

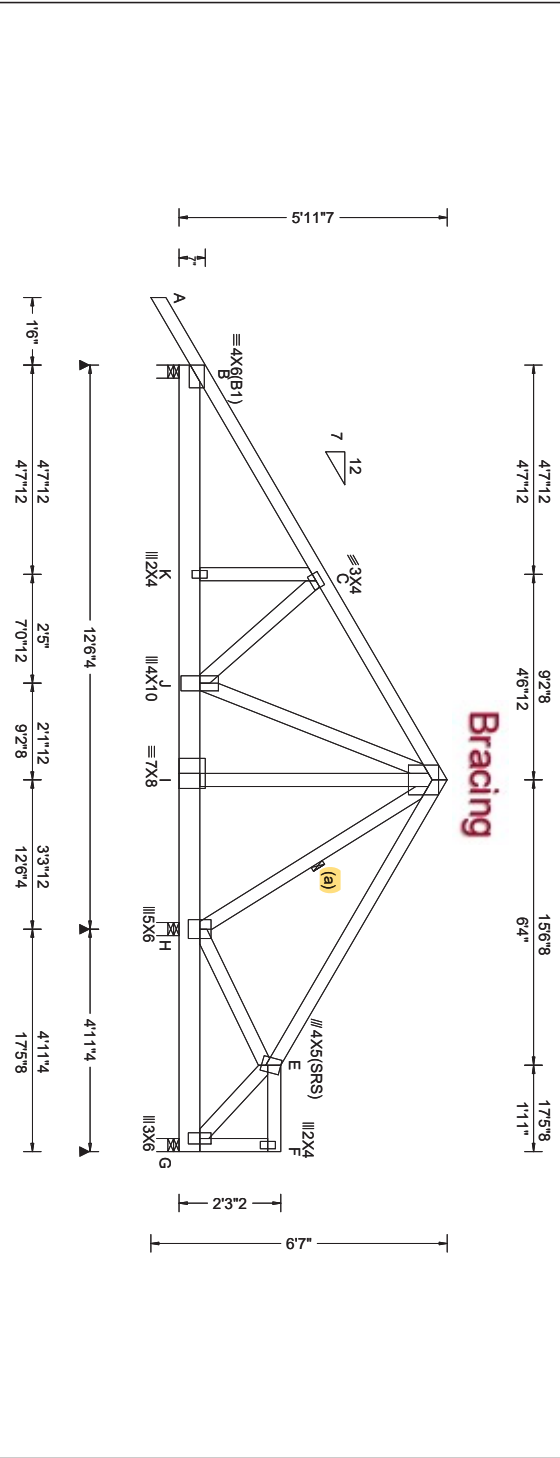
Lumber Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;	Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.
			B - C 110 - 613 C - D 221 - 598	I - D 523 - 147 H - E 273 - 462 D - H 173 - 696



COA #0218
 Florida Seal of Product Approval #FL 1999

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 For more information see these web sites: Alpine: alpinetw.com, TPI: tpinst.org, SBCEA: sbceacomponents.com, ICC: iccsafe.org, AWC: awc.org





Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)					
TTCL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/Def L/#	Loc	Gravity	Non-Gravity			
TCDL:	10.00	Speed: 130 mph	Pf: NA	Ce: NA		VERT(LL): 0.042 J 999 240	R+	/R-	/Rh	/Rw /U /RL		
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 0.065 J 999 180	H	4092	/-	/-	/712 /-	
BCDL:	10.00	Risk Category: II	Snow Duration: NA			HORZ(LL): 0.010 C - -	G	98	/-203	/-	/13 /-	/-
Des Ld:	40.00	EXP: C Kzt: NA				HORZ(TL): 0.021 C - -	Creep Factor: 2.0					
NCRCLL:	10.00	Mean Height: 15.00 ft	Building Code:				Max TC CSI: 0.260					
Softt:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.				Max BC CSI: 0.330					
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014				Max Web CSI: 0.431					
Spacing: 24.0 "		C&C Dist a: 3.00 ft	Rep Fac: Varies by Ld Case									
		Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)									
		GCap: 0.18	Plate Type(s):									
		Wind Duration: 1.60	WAVE			VIEW Var: 21.0;1.01A.0521.20						
Maximum Top Chord Forces Per Ply (lbs)												
Chords			Tens Comp.		Chords		Tens. Comp.					

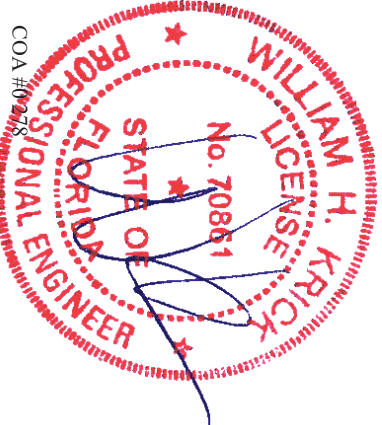
Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400F-2.0E;
Webs: 2x4 SP M-31;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Special Loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 pif at -1.50 to 63 pif at 7.06
TC: From 32 pif at 7.06 to 32 pif at 11.06
TC: From 63 pif at 11.06 to 63 pif at 17.46
BC: From 5 pif at -1.50 to 5 pif at 0.00
BC: From 20 pif at 0.00 to 20 pif at 7.06
BC: From 10 pif at 7.06 to 10 pif at 11.06
BC: From 20 pif at 11.06 to 20 pif at 17.46
BC: 2346 lb Conc. Load at 7.06
BC: 1090 lb Conc. Load at 9.06
BC: 1093 lb Conc. Load at 11.06

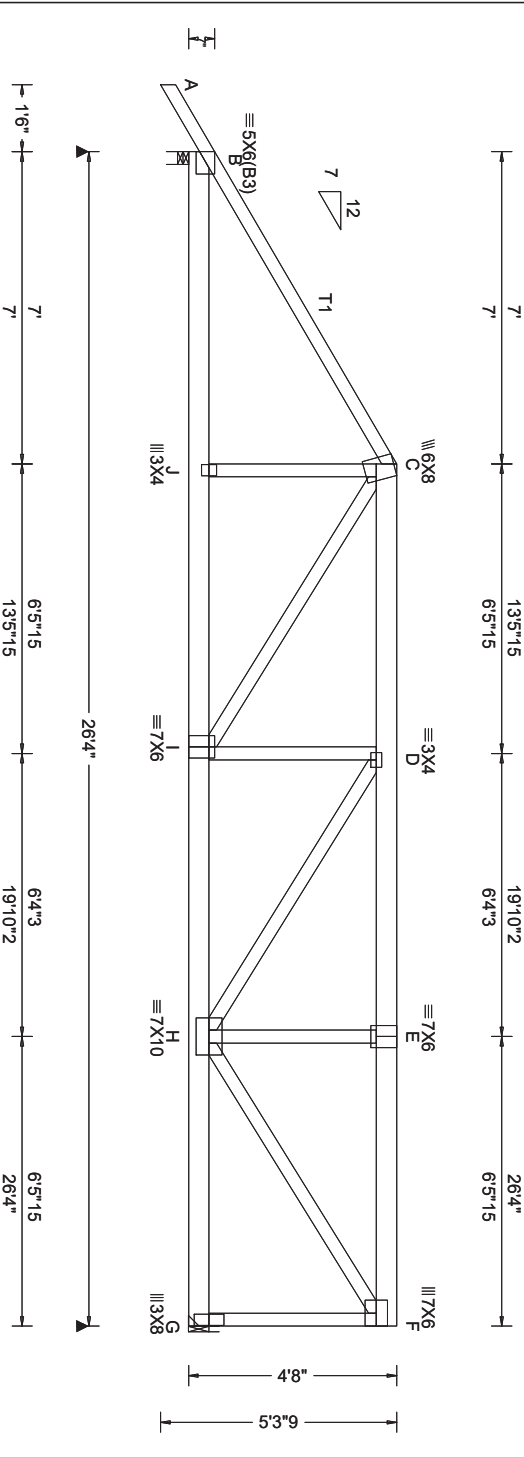
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0228
Florida Certificate of Product Approval #FL 1999

SECTN: 409502 /	HIPM	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T25 /
FROM: CDM		Qty: 1	Sunset 7		DwnNo: 041.23.0912.22043	
			Truss Label: D01		/ YK	02/10/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 11.28 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.094 D 999 240 VERT(CL): 0.197 D 999 180 HORZ(L): 0.024 H - - HORZ(TL): 0.050 H - - Creep Factor: 2.0 Max TC CSI: 0.357 Max BC CSI: 0.399 Max Web CSI: 0.646 VIEW Ver: 21.01.01A.0521.20	Gravity Loc R+ /R- /Rh /Rw /U /RL B 2236 /- /- /- /381 /- G 2346 /- /- /- /375 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.9 G Big Wld = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.
Lumber	Wind	Wind	VIEW Ver: 21.01.01A.0521.20	

Top chord: 2x6 SP 2400F-2.0E; T1 2x4 SP M-31;
 Bot chord: 2x6 SP 2400F-2.0E;
 Webs: 2x4 SP M-31;

Wind loads and reactions based on MWFRS.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

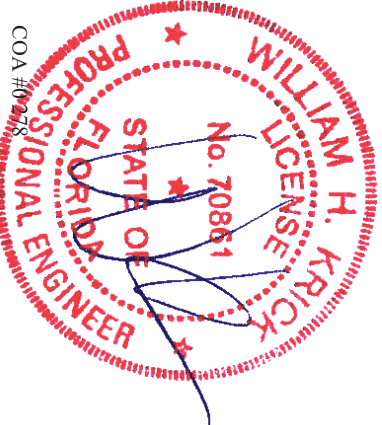
Bearing at location x=26'1" uses the following support conditions: 26'1" Bearing G (26'1", 9'1"2) HUS26 Supporting Member: (1)2x6 SP 2400F-2.0E (14) 0.148"x3" nails into supporting member, (6) 0.148"x3" nails into supported member.

Loading

#1 hip supports 7.0-0 jacks with no webs.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.



COA #09278
 Florida Certificate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

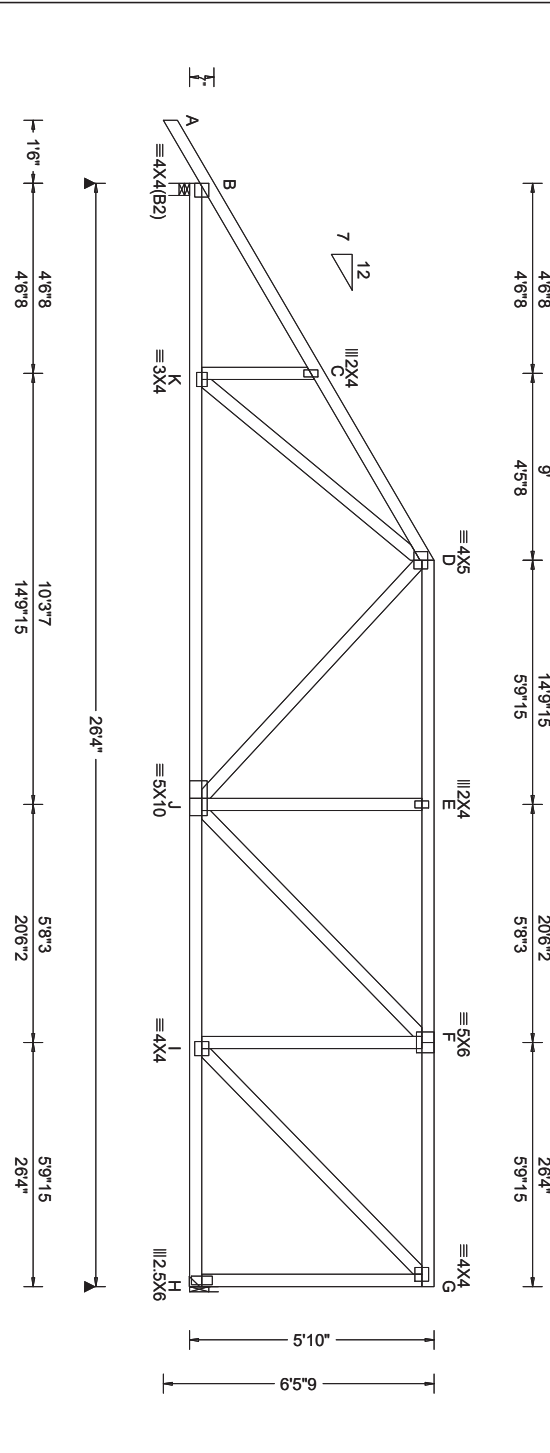
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For more information see these web sites: Alpine: alpinetw.com; TPI: tpist.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	2964 -471	I - H	3836 -623
J - I	2984 -470		
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - J	589 0	E - H	347 -912
C - I	1004 -165	H - F	3426 -544
D - H	201 -1161	F - G	429 -2222

SECTN: 409473 / FROM: CDM	HIPM Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: D02	Cust R 215 JRef: 1XN32150003 T30 / DwnNo: 041.23.0912.21281 / YK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCRCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(L): 0.049 E 999 240 VERT(CL): 0.100 E 999 180 HORZ(L): 0.017 I - - HORZ(TL): 0.035 I - - Creep Factor: 2.0 Max TC CSI: 0.234 Max BC CSI: 0.341 Max Web CSI: 0.437	Gravity / R- / Rh / Rw / U / RL Loc R+ / R- / Rh / Rw / U / RL B 1201 /- /- /733 /196 /210 H 1090 /- /- /568 /219 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.5 H Big Wld = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;		VIEW Ver: 21.01.01A.0521.20	B - C 601 -1738 E - F 720 -1326 C - D 679 -1678 F - G 508 -909 D - E 721 -1326

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=26'1" uses the following support conditions: 26'1" Bearing H (26'1", 9'1'2) HUS26 Supporting Member: (1)2x6 SP 2400F-2.0E (14) 0.148"x3" nails into supporting member, (4) 0.148"x3" nails into supported member.

Purlins

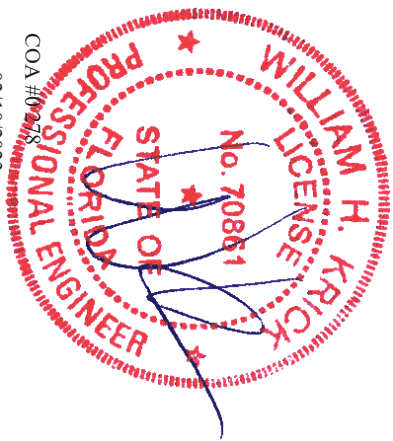
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right and vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



Florida State Seal of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

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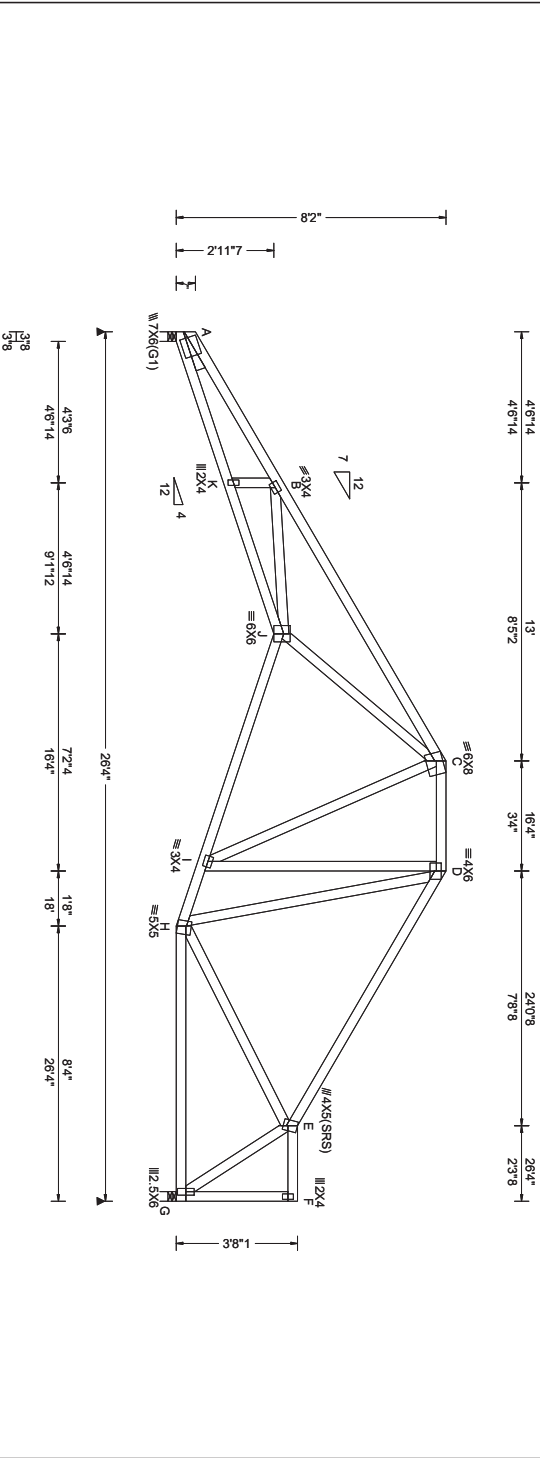
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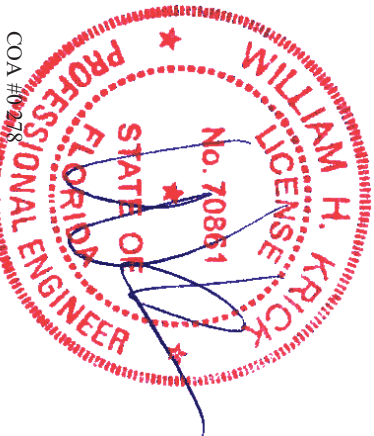


SECTN: 409475 /	SPEC	Ply: 1	Job Number: 23-8926	Cust R 215	JRef: 1XN32150003	T38 /
FROM: CDM		Qty: 1	Sunset 7			
			Truss Label: D04	DwnNo: 041.23.0912.19924		
				/ YK	02/10/2023	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.135 J 999 240 VERT(CL): 0.283 J 999 180 HORZ(L): 0.092 G - - HORZ(TL): 0.192 G - - Creep Factor: 2.0 Max TC CSI: 0.540 Max BC CSI: 0.430 Max Web CSI: 0.338	Loc R+ /R- /Rh /Rw /U /RL A 1108 /- /- /651 /171 /195 G 1102 /- /- /577 /191 /- Wind reactions based on MWFRS A Big Wld = 3.5 Min Req = 1.5 G Big Wld = 3.5 Min Req = 1.5 Bearings A & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31; Lt Stud Wedge: 2x4 SP M-31;			A - B 917 -3438 C - D 411 -1000 B - C 785 -2911 D - E 367 -1183

Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc. Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - K 3009 -890 I - H 1030 -261 K - J 3063 -907 H - G 730 -256 J - I 1296 -346 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - J 288 -482 I - D 500 -106 J - C 1928 -428 E - G 463 -1283 C - I 183 -542
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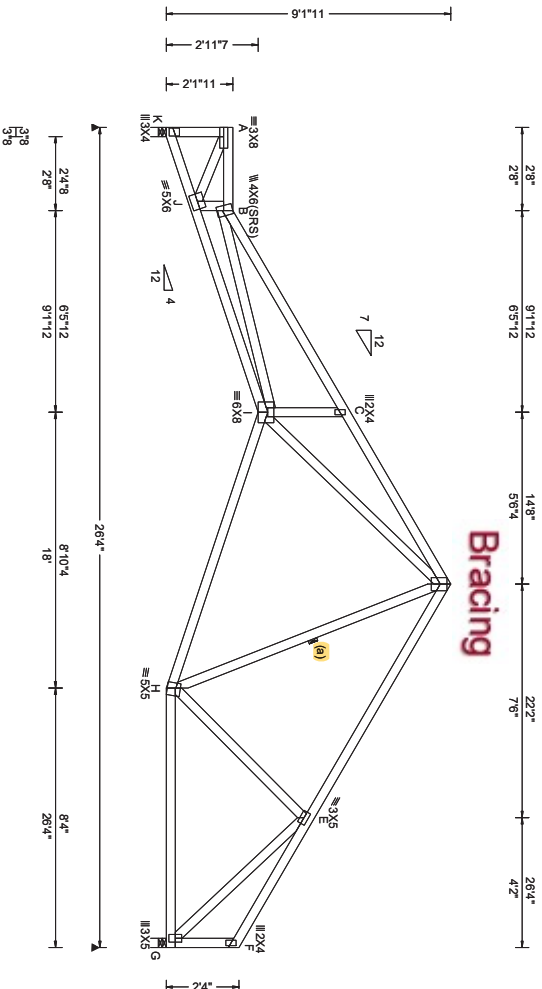


COA #0218
 Florida State of Product Approval #FL 1999

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SECTN: 409476 /	SPEC	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T42 /
FROM: CDM		Qty: 1	Sunset 7		Dwnbrc: 041.23.0912.19333	
			Truss Label: D05		/ YK	02/10/2023



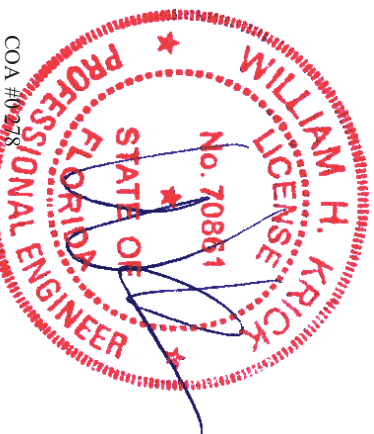
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(LL): 0.133 C 999 240 VERT(CL): 0.279 C 999 180 HORZ(LL): 0.087 G - - HORZ(TL): 0.183 G - - Creep Factor: 2.0 Max TC CSI: 0.307 Max BC CSI: 0.330 Max Web CSI: 0.468	Loc R+ /R- /Rh /Rw /U /RL K 1108 /- /- /607 /13 /180 G 1102 /- /- /609 /10 /- Wind reactions based on MWFRS K Big Wld = 3.5 Min Req = 1.5 G Big Wld = 3.5 Min Req = 1.5 Bearings K & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;			A - B 494 -2233 C - D 688 -3077 B - C 558 -3101 D - E 307 -1171

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

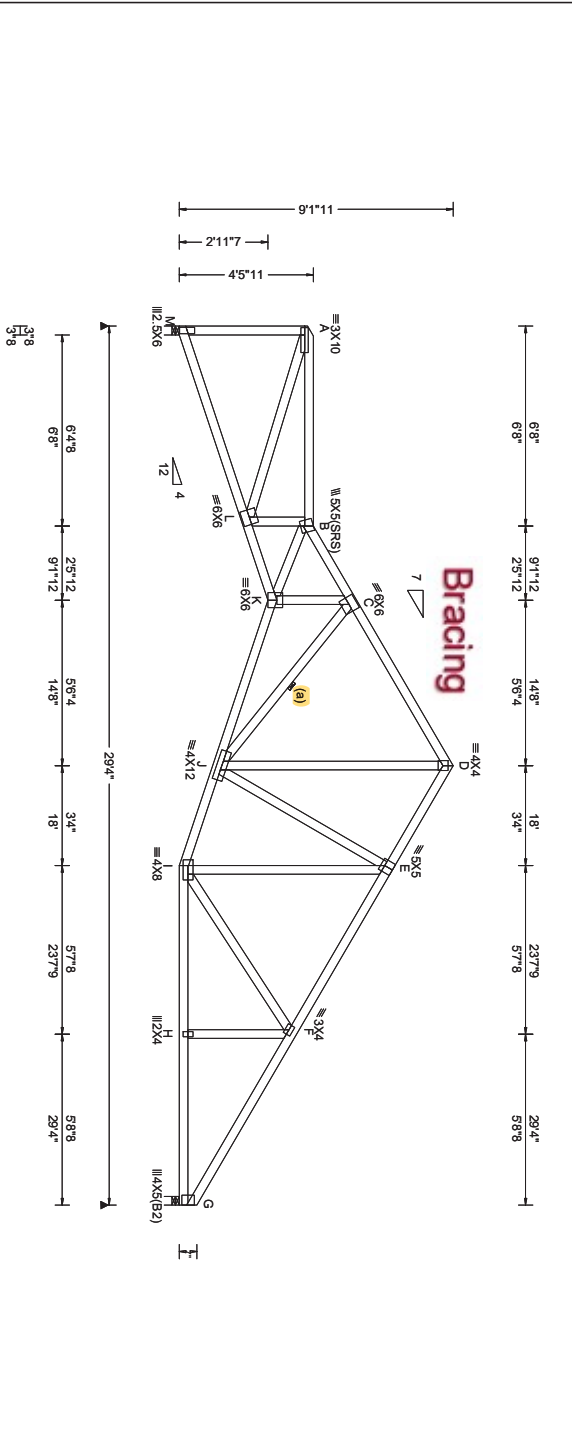
Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Deflection
Max JT VERT DEFL: LL: 0.13" DL: 0.15" See detail DEF/CAMB 1014 for camber recommendations.
Provide for adequate drainage of roof.



COA #02478
Flordate of Product Approval #FL 1999

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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,P7 in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL: 20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def L/#	Loc R+ / R-	Gravity / Rh	Non-Gravity / Rw / U		/ RL	
TCDL: 10.00	Speed: 130 mph	Pf: NA	Ce: NA	Ce: NA	VERT(L): 0.164 K 999 240	M 1230	- / -	1637	172	1220	
BCLL: 0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL): 0.344 K 999 180	G 1227	- / -	1717	18	1-	
BCDL: 10.00	Risk Category: II	Snow Duration: NA			HORZ(L): 0.105 G - -						
Des Ld: 40.00	EXP: C Kzt: NA				HORZ(T/L): 0.220 G - -						
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:			Creep Factor: 2.0						
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.			Max TC CSI: 0.455						
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014			Max BC CSI: 0.333						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes			Max Web CSI: 0.323						
	Loc. from endwall: not in 9.00 ft	FT/RT: 20(0)/10(0)									
	GCpl: 0.18	Plate Type(s):									
	Wind Duration: 1.60	WAVE			VIEW Ver: 21.01.01A.0521.20	A - B	699 - 2970	D - E	401 - 1430		

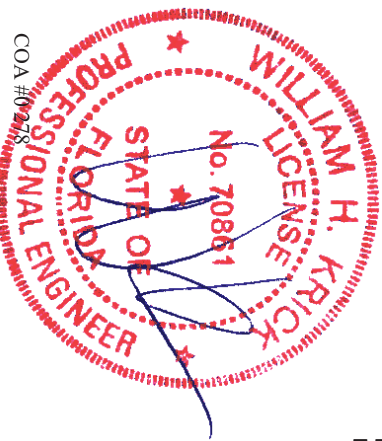
Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

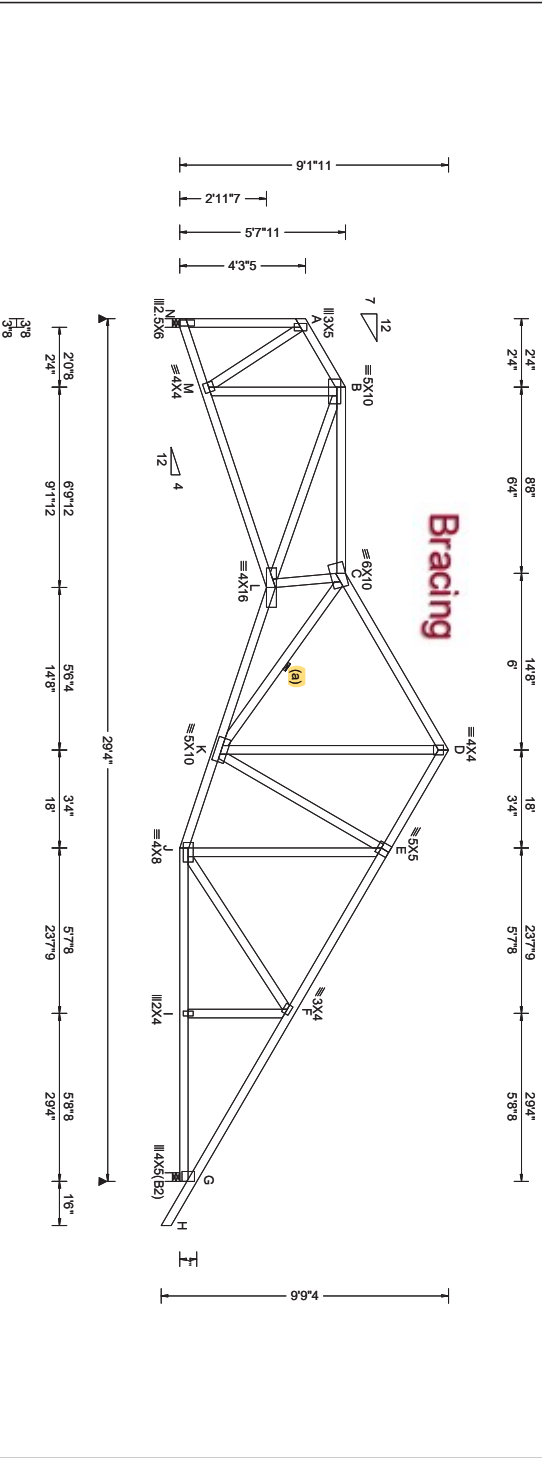
Deflection
Max JT VERT DEFL: LL: 0.16" DL: 0.18" See detail DEF/CAMB/014 for camber recommendations.
Provide for adequate drainage of roof.



COA #00278
Florida State Seal of Product Approval #FL 1999

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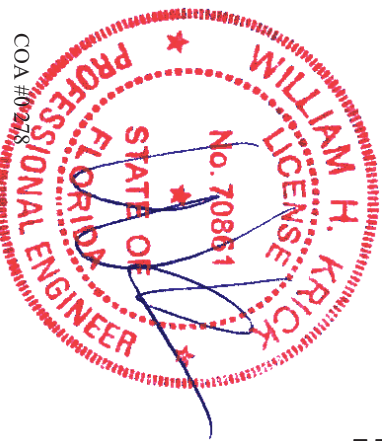
Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL: 20.00	Wind Std: ASCE 7-16	Wind Std: ASCE 7-16		Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def L/#	Gravity		Non-Gravity	
TCCL: 10.00	Speed: 130 mph	Speed: 130 mph		Pf: NA	Ce: NA	Ce: NA	VERT(L): 0.168 L 999 240	Loc R+ / R-	/ Rh	/ Rw	/ U / RL
BCLL: 0.00	Enclosure: Closed	Enclosure: Closed		Lu: NA	Cs: NA	-	VERT(CL): 0.349 L 999 180	N 1229 /- /-	/638 /61	/237	
BCDL: 10.00	Risk Category: II	Risk Category: II		EXP: C	Kzt: NA	-	HORZ(L): 0.108 G - - -	G /1332 /-	/-	/806 /30	/-
Des Ld: 40.00	Mean Height: 15.00 ft	Mean Height: 15.00 ft		Building Code:			HORZ(TL): 0.225 G - - -	Wind reactions based on MWFRS			
NCBCLL: 10.00	TCCL: 5.0 psf	TCCL: 5.0 psf		FBC 7th Ed. 2020 Res.			Creep Factor: 2.0	N Big Wld = 3.5 Min Req = 1.5			
Sofft: 2.00	BCDL: 5.0 psf	BCDL: 5.0 psf		TPI Std: 2014			Max TC CSI: 0.309	G Big Wld = 3.5 Min Req = 1.5			
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	MWFRS Parallel Dist: h to 2h		Rep Fac: Yes			Max BC CSI: 0.372	Bearings N & G are a rigid surface.			
Spacing: 24.0 "	C&C Dist a: 3.00 ft	C&C Dist a: 3.00 ft		FT/RT:20(0)/10(0)			Max Web CSI: 0.410	Members not listed have forces less than 375#			
	Loc. from endwall: not in 9.00 ft	Loc. from endwall: not in 9.00 ft		Plate Type(s):				Maximum Top Chord Forces Per Ply (lbs)			
	GCpl: 0.18	GCpl: 0.18		WAVE				Chords		Tens Comp.	
	Wind Duration: 1.60	Wind Duration: 1.60					VIEW Ver: 21.01.01A.0521.20	A - B		D - E	
								P - C	215 - 736	428 - 1424	
								E - F	2925	440	4544

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

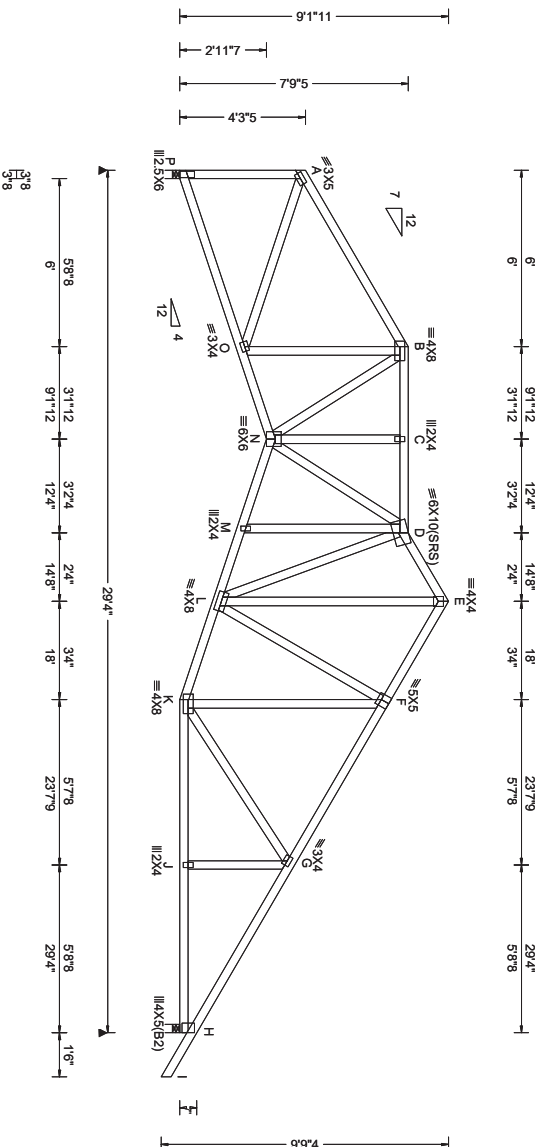
Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



Florida State Seal of Product Approval #FL 1999

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SECTN: 409481 /	SPEC	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T10
FROM: CDM		Qty: 1	Sunset 7	DwnNo: 041.23.0912.19446		
			Truss Label: D10	/ WHK	02/10/2023	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.070 D 999 240 VERT(CL): 0.146 D 999 180 HORZ(L): 0.048 H - - HORZ(TL): 0.100 H - - Creep Factor: 2.0 Max TC CSI: 0.249 Max BC CSI: 0.350 Max Web CSI: 0.518	Loc R+ /R- /Rh /Rw /U /RL P 1229 /- /- /641 /42 /237 H 1332 /- /- /814 /49 /- Wind reactions based on MWFRS P Big Wld = 3.5 Min Req = 1.5 H Big Wld = 3.5 Min Req = 1.5 Bearings P & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;			A - B 389 -1358 E - F 488 -1419 B - C 546 -1707 F - G 460 -1543 C - D 546 -1706 G - H 451 -1920 D - E 493 -1388

Purlins

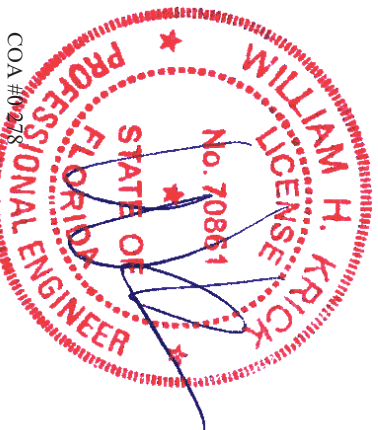
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0218
 Florida Seal of Product Approval #FL 1999

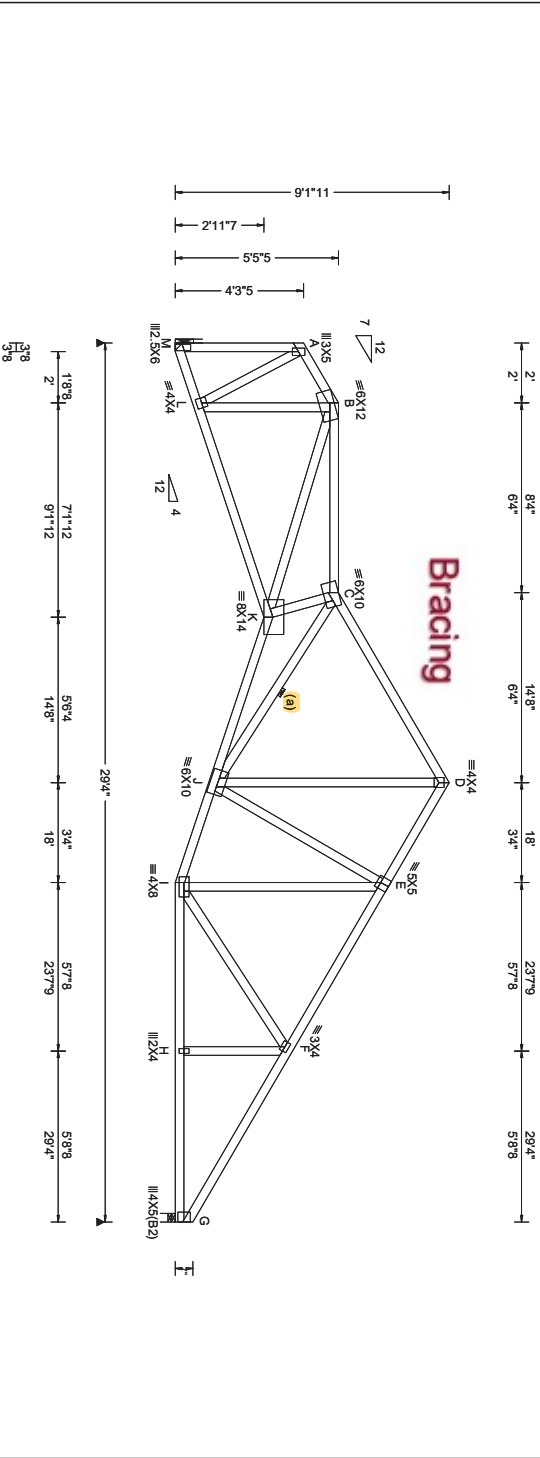
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SECTN: 409483 / FROM: CDM	SPEC Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: D12	Cust: R 215 JRef: 1XN32150003 T37 Dwnbnc: 041.23.0912.20380 / WHK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCRCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpl: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(LL): 0.208 K 999 240 VERT(CL): 0.435 K 807 180 HORZ(LL): 0.129 G - - HORZ(TL): 0.270 G - - Creep Factor: 2.0 Max TC CSI: 0.333 Max BC CSI: 0.428 Max Web CSI: 0.497	Gravity / R- / Rh / Rw / U / RL Loc R+ / R- / Rh / Rw / U / RL M 1232 /- /- /638 /63 /220 G 1227 /- /- /720 /16 /- Wind reactions based on MWFRS M Big Wld = - Min Req = - G Big Wld = 3.5 Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Wind	VIEW	Ver: 21.01.01A.0521.20	

Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP M-31;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0'

Bearing M: (0', 9'1"2) HUS28
 Supporting Member: (1)2x8 SP #2
 (22) 0.148"x3" nails into supporting member.
 (4) 0.148"x3" nails into supported member.

Purlins

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Maximum Web Forces Per Ply (lbs)	Maximum Bot Chord Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.	Chords Tens.Comp. Chords Tens. Comp.

A - M	347 -1266	C - K	724	0
A - L	1105 -328	C - J	756	-3217
B - L	394 -960	J - D	1068	-293
B - K	3258 -784	I - F	153	-393

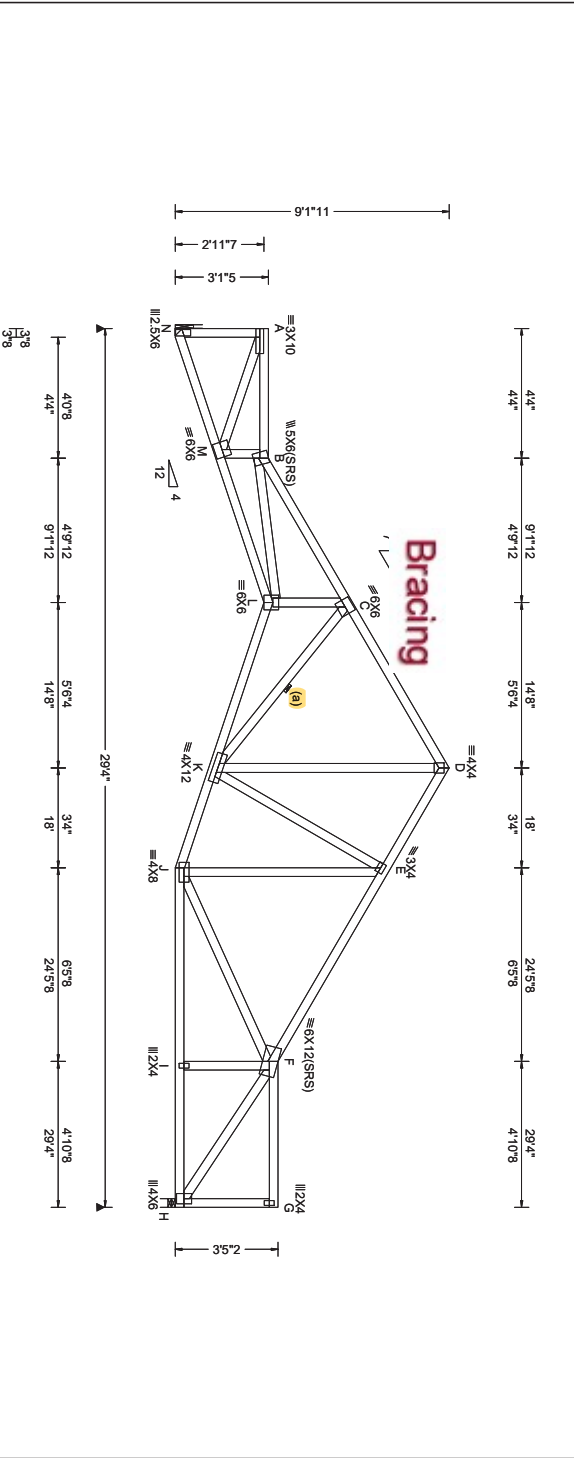
L - K	611 -104	I - H	1569	-261
K - J	4141 -810	H - G	1590	-280
J - I	1342 -168			

A - B	192 -667	D - E	431	-1431
B - C	958 -3679	E - F	408	-1550
C - D	414 -1491	F - G	408	-1943

A - B	192 -667	D - E	431	-1431
B - C	958 -3679	E - F	408	-1550
C - D	414 -1491	F - G	408	-1943



6750 Forum Drive
 Suite 305
 Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.22 ft TCCL: 5.0 psf BCDL: 5.0 psf C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.161 L 999 240 VERT(CL): 0.338 L 999 180 HORZ(L): 0.106 H - - HORZ(T/L): 0.222 H - - Creep Factor: 2.0 Max TC CSI: 0.189 Max BC CSI: 0.356 Max Web CSI: 0.592	Loc R+ /R- /Rh /Rw /U /RL N 1233 /- /- /652 /29 /155 H 1226 /- /- /643 /43 /- Wind reactions based on MWFRS N Big Wld = - Min Req = - H Big Wld = 3.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Wind	VIEW	Ver: 21.01.01A.0521.20	A - B 706 -2826 D - E 426 -1441 B - C 785 -3515 E - F 388 -1577 C - D 406 -1475

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
Bearing at location x=0' uses the following support conditions: 0'
Bearing N (0', 9'1"2) HUS28
Supporting Member: (1)2x8 SP #2
(22) 0.148"x3" nails into supporting member.
(4) 0.148"x3" nails into supported member.

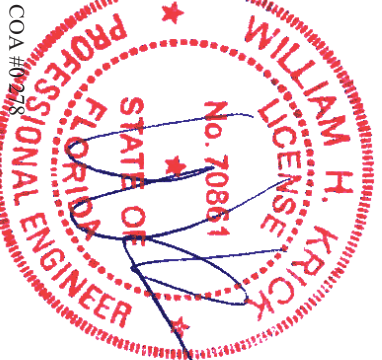
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Deflection
Max JT VERT DEFL: L.L. 0.16" D.L. 0.18". See detail DEFLCMB1014 for camber recommendations.
Provide for adequate drainage of roof.

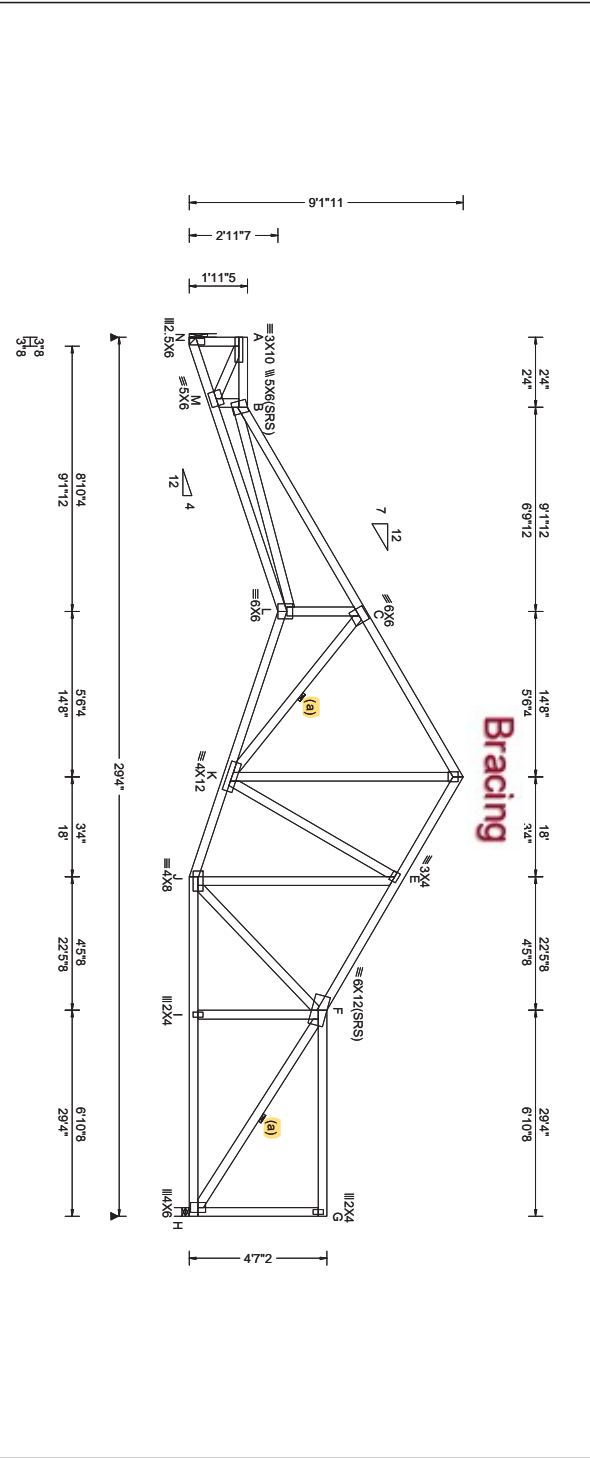
Maximum Web Forces Per Ply (lbs)	Tens.Comp.	Webs	Tens. Comp.
A - N	380 -1188	C - K	566 -2249
A - M	3009 -749	K - D	1136 -306
M - B	596 -2050	J - F	208 -430
L - C	2126 -415	F - H	508 -1971

Maximum Bot Chord Forces Per Ply (lbs)	Chords Tens.Comp.	Chords Tens. Comp.	
M - L	3337 -865	J - I	1663 -436
L - K	3097 -660	I - H	1688 -433
K - J	1355 -268		



Florida Certificate of Product Approval #FL 1999

SECON: 409486 / FROM: CDM	SPEC	Ply: 1 Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: D15	Cust R 215 JRef: 1XN32150003 T40 Dwnbnc: 041.23.0912.20654 / WHK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.158 L 999 240 VERT(CL): 0.330 L 999 180 HORZ(L): 0.107 H - - HORZ(T/L): 0.224 H - - Creep Factor: 2.0 Max TC CSI: 0.383 Max BC CSI: 0.372 Max Web CSI: 0.330	Loc R+ / R- / Rh / Rw / U / RL N 1233 /- /- /684 /9 /185 H 1226 /- /- /643 /84 /- Wind reactions based on MWFRS N Big Wld = - Min Req = - H Big Wld = 3.5 Bearing H is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Wind	VIEW	Ver: 21.01.01A.0521.20	

Lumber
 Top chord: 2x4 SP M-31;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP M-31;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
 Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0" uses the following support conditions: 0'
 Bearing N (0', 9'1"2) HUS28
 Supporting Member: (1)2x8 SP #2
 (22) 0.148"x3" nails into supporting member.
 (4) 0.148"x3" nails into supported member.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

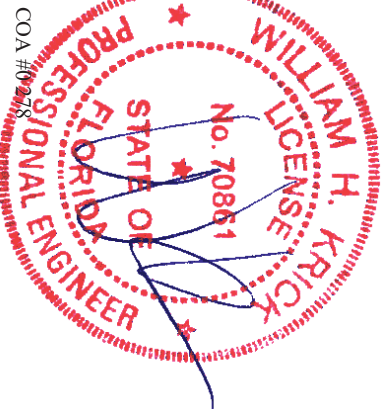
****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to Job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANS/ITP 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing the drawing number, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITP 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com, TPI: tpist.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

Florida State of Product Approval #FL 1999



A - B	580 -2398	D - E	427 -1422
B - C	829 -3597	E - F	427 -1537
C - D	425 -1482		

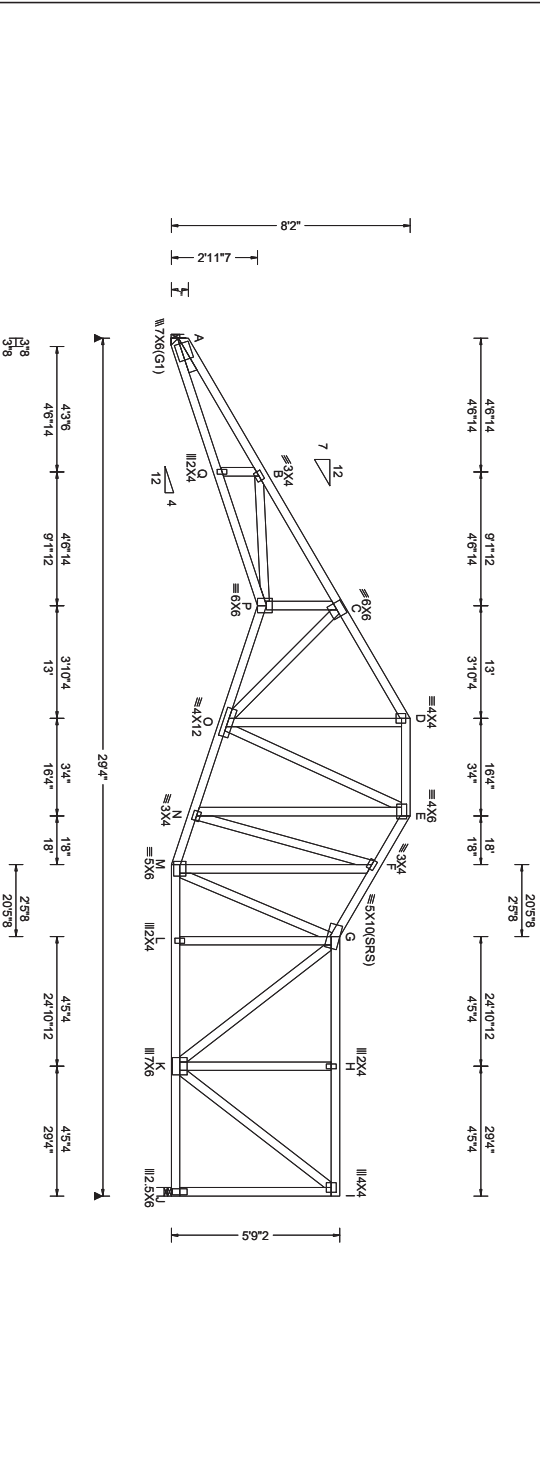
Maximum Bot Chord Forces Per Ply (lbs)	
Chords Tens.Comp. Chords Tens. Comp.	

M - L	3068 -878	J - I	1570 -462
L - K	3143 -763	I - H	1574 -460
K - J	1334 -315		

Maximum Web Forces Per Ply (lbs)	
Tens.Comp. Webs Tens. Comp.	

A - N	319 -1215	C - K	621 -2304
A - M	2677 -646	K - D	1116 -306
M - B	627 -2122	J - F	237 -442
L - C	2111 -427	F - H	542 -1856

SECTN: 409487 / FROM: CDM	SPEC Qty: 1	Job Number: 23-8926 Sunset 7 Truss Label: D16	Cust R 215 JRef: 1XN32150003 T28 DwnNo: 041.23.0912.2 1666 / WHK 02/10/2023
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCLL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.163 P 999 240 VERT(CL): 0.341 P 999 180 HORZ(L): 0.106 K - - HORZ(TL): 0.222 K - - Creep Factor: 2.0 Max TC CSI: 0.228 Max BC CSI: 0.426 Max Web CSI: 0.651	Gravity / Rh / Rw / U / RL Loc R+ / R- J 1233 /- /- /728 /183 /195 J 1226 /- /- /647 /231 /- Wind reactions based on MWFRS A Big Wld = - Min Req = - J Big Wld = 3.5 Bearing J is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Wind	VIEW Ver: 21.01.01A.0521.20		

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;
Lt Stud Wedge: 2x4 SP M-31;

Hangers / Ties

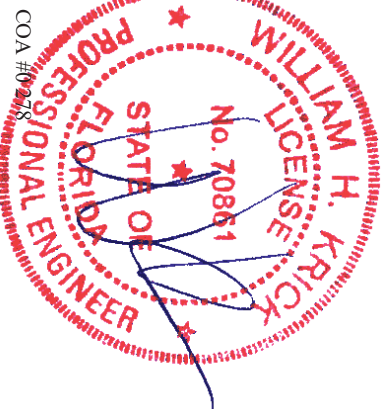
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bearing at location x=0' uses the following support conditions: 0'
Bearing A (0', 91'2) HUS28
Supporting Member: (1) 2x8 SP #2
(22) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

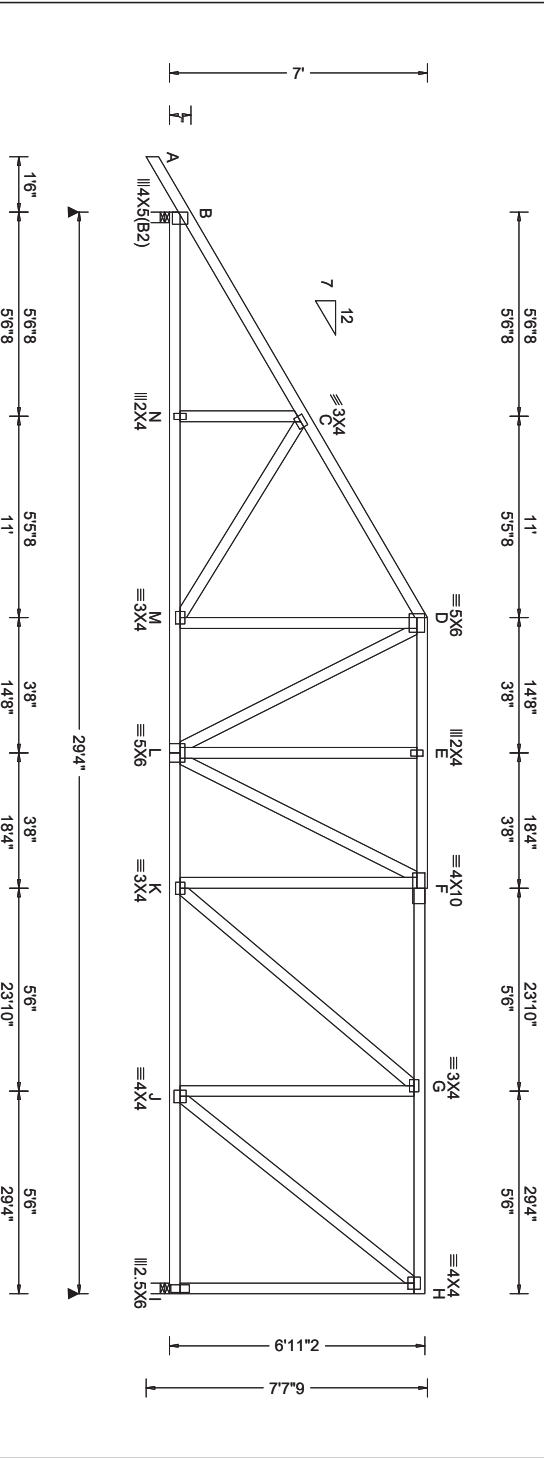
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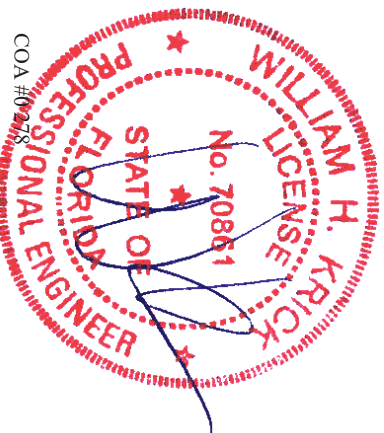
For more information see these web sites: Alpine: alpineitw.com, TPI: tpist.org, SBICA: sbicacomponents.com, ICC: iccsafe.org, AWC: awc.org

Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
A - Q	3269 -1129	N - M	1321 -482
Q - P	3317 -1147	M - L	1402 -541
P - O	3071 -1054	L - K	1404 -540
O - N	1305 -465		
Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
P - C	2110 -652	M - G	211 -377
C - O	715 -2104	G - K	263 -836
D - O	615 -171	K - I	1390 -593
O - E	484 -137	I - J	562 -1189

SECON: 409488 /	SPEC	Ply: 1	Job Number: 23-8926	Cust R 215	JRef: 1XN32150003	T41
FROM: CDM		Qty: 1	Sunset 7			
			Truss Label: D17			
				DwnNo: 041.23.0912.20046		
				/ WHK	02/10/2023	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCL: 10.00 Sofft: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Exposure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.056 M 999 240 VERT(CL): 0.119 M 999 180 HORZ(L): 0.021 J - - HORZ(TL): 0.044 J - - Creep Factor: 2.0 Max TC CSI: 0.246 Max BC CSI: 0.351 Max Web CSI: 0.522	Gravity / Rh / Rw / U / RL Loc R+ / R- / Rh / Rw / U / RL B 1326 /- /- /813 /137 /251 I 1215 /- /- /631 /- /- Wind reactions based on MWFRS B Big Wid = 3.5 Min Req = 1.5 I Big Wid = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;		VIEW Ver: 21.01.01A.0521.20	B - C 528 -1910 E - F 568 -1333 C - D 549 -1552 F - G 510 -1268 D - E 557 -1333 G - H 362 -833 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Purlins	In lieu of structural panels use purlins to brace all flat TC @ 24" oc.			B - N 1557 -615 L - K 1267 -509 N - M 1556 -616 K - J 873 -365 M - L 1266 -517 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp.
Wind	Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.			F - K 336 -362 J - H 1326 -577 K - G 628 -372 H - I 583 -1173 G - J 514 -907



COA #00278
 Florida State Seal of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

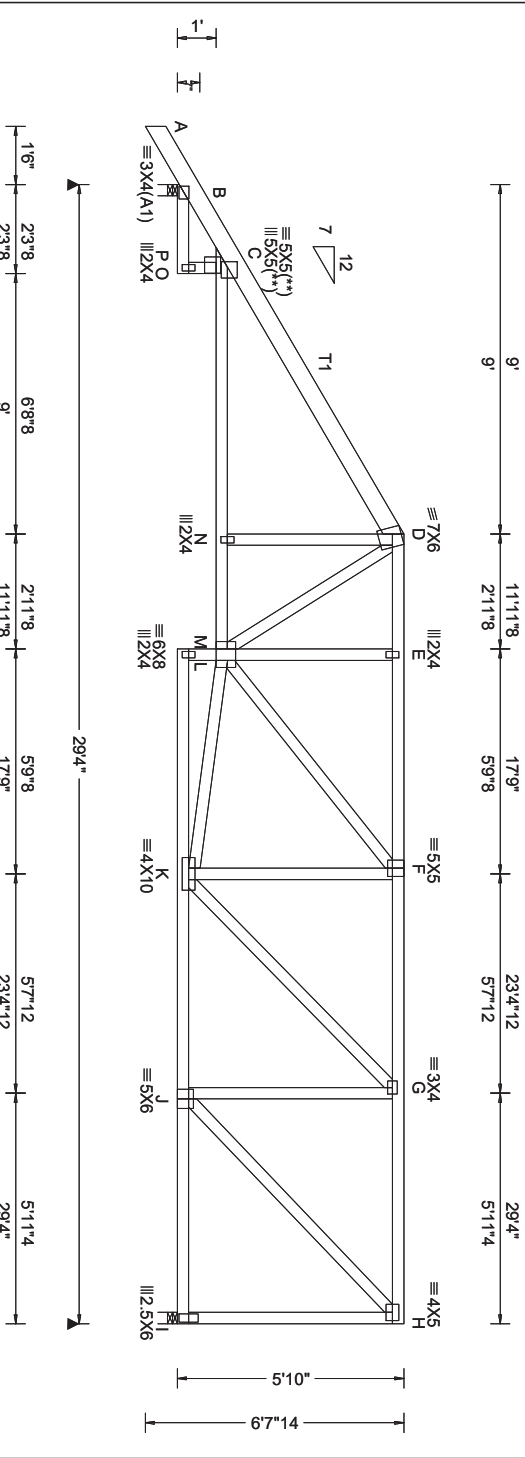
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Alpine, a division of ITW Building Components Group Inc, shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANS/ITP 11 or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANS/ITP 1 Sec.2.

For more information see these web sites: Alpine: alpinetw.com, TPI: tpinst.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

6750 Forum Drive
 Suite 305
 Orlando FL, 32821

SECTN: 409489 /	HIPM	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T7
FROM: CDM		Qty: 1	Sunset 7	DwnNo: 041.23.091222164		
			Truss Label: D18	/ WHK	02/10/2023	

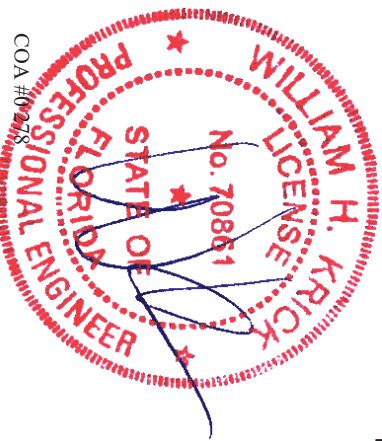


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _r in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(L): 0.216 O 999 240 VERT(CL): 0.444 O 788 180 HORZ(L): 0.163 J - - HORZ(TL): 0.336 J - - Creep Factor: 2.0 Max TC CSI: 0.604 Max BC CSI: 0.300 Max Web CSI: 0.487	Gravity / Rh / Rw / U / RL Loc R+ / R- / Rh / Rw / U / RL B 1331 /- /- /800 /220 /209 I 1210 /- /- /623 /240 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.5 I Big Wld = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; T1 2x6 SP 2400F-2.0E; Bot chord: 2x4 SP M-31; Webs: 2x4 SP M-31;		VIEW Ver: 21.01.01A.0521.20	B - C 218 -847 E - F 1014 -1939 C - D 865 2019 F - G 818 -1568 D - E 1021 -1962 G - H 560 -1053 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Plating Notes
 (***) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

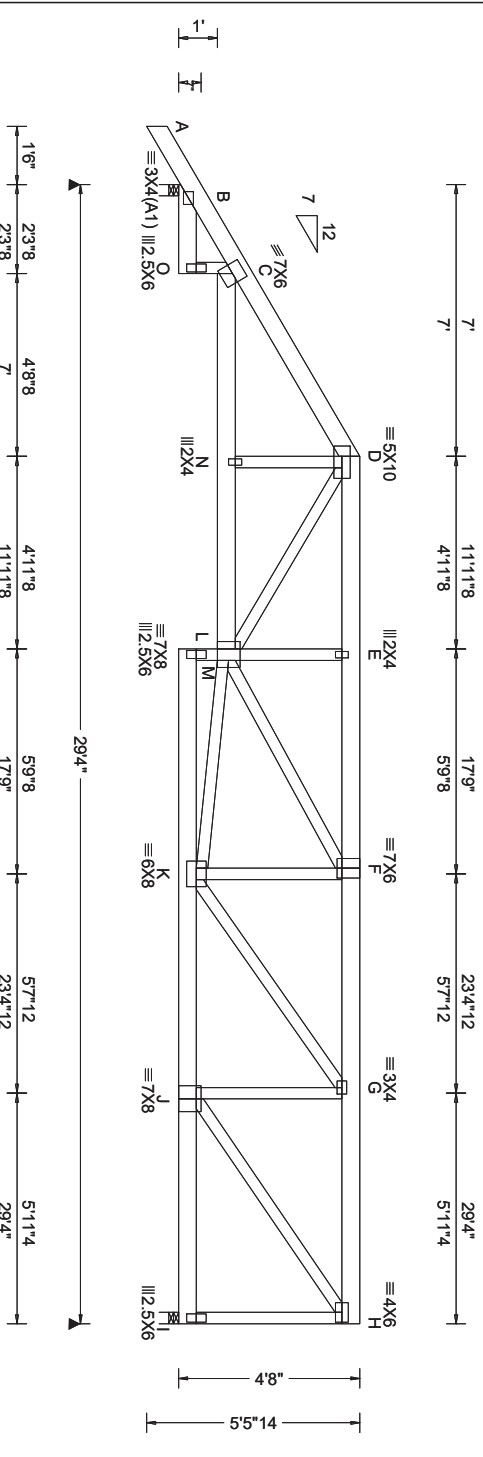


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 Florida State Seal of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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 For more information see these web sites: Alpine: alpinetw.com, TPI: tpinst.org, SBCA: sbcacomponents.com, ICC: iccsafe.org, AWC: awc.org

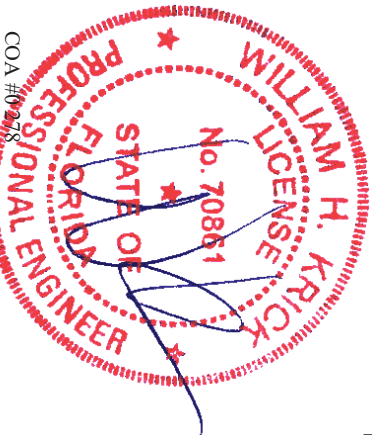
SECTN: 409503 /	HIP M	Ply: 2	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T8
FROM: CDM		Qty: 1	Sunset 7		DwnNbr: 041.23.0912.21350	
			Truss Label: D19		/ WHK	02/10/2023

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg/Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 11.28 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.226 N 999 240 VERT(CL): 0.472 N 741 180 HORZ(L): 0.172 J - - HORZ(TL): 0.359 J - - Creep Factor: 2.0 Max TC CSI: 0.935 Max BC CSI: 0.264 Max Web CSI: 0.209	Gravity Loc R+ /R- /Rh /Rw /U /RL B 2504 /- /- /- /424 /- I 2605 /- /- /- /416 /- Wind reactions based on MWFRS B Big Wld = 3.5 Min Req = 1.5 I Big Wld = 3.5 Min Req = 1.5 Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
Lumber	Top chord: 2x6 SP 2400F-2.0E; Bot chord: 2x6 SP 2400F-2.0E; Webs: 2x4 SP M-31;		VIEW Ver: 21.01.01A.0521.20	B - C 112 - 681 E - F 490 - 3019 C - D 421 - 2595 F - G 364 - 2229 D - E 493 - 3042 G - H 237 - 1490 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Nailnote Nail Schedule: 0.131"x3", min. nails Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.	Loading #1 hip supports 7-0-0 jacks W/2 panel TC and no end vert.
Purlins In lieu of structural panels use purlins to brace all flat TC @ 24" oc.	
Wind Wind loads and reactions based on MWFRS. Right end vertical not exposed to wind pressure. Wind loading based on both gable and hip roof types.	



COA #0278
Florida State Seal of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

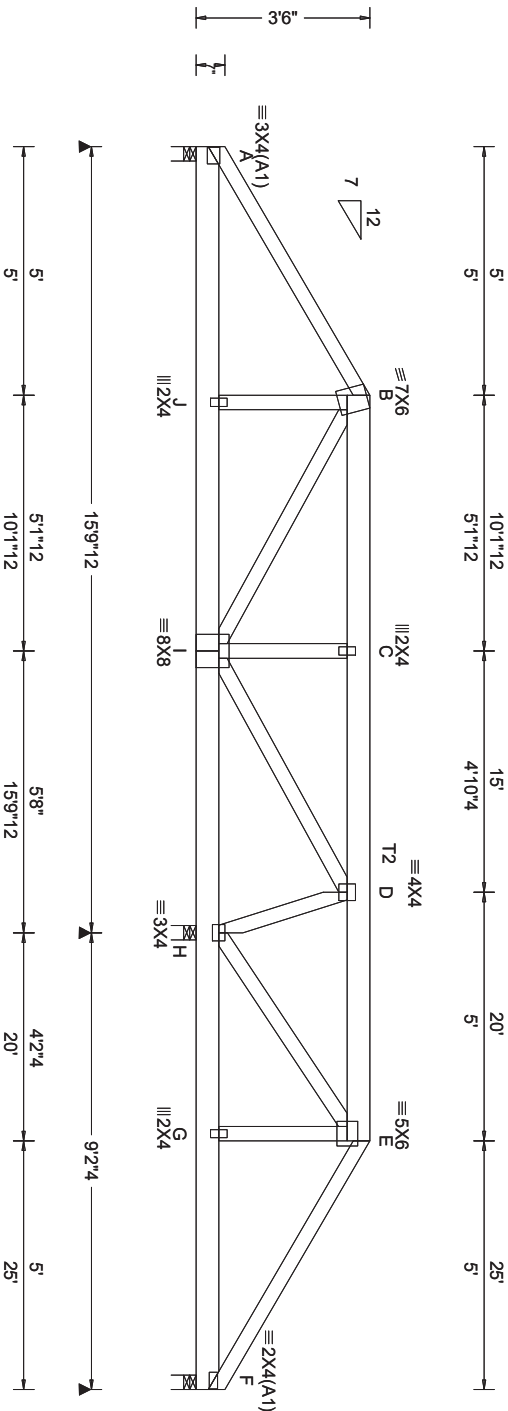
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

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SECTN: 409504 /	HIPS	Ply: 1	Job Number: 23-8926	Cust R 215	JRef: 1XN32150003	T23
FROM: CDM		Qty: 1	Sunset 7		DwnNo: 041.23.0912.19090	
			Truss Label: G01		/ WHK	02/10/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 11.14 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.017 C 999 240 VERT(CL): 0.036 C 999 180 HORZ(LL): 0.004 B - - HORZ(TL): 0.009 B - - Creep Factor: 2.0 Max TC CSI: 0.147 Max BC CSI: 0.115 Max Web CSI: 0.228	Loc R+ / R- / Rh / Rw / U / RL A 879 -/- /- /- /143 /- H 2022 -/- /- /- /304 /- F 341 -/- /- /- /83 /- Wind reactions based on MWFRS A Big Wid = 3.5 Min Req = 1.5 H Big Wid = 3.5 Min Req = 1.5 F Big Wid = 3.5 Min Req = 1.5 Bearings A, H, & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp. Chords Tens. Comp.
Lumber	Top chord: 2x4 SP M-31; T2 2x6 SP 2400F-2.0E; Bot chord: 2x6 SP 2400F-2.0E; Webs: 2x4 SP M-31;			

Loading

#1 hip supports 5-0-0 jacks with no webs.

Left side jacks have 5-0-0 setback with 0-0-0 cant and 1-6-0 overhang. End jacks have 5-0-0 setback with 0-0-0 cant and 1-6-0 overhang. Right side jacks have 5-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.



COA #0218
 Florida State Seal of Product Approval #FL 1999

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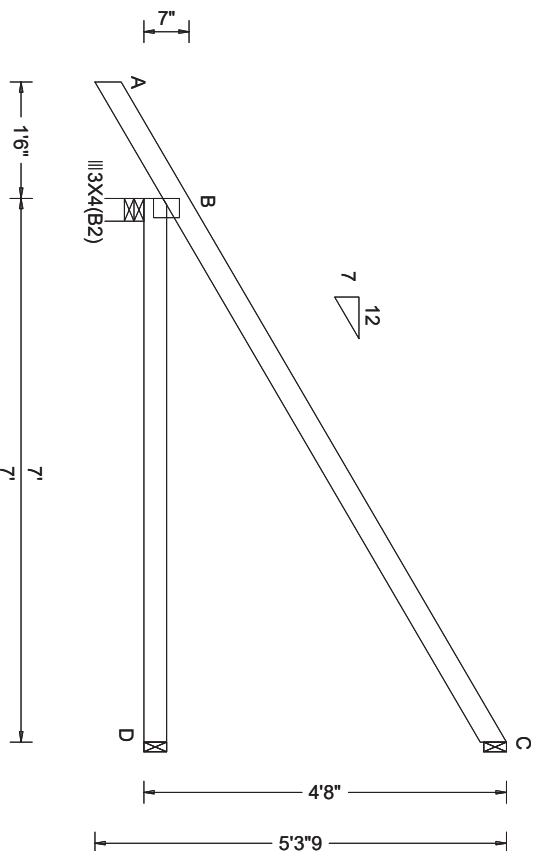
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▲ Maximum Reactions (lbs)	Gravity	/ Rh	/ Rw	/ U	/ RL
Loc R+ / R- / Rh / Rw / U / RL					
A 879 -/- /- /- /143 /-					
H 2022 -/- /- /- /304 /-					
F 341 -/- /- /- /83 /-					
Wind reactions based on MWFRS					
A Big Wid = 3.5 Min Req = 1.5					
H Big Wid = 3.5 Min Req = 1.5					
F Big Wid = 3.5 Min Req = 1.5					
Bearings A, H, & F are a rigid surface.					
Members not listed have forces less than 375#					
Maximum Top Chord Forces Per Ply (lbs)					
Chords Tens Comp. Chords Tens. Comp.					
A - B 227 -1315 C - D 162 -981					
B - C 162 -982 D - E 513 -74					
Maximum Bot Chord Forces Per Ply (lbs)					
Chords Tens Comp. Chords Tens. Comp.					
A - J 1073 -173 J - I 1087 -171					
Maximum Web Forces Per Ply (lbs)					
Webs Tens Comp. Webs Tens. Comp.					
C - I 187 -484 D - H 314 -1330					
I - D 1356 -182 H - E 137 -906					

SECN: 409490 /	EJAC	Ply: 1	Job Number: 23-8926	Cust: R 215	JRef: 1XN32150003	T9
FROM: CDM		Qty: 19	Sunset 7 Truss Label: J03	DwnNo: 041.23.0912.20622	/ WHK	02/10/2023



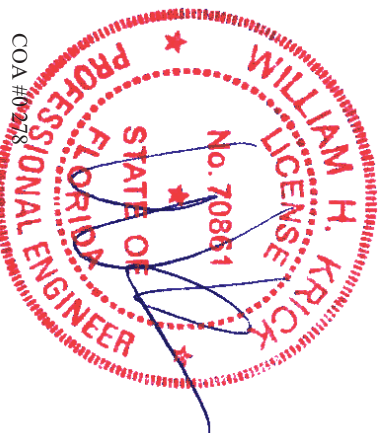
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCCL: 10.00 BCCL: 0.00 BCCL: 10.00 Des Ld: 40.00 NCBCCL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCCL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.009 B - - HORZ(TL): 0.018 B - - Creep Factor: 2.0 Max TC CSI: 0.314 Max BC CSI: 0.216 Max Web CSI: 0.000	Gravity / Rh / Rw / U / RL Loc R+ / R- B 406 /- /- /273 /30 /168 D 135 /- /- /73 /- /- C 200 /- /- /130 /113 /- Wind reactions based on MWFRS B Brg Wld = 3.5 Min Req = 1.5 D Brg Wld = 1.5 C Brg Wld = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#
Lumber	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



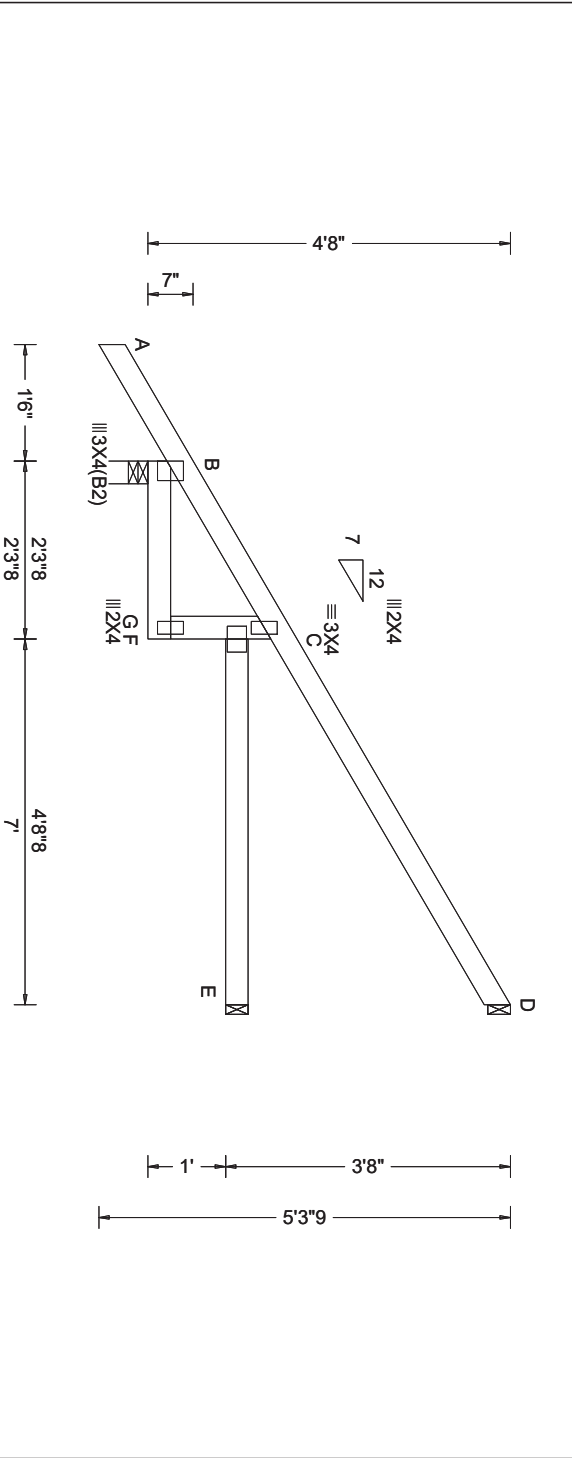
COA #09278
Certificate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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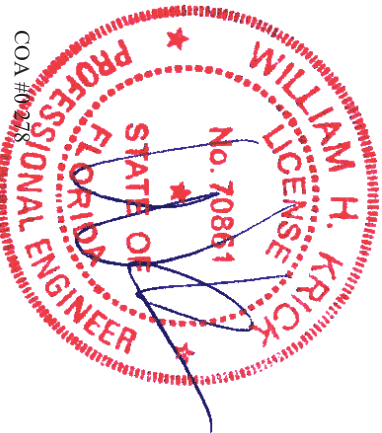
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)
TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Softt: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s):	PP Deflection in loc L/def L/# VERT(L): 0.140 F 596 240 VERT(CL): 0.282 F 296 180 HORZ(L): 0.093 C - - HORZ(TL): 0.186 C - - Creep Factor: 2.0 Max TC CSI: 0.373 Max BC CSI: 0.136 Max Web CSI: 0.067	Loc R+ / R- / Rh / Rw / U / RL B 406 /- /- /274 /31 /169 E 107 /- /- /61 /- /- D 210 /- /- /142 /104 /- Wind reactions based on MWFRS B Brg Wld = 3.5 Min Req = 1.5 E Brg Wld = 1.5 D Brg Wld = 1.5 Bearing B is a rigid surface. Members not listed have forces less than 375#
Lumber	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20	

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



Florida State Seal of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!

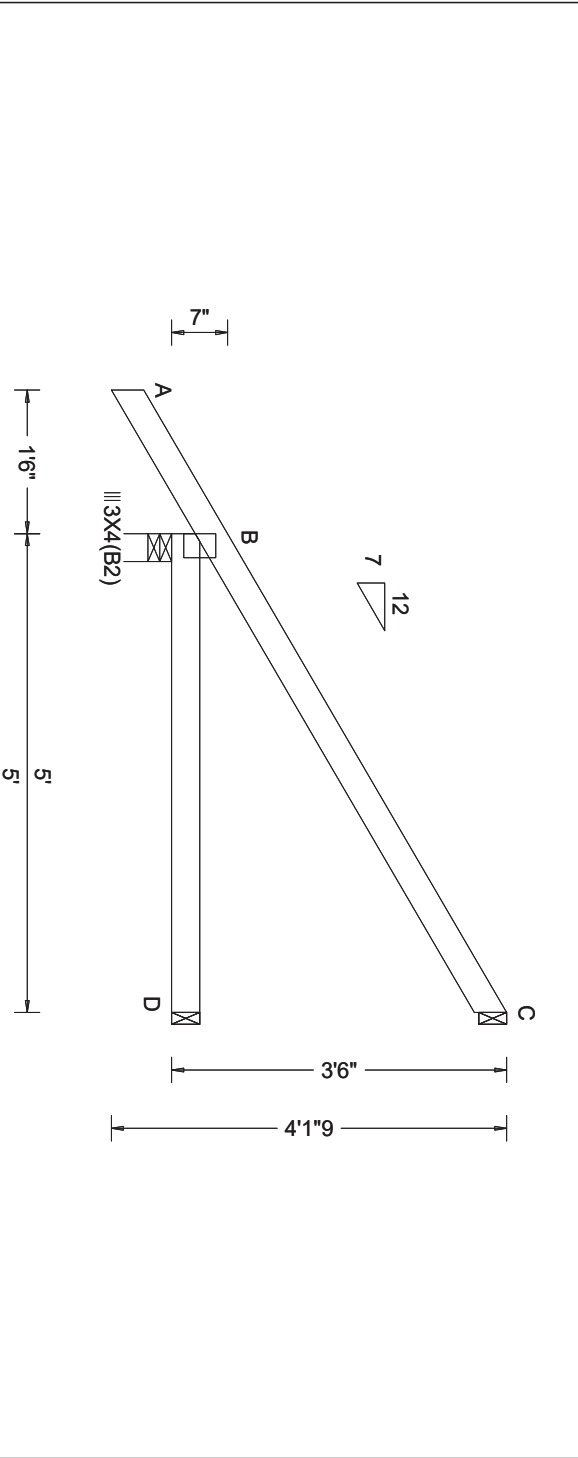
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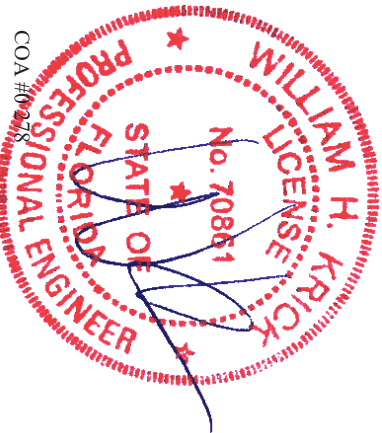
ALPINE
ANITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

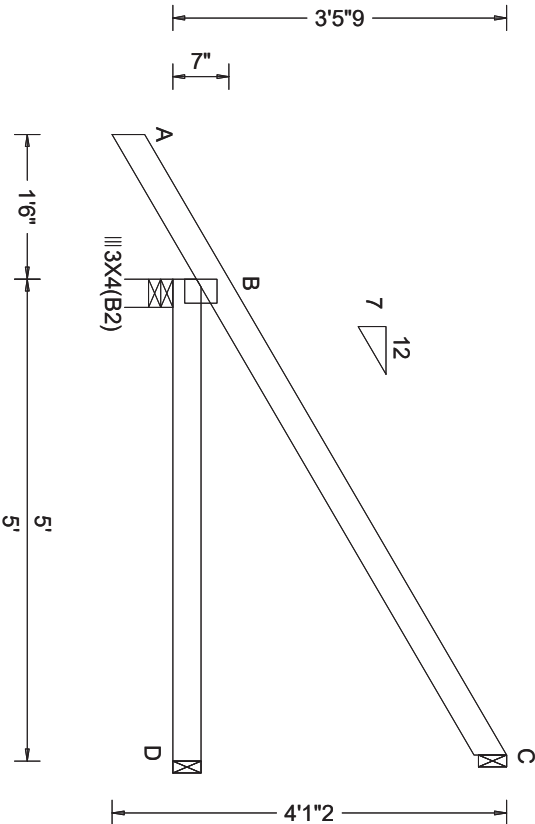


Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg P7 in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def L/#	Gravity	/ Rh	Non-Gravity	
TCDL:	10.00	Speed:	130 mph	Pf: NA			VERT(LL): NA	Loc R+ / R-	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL): NA	B 327	- / -	- / -	1225 / 29
BCDL:	10.00	Risk Category:	II	Snow Duration: NA			HORZ(LL): -0.003 C	D 95	- / -	- / -	151 / -
Des Ld:	40.00	EXP: C	Kzt: NA				HORZ(TL): 0.005 B	C 138	- / -	- / -	189 / 81
NCBCLL:	10.00	Mean Height:	15.00 ft	Building Code:			Creep Factor: 2.0	Wind reactions based on MWFRS			
Softt:	2.00	TCDL:	5.0 psf	FBC 7th Ed. 2020 Res.				B	Brig Wid = 3.5	Min Req = 1.5	
Load Duration:	1.25	BCDL:	5.0 psf	TPI Std: 2014				D	Brig Wid = 1.5		
		MWFRS Parallel Dist:	0 to h/2	Rep Fac: Yes				C	Brig Wid = 1.5		
		C&C Dist a:	3.00 ft	FT/RT: 20(0)/10(0)				Bearing B is a rigid surface.			
		Loc. from endwall:	not in 4.50 ft	Plate Type(s):				Members not listed have forces less than 375#			
		GCpl:	0.18								
		Wind Duration:	1.60	VIEW Ver:	21.01.01A.0521.20						

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.





Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg P _f in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)			
TCCL: 20.00	Wind Std: ASCE 7-16	Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpf: 0.18 Wind Duration: 1.60	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	Loc L/def	L/#
TCDL: 10.00			Pf: NA	Ce: NA		VERT(LL):	NA	
BCLL: 0.00			Lu: NA	Cs: NA		VERT(CL):	NA	
BCDL: 10.00			Snow Duration: NA			HORZ(LL):	-0.003	C - - -
Des Ld: 40.00			Building Code: FBC 7th Ed. 2020 Res.			HORZ(TL):	0.005	B - - -
NCBCLL: 10.00			TPI Std: 2014			Creep Factor:	2.0	
Sofft: 2.00			Rep Fac: Yes			Max TC CSI:	0.155	
Load Duration: 1.25			FT/RT:20(0)/10(0)			Max BC CSI:	0.104	
Spacing: 24.0 "			Plate Type(s):			Max Web CSI:	0.000	
			WAVE			VIEW Ver: 21.01.01A.0521.20		

▲ Maximum Reactions (lbs)			
Loc	R+ / R-	/ Rh	Non-Gravity / Rw / U / RL
B 326	- / -	- / -	1225 / 29 / 126
C 136	- / -	- / -	187 / 80 / -
D 95	- / -	- / -	151 / - / -

Wind reactions based on MWFRS

B Big Wid = 3.5 Min Req = 1.5

C Big Wid = 1.5

D Big Wid = 1.5

Bearing B is a rigid surface.

Members not listed have forces less than 375#

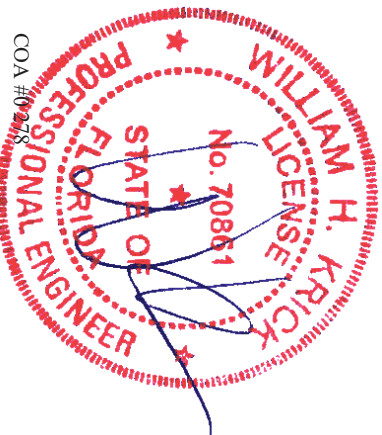
Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0278

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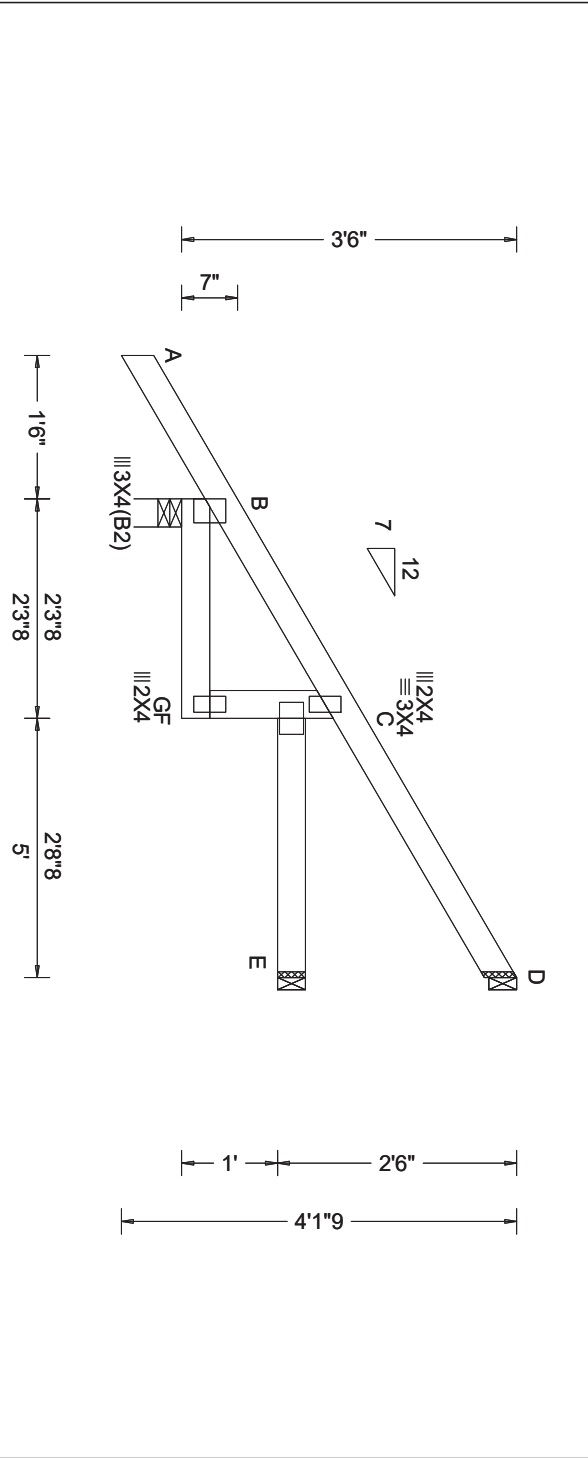
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Loading Criteria (psf)		Snow Criteria (Pg P _f in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def	L/def	L/#	
TCDL:	10.00	Speed: 130 mph	Pf: NA	Ce: NA		VERT(L):	0.041 F	999	240
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		VERT(CL):	0.082 F	726	180
BCDL:	10.00	Risk Category: II	Snow Duration: NA			HORZ(L):	0.027 C	-	-
Des Ld:	40.00	EXP: C Kzt: NA				HORZ(TL):	0.054 C	-	-
NCRCLL:	10.00	Mean Height: 15.00 ft	Building Code:			Creep Factor:	2.0		
Sofft:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.			Max TC CSI:	0.173		
Load Duration: 1.25		BCDL: 5.0 psf	TPI Std: 2014			Max BC CSI:	0.047		
		MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes			Max Web CSI:	0.030		
		C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)						
		Loc. from endwall: not in 4.50 ft	Plate Type(s):						
		GCpl: 0.18							
		Wind Duration: 1.60	WAVE						

▲ Maximum Reactions (lbs)				Non-Gravity			
Loc	R+ / R-	Gravity	/ Rh	/ Rw	/ U	/ RL	
B	327	-/-	-	1226	130	1127	
E	65	-/-	-	137	-	-/-	
D	149	-/-	-	1102	170	-/-	
Wind reactions based on MWFRS							
B	Brig Wid = 3.5 Min Req = 1.5						
E	Brig Wid = 1.5						
D	Brig Wid = 1.5						
Bearing B is a rigid surface.							
Members not listed have forces less than 375#							

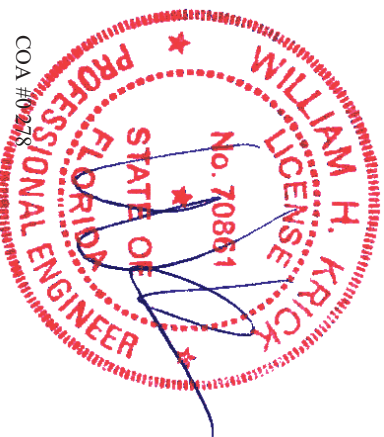
Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



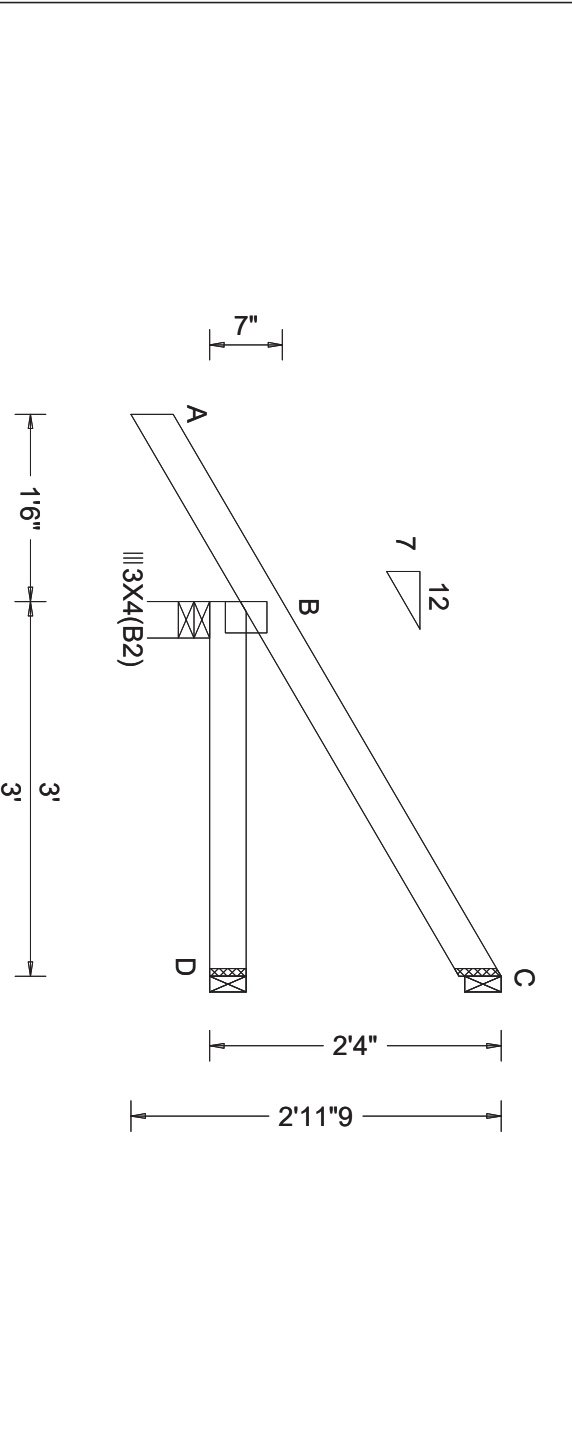
COA #0278
Florida State Board of Professional Engineers
Date of Product Approval #FL 1999

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****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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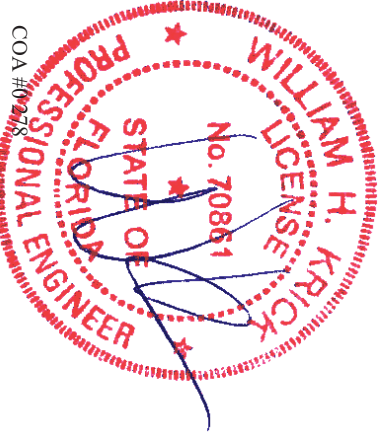
For more information see these web sites: Alpine: alpineitw.com; TPI: tpist.org; SBCCA: sbccacomponents.com; ICC: iccsafe.org; AWC: awc.org

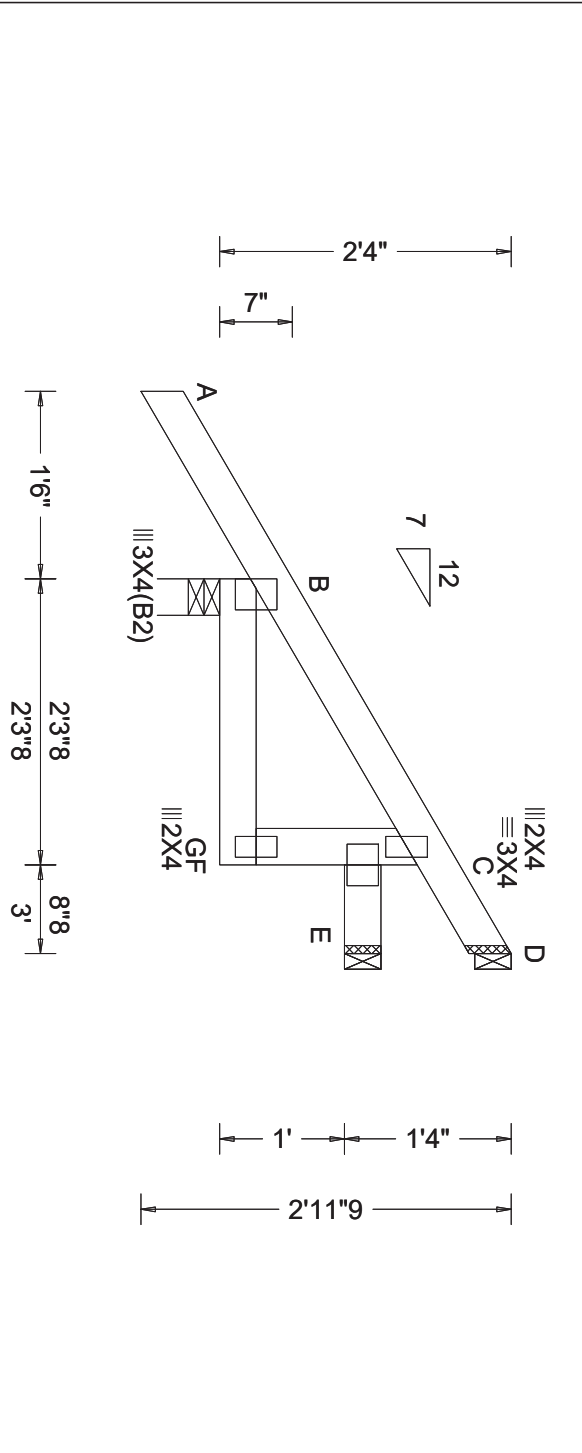


Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg P7 in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL:	20.00	Wind Std: ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def L/#	Loc	R+ / R-	/ Rh	Non-Gravity / Rw / U / RL
TCDL:	10.00	Speed: 130 mph	Pf: NA		Ce: NA	VERT(LL): NA	B 255	- / -	- / -	184 / 31 / 86
BCLL:	0.00	Enclosure: Closed	Lu: NA	Cs: NA		HORZ(LL): -0.001 C - -	D 56	- / -	- / -	131 / - / -
BCDL:	10.00	Risk Category: II	Snow Duration: NA			HORZ(TL): 0.001 C - -	C 69	- / -	142 / 46	- / -
Des Ld:	40.00	EXP: C Kzt: NA				Creep Factor: 2.0	Wind reactions based on MWFRS			
NCRCLL:	10.00	Mean Height: 15.00 ft	Building Code:				B	Brig Wid = 3.5	Min Req = 1.5	
Sofft:	2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.				D	Brig Wid = 1.5		
Load Duration: 1.25		BCDL: 5.0 psf	TPI Std: 2014				C	Brig Wid = 1.5		
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes				Bearing B is a rigid surface.			
		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)				Members not listed have forces less than 375#			
		Loc. from endwall: not in 4.50 ft	Plate Type(s):							
		GCpl: 0.18								
		Wind Duration: 1.60	WAVE							
							VIEW Ver: 21.01.01A.0521.20			

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

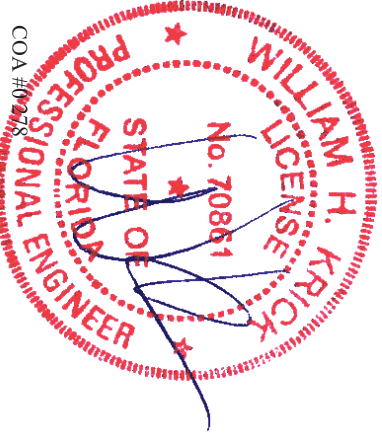




Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Def/CSI Criteria	▲ Maximum Reactions (lbs)					
TCCL: 20.00	Wind Std: ASCE 7-16	Wind Speed: 130 mph	Pg: NA Pf: NA Ce: NA	PP Deflection in loc L/def L/#	Loc	R+ / R-	Gravty / Rh	Non-Gravty / Rw / U / RL		
TCCL: 10.00	Speed: 130 mph	Enclosure: Closed	Lu: NA Cs: NA	VERT(L): 0.003 F 999 240	B 255	- / -	- / Rh	1/84	3/1	7/86
BCCL: 0.00	Risk Category: II	Risk Category: II	Snow Duration: NA	VERT(CL): 0.006 F 999 180	E 22	- / -	- / Rh	1/16	1/1	- / -
BCDL: 10.00	EXP: C Kzt: NA	EXP: C Kzt: NA		HORZ(L): -0.002 C - -	D 80	- / -	- / Rh	1/56	1/33	- / -
Des Ld: 40.00	Mean Height: 15.00 ft	Mean Height: 15.00 ft	Building Code: FBC 7th Ed. 2020 Res.	HORZ(TL): 0.003 C - -	Creep Factor: 2.0					
NCBCCL: 10.00	TCCL: 5.0 psf	TCCL: 5.0 psf	TPI Std: 2014		Max TC CSI: 0.072					
Soffit: 2.00	MWFRS Parallel Dist: 0 to h/2	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes		Max BC CSI: 0.018					
Load Duration: 1.25	C&C Dist a: 3.00 ft	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		Max Web CSI: 0.008					
Spacing: 24.0 "	Loc. from endwall: not in 4.50 ft	Loc. from endwall: not in 4.50 ft	Plate Type(s):		Bearing B is a rigid surface.					
	GCpf: 0.18	GCpf: 0.18			Members not listed have forces less than 375#					
	Wind Duration: 1.60	Wind Duration: 1.60	WAVE	VIEW Ver: 21.01.01A.0521.20						

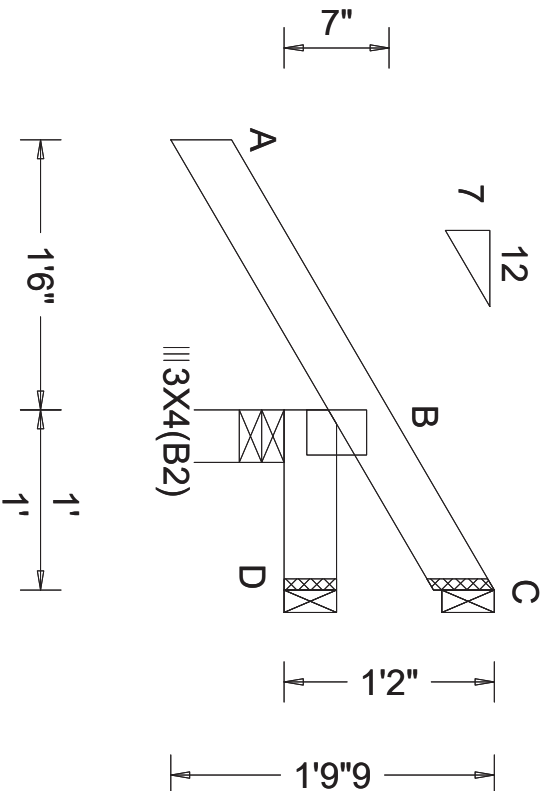
Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP M-31;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg P7 in PSF)		Def/CSI Criteria		▲ Maximum Reactions (lbs)			
TCCL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/def L/#	Loc R+	Gravity	/ Rh	Non-Gravity
TCDL:	10.00	Speed:	130 mph	Pf: NA			VERT(LL): NA		/ R-		/ Rw
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA	Ce: NA	VERT(CL): NA	B 229	/-	/-	/180 /51 /45
BCDL:	10.00	Risk Category:	II	Snow Duration: NA			HORZ(LL): -0.001 C	D 14	/-1	/-	/11 /2 /-
Des Ld:	40.00	EXP: C	Kzt: NA				HORZ(TL): 0.001 C	C -	/43	/-	/30 /44 /-
NCRCLL:	10.00	Mean Height:	15.00 ft	Building Code:			Creep Factor: 2.0	Wind reactions based on MWFRS			
Softt:	2.00	TCDL:	5.0 psf	FBC 7th Ed. 2020 Res.				B	Brig Wid = 3.5	Min Req = 1.5	
Load Duration:	1.25	BCDL:	5.0 psf	TPI Std: 2014				D	Brig Wid = 1.5		
		MWFRS Parallel Dist:	0 to h/2	Rep Fac: Yes				C	Brig Wid = 1.5		
		Loc. from endwall:	Any	FT/RT:20(0)/10(0)				Bearing B is a rigid surface.			
		GCpl:	0.18	Plate Type(s):				Members not listed have forces less than 375#			
		Wind Duration:	1.60	VIEW Ver:	21.01.01A.0521.20						

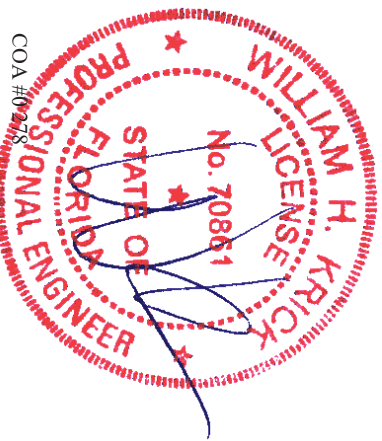
Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP M-31;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, $K_z t = 1.00$

Dr1	120 mph	wind	Speed, 15'	Mean Height,	Partially Enclosed,	Exposure C, Kzt = 1.00
Dr1	120 mph	wind	Speed, 15'	Mean Height,	Enclosed, Exposure D, Kzt = 1.00	
Dr1	100 mph	wind	Speed, 15'	Mean Height,	Partially Enclosed,	Exposure D, Kzt = 1.00

Max Gable Vertical Length													
Gable Vertical Spacing	E ₂₄ Vertical Species	Brace	No Braces	(1) 1x4 "L" Brace * (1) 2x4 "L" Brace * (2) 2x4 "L" Brace *** (1) 2x6 "L" Brace * (2) 2x6 "L" Brace ***									
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
24" o.c.	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	
			4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	
			Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"
		HF	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"
	DFL	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	
			Stud	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"
		Standard	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	
			#1 / #2	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"
		SPF	#3	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"
				Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"
16" o.c.	HF	Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	
			#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"
	SP	#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	
			#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"
	DFL	Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	
			Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"
	SPF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	
			Standard	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"
		HF	#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"
				#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"
12" o.c.	DFL	Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	
			Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch

#3		Southern Pine
Stud		Stud
Standard		Standard

Group B:

Hem-Fir	
#1 & Btr	
#1	

Douglas Fir-Larch

#1		Southern Pine
		Stud
#2		Standard

1x4 Braces shall be SRB (Stress-Rated Board).

Industrial 45 Stress-Rated Boards (Group B values may be used with these grades).

1x4 Braces shall be SRB (Stress-Rated Board).

Industrial 55 or Industrial 60 Stress-Rated Boards (Group B values may be used with these grades).

Gable Truss Detail Notes
Wind Load deflection criterion is $L/240$.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load), Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang

Attach 1" braces with 10d (0.128x3.00" min.) nails.

* For (1) 1" brace: space nails at 2' o.c.

In 18" end zones and 4' o.c. between zones.

* For (2) 1" braces: space nails at 3' o.c.

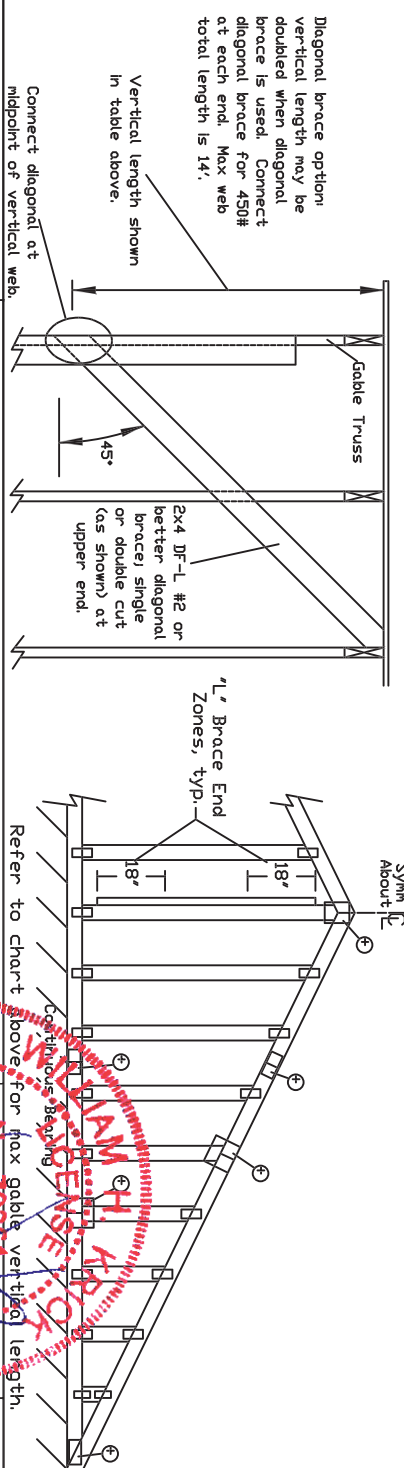
In 18" end zones and 6' o.c. between zones.

1" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



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Refer to chart above for max gable vertical length

3000

REF ASCE / -16- LAB14015

DATE 01/26/2018

DRWG A14015-ENC160118

MAX. TOT. LD. 60 PSF

APPENDIX A: SPLITTING



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

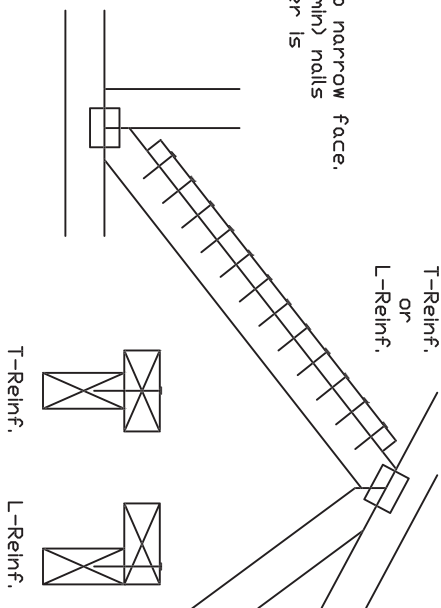
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

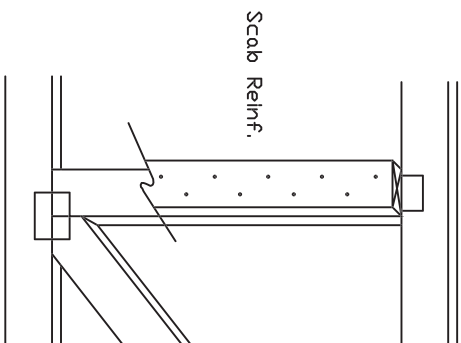
T-Reinforcement
or
L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.





155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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Tenases require adherence to fabricating, handling, shipping, installing, and bracing. Refer to the following sections of the BCSI Manual (TPI 11) for details: 1.1, 1.2, 1.3, 1.4, 1.5, 1.6, 1.7, 1.8, 1.9, 1.10, 1.11, 1.12, 1.13, 1.14, 1.15, 1.16, 1.17, 1.18, 1.19, 1.20, 1.21, 1.22, 1.23, 1.24, 1.25, 1.26, 1.27, 1.28, 1.29, 1.30, 1.31, 1.32, 1.33, 1.34, 1.35, 1.36, 1.37, 1.38, 1.39, 1.40, 1.41, 1.42, 1.43, 1.44, 1.45, 1.46, 1.47, 1.48, 1.49, 1.50, 1.51, 1.52, 1.53, 1.54, 1.55, 1.56, 1.57, 1.58, 1.59, 1.60, 1.61, 1.62, 1.63, 1.64, 1.65, 1.66, 1.67, 1.68, 1.69, 1.70, 1.71, 1.72, 1.73, 1.74, 1.75, 1.76, 1.77, 1.78, 1.79, 1.80, 1.81, 1.82, 1.83, 1.84, 1.85, 1.86, 1.87, 1.88, 1.89, 1.90, 1.91, 1.92, 1.93, 1.94, 1.95, 1.96, 1.97, 1.98, 1.99, 2.00, 2.01, 2.02, 2.03, 2.04, 2.05, 2.06, 2.07, 2.08, 2.09, 2.10, 2.11, 2.12, 2.13, 2.14, 2.15, 2.16, 2.17, 2.18, 2.19, 2.20, 2.21, 2.22, 2.23, 2.24, 2.25, 2.26, 2.27, 2.28, 2.29, 2.30, 2.31, 2.32, 2.33, 2.34, 2.35, 2.36, 2.37, 2.38, 2.39, 2.40, 2.41, 2.42, 2.43, 2.44, 2.45, 2.46, 2.47, 2.48, 2.49, 2.50, 2.51, 2.52, 2.53, 2.54, 2.55, 2.56, 2.57, 2.58, 2.59, 2.60, 2.61, 2.62, 2.63, 2.64, 2.65, 2.66, 2.67, 2.68, 2.69, 2.70, 2.71, 2.72, 2.73, 2.74, 2.75, 2.76, 2.77, 2.78, 2.79, 2.80, 2.81, 2.82, 2.83, 2.84, 2.85, 2.86, 2.87, 2.88, 2.89, 2.90, 2.91, 2.92, 2.93, 2.94, 2.95, 2.96, 2.97, 2.98, 2.99, 3.00, 3.01, 3.02, 3.03, 3.04, 3.05, 3.06, 3.07, 3.08, 3.09, 3.10, 3.11, 3.12, 3.13, 3.14, 3.15, 3.16, 3.17, 3.18, 3.19, 3.20, 3.21, 3.22, 3.23, 3.24, 3.25, 3.26, 3.27, 3.28, 3.29, 3.30, 3.31, 3.32, 3.33, 3.34, 3.35, 3.36, 3.37, 3.38, 3.39, 3.40, 3.41, 3.42, 3.43, 3.44, 3.45, 3.46, 3.47, 3.48, 3.49, 3.50, 3.51, 3.52, 3.53, 3.54, 3.55, 3.56, 3.57, 3.58, 3.59, 3.60, 3.61, 3.62, 3.63, 3.64, 3.65, 3.66, 3.67, 3.68, 3.69, 3.70, 3.71, 3.72, 3.73, 3.74, 3.75, 3.76, 3.77, 3.78, 3.79, 3.80, 3.81, 3.82, 3.83, 3.84, 3.85, 3.86, 3.87, 3.88, 3.89, 3.90, 3.91, 3.92, 3.93, 3.94, 3.95, 3.96, 3.97, 3.98, 3.99, 4.00, 4.01, 4.02, 4.03, 4.04, 4.05, 4.06, 4.07, 4.08, 4.09, 4.10, 4.11, 4.12, 4.13, 4.14, 4.15, 4.16, 4.17, 4.18, 4.19, 4.20, 4.21, 4.22, 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5.89, 5.90, 5.91, 5.92, 5.93, 5.94, 5.95, 5.96, 5.97, 5.98, 5.99, 6.00, 6.01, 6.02, 6.03, 6.04, 6.05, 6.06, 6.07, 6.08, 6.09, 6.10, 6.11, 6.12, 6.13, 6.14, 6.15, 6.16, 6.17, 6.18, 6.19, 6.20, 6.21, 6.22, 6.23, 6.24, 6.25, 6.26, 6.27, 6.28, 6.29, 6.30, 6.31, 6.32, 6.33, 6.34, 6.35, 6.36, 6.37, 6.38, 6.39, 6.40, 6.41, 6.42, 6.43, 6.44, 6.45, 6.46, 6.47, 6.48, 6.49, 6.50, 6.51, 6.52, 6.53, 6.54, 6.55, 6.56, 6.57, 6.58, 6.59, 6.60, 6.61, 6.62, 6.63, 6.64, 6.65, 6.66, 6.67, 6.68, 6.69, 6.70, 6.71, 6.72, 6.73, 6.74, 6.75, 6.76, 6.77, 6.78, 6.79, 6.80, 6.81, 6.82, 6.83, 6.84, 6.85, 6.86, 6.87, 6.88, 6.89, 6.90, 6.91, 6.92, 6.93, 6.94, 6.95, 6.96, 6.97, 6.98, 6.99, 7.00, 7.01, 7.02, 7.03, 7.04, 7.05, 7.06, 7.07, 7.08, 7.09, 7.10, 7.11, 7.12, 7.13, 7.14, 7.15, 7.16, 7.17, 7.18, 7.19, 7.20, 7.21, 7.22, 7.23, 7.24, 7.25, 7.26, 7.27, 7.28, 7.29, 7.30, 7.31, 7.32, 7.33, 7.34, 7.35, 7.36, 7.37, 7.38, 7.39, 7.40, 7.41, 7.42, 7.43, 7.44, 7.45, 7.46, 7.47, 7.48, 7.49, 7.50, 7.51, 7.52, 7.53, 7.54, 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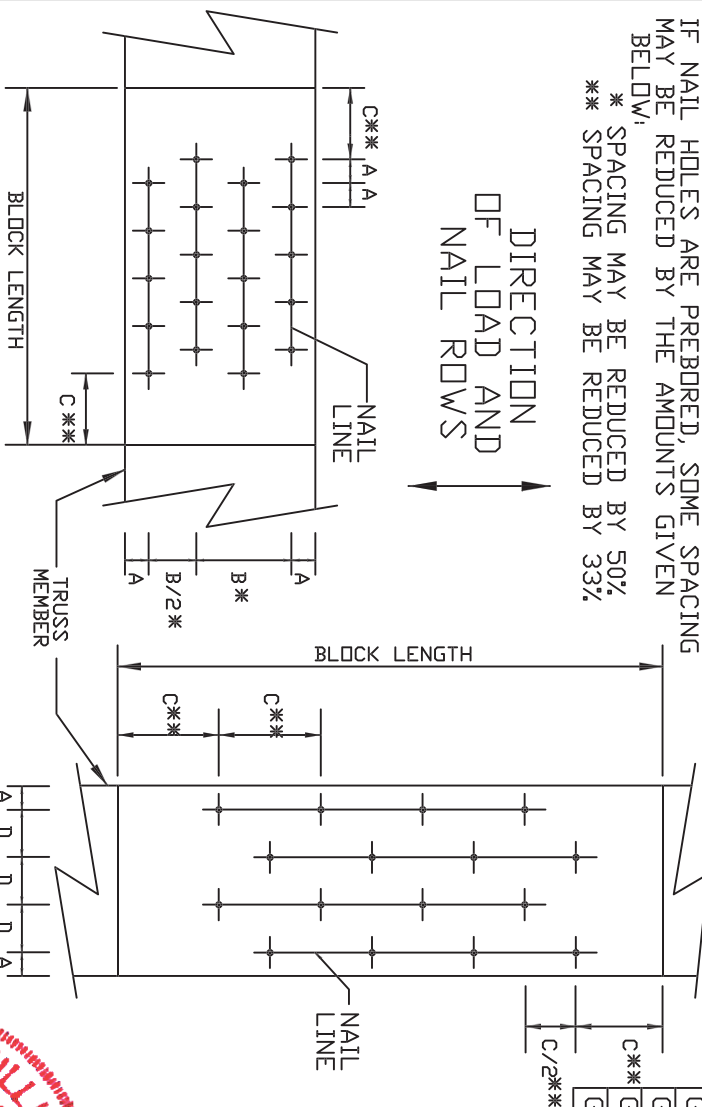
NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCE THIS DETAIL.

- LOAD PERPENDICULAR TO GRAIN
- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
 - B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
 - C - END DISTANCE (15 NAIL DIAMETERS)

- LOAD PARALLEL TO GRAIN
- A - EDGE DISTANCE (6 NAIL DIAMETERS)
 - C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
 - D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)
- IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
- * SPACING MAY BE REDUCED BY 50%
 - ** SPACING MAY BE REDUCED BY 33%



NAIL TYPE	MINIMUM NAIL SPACING DISTANCES			
	A	B*	C**	D
8d BDX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BDX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BDX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BDX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BDX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

No. 70861

STATE OF FLORIDA

PROFESSIONAL ENGINEER

02/10/2023

Florida Certificate of Product Approval #FL 1999

REF NAIL SPACE

DATE 10/01/14

DRWG CNNAILSP1014



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING. INSTALLERS MUST FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS. TRUSSES require adherence to fabricating, handling, shipping, installing, and bracing. Refer to the following sections of the AISC Steel Construction Manual (13th Edition) for details: 10.10.1, 10.10.2, 10.10.3, 10.10.4, 10.10.5, 10.10.6, 10.10.7, 10.10.8, 10.10.9, 10.10.10, 10.10.11, 10.10.12, 10.10.13, 10.10.14, 10.10.15, 10.10.16, 10.10.17, 10.10.18, 10.10.19, 10.10.20, 10.10.21, 10.10.22, 10.10.23, 10.10.24, 10.10.25, 10.10.26, 10.10.27, 10.10.28, 10.10.29, 10.10.30, 10.10.31, 10.10.32, 10.10.33, 10.10.34, 10.10.35, 10.10.36, 10.10.37, 10.10.38, 10.10.39, 10.10.40, 10.10.41, 10.10.42, 10.10.43, 10.10.44, 10.10.45, 10.10.46, 10.10.47, 10.10.48, 10.10.49, 10.10.50, 10.10.51, 10.10.52, 10.10.53, 10.10.54, 10.10.55, 10.10.56, 10.10.57, 10.10.58, 10.10.59, 10.10.60, 10.10.61, 10.10.62, 10.10.63, 10.10.64, 10.10.65, 10.10.66, 10.10.67, 10.10.68, 10.10.69, 10.10.70, 10.10.71, 10.10.72, 10.10.73, 10.10.74, 10.10.75, 10.10.76, 10.10.77, 10.10.78, 10.10.79, 10.10.80, 10.10.81, 10.10.82, 10.10.83, 10.10.84, 10.10.85, 10.10.86, 10.10.87, 10.10.88, 10.10.89, 10.10.90, 10.10.91, 10.10.92, 10.10.93, 10.10.94, 10.10.95, 10.10.96, 10.10.97, 10.10.98, 10.10.99, 10.10.100. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility for the design shown. The suitability and use of this drawing for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ALPINE: www.alpinetw.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the roof drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

L = Span of Truss (inches) D = Depth of Truss at Deflection Point (inches)			

Recommended Truss Deflection Limits			
Truss Type	L/D	Deflection Limits	
		Live Load	Total Load
Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)
Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)
Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)
Residential Floor Trusses	24	L/360 (vertical)	L/240 (vertical)
Commercial Floor Trusses	20	L/480 (vertical)	L/240 (vertical)
Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)

Truss Type	Recommended Camber		
Pitched Trusses	1.00 x Deflection from Actual Dead Load		
Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load		
Floor Trusses	(0.25 x Deflection from Live Load) + Actual Dead Load		
Flat Roof Trusses	(0.25 x Deflection from Live Load) + (1.5 x Design Dead Load Deflection)		

Note: The actual dead load may be considerably less than the design dead load.



155 Harlem Ave
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WARNING/AVVERTENZA: LEGGE E TUTTO ALI NOTIZI DI SOSTA
IMPORTANTE: FURNISHING THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing, and bracing. Refer to and follow the latest edition of BCSI Fabricating Component Safety Installation, by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached deck ceiling. SBCA's system for permanent lateral resistance of trusses shall be used. Trusses shall be braced in accordance with the details shown on drawings. The location of trusses and position of supports above and on the joint details, unless noted otherwise, shall refer to drawings 1604-2 for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, or failure of trusses to build in conformance with ANSI/TPI 1, or for handling, shipping, installation, or bracing of trusses.

Each person who uses this drawing, including the installer, shall accept responsibility for professional engineering and liability solely for the design and the building. The liability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

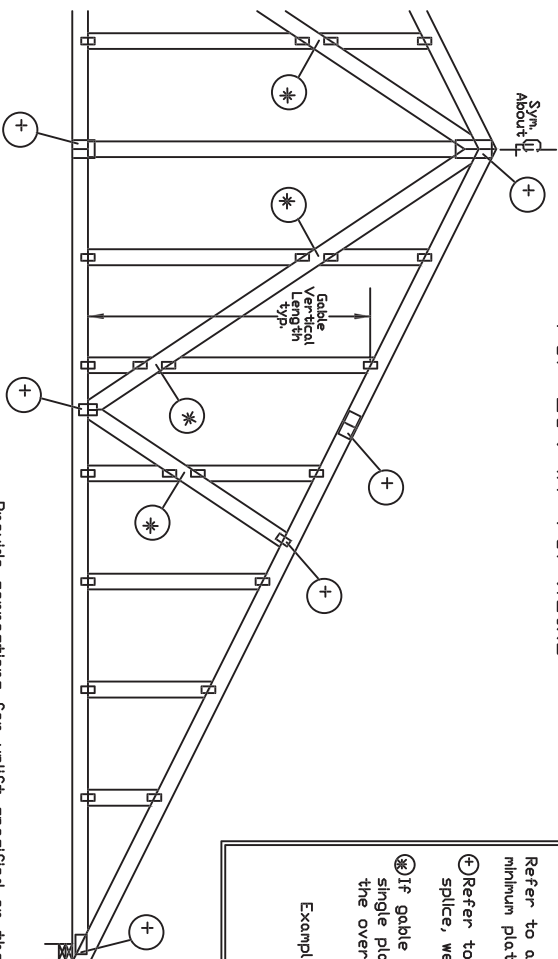
For more information see this job's general notes page and these web sites:
 ALPINE: www.alpine.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

FLORIDA
STATE OF
ENGINEER
No. 10861
CRA 00378
02/10/2023
Florida Certificate of Professional Engineer

Approval #FL 1999

REF	DEFLEC/CAMB
DATE	10/01/14
DRWG	DEFLEC/CAMB1014

Gable Detail For Let-in Verticals



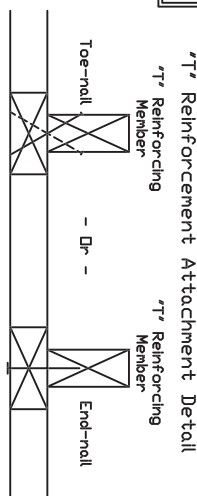
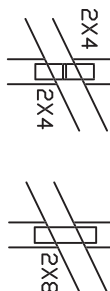
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with
End Driven Nails:
10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE
wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A22015ENC100118, A24015ENC100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A22030ENC100118, A24030ENC100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S22015ENC100118, S24015ENC100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S22030ENC100118, S24030ENC100118

See appropriate Alpine gable detail for maximum web reinforcement length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

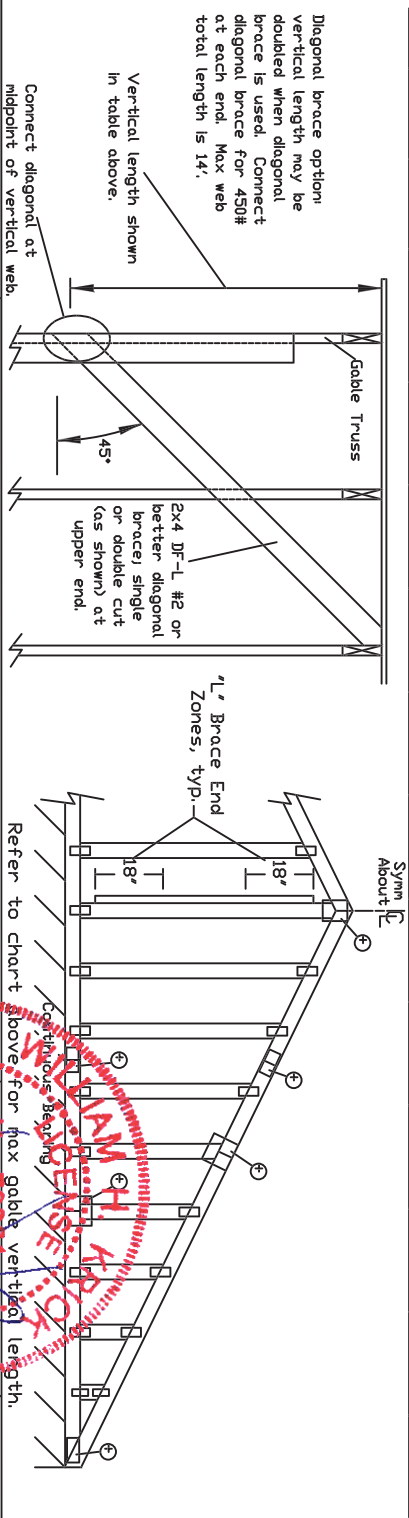
Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (from Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

IMPORTANT! FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extensive work in fabricating, handling, shipping, installing, and bracing. Refer to the following notes for details. These notes are for informational purposes only and do not constitute a contract. The truss manufacturer is responsible for providing the necessary bracing and blocking for the truss. The truss manufacturer is responsible for providing the necessary bracing and blocking for the truss. The truss manufacturer is responsible for providing the necessary bracing and blocking for the truss.

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Max Gable Vertical Length									
Gable Vertical Spacing	2x4 Species	Brace		No Braces	(1) 1x4 "L" Brace		(1) 2x4 "L" Brace		
		Grade	Grade		Group A	Group B	Group A	Group B	
12" o.c.	SPF	#1 / #2		3' 9"	6' 4"	6' 7"	7' 6"	7' 10"	
		#3		3' 7"	6' 3"	6' 6"	7' 5"	7' 8"	
		Stud		3' 7"	6' 3"	6' 6"	7' 5"	7' 8"	
	HF	Standard		3' 7"	5' 8"	6' 0"	7' 5"	7' 8"	
		#1		3' 11"	6' 5"	6' 8"	7' 7"	7' 11"	
		#2		3' 9"	6' 4"	6' 7"	7' 6"	7' 10"	
16" o.c.	SPF	#3		3' 8"	6' 0"	6' 4"	7' 5"	7' 9"	
		Stud		3' 8"	6' 0"	6' 4"	7' 5"	7' 9"	
		Standard		4' 3"	5' 3"	5' 7"	7' 0"	7' 6"	
	HF	#1 / #2		4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	
		#3		4' 1"	7' 2"	7' 5"	8' 6"	8' 10"	
		Stud		4' 1"	7' 2"	7' 5"	8' 6"	8' 10"	
24" o.c.	SPF	Standard		4' 1"	6' 11"	7' 5"	8' 6"	8' 10"	
		#1		4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	
		#2		4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	
	HF	#3		4' 2"	7' 3"	7' 6"	8' 6"	8' 10"	
		Stud		4' 2"	7' 3"	7' 6"	8' 6"	8' 10"	
		Standard		4' 1"	6' 5"	6' 10"	8' 6"	8' 10"	



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Trusses require adequate bracing in fabricating, handling, shipping, installing, and bracing. Refer to the following sections of the ASCE 7-16 for bracing requirements: 5.3.1, 5.3.2, 5.3.3, 5.3.4, 5.3.5, 5.3.6, 5.3.7, 5.3.8, 5.3.9, 5.3.10, 5.3.11, 5.3.12, 5.3.13, 5.3.14, 5.3.15, 5.3.16, 5.3.17, 5.3.18, 5.3.19, 5.3.20, 5.3.21, 5.3.22, 5.3.23, 5.3.24, 5.3.25, 5.3.26, 5.3.27, 5.3.28, 5.3.29, 5.3.30, 5.3.31, 5.3.32, 5.3.33, 5.3.34, 5.3.35, 5.3.36, 5.3.37, 5.3.38, 5.3.39, 5.3.40, 5.3.41, 5.3.42, 5.3.43, 5.3.44, 5.3.45, 5.3.46, 5.3.47, 5.3.48, 5.3.49, 5.3.50, 5.3.51, 5.3.52, 5.3.53, 5.3.54, 5.3.55, 5.3.56, 5.3.57, 5.3.58, 5.3.59, 5.3.60, 5.3.61, 5.3.62, 5.3.63, 5.3.64, 5.3.65, 5.3.66, 5.3.67, 5.3.68, 5.3.69, 5.3.70, 5.3.71, 5.3.72, 5.3.73, 5.3.74, 5.3.75, 5.3.76, 5.3.77, 5.3.78, 5.3.79, 5.3.80, 5.3.81, 5.3.82, 5.3.83, 5.3.84, 5.3.85, 5.3.86, 5.3.87, 5.3.88, 5.3.89, 5.3.90, 5.3.91, 5.3.92, 5.3.93, 5.3.94, 5.3.95, 5.3.96, 5.3.97, 5.3.98, 5.3.99, 5.3.100, 5.3.101, 5.3.102, 5.3.103, 5.3.104, 5.3.105, 5.3.106, 5.3.107, 5.3.108, 5.3.109, 5.3.110, 5.3.111, 5.3.112, 5.3.113, 5.3.114, 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5.3.670, 5.3.671, 5.3.672, 5.3.673, 5.3.674, 5.3.675, 5.3.676, 5.3.677, 5.3.678, 5.3.679, 5.3.680, 5.3.681, 5.3.682, 5.3.683, 5.3.684, 5.3.685, 5.3.686, 5.3.687, 5.3.688, 5.3.689, 5.3.690, 5.3.691, 5.3.692, 5.3.693, 5.3.694, 5.3.695, 5.3.696, 5.3.697, 5.3.698, 5.3.699, 5.3.700, 5.3.701, 5.3.702, 5.3.703, 5.3.704, 5.3.705, 5.3.706, 5.3.707, 5.3.708, 5.3.709, 5.3.710, 5.3.711, 5.3.712, 5.3.713, 5.3.714, 5.3.715, 5.3.716, 5.3.717, 5.3.718, 5.3.719, 5.3.720, 5.3.721, 5.3.722, 5.3.723, 5.3.724, 5.3.725, 5.3.726, 5.3.727, 5.3.728, 5.3.729, 5.3.730, 5.3.731, 5.3.732, 5.3.733, 5.3.734, 5.3.735, 5.3.736, 5.3.737, 5.3.738, 5.3.739, 5.3.740, 5.3.741, 5.3.742, 5.3.743, 5.3.744, 5.3.745, 5.3.746, 5.3.747, 5.3.748, 5.3.749, 5.3.750, 5.3.751, 5.3.752, 5.3.753, 5.3.754, 5.3.755, 5.3.756, 5.3.757, 5.3.758, 5.3.759, 5.3.760, 5.3.761, 5.3.762, 5.3.763, 5.3.764, 5.3.765, 5.3.766, 5.3.767, 5.3.768, 5.3.769, 5.3.770, 5.3.771, 5.3.772, 5.3.773, 5.3.774, 5.3.775, 5.3.776, 5.3.777, 5.3.778, 5.3.779, 5.3.780, 5.3.781, 5.3.782, 5.3.783, 5.3.784, 5.3.785, 5.3.786, 5.3.787, 5.3.788, 5.3.789, 5.3.790, 5.3.791, 5.3.792, 5.3.793, 5.3.794, 5.3.795, 5.3.796, 5.3.797, 5.3.798, 5.3.799, 5.3.800, 5.3.801, 5.3.802, 5.3.803, 5.3.804, 5.3.805, 5.3.806, 5.3.807, 5.3.808, 5.3.809, 5.3.810, 5.3.811, 5.3.812, 5.3.813, 5.3.814, 5.3.815, 5.3.816, 5.3.817, 5.3.818, 5.3.819, 5.3.820, 5.3.821, 5.3.822, 5.3.823, 5.3.824, 5.3.825, 5.3.826, 5.3.827, 5.3.828, 5.3.829, 5.3.830, 5.3.831, 5.3.832, 5.3.833, 5.3.834, 5.3.835, 5.3.836, 5.3.837, 5.3.838, 5.3.839, 5.3.840, 5.3.841, 5.3.842, 5.3.843, 5.3.844, 5.3.845, 5.3.846, 5.3.847, 5.3.848, 5.3.849, 5.3.850, 5.3.851, 5.3.852, 5.3.853, 5.3.854, 5.3.855, 5.3.856, 5.3.857, 5.3.858, 5.3.859, 5.3.860, 5.3.861, 5.3.862, 5.3.863, 5.3.864, 5.3.865, 5.3.866, 5.3.867, 5.3.868, 5.3.869, 5.3.870, 5.3.871, 5.3.872, 5.3.873, 5.3.874, 5.3.875, 5.3.876, 5.3.877, 5.3.878, 5.3.879, 5.3.880, 5.3.881, 5.3.882, 5.3.883, 5.3.884, 5.3.885, 5.3.886, 5.3.887, 5.3.888, 5.3.889, 5.3.890, 5.3.891, 5.3.892, 5.3.893, 5.3.894, 5.3.895, 5.3.896, 5.3.897, 5.3.898, 5.3.899, 5.3.900, 5.3.901, 5.3.902, 5.3.903, 5.3.904, 5.3.905, 5.3.906, 5.3.907, 5.3.908, 5.3.909, 5.3.910, 5.3.911, 5.3.912, 5.3.913, 5.3.914, 5.3.915, 5.3.916, 5.3.917, 5.3.918, 5.3.919, 5.3.920, 5.3.921, 5.3.922, 5.3.923, 5.3.924, 5.3.925, 5.3.926, 5.3.927, 5.3.928, 5.3.929, 5.3.930, 5.3.931, 5.3.932, 5.3.933, 5.3.934, 5.3.935, 5.3.936, 5.3.937, 5.3.938, 5.3.939, 5.3.940, 5.3.941, 5.3.942, 5.3.943, 5.3.944, 5.3.945, 5.3.946, 5.3.947, 5.3.948, 5.3.949, 5.3.950, 5.3.951, 5.3.952, 5.3.953, 5.3.954, 5.3.955, 5.3.956, 5.3.957, 5.3.958, 5.3.959, 5.3.960, 5.3.961, 5.3.962, 5.3.963, 5.3.964, 5.3.965, 5.3.966, 5.3.967, 5.3.968, 5.3.969, 5.3.970, 5.3.971, 5.3.972, 5.3.973, 5.3.974, 5.3.975, 5.3.976, 5.3.977, 5.3.978, 5.3.979, 5.3.980, 5.3.981, 5.3.982, 5.3.983, 5.3.984, 5.3.985, 5.3.986, 5.3.987, 5.3.988, 5.3.989, 5.3.990, 5.3.991, 5.3.992, 5.3.993, 5.3.994, 5.3.995, 5.3.996, 5.3.997, 5.3.998, 5.3.999, 5.4.000, 5.4.001, 5.4.002, 5.4.003, 5.4.004, 5.4.005, 5.4.006, 5.4.007, 5.4.008, 5.4.009, 5.4.010, 5.4.011, 5.4.012, 5.4.013, 5.4.014, 5.4.015, 5.4.016, 5.4.017, 5.4.018, 5.4.019, 5.4.020, 5.4.021, 5.4.022, 5.4.023, 5.4.024, 5.4.025, 5.4.026, 5.4.027, 5.4.028, 5.4.029, 5.4.030, 5.4.031, 5.4.032, 5.4.033, 5.4.034, 5.4.035, 5.4.036, 5.4.037, 5.4.038, 5.4.039, 5.4.040, 5.4.041, 5.4.042, 5.4.043, 5.4.044, 5.4.045, 5.4.046, 5.4.047, 5.4.048, 5.4.049, 5.4.050, 5.4.051, 5.4.052, 5.4.053, 5.4.054, 5.4.055, 5.4.056, 5.4.057, 5.4.058, 5.4.059, 5.4.060, 5.4.061, 5.4.062, 5.4.063, 5.4.064, 5.4.065, 5.4.066, 5.4.067, 5.4.068, 5.4.069, 5.4.070, 5.4.071, 5.4.072, 5.4.073, 5.4.074, 5.4.075, 5.4.076, 5.4.077, 5.4.078, 5.4.079, 5.4.080, 5.4.081, 5.4.082, 5.4.083, 5.4.084, 5.4.085, 5.4.086, 5.4.087, 5.4.088, 5.4.089, 5.4.090, 5.4.091, 5.4.092, 5.4.093, 5.4.094, 5.4.095, 5.4.096, 5.4.097, 5.4.098, 5.4.099, 5.4.100, 5.4.101, 5.4.102, 5.4.103, 5.4.104, 5.4.105, 5.4.106, 5.4.107, 5.4.108, 5.4.109, 5.4.110, 5.4.111, 5.4.112, 5.4.113, 5.4.114, 5.4.115, 5.4.116, 5.4.117, 5.4.118, 5.4.119, 5.4.120, 5.4.121,