ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TBT8228Z0122154601

Truss Fabricator: Anderson Truss Company

Job Identification: 7-290--Stephen Crawford Construc DANIELS -- , **

Truss Count: 37

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-CNBRGBLK-A11015EE-GBLLETIN-PIGBACKA-PIGBACKB-A11030EE-A11015EC-

#	Ref Description	Drawing#	Date
1	09851M1	07295024	10/22/07
2	09852A3	07295018	10/22/07
3	09853A6	07295003	10/22/07
4	09854A4	07295035	10/22/07
5	09855B2	07295005	10/22/07
6	0985681	07295006	10/22/07
7	09857 A8	07295030	10/22/07
8	09858F1	07295026	10/22/07
9	09859EJ8	07295015	10/22/07
10	09860AGE	07295001	10/22/07
11	09861EJ81	07295014	10/22/07
12	09862CJ7	07295008	10/22/07
13	09863HJ8	07295013	10/22/07
14	09864CJ5	07295009	10/22/07
15	09865CJ3	07295010	10/22/07
16	09866CJ1	07295011	10/22/07
17	09867A11	07295020	10/22/07
18	09868A1	07295012	10/22/07
19	09869A5	07295036	10/22/07
20	09870A7	07295002	10/22/07
21	09871AP2	07295029	10/22/07
22	09872AP	07295028	10/22/07
23	09873AP1	07295027	10/22/07
24	09874A12	07295031	10/22/07
25	09875C	07295016	10/22/07
26	09876C1	07295017	10/22/07
27	09877B3	07295025	10/22/07
28	0987884	07295022	10/22/07
29	09879BGE	07295021	10/22/07
30	09880 CGE	07295037	10/22/07
31	09881 D2	07295019	10/22/07
32	09882 - APGE	07295034	10/22/07
33 34	09883A10	07295004	10/22/07
35	09884A9	07295007 07295023	10/22/07
	09885 A2		10/22/07
36	09886 DOR	07295032	10/22/07

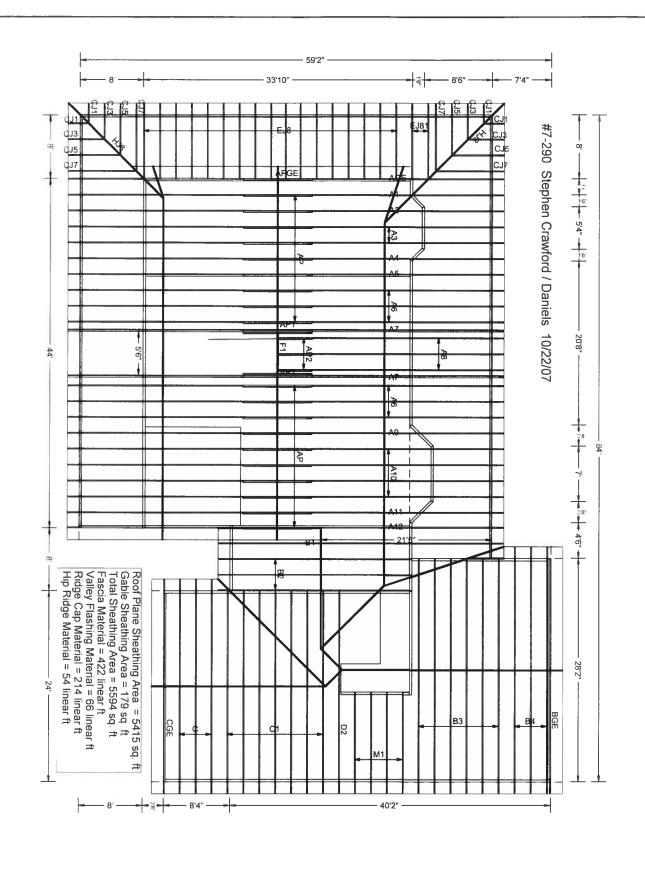
J.F.F.

Seal Date: 10/22/2007

-Truss Design Engineer-James F. Collins Jr. Florida License Number: 52212 1950 Marley Drive Haines City, FL 33844

#	<u>kei vescription</u>	Drawing#	<u> </u>
37	09887 DORGE	07295033	10/22/07
-			





1 OF 1 PAGE NO 7-290 JOB NO:

JOB DESCRIPTION:: Stephen Crawford Construc /: DANIELS

Calculated horizontal deflection is 0.11" due to live load and 0.19" due to dead load. Top chord 2x4 SP #2 Dense :T2 2x8 SP #1 Dense: Bot chord 2x8 SP #1 Dense Webs 2x4 SP #3 :Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500' PLT TYP. BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 11-0-0 . (A) Haines City, FL 33844
Ft Cartificate of Authorization # 0 278 Continuous lateral bracing equally spaced on 290 Stephen Crawford Construc DANIELS ALPINE Wave 1-0-14 L **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH IP: DR FABREACHIG. HANDLUGG, SHEPPIG. HISTALLING A BRACING OF FRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROPYISIONS OF THIS CHAITONAL DESIGN SPEC, BY AREA'N, AND IP!. IF BCG CONNECTOR PLAIES ARE HADE OF 20/18/16/66 (H.M.55/K) ASTH A653 GRADE 40/60 (H.K./H.55) GALV STECL. APPLY PLAIES TO LACH FACE OF TRUSS AND. HHLESS OHERSHISE LOCATED ON THIS DESIGN, POSITION PER DRAHIFMS 160A Z. ANY INSPECTION OF PLAIES FOLLOWED BY (I) SHALL BE FER ANHER X OF TPIT 2002 SEC. 3. A SEA, ON THIS DESIGN SHALL S **MARNUMG** RRISES REQUIRE EXPERE CARE IN FABRICATION, FAMOLING, SUPPING, INSTALLING AND BRACHE, RETER TO RESE (BUILDING COMPONENT SAFETY IN ORMATION), POBLISHED BY IPI (IRUSS PLATE INSTITUTE, ZIB HORTH LEE SHREET, SUILE 312, ALEXANDRIA, VA, ZZZIA) AND HICA (HOOD TRUSS COUNTEL OF WHEREA, 6300 CHIERPHSE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERHISE HOUGH, ALDISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERHISE HOUGH, ALDISON, HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION CHORD SHALL HAVE **WARNING** TRUSSES 1-6-0 8X8(E1) ≡ R=790 W=3.5" 3×4// Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)2 X 4 III 3×4/ 11-3-8 6X6**個** member 0ver 9 1.5X4 III 3×4≡ 2 Supports 12 R=1143 U=109 W=3.5" $\widehat{\mathbb{A}}$ 2.5X6 III 2×4 III Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ 9 ۵ SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-1.25 40.0 PSF 10.0 PSF 20.0 PSF 10.0 PSF 0.0 PSF DATE REF FROM SEQN DRW HCUSR8228 07295024 HC-ENG Scale =.25"/Ft. R8228- 9851 JB/AP 10/22/07

BUILDING OLSIGHER PER ANSI/TPI 1 SEC.

24.0"

JREF-

1TBT8228Z01

290 Stephen Crawford Construc DANIELS A3)

Bot chord 2x6 SP L chord 2x6 SP Webs 2x4 SP #22 :T1, T5 2x4 SP #2 Dense:

 $\widehat{\mathbb{A}}$ Continuous lateral bracing equally spaced on member

In lieu of structural panels use purlins to brace all flat TC $24\ensuremath{^{"}}\ensuremath{0\text{C}}.$

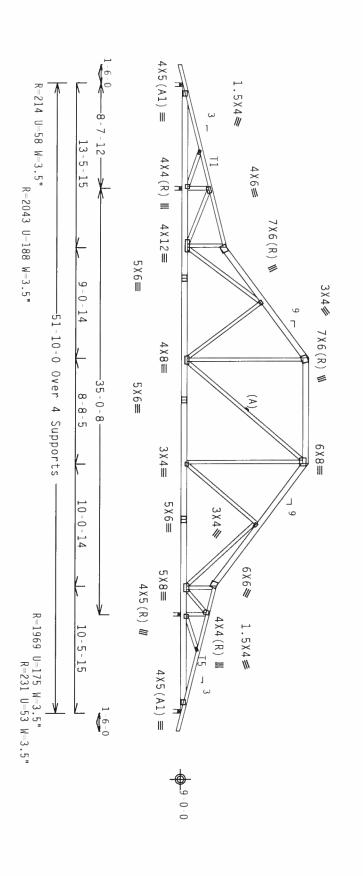
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WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



WARNING IRUSSES REQUIRE EXTREME CAME IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING.
RELER TO ESCI. (BUILDING COMPORENT SAFELY INFORMATION), PUBLISHED BY IFF (TRUSS PLATE INSTITUTE, 218
HORTH HEE STREET, SUITE 127. ALEXANDRIA, NA, 2231A) AND HICA (MODD IRUSSE COUNCEL OF AMERICA. 6300
ENTERPRISE LANE, MADISON, HI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNLESS
OTHERWISE INDICACITED TOP CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING. TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 7.36.0424

Design Crit:

PLT

TYP.

Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC, SHALL NOT UR RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BHILD THE FRANSE IN COMPORMANCE WITH PI, OR LABRICATION, AND DIG. SHEPPIG. HISALLING A BRACLING OF TRISSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THIS (MATIONAL DESIGN SPEC, BY ATRAD) AND FFI. ITH BCC COUNTECTOR PLAIRS ARE AND OF 70/19/16/AC, (H.H.5%) KEY, ASTH AGS GRADE 40/60 (H. K.H.5%) GALV. SHEEL, APPLY PLAIRS TO EACH FACE OF TRUSS, AND. DHESS OTHERNISE LOCALED ON THIS DESIGN, POSITION FIR BRAHHIGS 160A Z. ANY THIS PECTAL ON PROFESSIONAL ENGLISHED ON THE STORY SEC. 3. A SEAL ON THIS DESIGN ACCEPTANCE OF PROFESSIONAL ENGLISHERS REPRESENTED ANY TRISFECTION OF PROFESSIONAL ENGLISHERS REPRESENTED ANY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. ITH SUITABLE FOR ANY STATE OF SHOWN. ITH SUITABLE FER ANY THE SHOWN. ITH SUITABLE FER ANY THIS DESIGN ANY BUILDING IS THE RESPONSIBILITY AND DESCRIPTIONS.

Haines City, FL 33844
FI Conficate of Authorization # 0 279

ALPINE

CORIOR TATE O BC LL BC DL TC DL ַרר רר SPACING DUR.FAC. TOT.LD. 40.0 10.0 24.0" 1.25 20.0 10.0 PSF 0.0 PSF PSF PSF PSF DATE JREF-FROM SEQN-REF HC-ENG DRW HCUSR8228 07295018 R8228- 9852

JB/AP 23138

10/22/07

1TBT8228Z01

FL/-/4/-/-/R/-

Scale =.125"/Ft.

290 Stephen Crawford Construc DANIELS

A 6

l op Bot chord 2x6 SP #2 :11, T5 2x4 SP chord 2x6 SP #2 Webs 2x4 SP #3 #2 Dense:

Calculated horizontal deflection 0.18" due to dead load. is 0.12" due to live load and

In lieu of structural panels use 24° 0C. purlins to brace all flat TC **@**

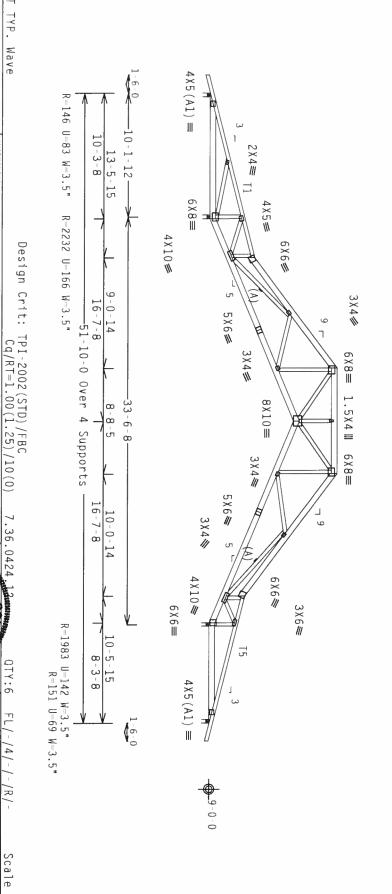
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC 0L=5.0 psf, wind BC DE=5.0 psf. Iw=1.00 GCpi(+/)=0.18

Wind reactions based on MWFRS pressures

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



TYP.

Wave

ITW Building Components Group, Inc.
Haines City, FL 33844
Ft Certificate of Amhorization # 0 279

BUILDING DESIGNER PER ANSI/IPI I SEC

ALPINE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR ITN BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FP; OR FABRICATHIN, INNOLUTE, SHEPPING, INSTALLING A BRACTING OF TRUSSES, DESIGN CONTREMS WITH APPLICABLE PROVISIONS OF DROS (MATIONAL DESIGN SEC, BY AFRA) AND IPI. ITR BCG CONNECTOR PLATES ARE MADE OF 20/18/16/66 (M.1/55/K) ASTH A653 GRADE 40/60 (M. K/M.55) GALV SILEL APPLY PLATES TO LACH FACE OF TRUSS AND. UNLESS ONHERHISL (GOATED ON THIS DUSING, POSITION PER DRAWFINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF THIS 20CEY FOR HIS DUSINGS ASCADED HIS SHALLS AND THIS SOLIAL FOR THE TRUSS CORPORATION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF THIS 20CEY FOR HIS BLEET OR SHALL BE PER ANNEX AS OF THIS 20CEY FOR HIS BLEET OR THE TRUSS CORPORATION.

CORION

BC DL TC DL

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR8228 07295003

JB/AP 23147

DATE REF

10/22/07

TC LL

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Scale =.125"/Ft. R8228- 9853

SPACING

24.0" 1.25

JREF-

1TBT8228Z01

FROM SEQN HC-ENG

DUR.FAC.

TOT.LD.

40.0

PSF PSF

0.0

MARNING IRUSSES REQUIRE EXTREME CARE IN FARRICATION, MANDEING, SHIPPING, INSTALLING AND BRACING, RETER TO REST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 MORTH LEE SIREET, SULTE 312" ALEXANDRIA, VA. 72314) AND HEAR (4000 TRUSS COUNCIL OF AMERICA, 6300 CHITERRISE LIAME, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE LUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD ETILLING.

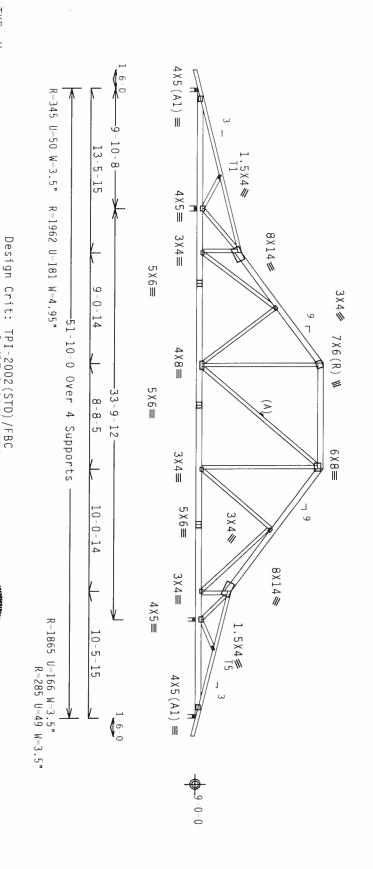
H op (A) Continuous lateral bracing equally spaced on member chord 2x6 SP #2 :T1, T5 2x4 SP chord 2x6 SP #2 Webs 2x4 SP #3 290 Stephen Crawford Construc DANIELS #2 Dense: A4) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 Wind reactions based on MWFRS pressures יממוודוורם מז זיימחת וווייי

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

In lieu of structural panels use purlins to brace all flat TC $24\ensuremath{\text{"}}\xspace$ 0C.

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HARNING IRVISES REQUIRE EXERTE CARE IN FABRICATION. HANDING. SHIPPING, INSTALLING AND BRACING. RELEA BE SHIPPING, CHOPOREN SAFITY INFORMATION POUR LINE OF FIFT (RRUSS PLATE INSTITUTE, 218 MONTH LIE STRUE, SUITE IZ 27 ALEXANDRIA, VA. 22-214) AND THE LIE STRUE, SUITE IZ 27 ALEXANDRIA, VA. 22-214) AND THE LIE STRUE AND SON, H 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST UNCTIONS. UNLESS CHILDREN AND SON H 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST UNCTIONS. UNLESS CHILDRENS INJUGACIÓN DIS CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARILES AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGID CELLING. TPI-2002(STD)/FBC __Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

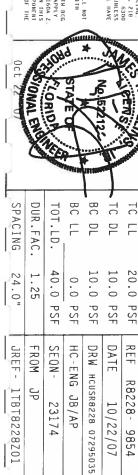
Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEGG, INC. SHALL NOT BE RESPONSIBLE FOR NAV DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH PET OR FAREFACTHIC, ANADIDED, SITE PET OR, THIS ALLING & BRACHIG OF TRUSSES.

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FI Corificate of Authorization # 0 779 Haines City, FL 33844

ALPINE



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R8228- 9854

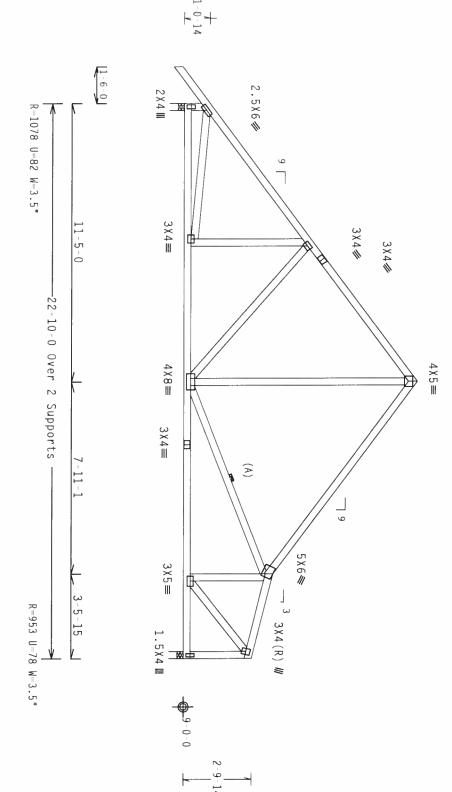
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Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 (A) Continuous lateral bracing equally spaced on member. 7 290 Stephen Crawford Construc DANIELS * 82) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



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Carrificate o	W Building C	/	✓ AL	\	\		PLT TYP. Wave
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tior # 0 778	Group, Inc.	<u> </u>					
FI Cartificate of A whatization # 0 779 BUILDING DESIGNER PER ANSI/IPI 1 SEC. 7.	ANY INSPECTION OF PLANES GLOBED BY (1) SINAL BE PER ANNEX AS OF IPI1 2002 SEC. 3. A SEAL ON HITS ANY INSPECTION OF PLANES SICHORED BY (1) SINAL BE PER ANNEX AS OF IPI1 2002 SEC. 3. A SEAL ON HITS ANY INSPECTION OF PLANES SINDER SINAL SINAL BE PER ANNEX AS OF IPI1 2002 SEC. 3. A SEAL ON HITS ANY INSPECTION OF PLANES SINDER SINAL SIN	CONNECTION CHAIRMAN AITH APPLICABLE PROFITAINS OF HIDS (MAILUMAN DESIGNA STEEL, BY AREA) AND FFT. ITH BEG CONNECTION FILES ARE AND OF 20/18/16/6A (MILESSEY) ASTH AGES GRADE 40/60 (M. K/MILES) GALV. SITELL APPLY PLATES 10 FACH FACE OF TRUSS AND, UNICESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAHINGS 160A Z.	BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMFORMACE WITH 1P1; OR FABRICATING, MANDILING, SHIPPING, INSTALLING & BRACING OF TRUSSES. TESTED FOR ONE WERE STITLE ABOUT TRUSTED FOR THE STATE OF THE	**IMPORTANT** FIRMISM A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM NGC. INC. SHALL NOT	ENTERPRISE LANE, HADISON, MI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OBJECTALS INDICATED TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE	**MARNING** FRUSES REQUIRE EXTREME CARE HE FARRICATION, HANDLING, SHIPPING, HUSIALLING AND BRACING. **EMARNING** FRUSES REQUIRE EXTREME CAPPORENT EXTENT LORGENATION), PUBLISHED BY FIR TRRUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SUITE 312, ALEXANDRA, VA. 22314) AND MICA GROOD RUSES CONNECT OF AFFERCA. 6300	Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.36.042
0ct 22 400	SONAL ENGINE	LONOT LE	STATE OF		1	S CONTRACTOR	24 DTY: 3
SPACING 24.0"	DUR.FAC. 1.25	TOT.LD.	BC LL	BC DL	TC DL	דכ רר	FL/-/4/-/-/R/-
24.0"	1.25	TOT.LD. 40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF DATE	20.0 PSF	-/-/R/-
JREF-	FROM JP	SEQN-		DRW H	DATE		Scale
JREF- 1TBT8228Z01	JP	23179	HC-ENG JB/AP	DRW HCUSR8228 07295005	10/22/07	REF R8228- 9855	Scale =.25"/Ft.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. (A) Continuous lateral bracing equally spaced on member. ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Conficate of Authorization # 0 779 7-290 Stephen Crawford Construc DANIELS 1-0-14 T ALPINE Wave -6-0 1.5X4 Ⅲ 2.5X6 R=1012 U=79 W=3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVALION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PLO REFLACES, ANY FAILUR AND FAILUR AND FAILUR FAILUR AND FAILUR **WARNING** IRUSSES HIQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING RETER TO BEST (BUILDING COMPONENT SAFITY INFORMATION), PUBLISHED BY IFF (TRUSS PLATE INSTITUTE, ZIB HORTH HEE STREET, SUITE 127, ALEXANDRIA, VA, 22314) AND HIGHOR OF HOSE COUNCIL OF AMERICA. 6300 CHIERRES'S LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERWISE HOUSEASTON OF THE SAFETY PRACTICES PRIOR TO PERFORM HIGHOR SHELLING. A PROPERLY ATTACHED RIGHD SELLING. 11-5-0 3 X 4≡ 3×4/ 3×5/ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 3 X 5 ≡ * -22 - 8 4 X 8≡ 5×5= В1) ·32-10-0 Over 3 Supports 7 - 11 - 1 8 9 8X14 1.5X4 III 3×5≡ Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 4 X 4≡ 7.36.0424 R-1537 U-134 W-3.5" 1.5X4 III 3 X 4 ≢ 13-5-15 CORIOR 3 X 4 (A1) ≡ R=385 U=66 W=3.5" 1 - 6 - 0SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/ 20.0 40.0 PSF 10.0 PSF 1.25 10.0 PSF 0.0 PSF PSF REF FROM SEQN-DATE DRW HCUSR8228 07295006 HC-ENG Scale = .1875"/Ft. R8228- 9856 JB/AP 23184 10/22/07

BUILDING DESIGNER PER ANSI/TPI 1 SEC

24.0"

JREF-

1TBT8228Z01

Bot In lieu of structural panels use purlins to brace all flat TC 24" $0\,\mathrm{C}_{\cdot}$. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Haines City, FL 33844
FI Contificate of Authorization # 0 278 ITW Building Components Group, Inc. chord 2x4 SP Chord 2x4 SP Webs 2x4 SP 290 Stephen Crawford Construc DANIE S ALPINE Wave 1 - 6 - 0#2 Dense #2 Dense #3 $2X4(A1) \equiv$ R=363 U=58 W=3.5" **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY JALUER TO BHILD THE TRREST IN COMPORNANCE WITH PICE OR FAREINGTHOM. HANDLING, SHIPPIG., HIS ALLING & BRACHEGO OF TRUSSES.

DESIGN CONFORMS HIT APPLICABLE PROYISIONS OF 1005 (MATIONAL DESIGN SPEC, BY ATAPA) AND IPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/16/CA (M.1857) ASTRONOMY AND THE STATES. AND THIS DESIGN SPECIAL APPLY PLATES TO EACH FACE OF TRUSS AND. HUNESS OTHERWISE LOCATED ON HIS DESIGN, POSITION FOR BRAHINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY C1) SHALL BE FER ANYELY AS OF IPI ZODOZ SEC. 3. A SEAL ON THIS DESIGN ACCEPTANCE OF PROFIESSIONAL ENGINEERING RESPONSIBILITY SOLELY OR THE TRUSS COMPONENT DESIGN SHOWN. HE SULFABLETTY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** IRUSSES REQUIRE CRIMINE CARE IN FÁBRICATION. HANDLING. SHIPPING, INSTALLING AND BRÁCING REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 HORTH LEE STRETT, SUITE 127, ALEXANDRA, VA. 22.114) AND HEAR (AUGDE TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAWL, MADISON, H. 53719) FOR \$AFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OHIGHENS HALL HAVE APROPURLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPURLY ATTACHED RIGHD CELLING. 10-1-12 10-3-8 1.5 X 4 ₩ Design Crit: 13-5-15 * A8 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) €X6≡ 3 X 4 ≡ R=1363 U=91 W=3.5" -26-9-8 Over 3 Supports **@** 4×8/ 5×6/1 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18 Wind reactions based on MWFRS pressures Shim all supports to solid bearing. Right end vertical not exposed to wind pressure. 3×4 **/** 9-0-14 3X4/ 16 - 6 - 04×8# 4×4≡ -2-11 BC DL SPACING DUR.FAC. ВС TC DL TC LL TOT.LD. R=597 U=103 FL/-/4/-/-/R/-1.5X4 Ⅲ 3×4≡ 40.0 24.0" 1.25 10.0 PSF 20.0 10.0 PSF 0.0 PSF PSF PSF _15-10-8 JREF DATE REF SEQN-HC-ENG DRW HCUSR8228 07295030 Scale = .25"/Ft. R8228- 9857 ۵ 1TBT8228Z01 JB/AP 23257 10/22/07

290 Stephen Crawford Construc DANIELS * F1

Bot Bot chord 2x4 SP #2 Dense chord 2x6 SP #1 Dense Webs 2x4 SP #3

110 mph wind, 22.67 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss must be installed as shown with top chord up

3 X 4 (R) Ⅲ

SPECIAL LOADS

-----(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From 60 PLF at 0.00 to 60 PLF at 5.50

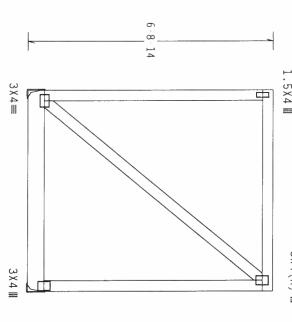
BC - From 20 PLF at 0.00 to 20 PLF at 5.50

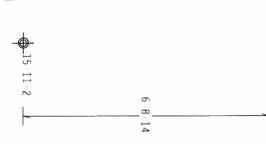
BC - 597 LB Conc. Load at 0.81, 2.81, 4.81 60 PLF at 5.50 20 PLF at 5.50 , 2.81, 4.81

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

OC in lieu of structural sheathing. The TC of this truss shall be braced with attached spans at 24"





R=1096 U=200 <-5-6-0 0ver \sim Supports -> R=1136 U=207

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

WARNING IRUSSES REQUIRE EXTRINE CARE IN FABRICATION. INABDING. SHIPPING, INSTALLING RECER TO BEST. (BUILDING COMPONEN SAFETY INFORMATION), PUBLISHED BY FET (TRIKS PLAIE IN 1001H LEE STREET, SUIDE 312. ALEXANDRIA, VA. 2231A) AND HIGA (AGOD TRUSS COUNCEL OF AND ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR OP PERFORMING INESE TUNCTIONERS SAIL INAVE PROPERLY ATTACHED BY COMBES SAIL INAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON OF A PROPERLY ATTACHED REGION CELLING.

ALPINE

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, I BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN TO THE INSTALLAR OF BUILD THE TRUSS IN COMFORE PLOOF FOR FARRICATHOR. HANDLING, SHIPPIRE, INSTALLING A BRAZELING OF TRUSSES.

DESIGN CONTORNS HITH APPLICABLE PROVISIONS OF DDS (MATIONAL DISIGN SPEC. BY AFRA) AND PLY. CONNECTOR PLATES ARE MADE OF 20/19/16GA (M.H/SS/K) ASIH MASS GRADE 40/50 (M. X/M.SS) GALY. LANES TO LATE ARE MADE OF 20/19/16GA (M.H/SS/K) ASIH MASS GRADE 40/50 (M. X/M.SS) GALY. LANES TO LATE ARE MADE OF 20/19/16GA (M.H/SS/K). ASIH MASS GRADE 10/50 (M. Y/M.SS) GALY. LANES TO THE SOURCES OBTERNISE (CGATED ON THIS DESIGN, POSITION PER BA ANY THESECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER'S AD THIS DESIGN SHOULD BE DESIGN SHOULD BE SHOULD BE DESIGN SHOULD BE SHOU

Haines City, FL 33844
FI Cartificate of Authorization #0 778 ITW Building Components Group, Inc. Haines City, FL 33844

BUILDING DESIGNER PER ANSI/IPI 1 SEC

0(0) 7.36.0424 PERMITTED OTY:1	FL/-/4/-/-/R/-	·/-/R/-	Scale =.375"/Ft.
INSTITUTE, 218	TC LL	20.0 PSF	REF R8228- 9858
OULESS 4	T'C DF	10.0 PSF DATE	DATE 10/22/07
No. Back	BC DL	10.0 PSF	DRW HCUSR8228 07295026
ORMANICE WITH	■ BC LL	0.0 PSF	HC-ENG JB/AP
DRAHINGS 160A Z.	TOT.LD.	40.0 PSF SEQN-	SEQN- 23339
ROSE COMPORENT SOS ONAL ENGRE	DUR.FAC.	1.25	
061 22 07	SPACING	24.0"	JREF- 1TBT8228Z01

Bot PLT Deflection meets t/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Haines City, FL 33844
Fi Conficate of Authorization # 0 079 TYP. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 -290 -- Stephen Crawford Construc DANIELS ALPINE Wave **IMPORTANT** FURBLES AS COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THG. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION ROOTHES DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PICTOR FARELY THE TRUSS IN COMPORMANCE WITH PICTOR FARELY THE APPLY PRICE OF THE SEC. 3. AREA AND THE APPLY PALES TO EACH FACE OF TRUSS. 3. AND THE APPLY PALES TO EACH FACE OF TRUSS AND. UNLESS OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DEATHORS 160A. Z. ANY INSPECTION OF PALES FOLORED BY (1) SHALL BE FOR NAMEZ AS OT FILE 2002 SEC. 3. A SEAL ON THIS DESIGN, POSITION OF THE ADMINISTRATION OF THE TOP THE THESE COMPONENT OF THE SOURCESSIONAL CHOINTERING RESPONSIBILITY FOR THE FRESPONSIBILITY OF THE BEST OCCUPIED THE SOURCE OF THE SOURCE **HARNIGG** FRUSE'S REQUIRE EXTREME CARE IN FABRICATION, HANDEING, SHIPPING, INSTALLING AND BRACING RETER TO RESE (BUILDING COMPONENT SAFETY IN ORBATION), PUBLISHED BY THE (TRUSS PLATE HISTITUE, 218 HORRH LEE STREET, SHIE 312, ALEXANDRA, NA, 2274)A DAN HERAC (HOOD TRUSS COUNCIL OF AMERICA, 6300 CHIERRE'SE LANE, HADISON, H 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS OUTCHASE AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION REGION SHALL HAVE A PROPERLY ATTACHED REGION OF REG ANY HISPICTION OF PLAIES FOLIORED BY (1) SMALL HE PER ANHEX AS OF TELL 2. DRAWING INDICALES ACCEPTANCE OF PROTESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER AUSI/IPI I SEC. 2. 1-6-0-> $2X4(A1) \equiv$ Design Crit: \overline{z} W =433 _ =60 W=3.5" € J8 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 8-0-0 Over 2 Supports Right end vertical not exposed to wind pressure. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 Wind reactions based on MWFRS pressures 7.36.042 R=309 U=37 1.5X4 III 1.5X4 III Φ SPACING BC DL DUR.FAC. BC LL TC DL TC LL TOT.LD. FL/=/4/-40.0 24.0" 1.25 10.0 PSF 20.0 PSF 10.0 PSF 0.0 -/R/= PSF PSF JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07295015 Scale = .5"/Ft. R8228 9859 1TBT8228Z01 CC/AP 23263 10/22/07

Bot Bearing blocks: Nail type: 10d_Box_or_Gun_(0.128"x3"._min.)_nails BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE 2 10.000' 1 1 2" 7 Rigid Surface Bearing block to be same size and species as bottom chord. Refer to drawing CNBRGBLK0207 for additional information. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation trusses. See "WARNING" note below. Truss spaced at 24.0" OC designed to support 1–4–0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Note: All Plates notched Wind reactions based on MWFRS pressures. See DWGS Al1015EE0207 & GBLLETIN0207 for more requirements PLT TYP. In lieu of FI Certificate of Authorization # 0 778 ITW Building Components Group, Inc. chord 2x4 SP #2 Dense :T3, T4, T5 2x6 SP chord 2x6 SP #2 Webs 2x4 SP #3 :W5, W8 2x4 SP #2 Dense: 290 Stephen Crawf rd Haines City, FL 33844 ALPINE structural panels use purlins to brace all flat TC Wave $4X5(A1) \equiv$ 9 R = 710Are + U = 10010-1-12 1.5X4 Except As Shown. **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR THE RCG, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, THE FRUST IN COMPORMANCE WITH PICE OR FARREACHING, AND LING. SHIPPING, INSTALLING A BRACHING OF THUSSES, DESIGN CONTRONS OF THE FROM **MARNING** IRUSELS REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 HORTH LET STREET, SUITE 312. ALEXANDRIA, NA. 27314) AND HEAL (HOOD TRUSS COUNCIL OF AMERICA. 6300 CHITERRISE LANE, HADISON, H. 53719) FOR SAFETY PRACTITES PRIOR TO PERFORMING THESE FUNCTIONS. DUELSS OTHERNISE LADICATED FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED RIGHED CELLING. BUILDING DESIGNER PER ANSI/IPI 1 SEC. n truc DANIELS W=3.52 X 4 III 8 X 8 ≡ 6 X 8 = -1-13 3 X 4 Ⅲ R=240 PLF U=27 PLF W=33-6-8 R=4123 U=499 W=3.5" 3×4/ 3×5≡ 3X4/ #2: Design Crit: 5 X 6 ≡ 10-5-0 16 - 119 AGE 51-10-0 located psf, wind ම TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) ò 7 X 6 (R) 24" 8 00 0ver = ВС É &-&-**T4** (A) (A) 4 Supports ဟ် 7X6 (R) The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuou lateral restraint to the gable end. All connections to be ###///designed by the building designer. BC - From 4 PLF at -1.50 to 4 PLF at -0.00
BC - From 20 PLF at -0.00 to 20 PLF at 51.83
BC - From 4 PLF at 51.83 to 4 PLF at 53.33
BC - From 4 PLF at 51.83 to 4 PLF at 53.33
PLT 1475 LB Conc. Load at (8.00.11.28), (43.83.11.28), (14.06.12.71)
PLT 309 LB Conc. Load at (10.06.11.28), (12.06.11.21), (14.06.12.71)
(16.06.14.21), (18.06.15.71), (20.06.17.21), (22.06.18.71), (24.06.19.10)
(25.92.19.10), (27.77.19.10), (29.77.19.10), (31.77.18.71), (33.77.17.21)
(35.77.15.71), (37.77.14.21), (39.77.12.71), (41.77.11.21)
BC Conc. Load at (8.06.9.04), (10.06.9.04) SPECIAL LOAD Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (A) Continuous lateral bracing equally spaced on member. MEMBER TO BE LATERALLY BRACED FOR WIND PERPENDICULAR TO TRUSS. BRACING SYSTEM BE DESIGNED AND FURNISHED BY OTHERS. 786≅ 10-10-14 9 61 65 65 61 61 20 DUR.FAC ONAL ENG 3×4〃 3×4// CORION 3X4 =4 X 4 (R) 8 42.17 43.83 5×6≡ .00 8-0-0 R = 502 $4X5(A1) \equiv$ E DUR.FAC.-1.25)
61 PLF at 11.66
65 PLF at 22.57
65 PLF at 421.26
65 PLF at 42.38
61 PLF at 43.83
61 PLF at 53.33 BC DL TC DL DUR.FAC. U=64 W=3.5" SPACING TOT.LD. FL/-1-6-0 LOADS /4/-/-/R/ continuous 40.0 10.0 10.0 20.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF JREF -SEQN-DATE DRW HCUSR8228 HC-ENG Scale R8228-1TBT8228Z01 =.125"/Ft. JB/AP 10/22/07 9860 07295001

Bot PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. ITW Building Components Group, Inc.
Haines City, FL 33844
Ft Cartificate of Authorization # 0 779 p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP 290 Stephen Crawford Construc DANIELS TYP. ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NGG, INC. SHALL NOT DE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FLARELATING, ANDIDING, SHIPPIG, HISTALLING & BRACHIG OF TRUSSES, DESIGN CONTROLATION, AND LICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPCC, BY ATEA) AND IPI. HISTOCOMPORED SHITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPCC, BY ATEA) AND IPI. COMPUTED SHAPLING TO PROTECTION OF THE SECOND OF THIS DESIGN, POSITION PER DRAWHIGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANHEX AS OF TPI1 2002 SEC. 3. A STAL ON THIS DESIGN SHAPLANCE OF PROFESSIONAL REGIONIFERING RESPONSIBILITY SOLELY FOR THE FURSE COMPONENT OF SECOND COMPONENT OF THE SECOND SHAPLANCE OF PROFESSIONAL REGIONIFERING RESPONSIBILITY SOLELY FOR THE FURSE COMPONENT OF THE SECOND SHAPLANCE OF PROFESSIONAL REGIONIFERING RESPONSIBILITY SOLELY FOR THE FURSE COMPONENT OF THE SECOND SHAPLANCE OF PROFESSIONAL REGIONIFERING RESPONSIBILITY OF THE PLATE OF THE PROFESSIONAL REGION OF THE SECOND SHAPLANCE OF THE SECOND SHA ***MARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING.

RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218

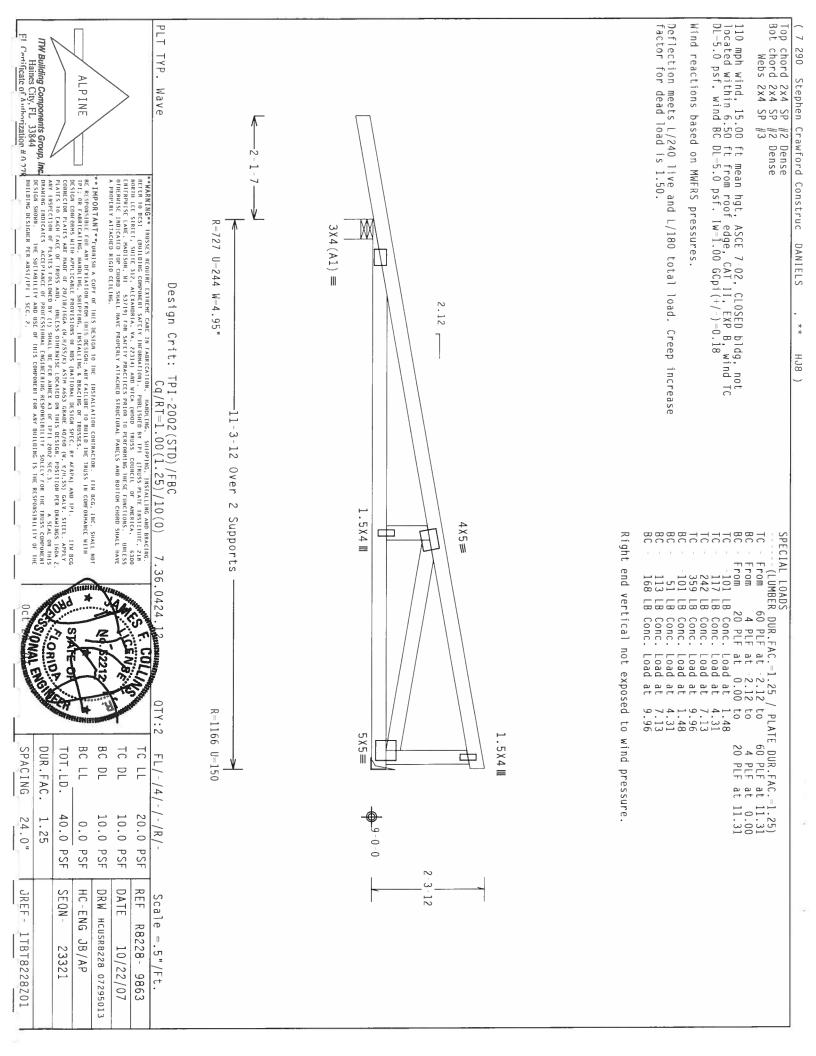
HORTH LEE STREET, SUITE 127. ALEXANDRIA, NA. 22314) AND HICA (MODO TRUSS COUNCIL OF AMERICA. 6300

ENTERPRISE LANT, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNIESS

BINERALSE (IDICALID FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD STRALL HAVE
A PROPERLY ATTACHED RIGID CELLING. BUILDING DESIGNER PER ANSI/IPI I SEC 1-6-0-2X4(A1) =Design Crit: R = 433N U=60 W=3.5" * EJ81) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -0-0 Over 2 Supports Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 Wind reactions based on MWFRS pressures R=309 U=37 CORIOR TATE O 1.5X4 III 1.5X4 III QTY:2 SPACING BC DL TC DL BC LL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 0.0 PSF PSF REF JREF DATE SEQN-HC-ENG DRW HCUSR8228 07295014 Scale =.5"/Ft. R8228- 9861 1TBT8228Z01 JB/AP 23270 10/22/07

Bot Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. ITW Building Components Group, Inc. Haines City, FL 33844 F1 Certificate of Amborization # 0.778 chord 2x4 SP chord 2x4 SP 290 Stephen Crawford Construc TYP. ALPINE Wave #2 Dense #2 Dense **IMPORTANT**FIRMISH A COPY OF 1015 DESIGN FO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAW DEVIATION PRODE THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH FILL OR FLARECATHO, HANDLING. SHEPPING, INSTALLING A BRACHING OF TRUSSES.

DESIGN CONTRECTOR PLATES ARE HADE OF 20/18/16/CA (H.H.5%X) ASTE HAS GRADE 40/50 (H.K./H.SS) GALV. SHIEL. APPLY PLATES TO EACH FACE OF TRUSS, AND. UNLESS OTHERWISE LOCATED ON HIS DESIGN, POSITION PER BRAHINGS 160A Z. ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. AS ANY HISPECTION OF PLATES TOLORED BY (1) SHALL BE PER AIMER AS OF THIS DOOZ SEC 3. THANKING** IRUSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, BLUER TO BCST. (BUILDING COMPONENT SAFETY IN ORBALION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 HORRH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 INTERPRETED THE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEROPHURE HIESE FUNCTIONS. UNITESS OHERWISE INDICATED TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE <u></u>1-6-0-✓ DANIELS 2X4(A1) =Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) R = 398U-60 W-3.5" w CJ7 -7-0-0 Over 3 Supports 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18 Wind reactions based on MWFRS pressures 130 CORIOR R-84 R=179 U=38 0 SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-14 10-9-6 9-0-0 1.25 40.0 PSF 10.0 PSF 20.0 PSF 24.0" 10.0 PSF 0.0 PSF JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07295008 Scale =.5"/Ft. R8228- 9862 1TBT8228Z01 JB/AP 10/22/07 23274



Top Bot PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. ITW Building Components Group, Inc. Haines City, FL 33844 Ft Contificate of Authorization #0 278 chord 2x4 SP chord 2x4 SP 290 Stephen Crawford Construc TYP. ALPINE Wave #2 Dense #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, IRC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FALLURE TO BHILD THE TRUSS IN COMPORMANCE WITH PPI: OR FAREACHING, HANDLING.

DESIGN CONTROLS SHE PROPICABLE PROVISIONS OF 1NDS (NATIONAL DESIGN SPEC, BY ATERA) AND TPI.

DESIGN CONTROLS ARE AND DE 20/18/18/GA (M.H.55X) ASTH ASS) GRADE 40/60 (M. K/M.SS) GALV. SIFEL. APPLY PLATES TO EACH FACE OF TRUSS. AND. JUNESS OTHERNISE LOCATED ON THIS DESIGN, POSITION FOR BRANHINGS 150A-Z. ANY HISPECTION OF PLATES TOLLOWED BY (1) SHALL BE FER ANNEX AS OT TPIT 2002 SEC. 3.

ANY HISPECTION OF PLATES TOLLOWED BY (1) SHALL BE FER ANNEX AS OT TPIT 2002 SEC. 3.

AS SAN OF THE SHALL AND THE SHALL AND THE TOWN OF THE TRUSS COMPONENT OF SHALL SHALL AND THE SHALL AS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF SHALL SHALL AND THE SHALL BY AND THE SHALL BY AND THE SHALL BE SHALL BE SHALL BY AND THE SHALL BE SHALL BY AND THE SHALL BE SHALL BY AND THE SHALL BE SHALL BE SHALL BY AND THE SHALL BY AND TH **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDIING, SHIPPING, INSTALLING AND BRACING, RECER TO BEST (BUILDING COMPOREN SACTEY INFORMATION), PUBLISHED BY TIP (TRUSS PLAIE INSTITUE, 218 HORTH LEE STREET, SUITE 312. ALEXANDRAL, NA, 223.14) AND HIGH CAG (MOOD TRUSS COUNCIL OF AMERICA, 6300 CHICENPLSE LUBE, MADISON, HI 53719) FOR SACETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS OTHERWISE LUBICATED FOR CHORD SMALL HAVE PROPERLY ATTACHED RIGID CEILING. **1** 6 0 √ DANIELS 2X4(A1) =Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) R-323 U-59 W-3.5" W 7 * -5-0-0 Over 3 Supports CJ5) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18 Wind reactions based on MWFRS pressures R=56 7.36.0424 R-121 U-26 SONAL ENGING 6 14 10-3-6 BC LL TC DL BC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-1.25 40.0 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF SEQN-DATE REF HC-ENG DRW HCUSR8228 07295009 Scale = .5"/Ft. R8228- 9864 JB/AP 10/22/07 23280

0ct 27

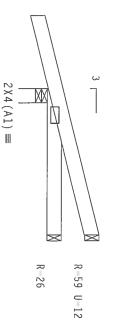
SPACING

24.0"

JREF-

1TBT8228Z01

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Top chord 2x4 SP Bot chord 2x4 SP -290 - Stephen Crawford Construc #2 Dense #2 Dense DANIELS CJ3) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures.





R=255 3-0-0 Over 3 Supports U-62 W-3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

MANNING RUSSES REQUERE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPORTH SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLAIF INSTITUTE, 218 MORTH LE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND MICAC (MODD TRUSS COUNCEL) OF AMERICA. 6300 THIERMESS LAWE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE HIDICATED NO COMPOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, BY TAILURE TO BUILD THE THISS IN COMPORMANCE WITH IP: OR FABRETATHAE, HANDLING, SHEPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CATALING, AND HELD THE PROVISIONS OF UNDS. (MAIDONAL DESIGN SECE, BY AFRA) AND IP! HE GEOMETICAN PRICE OF THIS ARE HADE OF 20/18/166A (M.H.)55/K) ASIM A653 GRADE 40/60 (M.K.)M.55) GAAV SIECEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OHERHISE LOCATED ON THIS DESIGN, POSITION PER DRAH/HADE SIGA A. ANY THIS PECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF FEIL 2002 SEC. 3. A SEAL ON THIS BESIGN AND ANY THIS PECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF FEIL 2002 SEC. 3. A SEAL ON THIS BRAHING INDICATES ACCEPTOMENT THE STORMAL THE SHALL AND USE OF THIS COMPONENT TO THE SECOND SHALL AND THE SHALL AND USE OF THIS COMPONENT TO BRAHING INDICATE ACCEPTOMENT TO THE SECOND SHALL AND THE SHALL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI 1 SEC.

ITW Building Components Group, Inc.
Haines City, FL 33844
El Certificate of Authorization # 0 278

ALPINE



DATE REF

10/22/07

Scale = .5"/Ft.

R8228- 9865

DRW HCUSR8228 07295010

23284

1TBT8228Z01

SPACING 24.0" 1.25 40.0 PSF 0.0 PSF JREF -SEQN-HC-ENG JB/AP

290 Stephen Crawford Construc DANIELS CJI

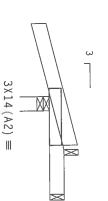
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Lanywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 CLOSED bldg, Located 5.0 psf, wind BC

Wind reactions based on MWFRS pressures.

R 51 U-33





R=10 U-1

1-6-0->k1 1-0> 2-0 R-238 U-87 W-3.5" 0 Over 3 Supports

MARNING IRUSSES REQUIRE EXIRENE CARE IN FARRICATION, JUANOLING, SHIPPING, INSTALLING AND BRACHIG, BETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (HRUSS PLANE INSTITUTE, 218 HORTH LEE SHREET, SUITE 132, ALEXANDERIA, WA, 22214) AND MECA (MOOD TRUSS COUNCIL OF ARERICA, 6300 CHIERRAISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO FERFORMING THESE FUNCTIONS. UNLESS OTHERNISE LINGLED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED RIGID CELLING.

PLT TYP. Wave

IMPORTANTTURNISH A COMY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THG. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE (DO BULLD THE TRUSS IN COMFORMANCE WITH PITTED FOR FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFIGERS WITH APPLICABLE PRBYSTONS OF BDS (MATIONAL DESIGN SPECE OF AFRA) AND TPI. THE BCG CONFICEROR PLATES ARE HADE OF 20/18/166A (M.H/SS/K) ASTH A653 GRADU 40/60 (M. K/M.SS) GALV. SIEEL APPLICATION OF FACE OF TRUSSES AND THE SOUTH AND THE SOUT

Haines City, FL 33844
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ALPINE

7.36.0424 CORION BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF

0.0 PSF

HC-ENG

JB/AP 23295

DRW HCUSR8228 07295011

SEQN-

DATE REF

10/22/07

Scale =.5"/Ft. R8228- 9866

QTY:4

1.25 24.0" JREF-1TBT8228Z01

-290--Stephen Crawford Construc DANIELS

Fop Bot chord 2x6 SP #2 :T1, T5 2x4 SP chord 2x6 SP #2 Webs 2x4 SP #3 #2 Dense:

Calculated horizontal deflection 0.19" due to dead load. is 0.12" due to live load and

In lieu of structural panels use purlins to brace all flat $24\ensuremath{^{"}}\ensuremath{0\text{C}}.$ TC

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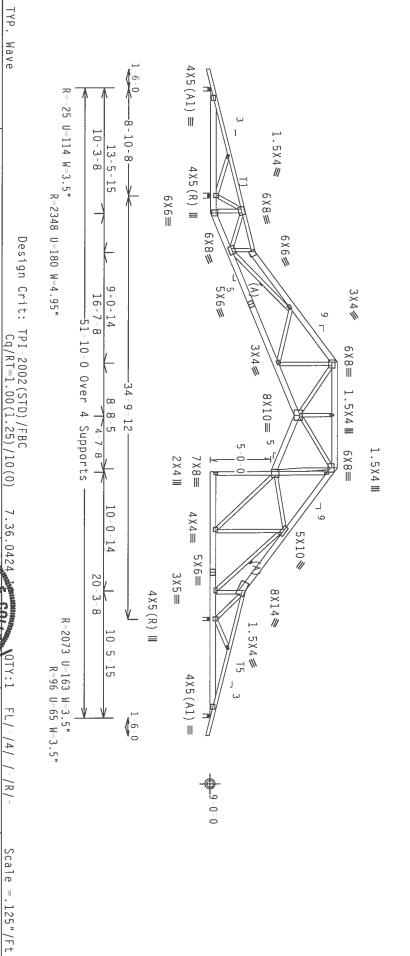
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/ $^{\prime}$)=0.18

Wind reactions based on MWFRS pressures

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



WARNING IRUSSES REQUIRE EXTRINE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFERE TO SEES. (BUILDING COMPORERY SAFETY INFORMATION), PUBLISHED BY FIF (IRUSS PLANE HISTIDUE, ZIB MRAIN LEE STREET, SUITE 312. ALEXANDRAIA, VA, 22314) AND HEAT (A (MODO TRUSS COUNCEL OF AMERICA. 6300 ENTERPRISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE (IDDICATED TOP CHORD SMALL MAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SMALL MAVE PROPERLY ATTACHED REGIONS OF STRUCTURAL PAWELS AND BOTTOM CHORD SMALL MAVE PROPERLY ATTACHED REGIONS OF STRUCTURAL PAWELS AND BOTTOM CHORD SMALL MAVE

TC DL BC DL

> 20.0 10.0 PSF

PSF

R8228-

9867

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PSF

DRW HCUSR8228 07295020

DATE

10/22/07

PSF PSF

HC-ENG

JB/AP

23301

JREF-FROM SEQN-

1TBT8228Z01

PLT

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEPLATION FROM HIS DESIGN. ANY FALURE TO BUILD THE TRISS. IN COMPORMANCE WITH FPI; OR FARELSTAING, MANDLING, SHIPPIG, HISTALLIGE & BRACHIGG OF TRUSSES. DESIGN AND THE PROPERTY OF THE STATE OF THE

ITW Building Components Group, Inc.
Haines City, FL 33844

Ft Contingeate of Authorization # 0 279

ALPINE

100t SONAL ENGINEE STATE OF BC LL SPACING DUR.FAC TOT.LD. 40.0 24.0" 1.25 0.0

290 Stephen Crawford Construc DANIELS A1

Bot chord 2x4 SP #2 Dense :T3, T4, T5 2x6 SP chord 2x6 SP #2 Webs 2x4 SP #3 12:

 Ξ Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ $24\ensuremath{^{\prime\prime}}$ OC.

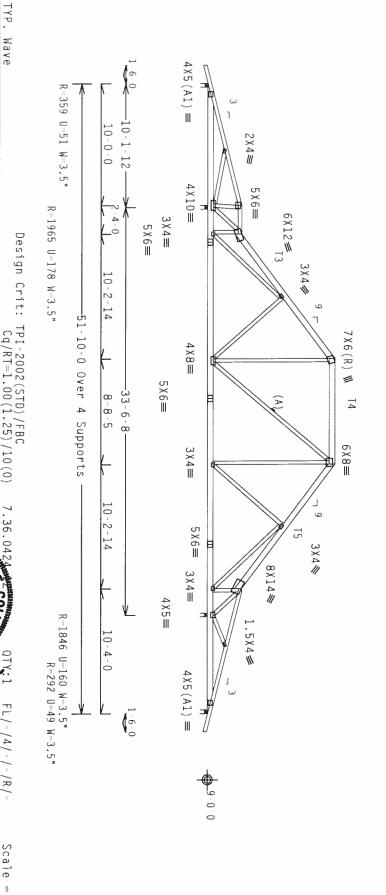
Wind reactions based on MWFRS pressures

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Lanywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Located d BC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping



NARNING TRUSS'S REQUIRE EXTREME CARE IN FARRICATION, "HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PIP (RUSS PLATE INSTITUTE, 210 HORTH LEE STREET, SUITE 317, ALEXANDRIA, VA, ZZ31A) AND BICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MAISSON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESS FINCTIONS. UNICESS OTHERSISE INACTION FOR SAFETY PRACTICES PRIOR TO PERFORMING INESS FINCTIONS. UNICESS A PROPERTY ATTACHED STRUCTURAL PARELS AND BRITCH CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARELS AND BRITCH CHORD SHALL HAVE

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Scale = .125"/Ft

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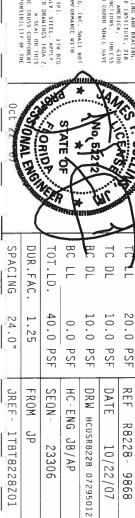
Wave

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IMPORTANT* INBITIST A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BOOK SHALL HAVE

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Top Bot chord 2x6 SP #2 :T1, T5 2x4 SP #2 Dense: chord 2x6 SP #2 Webs 2x4 SP #3 290 Stephen Crawford Construc DANIELS * Α5

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Continuous lateral bracing equally spaced on member.

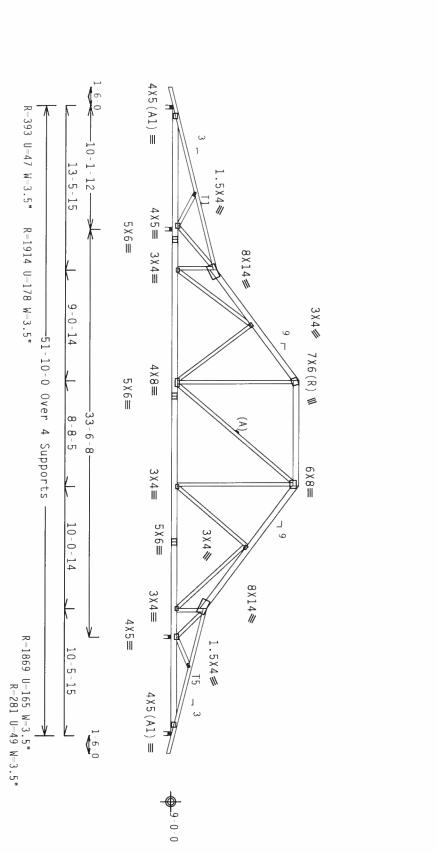
 Ξ

In lieu of structural panels use purlins to brace all flat TC @ $24\ensuremath{^{\prime\prime}}$ OC.

Wind reactions based on MWFRS pressures

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\cdot$



ITW Building Components Group, Inc. Haines City, FL 3844 Ft Contificate of Authorization # 0 778 ALPINE **IMPORTANT***URMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. THE, SHALL NOT BE RESPONSIBLE FOR ANY OUVALION FROM THIS DESIGN FOR FILES OF BRILD THE BRUSS IN COMPORMANCE WITH FPI; OR FARBLECKING, MANDLIG, SHIPPING, INSTALLING A BRACING OF BRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HDS (MATIONAL DESIGN SPIC, BY AFRY) AND FPI.

IN RECONNECTOR PLAIES ANE MORE OF ZO/18/16GA (M.1/SS/Z) ASTA ASSS GANDS 40/50 (M. K/H/SS) GALV. SIELL APPLY PLAIES TO LACH FACE OF TRUSS AND UNICES ONLERNIST LOCATED ON THIS DESIGN, POSITION FRE BRACHINGS 160A ANY INSPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER ANNEX AS DELIY ON THE PROPERTY AS ALL ON THIS DESIGN THE ACCESS CONTRACTOR OF THE BUSSES ONLY DO THE BUSSES **WARNING** IRUSSES REQUIRE EXIREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING RETER TO BEST QUILDING COMPONENT SATELY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, ZIB HORTH LEE STREET, SUIT ELTS, ALEXANDRIA, NA, 2214) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 CHIEDRRISE LAME, HADISON, HI 53/19) FOR SATELY PRACTICES PRIOR TO PERFORMING HESS FUNCTIONS. UNLESS OHIGHNISE HOLGATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) OZ SEC 3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT HE TRUS COMPONENT OF THE ORIO TC LL FL/-/4/-/-/R/-

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PLT TYP.

Wave

Design Crit:

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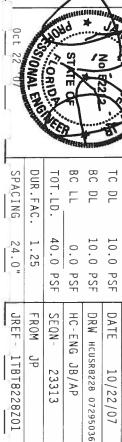
20.0

PSF

REF

9869

Scale = .125"/Ft R8228-



290 Stephen Crawford Construc DANIELS * AP2

chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.08 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details

SPECIAL LOADS

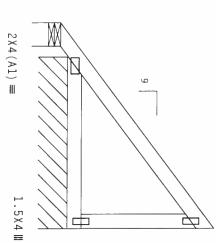
From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
rom 65 PLF at 0.00 to 65 PLF at 4.22
rom 4 PLF at 0.00 to 4 PLF at 4.22

Right end vertical not exposed to wind pressure.

In lieu of rigid ceiling use purlins to brace BC @

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 24" OC.

1.5X4 III



4-2-11 Over 2 Supports

70 U=69 W=5.833"

R=98PLF U=57 PLF W=3-6

ITW Building Components Group, Inc. Haines City, FL 33844 Ft Cartificate of Authorization # 0.279 TYP. ALPINE Wave

WARNING PRUSSIS REQUIRE EXTREME EARL IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELEATED BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (FRUSS PLATE INSTITUTE, 21B GORTH LEE SHERT, SUITE DIZ, ALEXANDREA, NA, 22214) AND MICHA (MODD TRUSS COUNCIL OF AMERICA. 6300 CHICERRISE LAME, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INCSE FUNCTIONS. UNIESS CHICERRISE LAME, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INCSE FUNCTIONS. UNIESS CHICERRISE LINGUATED TO PURIORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED RIGID CILLING. TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

Design Crit:

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, THC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION ROOM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPORANCE WITH PPI; OR FAREACHING, AND DIG. SHEPPIHG, HISTALLING A BRACHING OF TRUSSES. DESIGN CONTROLS OF THE PROPERTY OF THE PER ANNUAL AND THE SECOND OF PROPERTY OF THE SECOND OF THE PER ANNUAL AND THE SECOND OF THE PROPERTY OF THE PER ANNUAL AND THE SECOND OF THE PROPERTY OF THE PER ANNUAL AND THE SECOND OF THE PROPERTY OF THE PER ANNUAL AND THE SECOND OF THE PER ANNUAL AND THE PER ANNUAL AND

CORION SPACING BC LL BC DL TC DL L C DUR.FAC. TOT.LD. 40.0 20.0 1.25 10.0 PSF 24.0" 10.0 PSF 0.0 PSF PSF PSF JREF DATE REF SEQN-HC-ENG DRW HCUSR8228 07295029 R8228-1TBT8228Z01 JB/AP 10/22/07 23345 9871

FL/-/4/-/-/R/-

Scal e = .

.5"/Ft.

290 Stephen Crawford Construc DANIELS ΑP

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / #2 Dense #2 Dense #3

110 mph wind, 21.13 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

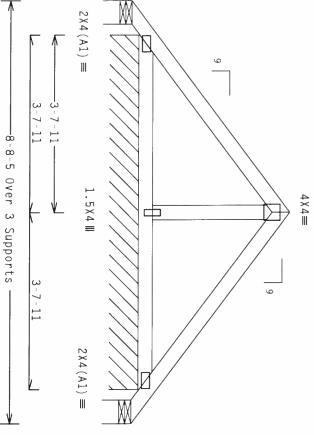
Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details

01 01 01 01 SPECIAL LOADS From From (LUMBER 65 PLF at 0.00 to 65 PLF at 4.35 to 4 PLF at 0.00 to DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
65 PLF at 0.00 to 65 PLF at 4.35
65 PLF at 4.35 to 65 PLF at 8.69
4 PLF at 0.00 to 4 PLF at 8.69

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



19-6-12

44 U=40 W=5.834"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 44

1 U=87

W=5.833" PLF U=32 PLF W=7-3-5

PLT

TYP.

Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION, MANDELING, SUPPRING INSTALLING AND BRACING RETER TO BEST (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY TIPI (TRUSS PLATE INSTITUTE, ZIB MORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22-214) AND MICHA (MORD TRUSS COUNCIL OF AMERICA. 6300 CHIERDRISE LINE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIEST FUNCTIONS. UNLESS OTHERNISE LIDICATED TO REDED SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGHT CELLING.

IMPORTANTDURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN FABLELIES BY TAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PT: OR FABRICATING, NHADLING, SHAPPING, HISTALLING A BRACING OF TRUSSES.

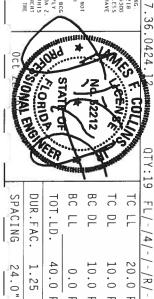
DESIGN CONTROCKATING, HANDLING, SHEPPING, HISTALLING A BRACING OF TRUSSES.

DESIGN CONTROCKS WITH APPLICABLE PROVISIONS OF HDS (MATIONAL DESIGN SPEC. BY ATAPA) AND TPI. ITH BCG CONTROCKS WITH APPLICABLE PROVISIONS OF HDS (MATIONAL DESIGN SPEC. BY ATAPA) AND TPI.

CONTROCKS OF THE ANALYSIS OF THIS SOURCE AND THIS DESIGN, POSITION PER DRAMBHINS 160A Z. ANY HISTECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX NO TPIL 2002 SEC.3. A SEAA ON HIST DRAMBHING INDICATES ACCUPANACE OF PROFESSIONAL ENGINEER HIG RESPONSIBILITY SOLILY FOR THE RESPONSIBILITY SOLILY FOR THE RESPONSIBILITY OF THE DRAMBHING INDICATES ACCUPANACE OF PROFESSIONAL ENGINEER HIG RESPONSIBILITY SOLILY FOR THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI I SEC

ITW Building Components Group, Inc. Haines City, FL 33844 Ft Contificate of Authorization # 0.278

ALPINE



40.0 PSF 10.0 PSF 24.0" 10.0 PSF 20.0 PSF 1.25 0.0 PSF JREF-DATE REF SEQN-HC-ENG DRW HCUSR8228 07295028 R8228-1TBT8228Z01 JB/AP 23349 10/22/07 9872

Scale =.5"/Ft.

Bot PLT Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. SPECIAL LOADS ITW Building Components Group, Inc. Haines City, FL 33844 FI Contificate of Amborization # 0 778 chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 290 Stephen Crawford Construc DANIELS TYP. -- (LUMBER DUR.FAC.=1.25 From 65 PLF at 0.00 From 65 PLF at 4.35 From 4 PLF at 0.00 ALPINE Wave -1.25 / PLATE DUR.FAC.=1.25)
0.00 to 65 PLF at 4.35
4.35 to 65 PLF at 8.69
0.00 to 4 PLF at 8.69 **IMPORTANT**FIRMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PI: OR FARRICALING, HANDLING, SHAPPING, HISTALLING A BRACHING OF FRUSSES. DESIGN CONTROLLING, THAT PICARLI PROVISIONS OF THIS CONTROLLING THE APPLICARLE APPLICABLE FOR THIS CONTROLLING THE APPLICABLE OF THE APPLICABLE OF THE APPLICABLE OF THE APPLICABLE APPLICABLE APPLICABLE APPLICABLE APPLICABLE APPLICABLE APPLICABLE APPLICABLE OF THE APPLICABLE APPLIC **WARNING** INVISES REQUIRE EXTREME CARE IN FARRICATION, INAMDENG, SHIPPING, INSTALLING AND BRACING RETER TO BEST. (BUILDING COMPOREN SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE STREET, SUITE ZIZ ALEXANDRIA, NA, 22213) AND METAC (MODO TRUSS COUNCIL OF AMERICA. 6300 CHICERRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OHICHMISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. 2X4(A1) =44 W=5.833" R=90 PLF W=7-3-5 9 Design Crit: 3-7-11 Creep increase φ AP1 ά TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) ဟ် 0ver 1.5X4 III 4 X 4≡ 卣 W Supports 9 Nailing Schedule: (10d_Box_or_Gun_(0.128"x3",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting. In lieu of rigid ceiling use purlins to brace BC @ COMPLETE 2X4(A1) =R 44 W=5.834" CORID TRUSSES REQUIRED BC LL ВС TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/ - /R/ -20.0 40.0 PSF 10.0 PSF 10.0 PSF 1.25 0.0 PSF 24" OC. PSF SEQN-DATE HC-ENG DRW HCUSR8228 07295027 REF Scal le =.5"/Ft. R8228- 9873 JB/AP 23353 10/22/07

BUILDING DESIGNER PER ANSI/IPI I SEC

SPACING

24.0"

JREF-

1TBT8228Z01

Hop Hop contractor. Special care must and installation of trusses. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. SPECIAL LOADS Note: All Plates (8) Wind reactions PLT TYP. Haines City, FL 3844
FI Contificate of Authorization # 0 278 chord 2x4 SP #2 Dense :T4, T5 2x6 SP #2: chord 2x6 SP #2 Webs 2x4 SP #3 (1) 2X4X FULL LENGTH OVERHANG SP#2 SCAB; ATTACH TO ONE FACE OF TRUSS WITH 10d BOX(0.128"X3.0") NAILS @ 2" OC WITHOUT SPLITTING LUMBER. 290 Stephen Crawford Construc DANIELS From (LUMBER ALPINE Wave DUR.FAC.=1.25 based שר ב ה ה ה ה ה 4X5(A1) =9 R = 398on Are 10. MWFRS pressures. U=88 W=3.5" 00 50 1.5X4 Except As Shown. **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE FO DRILLD THE FRUSS IN COMPORMANCE WITH TPI; OR FARBLECKTHO. HANDLE PROVISIONS OF 305 (HATIONAL DESIGN SEC, S. 4787A) AND TPI. THE BCG CONNECTOR PLATES ARE HADE OF 20/18/166A (H.M.55/K) ASTH A653 GRADE 40/60 (H. K/M.55) GALV. SIETL APPLY PLATES TO LACH TACT OF TRUSS AND. DULESS OHIERISE (DOALED ON THIS DESIGN, POSITION PER DRAWHROS 160A Z. ANY THIS PECTAGO THE ACT OF THE SEC SAME HADE OF COMPOSEND THE SEC 3. ANY THIS PLATES ACCEPTANCE OF PREFESSIONAL GROUND THE SEC 3. AS SAME THIS DESIGN SHOWN.

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DRAWHROE INDICATES ACCEPTANCE OF PROFESSIONAL GROUND THE SEC 3. AS SAME ON THIS DESIGN SHOWN.

THE SULTABBLETTY AND DESCRIPTION OF THE SULTABBLETY OF THE .0-1-12 OTHERWISE HIDIEATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CETALING. **MARNING** HRUSSIS REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACE REFER TO BEST. (BUILDING COMPONEN SACIFY INFORMATION). PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, HORNING THE STREET, SHITE JIS, ALEXANDRAL, VA, ZEJJA) AND NICA (MOOD TRUSS COUNCIL OF MERICA, HORNING LIES STREET, SHITE JIS, ALEXANDRAL, VA, ZEJJA) AND NICA (MOOD TRUSS COUNCIL OF MERICA, BUILDERFORMED LANGE, SHITE, SHITE JIS, ALEXANDRAL, VA, ZEJJA) AND NICA (MOOD TRUSS COUNCIL OF MERICA, UNICEPHISE LANGE, MADISON, NI SJ719) FOR SACELY PRACTICES PRIOR TO PERFORMING HEST FUNCTIONS. UNICEPHISE LANGE MERICAL PROPERTY OF THE PR 10 - 3 - 8BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2 X 4 ≡ t000 to 13-5 PLATE -15 BUR.FAC 7 X 8 ≡ 4 X 5 ≡ R=2395 U=281 W=3.5" 41.34 49.52 53.33 0.00 10.29 17.86 26.92 4X10 € Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)@9X9 4 9 5 X 6 ₪ 15 4×8/ 16-7-8 A12 ____22-6-8 __3-30311-12 9 51-10-0 3×4/ 3X4 ≤ €X8≡ 0ver 8X10= α ά 7 Supports 5-7-8 6 X 8≡ 5 X 8 = .5X6 Ⅲ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 R=2341 U=185 W=3.5" R=177 R=85 PLF U=20 PLF W=10-8-8 The building designer is responsible for the design of the roof and celling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer. Truss spaced at 24.0° OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. ₹5-6-0-¥₹5-6-0-¥₹⁴⁻⁰⁻⁰¥ Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ In lieu of structural panels use purlins to brace all flat TC @ 24" OC. See DWGS All015EE0207 & GBLLETIN0207 for more requirements (A) Continuous lateral bracing equally spaced on member. UNLESS 4 X 6 // MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS. 10-0-14 3X4// 4X4(R) // 4 X 8 == 8 ONAL ENGLAS CORIOR 19-3 TATE 8X14// 3 X 4 = 8-1-6 U=76* 3 \ 4 \ ₩ 0 ¥=3. 9 BC LL TC DL SPACING DUR.FAC. TOT.LD. TC FL/-/4/-1-6-0 R-42 PLF U=5 PLF W=7-8 D. (B) / - /R 40.0 10.0 20.0 10.0 PSF 24.0" 1.25 0.0 PSF PSF PSF PSF R-480 U-118 W-3.5" DATE JREF -REF DRW HCUSR8228 07295031 HC-ENG Sca æ R8228-1TBT8228Z01 =.125"/Ft. JB/AP 10/22/07 9874

Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP SS:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500' BC attic room floor loading: LL = 40.00 psf; DL = $6 \cdot 0 \cdot 0$ to $18 \cdot 0 \cdot 0$. Calculated horizontal deflection is 0.16" due to live load and 0.29" due to dead load. PLT TYP. ITW Building Components Group, Inc.
Haines City, FL 33844
Ft Cartificate of Authorization # 0 779 290 Stephen Crawford Construc DANIELS ALPINE Wave 1-6-0 8X8(E1) = R=1948 U=86 W=3.5" RETER 10 BCS.1 (BUILDING COMPONENT SACTIFY HIRMANION), PUBLISHED BY PI (TRUSS PLATE INSTITUTE, 219
1081H LEE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND HEGA (MODD TRUSS COUNCIL OF AMERICA, 6300
1011ERPSIS. LAME, HADISON, HT 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
01HERRISE (INDICATED IDE COROS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING. ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RUDESTIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT I BUILDING OF SIGNER PER AUSI/TPI 1 SEC. €X6≡ 8X10(R) / 1.5X4 W 11-1-9 6X8**≡** 3X12 III Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) .2-0-0 10.00 psf; from -24-0-0 Over 2 Supports 4 X 5 == ∞ 1-8-13 -12 ВЗ 4 X 5 == 0-0-5X10(R) III 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Wind reactions based on MWFRS pressures. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 8-1-9 6X8≡ 8X10(R) 1.5X4 III CLORNO 6X6**≡** R=1948 U=86 W=3.5" 3-0-0 8X8(E1) = 3X4// 1-6-0 SPACING BC DL TC DL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 24.0" 1.25 0.0 PSF JREF-REF FROM SEQN-DATE HC-ENG DRW HCUSR8228 07295016 Scale = .25"/Ft. R8228- 9875 1TBT8228Z01 JB/AP 23389 10/22/07

BC attic room floor loading: LL = 40.00 psf; DL 6-0-0 to 18-0-0 . Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP SS:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500' Calculated horizontal deflection is 0.16" due to live load and 0.29" due to dead load. PLT TYP. Haines City, FL 33844

Fi Cartificate of Authorization # 0 279 ITW Building Components Group, Inc. 290 Stephen Crawford Construc DANIELS ALPINE Wave 8X8(E1) = R=1840 U=69 W=3.5" 3X4/ 6 X 6≡ **IMPORTANT***TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, THG. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE OF BUILD THE THUSS IN COMPORMANCE WITH IP: OR FOREIGN. THE THUS OF THE STATE OF THE STATE.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY ATATA) AND IPI. ITH BEG CONNECTOR PLATES ARE ANDE OF 20/18/16/66, (M.H.SS)R, SALM HASS GROBE 40/50 (M. X/M.SS) GALV. SIEELA APPLY PLATES TO LACH FACE OF THUSS AND. UNLESS OTHERSTALL COATED ON THIS DESIGN, POSITION PER DRAWHING SHOW. A ANY INSPECTION OF FALTES FOLLOWED BY (I) SHALL BE PER ANNEX SO TIPIT 2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SULVEY AND THE MASS COMPONENT OF THE STATE OF THE STA **WARNING** TRUSSES REQUIRE EXTREME CARE IN LABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPOSED SAFETY INFORMATION), PUBLISHED BY THE LIREST, SUITE 312. ALEXANDRIA, VA. 22314) AND MICHA (MODD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAME, MADISON, WI 53710) FOR SAFETY PRACTICES PRIOR TO PERFORMING HESE FUNCTIONS, UNLESS OHHERMISE LIDICATED TO PERFORM A MADISON, WI ESSI OHHERMISE LIDICATED TO PERFORM A MADISON, WILLIAM FOR SAFETY PRACTICES PRIOR TO PROPERTY ATTACHED STRUCTURAL PARTIES AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARTIES AND BOTTOM CHORD SHALL HAVE BUILDING OFSIGNLR PER ANSI/IPI 1 SEC. 8X10(R) / 1.5X4 III 11-1-9 6X8≡ 3 X 1 2 III 12-0-0 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 10.00 psf; from 24-0-0 C1) 4 X 5 == Over 2 Supports 8 1-8-13 0 12 83 0 4 X 5 **=** 5X10(R) | Ö T3 8-1-9 9 Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 3 X 1 2 III 6X8≡ 7.36.0424 H 8X10(R) 1.5X4 III 0ct €X6≡ ONAL ENGINEE R=1951 U=87 W=3.5" 8X8(E1) =3×4∥ 1-6-0 SPACING BC LL BC DL TC LL F DUR.FAC. TOT.LD. FL/-/4/-/-/R/-10.0 PSF 40.0 PSF 20.0 PSF 24.0" 10.0 PSF 1.25 0.0 PSF JREF-REF FROM DATE SEON DRW HCUSR8228 07295017 HC-ENG Scale = .25"/Ft. R8228-1TBT8228Z01 JB/AP 10/22/07 23398 9876

:T4 2x4 SP #2 Dense:
Bot chord 2x8 SP #1 Dense :B2 2x8 SP :B3 2x4 SP #2 Dense:
 Webs 2x4 SP #3
:Rt Slider 2x4 SP #3:
Lt Wedge 2x4 SP #3: PLT TYP. Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. ITW Building Components Group, Inc.
Haines City, FL 33844

F1 Configure Continuity Haines City (1) chord 2x8 SP SS :T1 2x6 SP 290 Stephen Crawford Construc ALPINE Wave 1-0-14 7 **IMPORTANT***URHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE 10 BULLOT THE TRUSS IN COMPORMANCE WITH PL. OR FARBEACHING. HANDLING. SHIPPIDE, INSTALLING A BRACHING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF UDS (MATIONAL DESIGN SPEC, BY AFRA) AND IPI. THE GOODWICE COMPORTS HIT APPLICABLE PROVISIONS OF UDS (MATIONAL DESIGN SPEC, BY AFRA) AND IPI. APPLY PLATES TO EACH FACE OF TRUSS AND. DURESS OTHERNISE LOCALED ON THIS DESIGN, POSITION PER DOWNHAS 160A Z. ANY THIS ECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANKEX 3 OF IPI1 2002 SEC. 3. A SLAL ON THIS DESIGN SHOWN. THE SHALL OF PROVISIONAL THE SHALL BE PER ANKEX 3 OF IPI1 2002 FOR THE THESS COMPONENT DESIGN SHOWN. THE SHALL BE PER ANKEX 3 OF IPI1 2002 THE TRUSS COMPONENT DESIGN SHOWN. THE SHALL BE PER ANKEX 3 OF IPI1 2002 THE RESPONSIBILITY OF THE REFER TO BESS. (BUILDING COMPONENT SAFETY INFORMATION), INAULING, SUIPPING, INSTALLING AND BRACING, UNDER THE SESS. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FT (FRUNS PLATE INSTITUTE, 218 UNDER THE STREET, SUITE 127. ALEXANDEAL, VA. 232.14) AND HIGA (MODD TRUSS COUNCEL OF AFERCA. 6300 CHIEGRAPISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TURCTIONS, INNESS OTHERWISE HOLGALID TOP CHORD SHALL HAVE PROPERLY ATTACHED RIGHD CELLING. BUILDING DESIGNER PER ANSI/IPI I SEC 1-6-0 6X6(B2) =DANIELS R=2327 U=101 W=3.5" SS: 2.523 6X6**個** Design Crit: 2.5X6 2×4 III 6×6≡ 6 X 8 = 3 X 1 2 Ⅲ ВЗ 4-1-0 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 9 28-2-0 Over 2 Supports 82 $3 \times 5 =$ 13 ∞ ò 5X12(R) III 5-10-13 -12 1₿₿X4 III 3X5**≡** BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 20-0-0 . Calculated horizontal deflection is $0.15\mbox{"}$ $0.26\mbox{"}$ due to dead load. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures. 3-11-9 7.36.042 6X8(R) Ⅲ 3X12 III 9 6X6≡ 6×6// SONAL ENG! CORIO 1.5X4 Ⅲ 3×4// -2-0 R=2071 U=81 W=3.5" 3×4// QTY:6 8X8(E1) = SPACING BC LL BC DL TC DL DUR.FAC. TOT.LD. TC LL FL/-/4/-/-/R/due to live load and 40.0 PSF 10.0 PSF 1.25 10.0 PSF 20.0 PSF 0.0 PSF FROM SEQN-DATE REF HC-ENG JB/AP DRW HCUSR8228 07295025 Scale =.1875"/Ft. R8228-23651 10/22/07 9877

100t

24.0"

JREF -

1TBT8228Z01

Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. :T4 2x4 SP #2 Dense:
Bot chord 2x8 SP #1 Dense :B2 2x8 SP SS:
B3 2x4 SP #2 Dense:
 Webs 2x4 SP #3
:Rt Slider 2x4 SP #3:
Lt Wedge 2x4 SP #3: Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\cdot$ PLT TYP. Haines City, FL 33844
Fr Conficate of Authorization # 0.079 290 - Stephen Crawford Construc chord 2x8 SP SS :T1 2x6 SP #2: 1 - 0 - 14ALPINE Wave 1-6-0 6X6(B2) = R=2325 U=102 W=3.5" 6X6**個** **IMPORTANT***URNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. INC. SHALL HE RESONANCE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAITHER TO BRITCH THE RUSSE IN COMPORMANCE WITH PI: OR LARRICATIO. HANDLING. SHAPPING. HISTALLING & BRACHEG OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROPYISIONS OF DBS (HALTONAL DESIGN SPEC. BY AFRA) AND FPI. DESIGN CONFORMS WITH APPLICABLE PROPYISIONS OF DBS (HALTONAL DESIGN SPEC. BY AFRA) AND FPI. COMMICCION PIATES AND AND OF 70/180/1804 (HALTONAC MISSIONAL DBS GRANE MADOS OF 70/180/1804 (HALTONAC MISSIONAL DBS GRANE MADOS OF 70/180/1804 (HALTONAC MISSIONAL DBS GRANE MADOS OF THE ABOUTTO OF THE ARCHADOL OF THE **MARNING** IRUSSES REONIRE EXIRENE CARE IN FARREALION, HANDING, SUPPHIG, INSTALLING AND BRACHIG. REFERE TO BEST (BUILDING COMPORENE SAFTY INFORMATION), PORLISHED BY THE (HAUSS PLATE INSTITUTE, 218 1072). ALEXANDRAL, VA, 22314) AND WICA (MODD IRUSS COUNCIL OF AMERICA. 6300 INTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES BRIOR TO PERIORMING HIESE FUNCTIONS. UNLESS OTHERWISE HINDEACTOR OF GROUND HIS SAFETY PRACTICES BRIOR TO PERIORMING HIESE FUNCTIONS. UNLESS OTHERWISE HINDEACTOR OF GROUND HIS SAFETY PRACTICES BRIOR TO PERIORMING HIGH CHORD SHALL HAVE A PROPURLY ATTACHED BRIGHD CHORD SHALL HAVE PROPURLY ATTACHED RIGHT CHORD SHALL HAVE A PROPURLY ATTACHED RIGHT CHORD SHALL HAVE DRAWING INDICATES ACCEPTANCE OF PROF DESIGN SHOWN. THE SUITABILLITY AND BUILDING DESIGNER PER ANSI/TPI 1 SEC. DESIGN CONFORMS WITH AMPLICABLE PROVISIONS OF NDS. (MAITONAL DESIGN SPEC. BY AFAPA) AND IPI.
CONNECTOR PLATES ARE MADE OF 20/101/166A (M.M.1/55)K) ASTH A653 GRADE 40/50 (M.K./M.55) GALVE STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNICES OTHERWISE LOCATED ON THIS DESIGN, POSTITION PER ROMANINGS 160A 2
ANY INSPECTION OF PLATES OFLOWED BY (1) SHALL BE PER AMHEX A3 OF IPI1 2002 SEC.3. A SEAL ON THIS
DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT 2.5X6W 2 X 4 III 6X6≡ 11-1-9 3 X 1 2 III 6X8**≡** DANIELS Design Crit: 9 28-2-0 Over 2 Supports 3X5≡ 82 œ 13 ò 5 X 1 2 (R) III TPI-2002 (STD) /FBC Cq/RT=1.00 (1.25) /10(0) 5-10-13 12 B⊉.5X4 III Ö 3×5≡ 3-11-9 6X8(R) Ⅲ 3X12 III FOR THE TRUSS COMPONENT HE RESPONSIBILITY OF THE ₽ 6X6≡ 6×6// 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 1.5X4 III BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 20-0-0 . Calculated horizontal deflection is 0.14" due to live load 0.26" due to dead load. Wind reactions based on MWFRS pressures. 3 X 4 🕼 SHALL NOT 7.36.042 T4 ل و 'n 3X4// R=2171 U=62 W=3.5" 8X8(E1) ≡ CORIO MALEN 1-6-0 QTY:3 ВС TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-2 40.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF DATE REF JREF- 1TBT8228Z01 SEQN-HC-ENG FROM DRW HCUSR8228 07295022 Scale =.1875"/Ft. R8228- 9878 JB/AP 23657 10/22/07

Bot :B3 End :Top Note: All Plates Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Collar tie braced with continuous lateral bracing rigid ceiling. Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chomust not be cut or notched. PLT TYP. ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 0.278 chord 2x8 SP #1 Dense 2x4 SP #2 Dense: Webs 2x4 SP #3 verticals not exposed to wind pressure 290 Stephen Crawford Construc chord 2x4 SP #2 Dense 2x8 SP <u>|</u> |-0-14 ALPINE Wave 4 X 1 0 III 9 Are 1.5X4 Except As Shown. R=309/-205 3×4/ **IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NEG. THE C. SHALL NOT BY RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BHILD THE TRUSS IN COMPORMANCE WITH PILO OR FARRICATING, AMOUNTING, SUMPPING, HISANCILLING A BRACHEG OF TRUSSES, OR AN ATALA) AND TELL DESIGN CONTRONS OF THE APPLICABLE PROVISIONS OF ANDS. (MALIONAL DESIGN SPECE, BY ATALA) AND TELL THE CONTROL OF THE APPLICABLE PROVISIONS OF ANDS. (MALIONAL DESIGN SPECE), BY ATALA) AND TELL APPLICABLE PROVISIONS OF ANDS. (MALIONAL DESIGN SPECE), BY ATALA) AND THE APPLICABLE OF THE AP DRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMP BUILDING DESIGNER PER ANSI/IPI I SEC. 2. :13 :B2 2x8 SP SS: PROPERLY ATTACHED RIGID CEILING. 3×4/ 2x8 SP 7Jd 6×6/ 4X12(R) III 6 X 6 ≡ U=31 PLF W=5-0-0 R=241 PLF U=12 PLF 13-0-0 DANIELS #1 Dense: 2-4-10 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 9 28-2-0 a t В2 W=16-0-0Top chord BGE 3 ∞ Over 3 Supports 0 ò 4X8(R) III 4-11-3 0 12 ВЗ BRITHING 4X12(R) Ⅲ 4-5-7 R=308 PLF T4 2 X 4 III € X 6 ≡ 3×4// Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 19-0-0. See DWGS Al1015EE0207 & GBLLETIN0207 for more requirements Negative reaction(s) of -1024# MAX. (load case requires uplift connection. U=20 PLF W=7-2-0 6×6// .36.0424 'n 10CE 3X4// S/ONAL ENG! CORIOR -8-6 1-6-0 4 X 1 0 Ⅲ -1024# MAX. (See below) from a non-wind 9-0-0 BC LL DUR.FAC. ВС TC DL TC LL SPACING TOT.LD. FL/-/4/-2 / - /R/ 40.0 10.0 20.0 10.0 PSF 24.0" 1.25 0.0 PSF PSF PSF PSF JREF-FROM SEQN-DATE REF DRW HCUSR8228 07295021 HC-ENG Scale =.1875"/Ft R8228-1TBT8228Z01 JB/AP 10/22/07 9879

Bot Truss spaced at 24.0" OC designed to support 1–4–0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord Collar tie braced with continuous lateral bracing at 24" OC. rigid ceiling. must not be cut or notched. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ PLT TYP. Note: All Plates ITW Building Components Group, Inc. Haines City, FL 33844 FI Cartificate of Amborization # 0.278 chord verticals not exposed to wind -290 -- Stephen Crawford Construc ALPINE 2×4 2×8 2×4 Wave 444 4 X 1 0 III #2 Dense :T3, T4 2x8 SP SS: #1 Dense :B3 2x4 SP #2 Dense: #3 Are R=220 PLF -8-6 1.5X4 **IMPORTANT**THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH THE THE REAL THAT AND A THE PROPERTY OF THE PROPERTY **WARNING** IRUSSES REQUIRE EXTREME FARE IN FARRICATION HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BCST. (BUILDING COMPONENT SAFETY IRCORATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SHITE 312. ALEXANDRIA, NA, 223-213) AND HICLA (MODD TRUSS COUNCEL OF AMERICA. 6300 EXHIERBRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OHIGHNISE HADISON, MI 63719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS A PROPERLY ATTACHED BY CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE = 9 X 9 Except As Shown. U=16 PLF W=3-0-0 R=143 PLF U=14 PLF pressure 例 9 X 9 DANIELS 10 3X12 III 12-0-0-10-7 10-3-10 9 W = 18 - 0 - 0CGE) -24 Ó Ö Over 4 X 6 (R) III 2-3-3 $\frac{2}{2}$ ВЗ Ö ò 3 Supports Ó T 4 The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 18-0-0 . Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 See DWGS A11015EE0207 & GBLLETIN0207 for more requirements designed by the building designer. 7-10-7 7.36.0424 3 X 1 2 III 6X6/ ∞ SIONAL -216 PLF U-17 PLF ₩ CORION € X 6 = 3 X 4 /// 0-0-0 -8-6 3-0 BC LL 6-0 ВС TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/=/4/-4 X 10 III 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC 2 20.0 9-0-0 24.0" 10.0 PSF 40.0 PSF 1.25 10.0 PSF 0.0 PSF PSF JREF -SEQN-DATE FROM DRW HCUSR8228 07295037 HC-ENG Scale R8228 1TBT8228Z01 =.25"/Ft. JB/AP 23680 10/22/07 9880

Bot chord 2x8 SP #1 Dense: :B3 2x4 SP #2 Dense: Webs 2x4 SP #3 :Lt Wedge 2x4 SP #3: Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling. Calculated horizontal deflection 0.32" due to dead load. : T4 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. PLT TYP. ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 0 278 290 chord 2x8 2x4 SP Stephen Crawford Construc ALPINE Wave SP #1 Dense SP SS :T1 2x6 Dense: 1-0-14 7 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH IPT, OR FARREACTING, AND DISCOULTING, SHEPPIG, HISTALLING A BRACHIG OF TRUSSES, BY ATRA) AND TET. ITH BCG DESIGN CONTROWS WITH APPLICABLE PROVISIONS OF THIS CONTROWS WITH APPLICABLE PROVISIONS OF THIS CONTROL SHEED, AND THE BCG CONTROL AND THE ARCHITECTOR PLAIST ARE MADE OF 70/103/166A, (H.H.155K), ASTH ASSO GRADE 40/40 (H. K.M.55), GALV. SHELL APPLY PLAIES TO EACH FACE OF TRUSS, AND. JUHESS OTHERMISE LOCATED OF THIS DESIGN, POSITION PER BRANDES GOA Z. ANY LUSSECTION OF PLAIES FOLOCHED BY CONTROL SHEEL, APPLY PLAIES TO EACH FACE OF TRUSS, AND. JUHESS OTHERMISE LOCATED OF THIS DESIGN, POSITION PER BRANDES GOA Z. ANY LUSSECTION OF PLAIES FOLOCHED BY CONTROL SHEEL APPLY PLAIES TO EACH FACE OF TRUSS, AND LUBES OF THE TRUSS COMPONERS TO BUILDING DESIGNER PER DESIGN SHOWN. I :B2 2x8 SP A PROPERLY ATTACHED RIGID CELLING SP 1-6-0 5X5 (B2) is 0.20" DANIELS R=1997 SS: SUTTABILITY AND USE H 6X6**個** due to live load and Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)U=83 W=3.5" 1.5X4 III 3×4/ 6 X 6≡ 11-1-9 3X12 III 4X8(R) III D2) 0 -24-0-0 Over 2 Supports 9 3 X 4 ≡ BRIGLING 82 8 ű Ó 4X10(R) III 5-10-13 12 Ö ВЗ 1.5X4 W 3 X 4 ≡ Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ $^{\prime}$)=0.18 BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 20-0-0. Wind reactions based on MWFRS pressures (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. 7.36.042 4X12(R) Ⅲ 2 3 - 11 - 9.5X8 Ⅲ 7 X 6≡ 6×6// 4 X 5 (R) Ⅲ R=2047 U=74 W=3.5" 3-0-0 TATE OF 5 X 8 (**) 🥠 3 X 4 Ⅲ 5 X 5 // BC LL SPACING DUR.FAC. ВС TC DL TC LL TOT.LD. FL/-/4/-/-/R/-D 10.0 20.0 24.0" 40.0 PSF 10.0 PSF 1.25 0.0 PSF PSF PSF JREF -SEQN-FROM DATE REF HC-ENG DRW HCUSR8228 07295019 Scale R8228-1TBT8228Z01 =.1875"/ft. JB/AP 10/22/07 9881

110 mph wind, 21.13 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 lop Bot Wind reactions based on MWFRS pressures. p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP 290 Stephen Crawford Construc #2 Dense #2 Dense #3 DANIELS * APGE Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. TC BC SPECIAL LOADS From From (LUMBER DUR.FAC.=1.25 / 65 PLF at 0.00 to 65 PLF at 4.35 to 4 PLF at 0.00 to / PLATE DUR.FAC.=1.25)
) to 65 PLF at 4.35
; to 65 PLF at 8.69
) to 4 PLF at 8.69

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details

In lieu of rigid ceiling use purlins to brace

B C

@

24" OC

See DWGS All030EE0207 & GBLLETIN0207 for more requirements

R=262X4(A1) =U=63 W=5.833" R=124 PLF U=60 PLF W=7-3-5 1.5X4 III 1.5X4 III 9 φ ပ် 0ver 1.5X4 II 4×4≡ ф W Supports 1.5X4 Ⅲ 1.5X4 III 9 Ф 2X4(A1) =26 U=12 W=5.833"

19-6-

WARNING IRUSSES REQUIRE EXTREME CARE IN FABRICATION. INAUDITNG, SHIPPING, INSTALLING AND BRACING. REFER TO BOST. (BUILDING COMPORENT SAFETY HIROMATION), PUBLISHED BY TEL (FAUSS PLATE INSTITUTE, 21B URBH LUTE STREET, SHIFE 312, ALEXANDRIA, VA., 223-14) AND HICA (MODO TRUSS COUNCILS OF AMERICA, 6300 ENTIFERENCE, SHIFE 312, ALEXANDRIA, VA., 223-14) AND HICA (MODO TRUSS COUNCILS OF AMERICA, 6300 ENTIFERENCE, SHIFE 312, ALEXANDRIA, VA., 223-14) AND HICA (MODO TRUSS COUNCILS FROM THE THEORY OF AMERICA, 1801 OF AM Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.36.0424

PLT TYP.

Wave

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Amborization # 0.278 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI; OR FAREACTION, ANDIDING, SHIPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN CONTROLATION, AND LINE. TROPYISIONS OF HIDS SCIENTING FOR THE SECOND AND LINE. THE COMMERCIAN PLATES ARE ADDOOR OF 201/B01/GACA, WINTEXEX, ASTEN ASS DRAME 40/GG W. K/H.SS) GALV. SHELL, APPLY PLATES TO FACH FACE OF TRUSS AND, UNILESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHINGS 1660A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANHEX AS OF THIS 200EX SEC. 3. A SEAL ON THIS DESIGN. POSITION OF THE SECONDOMINISTS OF THE DRAWING INDICATES ACCEPTANCE OF PROF DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER ANSI/IPI I SEC.

ALPINE

ONAL ENGINEE SPACING BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. 40.0 20.0 10.0 10.0 PSF 1.25 24.0" 0.0 PSF PSF PSF PSF JREF -SEQN-DATE DRW HCUSR8228 HC-ENG R8228-1TBT8228Z01 JB/AP 23699 10/22/07

FL/-/4/-/-/R/

Scale =.5"/Ft.

9882

07295034

Bot chord 2x6 SP chord 2x6 SP Webs 2x4 SP 290 Stephen Crawford Construc DANIELS #2 :T1, T5 2x4 SP #2 Dense: #2 #3 A10 Negative reaction(s) of -184# MAX. (S load case requires uplift connection. (See below) from a non-wind

Calculated horizontal deflection is 0.14" due to live load and 0.22" due to dead load. (A) Continuous lateral bracing equally spaced on member.

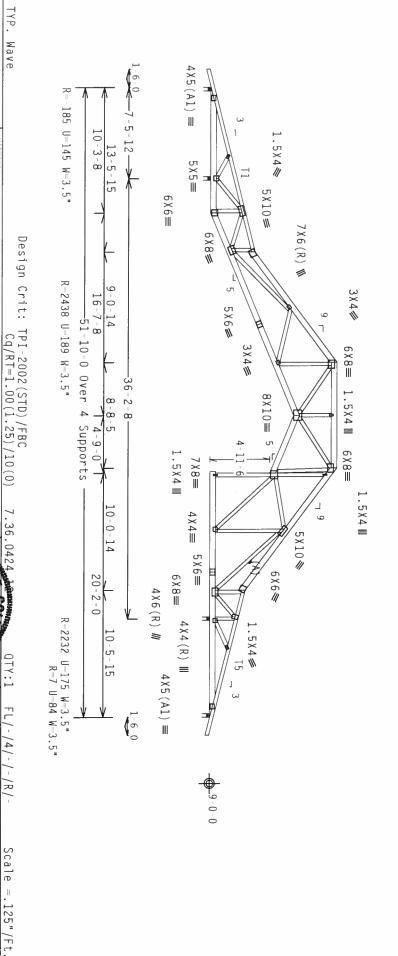
In lieu of structural panels use purlins to brace all flat TC 24" OC. **(e)**

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 located within 6.50 DL=5.0 psf, wind BC ft mean hgt, ASCE 7-02, CLOSED bldg, not ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



ITW Building Components Group, Inc.
Haines City, FL 3844
FI Certificate of Amborization # 0 278 ALPINE **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FP: OR FARRLACHING, HANDLING, SHIPPILG, HISTALLING & BRACHING OF TRUSSES, DESIGN CONTRORS WITH APPLICABLE PROVISIONS OF THIS CALLING A BRACHING OF THE SECOND CONTROLS OF THIS ARE HADE OF 20/18/166A (W.1/55/K) ANTH MASS GRADE 40/60 (W. KYM.SS) GALV. STEEL, APPLY PLAIES TO LACH FARE OF TRUSS AND. INHESS OHIERHISE LOCALED ON THIS DESIGN, POSITION PER DRAMFHING SIGA X. ANY INSPECTION OF PLAIES FOLLOWED BY (I) SHALL BE PER ANHER AS OF FPIL 2002 SEC.3. A SEAL ON THIS DRAMFHIGHDICALES ACCEPTIONED BY (I) SHALL BE PER ANHER AS OF FPIL 2002 SEC.3. A SEAL ON THIS DRAMFHIGHDICALES ACCEPTIONED BY (I) SHALL BE PER ANHER AS OF FPIL 2002 SEC.3. A SEAL ON THIS DRAMFHIGHDICALES ACCEPTIONED BY (I) SHALL BE PER ANHER AS OF FPIL 2002 SEC.3. **WARNING** IRUSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RELEK TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SUIF 217. ALEXANDRIA, VA., 223.14) AND HICKA (MODO TRUSS COUNCIL OF AMERICA. 6300 CHIEGRASS LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISC LINE, LADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISC LINGLATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/IPI 1 SEC. .36.0424 JONAL ENGINEE ATE BC LL BC DL TC DL SPACING TOT.LD. TC LL DUR.FAC FL/-/4/-/-/R/-20.0 40.0 10.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF JREF -FROM DATE REF SEQN-DRW HCUSR8228 07295004 HC-ENG Scal R8228- 9883

1TBT8228Z01

JB/AP 23723

10/22/07

290 Stephen Crawford Construc DANIELS #2 Dense:

Α9

t chord 2x6 SP t chord 2x6 SP Webs 2x4 SP #2 :T1, T5 2x4 SP #2 #3

Calculated horizontal deflection is 0.12" 0.19" due to dead load. due to live load and

In lieu of structural panels use purlins to brace all flat TC $24\mbox{\,\sc n}$ 0C.

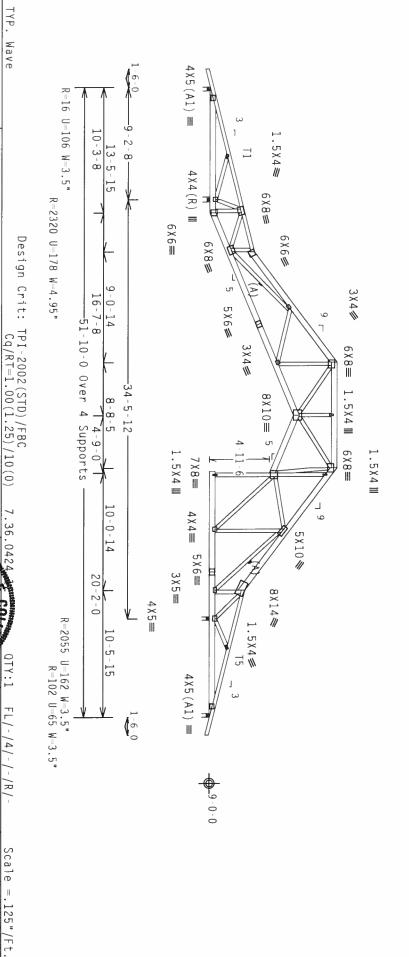
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



WARNING IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHEPPING, INSTALLING AND BRACING, RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE SHEET, SUITE IIZ, ALEXANDRIA, NA, 22214) AND META (MOOD TRUSS COUNCEL OF AMERICA, 6300 CHITEPRESE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS CHIMENESS HOLOCATED NO PUBLOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED THE CHORD SHALL HAVE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; APPLY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARRICATHO, HANDLING, SHIPPIDE, HISTALLING A RRACING OF TRUSSES.

DESIGN CONTRORS, HIH APPLICABLE PROPISIONS OF DBS (MATIONAL DESIGN SEC. BY ATRA) AND TP!. 114 BCG CONNECTOR PLATES ARE HADE OF 70/18/1666 (H.M.155/P), ASTH A653 GRADE 40/60 (H. K/M.5S) GALV SIEEL. APPLY LALEES TO LACH FACE OF TRUSS AND. HULLSS OTHERHISE LOCALED ON THIS DESIGN, POSITION PER DRAWHISS 160A. A. ANY INSPECTION OF PRATES FOLLOWED BY (1) SHALL BE PER ANHEX AS OF TPI] 2002 SEC. 3. A STAL ON THIS BESIGN SHOWN. THE SULFAMOLE OF PROFESSIONAL ENGLITERING RESPONSIBILITY SOLEY OR HILL THAN CORPORATION.

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Amborization # 0.778

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

ALPINE

130 SONAL ENGLINE SPACING DUR.FAC TC DL IC LL TOT.LD. 20.0 24.0" 1.25 40.0 PSF 10.0 10.0 PSF 0.0 PSF PSF PSF SEQN-DATE JREF -FROM HC-ENG DRW HCUSR8228 07295007 R8228- 9884 1TBT8228Z01 JB/AP

23729

10/22/07

290 Stephen Crawford Construc DANIELS #2:

Α2

Bot t chord 2x4 SP t chord 2x6 SP Webs 2x4 SP #2 Dense :T3, T4, T5 2x6 SP #2 #3

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC

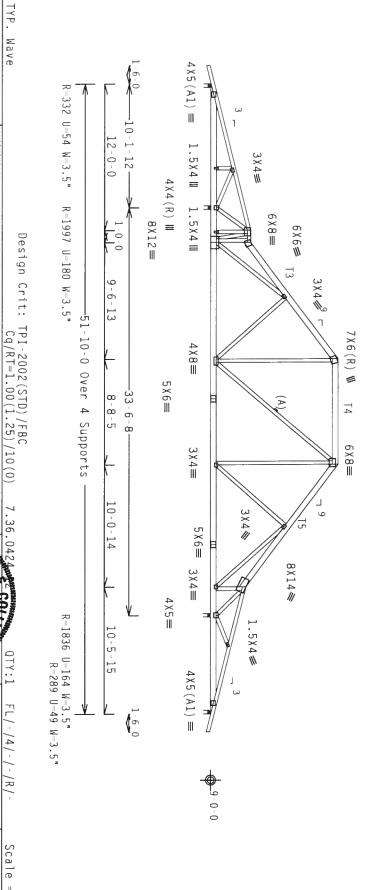
 $\widehat{\mathbb{A}}$ Continuous lateral bracing equally spaced on member

In 24" lieu of structural panels use purlins to brace all flat TC

(e)

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping



WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO BEST. (BUILDING CHRYDNETH SAFETY INFORMATION), PHBLISHED BY TIP (TRUSS PLATE INSTITUTE, ZIB UNBH LLEE SHRIT, SUILE IZZ, ALEXANDRIA, VA, AZZIJA) AND HICA (HODO TRUSS COUNCIL OF AMERICA. 6300 CHIEGRRISE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERNISE LINE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERNISE HINDLED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED RIGHD CELLING.

PLT

TYP.

Wave

7.36.042

FL/-/4/-/-/R/-

Scal

le = .125"/Ft.R8228- 9885

10/22/07

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG. THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TAILURE TO BUILD THE BRUSS IN COMPORMANCE HITH IP: OR FAREFACTHOE, HANDLIGE, SHEPTHOE, INSTALLING A BRACHES OF TRUSSES.

BESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THOS (MATIONAL DESIGN SPEC, BY ATRYA) AND TPI. THE RECOMMENT OF THE ARE AND OF TOTAL OF THE ACT.

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ALEY LISTEDITION OF PLATES FOLLOWED BY (1) SHALL BE FER ABILEX A OF TPI 2002 SEC. 3.

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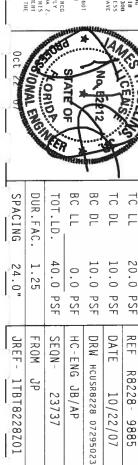
ALEY LISTEDITION OF PLATES FOLLOWED BY (1) SHALL BE FER ABILEX A OF TPI 2002 SEC. 3.

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ANY INSPECTION OF PARTS FOLLOWED BY C) SMALL BE
DEAMNING INDICATES ACCEPTANCE OF PROTESSIONAL ENGINE
DESIGN SHOWN. THE SUFFABILITY AND USE OF THIS C
BUILDING DESIGNER PER ANS//PP1 SEC. 2.

ITW Building Components Group, Inc. Haines City, FL 33844 F1 Certificate of Amhorization # 0.778

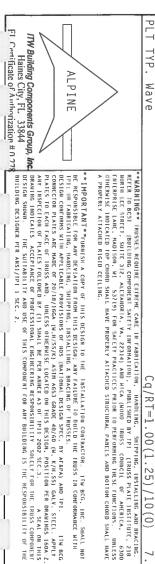
ALPINE



JB/AP 23737

1TBT8228Z01

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ 7-290--Stephen Crawford Construc DANIELS p chord 2x4 SP // t chord 2x4 SP // Webs 2x4 SP // #2 Dense #2 Dense #3 **1** 8 0 → $2X4(A1) \equiv$ 9 R = 375M* U=35 W=3.5" DOR) 3 - 0 - 8-6 - 1 - 00ver 1.5X4 III $4 \times 4 \equiv$ 中 2 Supports 3-0 ά anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. R-375 U-35 W-3.5" Wind reactions based on MWFRS pressures. $2X4(A1) \equiv$ M**1-8-0-**√



Design Crit: TPI-2002(STD)Cq/RT=1.00(1.25)/10(0)

7.36.042

QTY:5

FL/-/4/-/-/R/-

Scale = .5"/Ft. REF R8228- 9886

TC DL

20.0 PSF 10.0 PSF

REF DATE

10/22/07

0

10.0 PSF

DRW HCUSR8228 07295032

TC LL

CORION

TOT.LD.

SEQN-

HC-ENG

JB/AP 47459

SPACING

40.0 PSF 1.25 24.0"

JREF-

1TBT8228Z01

290 Stephen Crawford Construc DANIELS * DORGE

p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

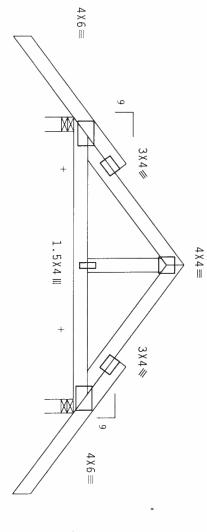
MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

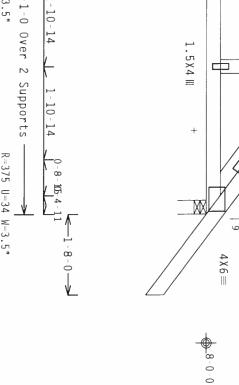
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

See DWGS All015EC0207 & GBLLETIN0207 for more requirements





1-8-0-

4 - 101 - 8 - 15

R = 375

U=34 W=3.5"

6-1-0

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

WARNING* RUSSYS REQUINE EXTREHE CARE IN FARNICATION. INABELING, SHIPPING, INSTALLING AND BRACING. RELER TO BEST. (BUILDING COMPONENT SAFELY INFORMATION). PHOLISING BY DET (TRUSS PLATE INSTITUTE, 2218 UNDER LEE, SUITE 312, ALEXANDRAL, VA., 22314) AND NEACH (400D TRUSS COUNCE) OF AMERICA. 6300 CHIERRENS LANE, MADISON, NE 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OFHERMISS. HOLDSKAFELD FOR THOSE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

IMPORTANTrurrish a copy of this design to the installation contractor. The beg, inc. shall not be responsible for any deviation from this design, any action of building the sizes in compormance with the responsibility for farrishes.

PISTOR FARRISHING, HANDLING, SUPPTHO, INSTALLING A BRACHING OF TRUSSES.

DESIGN CONFIDENCY WITH APPLICABLE PROVISIONS OF HDS (MATIONAL DESIGN SPEC, SY AREA) AND TPI.

THE BESTOR CONFIDENCY WITH APPLICABLE PROVISIONS OF HDS (MATIONAL DESIGN SPEC, SY AREA) AND TPI.

CONNECTOR PLATES ARE HADE OF 20/18/16GA (W.H/SS/K) ASTH A653 GRADE 40/60 (W. K/HLSS) GALV. SITEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER BRACHINGS 16GA Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANDEX AS OF TPI 2002 SEC.3.

ASTAL ON THIS SHARM IN THE SULFABLE OF PROFESSIONAL REGIMENT AND RESPONSIBILITY SOLLLY FOR HIE BRACK COMPONENT OF THE SULFABLE OF THE SULFABLE OF THE RESPONSIBILITY OF THE SULFABLE OF THE RESPONSIBILITY OF THE

ITW Building Components Group, Inc. Haines City, FL 33844 Fl Cartificate of Amborization # 0.278

BUILDING DESIGNER PER ANSI/1P1 1 SEC.

ALPINE

7.36.0424 * FL/-/4/-/-/R/-

S. L. S.	ול בר	20.0 PSF	REF R8228- 9887
10=10s/	TC DL	10.0 PSF	DATE 10/22/07
THE BOTTON	BC DL	10.0 PSF	DRW HCUSR8228 07295033
*	BC LL	0.0 PSF	HC-ENG JB/AP
PIAIE OF CER	TOT.LD.	40.0 PSF	SEQN- 47466
SOR	DUR.FAC.	1.25	
OC TOWN	SPACING	24.0"	JREF- 1TBT8228Z01

Scale =

5"/Ft

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

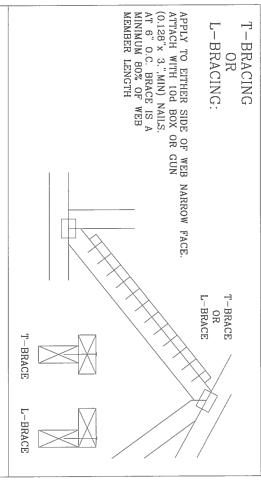
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

1-2XB	SAC	1 ROW	8X8
1-2X6 2-2X4(*)	2X4	1 ROW	9X6 9X8
2-2X4	2X6	2 ROWS	2X3 OR 2X4
1-2X4	2X4	1 ROW	OR
SCAB BRACE	T OR L-BRACE	BRACING	SIZE
E BRACING	ALTERNATIVE BRACING	SPECIFIED CLB	WEB MEMBER

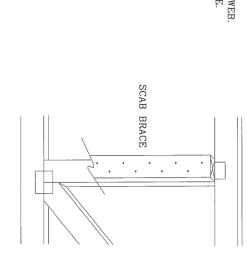
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



SCAB BRACING:

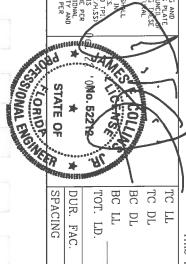
APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

PSF

DATE DRWG -ENG



PSF PSF

MLH/KAR

BRCLBSUB0207

CLB SUBST. 2/23/07



WARNING TRUSSES REGUIRE EXTREME CARE IN FABRICATION (MADILING, SHIPPING, INSTALLING AND BRACING REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TRY CIRCUSS PLAIN INSTITUTE, 218 NIBTH, LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VICA (VUDD TRUSS COUNCIL), AMPRICA, 6300 ENTERPRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING JAEST CHARTEN, UNLESS OTHERVISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

***INDEPTANT** FURNISH CIDY OF THIS DESIGN TO INSTALLATION CONFRACTOR. ITV BGG, INC., SANL DER RESPONSIBLE CIDR BAND EVALUATION CONFRACTOR. ITV BGG, INC., SANL COMFORMANCE WITH THIS DESIGN ANY FAILURE OF BRILD THE TRUSS IN DESIGN COMFORMS WITH A PROPERTY OF THE SECOND ANY FAILURE OF BRADE OF THIS SECOND ANY FAILURE OF BRADE OF THIS SECOND ANY FAILURE OF BRADE OF THIS SECOND OF THIS DESIGN SECOND OF THIS SECOND OF THIS DESIGN SECOND OF THIS SECOND OF THIS SECOND OF THIS DESIGN SECOND OF THIS SECOND OF THIS DEVELOPMENT OF THE DATE OF THIS SECOND OF THIS SEC

BEARING BLOCK NAIL SPACING DETAIL

MAXIMUM NUMBER OF NAIL

LINES

PARALLEL CHORD SIZE

 $^{\mathrm{TO}}$

GRAIN

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- CBA EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS) SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS) END DISTANCE (15 NAIL DIAMETERS)

핗 NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW: * SPACING MAY BE REDUCED BY 50% ** SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN

VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD LENGTH OF C** (12" MINIMUM -BLOCK SPECIFIED ON SEALED 24 MAXIMUM) AAAA DESIGN C** LINE DIRECTION OF LOAD AND NAIL ROWS B/2* ₩ ۱,>

													_
GUN (0.131"X 3.", MIN)	GUN (0.120"X 3.",MIN)	GUN (0.131"X 2.5", MIN)	GUN (0.120"X 2.5", MIN)	16d COMMON (0.162"X 3.5", MIN)	12d COMMON (0.148"X 3.25", MIN)	10d COMMON (0.148"X 3.", MIN)	8d COMMON (0.131"X 2.5", MIN)	20d BOX (0.148"X 4.",MIN)	16d BOX (0.135"X 3.5", MIN)	12d BOX (0.128"X 3.25",MIN)	10d BOX (0.128"X 3.", MIN)	8d BOX (0.113"X 2.5",MIN)	NAIL TYPE
ω	ယ	ယ	သ	N	2	₽	ω	N	3	ಬ	3	ω	2X4
5	6	2	6	4	4	4	Q	4	5	2	5	6	2X6
7	8	7	8	ර	6	6	7	5	7	7	7	9	8X8
10	11	10	11	8	8	8	10	6	10	10	10	12	2X10
12	14	12	14	10	10	10	12	8	12	12	12	15	2X12

MINIMUM NAIL SPACING DISTANCES

	עזע	DISTAINCES	
NAIL TYPE	Α	₩	C**
8d BOX (0.113"X 2.5", MIN)	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"X 3.", MIN)	7/8"	1 5/8"	ಬ್ಳ
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	ಬ್ಬ
16d BOX (0.135"X 3.5", MIN)	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"
8d COMMON (0.131"X 2.5", MIN)	7/8"	1 5/8"	2,
10d COMMON (0.148"X 3.", MIN)	1"	1 7/8"	2 1/4"
12d COMMON (0.148"X 3.25", MIN)	1"	1 7/8"	2 1/4"
16d COMMON (0.162"X 3.5", MIN)	ı,	ಸ್ತ	2 1/2"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 2.5", MIN)	7/8"	1 5/8"	2,
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	ಬ್ಜ

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

- 1					_
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0	STAT	. No. 5224	() () () () () () () () () () () () () (C
CORIO	STATE OF	2242	NSE		<u>,</u>
3	EP .	Eller A	C. C.		

WHERDER ANTER FURNISH COPY OF THIS DESIGN TO INSTALLATION COMPACTER. ITY BEG, INC.

NOT BE RESPONSUBE FOR ANY DEVIATION FROM HIS DESIGNA MAY PALLIAR BUILD HE TRUE

DESIGNA COPERBERS VITH APPRICABLE PROVISIONS OF ANY DAVISION SPEC, BY AREAS A

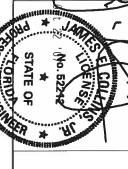
THAT BEG CONNECTER PLAITS ARE HADE OF 2018/166 AVAINABLAND STRING SPEC, BY AREAS A

TO ANY STELL AND APPLY PLAITS ARE HADE OF 2018/166 AVAINABLAND STRING SPECE BY AREAS AND AREAS OF THATES OF ANY DESIGNATION OF A STRING SPECE BY A STRING TRUS

WARNINA TRUSSIS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI GBUILDING COMPONENT SAFETY IN GRRHATION, PUBLISHED BY TPI CIRLISS PLASINISTITUTE, 218 MIRCH LEE SIR, SUITE 213 ALEXANDER, VA. 22314) AND VITA. VOCOD TRUSS COLORIO MARCICA, 6300 ENTERRISE LN, HADISON, VI 33719) FOR SAFETY PRACTICES PRIDE TO PERCENDIG THE: FUNCTIONS, UNLESS OTHERUSE INDICATED. THE CHORD SHALL HAVE PROPERTY ATTACHED STUCTURAL PRANCLS OFFICE COLORIO SHALL HAVE PROPERTY ATTACHED STUCTURAL PRANCLS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STUCTURAL PRANCLS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED RIGID CEILING.

ITW BUILDING COMPONENTS GROUP, POMPANO BEACH, FLORIDA

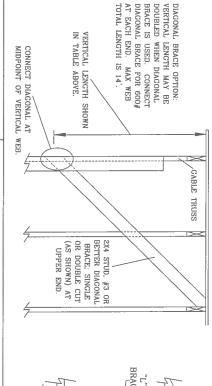
ALPINE

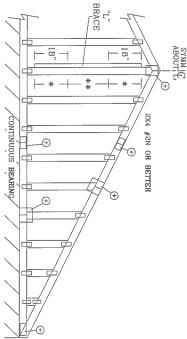


DRWG DATE -ENG SJP/KAR CNBRGBLK0207 2/23/07 BEARING BLOCK

ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE \bigcirc

			M 2		X	0	.(J.	A I	31		E 6	,,	V	E		Z.	Ί	C		L 4	_	L	E	_			H	GABLE
	i		;	7	<u>)</u>	TTT	I	SIT	Ω 		<u> </u>	1	V.) J	TIL	I I	OL'I				j)	TIT		O'L'I		SPECIES	GABLE VERTICAL
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	DRACE
	4' 11"	0 0	5,0	υ ₁	5.4"	4' 9"	4' 9"	4' 9"		4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4, 5,	3' 10"	4' 0"	4' 0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	NO
	7' 5"	8,5	8, 5,	8, 5,	8, 5,	7' 3"	8	හ _ු	8' 5"	6' 5"	7' 6"	7' 7"	7' B"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5 3	6' 1"	6, 2,	6' 8"	6' 8"	ත _.	6' 0"	6' 0"	6' 8"	GROUP A	(1) 1X4 L
	7: 5:	8' 7"	8′ 5″	9' 1"	9' 1"	7' 3"	8, 5,	8' 5"	8,	6, 2,	7' 6"	7' 7"	в; ц	8 3"	6' 4"	7' 4"	7' 4"	7' 10"	5' 3"	6' 1"	6' 2"	7' 2"	7' 2"	5' &"	6' 0"	6'0"	6' 10"	GROUP B	DRACE .
	9' 10"	10' 0"	10' 0"	10' 0"	10′0″	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7'11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) ZA4 L
	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9, 6,	9' 6"	9' 9"	9' 9"	B' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	BRACE
	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9' 5"	9' 5"	9' 5"	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(c) cX4 L
- 1	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	BKACE ***
- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14'0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 5"	12' 5"		12' 5"	10' 7"	12' 3"	12' 4"	12' 5"	GROUP A	(1) 2X6 L
- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 6"	12' 8"	13' 5"	13′ 5″	10' 7"	12' 3"	12' 4"	12' 9"	GROUP B	BRACE .
- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14'0"	14' 0"	14' 0"	GROUP A	(S) SX6 L
- 1	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14′0″	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **
GABLE END SUPPORTS LOAD FROM 4' 0"		CONTINUOUS BEARING (5 PSF TC DEAD LOAD).	PROVIDE LIBITET CONNECTIONS FOR BU DIE OVER	LIVE LOAD DEFLECTION CRITERIA IS L/240.	CADDED INCODE DUINIE NOTES.	CARLE TRUSS DETAIL NOTES			#22		SOUTHERN PINE DOUGLAS FIR-LARCH	0	A COLUMN	HEX-FIX	GROOT D.	CDOID B.		֓֞֝֟֜֟֝֝֟֝֝֝֝֝֝֓֓֟֝֟֝֟֝֟֝ ֓֓֓֞֓֞֓֞֓֞֓֞֞֞֓֞֞֜֞֞֓֓֞֞֜֜֞֜֓֓֞֞֜֜	S	STIID #3	R-LARCH SOUTH		STUD #3 ST	#1 / #2 STANDARD #2 STUD	,001	CBOILD V:	BRACING GROUP SPECIES AND GRADES:		





ROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). NBLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG. /E LOAD DEFLECTION CRITERIA IS L/240.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. ATTACH EACH "L" BRACE WITH 10d NAILS.

MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

_		DI ATTEC		SEAR COLLCE AND LIEST DIATES	TOS AVEC
	FOR	DESIGN	TRUSS	REFER TO COMMON TRUSS DESIGN FOR	REFER TO
		2.5X4		REATER THAN 11' 6"	REATER T
		100		LESS THAN 11' 6"	LESS TH
		AAG	. BUT	REATER THAN 4' O", BUT	REATER T
	2X3	1X4 OR 2X3		4' 0"	ESS THAN 4' 0"
	CE	NO SPLICE		VERTICAL LENGTH	VERTIC/
	01	TE SIZES	PLA	GABLE VERTICAL PLATE SIZES	GABLE V

MEMORANION TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI GDILIDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CRUSS PLATINISTITUTE. 210 NIBRH LEE ETK, SUITE BLE, ALEXANDRIA, VA 2314) AND WTGA VOODO TRUSS COUNCIL, ACHERICA, 6300 ENTERRISE LN, HADISIN, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THE FUNCTIONS. UNLESS OTHER USE INDICATED. THE PARTIED SHALL HAVE PROPERLY ATTACHED STRUCTUPAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTUPAL. AÉ PER ASSIDNAL) CABILITY AND CR, PER * CENSE STATE OF No. 52212 'n MAX. MAX. TOT. LD.

WHIPERFANIES FURNISH COPY OF THIS DESIGN TO INSTALLATION CONFRACTOR TTY DEG. NO. WIND THE RESENSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE OF DAILO ME FOREST CONFIDENCIAL THE REASON ANY FAILURE OF DAILO ME FOREST CONFIDENCIAL THE TOP OF FARRICATION FROM THIS DESIGN, ANY FAILURE OF DAILO ME FOREST CONFIDENCIAL TOP OF THE PROPERTY OF THE SECOND THIS DEPOSIT OF THE SECOND OF THE SECOND THIS DESIGN AND THIS DESIGN AND THE SECOND THE SECOND THE SECOND THIS DESIGN AND THIS DESIGN AND THIS DESIGN AND THE SECOND THIS DAILO MESS OF THE SECOND THIS DESIGN AND THE SUITABLE THE DESIGN AND THIS DESIGN AND THE SUITABLE THE SUITABLE THE SECOND THE SECOND THIS DESIGN AND THE SUITABLE THE SECOND THIS DESIGN AND THE SUITABLE THE SECOND THIS DESIGN AND THE SUITABLE THE SECOND THIS DESIGN AND THE DESIGN AND THE SUITABLE THE SECOND THIS DESIGN AND THE SUITABLE THE SECOND THE SECOND THE SECOND THE SUITABLE THE SECOND THE SECO

ITW BUILDING COMPONENTS GROUP POMPANO BEACH, FLORIDA

ALPINE

CORIOT PROBLEM

60

DRWG DATE REF A11015EE0207 2/23/07 ASCE7-02-GAB11015

24.0"

PSF

AND HEEL PLATES.

REFER

TO

CHART ABOVE FOR MAX GABLE VERTICAL

SPACING



ABOUT (



EXAMPLE:



2X8

VERTICAL LENGTH TYP.

+





ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

HAND DRIVEN NAILS: 10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS (4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD

8d COMMON (0.131"X 2.5", MIN) TOENAILS AT 4" O.C. PLUS

GUN DRIVEN NAILS:

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD

ASCE 7-93 GABLE DETAIL DRAWINGS

ASCE 7-98 GABLE DETAIL DRAWINGS A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A09030EN0207, A08030EN0207, A09030EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A09050EN0207, A0

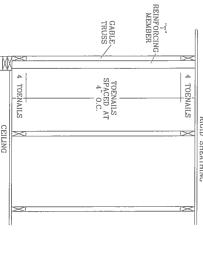
ASCE 7-02 GABLE DETAIL DRAWINGS A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207 A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207

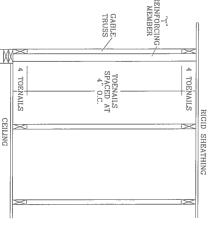
ASCE 7-05 GABLE DETAIL DRAWINGS A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A1030EE0207, A1030EE0207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

VERTICAL LENGTH.







MAVARNINGA. TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS (BUILDING COHEDNAY SAFETY INFORMATION, PUBLISHED BY FIP CTRUSS PLINSTITUTE, 218 NORTH LEE STR. SUITE 128, ALEXANDRIA, VA. 22340 AND VITA. VOODD TRUSS PLINSTITUTE, 218 NORTH LEE STR. SUITE 128, ALEXANDRIA, VA. 22340 AND VITA. VOODD TRUSS COLVET, ARKRICA, 6300 ENTERRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORME THE FUNCTIONS. UNLESS OTHER VISE ROUGH THE PROPERTY ATTACHED STALL HAVE PROPERTY ATTACHED STALL HAVE PROPERTY ATTACHED RIGID CEILING.

WHEREKANIA" FURNISH COPY OF THIS BESIGN TO INSTALLATION COMPACTER. ITY BEG, IN NOT BE RESPONSIBLE FOR ANY BEYLANDIN PROM HIS DISCIONA MAY FALLURE TO BUILD HE TRANSCED FOR ANY BEYLANDISH FOR ANY BEYLANDISH FOR ANY BEYLANDISH BRACKING OF FROM THE TRANSCED COMPACTER PLATES, ARE HADE OF 20/18/166A VHIS ON AND STEEL BY AFRAY ITY. BEG CONNECTER PLATES, ARE HADE OF 20/18/166A VHIS ON AND STEEL BY ARE AND OF 20/18/166A VHIS ON AND STEEL BY COMPACTED DN OF STEEL STEEL TO EACH FOR ARE OF TRANSCED OF ANY INSPECTION OF PLATES, FOLLOWED BY CONSTRUCTION OF ANY BOAY BY AND STEEL BY CONSTRUCTION OF ANY BY DESIGNER, PER

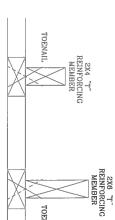
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CORION

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE





TOENAIL

SBCCI WIND LOAD. 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTON CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

EED "T" REINF. SBCI ASS RH MBR. SIZE 10 % 10 H 2x4 10 % 50 H 2x6 50 % 50 H 2x4 10 % 10 H 2x4 20 % 10 H 2x4 20 % 10 H 2x4 20 % 10 H 2x4 10 % 20 H 2x4 20 % 10 H 2x4 20 % 10 H 2x4 20 % 20 H 2x4 20 % 10 H 2x4 20 % 20																						
SBCCI 10	30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	AND MRH	Udddo UNIM
	2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE	JUNE "TE												
ASCE 50 10 10 10 10 10 10 10 10 10 10 10 10 10	7 01	10 %	2 0	0 %	20 %	20 %	10 %	10 %	30 %	2, 01	20 %	20 %	40 %	10 %		10 %		10 %			SBCCI	
	30 %	20 %	20 %	20 %	40 %	2 01	30 %	20 %	50 %	2 01		10 %	40 %	10 %	50 %	7 01		10 %	50 %		ASCE	

EXAMPLE:

MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4 "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7" ASCE WIND SPEED = 100 MPH "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

		MAX SPACING 24.0"	ING	SPAC	MAX	TATE OF
		ANY		DUR. FAC.	DUR.	*
		MAX TOT. LD. 60 PSF	LD.	TOT.	MAX	16. 82212 H
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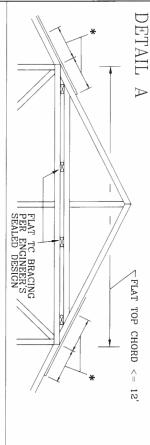
PIGGYBACK DETAIL

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

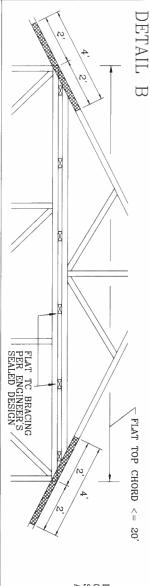
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS. MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE

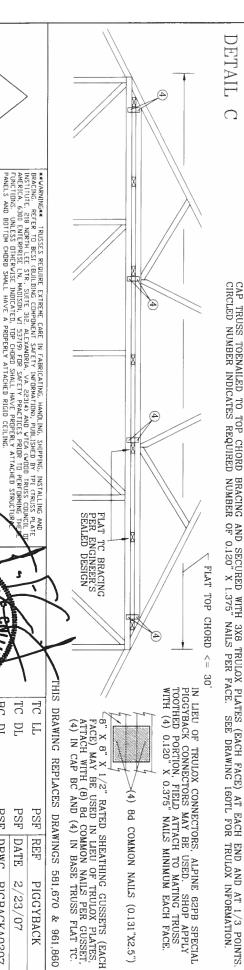


PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.



IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

REPLACES DRAWINGS 581,670 & 961,860

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	ANSI/TPI I SEC. 2.	C. ANDEX A3 OF 1911-200 SEC. 31. A SEAL DI THIS DRAWING MINISTANCE ACCEPTANCE OF PROFESSIONAL EXCEPTION OF THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND LINE OF THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND	ITW, BCC COMMERCIAR PLATES ARE MADE DE POVISIONA OF MAD VINITUMENT LEGION SECU. BY HEAVY AND INC. SECU. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS BEST OF THE PROPERTY OF THE	NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGNATION ANY FAILURE IT BUILD HE RIUSS WILL CONFIDENCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGNATION FOR THE SING WAS AND THE OR THE SING THE SING THE SING THAT THE SING	PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.	AMERICA, 630 ENTERPERE LNI, MADISTAD, WI 53719 FOR SHELL HAVE PROPERLY ATTACHED STRUCTURE FUNCTIONS: UNLESS OTHERVISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURE	IRRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TO CIRCUS PLATE
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ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

TOP CHORD CHORD WEBS 2X4 2X4 2X4 ### 9R 9R BETTER BETTER BETTER

PIGGYBACK DETAII

(4) 6d BOX (0.099"X 2.", MIN) NAILS.

JOINT

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52

SPACE PIGGYBACK VERTICALS AT 4' OC MAX REFER TO SEALED DESIGN FOR DASHED PLATES

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS To

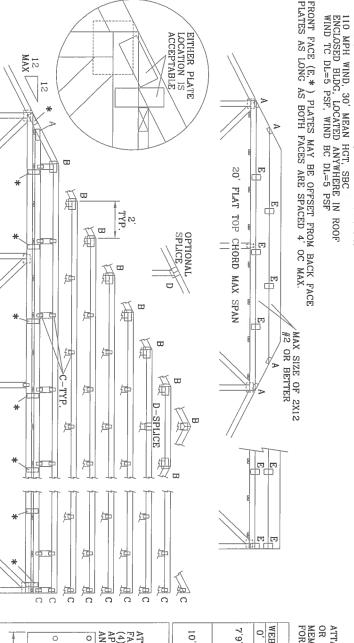
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

ČB" X 8" X 1/2" RATED SHEATHING GUSSETS (EACFACE) MAY BE USED IN LIEU OF TRULOX PLATES ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TO 30



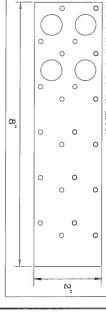
EITHER PLATE
LOCATION IS
ACCEPTABLE

ACH EQU			Γ,Ω	,ss	9	
TRULOX I AL, PER I	(F)	D	С	В	Α	
ACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS EQUAL, PER FACE PER PLY. (4) NAILS IN EACH IBER TO BE CONNECTED. REFER TO DRAWING 160 TL	4X6 OF	5X4	1.5X3	4X6	2X4	
TH (8) 0. PLY. (4)	OTATED V	5X5	1.5X4	5X6	2.5X4	
120" X 1 NAILS IN	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY	5X5	1.5X4	5X6	2.5X4	
.375" NA: EACH ING 160	4' OC, Y	5X6	1.5X4	5X6	3X5	
1 5						

ATTACH TRULOX PLATES WIT OR EQUAL, PER FACE PER F MEMBER TO BE CONNECTED. FOR TRULOX INFORMATION.

	10' TO 14'	7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
* PIGGYBACK SPECIAL PLATE	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC	7'9" TO 10' MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.	NO BRACING	REQUIRED BRACING	WEB BRACING CHART

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FAND SPACE 4' OC OR LESS. FACE



HIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

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7		~ <u>''''''''</u>		N. S.	A	_	
	THE STREET	2710000	_				
	SPACING	47 1.15	1.60	1 35 1 35	1.33	55	MAX
		47 PSF AT 1.15 DUR. FAC	טטת.	50 PSF AT	DUR.	55 PSF AT	MAX LOADING
	24.0"	AT FAC.	FAC.	AT	FAC.	AT	DING

DLJ/KAR PIGBACKB0207 2/23/07 PIGGYBACK



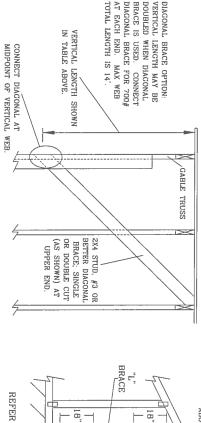
MAX

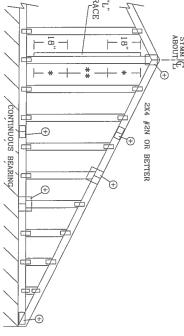
*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

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ASCE 7-02: 110 MPH WIND SPEED, 30, MEAN HEIGHT, ENCLOSED, 1.00, EXPOSURE

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	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 7"	4' 9"	4' 9"	4' 11"	5, 1,	4' 6"	4' 6"	4' 6"	4' 7"	4 2"	4 4"	4' 4"	4' 6"	4' 7"	4 1"	4' 1"	4' 1"	4, 2,	3. 8.	3′ 9"	3' 9"	3' 11"	4' 0"	3' 7"	3' 7"	3' 7"	3' 8"	BRACES	N O
	6' 9"	7' 9"	7' 11"	8' 0"	8 0	6' 7"	7' 8"	7' 8"	8, 0,	5' 10"	6' 9"	6' 10"	7' 3"	7' 3"	5, 8,	8' 0"	6, 8,	7' 3"	4' 9"	5' 6"	5' 7"	6' 4"	6' 4"	4' 8"	5	5, 5,	6' 4"	GROUP A	(1) 1X4 "L"
	6' 9"	7' 9"	7' 11"	8' 7"	8' 7"	6' 7"	7' 8"	7' 8"	8, 2,	5' 10"	6, 9,	6' 10"	7' 9"	7' 9"	5, 8,	8' 0"	6' 8"	7' 5"	4' 9"	5' 6"	5' 7"	6' 10"	6' 10"	4' 8"	5' 5"	5' 5"	6' 6"	GROUP B	" BRACE *
	8' 10"	9' 5"	9' 5"	9' 5"	9' 5"	8' 8"	9' 5"	9' 5"	9.5	7' 8"	8' 7"	8' 7"	8' 7"	8' 7"	7' 6"	8' 7"	8' 7"	8' 7"	6' 3"	7' 3"	7' 4"	7' 6"	7' 6"	6' 1"	7' 1"	7' 2"	7' 6"	GROUP A	(1) 2X4 "L"
	8' 10"	9' 11"	9' 11"	10' 2"	10' 2"	8' 8"	9' 5"	9' 5"	9' 8"	7' 8"	8' 11"	9' 0"	9 3"	9' 3"	7' 6"	8' 7"	8' 7"	8' 10"	6' 3"	7' 3"	7' 4"	8' 1"	1,8	6' 1"	7' 1"	7' 2"	7' 8"	GROUP B	" BRACE *
	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	11' 3"	10' 3"	10' 3"	10' 3"	10' 3"	10' 3"	10' 1"	10' 3"	10' 3"	10' 3"	B' 5"	8' 11"	8' 11"	8' 11"	8' 11"	8' 3"	8' 11"	8' 11"	8' 11"	GROUP A	(2) 2X4 "L"
	11' 7"	11' 10"	11' 10"	12' 1"	12' 1"	11' 3"	11' 3"	11' 3"	11' 7"	10' 4"	10' 9"	10′9″	11' 0"	11' 0"	10' 1"	10′ 3″	10' 3"	10' 6"	8 5	9' 5"	9' 5"	9' 7"	9' 7"	8' 3"	8' 11"	8' 11"	9 2,	GROUP B	" BRACE **
	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	13' 5"	13' 5"	13' 5"	13' 5"	11' 8"	13' 5"	13' 5"	13' 5"	9' 9"	11' 4"	11' 5"	11' 9"	11' 9"	9' 6"	11' 1"	11' 2"	11' 9"	GROUP A	(1) 2X6 "L"
	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	14' 0"	14' 0"	14'0"	14' 0"	11' 8"	13' 5"	13' 5"	13' 10"	9' 9"	11' 4"	11' 5"	12′ 8″	12' 8"	9' 6"	11' 1"	11' 2"	12' 1"	GROUP B	" BRACE *
		- 1	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"		14' 0"	14'0"	14' 0"	12' 11"	14'0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L" BRACE
- 1			14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **





GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG. OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

PEAK, SPLICE, AND HEEL PLATES.	REFER TO COMMON TRUSS DESIGN FOR	GREATER THAN 11' 6"	LESS THAN 11' 6"	GREATER THAN 4' O", BUT	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
PLATES.	S DESIGN FOR	2.5X4	100	VAG	1X4 OR 2X3	NO SPLICE	TE SIZES

WHITER MATERIAL FLEWISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR ITY BCC, INC. SHANNIE BE RESPONSIBLE COR MAY DEVIATION FOR THIS DESIGN, ANY FALLIRE II BRILD THE TRUSS IN COMPROMETE WITH IPT, DR FABRICATING, HANDLING, SHEPPING, INSTALLING & BRACING OF METASSES.

DESIGN CONTROLL PROBLEME REPORTING THE PROVISIONS OF MASS YAM ESTIME RESIGN SPEC, BY AFFAN AND ITT ITY, BCC CONNECTIOR PLATES ARE MADE OF 2018 JACKS STANDES OF THE PROVISE CONNECTIOR PLATES OF ARE MADE OF 2018 JACKS STANDES OF THE PROVISE CONNECTIOR PLATES OF ARE MADE OF 2018 JACKS STANDES OF THE PROVISE CONNECTIOR PLATES OF THE STANDES OF THE PROPERTY OF THE PROVISE CONNECTION OF THE STANDES OF THE PROVISE CONNECTION OF THE STANDES OF THE PROPERTY OF THE PROPERTY OF THE STANDES OF THE PROPERTY OF THE STANDES OF JAMES

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CHART ABOVE FOR MAX GABLE

FERTICAL LENGTH

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ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

22

DOUGLAS FIR-LARCH

HEM-FIR #1 & BTR #1

22

SOUTHERN PINE

GABLE TRUSS DETAIL NOTES:

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

#3

STANDARD

DOUGLAS FIR-LARCH

SOUTHERN PINE

#3 STUD STANDARD

STANDARD STUD

GROUP

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BRACING GROUP SPECIES AND GRADES:

GROUP

A:

ASCE 7-98: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I 11 1.00, EXPOSURE

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			j	<i>ا</i> ر) j	TIT	I,	STI	C D T			1	₩.)	TIT	I,	STI	Q J J		L'H'	j j	υ. Τ)	TIT	I,	ULL	27	SPECIES GRADE	CABLE VERTICAL
	STANDARD	STUD	#3	#22	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 11"	5,0	5,0	"	5,4	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4' 0"	4' 0"	4' 2"	4' 3"	3′ 9″	3' 9"	3' 9"	3' 10"	BRACES	N C
	7' 5"	8, 5,	8. 5,	8. 5."	8,5	7' 3"	8, 5,	8, 5,	1	6, 2,	7' 6"	7' 7"	7' 8"	Ι.	6' 4"	7' 4"	7' 4"	7' 8"	5. G	6' 1"	6, 2,	6 8	6' 8"	ر ا ا	6'0"	6' 0"	6' 8"	GROUP A	(1) 1X4 "L"
	7' 5"	8' 7"	8 5	9' 1"	9′ 1"	7' 3"	8,5	"	8,	6, 2,	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5 3	6'1"	6' 2"	7' 2"	7' 2"	රු හැ	6' 0"	6' 0"	6' 10"	GROUP B	BRACE *
	9' 10"	10' 0"	10' 0"	10' 0"	10, 0,	9' 7"	10' 0"	1 1	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
MAS	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"	8' 1"	8, 6,	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *
	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	1	10' 10"	10' 10"	9' 4"	9' 5"	9' 5"	9' 5"	9' 5"	9' 1"	9' 5"	9' 5"	9,	GROUP A	(2) 2X4 "L"
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"		10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	BRACE **
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"		12, 2,	12' 5"	12' 5"	10' 7"	1		12' 5"	GROUP A	(1) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 6"	12' 8"	13' 5"	13′ 5″		12' 3"	12' 4"	12' 9"	GROUP B	BRACE *
	14' 0"	14' 0"	14' 0"	1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	- 1	14' 0"	- 1	14' 0"	- 1	14' 0"	14' 0"	B GROUP A	(2) 2X6 "L"
	14' 0	14' 0	14' 0	14' 0	14' 0	14'0	14' 0	14' 0	14'0	14' 0	14' 0	14' 0	14' 0	14'0	14' 0	14' 0	14' 0	14' 0	14'0	14' 0	14'0	14' 0	14'0	14, 0	14' 0	14' 0	14' 0	GROUP	"L" BRACE

DOUGLAS FIR-LARCH

SOUTHERN PINE #3

STANDARD

STANDARD

SPRUCE-PINE-FIR #1 / #2 STANDARD

#3

GUTS

#3

STANDARD

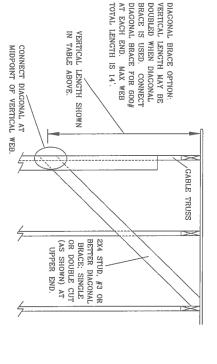
HEM-FIR 2 STUD

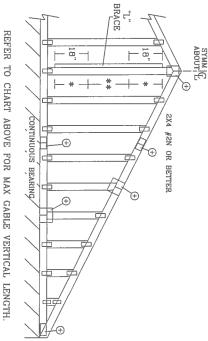
BRACING GROUP SPECIES AND GRADES:

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GROUP

A





GABLE TRUSS DETAIL NOTES

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

HEM-FIR #1 & BTR #1 GROUP

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12 12

GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG. OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

_		2 507		DATED THANK II' O"
		2X4	BUT	EATER THAN 4' 0", LESS THAN 11' 6"
		1X4 OR 2X3		SS THAN 4' 0"
		NO SPLICE		VERTICAL LENGTH
		TE SIZES	PLA'	ABLE VERTICAL PLATE SIZES
•	ı		l	

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REFER TO COMMON TRUSS DESIGN FOR PEAK. SPLICE, AND HEEL PLATES.

WHERE MANIET FURNISH COPY OF THIS DESIGN TO INSTALLATION COMPACTOR. ITY BCC, INC., SHAWLED RESPONSIBLE FOR ANY DEVLATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRISSS IN CONFIDENCE SHITH APPLICABLE PROPERTY.

DESIGN CONFIDENCE SHITH APPLICABLE PROPERTY OF MIS CHANDIGNE, INSTALLING & BRACHED OF TRISSSS. IN CONFIDENCE OF THE SHAWLED FOR THE SHAPE AND CONFIDENCE OF THE SHAPE AND CONFIDENCE **EVARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS GUILLING COMPONENT SAFETY INFORMATION, PUBLISHED BY IPI (TRUSS CHAFE INSTITUTE, 218 MORTH LEE STE, SUITE ELZ, ALEXANDRIA, VA. 22314) AND WICK (VIOLD TRUSS COUNCI, MARRICA, 6300 ENTERPRISE LN, HANDSON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THE FUNCTIONS. UNICESS DIFFERISE INDICATED, TOP CHARD SHALL HAVE PROPERLY ATTACHED STRUCTURE. minimin. STONAL ENGINE No. 52212 STATE OF * MAX. MAX. TOT. SPACING LD. 60 24.0" PSF DATE DRWG

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2/23/07

ASCE7-98-GAB11015

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE



From: The Columbia County Building & Zoning Department

Plan Review

135 NE Hernando Av.

P.O. Box 1529

Lake City Florida 32056-1529

Reference to a building permit application Number: 0711-70

Applicant Walter & Jennifer Daniels, Owner /Builders

Property: ID# 17-6s-17-09690-003

On the date of December 3, 2007 application 0711-70 and plans for construction of a single family dwelling were reviewed. The following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

<u>Please include application number 0711-70 and when making reference to this application.</u>

This is a plan review for compliance with the Florida Residential

Codes 2004 only and doesn't make any consideration toward the land

use and zoning requirement

1. The Florida Residential Building Code section R309.2 requires that the garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.

(Over)

- Please provide the separation between the garage and the second story habitable space.
- 2. The Florida Residential Building Code section R309.1 requires openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors. Please install an entry door from the garage into the dwelling area which will comply with section R309.1.
- 3. The second story bonus area shows several rooms which could be used as bedrooms. To be in compliance with the Florida Residential Building Code section R310.1 emergency escape and rescue. Each sleeping room shall have at least one openable emergency escape and rescue opening. All second story emergency escape and rescue openings shall have a minimum net clear opening of 5.7 square feet with a minimum net clear opening height shall be 24 inches and a minimum net clear opening width shall be 20 inches.
- 4. While constructing the stair shown on the plans please refer to section R311 of the Florida Residential Building Code which regulates construction of stairways.
- 5. Please refer to section R313 of the Florida Residential Building Code which requires the installation of smoke alarms. Smoke alarms shall be installed in the following locations: In each sleeping room, outside each separate sleeping area in the immediate vicinity of the bedrooms.
 - When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. The alarm shall be clearly audible in all bedrooms over background noise levels with all intervening doors closed. All smoke alarms shall be listed and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.
- 6. On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Also include the total amperage rating of the overcurrent protection device.

Thank You: <

be Haling Joe Haltiwanger Plan Examiner

Columbia County Building Department