



**TowerCore**

11500 Belmack Blvd. N.

Odessa, FL 33556

813-417-3747

Tony.sloan@towercorecontracting.com

Troy Crews  
Columbia County  
Building and Zoning  
RE: Permit #000032030, New Foundation Design

Dear Mr. Crews,

I've enclosed the new foundation design along with the calculations for your review. There are no proposed location changes for the tower therefore we will utilize the same site plan. The rest of the drawings other than the proposed foundation design change are still in effect. Crews are ready to mobilize and our customer is eager to complete this site. If you could please review and let us know if everything is okay so we can re-start the development I would have appreciated. Thank you.

Tony Sloan

BOARD OF COUNTY COMMISSIONERS  
OFFICE OF  
**BUILDING & ZONING**  
COLUMBIA COUNTY, FLORIDA

**BUILDING PERMIT RECEIPT**

RECEIPT NUMBER / PERMIT NUMBER 000032030 DATE 06/11/2014

APPLICANT TURK BEKTAS

OWNER NEW CINGULAR WIRELESS,PCS,LLC

CONTRACTOR TURK BEKTAS

PARCEL ID NUMBER 19-6S-16-03869-105 NUMBER OF EXISTING DWELLINGS 0

TYPE OF DEVELOPMENT COMMUNICATIONS TOWER

COMMENTS SE-0354

**FEES:**

BUILDING PERMIT 950 00 CERTIFICATION FEE 0 00

ZONING FEE 50 00 SURCHARGE FEE 0 00

FLOOD ZONE FEE \_\_\_\_\_ FLOOD DEVELOPMENT PERMIT \_\_\_\_\_

MOBILE HOME PERMIT \_\_\_\_\_ RELOCATION PERMIT \_\_\_\_\_

TRAVEL TRAILER PERMIT \_\_\_\_\_ RECONNECTION PERMIT \_\_\_\_\_

UTILITY POLE PERMIT \_\_\_\_\_ WASTE ASSESSMENT FEE \_\_\_\_\_

FIRE FEE (5 ACRES OR LESS) \_\_\_\_\_ CULVERT PERMIT \_\_\_\_\_

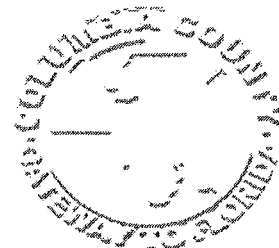
FIRE FEE (MORE THAN 5 ACRES) \_\_\_\_\_ RENEWAL FEE \_\_\_\_\_

CHECK NUMBER 2047 **TOTAL FEES CHARGES** 1000.00

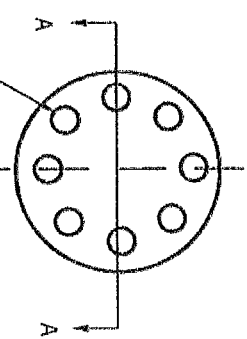
MAKE CHECKS PAYABLE TO BCC (Board of County Commissioners)

NOTE A SEPARATE CHECK IS REQUIRED FOR THE CULVERT WAIVER PERMITS

135 NE HERNANDO AVE.  
SUITE B-21  
LAKE CITY, FL 32055  
Phone: 386-758-1008  
Fax: 386-758-2160



Revised Shaft  
 ok 11-18-14  
 11-18-14



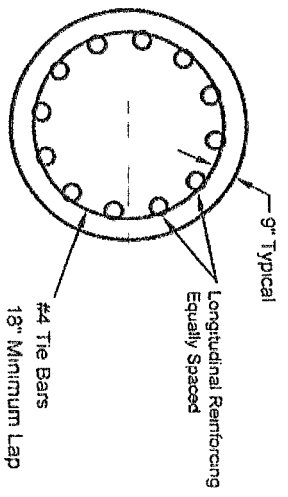
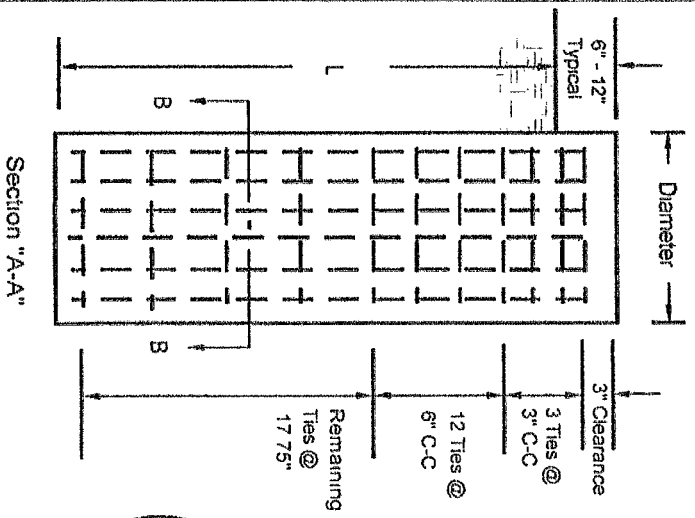
Bolt Circle = 7'1"  
 Based on Tower Drawings

Foundation Layout		Drilled Shaft Information			Tower Loading			
Number of Shafts	"X" Dim	"Y" Dim	Shaft Diameter (IN)	Embedment Depth "L" (FT)	Long. Rebar	Axial Comp. (Kips)	OTM (FT-Kips)	Shear (Kips)
1	*	*	108" **	36'	54 - #10	68.1 <sup>K</sup>	6,414.9 <sup>FT-K</sup>	46.3 <sup>K</sup>

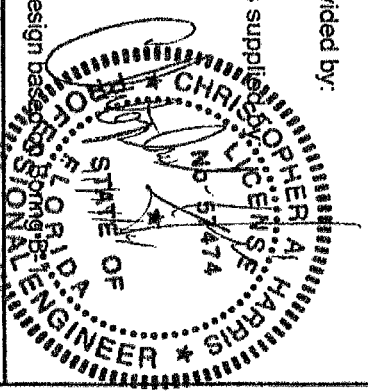
\*\* If needed for ground stability, permanent casing may be installed in the top 20'

Notes:

- 1 Drilled shafts shall be installed in accordance with ACI-336 (Latest Edition).
- 2 Concrete work shall be in accordance with ACI-318 (Latest Edition), and have a minimum 28 day strength of 4,000 psi
- 3 All reinforcing steel shall be deformed billet steel conforming to ASTM A-615, Grade 60.
- 4 All reinforcing steel shall have a minimum of three inches (3") coverage.
5. RWH Engineering will not be responsible for foundation integrity if tower is set within 72 hours of concrete placement, or without prior verification of concrete strength.
- 6 Final length of foundation may vary depending on actual in situ soil conditions relative to boring data supplied
- 7 Tower foundation design based on soil boring data provided by: EGSci Project #2014 10579344 Dated 9/12/14 Boring B-1
- 8 Tower layout and foundation design loading conditions supplied by Nello Drawing #227350 Dated 2/28/14 EIA/TIA-222-G



Date	10/31/14	American Tower Corporation
Scale	None	195' Monopole Foundation
Approved By		Ft. White West Site
		Columbia County, Florida



Summary of Pile Response(s)

Definitions of Pile-head Loading Conditions:

- Load Type 1: Load 1 = Shear, lbs, and Load 2 = Moment, in-lbs
- Load Type 2: Load 1 = Shear, lbs, and Load 2 = Slope, radians
- Load Type 3: Load 1 = Shear, lbs, and Load 2 = Rotational Stiffness, in-lbs/radian
- Load Type 4: Load 1 = Top Deflection, inches, and Load 2 = Moment, in-lbs
- Load Type 5: Load 1 = Top Deflection, inches, and Load 2 = Slope, radians

Load Case No.	Load Type No.	Pile-head Condition		Axial Loading lbs	Pile-head Deflection inches	Maximum Moment in Pile in-lbs	Maximum Shear in Pile lbs	Pile-head Rotation radians
		1 V(lbs) or Y(inches)	2 in-lb, rad., or in-lb/rad.					
1	1	V = 23150.	M = 38489400.	681000.	0.56962138	40858917.	-2070701.	-0.002203310
2	1	V = 46300.	M = 76970800.	681000.	1.07059431	81355523.	-430321.	-0.00951663
3	1	V = 61733.	M = 1.026E+08	998000.	3.08277523	108170116	-6108981.	-0.01413742

The analysis ended normally.

Bending properties relative to the magnitude of bending moment developed in the pile.

Output Summary for Load Case No. 3:

Pile-head deflection = 3.08277752 inches  
 Computed slope at pile head = -0.0141374 radians  
 Maximum bending moment = 10817011.6 Inch-lbs  
 Maximum shear force = -610898.1 lbs  
 Capth of maximum bending moment = 9.6200000 feet below pile head  
 Capth of maximum shear force = 23.1100000 feet below pile head  
 Number of iterations = 49  
 Number of zero deflection points = 1

Pile-head Deflection vs. Pile Length for Load Case 3

Boundary Condition Type 1, Shear and Moment

Shear = 61733.1b  
 Moment = 102538400.1in-lb  
 Axial Load = 90800.1b

Pile Length feet	Pile Head Deflection inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	3.08277752	10817011.6	-610898.
35.1500	3.7997140	107818776.	-676484.
33.3000	4.4392053	107603864.	-713140.
31.4500	5.2369334	107408364.	-746197.
29.6000	5.9522051	107337240.	-773414.
27.7500	10.1183336	107458069.	-856038.

Pile White West (B1) - FL 1p70										
24.420	-0.1139	44126552	-601875.	-0.008438	0.000	2.959E+13	1360.0403	53018.	0.000	0.000
24.790	-0.1513	41471043	-594773.	-0.008411	0.000	2.961E+13	1839.2778	51959.	0.000	0.000
25.160	-0.1888	38351780.	-531508.	-0.008425	0.000	2.964E+13	2334.0346	54899.	0.000	0.000
25.530	-0.2262	36274527.	-574017.	-0.008420	0.000	2.966E+13	2844.3155	55839.	0.000	0.000
25.900	-0.2535	33761244.	-550216.	-0.008415	0.000	2.968E+13	3370.1262	56779.	0.000	0.000
26.270	-0.2809	31310595.	-544051.	-0.008410	0.000	2.971E+13	3911.4734	57715.	0.000	0.000
26.640	-0.3082	28935955.	-525447.	-0.008405	0.000	2.973E+13	4468.3644	58660.	0.000	0.000
27.010	-0.3355	26651408.	-504337.	-0.008401	0.000	2.975E+13	5040.8075	59460.	0.000	0.000
27.380	-0.4128	24465216.	-480650.	-0.008397	0.000	2.977E+13	5628.8113	60340.	0.000	0.000
27.750	-0.4501	22389995.	-454319.	-0.008394	0.000	2.979E+13	6212.3847	61480.	0.000	0.000
28.120	-0.4874	20437635.	-425272.	-0.008391	0.000	2.980E+13	6851.5370	62421.	0.000	0.000
28.490	-0.5246	18620342.	-393442.	-0.008388	0.000	2.982E+13	7486.2773	63361.	0.000	0.000
28.860	-0.5618	16950630.	-358957.	-0.008385	0.000	2.984E+13	8047.8568	64600.	0.000	0.000
29.230	-0.5991	15439568.	-322723.	-0.008383	0.000	2.985E+13	8273.6053	64121.	0.000	0.000
29.600	-0.6363	14091609.	-298078.	-0.008380	0.000	2.986E+13	8627.8923	60753.	0.000	0.000
29.970	-0.6735	12799396.	-285432.	-0.008378	0.000	2.987E+13	8868.3538	62010.	0.000	0.000
30.340	-0.7107	11563728.	-272610.	-0.008377	0.000	2.988E+13	9077.1636	61653.	0.000	0.000
30.710	-0.7479	10385372.	-259620.	-0.008375	0.000	2.989E+13	9244.4707	60748.	0.000	0.000
31.080	-0.7850	9265058.	-246466.	-0.008374	0.000	2.990E+13	9380.4050	60856.	0.000	0.000
31.450	-0.8222	8203560.	-233156.	-0.008372	0.000	2.991E+13	9485.0790	60281.	0.000	0.000
31.820	-0.8594	7201379.	-219695.	-0.008371	0.000	2.991E+13	9548.5915	615750.	0.000	0.000
32.190	-0.8966	6259357.	-206087.	-0.008370	0.000	2.991E+13	9580.0301	615258.	0.000	0.000
32.560	-0.9337	5378072.	-192338.	-0.008369	0.000	2.991E+13	9582.4717	614800.	0.000	0.000
32.930	-0.9709	4558145.	-178451.	-0.008368	0.000	2.991E+13	9542.9814	614374.	0.000	0.000
33.300	-1.0080	3800178.	-164430.	-0.008368	0.000	2.991E+13	9472.6311	613974.	0.000	0.000
33.670	-1.0452	3104754.	-150280.	-0.008367	0.000	2.991E+13	9281.4703	613509.	0.000	0.000
34.040	-1.0823	2472440.	-136003.	-0.008367	0.000	2.991E+13	9229.5471	613248.	0.000	0.000
34.410	-1.1195	1903797.	-121603.	-0.008367	0.000	2.991E+13	9256.9940	612917.	0.000	0.000
34.780	-1.1566	1399356.	-105737.	-0.008366	0.000	2.991E+13	9300.6149	61307.	0.000	0.000
35.150	-1.1938	971417.	-88459.	-0.008366	0.000	2.991E+13	9311.4940	61549.	0.000	0.000
35.520	-1.2309	620385.	-71926.	-0.008366	0.000	2.991E+13	9341.4321	61217.	0.000	0.000
35.890	-1.2681	347454.	-53460.	-0.008366	0.000	2.991E+13	9370.8367	61394.	0.000	0.000
36.260	-1.3052	152503.	-35766.	-0.008366	0.000	2.991E+13	9399.6521	61395.	0.000	0.000
36.630	-1.3424	36300.	-17945.	-0.008366	0.000	2.991E+13	4027.8298	13322.	0.000	0.000
37.000	-1.3795	0.000	0.000	-0.008366	0.000	2.991E+13	4055.3990	15225.2549	0.000	0.000

\* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for "nonlinear".

9.670	1.5013	1.082E+08	-2145.9534	-0.0115	0.000	4.638E+12	-2865.0509	7543.8295	0.000
9.990	1.5505	1.081E+08	-15285.	-0.0114	0.000	4.638E+12	-3053.6198	8744.2920	0.000
10.350	1.5001	1.080E+08	-29253.	-0.0113	0.000	4.638E+12	-3238.1996	9584.2805	0.000
10.730	1.4502	1.079E+08	-44031.	-0.0112	0.000	4.638E+12	-3418.2607	10463.	0.000
11.100	1.4007	1.077E+08	-59596.	-0.0111	0.000	4.639E+12	-3593.2800	11390.	0.000
11.470	1.3517	1.074E+08	-75927.	-0.0110	0.000	4.639E+12	-3762.7395	12359.	0.000
11.840	1.3032	1.070E+08	-92966.	-0.0109	0.000	4.640E+12	-3926.1244	13376.	0.000
12.210	1.2551	1.065E+08	-110776.	-0.0108	0.000	4.641E+12	-4082.9219	14441.	0.000
12.580	1.2074	1.060E+08	-129237.	-0.0107	0.000	4.642E+12	-4232.9487	15563.	0.000
12.950	1.1602	1.054E+08	-148359.	-0.0106	0.000	4.644E+12	-4380.4257	16763.	0.000
13.320	1.1135	1.047E+08	-168119.	-0.0105	0.000	4.645E+12	-4520.4108	18025.	0.000
13.690	1.0672	1.039E+08	-188482.	-0.0104	0.000	4.647E+12	-4652.4034	19356.	0.000
14.060	1.0213	1.030E+08	-209413.	-0.0103	0.000	4.651E+12	-4775.8963	20762.	0.000
14.430	0.9759	1.021E+08	-230872.	-0.0102	0.000	4.653E+12	-4890.3774	22249.	0.000
14.800	0.9309	1.010E+08	-252819.	-0.0101	0.000	4.656E+12	-4995.5555	23825.	0.000
15.170	0.8864	99837344.	-275208.	-0.009989	0.000	4.659E+12	-5090.1555	25498.	0.000
15.540	0.8422	98569255.	-297999.	-0.009894	0.000	4.659E+12	-5174.3577	27273	0.000
15.910	0.7985	9715912.	-321132.	-0.009801	0.000	4.662E+12	-5247.3298	29177	0.000
16.280	0.7552	95725599.	-344565.	-0.009709	0.000	4.665E+12	-5308.4590	31210	0.000
16.650	0.7123	94147210.	-368200.	-0.009619	0.000	4.669E+12	-5337.9271	33274	0.000
17.020	0.6698	92463645.	-391508.	-0.009530	0.000	4.673E+12	-5361.1897	34214	0.000
17.390	0.6277	90678299.	-413999.	-0.009443	0.000	4.678E+12	-4969.5368	35154	0.000
17.760	0.5859	88794951.	-435605.	-0.009358	0.000	4.683E+12	-4763.1518	36094.	0.000
18.130	0.5446	86837670.	-456263.	-0.009275	0.000	4.688E+12	-4542.2082	37035.	0.000
18.500	0.5036	84750813.	-475908.	-0.009194	0.000	4.694E+12	-4306.0686	37975.	0.000
18.870	0.4629	82599019.	-494477.	-0.009115	0.000	4.700E+12	-4057.2851	38915.	0.000
19.240	0.4226	80367210.	-5121905.	-0.009038	0.000	4.706E+12	-3793.5086	39855.	0.000
19.610	0.3827	78060585.	-528133.	-0.008963	0.000	4.713E+12	-3515.9190	40795.	0.000
19.980	0.3430	75684619.	-543096.	-0.008891	0.000	4.721E+12	-3224.4246	41735.	0.000
20.350	0.3037	73245059.	-556735.	-0.008821	0.000	4.729E+12	-2919.1625	42675.	0.000
20.720	0.2647	70747924.	-568938.	-0.008753	0.000	4.738E+12	-2600.2481	43615.	0.000
21.090	0.2260	68199503.	-579795.	-0.008688	0.000	4.747E+12	-2267.7056	44555.	0.000
21.460	0.1875	655606349.	-58936.	-0.008626	0.000	4.758E+12	-1921.7874	45497.	0.000
21.830	0.1494	62975236.	-596831.	-0.008566	0.000	4.769E+12	-1562.3748	46437.	0.000
22.200	0.1115	60313399.	-602940.	-0.008508	0.000	4.781E+12	-1189.5775	47377.	0.000
22.570	0.0738	57628038.	-607355.	-0.008447	0.000	2.946E+13	-803.4110	48317.	0.000
22.940	0.0362	54926836.	-610040.	-0.008384	0.000	2.949E+13	-401.7652	49257.	0.000
23.310	-0.001364	52217710.	-610898.	-0.008316	0.000	2.951E+13	15.4162	50198.	0.000
23.680	-0.0339	49508934.	-609869.	-0.008245	0.000	2.954E+13	448.1111	51138.	0.000
24.050	-0.0754	46808890.	-606884	-0.008175	0.000	2.956E+13	896.3188	52078.	0.000

Ft White West (B1)-FL1P70

Ft White West (81)-Fl.1p70  
 Computed Values of Pile Loading and Deflection  
 for Lateral Loading for Load Case Number 3

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 61773.3 lbs  
 Applied moment at pile head = 102638400.0 in-lbs  
 Axial thrust load on pile head = 908000.0 lbs

Depth X feet	Deflect.		Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness lb-in <sup>2</sup>	Soil Res. p lb/in	Soil Spn. E <sub>s</sub> <sup>in</sup> lb/inch	Disturb Lat. Load lb/inch
	Y inches	Y inches								
0.00	3.0828	1.0265E+08	61773.	-0.0141	0.000	4.650E+12	0.000	0.000	0.000	
0.370	3.0202	1.0295E+08	61773.	-0.0140	0.000	4.650E+12	0.000	0.000	0.000	
0.740	2.9581	1.0325E+08	61773.	-0.0139	0.000	4.648E+12	0.000	0.000	0.000	
1.110	2.8964	1.0355E+08	61773.	-0.0138	0.000	4.648E+12	0.000	0.000	0.000	
1.480	2.8352	1.0385E+08	61773.	-0.0137	0.000	4.647E+12	0.000	0.000	0.000	
1.850	2.7744	1.0440E+08	61773.	-0.0136	0.000	4.646E+12	0.000	0.000	0.000	
2.220	2.7140	1.0440E+08	61773.	-0.0135	0.000	4.646E+12	0.000	0.000	0.000	
2.590	2.6541	1.0440E+08	61773.	-0.0134	0.000	4.645E+12	0.000	0.000	0.000	
2.960	2.5946	1.0495E+08	61773.	-0.0133	0.000	4.645E+12	0.000	0.000	0.000	
3.330	2.5356	1.0525E+08	61511.	-0.0132	0.000	4.644E+12	-100.1054	175.2099	0.000	
3.700	2.4770	1.0544E+08	60828.	-0.0131	0.000	4.644E+12	-207.4350	371.8270	0.000	
4.070	2.4189	1.0575E+08	59680.	-0.0130	0.000	4.643E+12	-309.6439	568.3641	0.000	
4.440	2.3612	1.0605E+08	58090.	-0.0129	0.000	4.643E+12	-406.7795	764.9012	0.000	
4.810	2.3040	1.0632E+08	56079.	-0.0128	0.000	4.642E+12	-498.9059	961.4383	0.000	
5.180	2.2472	1.0655E+08	53671.	-0.0127	0.000	4.641E+12	-586.0831	1157.9755	0.000	
5.550	2.1909	1.0675E+08	50836.	-0.0126	0.000	4.641E+12	-668.3712	1354.5125	0.000	
5.920	2.1350	1.0695E+08	47746.	-0.0125	0.000	4.640E+12	-745.8303	1551.0497	0.000	
6.290	2.0796	1.0722E+08	44273.	-0.0124	0.000	4.640E+12	-818.5225	1747.5863	0.000	
6.660	2.0246	1.0735E+08	40438.	-0.0123	0.000	4.639E+12	-886.5069	1944.1240	0.000	
7.030	1.9701	1.0755E+08	36412.	-0.0122	0.000	4.639E+12	-949.3450	2140.6511	0.000	
7.400	1.9160	1.0775E+08	32054.	-0.0121	0.000	4.639E+12	-1008.5977	2337.1982	0.000	
7.770	1.8624	1.0785E+08	27455.	-0.0120	0.000	4.638E+12	-1062.8261	2533.7353	0.000	
8.140	1.8093	1.0795E+08	22636.	-0.0119	0.000	4.638E+12	-1112.5915	2730.2724	0.000	
8.510	1.7566	1.0805E+08	17595.	-0.0118	0.000	4.638E+12	-1157.9543	2926.8095	0.000	
8.880	1.7044	1.0815E+08	12313.	-0.0117	0.000	4.638E+12	-1198.9775	3123.3467	0.000	
9.250	1.6526	1.0815E+08	6957.7594	-0.0116	0.000	4.638E+12	-1235.7205	3319.8838	0.000	



Output Summary for Load Case No. 2:

Pile-head deflection = 1.8705943 inches  
 Computed slope at pile head = -0.0089166 radians  
 Maximum bending moment = 81356533. Inch-lbs  
 Maximum shear force = -430321. lbs  
 Depth of maximum bending moment = 9.9900000 feet below pile head  
 Depth of maximum shear force = 23.6800000 feet below pile head  
 Number of iterations = 52  
 Number of zero deflection points = 1

Pile head Deflection vs. Pile length for Load Case 2

Boundary Condition Type 1, Shear and Moment

Shear = 45300. lb  
 Moment = 76978300. in-lb  
 Axial Load = 68100. lb

Pile Length Feet	Pile Head Deflection Inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	1.8705943	81356533.	-430321.
35.1500	2.2498857	81144708.	-475623.
33.3000	2.5553534	80980557.	-509757.
31.4500	2.9593293	80804510.	-542079.
29.6000	3.3390896	80677503.	-564702.
27.7500	4.9144083	80346127.	-619022.
25.9000	8.2998628	80317578.	-681442.

Pile White West (B1)-FL.1D70									
24.790	-0.0717	31055289.	-424190.	-0.004973	0.000	2.969E+13	871.1797	53959.	0.000
25.150	-0.0933	31182976.	-419682.	-0.004958	0.000	2.971E+13	1159.2205	54899.	0.000
25.530	-0.1158	29332515.	-413876.	-0.004953	0.000	2.973E+13	1456.3473	55839.	0.000
25.900	-0.1378	27518762.	-405730.	-0.004959	0.000	2.974E+13	1762.5629	56779.	0.000
26.270	-0.1598	25723755.	-398204.	-0.004955	0.000	2.976E+13	2077.8707	57719.	0.000
26.640	-0.1818	23977708.	-388258.	-0.004951	0.000	2.977E+13	2402.2749	58660.	0.000
27.010	-0.2038	22279018.	-376851.	-0.004948	0.000	2.979E+13	2735.7803	59620.	0.000
27.380	-0.2258	20634259.	-363944.	-0.004945	0.000	2.980E+13	3078.3923	60540.	0.000
27.750	-0.2477	19050186.	-349495.	-0.004942	0.000	2.982E+13	3430.1167	61480.	0.000
28.120	-0.2697	17533731.	-333464.	-0.004939	0.000	2.983E+13	3790.9395	62421.	0.000
28.490	-0.2916	16092009.	-315811.	-0.004934	0.000	2.985E+13	4160.9272	63361.	0.000
28.860	-0.3135	14732314.	-296455.	-0.004932	0.000	2.987E+13	4540.0264	64301.	0.000
29.230	-0.3354	13462117.	-275475.	-0.004930	0.000	2.988E+13	4928.2634	65241.	0.000
29.600	-0.3573	12289075.	-259160.	-0.004929	0.000	2.988E+13	2447.9496	25094.	0.000
29.970	-0.3792	11164289.	-248150.	-0.004929	0.000	2.988E+13	2484.6074	27895.	0.000
30.340	-0.4011	10088483.	-237040.	-0.004927	0.000	2.989E+13	2519.6993	26806.	0.000
30.710	-0.4229	9062150.	-225778.	-0.004926	0.000	2.990E+13	2553.3734	24959.	0.000
31.080	-0.4448	8086152.	-214469.	-0.004924	0.000	2.992E+13	2585.7568	21811.	0.000
31.450	-0.4667	7161728.	-202819.	-0.004923	0.000	2.993E+13	2616.9599	24999.	0.000
31.820	-0.4885	6288194.	-191133.	-0.004922	0.000	2.993E+13	2647.0788	24059.	0.000
32.190	-0.5104	5467443.	-179315.	-0.004921	0.000	2.993E+13	2676.1978	21282.	0.000
32.560	-0.5322	4699149.	-167371.	-0.004920	0.000	2.993E+13	2704.3917	22561.	0.000
32.930	-0.5541	3984168.	-155302.	-0.004919	0.000	2.993E+13	2731.7265	21891.	0.000
33.300	-0.5759	3323039.	-143115.	-0.004919	0.000	2.993E+13	2758.2615	21255.	0.000
33.670	-0.5977	2716286.	-130811.	-0.004919	0.000	2.993E+13	2784.0494	20680.	0.000
34.040	-0.6196	2164416.	-118394.	-0.004919	0.000	2.993E+13	2809.1380	20131.	0.000
34.410	-0.6414	1667923.	-105867.	-0.004918	0.000	2.993E+13	2833.5703	19614.	0.000
34.780	-0.6633	1227291.	-92080.	-0.004918	0.000	2.993E+13	3176.9098	22606.	0.000
35.150	-0.6851	853230.	-77825.	-0.004918	0.000	2.993E+13	3404.3671	22069.	0.000
35.520	-0.7069	546281.	-61350.	-0.004918	0.000	2.993E+13	3431.1750	21550.	0.000
35.890	-0.7288	306973.	-46558.	-0.004918	0.000	2.993E+13	3457.3685	21064.	0.000
35.260	-0.7506	135822.	-31150.	-0.004918	0.000	2.993E+13	3482.9795	20603.	0.000
35.630	-0.7724	33333.	-15330.	-0.004918	0.000	2.993E+13	3508.0375	20164.	0.000
37.000	-0.7943	0.000	0.000	-0.004918	0.000	2.993E+13	3532.5700	9873.6353	0.000

\* This analysis computed pile response using nonlinear moment-curvature relationships. Values of total stress due to combined axial and bending stresses are computed only for elastic sections only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for nonlinear bending properties relative to the magnitude of bending moment developed in the pile.

Ft. White West (81) -FL 1970

9.990	0.9229	\$135533.	-4587.5435	0.006873	0.000	4.663E+12	-2628.7272	12647.	0.000
10.360	0.8926	\$1310988.	-16998.	0.006796	0.000	4.663E+12	-2785.9055	13858.	0.000
10.730	0.8626	\$1210499.	-73617.	0.006718	0.000	4.664E+12	-2939.0052	15128.	0.000
11.100	0.8329	\$1052049.	-42996	0.006641	0.000	4.664E+12	-3087.5086	16459.	0.000
11.470	0.8036	\$8832707.	-57024	0.006564	0.000	4.665E+12	-3231.2144	17853.	0.000
11.840	0.7746	\$6549644.	-71678.	0.006487	0.000	4.665E+12	-3369.4069	19314.	0.000
12.210	0.7460	\$4290132.	-89912.	0.006411	0.000	4.666E+12	-3501.9550	20843.	0.000
12.580	0.7177	\$231562.	102752	0.006335	0.000	4.667E+12	-3628.5318	22448.	0.000
12.950	0.6897	\$9291437.	-119050	0.006259	0.000	4.668E+12	-3708.2881	23871.	0.000
13.320	0.6621	\$728185.	-135496	0.006184	0.000	4.670E+12	-3899.9844	24812.	0.000
13.690	0.6348	\$4891971.	-151834	0.006109	0.000	4.671E+12	-3981.9096	25752.	0.000
14.060	0.6079	\$2333150.	-168170	0.006035	0.000	4.673E+12	-3954.2579	26592.	0.000
14.430	0.5812	\$662269.	184313	0.005962	0.000	4.675E+12	-3617.2194	27532.	0.000
14.800	0.5549	\$7570058.	-209271	0.005890	0.000	4.677E+12	-3570.9794	28573.	0.000
15.170	0.5289	\$4827428.	-216003	0.005819	0.000	4.680E+12	-3515.7180	29513.	0.000
15.540	0.5032	\$3835468.	-231471	0.005748	0.000	4.683E+12	-3451.6105	30453.	0.000
15.910	0.4779	\$2275444.	-24634.	0.005679	0.000	4.685E+12	-3378.8261	31393.	0.000
16.280	0.4528	\$1648791.	-251456	0.005610	0.000	4.689E+12	-3297.5285	32333	0.000
16.650	0.4281	\$945711.	-27898.	0.005543	0.000	4.692E+12	-3207.8752	33274.	0.000
17.020	0.4036	\$6202171.	-28923.	0.005477	0.000	4.696E+12	-3110.0172	34214.	0.000
17.390	0.3794	\$7885903.	-30497.	0.005412	0.000	4.700E+12	-3004.0991	35154.	0.000
17.760	0.3535	\$6510193.	-316582.	0.005349	0.000	4.704E+12	-2890.2586	36094.	0.000
18.130	0.3319	\$5077887.	-329145.	0.005287	0.000	4.709E+12	-2768.6263	37035.	0.000
18.500	0.3086	\$3590783.	-344151.	0.005226	0.000	4.713E+12	-2639.3256	37975.	0.000
18.870	0.2855	\$2051630.	-35565.	0.005167	0.000	4.719E+12	-2502.4727	38915.	0.000
19.240	0.2627	\$9463127.	-365356.	0.005109	0.000	4.725E+12	-2358.1761	39855.	0.000
19.610	0.2401	\$3828118.	-373490.	0.005073	0.000	1.712E+13	-2206.5367	40795.	0.000
19.980	0.2177	\$749606.	-382930.	0.005061	0.000	2.947E+13	-2045.9802	41736.	0.000
20.350	0.1952	\$5430758.	-391638.	0.005053	0.000	2.948E+13	-1876.2858	42676.	0.000
20.720	0.1728	\$3674919.	-399571.	0.005044	0.000	2.950E+13	-1697.4064	43616.	0.000
21.090	0.1504	\$1885616.	-406690.	0.005037	0.000	2.952E+13	-1509.4135	44556.	0.000
21.460	0.1281	\$9066554.	-412955.	0.005029	0.000	2.953E+13	-1312.2974	45497.	0.000
21.830	0.1058	\$4821621.	-418323.	0.005021	0.000	2.955E+13	-1106.0673	46437.	0.000
22.200	0.0835	\$6354380.	-422756.	0.005014	0.000	2.957E+13	-890.7310	47377.	0.000
22.570	0.0612	\$4470578.	-426213.	0.005008	0.000	2.959E+13	-666.2953	48317.	0.000
22.940	0.0390	\$2573138.	-428653.	0.005001	0.000	2.960E+13	-432.7656	49257.	0.000
23.310	0.0168	\$4667166.	-430036.	0.004995	0.000	2.962E+13	-190.1462	50198.	0.000
23.680	-0.005345	\$3757443.	-430321.	0.004989	0.000	2.964E+13	61.5596	51138.	0.000
24.050	-0.0275	\$684932.	-429469.	0.004983	0.000	2.966E+13	322.3497	52078.	0.000
24.420	-0.0496	\$4946773.	-42438.	0.004978	0.000	2.967E+13	592.2231	53018.	0.000

Pt. White West (E1)-Fl. 1p70  
for Lateral Loading for Load Case Number 2

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 46300.0 lbs  
 Applied moment at pile head = 76978800.0 in-lbs  
 Axial thrust load on pile head = 68100.0 lbs

Depth X feet	Deflect. Y inches	Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness lb-in <sup>2</sup>	Soil Res. p lb/in	Soil Spr. Es* lb/inch	Distrib Lat. Load lb/inch
0.00	1.8706	76978800.	46300.	-0.008917	0.000	4.674E+12	0.000	0.000	0.000
0.370	1.8312	77187057.	46300.	-0.008843	0.000	4.674E+12	0.000	0.000	0.000
0.740	1.7921	77395292.	46300.	-0.008770	0.000	4.673E+12	0.000	0.000	0.000
1.110	1.7533	77603504.	46300.	-0.008696	0.000	4.673E+12	0.000	0.000	0.000
1.480	1.7148	77811695.	46300.	-0.008623	0.000	4.672E+12	0.000	0.000	0.000
1.850	1.6767	78019863.	46300.	-0.008548	0.000	4.672E+12	0.000	0.000	0.000
2.220	1.6385	78228008.	46300.	-0.008474	0.000	4.671E+12	0.000	0.000	0.000
2.590	1.6015	78436131.	46300.	-0.008400	0.000	4.670E+12	0.000	0.000	0.000
2.960	1.5643	78644232.	46300.	-0.008325	0.000	4.670E+12	0.000	0.000	0.000
3.330	1.5275	78852310.	46166.	-0.008250	0.000	4.669E+12	0.000	0.000	0.000
3.700	1.4911	79059176.	45755.	-0.008175	0.000	4.668E+12	-60.3070	175.2899	0.000
4.070	1.4549	79263558.	45064.	-0.008100	0.000	4.668E+12	-124.8702	371.8270	0.000
4.440	1.4192	79464246.	44108.	-0.008024	0.000	4.668E+12	-186.2479	568.3641	0.000
4.810	1.3837	79660091.	42900.	-0.007949	0.000	4.667E+12	-244.4847	764.9012	0.000
5.180	1.3486	79850006.	41454.	-0.007873	0.000	4.667E+12	-299.6251	961.4383	0.000
5.550	1.3138	80032965.	39784.	-0.007797	0.000	4.667E+12	-351.7139	1157.9755	0.000
5.920	1.2793	80206000.	37902.	-0.007720	0.000	4.666E+12	-400.7962	1354.5126	0.000
6.290	1.2452	80374201.	35822.	-0.007644	0.000	4.666E+12	-446.9168	1551.0497	0.000
6.660	1.2115	80538717.	33556.	-0.007568	0.000	4.665E+12	-490.1211	1747.5868	0.000
7.030	1.1780	80697675.	31117.	-0.007491	0.000	4.665E+12	-530.4544	1944.1240	0.000
7.400	1.1449	80811569.	28518.	-0.007414	0.000	4.665E+12	-567.9621	2140.6611	0.000
7.770	1.1122	809334481.	25772.	-0.007337	0.000	4.665E+12	-602.6897	2337.1982	0.000
8.140	1.0798	810444857.	22888.	-0.007260	0.000	4.664E+12	-634.6829	2535.7353	0.000
8.510	1.0477	81142121.	19881.	-0.007183	0.000	4.664E+12	-663.9873	2730.2724	0.000
8.880	1.0160	81225746.	16761.	-0.007105	0.000	4.664E+12	-690.6465	2926.8095	0.000
9.250	0.9846	81295258.	13540.	-0.007028	0.000	4.663E+12	-714.7127	3125.3467	0.000
9.620	0.9536	81350233.	6427.0051	-0.006951	0.000	4.663E+12	-736.2254	3319.8838	0.000

Ft White West (B1)-Fl. 1p70

Pile-head deflection = 0.5696214 inches  
 Computed slope at pile head = -0.0020331 radians  
 Maximum bending moment = 40858917. inch-lbs  
 Maximum shear force = -207070. lbs  
 Depth of maximum bending moment = 10.3600000 feet below pile head  
 Depth of maximum shear force = 25.9000000 feet below pile head  
 Number of iterations = 22  
 Number of zero deflection points = 1

Pile-head Deflection vs. Pile length for load Case 1

Boundary Condition Type 1, Shear and Moment

Shear = 23150. lb  
 Moment = 38489400. in-lb  
 Axial load = 68100. lb

Pile Length feet	Pile Head Deflection inches	Maximum Moment In-lbs	Maximum Shear lbs
37.0000	0.5696214	40858917.	-207070.
35.1500	0.7442185	40756470.	-222215.
33.3000	0.8973385	40657925.	-236799.
31.4500	1.0576334	40573577.	-256375.
29.6000	1.1891559	40506291.	-267275.
27.7500	1.5264730	40375926.	-288854.
25.9000	2.1087076	40234818.	-320294.
24.0500	3.3362940	40081302.	-361973.
22.2000	5.9177757	39987835.	-404454.
20.3500	10.9020022	40336089.	-494109.

26.270	-0.006651	16736605.	-206899.	-0.001658	0.000	2.984E+13	86.4589	57713.	0.000													
26.640	-0.0140	15816329.	-206296.	-0.001655	0.000	2.985E+13	185.0436	58650.	0.000													
27.010	-0.0214	14905700.	-205249.	-0.001653	0.000	2.986E+13	286.6033	59600.	0.000													
27.380	-0.0287	13997720.	-203744.	-0.001651	0.000	2.987E+13	391.1396	60540.	0.000													
27.750	-0.0360	13097450.	-201769.	-0.001649	0.000	2.987E+13	498.6544	61480.	0.000													
28.120	-0.0433	12207010.	-199309.	-0.001647	0.000	2.988E+13	609.1504	62421.	0.000													
28.490	-0.0506	11326578.	-196353.	-0.001645	0.000	2.988E+13	722.6305	63361.	0.000													
28.860	-0.0579	10464391.	-192886.	-0.001644	0.000	2.989E+13	839.0979	64301.	0.000													
29.230	-0.0652	9616745.	-188895.	-0.001642	0.000	2.990E+13	958.5563	65241.	0.000													
29.600	-0.0725	8787996.	-183119.	-0.001641	0.000	2.991E+13	1083.1936	66181.	0.000													
29.970	-0.0798	7991639.	-175735.	-0.001640	0.000	2.992E+13	1214.9726	67121.	0.000													
30.340	-0.0871	7226460.	-168180.	-0.001639	0.000	2.993E+13	1354.0895	68061.	0.000													
30.710	-0.0944	6499189.	-160466.	-0.001637	0.000	2.993E+13	1501.7847	69001.	0.000													
31.080	-0.1016	5804514.	-152601.	-0.001637	0.000	2.993E+13	1658.1936	70041.	0.000													
31.450	-0.1089	5145082.	-144594.	-0.001636	0.000	2.993E+13	1824.3301	71081.	0.000													
31.820	-0.1161	4521507.	-136452.	-0.001635	0.000	2.993E+13	2000.5179	72121.	0.000													
32.190	-0.1234	3934373.	-128182.	-0.001634	0.000	2.993E+13	2187.7631	73161.	0.000													
32.560	-0.1307	3384236.	-119790.	-0.001634	0.000	2.993E+13	2385.1779	74201.	0.000													
32.930	-0.1379	2871627.	-111280.	-0.001633	0.000	2.993E+13	2592.7847	75241.	0.000													
33.300	-0.1452	2397058.	-102657.	-0.001633	0.000	2.993E+13	2810.6030	76281.	0.000													
33.670	-0.1524	1961018.	-93926.	-0.001633	0.000	2.993E+13	3038.7368	77321.	0.000													
34.040	-0.1597	1563981.	-85092.	-0.001632	0.000	2.993E+13	3277.1851	78361.	0.000													
34.410	-0.1669	1206401.	-76154.	-0.001632	0.000	2.993E+13	3525.9504	79401.	0.000													
34.780	-0.1742	888719.	-66294.	-0.001632	0.000	2.993E+13	3785.1220	80441.	0.000													
35.150	-0.1814	618693.	-55506.	-0.001632	0.000	2.993E+13	4054.7007	81481.	0.000													
35.520	-0.1887	396811.	-44610.	-0.001632	0.000	2.993E+13	4334.7847	82521.	0.000													
35.890	-0.1959	223547.	-33608.	-0.001632	0.000	2.993E+13	4625.2726	83561.	0.000													
36.260	-0.2031	99360.	-22504.	-0.001632	0.000	2.993E+13	4926.1663	84601.	0.000													
36.630	-0.2104	24700.	-11300.	-0.001632	0.000	2.993E+13	5237.4631	85641.	0.000													
37.000	-0.2176	0.000	0.000	-0.001632	0.000	2.993E+13	5559.0631	86681.	0.000													

\* This analysis computed pile response using nonlinear moment-curvature relationships for elastic sections only and do not equal the actual stresses in concrete and steel. Values of total stress only and do not equal the actual stresses in concrete and steel. Stresses in concrete and steel may be interpolated from the output for nonlinear bending properties relative to the magnitude of bending moment developed in the pile.

Ft White West (B1)-Fl. 1P70

11.470	0.3024	40714808.	-19894.	-0.001848	0.000	2.962E+13	-1369	7727	20111.	0.000
11.840	0.2942	40613535.	-26032.	-0.001841	0.000	2.962E+13	-1394	9347	21051.	0.000
12.210	0.2861	40484759.	-32274.	-0.001835	0.000	2.962E+13	-1416	8621	21991.	0.000
12.580	0.2779	40328051.	-38606.	-0.001829	0.000	2.962E+13	-1435	4217	22931.	0.000
12.950	0.2698	40143043.	-45013.	-0.001823	0.000	2.963E+13	-1450	6799	23871.	0.000
13.320	0.2617	39929435.	-51481.	-0.001817	0.000	2.963E+13	-1462	6532	24812.	0.000
13.690	0.2537	39686991.	-57994.	-0.001811	0.000	2.963E+13	-1471	3576	25752.	0.000
14.060	0.2457	39415540.	-64539.	-0.001805	0.000	2.963E+13	-1476	8091	26692.	0.000
14.430	0.2377	39114974.	-71101.	-0.001800	0.000	2.964E+13	-1479	0233	27632.	0.000
14.800	0.2297	38785249.	-77666.	-0.001794	0.000	2.964E+13	-1478	0155	28573.	0.000
15.170	0.2217	38426385.	-84219.	-0.001788	0.000	2.964E+13	-1473	8008	29513.	0.000
15.540	0.2138	38038466.	-90746.	-0.001782	0.000	2.965E+13	-1466	3940	30453.	0.000
15.910	0.2059	37621637.	-97233.	-0.001777	0.000	2.965E+13	-1455	8294	31393.	0.000
16.280	0.1980	37176107.	-103667.	-0.001771	0.000	2.966E+13	-1442	0610	32333.	0.000
16.650	0.1902	36702147.	-110032.	-0.001765	0.000	2.966E+13	-1425	1526	33274.	0.000
17.020	0.1823	36200091.	-116315.	-0.001760	0.000	2.966E+13	-1405	1272	34214.	0.000
17.390	0.1745	35670333.	-122503.	-0.001755	0.000	2.967E+13	-1381	9678	35154.	0.000
17.760	0.1668	35113329.	-128580.	-0.001749	0.000	2.967E+13	-1355	6968	36094.	0.000
18.130	0.1590	34529598.	-134534.	-0.001744	0.000	2.968E+13	-1326	3260	37035.	0.000
18.500	0.1513	33919720.	-140351.	-0.001739	0.000	2.968E+13	-1293	8671	37975.	0.000
18.870	0.1436	33284332.	-146017.	-0.001734	0.000	2.969E+13	-1258	3308	38915.	0.000
19.240	0.1359	32624137.	-151518.	-0.001729	0.000	2.969E+13	-1219	7279	39855.	0.000
19.610	0.1282	31939896.	-156841.	-0.001724	0.000	2.970E+13	-1178	0681	40795.	0.000
19.980	0.1206	31232428.	-161973.	-0.001719	0.000	2.971E+13	-1133	3609	41736.	0.000
20.350	0.1129	30502617.	-166899.	-0.001715	0.000	2.972E+13	-1085	6154	42676.	0.000
20.720	0.1053	29751403.	-171606.	-0.001710	0.000	2.972E+13	-1034	8397	43616.	0.000
21.090	0.0978	28979788.	-176082.	-0.001706	0.000	2.973E+13	-981	0418	44556.	0.000
21.460	0.0902	28188831.	-180311.	-0.001702	0.000	2.974E+13	-924	2288	45497.	0.000
21.830	0.0826	27379653.	-184282.	-0.001697	0.000	2.974E+13	-864	4075	46437.	0.000
22.200	0.0751	26553433.	-187981.	-0.001693	0.000	2.975E+13	-801	5839	47377.	0.000
22.570	0.0676	25711410.	-191393.	-0.001690	0.000	2.976E+13	-735	7636	48317.	0.000
22.940	0.0601	24854881.	-194507.	-0.001686	0.000	2.977E+13	-666	9516	49257.	0.000
23.310	0.0526	23985203.	-197309.	-0.001682	0.000	2.977E+13	-595	1521	50198.	0.000
23.680	0.0452	23103792.	-199786.	-0.001679	0.000	2.978E+13	-520	3689	51138.	0.000
24.050	0.0377	22212121.	-201924.	-0.001675	0.000	2.979E+13	-442	6053	52078.	0.000
24.420	0.0303	21311723.	-203710.	-0.001672	0.000	2.980E+13	-361	8638	53018.	0.000
24.790	0.0229	20404191.	-205130.	-0.001669	0.000	2.981E+13	-278	1465	53959.	0.000
25.160	0.0155	19491175.	-206173.	-0.001666	0.000	2.981E+13	-191	4550	54899.	0.000
25.530	0.008094	18574384.	-206824.	-0.001663	0.000	2.982E+13	-101	7900	55839.	0.000
25.900	0.000716	17655585.	-207070.	-0.001660	0.000	2.983E+13	-9	1521	56779.	0.000

shear force at pile head  
 applied moment at pile head  
 axial thrust at pile head

F White West ( )-FL, 770

1150.4 lbs  
 384.1400.4 in-lb  
 1100.4 lbs

Def	Y	Height	Area	Shear Force	Moment	axial Thrust	Stress	Strain	Total Strain	Bracing	Rolling	Sc	Spr.	Dist
	inches	in	sq in	lbs	in-lb	lbs	psi	psi	psi	in	in	in	in	in
1	0.00	0.566	384.1	231.1	-0	32033	0	30	2	54E+1	€	100	0.04	0.000
2	0.370	0.566	385.1	231.1	-0	32027	0	30	2	54E+1	€	100	0.04	0.000
3	0.740	0.551	386.1	231.1	-0	32022	0	30	2	54E+1	€	100	0.04	0.000
4	1.110	0.544	387.1	231.1	-0	32016	0	30	2	54E+1	€	100	0.04	0.000
5	1.480	0.531	389.1	231.1	-0	32010	0	30	2	54E+1	€	100	0.04	0.000
6	1.850	0.524	390.1	231.1	-0	32004	0	30	2	54E+1	€	100	0.04	0.000
7	2.220	0.511	391.1	231.1	-0	31998	0	30	2	54E+1	€	100	0.04	0.000
8	2.590	0.507	392.1	231.1	-0	31992	0	30	2	53E+1	€	100	0.04	0.000
9	2.960	0.494	393.1	231.1	-0	31986	0	30	2	53E+1	€	100	0.04	0.000
10	3.330	0.481	394.1	231.1	-0	31981	0	30	2	53E+1	-19	223	75.28	0.000
11	3.700	0.481	395.1	229.1	-0	31975	0	30	2	53E+1	-40	314	71.82	0.000
12	4.070	0.471	396.1	227.1	-0	31969	0	30	2	53E+1	-60	364	58.36	0.000
13	4.440	0.461	397.1	224.1	-0	31963	0	30	2	53E+1	-79	310	54.90	0.000
14	4.810	0.451	398.1	220.1	-0	31957	0	30	2	53E+1	-98	386	51.43	0.000
15	5.180	0.441	399.1	215.1	-0	31951	0	30	2	53E+1	-116	528	57.97	0.000
16	5.550	0.431	400.1	210.1	-0	31945	0	30	2	53E+1	-133	371	54.51	0.000
17	5.920	0.421	401.1	203.1	-0	31939	0	30	2	53E+1	-149	350	51.04	0.000
18	6.290	0.411	402.1	196.1	-0	31933	0	30	2	52E+1	-165	302	47.58	0.000
19	6.660	0.41	402.1	189.1	-0	31927	0	30	2	52E+1	-180	161	44.12	0.000
20	7.030	0.401	403.1	180.1	-0	31921	0	30	2	52E+1	-194	364	40.66	0.000
21	7.400	0.391	404.1	171.1	-0	31915	0	30	2	52E+1	-207	546	37.19	0.000
22	7.770	0.381	405.1	162.1	-0	31909	0	30	2	52E+1	-220	544	33.73	0.000
23	8.140	0.371	405.1	152.1	-0	31903	0	30	2	52E+1	-232	393	30.27	0.000
24	8.510	0.361	406.1	141.1	-0	31896	0	30	2	52E+1	-243	330	26.80	0.000
25	8.880	0.361	407.1	130.1	-0	31890	0	30	2	52E+1	-253	990	23.34	0.000
26	9.250	0.351	407.1	119.1	-0	31884	0	30	2	52E+1	-263	911	19.88	0.000
27	9.620	0.341	408.1	704.5	-0	31878	0	30	2	52E+1	-1193	988	15.41	0.000
28	9.990	0.331	408.1	313.6	-0	31872	0	30	2	52E+1	-1235	393	16.35	0.000
29	10.360	0.321	408.1	256.7	-0	31866	0	30	2	52E+1	-1273	597	17.29	0.000
30	10.730	0.311	408.1	991.5	-0	31860	0	30	2	52E+1	-1309	772	18.23	0.000
31	11.100	0.311	407.1	-138.1	-0	31854	0	30	2	52E+1	-1341	892	19.17	0.000



1	3.100	18,137.91	0.130000
2	19.800	18,159.73	0.130000

Note: The values in moment capacity in the table above are for factor of safety reduction factor (hi-factor).

In ACI 318-08 the value of the strength reduction factor  $\phi$  is 0.75 for spiral reinforcement (0.7).

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity according to ACI 318-08, section 9.3.2 for the value required by the design standard being followed.

The following table presents factored moment capacity and corresponding tendon stresses computed for common systems for use for tendon design.

Area Load No.	Rectangular Section	Normal Capacity kip	Ultimate Factored Moment (kip-ft)	Ultimate Factor of Safety	Tendon Stress (ksi)
1	0.65	1,007.9	4,265	11.05.13	4596 (1803.19)
2	0.65	1,859.7	5,020	11.58.82	4616 (1911.13)
1	0.70	1,007.9	4,670	12.05.53	4576 (15834.14)
2	0.70	1,859.7	6,560	12.01.80	4595 (12541.17)
1	0.75	1,007.9	5,075	13.05.93	4454 (18708.14)
2	0.75	1,859.7	6,100	13.44.75	4486 (12620.14)

Computed values for lateral loading and base number 1

Primary	f Res	fts fo	Nomin	(Unf	tored	Momen	Capac	y for	ectici	1	7 Whi West (B1)-f 1p7o							
											0.000	0.000	0.000	0.000				
0.000	7	1725	..	2	152711	0.	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
0.000	7	1734	1.	2	162415	1.	0.001	0.009	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006
0.000	7	1735	1.	2	174865	1.	0.001	0.025	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006
0.000	7	1744	1.	2	173200	1.	0.001	0.012	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006	0.006
0.000	7	1745	1.	2	195521	1.	0.001	0.051	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007
0.000	7	1755	1.	2	140088	1.	0.001	0.030	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007
0.000	7	1756	1.	2	104783	1.	0.001	0.091	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007
0.000	7	1765	1.	2	108705	1.	0.001	0.039	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007
0.000	7	1766	1.	2	157709	1.	0.001	0.034	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007	0.007
0.000	7	1766	1.	2	128278	1.	0.001	0.039	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008
0.000	7	1776	1.	2	104700	1.	0.001	0.045	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008
0.000	7	1777	1.	2	154200	1.	0.001	0.052	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008
0.000	7	1777	1.	2	193733	1.	0.001	0.050	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008
0.000	7	1781	1.	2	158961	1.	0.001	0.059	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008	0.008
0.000	7	1781	1.	2	153681	1.	0.001	0.080	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009
0.000	7	1781	1.	2	22583	1.	0.002	0.091	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009
0.000	7	1781	1.	2	32357	1.	0.002	0.100	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009
0.000	7	1791	1.	2	53207	1.	0.002	0.110	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009
0.000	7	1791	1.	2	34757	1.	0.002	0.117	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009	0.009
0.000	7	1791	1.	2	82554	1.	0.002	0.058	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010	0.010
0.000	7	1801	1.	2	56372	1.	0.002	0.098	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011
0.000	7	1811	1.	2	48515	1.	0.002	0.118	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013	0.013
0.000	7	1811	1.	2	56707	1.	0.002	0.166	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014	0.014
0.000	7	1821	1.	2	77568	1.	0.003	0.041	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015	0.015
0.000	7	1821	1.	2	107910	1.	0.003	0.057	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016
0.000	7	1831	1.	2	46455	1.	0.003	0.073	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017	0.017
0.000	7	1831	1.	2	92051	1.	0.003	0.003	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018	0.018
0.000	7	1831	1.	2	43544	1.	0.003	0.053	0.019	0.019	0.019	0.019	0.019	0.019	0.019	0.019	0.019	0.019
0.000	7	1831	1.	2	99485	1.	0.003	0.112	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020

Maximum level of moment at maximum deflection  
 Minimum level of moment at maximum deflection  
 Load Factor  
 Wind Speed  
 Exposure Category  
 Section Modulus  
 Safety Factor  
 Plate Thickness  
 Corrosion Allowance  
 Slab Thickness  
 Pa 18





Branding	Branding	Branding	D. th to	M. Comp	M. Tens	Max oncrete	Ma Steel	Run			
Structure	Moment	St fness	Axis	rain	rain	ress	ress	Msg			
in/in.	in kip	k-in2	in	/in	/in	si	si				
0.00107	78338	165	05223	15	5818	00206	009577	39891	-6	30000	CY
0.00109	78536	162	55935	15	18774	00209	309751	39135	-6	30000	CY
0.00121	79621	147	123097	15	00300	00227	31087	39470	6	30000	CY
0.00133	80536	134	56455	15	00306	00246	31198	39369	6	30000	CY
0.00145	81092	124	185794	15	13454	00264	31309	38666	6	30000	CY
0.00157	81598	115	174251	15	12482	00282	31420	39864	6	30000	CY
0.00166	82046	107	198396	15	75156	00301	31531	39531	6	30000	CY
0.00181	82304	106	149832	15	57440	00319	31643	38996	6	30000	CY
0.00192	182494	94	104898	15	42152	00337	31754	99276	6	30000	CY
0.00206	18266	81	18866	15	29308	00355	31866	99836	6	30000	CY
0.00217	18280	8	32674	15	18866	00374	31977	97720	6	30000	CY
0.00225	18280	7	57768	15	22823	00395	32085	99844	6	30000	CY

axial trust force = 90 300 k 5

Branding	Branding	Branding	D. th to	M. Comp	M. Tens	Max oncrete	Ma Steel	Run		
Structure	Moment	St fness	Axis	rain	rain	ress	ress	Msg		
in/in.	in kip	k-in2	in	/in	/in	si	si			
0.00002	748	35307	299	41230	6	98295	00001	06593	45271	C
0.00005	1493	298	50762	5	50443	00002	00002	12197	84048	C
0.00007	2234	297	126076	5	01168	00004	00003	17761	22825	C
0.00010	2972	297	52420	5	26535	00005	00005	23284	61603	C
0.00012	3707	296	63129	5	81758	00006	00006	28768	00381	C
0.00015	4438	295	66018	5	51910	00008	00007	34211	39159	C
0.00017	5166	295	64432	5	30592	00009	00009	39614	77937	C
0.00020	5890	294	60048	5	14606	00011	00010	44977	16715	C
0.00022	5890	261	08932	2	21159	00006	00017	25976	17095	C
0.00025	5890	235	08039	2	81041	00006	00026	28392	77455	C
0.00027	5890	214	07308	2	48067	00007	00022	30804	37835	C
0.00030	5890	196	06699	2	19657	00008	00024	33197	98291	C
0.00032	5890	181	67722	2	95713	00008	00026	35584	58735	C
0.00035	5890	168	05742	2	75279	00005	00028	37964	19178	C
0.00037	5890	157	05359	2	57653	00005	00034	40337	79607	C
0.00040	5890	147	30024	2	42133	00014	00032	42705	40044	C
0.00042	5890	138	04728	2	28026	00013	00034	45051	00544	C
0.00045	5890	130	04466	2	15558	00011	00036	47394	61021	C
0.00047	5890	124	09494	2	04471	00017	00038	49734	21495	C
0.00050	5890	117	04015	2	94556	00017	00041	52064	81956	C









Concrete Properties:

Compressive Strength of Concrete = 4000.0000 psi  
 Modulus of Elasticity of Concrete = 3504997. psi  
 Modulus of Rupture of Concrete = -474.34154 psi  
 Compression Strain at Peak Stress = 0.00189  
 Tensile Strain at Fracture of Concrete = -0.0001154  
 Maximum Coarse Aggregate Size = 0.75000 in

Number of Axial Thrust Force Values: Determined from file-head loadings = 2

Number	Axial Thrust Force kips
1	68.100
2	90.800

Definitions of Run Messages and Notes:

C = concrete in section has cracked in tension.  
 Y = stress in reinforcing steel has reached yield stress.  
 T = ACI 318-08 criteria for tension-controlled section met, tensile strain in reinforcement exceeds 0.005 while simultaneously compressive strain in concrete more than 0.003. See ACI 318-08, Section 10.3.4.  
 Z = depth of tensile zone in concrete section is less than 10 percent of section depth.  
 Bending Stiffness (EI) = Computed Bending Moment / Curvature.  
 Position of neutral axis is measured from edge of compression side of pile.  
 Compressive stresses and strains are positive in sign.  
 Tensile stresses and strains are negative in sign.

Axial Thrust Force = 68.100 kips

				F	Write	Size	(1)	FL
23	1.27000	1.27000	1.27000	17.066	11	21.37	57	
24	1.27000	1.27000	1.27000	13.646	11	19.91	55	
25	1.27000	1.27000	1.27000	11.689	11	19.17	54	
26	1.27000	1.27000	1.27000	13.169	11	19.23	57	
27	1.27000	1.27000	1.27000	11.065	11	5.15	46	
28	1.27000	1.27000	1.27000	11.365	11	9.00	46	
29	1.27000	1.27000	1.27000	11.065	11	5.15	46	
30	1.27000	1.27000	1.27000	13.169	11	19.23	57	
31	1.27000	1.27000	1.27000	11.689	11	15.17	54	
32	1.27000	1.27000	1.27000	11.689	11	15.17	54	
33	1.27000	1.27000	1.27000	11.646	11	19.91	55	
34	1.27000	1.27000	1.27000	17.066	11	21.37	57	
35	1.27000	1.27000	1.27000	11.985	11	18.51	52	
36	1.27000	1.27000	1.27000	11.445	11	12.26	53	
37	1.27000	1.27000	1.27000	11.492	11	13.58	54	
38	1.27000	1.27000	1.27000	11.182	11	18.42	52	
39	1.27000	1.27000	1.27000	17.572	11	20.73	57	
40	1.27000	1.27000	1.27000	11.724	11	12.56	54	
41	1.27000	1.27000	1.27000	11.703	11	13.65	54	
42	1.27000	1.27000	1.27000	11.579	11	14.28	54	
43	1.27000	1.27000	1.27000	11.703	11	13.65	54	
44	1.27000	1.27000	1.27000	11.724	11	12.56	54	
45	1.27000	1.27000	1.27000	11.572	11	14.07	54	
46	1.27000	1.27000	1.27000	11.182	11	18.42	52	
47	1.27000	1.27000	1.27000	11.492	11	13.58	54	
48	1.27000	1.27000	1.27000	11.445	11	12.26	53	
49	1.27000	1.27000	1.27000	11.985	11	18.51	52	
50	1.27000	1.27000	1.27000	11.066	11	14.37	53	
51	1.27000	1.27000	1.27000	11.646	11	19.91	55	
52	1.27000	1.27000	1.27000	11.689	11	19.17	54	
53	1.27000	1.27000	1.27000	11.169	11	19.23	57	
54	1.27000	1.27000	1.27000	11.065	11	5.15	46	

NOTE: The positions of the above letters will correspond to the following:

Minimum spacing between any two bars is not less than 1/16 inch (1.5625 mm) and 47

Spacing to aggregate size is 5.0 mm

Edge-to-Edge Bar Spacing  
 Maximum Concrete Aggregate Size  
 Ratio of Bar Spacing to Aggregate Size  
 Offset of Center of Rebar Cap from Center of Pier

Axial Structural Capacities

Nom. Axial Structural Capacity:  $P_n = 0.85 f_c' A_g \rho_s F_y$   
 Tensile Load for Cracking of Concrete:  $T_c$   
 Nominal Axial Tensile Capacity:  $T_n$

Reinforcing Bar Dimensions and Positions

Bar Number	Bar Diam. inches	Bar Area sq. in.	K inches	Y inches
1	1.27000	1.2700	14.365	0.00
2	1.27000	1.2700	14.365	5.15
3	1.27000	1.2700	13.169	10.23
4	1.27000	1.2700	11.689	15.17
5	1.27000	1.2700	13.646	19.91
6	1.27000	1.2700	17.066	24.37
7	1.27000	1.2700	13.985	28.51
8	1.27000	1.2700	10.445	32.26
9	1.27000	1.2700	16.492	35.58
10	1.27000	1.2700	12.182	38.42
11	1.27000	1.2700	17.572	40.73
12	1.27000	1.2700	12.724	42.56
13	1.27000	1.2700	17.703	43.65
14	1.27000	1.2700	2.575	44.28
15	1.27000	1.2700	12.575	44.28
16	1.27000	1.2700	17.703	43.65
17	1.27000	1.2700	12.724	42.56
18	1.27000	1.2700	17.572	40.73
19	1.27000	1.2700	12.182	38.42
20	1.27000	1.2700	16.492	35.58
21	1.27000	1.2700	10.445	32.26
22	1.27000	1.2700	12.182	38.42

Edge

Number of loads specified = 3

Ft White West (B1)-FL.1p70

Load No.	Load Type	Condition 1	Condition 2	Axial Thrust Force, lbs	Compute Top y vs. Pile Length
1	1	V = 23150. lbs	M = 38489400. in-lbs	68100.	Yes
2	1	V = 46300. lbs	M = 76978800. in-lbs	68100.	Yes
3	1	V = 61733. lbs	M = 102638400. in-lbs	90800.	Yes

V = perpendicular shear force applied to pile head  
M = bending moment applied to pile head  
y = lateral deflection relative to pile axis  
S = pile slope relative to original pile batter angle  
R = rotational stiffness applied to pile head  
Axial thrust is assumed to be acting axially for all pile batter angles.

-----  
Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness  
-----

Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

Pile Section No. 1:  
-----

-----  
Dimensions and Properties of Drilled Shaft (Bored Pile):  
-----

Length of Section = 37.00000 ft  
Shaft Diameter = 108.00000 in  
Concrete Cover Thickness = 9.00000 in  
Number of Reinforcing Bars = 54 bars  
Yield Stress of Reinforcing Bars = 60000. psi  
Modulus of Elasticity of Reinforcing Bars = 29000000. psi  
Gross Area of Shaft = 9160.88418 sq. in.  
Total Area of Reinforcing Steel = 68.58000 sq. in.  
Area Ratio of Steel Reinforcement = 0.75 percent

Ft White West (B1)-FL.1p70

1	Sand (Reese, et al.)	3.000	95.000	--	27.000	--	default
		9.500	95.000	--	27.000	--	default
2	Sand (Reese, et al.)	9.500	110.000	--	30.000	--	default
		29.500	110.000	--	30.000	--	default
3	Stiff Clay w/o Free Water	29.500	110.000	1100.000	--	default	--
		34.500	110.000	1100.000	--	default	--
4	Stiff Clay w/o Free Water	34.500	110.000	1300.000	--	default	--
		39.500	110.000	1300.000	--	default	--
5	Stiff Clay w/o Free Water	39.500	37.600	700.000	--	default	--
		43.500	37.600	700.000	--	default	--
6	Sand (Reese, et al.)	43.500	52.600	--	35.000	--	default
		51.000	52.600	--	35.000	--	default

Loading Type

Static loading criteria were used when computing p-y curves for all analyses.

Pile-head Loading and Pile-head Fixity Conditions

Ft White West (B1)-FL.1p70

Distance from top of pile to top of layer = 39.50000 ft  
 Distance from top of pile to bottom of layer = 43.50000 ft  
 Effective unit weight at top of layer = 37.60000 pcf  
 Effective unit weight at bottom of layer = 37.60000 pcf  
 Undrained cohesion at top of layer = 700.00000 psf  
 Undrained cohesion at bottom of layer = 700.00000 psf  
 Epsilon-50 at top of layer = 0.0000  
 Epsilon-50 at bottom of layer = 0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 6 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 43.50000 ft  
 Distance from top of pile to bottom of layer = 51.00000 ft  
 Effective unit weight at top of layer = 52.60000 pcf  
 Effective unit weight at bottom of layer = 52.60000 pcf  
 Friction angle at top of layer = 35.00000 deg.  
 Friction angle at bottom of layer = 35.00000 deg.  
 Subgrade k at top of layer = 0.0000 pci  
 Subgrade k at bottom of layer = 0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.

(Depth of lowest soil layer extends 14.00 ft below pile tip)

Summary of Soil Properties

Layer	Layer	Layer	Effective	Undrained	Angle of	Strain
Layer	Soil Type	Depth	Unit Wt.	Cohesion	Friction	Factor
Num.	(p-y Curve Criteria)	ft	pcf	psf	deg.	Epsilon 50
						pci

Ft White West (B1)-Fl. 1p70

Layer 2 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of Layer	=	9.50000 ft
Distance from top of pile to bottom of Layer	=	29.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Friction angle at top of Layer	=	30.00000 deg.
Friction angle at bottom of Layer	=	30.00000 deg.
Subgrade k at top of Layer	=	0.0000 pci
Subgrade k at bottom of Layer	=	0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.

Layer 3 is stiff clay without free water

Distance from top of pile to top of Layer	=	29.50000 ft
Distance from top of pile to bottom of Layer	=	34.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Undrained cohesion at top of Layer	=	1100.00000 psf
Undrained cohesion at bottom of Layer	=	1100.00000 psf
Epsilon-50 at top of Layer	=	0.0000
Epsilon-50 at bottom of Layer	=	0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 4 is stiff clay without free water

Distance from top of pile to top of Layer	=	34.50000 ft
Distance from top of pile to bottom of Layer	=	39.50000 ft
Effective unit weight at top of Layer	=	110.00000 pcf
Effective unit weight at bottom of Layer	=	110.00000 pcf
Undrained cohesion at top of Layer	=	1300.00000 psf
Undrained cohesion at bottom of Layer	=	1300.00000 psf
Epsilon-50 at top of Layer	=	0.0000
Epsilon-50 at bottom of Layer	=	0.0000

NOTE: Internal default values for Epsilon-50 will be computed for this soil layer.

Layer 5 is stiff clay without free water

Input Structural Properties:

Pile Section No. 1:

Section Type = Drilled Shaft (Bored Pile)  
 Section Length = 37.00000 ft  
 Section Diameter = 108.00000 in

Ground Slope and Pile Batter Angles

Ground Slope Angle = 0.000 degrees  
 = 0.000 radians  
 Pile Batter Angle = 0.000 degrees  
 = 0.000 radians

Soil and Rock Layering Information

The soil profile is modelled using 6 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 3.00000 ft  
 Distance from top of pile to bottom of layer = 9.50000 ft  
 Effective unit weight at top of layer = 95.00000 pcf  
 Effective unit weight at bottom of layer = 95.00000 pcf  
 Friction angle at top of layer = 27.00000 deg.  
 Friction angle at bottom of layer = 27.00000 deg.  
 Subgrade k at top of layer = 0.0000 pci  
 Subgrade k at bottom of layer = 0.0000 pci

NOTE: Internal default values for subgrade k will be computed for this soil layer.



Ft White West (B1)-Fl.1p7o

- Use unfactored loads in computations (conventional analysis)
- Compute pile response under loading and nonlinear bending properties of pile (only if nonlinear pile properties are input)
- Use of p-y modification factors for p-y curves not selected
- Loading by lateral soil movements acting on pile not selected
- Input of shear resistance at the pile tip not selected
- Computation of pile-head foundation stiffness matrix not selected
- Push-over analysis of pile not selected
- Buckling analysis of pile not selected

Output Options:

- No p-y curves to be computed and reported for user-specified depths
- Values of pile-head deflection, bending moment, shear force, and soil reaction are printed for full length of pile.
- Printing Increment (nodal spacing of output points) = 1

Pile Structural Properties and Geometry

Total number of pile sections = 1

Total length of pile = 37.00 ft

Depth of ground surface below top of pile = 3.00 ft

Pile diameter values used for p-y curve computations are defined using 2 points.

p-y curves are computed using pile diameter values interpolated with depth over the length of the pile.

Point	Depth X ft	Pile Diameter in
1	0.00000	108.0000000
2	37.00000	108.0000000

-----  
Problem Title  
-----

Project Name: Ft. White West Site

Location: Columbia County, FL

Type: 195' Monopole

Description: Nello

Engineer: Christopher A. Harris, PE

-----  
Program Options and Settings  
-----

Engineering Units of Input Data and Computations:

- Engineering units are US Customary Units (pounds, feet, inches)

Analysis Control Options:

- Maximum number of iterations allowed = 500
- Deflection tolerance for convergence = 1.0000E-05 in
- Maximum allowable deflection = 100.0000 in
- Number of pile increments = 100

Loading Type and Number of Cycles of Loading:

- Static loading specified

Computational Options:

LPile Plus for Windows, Version 2013-07.005

Analysis of Individual Piles and Drilled Shafts  
Subjected to Lateral Loading Using the p-y Method

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Christopher A. Harris, P.E.  
R.W. Harris, Inc.

Serial Number of Security Device: 228746911

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is forbidden by the software license agreement.

Files Used for Analysis

Path to file locations: C:\Users\Chris Harris\Documents\Ensoft\LPile2013\DATA\2014\  
Name of input data file: Ft White West (B1)-FL.1p7d  
Name of output report file: Ft White West (B1)-FL.1p7o  
Name of plot output file: Ft White West (B1)-FL.1p7p  
Name of runtime message file: Ft White West (B1)-FL.1p7r

Date and Time of Analysis

Date: November 14, 2014 Time: 11:34:33  
Page 1

DESIGN CRITERIA

Table with 3 columns: Basic Wind Speed (3-second gust), Wind Exposure (C), and various design criteria like Roof Live Load, Floor Live Load, etc.

Table with 4 columns: IBC CODE, EDITION, CONST. TYPE, OCCUPANCY/USE GROUP. Lists codes like Florida, IBC Section, etc.

FOR ARCHITECT USE ONLY

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FOR ENGINEER USE ONLY

THIRD PARTY AGENCIES

DATE 1-16-2014 CERT NO. SMP-47

PLAN NUMBER MF27-D-9572

APPROVED BY WILLIAM TEGELER

Signature

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Structural design notes including general design notes, material specifications, and construction details.

STRUCTURAL DESIGN BY RALEY AND ASSOCIATES, INC. 4913 SHED ROAD BOSSIER CITY, LA 71111

\* OR 480psf (0.1) over on area 2'-6" x 2'-6" in equipment room

These prints comply with the Florida Manufactured Building and adopted Codes and all required components shall comply with 9B-72

CONCRETE TYPE: 18; OCCUPANCY: 18; ALLOWABLE NO. OF FLOORS: 1; WIND VELOCITY: 150 mph; ELEVATION OF FINISH FLOOR: 0; PLAN NO.: MF27-D-9572; ALLOW. FLOOR LOAD: 250 PSF; APPROVAL DATE: 1/16/2014; APPROVAL DRAWING MANUFACTURER: Fibrebond

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

THREE PRINTS ARE USED: GENERAL NOTES, FOUNDATIONS, SPECIAL DETAILS. THESE REVIEWS OF THE DRAWINGS ARE FOR THE MANUFACTURER'S USE ONLY. THIS DOCUMENT IS NOT VALID UNLESS SIGNED BY THE MANUFACTURER.

Professional seals for Frank J. Raley, Professional Engineer (No. 87920) and William Tegeler, Professional Engineer (No. 87920). Includes company logo for Fibrebond.

11'-5" X 12'-0" EQUIPMENT SHELTER MANUFACTURED BUILDING DATA SHEET

Table with 6 columns: REV, BY, DATE, APPR, DATE, REVISION. Revision table.

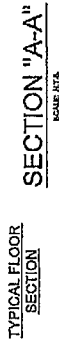
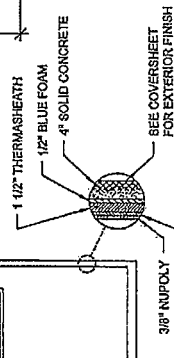
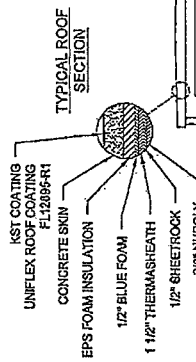
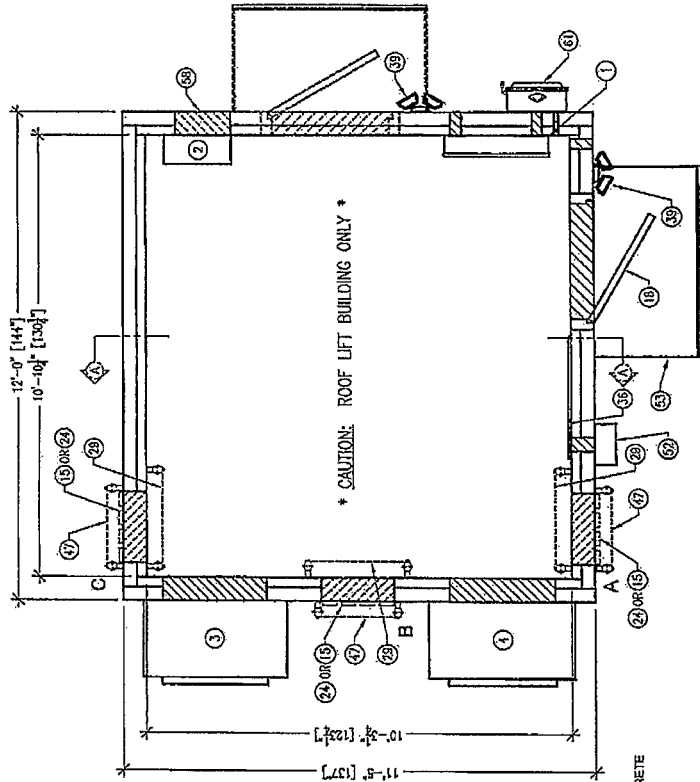
Table with 3 columns: SHEET NO., REV, DESCRIPTION. Drawing sheet information table.

REFERENCED STANDARD DRAWINGS: SHEET NO., REV., DESCRIPTION. List of referenced drawings.

RECEIVED FOR FILE COPY COMPLIANCE PLANS EXAMINER COLUMBIA COUNTY BUILDING DEPARTMENT

SPECIAL CONDITIONS/LIMITATIONS: SEE SHEET 1-2, 1-3 & 1-4 FOR FLOORING. SEE GENERAL DESIGN NOTES 1, 3, 4, 7, 8, 9, 10 & 11. THE USE OF THIS BUILDING IS NOT APPROVED FOR HAZARDOUS MATERIALS IN EXCESS OF RISK. MATERIALS LISTED IN IBC 2010 TABLE 307.1(1)

- NOTE:
1. ALL HELIX WARRIED FIBEROCK SHALL BE INSTALLED, TESTED, REMOVED & PACKED IN BUILDING PRIOR TO SHIPMENT.
  2. ITEM 15, 24, 25 & 47 ARE SHOWN IN ALL OPTIONAL LOCATIONS. SEE BUILDING COVERSHEET FOR SELECTED LOCATIONS.
  3. SECURE WITH LOCKWASHERS AND REDUCING WASHER. CAP OFF PENETRATION. DO NOT GLUE.
  4. CONNECT #2 SOLID TINNED WITH TWO-BOLT HOLE NON-COMPRESSION CORRUG #2 SOLID TINNED WITH TWO-BOLT HOLE NON-COMPRESSION SET. SURED TYPE LUG. TO FLOOR FRAME AND CONVI.
  5. SEE ALSO INTERIOR DETAILS SHEET FOR EXTERIOR GROUND BAR MOUNTING HARDWARE AND DETAIL.
  6. CUT RIGID CAST-IN CONDUITS 10" ABOVE FLOOR. ASSEMBLE CONNECTORS AND SEALTIGHT.



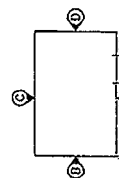
These prints comply with the Florida Man Code and all required components shall comply with SB-72.

APPROVED BY

**NIA INC.**

Approval of this document does not authorize or approve any deviation or modification from the requirements of applicable State Laws.

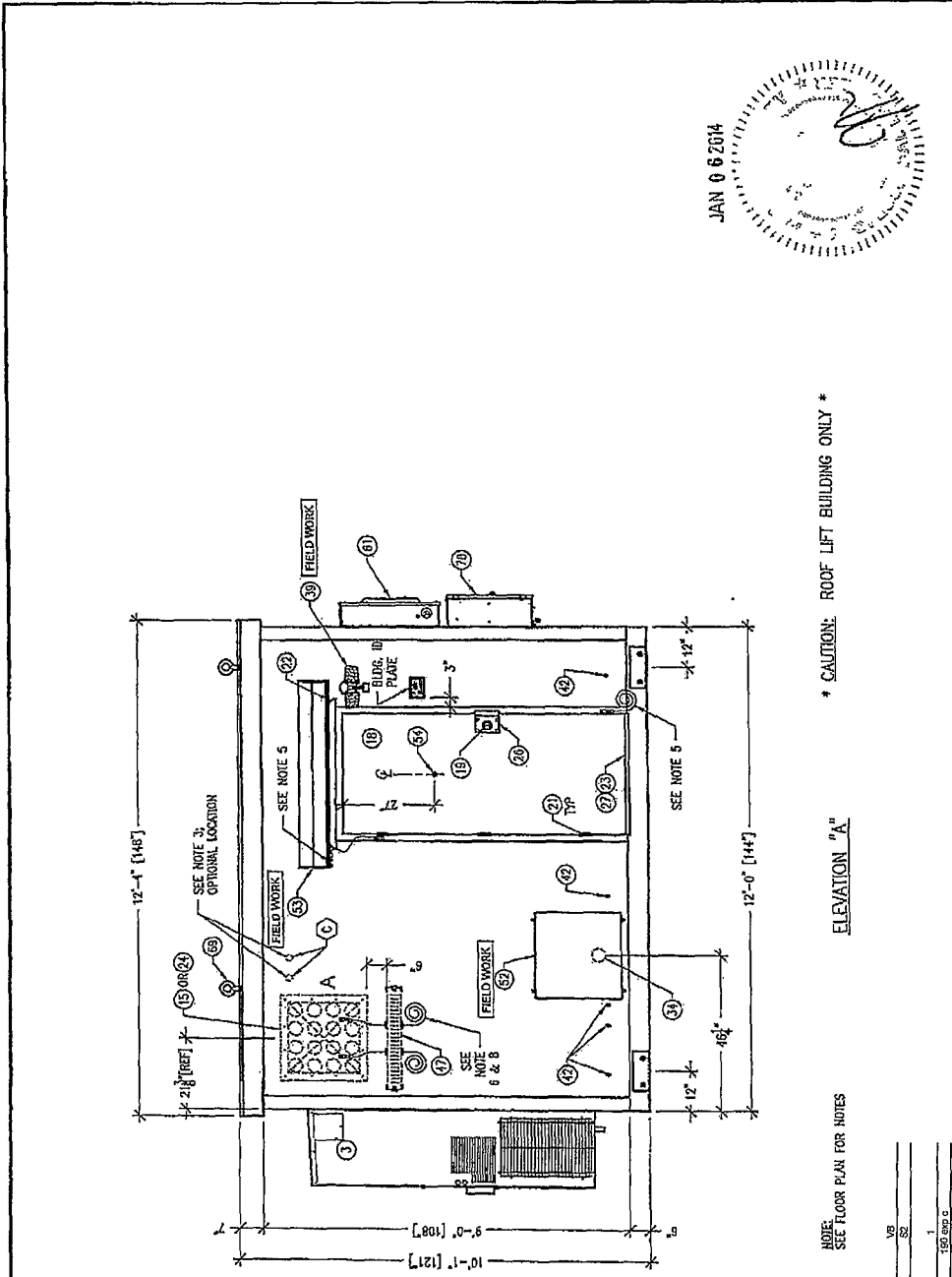
Const. Type	VB
Occupancy	S2
Wind Velocity	1
Fire Rating of Ext. Wall	150 min. C
Ext. Wall	0
Plan No.	MF77-D-9577
Allow. Floor Load	250 PSF
Manufact. Date	1/16/2014
Manufacturer	Fiberbond



JAN 06 2014



<p><b>FIBERBOND</b></p>	
1800 DAVENPORT DRIVE MINDEN, LA 71055 ph. (800) 924-2814 www.fiberbond.com	
<b>AT&amp;T MOBILITY</b> <b>11'-5" X 12'-0" EQUIPMENT SHELTER</b> FLOOR PLAN	
DATE	1/16/2014
SCALE	3/8" = 1'-0"
REV.	1-1
BY	REB
CHKD.	REB
APP.	REB
DATE	1/16/2014
REASON	
REV.	BY
DATE	REASON



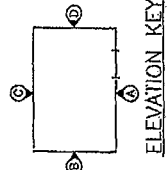
\* CAUTION: ROOF LIFT BUILDING ONLY \*

ELEVATION "A"

NOTE: SEE FLOOR PLAN FOR NOTES

Const. Type: VB  
 Occupancy: S2  
 Allowable No. of Floors: 1  
 Area: 350 SQ. FT.  
 Fire Rating of: 0  
 Plan No.: MFT27-D-5577  
 Allow. Floor' Load: 250 PSF  
 Approval Date: 11/19/2014  
 Manufacturer: FibreBond  
 Approved by: \_\_\_\_\_  
 from the requirements of applicable State Laws.

APPROVED BY  
**NIA INC.**  
 APPROVED BY: \_\_\_\_\_  
 DATE: \_\_\_\_\_



ELEVATION KEY

REV.	BY	DATE	APP.	DATE	ISS.	DATE	REVISION

1850 DANFORTH DRIVE MINNEN, LA. 71055  
 ph. (504) 624-2914 www.fibrebond.com

**FI** FIBREBOND

AT&T MOBILITY

11'-5" X 12'-0" EQUIPMENT SHELTER

EXTERIOR ELEVATION "A"

DATE: 11/19/14  
 SCALE: 1/8"=1'-0"  
 SHEET NO. 1-2  
 DRAWING NO. D-9577

These prints comply with the Florida Manufactured Building Act and adopted Codes and all other applicable codes and comply with 88-32.

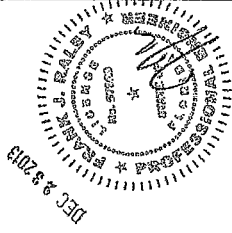
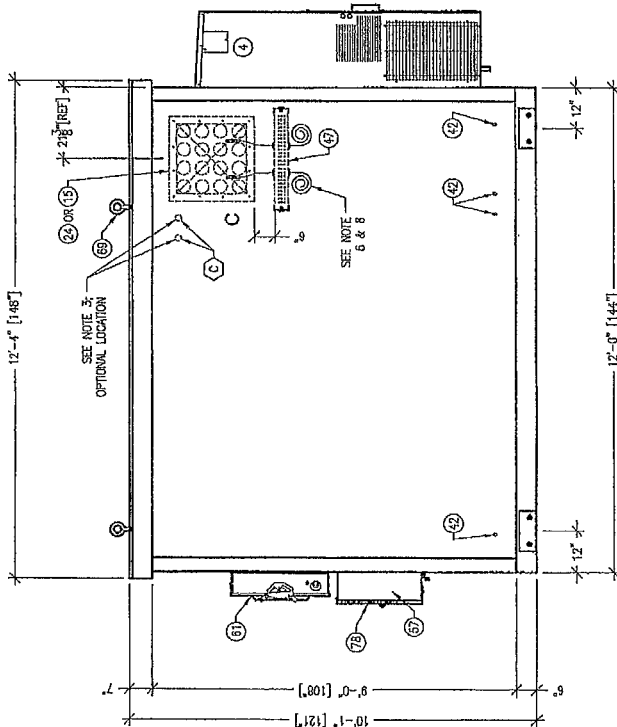
APPROVED BY



Approval of this document does not authorize anyone to approve any deviation or variations from the requirements of applicable State Laws.

Const. Type:	VB
Occupancy:	S2
Allowable No. of Units:	1
Wind Speed:	130 mph G
Fire Rating of Ext. Walls:	0
Plan No.:	MF127-D-2577
Allow. Floor Load:	250 PSF
Approval Date:	7/19/2014
Manufacturer:	Fiberglass

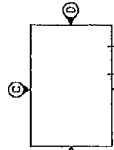
NOTE: SEE FLOOR PLAN FOR NOTES



1800 DAMENPORT DRIVE MINNEN LA, 71065 P.O. BOX 9247-2614 www.fiberglass.com	FIBERGLASS
AT&T MOBILITY	REV: 1-3
11'-5" X 12'-0" EQUIPMENT SHELTER	DATE: 3/8/14
EXTERIOR ELEVATION "C"	PROJ: B D-9577
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:
REV. BY:	DATE:

PLEASE READ THE LEGEND AND GENERAL NOTES AND SPECIFICATIONS. ANY USE OF THIS DRAWING FOR ANY PURPOSE OTHER THAN THAT FOR WHICH IT WAS DESIGNED OR FOR WHICH IT IS SPECIFICALLY APPROVED FOR EACH PROJECT BY THE UNDERSIGNED IS AT THE USER'S SOLE RISK. THE USER ASSUMES ALL LIABILITY FOR ANY DAMAGE OR INJURY TO PERSONS OR PROPERTY OF ANY KIND WHICH MAY OCCUR AS A RESULT OF THE USER'S USE OF THIS DRAWING FOR ANY PURPOSE OTHER THAN THAT FOR WHICH IT WAS SPECIFICALLY APPROVED FOR EACH PROJECT BY THE UNDERSIGNED.

ELEVATION KEY

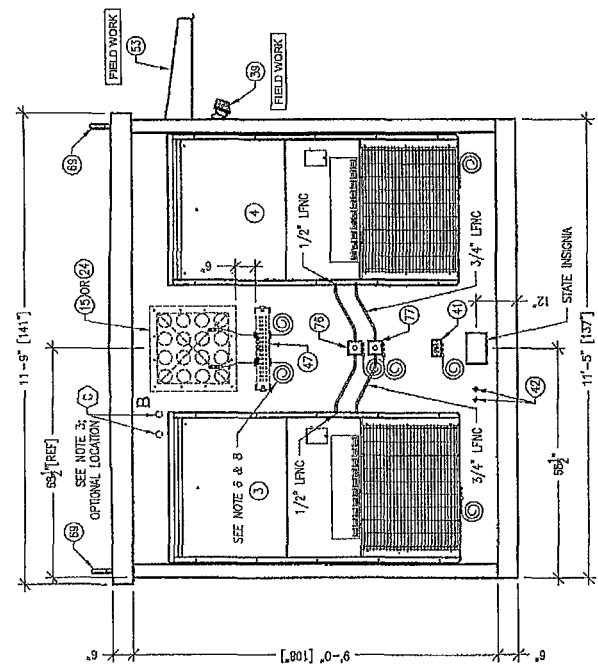


TRANSFORMING ENERGY WITH US  
 Florida Manufactured Building  
 Act and adopted Codes and  
 all required components shall  
 comply with 9B-72



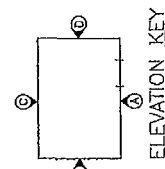
Approval of this document does not authorize or approve any deviation or deviations  
 from the requirements of applicable State Laws.

Contract Type: VB  
 Code: SE  
 Allowable No.: 1  
 Wind Velocity: 150 mph c  
 Fire Rating: 0  
 Plan No.: MF 17-05-9577  
 Approval Date: 4/16/2014  
 Manufacturer: FibreBond

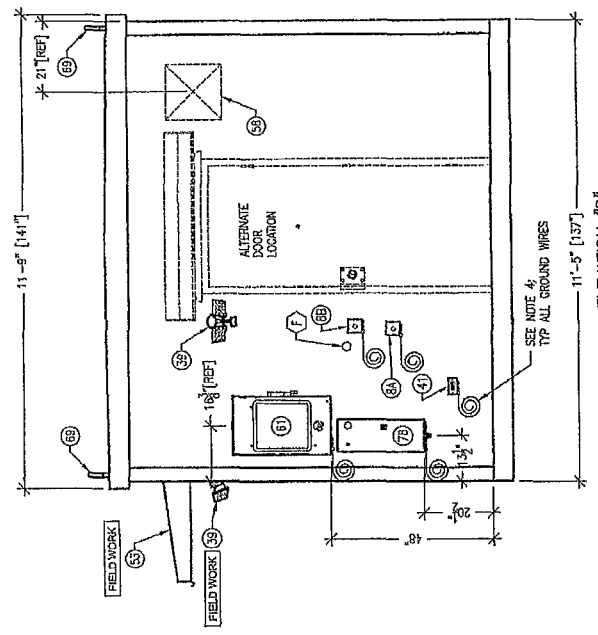


ELEVATION "B"

NOTE:  
 SEE FLOOR PLAN FOR NOTES

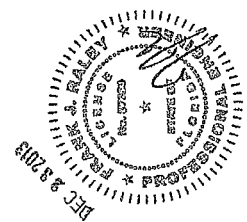


ELEVATION KEY



ELEVATION "D"

\* CAUTION: ROOF LIFT BUILDING ONLY \*



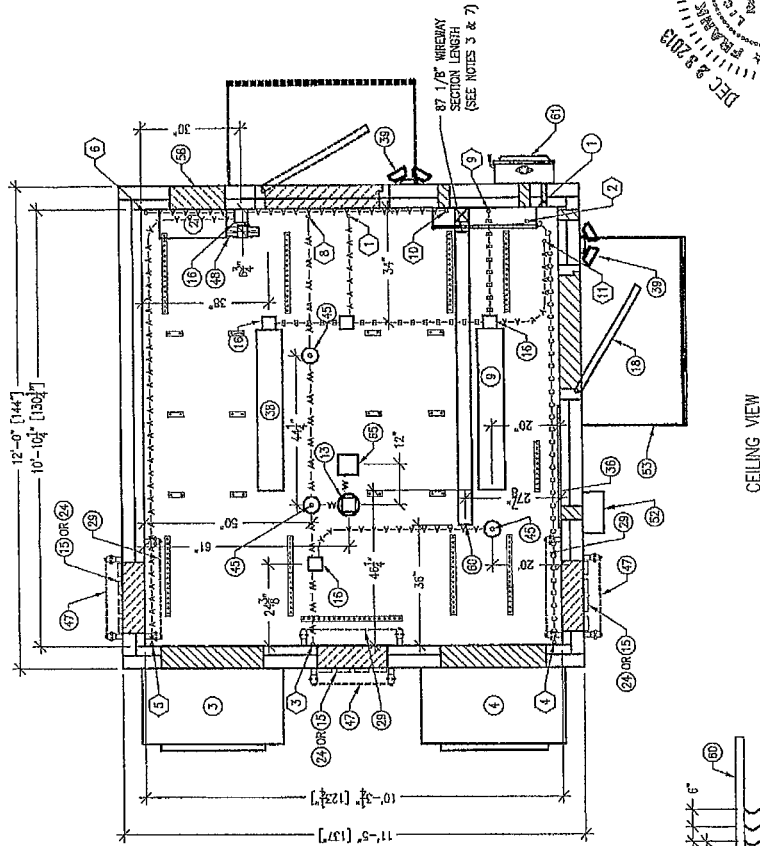
1320 BAYVIEW DRIVE, MANDALAY, LA 70305  
 ph: (809) 822-2914 www.fibrebond.com

AT&T MOBILITY  
 11'-5" X 12'-0" EQUIPMENT SHELTER  
 EXTERIOR ELEVATIONS "B" & "D"

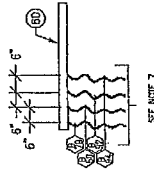
REV.	BY	DATE	REVISION

FIBREBOND  
 3/8" x 1/4" x 1/4"  
 B Dec D-9577





CEILING VIEW



- NOTES:**
1. ALL MEASUREMENTS SHOWN ARE ACQUIRE WITHIN ±1/4" ALL ADJACENT BOXES SHALL BE THE SAME DIMENSION.
  2. THE LOCATIONS OF # 4, # 4, # 4 AND # 11/16" X 4 11/16" JUNCTION BOXES AND SMALLER IF THIS CAN BE ADVISED AS REQUIRED TO THESE COMPONENTS WILL REQUIRE THEM TO BE CUT.
  3. MOUNT ON UNRESTRICT.
  4. CONDUIT SHOULD BE INSTALLED AS SHOWN IF POSSIBLE. ANY CHANGES TO 80° BENDS, OFFSETS AND SADDLES MAY BE DONE WHEN NEEDED AND SHOULD BE UNDER THE GUIDANCE OF A QUALIFIED SENIOR CREW LEADER. ANY CHANGE MUST BE WITHIN THE GUIDELINES OF STANDARD OPERATING PROCEDURE FOR GENERAL CONDUIT RULES.
  5. ALL CONDUIT TO HAVE A MINIMUM OF 3/4" BETWEEN THEM.
  6. ALL CONDUIT TO BE 1/2" UNLESS OTHERWISE NOTED.
  7. PULL 8'-0" OF FLEXIBLE METALLIC CONDUIT TO FLOOR. CONNECT TO REAR WITH METALLIC CONDUIT. THINWALL 90° CONNECTOR TO EACH REAR WITH METALLIC CONDUIT.
  8. FEELS 15, 24, 28, 37, 47 AND 58 ARE SHOWN IN ALL OPTIONAL LOCATIONS. INSTALL ONLY AT INSTALLED WAGESHEET, LOCATE AS SHOWN ON ELEVATION VIEW. SEE BUILDING COVERSHEET FOR SELECTED LOCATIONS.
  9. REMOVED NOTE.
  10. ALL LIGHT SWITCHES TO BE MOUNTED AT 54" AFF UNLESS NOTED OTHERWISE.
  11. ALL RECEPTACLES TO BE MOUNTED AT 18" AFF UNLESS NOTED OTHERWISE.
  12. ALL RECEPTACLES SHALL BE EMT UNLESS OTHERWISE NOTED.
  13. ALL CONDUIT THROUGH A WALL SHALL BE RIGID UNLESS OTHERWISE NOTED.

A/C	CONDUIT KEY	PVC
AA	1 1/2"	XB = 1 1/2"
AB	1 1/2"	XC = 1"
AC	1 1/2"	XD = 1 1/4"
AD	1 1/2"	XE = 1 1/2"
AE	1 1/2"	XF = 2"
AF	2"	YG = 2 1/2"
AG	2 1/2"	XH = 3"
AH	3"	XI = 3 1/2"
AI	3 1/2"	XJ = 4"
AJ	4"	

OOOOOO = FLEXIBLE NONMETALLIC CONDUIT

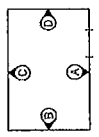
These prints comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 9B-72

APPROVED BY

**NTA INC.**

Approval Date: 11/05/2014  
Manufacturer: ETC

Approval of this document does not constitute or approve any deviation or deviations from the requirements of applicable State Laws.



ELEVATION KEY

1500 DANFORTH DRIVE MANDEN, LA 71055  
PH: (800) 626-5514 www.Fibribond.com

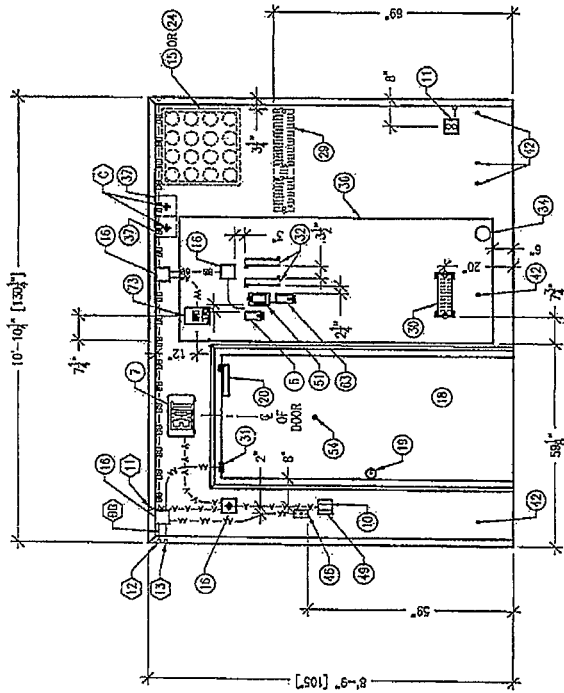
**AT&T MOBILITY**  
**FIBRIBOND**

11'-5" X 12'-0" EQUIPMENT SHELTER  
INTERIOR CEILING VIEW

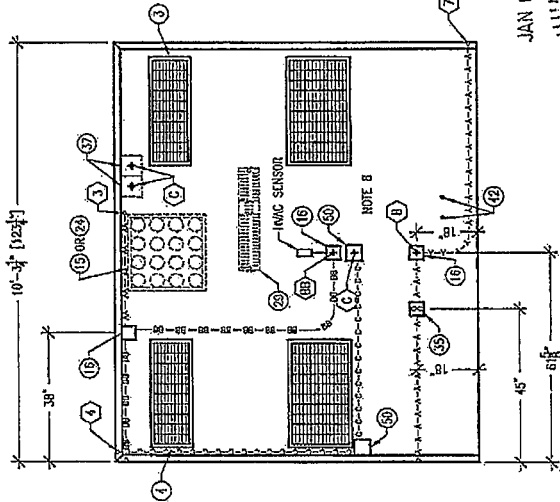
DATE: 11/05/14  
REV: 2-1  
B Rev D-9577

REV.	BY	DATE	REVISION

THESE PRINTS ARE THE PROPERTY OF FIBRIBOND AND ARE TO BE USED ONLY FOR THE PROJECT AND SITE SPECIFICALLY IDENTIFIED HEREON. ANY USE OF THESE PRINTS FOR ANY OTHER PROJECT OR SITE WITHOUT THE WRITTEN PERMISSION OF FIBRIBOND IS STRICTLY PROHIBITED. THESE PRINTS ARE TO BE USED ONLY FOR THE PROJECT AND SITE IDENTIFIED HEREON. ANY USE OF THESE PRINTS FOR ANY OTHER PROJECT OR SITE WITHOUT THE WRITTEN PERMISSION OF FIBRIBOND IS STRICTLY PROHIBITED.



ELEVATION "A"



ELEVATION "B"

JAN 0 6 2014



Const. Type:	VB
Occupancy:	SZ
Allowable No. of Floors:	1
Wind Velocity:	150 MBG 5
Floor Rating:	0
Plan No.:	MF727-D-9577
Allow. Floor Load:	250 PSF
Approval Date:	1/16/2014
Manufacturer:	Fibrebond

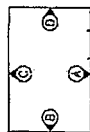
These prints comply with the Florida Manufacture Building Act and subcode Codes and all required components shall comply with 99-72

APPROVED BY



Approval of this document does not authorize or approve any deviation or alterations from the requirements of applicable State Laws.

NOTE:  
SEE INTERIOR CEILING VIEW FOR NOTES



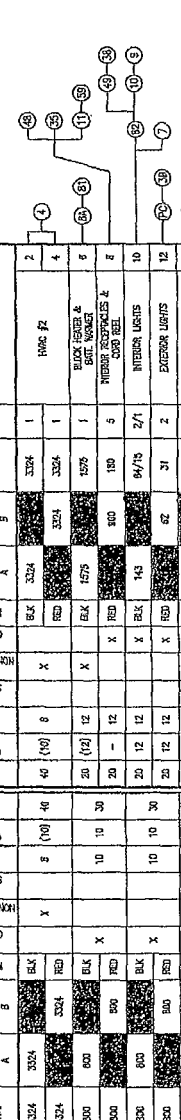
ELEVATION KEY

1900 DAVENPORT DRIVE MINDEN, LA 71055 ph. (504) 824-2914 www.fibrebond.com	
FIBREBOND	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
INTERIOR ELEVATIONS "A" & "B"	
REV	DATE
1	01/16/2014
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3/16/14-01 2-2  
B Ref D-9577



These prints comply with the Florida Manufacture Law, Act and exposed conductors and all required components shall comply with 9B-72. APPROVED BY NIA INC. Consist. Type: Conductor; Wire Size: 180 and 2; Material: 180 and 2; Fire Rating of Wire: 180 and 2; Allow. Floor Load: 180 and 2; Approval Date: 11/20/2017; Manufacturer: Fibresond. Approval of this document does not authorize approval of any deviation or variations from the requirements of applicable State Laws.



CIRCUIT NO.	DESCRIPTION	LOAD PER PHASE (VA)			WIRE COLOR	LOAD CONTINUOUS	LOADS NON-CONTINUOUS	LOADS SUB-PANEL	WIRE SIZE	GROUNDING WIRE SIZE	WIRE TYPE	TRIP	PHASE	LOAD PER PHASE (VA)	UNIT VA	%	DESCRIPTION
		A	B	S													
1	NOC #1	3324	3324	3324	BLK				40	100	40			3324	1		PHASE #2
2	RECEPT #1	600	600	600	BLK	X			10	10	30			600	1		BLACK RECEPT & BATT. WAGON
3	RECEPT #2	600	600	600	BLK	X			10	10	30			600	1		INTERIOR RECEPTACLES & CABINETS
4	RECEPT #3	600	600	600	BLK	X			10	10	30			600	1		INTERIOR LIGHTS
5	RECEPT #4	600	600	600	BLK	X			10	10	30			600	1		EXTERIOR LIGHTS
6	RECEPT #5	600	600	600	BLK	X			10	10	30			600	1		EXTERIOR GPO RECEPTACLES
7	RECEPT #6	600	600	600	BLK	X			10	10	30			600	1		BATT. CHARGER & GPO RECEPT
8	RECEPT #7	600	600	600	BLK	X			10	10	30			600	1		BATT. CONTROLLER
9	RECEPT #8	600	600	600	BLK	X			10	10	30			600	1		
10	RECEPT #9	600	600	600	BLK	X			10	10	30			600	1		
11	RECEPT #10	600	600	600	BLK	X			10	10	30			600	1		
12	RECEPT #11	600	600	600	BLK	X			10	10	30			600	1		
13	RECEPT #12	600	600	600	BLK	X			10	10	30			600	1		
14	RECEPT #13	600	600	600	BLK	X			10	10	30			600	1		
15	RECEPT #14	600	600	600	BLK	X			10	10	30			600	1		
16	RECEPT #15	600	600	600	BLK	X			10	10	30			600	1		
17	RECEPT #16	600	600	600	BLK	X			10	10	30			600	1		
18	RECEPT #17	600	600	600	BLK	X			10	10	30			600	1		
19	RECEPT #18	600	600	600	BLK	X			10	10	30			600	1		
20	RECEPT #19	600	600	600	BLK	X			10	10	30			600	1		
21	RECEPT #20	600	600	600	BLK	X			10	10	30			600	1		
22	RECEPT #21	600	600	600	BLK	X			10	10	30			600	1		
23	RECEPT #22	600	600	600	BLK	X			10	10	30			600	1		
24	RECEPT #23	600	600	600	BLK	X			10	10	30			600	1		
25	RECEPT #24	600	600	600	BLK	X			10	10	30			600	1		
26	RECEPT #25	600	600	600	BLK	X			10	10	30			600	1		
27	RECEPT #26	600	600	600	BLK	X			10	10	30			600	1		
28	RECEPT #27	600	600	600	BLK	X			10	10	30			600	1		
29	RECEPT #28	600	600	600	BLK	X			10	10	30			600	1		
30	RECEPT #29	600	600	600	BLK	X			10	10	30			600	1		
31	RECEPT #30	600	600	600	BLK	X			10	10	30			600	1		
32	RECEPT #31	600	600	600	BLK	X			10	10	30			600	1		
33	RECEPT #32	600	600	600	BLK	X			10	10	30			600	1		
34	RECEPT #33	600	600	600	BLK	X			10	10	30			600	1		
35	RECEPT #34	600	600	600	BLK	X			10	10	30			600	1		
36	RECEPT #35	600	600	600	BLK	X			10	10	30			600	1		
37	RECEPT #36	600	600	600	BLK	X			10	10	30			600	1		
38	RECEPT #37	600	600	600	BLK	X			10	10	30			600	1		
39	RECEPT #38	600	600	600	BLK	X			10	10	30			600	1		
40	RECEPT #39	600	600	600	BLK	X			10	10	30			600	1		
41	RECEPT #40	600	600	600	BLK	X			10	10	30			600	1		
42	TOTAL VA	7200	7200	7200													TOTAL VA
43	TOTAL VA CONTINUOUS	3324	3324	3324													TOTAL VA CONTINUOUS
44	TOTAL VA NON-CONTINUOUS	3876	3876	3876													TOTAL VA NON-CONTINUOUS
45	TOTAL VA SUB-PANEL	-	-	-													TOTAL VA SUB-PANEL
46	TOTAL VA	3631	3631	3631													TOTAL VA

PANEL DESIGNATION: ELECTRICAL PANEL (PANEL 1)  
 MAIN BREAKER: 200 AMP  
 MAIN BREAKER ALC RATING: 22000 ALC  
 BRANCH BREAKER ALC RATING: 10000 ALC  
 BRANCH BREAKER TYPE: SQUARE D - EMT ON  
 VOLTAGE: 120/240  
 CIRCUIT: 60  
 PHASE: 1  
 WIRE: 3  
 MAIN COPPER BUS: 200 ANFS  
 NEUTRAL: 200 ANFS

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1500 DAVENPORT DRIVE MINNIE, LA. 71055  
 ph: (504) 884-9574 www.fibresond.com  
 AT&T MOBILITY  
 11'-5" X 12'-0" EQUIPMENT SHELTER FIBRESOND  
 ELECTRICAL SCHEMATIC #1  
 NONE  
 3-1  
 REC D-9577

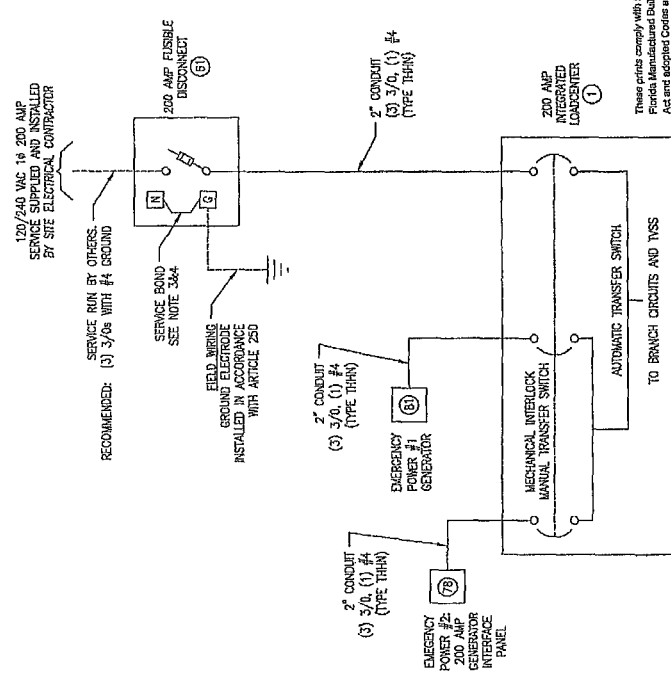
THESE PLANS ARE THE PROPERTY OF FIBRESOND. ANY REPRODUCTION OR USE OF THESE PLANS WITHOUT THE WRITTEN PERMISSION OF FIBRESOND IS STRICTLY PROHIBITED. FIBRESOND MAKES NO REPRESENTATION OR WARRANTY AS TO THE ACCURACY OR COMPLETENESS OF THESE PLANS. THE USER SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND REGULATIONS FROM THE APPLICABLE LOCAL, STATE AND FEDERAL AGENCIES. FIBRESOND SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO PERSONS OR PROPERTY ARISING FROM THE USE OF THESE PLANS.

NOTES:  
 1. ALL WIRE TO BE #12 THRU #18 UNLESS NOTED OTHERWISE.  
 2. ALL WORK TO CONFORM TO M.E.C. LATEST STATE ADOPTED EDITION.  
 3. LABEL SERVICE DISCONNECT WITH A RED TAG.  
 4. SWITCH LES CONDUCTORS SHALL BE THE SAME COLOR AS CIRCUIT CONDUCTORS.  
 5. SEE ELECTRICAL SCHEMATIC #2 SHEET FOR WIRE SIZES.  
 6. PROVIDE GROUND CONDUCTOR PER FUSIBLE HERMETIC CONTACTOR.  
 7. ALL GPO RECEPTACLES TO HAVE A DEDICATED GROUND WIRE.  
 8. EQUIPMENT TERMINATION LUGS AND CONDUCTORS ARE RATED AT A MINIMUM OF 75°C.

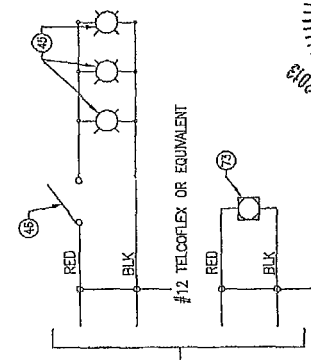
DEC 2 2018  
 FIBRESOND  
 1500 DAVENPORT DRIVE MINNIE, LA. 71055  
 ph: (504) 884-9574 www.fibresond.com



- NOTES:
1. DASHED LINES DENOTE FIELD WORK.
  2. BUILDING ELECTRICAL SYSTEM IS RATED AT 10,000 A.L.C. F. HIGHER RATINGS ARE REQUIRED, IT IS THE RESPONSIBILITY OF THE CONTRACTOR/ENGINEER TO MEET SUCH REQUIREMENTS.
  3. SERVICE BOND IS TO BE MADE BY DEVICES (STRAPS, SCREWS, ETC.) SUPPLIED BY EQUIPMENT MANUFACTURER. IF NO SUCH BOND IS TO BE MADE IN ACCORDANCE WITH NEC ARTICLE 250.
  4. WHEN SERVICE OVER CURRENT DISCONNECT IS FIELD INSTALLED AND HAS A NEUTRAL TO GROUND CONNECTION ESTABLISHED, REMOVE NEUTRAL TO GROUND CONNECTION IN TRANSFER SWITCH CONDUCTOR OVER CURRENT PROTECTION DEVICES ARE SELECTED IN ACCORDANCE WITH NEC (ARTICLE 240).
  5. CONDUCTOR SIZES IS SELECTED FROM NEC (ARTICLE 215.2).
  6. ALL CONDUITS SHALL BE RATED FOR USE WITH THE FIELD INSTALLED OVER CURRENT DISCONNECTS.
  7. ALL CONDUITS SHALL BE COPPER.
  8. LABEL SERVICE DISCONNECT WITH A RED TAG.
  9. EQUIPMENT TERMINATION LUGS & CONDUCTORS ARE RATED AT A MINIMUM OF 75 C.
  10. RECOMMENDED SERVICE ENTRANCE CONDUCTOR SIZE: (3) 3/0 AWG & (1) #4 GROUND
  - 11.



POWER RISER BLOCK DIAGRAM



Cond. Type:	VS
Permit Manufacturer:	S2
Code:	1
Wind Velocity:	150 mph c
Fire Rating of Ex. Wall:	0
Plan No.:	ME72AD-9577
Approved Dist.:	250 PSF
Manufacturer:	21760074
Notes:	Expanded

These prints comply with the Florida Manufacture Code and all required components shall comply with 95-72.

APPROVED BY

**NIA INC.**

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

1500 DRYDENPORT DRIVE, MINNENEA, LA 71055 PH: (504) 884-5874 www.fiberbond.com			
FIBERSOND			
AT&T MOBILITY			
11'-5" X 12'-0" EQUIPMENT SHELTER			
ELECTRICAL SCHEMATIC #2			
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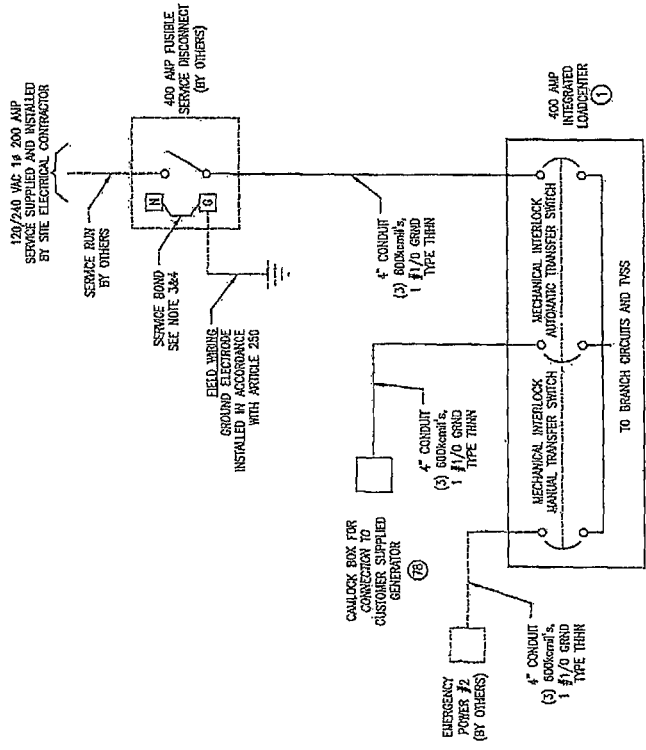
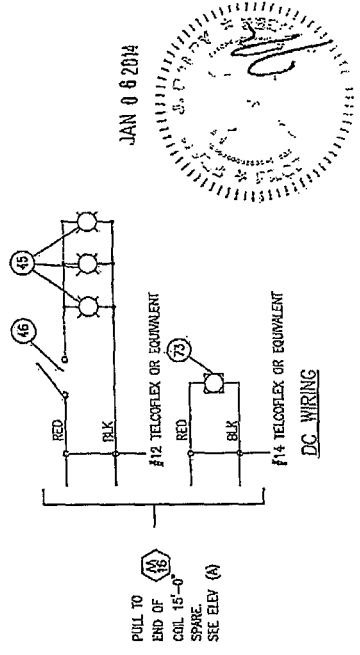
- NOTES:**
- DASHED LINES DENOTE FIELD WIRE.
  - ALL WIRING SHALL BE INSTALLED IN ACCORDANCE WITH THE NATIONAL ELECTRICAL CODE (NEC) AT 2000 A.I.C. IF HIGHER BUILDING CODES ARE APPLICABLE. THE RESPONSIBILITY OF THE CONTRACTOR TO MEET SUCH REQUIREMENTS.
  - SERVICE BOND IS TO BE MADE BY DEVICES (STRAPS, SCREWS, ETC.) SUPPLIED BY EQUIPMENT MANUFACTURER. IF NO SUCH DEVICE IS SUPPLIED, BOND IS TO BE MADE IN ACCORDANCE WITH NEC ARTICLE 250. WHEN SERVICE OVERCURRENT DISCONNECT IS FIELD INSTALLED AND HAS A NEUTRAL TO GROUND CONNECTION ESTABLISHED, REMOVE NEUTRAL TO GROUND CONNECTION IN TRANSFER SWITCH.
  - CONDUCTOR OVERCURRENT PROTECTION DEVICES ARE SELECTED IN ACCORDANCE WITH NEC (ARTICLE 240.3).
  - CONDUCTOR SIZING IS SELECTED FROM NEC (ARTICLE 214.3).
  - ALL LUGS MUST BE PROTECTED FROM WEAR THAT LUGS SHALL BE USED FOR ALL CONDUCTORS.
  - ALL CONDUCTORS SHALL BE COPPER.
  - LABEL SERVICE DISCONNECT WITH A RED TAG.
  - RECOMMENDED SERVICE ENTRANCE CONDUCTOR SIZE: (3) 600 KCMIL & (1) 1/0 GROUND.
  - EQUIPMENT TERMINATION LUGS & CONDUCTORS ARE RATED AT A MINIMUM OF 75 °C.

These prints comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 88-72.

**APPROVED BY**

**NFA INC.**

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.



**POWER RISER BLOCK DIAGRAM**

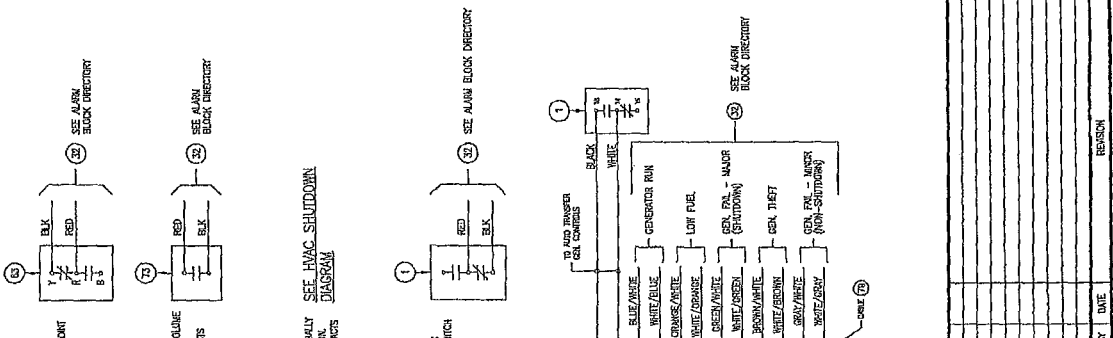
1300 DANVERPORT DRIVE MINDEN, LA 70655 PH: (800) 824-2614 www.fibrebond.com	
FIBREBOND	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
ELECTRICAL SCHEMATIC #2	
DATE	3-2-1
NO.	D-9577
REV. 1	DATE
REV. 2	DATE
REV. 3	DATE
REV. 4	DATE
REV. 5	DATE
REV. 6	DATE
REV. 7	DATE
REV. 8	DATE
REV. 9	DATE
REV. 10	DATE
REV. 11	DATE
REV. 12	DATE
REV. 13	DATE
REV. 14	DATE
REV. 15	DATE
REV. 16	DATE
REV. 17	DATE
REV. 18	DATE
REV. 19	DATE
REV. 20	DATE
REV. 21	DATE
REV. 22	DATE
REV. 23	DATE
REV. 24	DATE
REV. 25	DATE
REV. 26	DATE
REV. 27	DATE
REV. 28	DATE
REV. 29	DATE
REV. 30	DATE
REV. 31	DATE
REV. 32	DATE
REV. 33	DATE
REV. 34	DATE
REV. 35	DATE
REV. 36	DATE
REV. 37	DATE
REV. 38	DATE
REV. 39	DATE
REV. 40	DATE
REV. 41	DATE
REV. 42	DATE
REV. 43	DATE
REV. 44	DATE
REV. 45	DATE
REV. 46	DATE
REV. 47	DATE
REV. 48	DATE
REV. 49	DATE
REV. 50	DATE
REV. 51	DATE
REV. 52	DATE
REV. 53	DATE
REV. 54	DATE
REV. 55	DATE
REV. 56	DATE
REV. 57	DATE
REV. 58	DATE
REV. 59	DATE
REV. 60	DATE
REV. 61	DATE
REV. 62	DATE
REV. 63	DATE
REV. 64	DATE
REV. 65	DATE
REV. 66	DATE
REV. 67	DATE
REV. 68	DATE
REV. 69	DATE
REV. 70	DATE
REV. 71	DATE
REV. 72	DATE
REV. 73	DATE
REV. 74	DATE
REV. 75	DATE
REV. 76	DATE
REV. 77	DATE
REV. 78	DATE
REV. 79	DATE
REV. 80	DATE
REV. 81	DATE
REV. 82	DATE
REV. 83	DATE
REV. 84	DATE
REV. 85	DATE
REV. 86	DATE
REV. 87	DATE
REV. 88	DATE
REV. 89	DATE
REV. 90	DATE
REV. 91	DATE
REV. 92	DATE
REV. 93	DATE
REV. 94	DATE
REV. 95	DATE
REV. 96	DATE
REV. 97	DATE
REV. 98	DATE
REV. 99	DATE
REV. 100	DATE

These prints comply with the Fire Alarm Building Code and all required components shall comply with 95-72.

APPROVED BY  
**NIA INC.**

Approval of this document does not authorize or approve any deviation or variations from the requirements of applicable State Laws.

Const. Type: VB  
 Occupancy: SB  
 Building No.: 1  
 Fire Rating of Ext. Walls: 190 and c  
 Fire Rating of Ext. Windows: 0  
 Fire Rating of Ext. Doors: 0  
 Fire Rating of Pen. No.: MF122-04-9577  
 Allow. Floor Load: 250 PSF  
 Proposed Date: 1/16/2014  
 Address: Fibresond  
 City: Manden, LA



**INTRUSION ALARM**  
 THE MAGNETIC DOOR ALARMS ARE WHEED NORMALLY OPEN. CONTACTS CLOSE UPON CONTACTS OPEN SENDS AN ALARM.

**HIGH TEMPERATURE ALARM**  
 CONTACTS OPEN UPON TEMPERATURE INCREASE (OR FUL) ABOVE THE SET POINT SENDS AN ALARM.

**HIGH HUMIDITY ALARM**  
 CONTACTS OPEN UPON HUMIDITY INCREASE (OR FUL) ABOVE THE SET POINT SENDS AN ALARM.

**SURGE ARRESTOR ALARM**  
 FOR SURGE ARRESTOR ALARM, CONTACT TO BE NORMALLY CLOSED. WHEN SURGE ARRESTOR IS DETECTED THESE CONTACTS NORMALLY CLOSED CONTACTS WILL OPEN IF ELEMENTS ARE BLIND OR IF A.C. POWER FALLS SENDS AN ALARM.

**COMMERCIAL POWER FAIL**  
 THE COMMERCIAL POWER REL. RELAY IS NORMALLY CLOSED. COMMERCIAL POWER IS THE POWER SUPPLIED BY THE UTILITY COMPANY. THE RELAY WILL OPEN UPON POWER LOSS. CONTACTS OF THE RELAY NORMALLY CLOSE UPON LOSS OF COMMERCIAL POWER. SENDS AN ALARM. (THIS ALARM MUST NOT BE CONNECTED TO THE SYSTEM SUCH THAT IT RECEIVES POWER FROM A REDUNDANT UPS SYSTEM, OR ANY OTHER REDUNDANT POWER SOURCE).

**HVAC FAIL ALARM #1**  
 WHEN HIGH PRESSURE OR LOW PRESSURE SWITCHES ACTIVATE THE LOCK OUT RELAY THE ALARM CONTACTS WILL OPEN SENDS AN ALARM.

**HVAC FAIL ALARM #2**  
 WHEN HIGH PRESSURE OR LOW PRESSURE SWITCHES ACTIVATE THE LOCK OUT RELAY THE ALARM CONTACTS WILL OPEN SENDS AN ALARM.

**LOW TEMPERATURE ALARM**  
 CONTACTS OPEN UPON TEMPERATURE DECREASE (OR FUL) BELOW THE SET POINT SENDS AN ALARM.

**HYDROGEN GAS DETECTOR**  
 THE GAS DETECTOR IS SET AT 1% BY VOLUME OR 10,000 PPM. IF HYDROGEN GAS IS DETECTED OVER 1% BY VOLUME CONTACTS CLOSE SENDS AN ALARM.

**SMOKE DETECTOR**  
 THE SMOKE ALARM CONTACTS ARE NORMALLY CLOSED. WHEN A "NO SMOKE" CONDITION WHEN SMOKE IS DETECTED THESE CONTACTS OPEN SENDS AN ALARM.

**TRANSFER SWITCH ALARM**  
 THE TRANSFER SWITCH ALARM CONTACTS ARE NORMALLY CLOSED. WHEN THE SWITCH OPERATES AND SWITCHES TO THE EMERGENCY POWER POSITION CONTACTS (OPEN FROM THE LOAD FUL)

NOTES:  
 1. ALL ALARMS OPEN ON ALARM UNLESS OTHERWISE NOTED.  
 2. ALARM WIRES TO BE 22/2 WIRE UNLESS OTHERWISE SPECIFIED.  
 C=RED  
 W=WHITE  
 3. SEE LUG WIRING FOR SPECIFIC TERMINAL LOCATIONS.  
 4. FOR AC, USE WHT & BLK; FOR DC, USE BLK & RED.







ITEM LIST

ITEM NO.	FOREBOND PART NO.	MANUFACTURER PART NO.	MANUFACTURER	DESCRIPTION
1	85-1112	85-1112	GREEN	GREEN
2	85-1113	85-1113	GREEN	GREEN
3	85-1114	85-1114	GREEN	GREEN
4	85-1115	85-1115	GREEN	GREEN
5	85-1116	85-1116	GREEN	GREEN
6	85-1117	85-1117	GREEN	GREEN
7	85-1118	85-1118	GREEN	GREEN
8	85-1119	85-1119	GREEN	GREEN
9	85-1120	85-1120	GREEN	GREEN
10	85-1121	85-1121	GREEN	GREEN
11	85-1122	85-1122	GREEN	GREEN
12	85-1123	85-1123	GREEN	GREEN
13	85-1124	85-1124	GREEN	GREEN
14	85-1125	85-1125	GREEN	GREEN
15	85-1126	85-1126	GREEN	GREEN
16	85-1127	85-1127	GREEN	GREEN
17	85-1128	85-1128	GREEN	GREEN
18	85-1129	85-1129	GREEN	GREEN
19	85-1130	85-1130	GREEN	GREEN
20	85-1131	85-1131	GREEN	GREEN
21	85-1132	85-1132	GREEN	GREEN
22	85-1133	85-1133	GREEN	GREEN
23	85-1134	85-1134	GREEN	GREEN
24	85-1135	85-1135	GREEN	GREEN
25	85-1136	85-1136	GREEN	GREEN
26	85-1137	85-1137	GREEN	GREEN
27	85-1138	85-1138	GREEN	GREEN
28	85-1139	85-1139	GREEN	GREEN
29	85-1140	85-1140	GREEN	GREEN
30	85-1141	85-1141	GREEN	GREEN
31	85-1142	85-1142	GREEN	GREEN
32	85-1143	85-1143	GREEN	GREEN
33	85-1144	85-1144	GREEN	GREEN
34	85-1145	85-1145	GREEN	GREEN
35	85-1146	85-1146	GREEN	GREEN
36	85-1147	85-1147	GREEN	GREEN
37	85-1148	85-1148	GREEN	GREEN
38	85-1149	85-1149	GREEN	GREEN
39	85-1150	85-1150	GREEN	GREEN
40	85-1151	85-1151	GREEN	GREEN
41	85-1152	85-1152	GREEN	GREEN
42	85-1153	85-1153	GREEN	GREEN
43	85-1154	85-1154	GREEN	GREEN
44	85-1155	85-1155	GREEN	GREEN
45	85-1156	85-1156	GREEN	GREEN
46	85-1157	85-1157	GREEN	GREEN
47	85-1158	85-1158	GREEN	GREEN
48	85-1159	85-1159	GREEN	GREEN
49	85-1160	85-1160	GREEN	GREEN
50	85-1161	85-1161	GREEN	GREEN
51	85-1162	85-1162	GREEN	GREEN
52	85-1163	85-1163	GREEN	GREEN
53	85-1164	85-1164	GREEN	GREEN
54	85-1165	85-1165	GREEN	GREEN
55	85-1166	85-1166	GREEN	GREEN
56	85-1167	85-1167	GREEN	GREEN
57	85-1168	85-1168	GREEN	GREEN
58	85-1169	85-1169	GREEN	GREEN
59	85-1170	85-1170	GREEN	GREEN
60	85-1171	85-1171	GREEN	GREEN
61	85-1172	85-1172	GREEN	GREEN
62	85-1173	85-1173	GREEN	GREEN
63	85-1174	85-1174	GREEN	GREEN
64	85-1175	85-1175	GREEN	GREEN
65	85-1176	85-1176	GREEN	GREEN
66	85-1177	85-1177	GREEN	GREEN
67	85-1178	85-1178	GREEN	GREEN
68	85-1179	85-1179	GREEN	GREEN
69	85-1180	85-1180	GREEN	GREEN
70	85-1181	85-1181	GREEN	GREEN
71	85-1182	85-1182	GREEN	GREEN
72	85-1183	85-1183	GREEN	GREEN
73	85-1184	85-1184	GREEN	GREEN
74	85-1185	85-1185	GREEN	GREEN
75	85-1186	85-1186	GREEN	GREEN
76	85-1187	85-1187	GREEN	GREEN
77	85-1188	85-1188	GREEN	GREEN
78	85-1189	85-1189	GREEN	GREEN
79	85-1190	85-1190	GREEN	GREEN
80	85-1191	85-1191	GREEN	GREEN
81	85-1192	85-1192	GREEN	GREEN
82	85-1193	85-1193	GREEN	GREEN
83	85-1194	85-1194	GREEN	GREEN
84	85-1195	85-1195	GREEN	GREEN
85	85-1196	85-1196	GREEN	GREEN
86	85-1197	85-1197	GREEN	GREEN
87	85-1198	85-1198	GREEN	GREEN
88	85-1199	85-1199	GREEN	GREEN
89	85-1200	85-1200	GREEN	GREEN
90	85-1201	85-1201	GREEN	GREEN
91	85-1202	85-1202	GREEN	GREEN
92	85-1203	85-1203	GREEN	GREEN
93	85-1204	85-1204	GREEN	GREEN
94	85-1205	85-1205	GREEN	GREEN
95	85-1206	85-1206	GREEN	GREEN
96	85-1207	85-1207	GREEN	GREEN
97	85-1208	85-1208	GREEN	GREEN
98	85-1209	85-1209	GREEN	GREEN
99	85-1210	85-1210	GREEN	GREEN
100	85-1211	85-1211	GREEN	GREEN

ITEM LIST

ITEM NO.	FOREBOND PART NO.	MANUFACTURER PART NO.	MANUFACTURER	DESCRIPTION
1	85-1212	85-1212	GREEN	GREEN
2	85-1213	85-1213	GREEN	GREEN
3	85-1214	85-1214	GREEN	GREEN
4	85-1215	85-1215	GREEN	GREEN
5	85-1216	85-1216	GREEN	GREEN
6	85-1217	85-1217	GREEN	GREEN
7	85-1218	85-1218	GREEN	GREEN
8	85-1219	85-1219	GREEN	GREEN
9	85-1220	85-1220	GREEN	GREEN
10	85-1221	85-1221	GREEN	GREEN
11	85-1222	85-1222	GREEN	GREEN
12	85-1223	85-1223	GREEN	GREEN
13	85-1224	85-1224	GREEN	GREEN
14	85-1225	85-1225	GREEN	GREEN
15	85-1226	85-1226	GREEN	GREEN
16	85-1227	85-1227	GREEN	GREEN
17	85-1228	85-1228	GREEN	GREEN
18	85-1229	85-1229	GREEN	GREEN
19	85-1230	85-1230	GREEN	GREEN
20	85-1231	85-1231	GREEN	GREEN
21	85-1232	85-1232	GREEN	GREEN
22	85-1233	85-1233	GREEN	GREEN
23	85-1234	85-1234	GREEN	GREEN
24	85-1235	85-1235	GREEN	GREEN
25	85-1236	85-1236	GREEN	GREEN
26	85-1237	85-1237	GREEN	GREEN
27	85-1238	85-1238	GREEN	GREEN
28	85-1239	85-1239	GREEN	GREEN
29	85-1240	85-1240	GREEN	GREEN
30	85-1241	85-1241	GREEN	GREEN
31	85-1242	85-1242	GREEN	GREEN
32	85-1243	85-1243	GREEN	GREEN
33	85-1244	85-1244	GREEN	GREEN
34	85-1245	85-1245	GREEN	GREEN
35	85-1246	85-1246	GREEN	GREEN
36	85-1247	85-1247	GREEN	GREEN
37	85-1248	85-1248	GREEN	GREEN
38	85-1249	85-1249	GREEN	GREEN
39	85-1250	85-1250	GREEN	GREEN
40	85-1251	85-1251	GREEN	GREEN
41	85-1252	85-1252	GREEN	GREEN
42	85-1253	85-1253	GREEN	GREEN
43	85-1254	85-1254	GREEN	GREEN
44	85-1255	85-1255	GREEN	GREEN
45	85-1256	85-1256	GREEN	GREEN
46	85-1257	85-1257	GREEN	GREEN
47	85-1258	85-1258	GREEN	GREEN
48	85-1259	85-1259	GREEN	GREEN
49	85-1260	85-1260	GREEN	GREEN
50	85-1261	85-1261	GREEN	GREEN
51	85-1262	85-1262	GREEN	GREEN
52	85-1263	85-1263	GREEN	GREEN
53	85-1264	85-1264	GREEN	GREEN
54	85-1265	85-1265	GREEN	GREEN
55	85-1266	85-1266	GREEN	GREEN
56	85-1267	85-1267	GREEN	GREEN
57	85-1268	85-1268	GREEN	GREEN
58	85-1269	85-1269	GREEN	GREEN
59	85-1270	85-1270	GREEN	GREEN
60	85-1271	85-1271	GREEN	GREEN
61	85-1272	85-1272	GREEN	GREEN
62	85-1273	85-1273	GREEN	GREEN
63	85-1274	85-1274	GREEN	GREEN
64	85-1275	85-1275	GREEN	GREEN
65	85-1276	85-1276	GREEN	GREEN
66	85-1277	85-1277	GREEN	GREEN
67	85-1278	85-1278	GREEN	GREEN
68	85-1279	85-1279	GREEN	GREEN
69	85-1280	85-1280	GREEN	GREEN
70	85-1281	85-1281	GREEN	GREEN
71	85-1282	85-1282	GREEN	GREEN
72	85-1283	85-1283	GREEN	GREEN
73	85-1284	85-1284	GREEN	GREEN
74	85-1285	85-1285	GREEN	GREEN
75	85-1286	85-1286	GREEN	GREEN
76	85-1287	85-1287	GREEN	GREEN
77	85-1288	85-1288	GREEN	GREEN
78	85-1289	85-1289	GREEN	GREEN
79	85-1290	85-1290	GREEN	GREEN
80	85-1291	85-1291	GREEN	GREEN
81	85-1292	85-1292	GREEN	GREEN
82	85-1293	85-1293	GREEN	GREEN
83	85-1294	85-1294	GREEN	GREEN
84	85-1295	85-1295	GREEN	GREEN
85	85-1296	85-1296	GREEN	GREEN
86	85-1297	85-1297	GREEN	GREEN
87	85-1298	85-1298	GREEN	GREEN
88	85-1299	85-1299	GREEN	GREEN
89	85-1300	85-1300	GREEN	GREEN
90	85-1301	85-1301	GREEN	GREEN
91	85-1302	85-1302	GREEN	GREEN
92	85-1303	85-1303	GREEN	GREEN
93	85-1304	85-1304	GREEN	GREEN
94	85-1305	85-1305	GREEN	GREEN
95	85-1306	85-1306	GREEN	GREEN
96	85-1307	85-1307	GREEN	GREEN
97	85-1308	85-1308	GREEN	GREEN
98	85-1309	85-1309	GREEN	GREEN
99	85-1310	85-1310	GREEN	GREEN
100	85-1311	85-1311	GREEN	GREEN

NOTE: 1. ITEMS ARE SUBJECT TO CHANGE DUE TO MANUFACTURING AND EQUAL REQUIREMENTS MAY CHANGE DUE TO MANUFACTURING SPECIFICATIONS.

These prints comply with the National Manufacture Building Code and all applicable codes and comply with 98-72. APPROVED BY NIA INC.

Approval of this document does not authorize or approve any deviation or conditions from the requirements of applicable State Laws.

Const. Type: V3  
Occupancy: S2  
Allowable No. of Stories: 1  
Wind Velocity: 130 mph  
Ex. Walls: 0  
Plan No.: MFTZ-D-9577  
Allow. Floor Load: 250 PSF  
Approval Date: 11/16/2014  
Signature: [Signature]

PACKING LIST

ITEM NO.	QTY.	FOREBOND PART NO.	MANUFACTURER PART NO.	DESCRIPTION
1	1	85-1312	85-1312	GREEN
2	1	85-1313	85-1313	GREEN
3	1	85-1314	85-1314	GREEN
4	1	85-1315	85-1315	GREEN
5	1	85-1316	85-1316	GREEN
6	1	85-1317	85-1317	GREEN
7	1	85-1318	85-1318	GREEN
8	1	85-1319	85-1319	GREEN
9	1	85-1320	85-1320	GREEN
10	1	85-1321	85-1321	GREEN
11	1	85-1322	85-1322	GREEN
12	1	85-1323	85-1323</	

ITEM LIST

ITEM	PIREBOND PART NO.	MANUFACTURER PART NO.	DESCRIPTION
11	82-1136	82-1136	...
12	82-1137	82-1137	...
13	82-1138	82-1138	...
14	82-1139	82-1139	...
15	82-1140	82-1140	...
16	82-1141	82-1141	...
17	82-1142	82-1142	...
18	82-1143	82-1143	...
19	82-1144	82-1144	...
20	82-1145	82-1145	...
21	82-1146	82-1146	...
22	82-1147	82-1147	...
23	82-1148	82-1148	...
24	82-1149	82-1149	...
25	82-1150	82-1150	...
26	82-1151	82-1151	...
27	82-1152	82-1152	...
28	82-1153	82-1153	...
29	82-1154	82-1154	...
30	82-1155	82-1155	...
31	82-1156	82-1156	...
32	82-1157	82-1157	...
33	82-1158	82-1158	...
34	82-1159	82-1159	...
35	82-1160	82-1160	...
36	82-1161	82-1161	...
37	82-1162	82-1162	...
38	82-1163	82-1163	...
39	82-1164	82-1164	...
40	82-1165	82-1165	...
41	82-1166	82-1166	...
42	82-1167	82-1167	...
43	82-1168	82-1168	...
44	82-1169	82-1169	...
45	82-1170	82-1170	...
46	82-1171	82-1171	...
47	82-1172	82-1172	...
48	82-1173	82-1173	...
49	82-1174	82-1174	...
50	82-1175	82-1175	...
51	82-1176	82-1176	...
52	82-1177	82-1177	...
53	82-1178	82-1178	...
54	82-1179	82-1179	...
55	82-1180	82-1180	...
56	82-1181	82-1181	...
57	82-1182	82-1182	...
58	82-1183	82-1183	...
59	82-1184	82-1184	...
60	82-1185	82-1185	...
61	82-1186	82-1186	...
62	82-1187	82-1187	...
63	82-1188	82-1188	...
64	82-1189	82-1189	...
65	82-1190	82-1190	...
66	82-1191	82-1191	...
67	82-1192	82-1192	...
68	82-1193	82-1193	...
69	82-1194	82-1194	...
70	82-1195	82-1195	...
71	82-1196	82-1196	...
72	82-1197	82-1197	...
73	82-1198	82-1198	...
74	82-1199	82-1199	...
75	82-1200	82-1200	...
76	82-1201	82-1201	...
77	82-1202	82-1202	...
78	82-1203	82-1203	...
79	82-1204	82-1204	...
80	82-1205	82-1205	...
81	82-1206	82-1206	...
82	82-1207	82-1207	...
83	82-1208	82-1208	...
84	82-1209	82-1209	...
85	82-1210	82-1210	...
86	82-1211	82-1211	...
87	82-1212	82-1212	...
88	82-1213	82-1213	...
89	82-1214	82-1214	...
90	82-1215	82-1215	...
91	82-1216	82-1216	...
92	82-1217	82-1217	...
93	82-1218	82-1218	...
94	82-1219	82-1219	...
95	82-1220	82-1220	...
96	82-1221	82-1221	...
97	82-1222	82-1222	...
98	82-1223	82-1223	...
99	82-1224	82-1224	...
100	82-1225	82-1225	...

ITEM LIST

ITEM	PIREBOND PART NO.	MANUFACTURER PART NO.	DESCRIPTION
11	82-1136	82-1136	...
12	82-1137	82-1137	...
13	82-1138	82-1138	...
14	82-1139	82-1139	...
15	82-1140	82-1140	...
16	82-1141	82-1141	...
17	82-1142	82-1142	...
18	82-1143	82-1143	...
19	82-1144	82-1144	...
20	82-1145	82-1145	...
21	82-1146	82-1146	...
22	82-1147	82-1147	...
23	82-1148	82-1148	...
24	82-1149	82-1149	...
25	82-1150	82-1150	...
26	82-1151	82-1151	...
27	82-1152	82-1152	...
28	82-1153	82-1153	...
29	82-1154	82-1154	...
30	82-1155	82-1155	...
31	82-1156	82-1156	...
32	82-1157	82-1157	...
33	82-1158	82-1158	...
34	82-1159	82-1159	...
35	82-1160	82-1160	...
36	82-1161	82-1161	...
37	82-1162	82-1162	...
38	82-1163	82-1163	...
39	82-1164	82-1164	...
40	82-1165	82-1165	...
41	82-1166	82-1166	...
42	82-1167	82-1167	...
43	82-1168	82-1168	...
44	82-1169	82-1169	...
45	82-1170	82-1170	...
46	82-1171	82-1171	...
47	82-1172	82-1172	...
48	82-1173	82-1173	...
49	82-1174	82-1174	...
50	82-1175	82-1175	...
51	82-1176	82-1176	...
52	82-1177	82-1177	...
53	82-1178	82-1178	...
54	82-1179	82-1179	...
55	82-1180	82-1180	...
56	82-1181	82-1181	...
57	82-1182	82-1182	...
58	82-1183	82-1183	...
59	82-1184	82-1184	...
60	82-1185	82-1185	...
61	82-1186	82-1186	...
62	82-1187	82-1187	...
63	82-1188	82-1188	...
64	82-1189	82-1189	...
65	82-1190	82-1190	...
66	82-1191	82-1191	...
67	82-1192	82-1192	...
68	82-1193	82-1193	...
69	82-1194	82-1194	...
70	82-1195	82-1195	...
71	82-1196	82-1196	...
72	82-1197	82-1197	...
73	82-1198	82-1198	...
74	82-1199	82-1199	...
75	82-1200	82-1200	...
76	82-1201	82-1201	...
77	82-1202	82-1202	...
78	82-1203	82-1203	...
79	82-1204	82-1204	...
80	82-1205	82-1205	...
81	82-1206	82-1206	...
82	82-1207	82-1207	...
83	82-1208	82-1208	...
84	82-1209	82-1209	...
85	82-1210	82-1210	...
86	82-1211	82-1211	...
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88	82-1213	82-1213	...
89	82-1214	82-1214	...
90	82-1215	82-1215	...
91	82-1216	82-1216	...
92	82-1217	82-1217	...
93	82-1218	82-1218	...
94	82-1219	82-1219	...
95	82-1220	82-1220	...
96	82-1221	82-1221	...
97	82-1222	82-1222	...
98	82-1223	82-1223	...
99	82-1224	82-1224	...
100	82-1225	82-1225	...

PACKING LIST

ITEM NO.	QUANTITY	PIREBOND PART NO.	MANUFACTURER PART NO.	DESCRIPTION
1	1	82-1136	82-1136	...
2	1	82-1137	82-1137	...
3	1	82-1138	82-1138	...
4	1	82-1139	82-1139	...
5	1	82-1140	82-1140	...
6	1	82-1141	82-1141	...
7	1	82-1142	82-1142	...
8	1	82-1143	82-1143	...
9	1	82-1144	82-1144	...
10	1	82-1145	82-1145	...
11	1	82-1146	82-1146	...
12	1	82-1147	82-1147	...
13	1	82-1148	82-1148	...
14	1	82-1149	82-1149	...
15	1	82-1150	82-1150	...
16	1	82-1151	82-1151	...
17	1	82-1152	82-1152	...
18	1	82-1153	82-1153	...
19	1	82-1154	82-1154	...
20	1	82-1155	82-1155	...
21	1	82-1156	82-1156	...
22	1	82-1157	82-1157	...
23	1	82-1158	82-1158	...
24	1	82-1159	82-1159	...
25	1	82-1160	82-1160	...
26	1	82-1161	82-1161	...
27	1	82-1162	82-1162	...
28	1	82-1163	82-1163	...
29	1	82-1164	82-1164	...
30	1	82-1165	82-1165	...
31	1	82-1166	82-1166	...
32	1	82-1167	82-1167	...
33	1	82-1168	82-1168	...
34	1	82-1169	82-1169	...
35	1	82-1170	82-1170	...
36	1	82-1171	82-1171	...
37	1	82-1172	82-1172	...
38	1	82-1173	82-1173	...
39	1	82-1174	82-1174	...
40	1	82-1175	82-1175	...
41	1	82-1176	82-1176	...
42	1	82-1177	82-1177	...
43	1	82-1178	82-1178	...
44	1	82-1179	82-1179	...
45	1	82-1180	82-1180	...
46	1	82-1181	82-1181	...
47	1	82-1182	82-1182	...
48	1	82-1183	82-1183	...
49	1	82-1184	82-1184	...
50	1	82-1185	82-1185	...
51	1	82-1186	82-1186	...
52	1	82-1187	82-1187	...
53	1	82-1188	82-1188	...
54	1	82-1189	82-1189	...
55	1	82-1190	82-1190	...
56	1	82-1191	82-1191	...
57	1	82-1192	82-1192	...
58	1	82-1193	82-1193	...
59	1	82-1194	82-1194	...
60	1	82-1195	82-1195	...
61	1	82-1196	82-1196	...
62	1	82-1197	82-1197	...
63	1	82-1198	82-1198	...
64	1	82-1199	82-1199	...
65	1	82-1200	82-1200	...
66	1	82-1201	82-1201	...
67	1	82-1202	82-1202	...
68	1	82-1203	82-1203	...
69	1	82-1204	82-1204	...
70	1	82-1205	82-1205	...
71	1	82-1206	82-1206	...
72	1	82-1207	82-1207	...
73	1	82-1208	82-1208	...
74	1	82-1209	82-1209	...
75	1	82-1210	82-1210	...
76	1	82-1211	82-1211	...
77	1	82-1212	82-1212	...
78	1	82-1213	82-1213	...
79	1	82-1214	82-1214	...
80	1	82-1215	82-1215	...
81	1	82-1216	82-1216	...
82	1	82-1217	82-1217	...
83	1	82-1218	82-1218	...
84	1	82-1219	82-1219	...
85	1	82-1220	82-1220	...
86	1	82-1221	82-1221	...
87	1	82-1222	82-1222	...
88	1	82-1223	82-1223	...
89	1	82-1224	82-1224	...
90	1	82-1225	82-1225	...

These prints comply with the Florida Manufactured Building Act and adopted Codes and all other applicable codes and components shall comply with AS-72

APPROVED BY

**NIA INC.**

Approval of this document does not authorize or approve any deviation or deviations from the requirements of applicable State Laws.

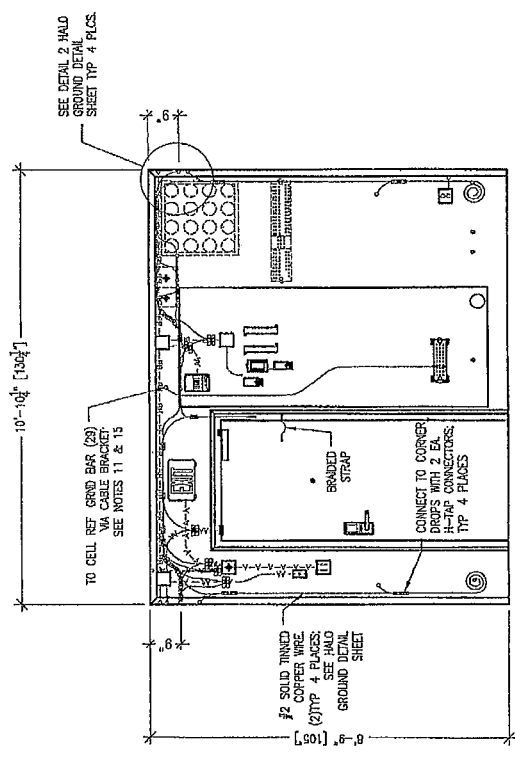
Count: Type: 0  
 Allowed to No. 0  
 of Floors: 1  
 Wind Velocity: 120 mph  
 Fire Rating of Deck: 0  
 Allow. Floor Load: 250 PSF  
 Approval Date: 11/25/21  
 Manufacturer: Fibreboard

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1800 DAVENPORT DRIVE WINDEN, LA 71085  
 Ph: (800) 824-

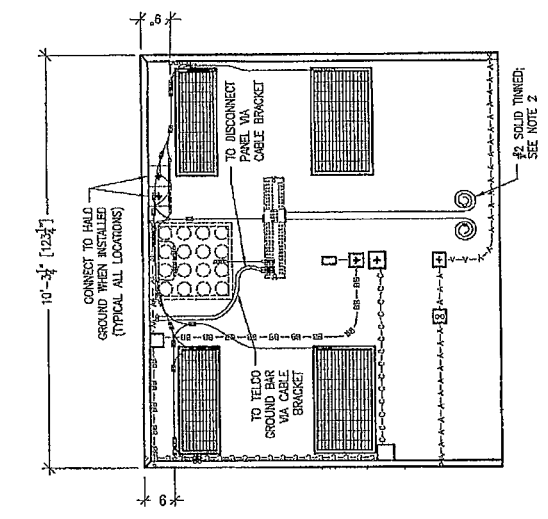






NOTES:  
 #2 SOLID TINNED COPPER WIRE SHALL BE USED FOR ALL WIRING.  
 SEE HALO GROUND DETAILS SHEET FOR DETAILS.

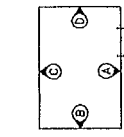
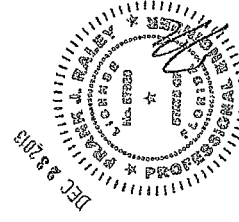
ELEVATION "A"



These plans comply with the Florida Manufactured Building Act and adopted Codes and all required components shall comply with 88-72

APPROVED BY  
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ELEVATION "B"



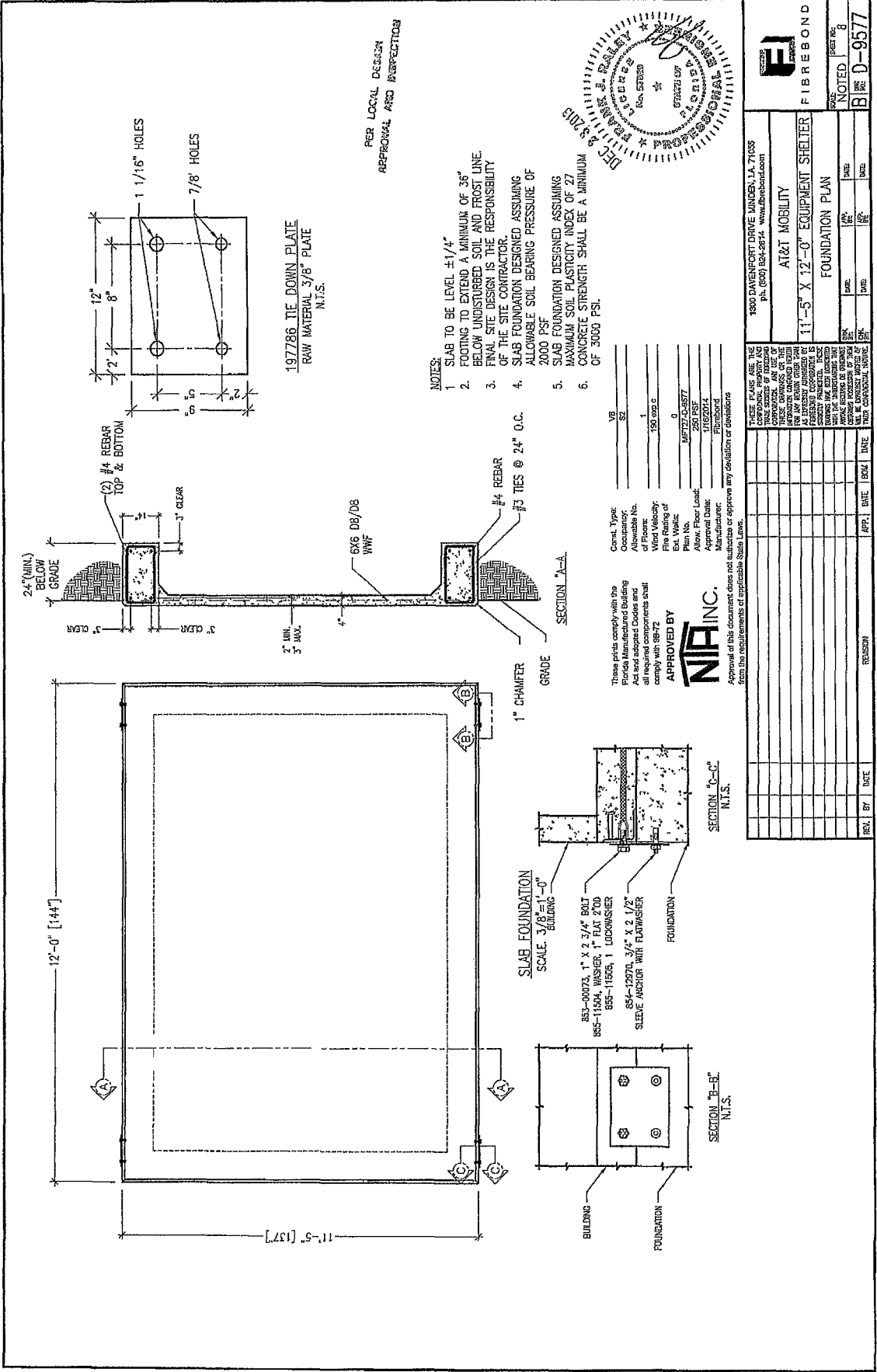
ELEVATION KEY

1300 DANFORTH DRIVE MINNEAPOLIS, MN 55412 PH: (612) 824-2814 www.fibrebond.com	
AT&T MOBILITY	
11'-5" X 12'-0" EQUIPMENT SHELTER	
HALO GROUND ELEVATIONS "A" & "B"	
DATE	7-2
SCALE	3/8"=1'-0"
PROJECT NO.	B-9577
REV.	DATE
APP.	DATE
DESIGNER	DATE









PER LOCAL DESIGN  
APPROVAL AND INSPECTION

197786 TIE DOWN PLATE  
RAW MATERIAL 3/8" PLATE  
N.T.S.

- NOTES:
1. SLAB TO BE LEVEL  $\pm 1/4"$
  2. FOOTING TO EXTEND A MINIMUM OF 36" BELOW UNDISTURBED SOIL AND FROST LINE.
  3. FINAL SITE DESIGN IS THE RESPONSIBILITY OF THE SITE CONTRACTOR.
  4. SLAB FOUNDATION DESIGNED ASSUMING ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF
  5. SLAB FOUNDATION DESIGNED ASSUMING MAXIMUM SOIL PLASTICITY INDEX OF 27
  6. CONCRETE STRENGTH SHALL BE A MINIMUM OF 3000 PSI.

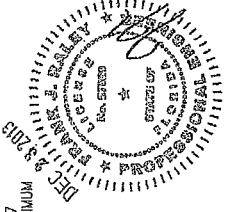
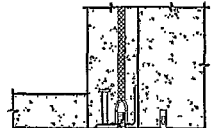
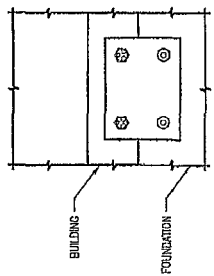
These prints comply with the Florida Manufactured Building Act and adopted Codes and Rules of the State of Florida and comply with 98-22.

APPROVED BY  
**NIA INC.**

Approval of this document does not constitute or approve any deviation or deviations from the requirements of applicable State Laws.

Com. Type	V5
Occupancy	1
Plan No.	190 050 c
Wind Velocity	0
Fire Rating of	M-20 0577
Est. Value	250 PSF
Allowable Load	112/2014
Approved Date	Florida
Manufacturer	Florida

SLAB FOUNDATION  
SCALE 3/8" = 1'-0"  
BUILDING



1930 DAVENPORT DRIVE WINDER, FLA. 34085 ph. (800) 294-2874 www.fiberbond.com	
FIBREBOND	
AT&T MOBILITY	
FOUNDATION PLAN	
REV.	DATE
1	1/27/04
2	2/18/04
3	3/1/04
4	3/15/04
5	3/22/04
6	3/22/04
7	3/22/04
8	3/22/04
9	3/22/04
10	3/22/04
11	3/22/04
12	3/22/04
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92	3/22/04
93	3/22/04
94	3/22/04
95	3/22/04
96	3/22/04
97	3/22/04
98	3/22/04
99	3/22/04
100	3/22/04

NOTED  
REVISED  
DATE

11'-5" X 12'-0" EQUIPMENT SHELTER

FOUNDATION PLAN

REV. DATE

1 1/27/04

2 2/18/04

3 3/1/04

4 3/15/04

5 3/22/04

6 3/22/04

7 3/22/04

8 3/22/04

9 3/22/04

10 3/22/04

11 3/22/04

12 3/22/04

13 3/22/04





