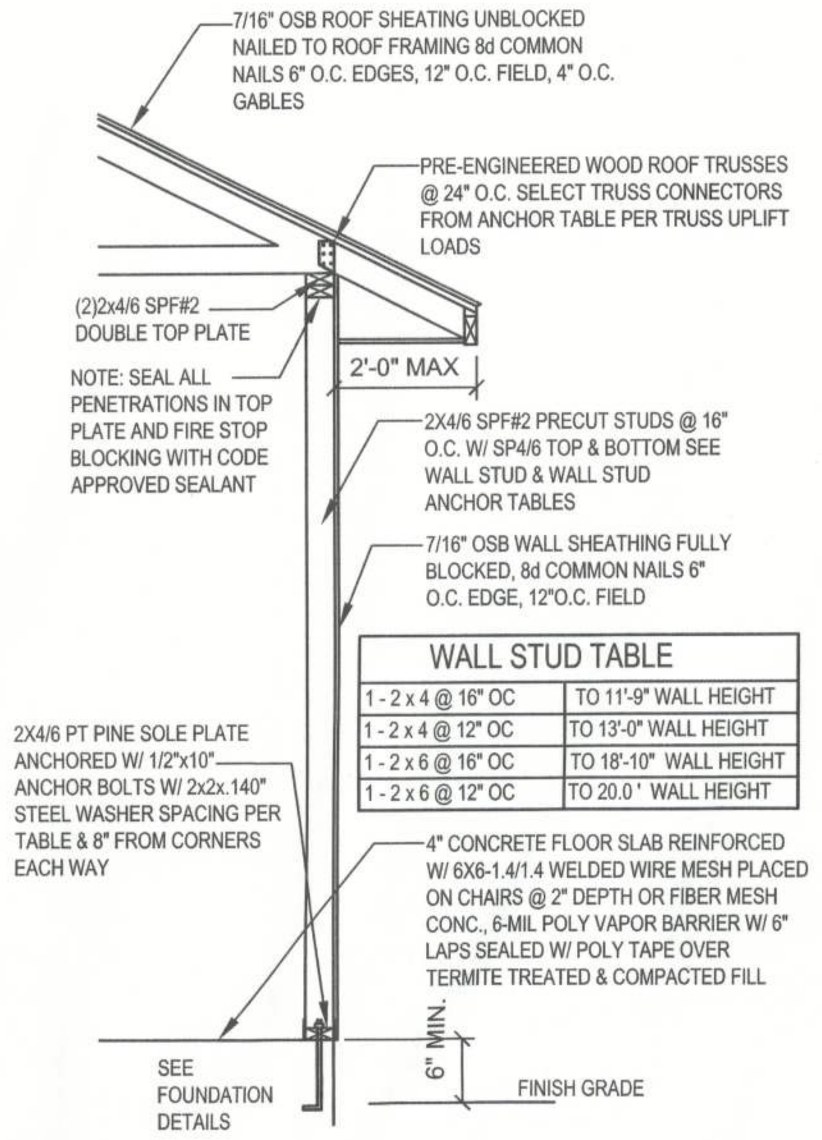


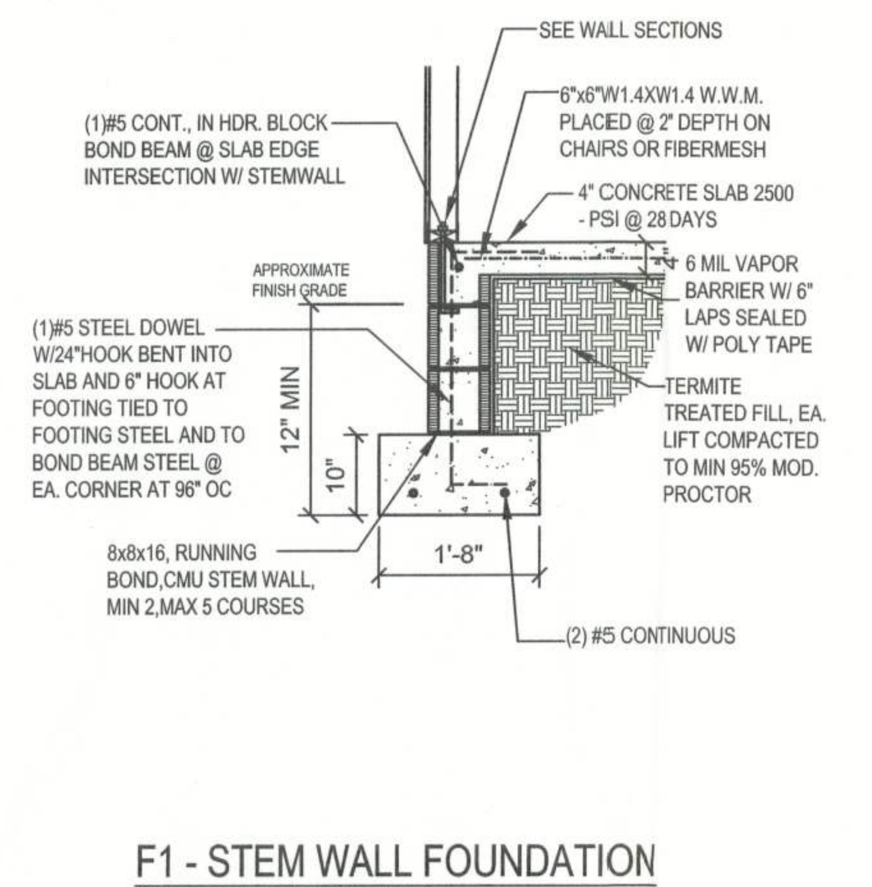
WALL STUD TABLE

1 - 2 x 4 @ 16" OC	TO 11'-9" WALL HEIGHT
1 - 2 x 4 @ 12" OC	TO 13'-0" WALL HEIGHT
1 - 2 x 6 @ 16" OC	TO 18'-10" WALL HEIGHT
1 - 2 x 6 @ 12" OC	TO 20.0 ' WALL HEIGHT

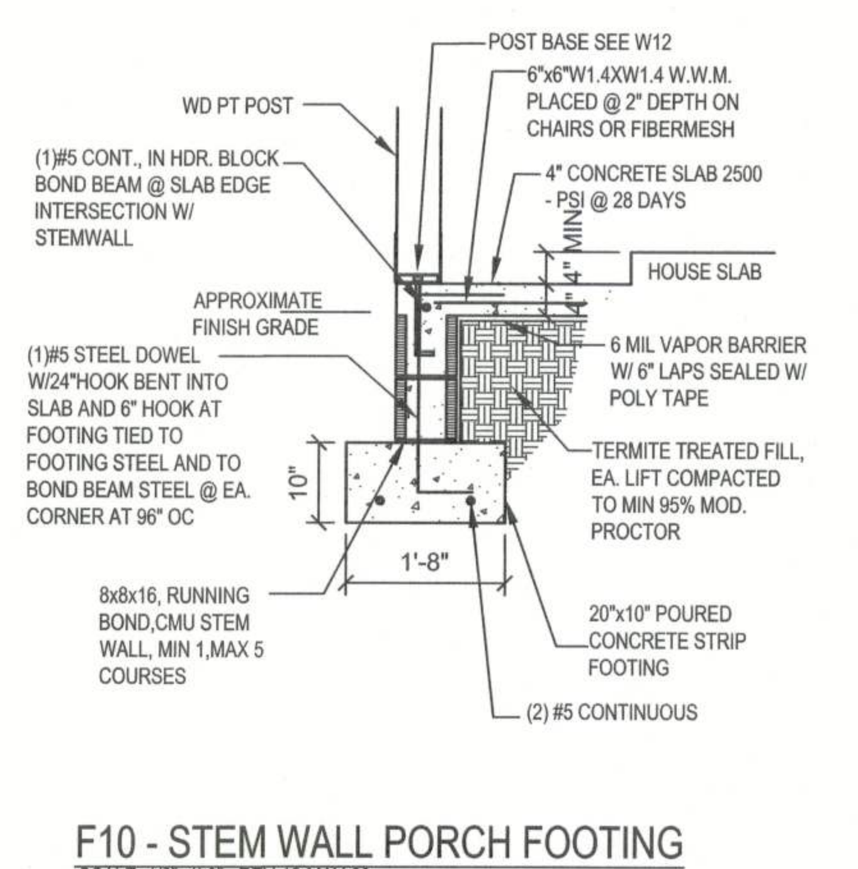
STUD ANCHOR TABLE

TYPICAL TRUSS UPLIFT & MAX 10'-0" WALL HIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE SPH4/SPH6 SPACING
770 LB	48" O.C.	48" O.C.	N/A
950 LB	48" O.C.	32" O.C.	N/A
1270 LB	32" O.C.	16" O.C.	32" O.C.
1500 LB	24" O.C.	16" O.C.	16" O.C.
2200 LB	LTTI31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK
NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6			
NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.			

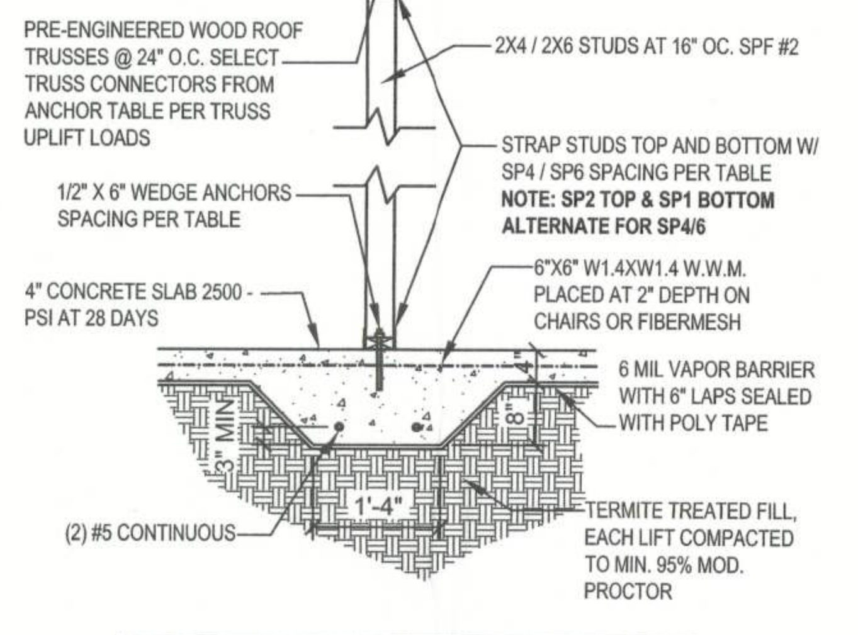
W1 - SINGLE STORY EXT. WALL SECTION
SCALE: 1/2"=1'-0" REV-22-AUG-03



F1 - STEM WALL FOUNDATION
SCALE: 1/2"=1'-0" REV-27-MAY-03

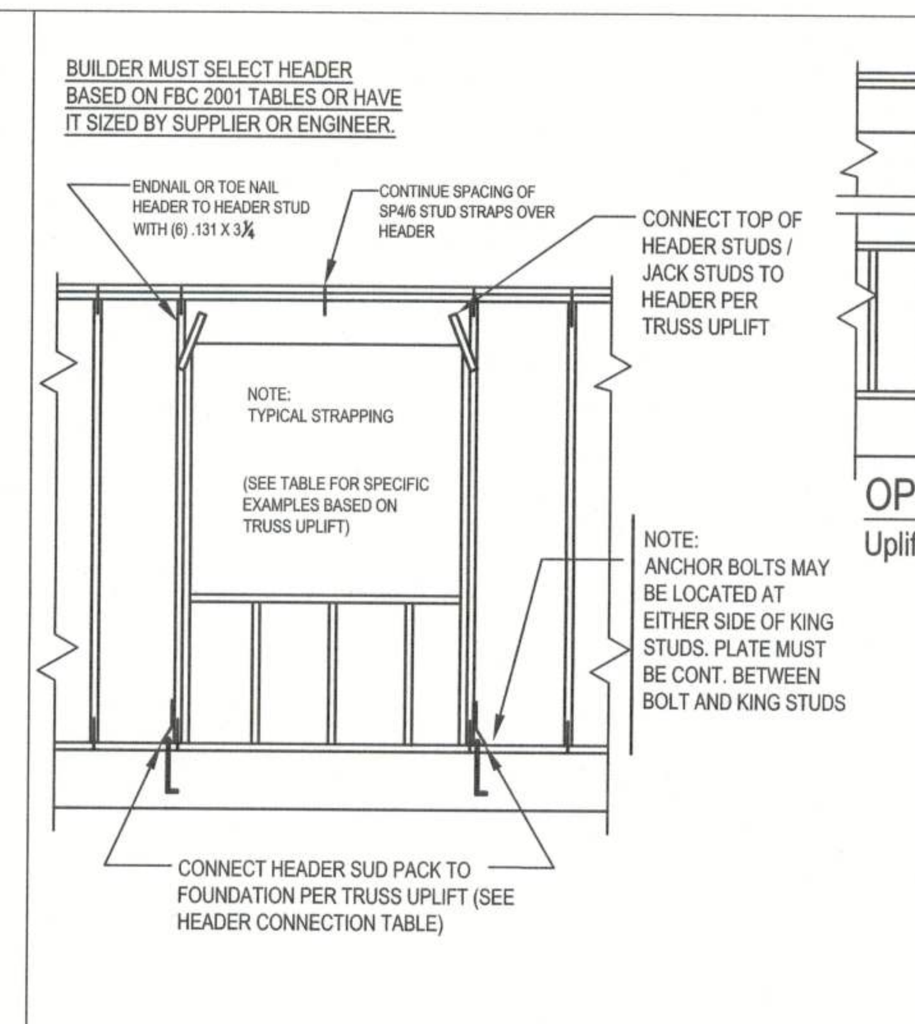


F10 - STEM WALL PORCH FOOTING
SCALE: 1/2"=1'-0" REV-16-MAY-03

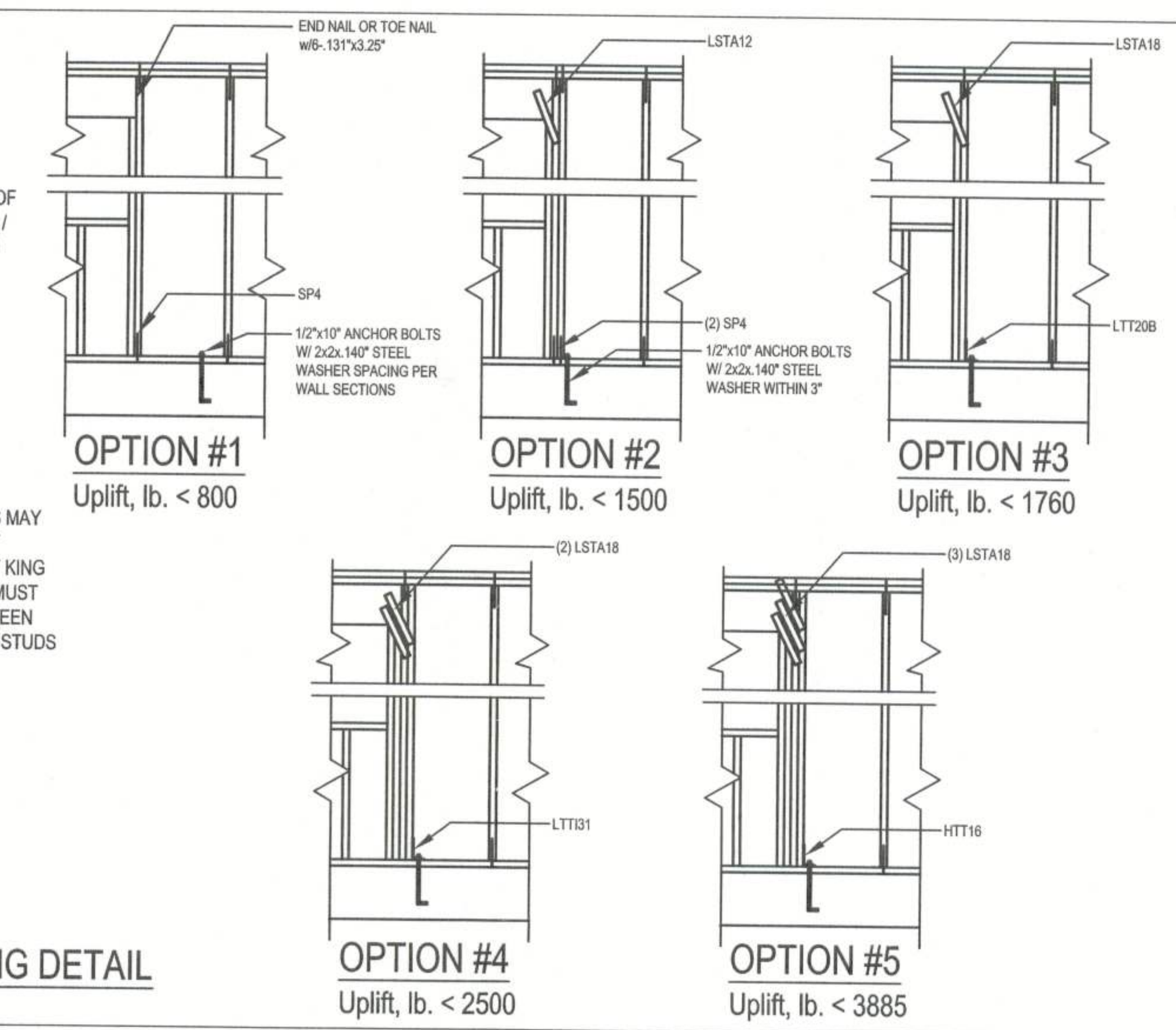


TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP6 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2.5A
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTTI31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

F4 - INTERIOR BEARING FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03



W13-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV 22-AUG-03



Load Bearing Header Sizing Methods (BY BUILDER)
1. Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
2. Use supplier pubished data or Southern pine span tables.
3. For engineered lumber beams have suppliers engineer size beam.

Jack Studs and King Studs (BY BUILDER)
4. Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
5. Use one jack stud for every 3000 lb vertical load.
6. Total king plus jack studs = studs needed to be there if no opening was there.

Header Uplift Connections (BY BUILDER)
7. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.
8. Select header connections from table below or mfg. catalog to connect header to stud (top connection) and stud to foundation (bottom connection).

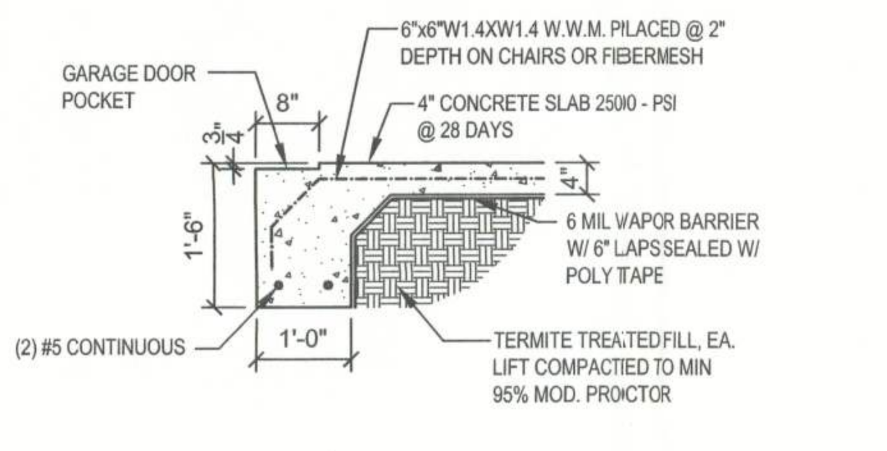
Option #	Uplift, lb.	Top Connector		Bottom Connector	
#1	< 800	End nail or toe nail w/6-.131"x3.25"		SP4, 6-10dx1½"	690
#2	< 1500	LSTA12, 10-10d	755	(2) SP4, 6-10dx1½", ½" AB	1380
#3	< 1750	LSTA18, 14-10d	1055	LTT20B, 10-16d ½" AB	1750
#4	< 2500	(2) LSTA18, 14-10d	2110	LTTI31, 18-10d ½"x10" AB	2185
#5	< 3885	(3) LSTA18, 14-10d	3480	HTT16, 18-16d, ⅝"x10" AB	4175
Uplift greater than 3885 lb requires engineering design					

FBC2001, TABLE 2308.3A
Header Spans For Exterior Bearing Walls Supporting Roof+Ceiling (20psf+20psf)

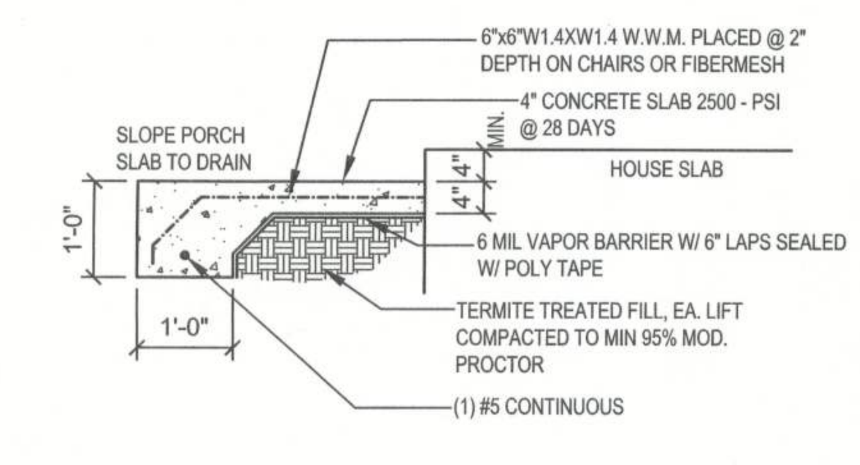
NOTES: NJ = Number of jack studs required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on header.

Header Sizes	20 Span	20 NJ	28 Span	28 NJ	36 Span
2-2x4	3-6	1	3-2	1	2-10
2-2x6	5-5	1	4-8	1	4-2
2-2x8	6-10	1	5-11	2	5-4
2-2x10	8-5	2	7-3	2	6-6
2-2x12	9-9	2	8-5	2	7-6
3-2x8	8-4	1	7-5	1	6-8
3-2x10	10-6	1	9-1	2	8-2
3-2x12	12-2	2	10-7	2	9-5
4-2x8	9-2	1	8-4	1	7-6
4-2x10	11-8	1	10-6	1	9-5
4-2x12	14-1	1	12-2	2	10-11

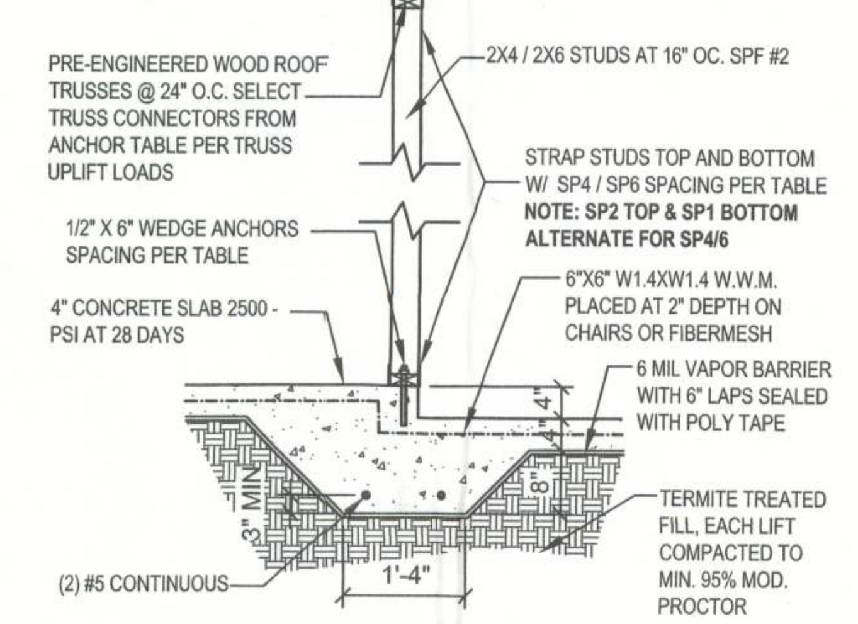
(Header Spans (ft-in); Building Width / Truss Span (ft))



F8 - GARAGE DOOR POCKET
SCALE: 1/2"=1'-0" REV-07-JAN-03

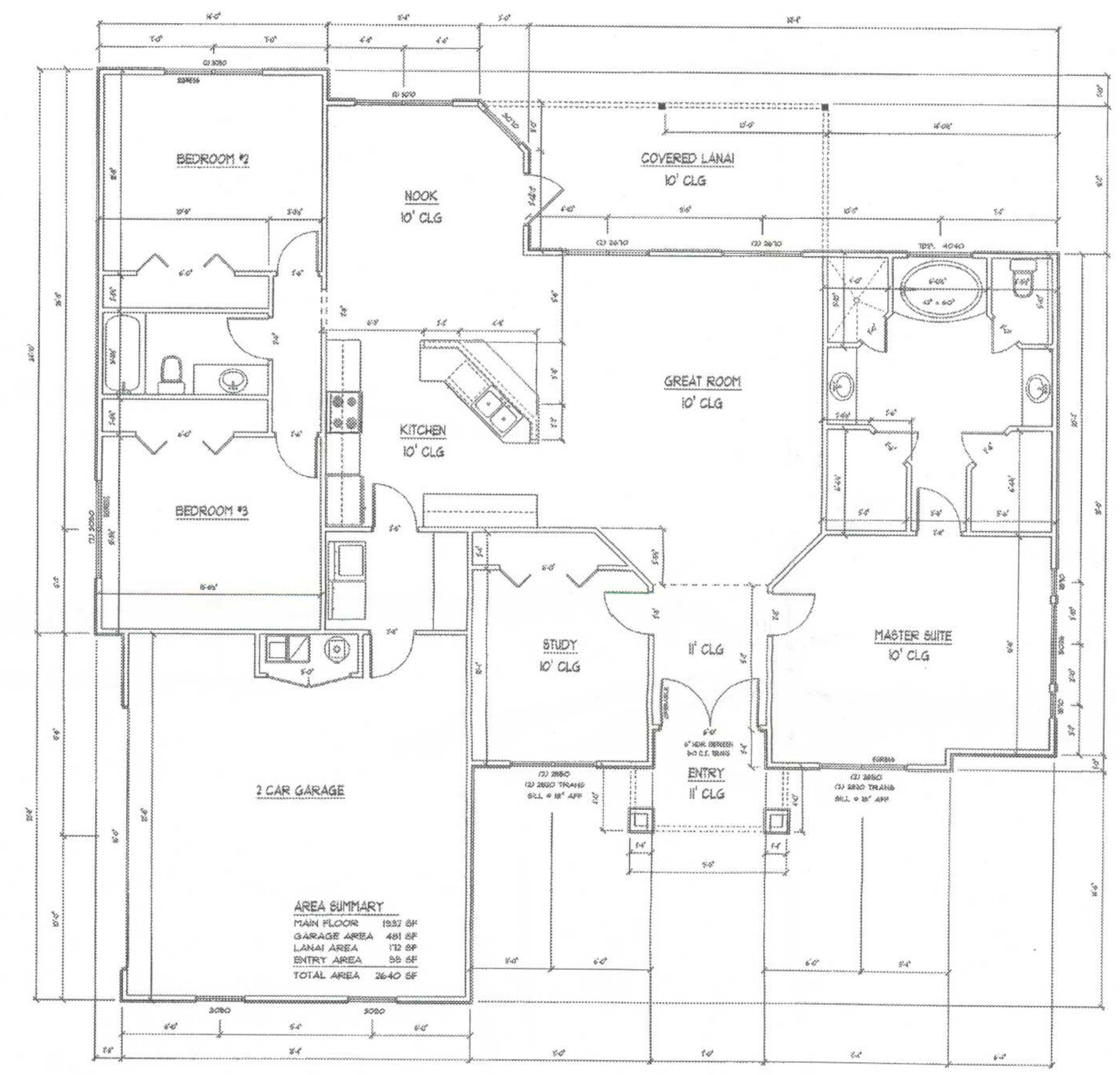


F2 - PORCH SLAB
SCALE: 1/2"=1'-0" REV-22-AUG-03



TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP6 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2.5A
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTTI31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

F5 - INTERIOR BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03



FLOOR PLAN

N2-GENERAL NOTES:

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

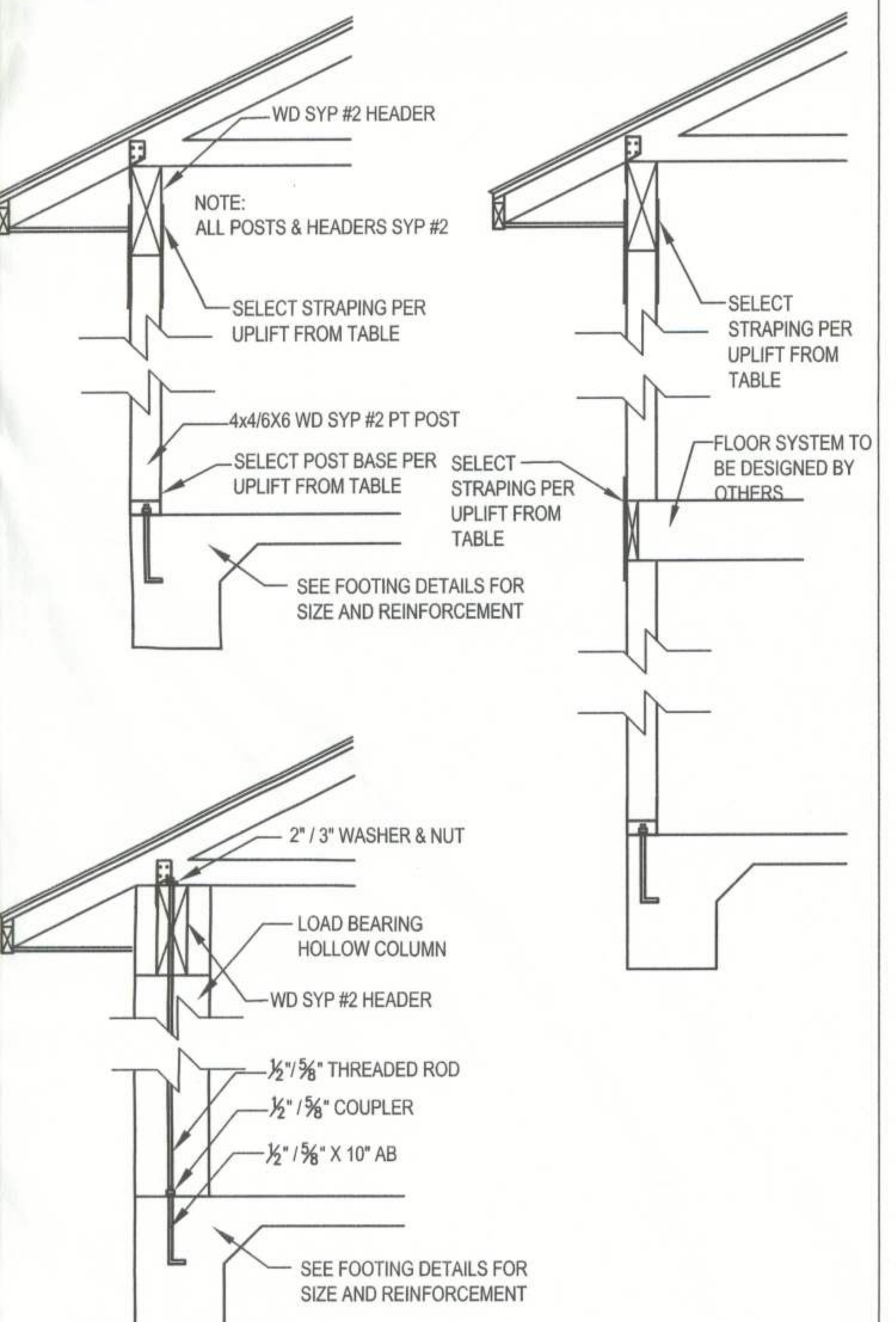
REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAPS SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

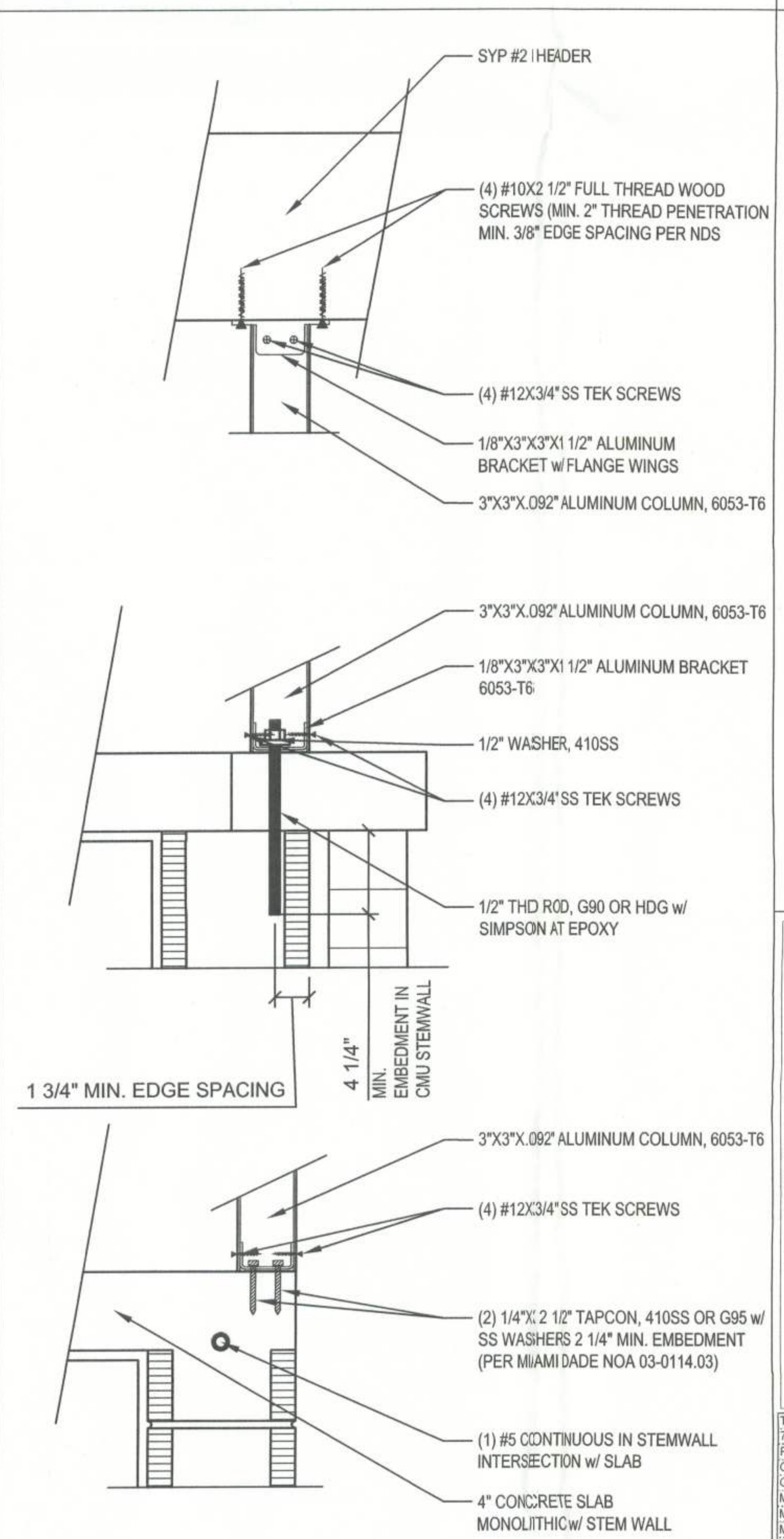
WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

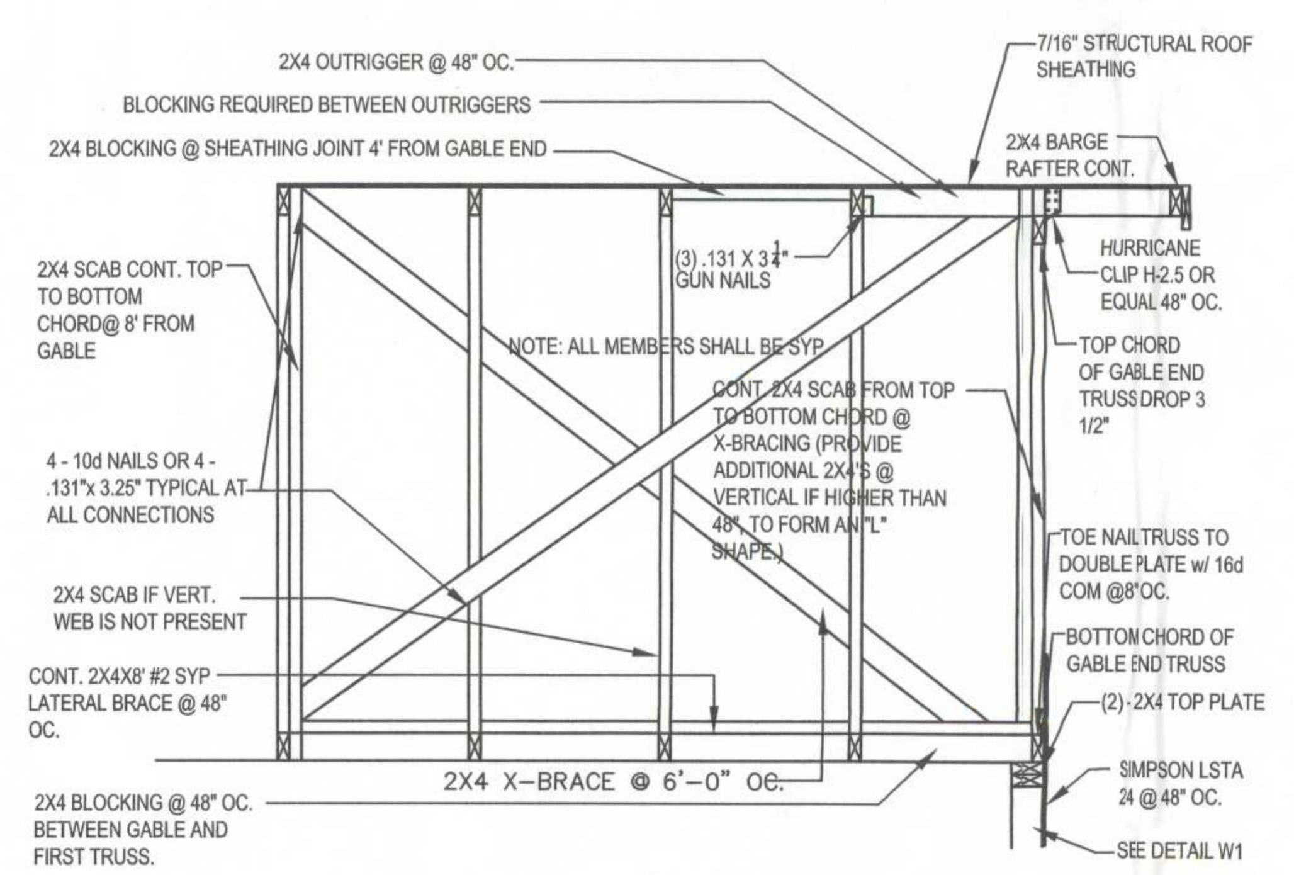


SYP #2 PT WD POSTS			
TYPICAL POST UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPING	HEADER STRAPING
555 LB	ABA44 W/ (6)-10d & ½" AB	(2) LSTA21 W/ (6)-10d EA.	(2) LSTA21 W/ (6)-10d EA.
720 LB	ABA66 W/ (8)-16d & ⅝" AB	(2) LSTA21 W/ (8)-10d EA.	(2) LSTA21 W/ (8)-10d EA.
2200 LB	ABU44 W/ (12)-16d, (2)½ BOLTS & ⅝"AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.
2300 LB	ABU66 W/ (12)-16d, (2)½ BOLTS & ⅝"AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.
HOLLOW COLUMN			
1500 LB	½" X 10" AB ATTACHED TO ½" THREADED ROD WITH ½" COUPLER THRU COLUMN & HEADER WITH 2" WASHER & NUT TOP		
2300 LB	⅝" X 10" AB ATTACHED TO ⅝" THREADED ROD WITH ⅝" COUPLER THRU COLUMN & HEADER WITH 3" WASHER & NUT TOP		

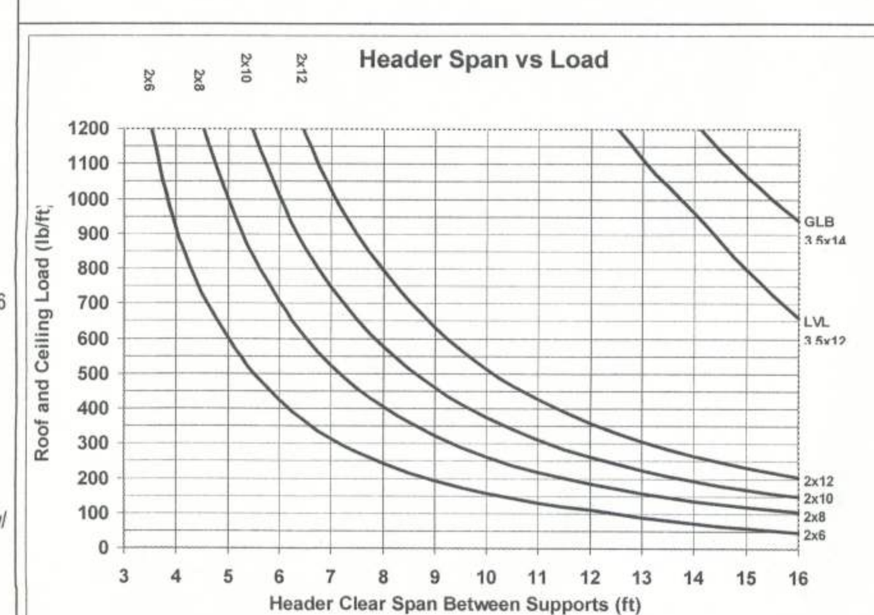
W12 - PORCH HEADER ANCHORS
SCALE: N.T.S. REV-18-JUL-03



W44 - ALUMINUM PORCH POST & HEADER ANCHORS
SCALE: N.T.S. REV-09-MAY-04



W10 - TYPICAL GABLE END (X-BRACING)
SCALE: 1/2"=1'-0"



Truss Design Loading: LL = 20psf, DL = 20psf		
7' Jack Truss	220	Half of 7' truss span + 24" overhang
Porch width, 8'	240	Half of 8' truss span + 24" overhang
Garage width, 20'	480	Half of 20' truss span + 24" overhang
Garage width, 24'	500	Half of 24' truss span + 24" overhang
Main house width, 32'	720	Half of 32' truss span + 24" overhang
Main house width, 40'	880	Half of 40' truss span + 24" overhang
Main house width, 60'	1280	Half of 60' truss span + 24" overhang
Bonus Room, above garage	1025	Half of 24' truss span and 14' bonus room

Notes:
1. Deflection limits: TL=L/240, LL=L/180
2. Duration factor, Cd = 1.25, applied to Fb and Fv. (No increase to E or Fc⊥ for duration of load.)
3. 2x headers are SYP#2 with OSB flitch spacer and (2) 12dS 16"OC. (strength increased 5% for OSB)
4. 3.5" x 14" GLB is Anthony Power Header, 2600 Fb, 1.9 E, or GP (2)1.75x14 LVL, 2850 Fb, 2.0 E.
5. 3.5" x 12" LVL is Blue Linx - GP Lam, (2) 1.75" x 11.78" LVL, 2900 Fb, 2.0 E, 285 Fv.
6. Minimum bearing for SYP header, Fc = 565psi, 2500 lb per jack stud. (For SPF plate, Fc = 425psi, 2200 lb per
7. Shear strength is increased 50% for minimal splits and checks headers cut to size on site.
8. Deflection has not been adjusted for creep due to long term loading, Kcr = 1.0.
9. The chart is based on NDS2001 bending, horizontal shear, and deflection requirements.
10. Chart is for uniform loads only.
11. (For non-uniform loads sum the loads on the header; divide by header span; and multiply by 2.)

W71 - HEADER SPANS FOR ROOF/CEILING LOAD

N5 - TRUSS UPLIFT CONNECTOR TABLE
REV-25-AUG-03

All connectors are Simpson Strongttie, uno. Selectop and bottom connections from this table or SST catalog to meet truss uplift. Usee fasteners as pecified.

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	600	H2.5A	5-8d	5-8d
620	720	H10	6-10dx1½"	6-10dx 1½"
850	990	LTS12	8-8dx1½"	8-8dx 1½"
1245	1450	HTS20	10-10d or 12-10dx1½"	10-10d or 12-10dx 1½"
1265	1470	H16, H16-2	10-10dx1½"	2-10dx 1½"
1785	2050	LGT2	14-10d Siker	16-16d Sinker
3655	4200	MGT	⅝" Thd. Rod	22-10d
SPF	**SYP**	**Strap Connector**	**To One Member**	**To Other Member**
760	885	SP4	6-10dx1½"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d	7-10d
1170	1360	SPH4	12-10dx1½"	N/A
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d
SPF	**SYP**	**Column Anchor**	**To Foundation**	**To Column / Truss**
1160	1350	LTT19	⅝"x 16" AB	8-16d Sinkers
1985	2310	LTTI31	⅝"x 16" AB	18-10dx 1½"
2385	2775	HD2A	⅝"x 16" AB	2-⅝" Bolts
3590	4175	HTT16	⅝"x 16" AB	18-16d
1975	2300	ABU66	⅝"x 16" AB	12-16d

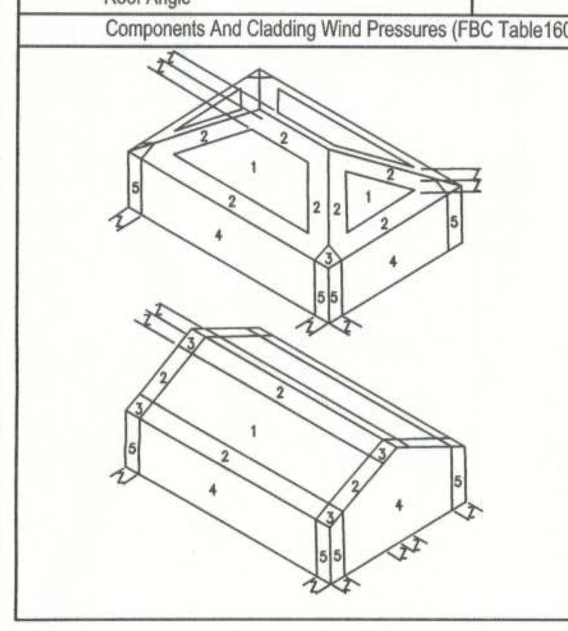
Studs Supporting Trusses: The builder is responssible for gravity loads, but you should put an extra 2x4 stud under truss bearing location for each 3000 lb of reaction. Check the minimum bearing requiremets of the truss and top plate (SPF, Fc=425psi=2230lb/ply).

Manufacturer and product number are listed for eexample not endorsment. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the exxample tables as log as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased for wind duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=.84*16d, 10dx1½"=.80*10d, 10d=12d=16d sinker). SPF=.86*SYP

N4-WIND LOAD DESIGN DATA

(Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with >10% slope.)

Basic Wind Speed	110 MPH
Wind Exposure	B
Wind Importance Factor	1.0
Building Category	II
Internal pressure Coefficient	N/A (Enclosed)
Building not in the high velocity hurricane zone	
Building not in the wind-borne debris region	
Mean Roof Height	< 30 ft
Roof Angle	10-45 degrees
Components And Cladding Wind Pressures (FBC Table1606.2 B&C)	



Zone	Effective Wind Area (ft2) 10		100	
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6

Total Shear Wall Segments
2'-4"min for 8'-0"H wall 2'-10"min for 10'-0"H wall

	Transverse	Longitudinal
Required	34.5'	31.5'
Actual	66.5'	59.2'

All exterior walls are type II shear walls. ACTUAL SHEAR WALL length is the total of all wall segments with full height sheathing and width to height ratio greater than 1: 3.5 (plus special shear wall segments if noted.) REQUIRED SHEAR WALL length is from WFCM-2001, table 3.17A & 3.17B with table 3.17E adjustment for type II shear wall (or equivalent calculation) REV-27-Jun-03

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of record for the trusses and did not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work.
* Confirm that the foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise
* Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.
* Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately.
* Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.
* Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions; including interior bearing walls.
* Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder.

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 wind loads,, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.
This drawing is not valid for construction unless raised seal is affixed.

REV-06-OCT-03

REV-22-AUG-03

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disosway P.E.
POB 868, Lake City, FL 32056 Phone: (386) 754-5419
Fax: (386) 269-4871 Email: windloadengineer@bellsouth.net

Location: Lot 36 Wise Estates S/D Columbia Coutny, Florida

The Nathan 4 Bed
Lot 36 Wise Estates S/D

Builder: Ewpl, Inc.

Designer: DDS

Approved: FLPE#53915
26SEP05
26Sep05

Revisions:

Sheet S-1 of 2 Sheets
Windload Engineering

Job # 509232

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