

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1T158228Z0206134440

Permit #

26936

Truss Fabricator: Anderson Truss Company
Job Identification: 8-140--OWNER BUILDER Baker -- , **
Truss Count: 30
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Versions 7.24, 7.36, 7.37.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
the seal date per section 61G15-31.003(5a) of the FAC
Address:
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-TCFILLER-BCFILLER-140GS-VALTRUSS-140GC-160TL-A11015EE-GBLLETIN-PIGBACKB-

Seal Date: 06/06/2008

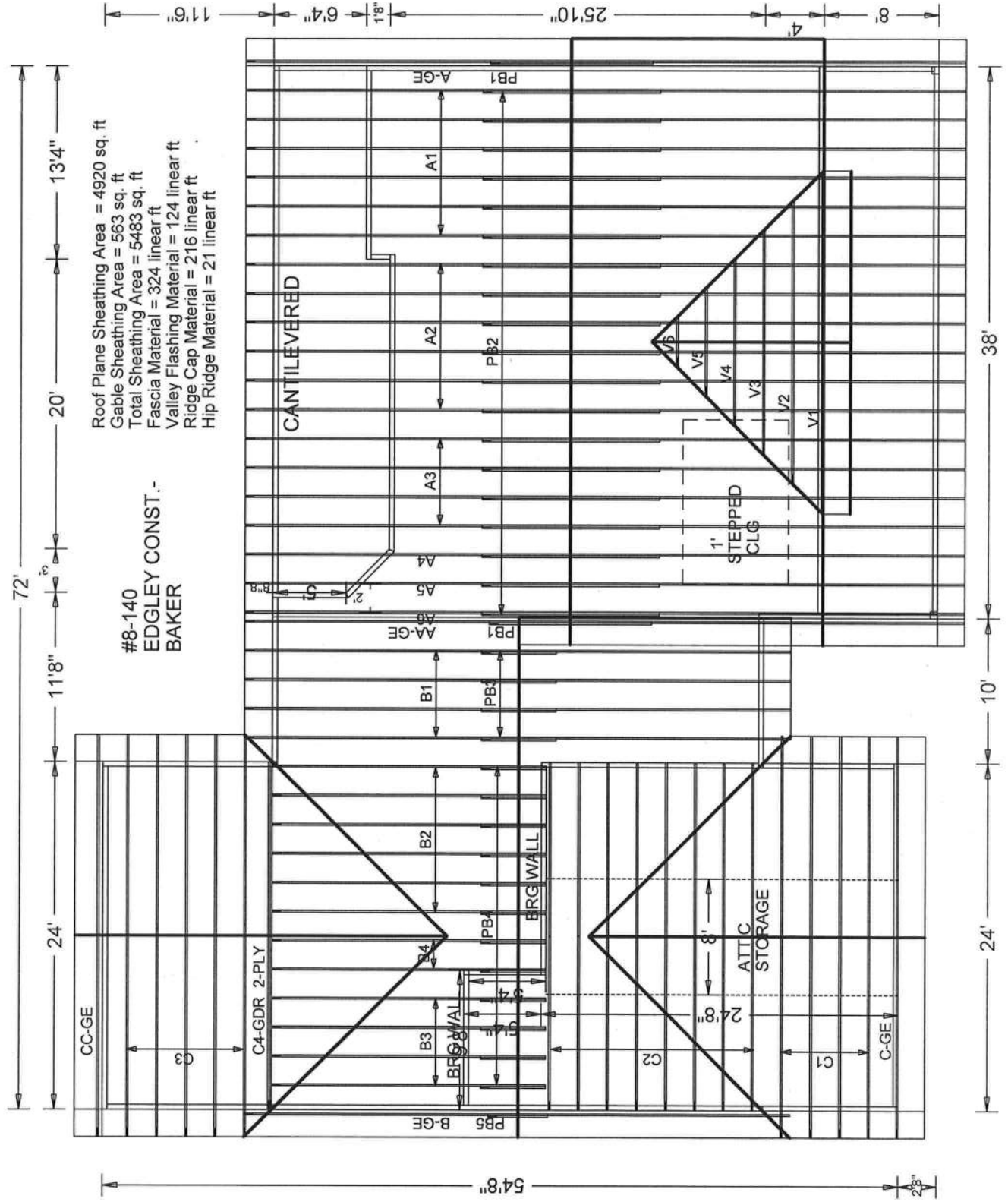
-Truss Design Engineer-
Doug Fleming

Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	09152--A2		08158008	06/06/08
2	09153--A3		08158022	06/06/08
3	09154--A4		08158023	06/06/08
4	09155--A5		08158024	06/06/08
5	09156--A6		08158009	06/06/08
6	09157--AA-GE		08158010	06/06/08
7	09158--A1		08158025	06/06/08
8	09159--A-GE		08158026	06/06/08
9	09160--V1		08158011	06/06/08
10	09161--V2		08158003	06/06/08
11	09162--V3		08158004	06/06/08
12	09163--V4		08158005	06/06/08
13	09164--V5		08158006	06/06/08
14	09165--V6		08158007	06/06/08
15	09166--B-GE		08158027	06/06/08
16	09167--B3		08158012	06/06/08
17	09168--B1		08158013	06/06/08
18	09169--B2		08158014	06/06/08
19	09170--B4		08158032	06/06/08
20	09171--C4-GDR		08158015	06/06/08
21	09172--C1		08158016	06/06/08
22	09173--C2		08158017	06/06/08
23	09174--C3		08158018	06/06/08
24	09175--C-GE		08158028	06/06/08
25	09176--CC-GE		08158029	06/06/08
26	09177--PB1		08158019	06/06/08
27	09178--PB2		08158020	06/06/08
28	09179--PB3		08158030	06/06/08
29	09180--PB5		08158031	06/06/08
30	09181--PB4		08158021	06/06/08

Repair Charge: \$13.75 per Customer Agreement.
Amount to be invoiced separately.





Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

LT Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

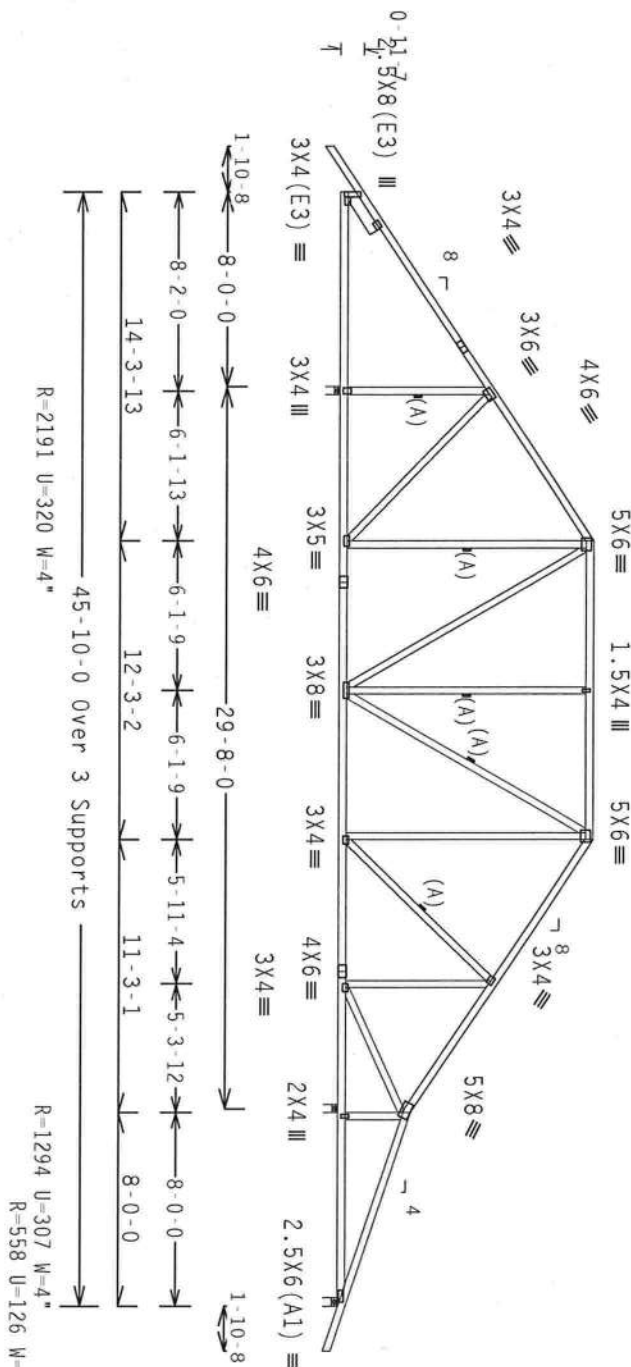
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $Gcpl(+/-)=0.55$

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

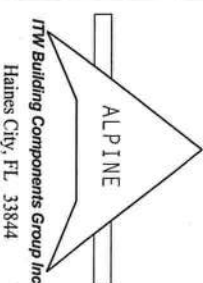
7.24.12

QTY: 6 FL/-/4/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS COMPANY OF AMERICA), 6200 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD INDOOR COUNCIL OF AMERICA), 6200 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. BY AWARD AND TPI. DONOR TO THE BCS, INC. SHALL BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. BY AWARD AND TPI. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC. 3.1 FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL 278 40 278



TC LL	20.0 PSF	REF	R8228- 9152
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158008
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	170256
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf (+/-)=0.55

See DWGS TCILLER0207 and BCILLER0207 for filler details.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Scale = 125"/Ft+

4.1238
DOUGLAS FLEMING
LICENSE
NO 66648
QTY

TC LL	20.0 PSF	REF	R8228- 9153
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 081580222

BC LL	0.0 PSF	HC-ENG DF/DF
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BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170279
DUP EAC	1 25	FROM AU

BOOK: FAC.	1:23
FROM	AM

SPACING 24.0" JREF - ITI58228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #3

1.0 Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C. including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI(+/-)-0.55

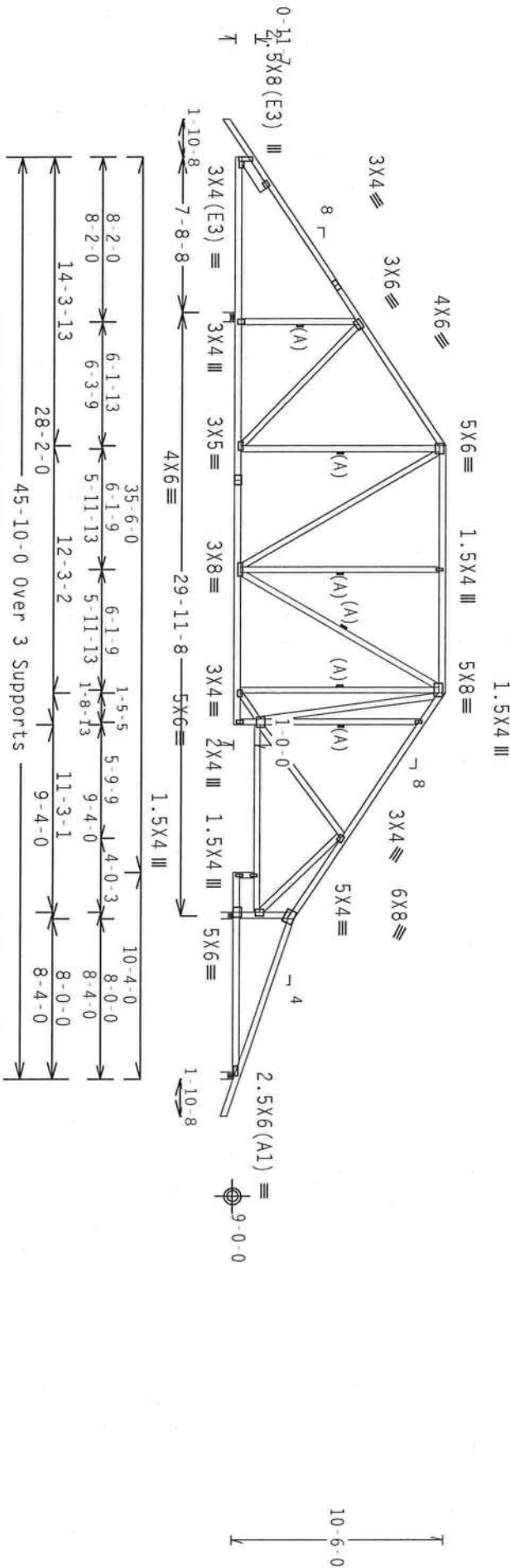
Wind reactions based on MMFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=2158 U=320 W=5.688"

R=1444 U=329 W=4"

R=441 U=107 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.13

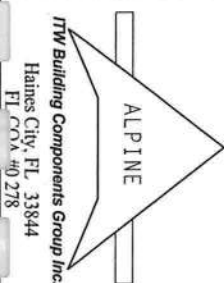
QTY: 1

FL/-/4/-/-/R/-

Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2180 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WCA (WOOD CONNECTIONS COMMITTEE OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. BY A/B/A/P/A AND TPI. THE BCG, INC. PROVIDES NO WARRANTIES OR ASSURANCES OF ANY KIND, INCLUDING DESIGN SPEC. BY A/B/A/P/A AND TPI. THE BCG, INC. PROVIDES NO WARRANTIES OR ASSURANCES OF ANY KIND, INCLUDING DESIGN SPEC. BY A/B/A/P/A AND TPI. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL 0776 40 278

TC LL	20.0 PSF	REF	R8228- 9154
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158023
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SECON-	170268
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	17158228202

Webbs	2x4	SP	#3
Filler	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C.
Including a lateral brace at chord ends.

110 mph wind; 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)=0.55

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.123

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING THESE BUILDING COMPONENTS EXISTED CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PROJECTS REFER TO GC51 (THROUGH COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRESS PATEL INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) OR 0000 TRUSS COMPANY OF AMERICA, 63000 INTERSTATE LAKE, MADISON, MI 48139 FOR SAFETY PRACTICES AND PRIOR TO DEMONSTRATION OF FUNCTIONS. UNDESIGNATED COMPONENTS INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AITPA) AND TPI. CONSTRUCTION OF THE TRUSS SHALL BE IN ACCORDANCE WITH THE NDS (NATIONAL DESIGN SPEC, BY AITPA) AND TPI.

CONCRETE FOR PLATES, MADE OF 20/30 MESH (THICKNESS 16.00, GRADE 40, 20/30) (C/W-35) G/40, STEEL, APPLIED TO EACH FACE OF THUS8 MAT. THUS8 (THICKNESS 16.00, GRADE 40, 20/30) POSITION PER DRAWING, 16.00-2. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TP11-2002 SEC.3.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE THUS8 COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TP11 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 9155
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158024
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON -	170272
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1T158228202

[illegible]

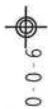
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC. bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.55

psf. $I_w=1.00$ gcpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

 $Cq/RT=1.00(1.25)/0(0)$

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

SHALL HAVE

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

6.04
OT

30, 90

SPACING

JREF- 1T158228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2 :B3 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:
Stack Chord 12 2x4 SP #2 Dense:
Stack Chord 17 2x4 SP #2 Dense:
Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

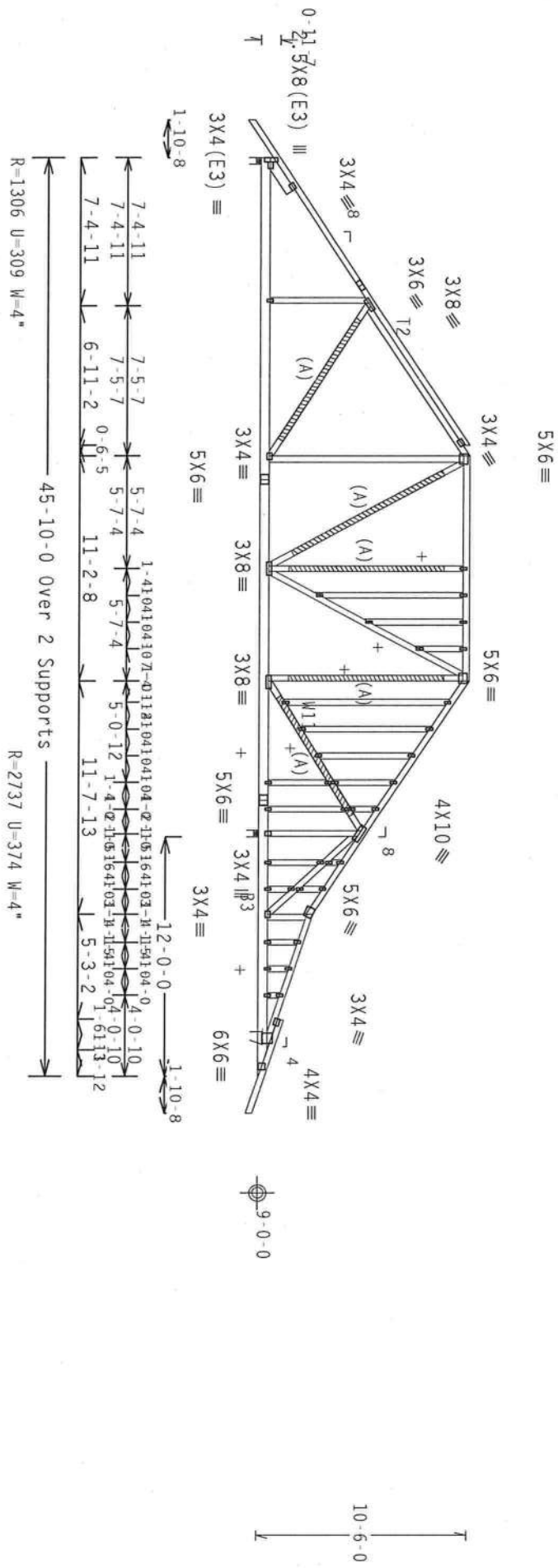
SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC, bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.55
Wind reactions based on MWFRS pressures.
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

PLT TYP. Wave QTY: 1 FL/-/4/-/R/- Scale = .125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS CONNECT) OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, BY A/RNA AND TPI.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL 773 40 278



TC LL	20.0 PSF	REF	R8228- 9157
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158010
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEON-	170275
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	UREF-	17158228202

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(A) Continuous lateral bracing equally spaced on member.



SPACING	24.0"	10FE	11FE6326703
CONC. 1.01	4.50	1.001	ALL

TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 0815802
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	170250
DUR.FAC.	1.25	FROM AH	
SPACING	24.0"	JREF-	1T158228202

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x6	SP	#2	
	webs	2x4	SP	#3	:W44

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

+ MEMBER TO BE Laterally Braced For Wind Loads Perpendicular To Truss. Bracing System To Be Designed And Furnished By Others.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24 OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/0(0)$

7.24.123

QTY:1

FL/-/4/-/-/R/-

Scale = .125"/Ft.

WARNING—TRIPES, BUILDING EXTREM CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PROTECTING TRIPES TO AVOID COMPROMISE SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND APCA 46000 TRUSS COMPANY OF AMERICA, 65000 INTERSTATE LAKE, MADISON, WI, 53719 FOR SAFETY PRACTICES, PLEASE TO PERFORM THE BEST OF WORKMANSHIP. INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group Inc

Haines City, FL 33844



TC LL	20.0 PSF	REF	R8228- 9159
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCU8R8228 08158026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	170182
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

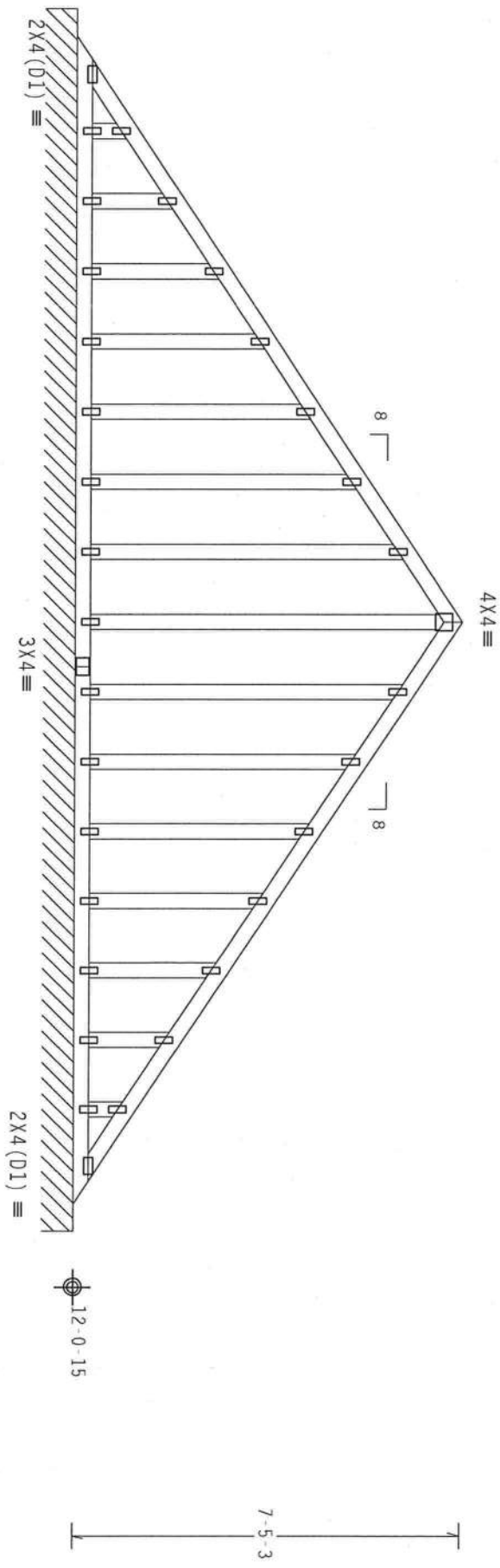
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

110 mph wind, 15.94 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MMFRS pressures.
See DWG VALTRUSS0207 for valley details.



R=164 PLF U=7 PLF W=23-4-2

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.123

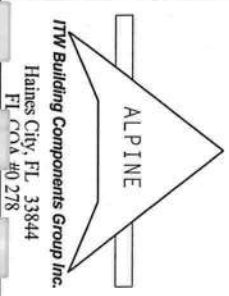
QTY:1

FL/-/4/-/-/R/-

Scale = .3125"/ft.

****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.



TC LL	20.0 PSF	REF	R8228 - 9160
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	46877
DUR. FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T158228202

110 mph wind, 16.78 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.
See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.12

QTY:1 FL/-/4/-/-/R/-

Scale = .375" / Ft.

No. 66648

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

* * * IMPORTANT * * * OBTAIN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG., INC. SHALL MAKE THE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. AN FAILURE TO BUILD THE TUBS IN CONFORMANCE WITH THE REG. OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BACKLOG OF TUBSSES.

TYPICAL ROOMS WITH APPLICABLE PROVISIONS OF MGS (NATIONAL DESIGN SPEC. BY AIRRA) AND TPI.

CONDUCTOR PLATES ARE MADE OF 20/18/60A (ALUMINUM) ASHT A665 STANDARD 46/60 (46-67/455) GALV. STEEL. APPLY

AN INSPECTION OF PLATES FOLLOWED BY SURVEY OF THE DESIGN. POSITION PER DRAWINGS 1406-2.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLIDITY FOR THE TUBS COMPONENT.

DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

06. 90

TC LL	20.0 PSF	REF R8228- 9161
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUR8228 08158003
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 46870
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1T158228202

REF	R8228- 9161
DATE	06/06/08
DRW	HCUSR8228 08158003
HC-ENG	DF/DF
SEQN-	46870
FROM	AH
JREF-	1T158228Z02

THE UNIVERSITY OF CHICAGO

110 mph wind, 17.45 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCDF (+/-)=0.18

See DWG VALTRUSS0207 for valley details.



Scale = .5"/Ft.

1.123
DOUGLAS FLEMING
LICENSE
No. 66648
QTY

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Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webs	2x4	SP	#3	

110 mph wind, 18.11 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCPI (+/-)=0.18

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.12

QTY:1

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

WARNING: THESE BUILDING COMPONENTS, WHEN USED IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING, ARE TO BE USED ONLY FOR THE PURPOSES SPECIFIED HEREIN. ANY OTHER USE IS PROHIBITED. FAILURE TO FOLLOW THESE PRECAUTIONS MAY RESULT IN PERSONAL INJURY OR DEATH.

REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE GROSS PANEL INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NICKI GOOD TRUSS COMPANY OF AMERICA, 65000 MIDWINTER LAKE, MOUNTAIN VIEW, MO 64080, AT 800-769-5379 FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNDESIRABLE DAMAGE TO PROPERTY ATTACHED TO TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 9163
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158005
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	46860
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228Z02

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webs	2x4	SP	#3	

110 mph wind, 18.78 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$



Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 9164
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158006
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	46856
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

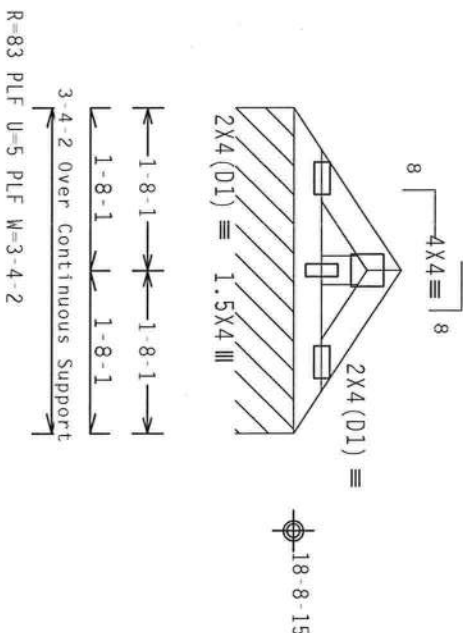
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.45 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

See DWG VALTRUSS0207 for valley details.



PLT TYP. Wave

Design Crit: TP1-2002 (STD) /FBC

$C_q/RT=1.00(1.25)/0(0)$

7.24.1230

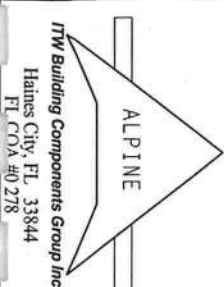
QTY: 1

FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. CONSULT COMPANIES WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ITW BCG CONSTRUCTION OF TRUSSES SHALL BE IN ACCORDANCE WITH THE TPI DESIGN SPEC. ALL TRUSSES SHALL BE DESIGNED TO MEET THE REQUIREMENTS OF THE TPI DESIGN SPEC. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 9165
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUR8228 08158007
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT. LD.	40.0 PSF	SEQN- 46852
DUR. FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1T158228202

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	Wbs	2x4	SP	#3	

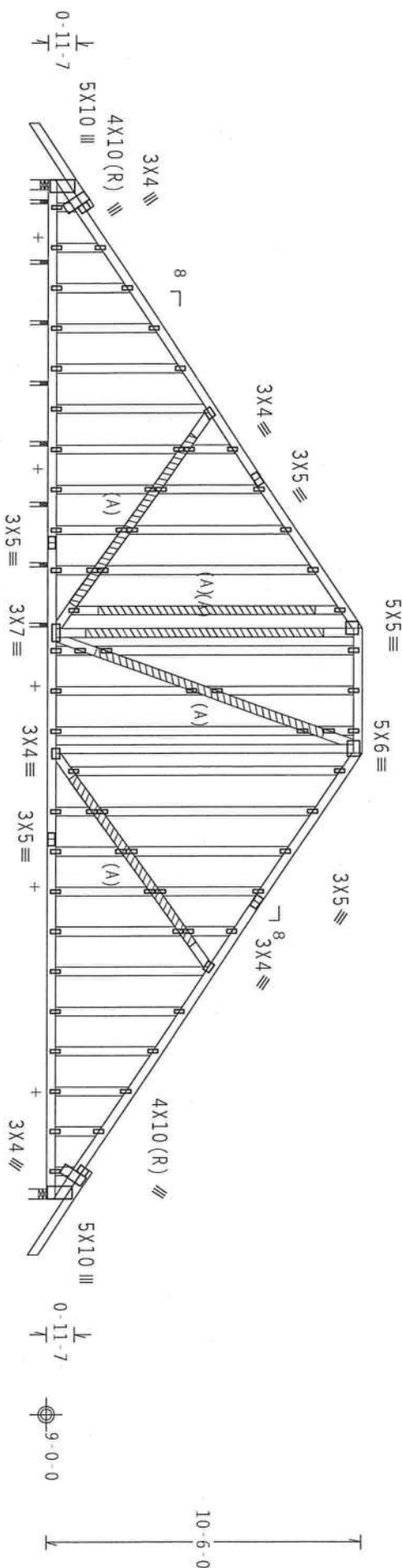
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

+ MEMBER TO BE Laterally Braced for Wind Loads Perpendicular to Truss. Bracing System to be Designed and Furnished by Others.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

[illegible]

$R=226$ $U=48$ $W=4''$ $R=209$ $U=4$ $W=1.5''$ $R=935$ $U=93$ $W=4''$
 $R=152$ $U=26$ $W=1.5''$ $R=144$ $U=36$ $W=1.5''$

R=38 R_w=56 U=60 W=1.5" R=221 U=12 W=1.5" R=904 U=13 W=1.5"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: $TPI - 2002(STD) / FBC$

PLT TYP. Wave

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1 FL/-/4/-/-/R/-/-

Scale = .1875"/Ft.

WARNING—TRUCKS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRIVING. REFER TO GC&I (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TROSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (IRON TROSS COUNCIL OF AMERICA, 65000 UNIVERSITY LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PERTAINING TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CEILING.

ALPINE

ITW Building Components Group Inc

Haines City, FL 33844
FL COA #0278



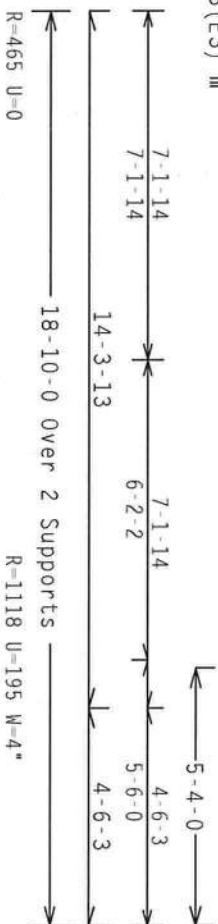
TC LL	20.0 PSF	REF	R8228- 9166
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCU8R8228 08158022
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	46919
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24 OC.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.12

QTY:4

FL/-/-/4/-/-/8/-/-

Scale = .25" / Ft.

WARNING THESE TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACKETING TO MEET THE FOLLOWING COMPONENTS INFORMATION. PUBLISHED BY THE TRUSS PLATING INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND IRCA GIRON TRUSS COMPANY OF AMERICA, 65000 ENTERPRISE LANE, MOBILE, AL 36689 FOR SAFETY PRACTICES PLEASE TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-7. CONNECTION PLATES ARE MADE OF 20/10/16GA (W, H/55/K) A514M A553 GRADE 40/60 (W, K/H/55) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844
FL COA #0278



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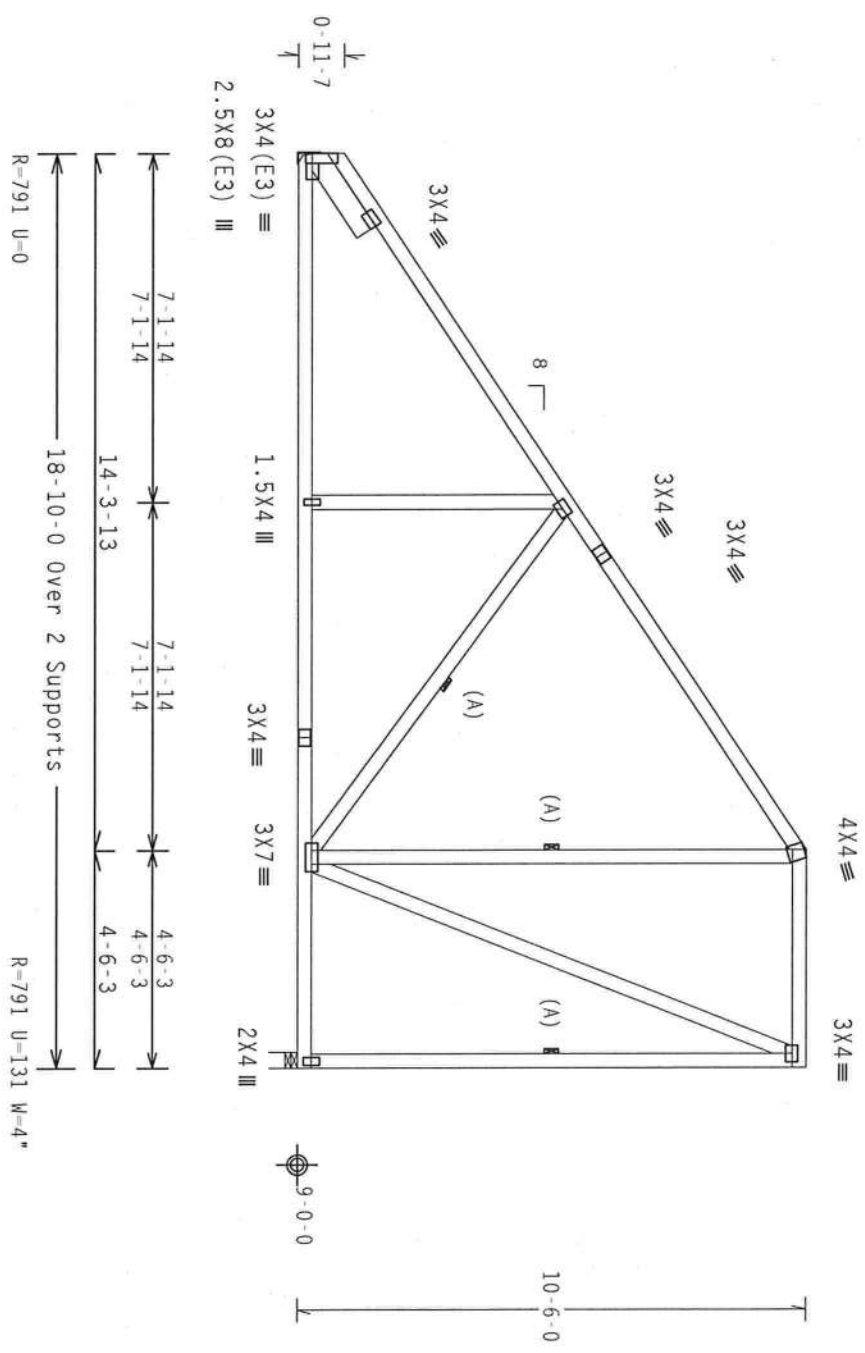
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18



FL/-/4/-/-R/-		Scale - .1875"/Ft.
TC LL	20.0 PSF	REF R8228- 9168
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCSUR8228 08158013
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170283
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	UREF- 1T158228202

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'
(A) Continuous lateral bracing equally spaced on member.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCP1 (+/-)=0.18
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

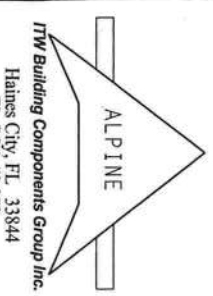
7.24.12

QTY: 8 FL/-/4/-/-/R/-

Scale = .25"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PDA) AND TPI. CONNECTION PLATES ARE MADE OF 2014/T606 (W/AL/SI/SI) ALUMINUM BRASS BRASS 40/60 (W, K/21/55) GALV. STEEL. APPLY TO ALL CONNECTIONS. ALL TRUSSES SHALL BE DESIGNED, MANUFACTURED AND TESTED IN ACCORDANCE WITH TPI-2002(STD)/FBC. ANY INSPECTION OF PLATES FOLLOWED BY PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 9169
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158014
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	40.0 PSF	SEQN-	170287
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228Z02

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

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:lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'
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Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

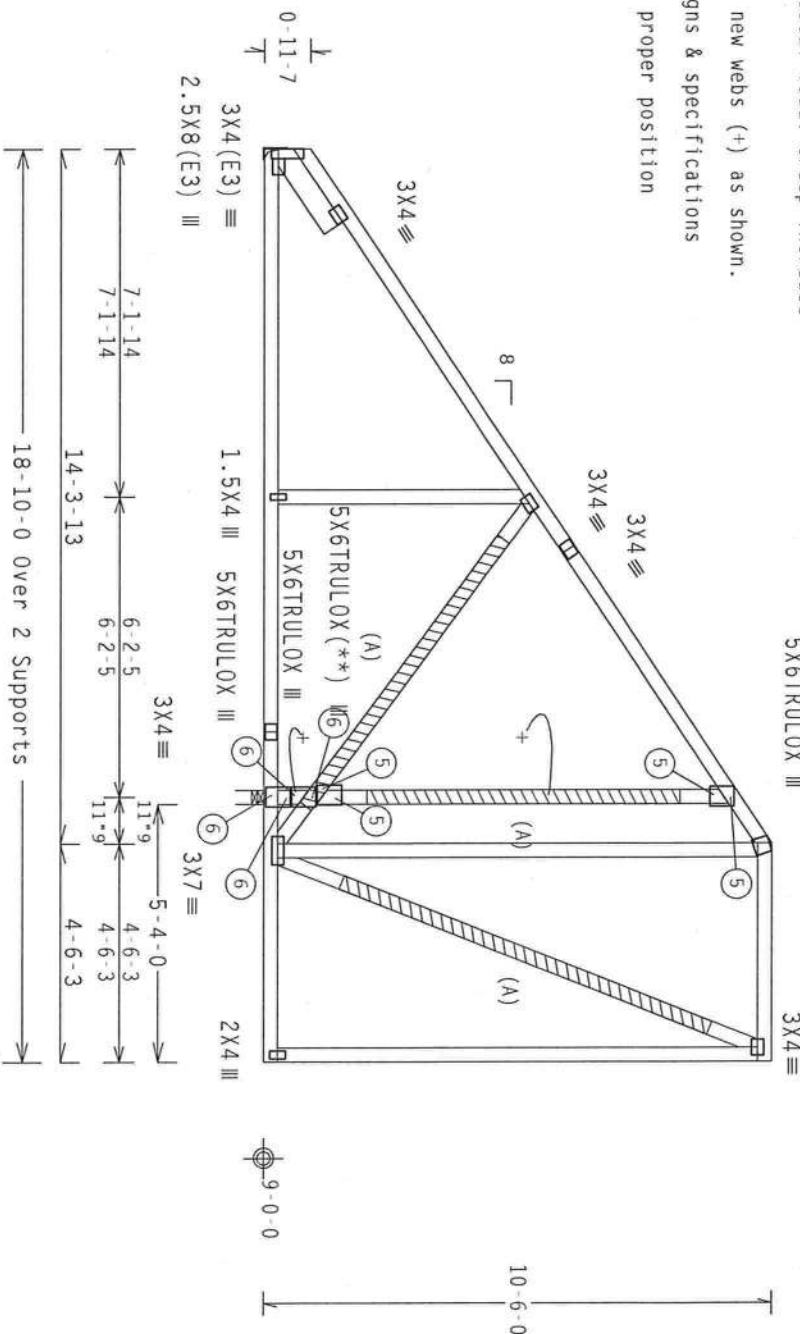
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

This truss is repaired to plate in two new webs (+) as shown.

Repair(s) must comply with Alpine designs & specifications

Shore Truss and any supported spans in proper position as repair is being made.



(**) I plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 1601L for nailing and trulox plate requirements.

4X4 ≡

PLT TYP. Wave, TruJox

TRUSS REPAIR

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.0421 QTY:

QTY:2 FL/-/4/-/-/R/-/-

Scale = .25"/Ft.

R=467 U=0

R=1116 U=195 W=3.5"

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PROPER SOLUTION IS TO SCRAP AND THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED. THEREFORE, IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE, IF ANY, AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING.



TC LL	20.0 PSF	REF	R8228- 9170
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCSUR8228 08158032
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	33617
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

SPECIAL LOADS

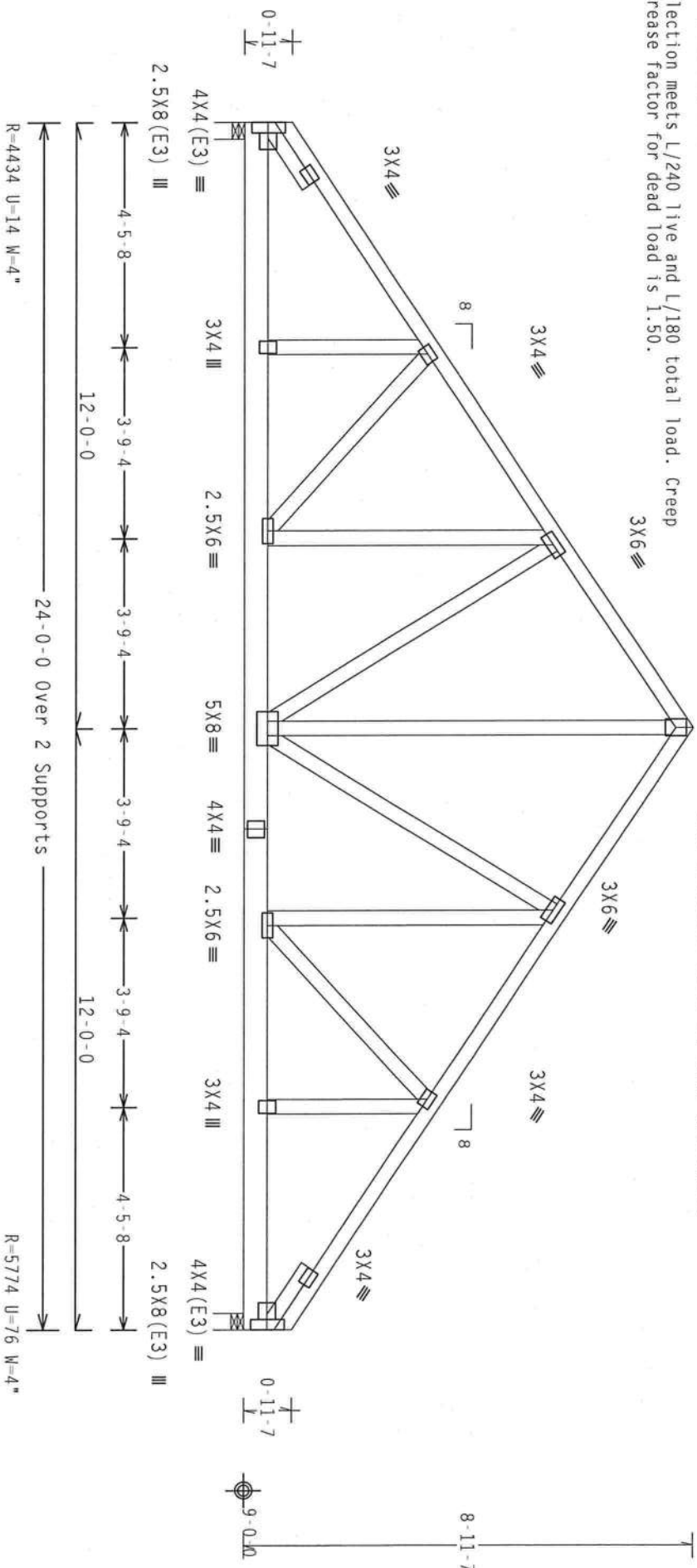
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at 0.00 to 64 PLF at 12.00
TC - From 64 PLF at 12.00 to 64 PLF at 24.00
BC - From 20 PLF at 0.00 to 20 PLF at 24.00
BC - 465 LB Conc. Load at 1.69, 3.69, 5.69, 7.69
BC - 791 LB Conc. Load at 9.69, 11.69, 13.69, 15.69, 17.69
19.69, 21.69, 23.69

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.) nails)
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 5.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)-0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.12

QTY: 1

FL/-/4/-/R/-

Scale = .3125"/ft.

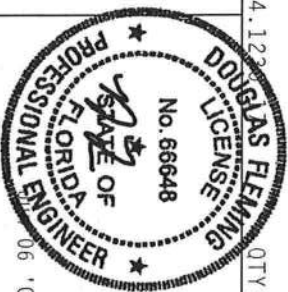
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ALPINE

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Haines City, FL 33844

PL 2011 #0 278



TC LL	20.0 PSF	REF	R8228 - 9171
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158015
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON	46976
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	17158228202

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Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
:B3 2x4 SP #2 Dense:
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Roof overhang supports 2.00 psf soffit load.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT 11, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI(+/-)=0.18



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.24.1230

QTY:4

FL/-/4/-/-/R/-/

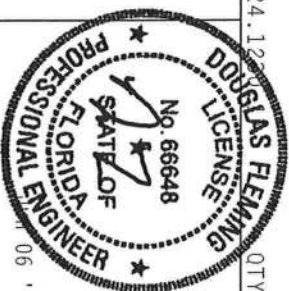
Scale = .25" / Ft.

"WARNING" - "TRUCKS REQUIRE EXTREME CARE IN LOADING, UNLOADING, SHIPPING, INSTALLING AND BRACING REFER TO GC-1 (BUILDING COMPONENT SAFETY INFORMATION) . PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA., 22314 AND WFLA (WOOD TRUSS COUNCIL OF AMERICA), 6700 UNIVERSITY LAKE DRIVE, SUITE #150, MIAMI FL 33197 FOR SAFETY PRACTICES & PROCEDURES PERTAINING THESE FUNCTIONS." UNLESS OTHERWISE INDICATED FOR CHORD SHAFT HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STUCCO CEILING.

ALPINE

ITW Building Components Group Inc

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 9172
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158016
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	170239
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

THE UNIVERSITY OF CHICAGO LIBRARY

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.08" due to live load and 0.18" due to dead load.

due to dead load.

Collar-tiebraced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

4X5(R) III

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS"). TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



Scale = .25"/Ft.

ALPINE

Haines City, FL 33844
FL COA #0278



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach
with 8d Box or Gun (0.113"x2.5".min.) nails @ 6" OC.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

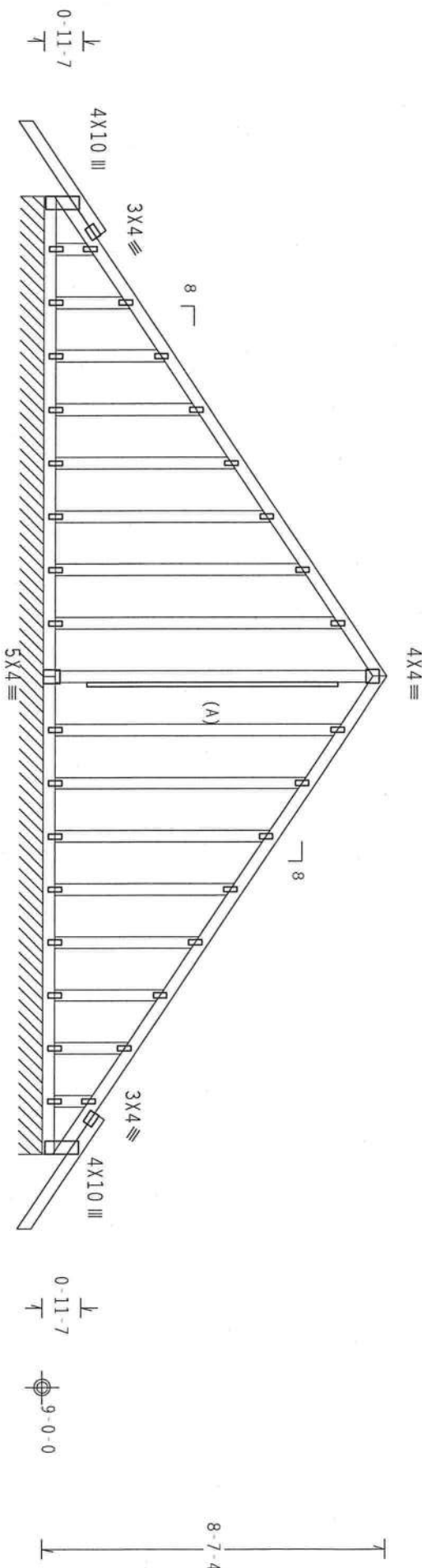
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0
psf. $I_w=1.00$ GCPI (+/-)-0.18

Wind reactions based on MMFRS pressures.

See DWS A11015EE0207 & GBLLETTM0207 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



R=204 PLF U=30 PLF W=24-0-0

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 $C_q/RT=1.00(1.25)/0(0)$

OTV:1 FL/-/4/-/R/-

Scale = .25"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2108 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6306 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



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Haines City, FL 33844

FL COA #0278



TC LL	20.0 PSF	REF R8228- 9175
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158028
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 47047
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1T158228202

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	
:Stack	Chord	T1	2x4	SP #2 Dense:
:Stack	Chord	T4	2x4	SP #2 Dense:

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE Laterally Braced For Wind Loads Perpendicular To Truss. Bracing System To Be Designed And Furnished By Others.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

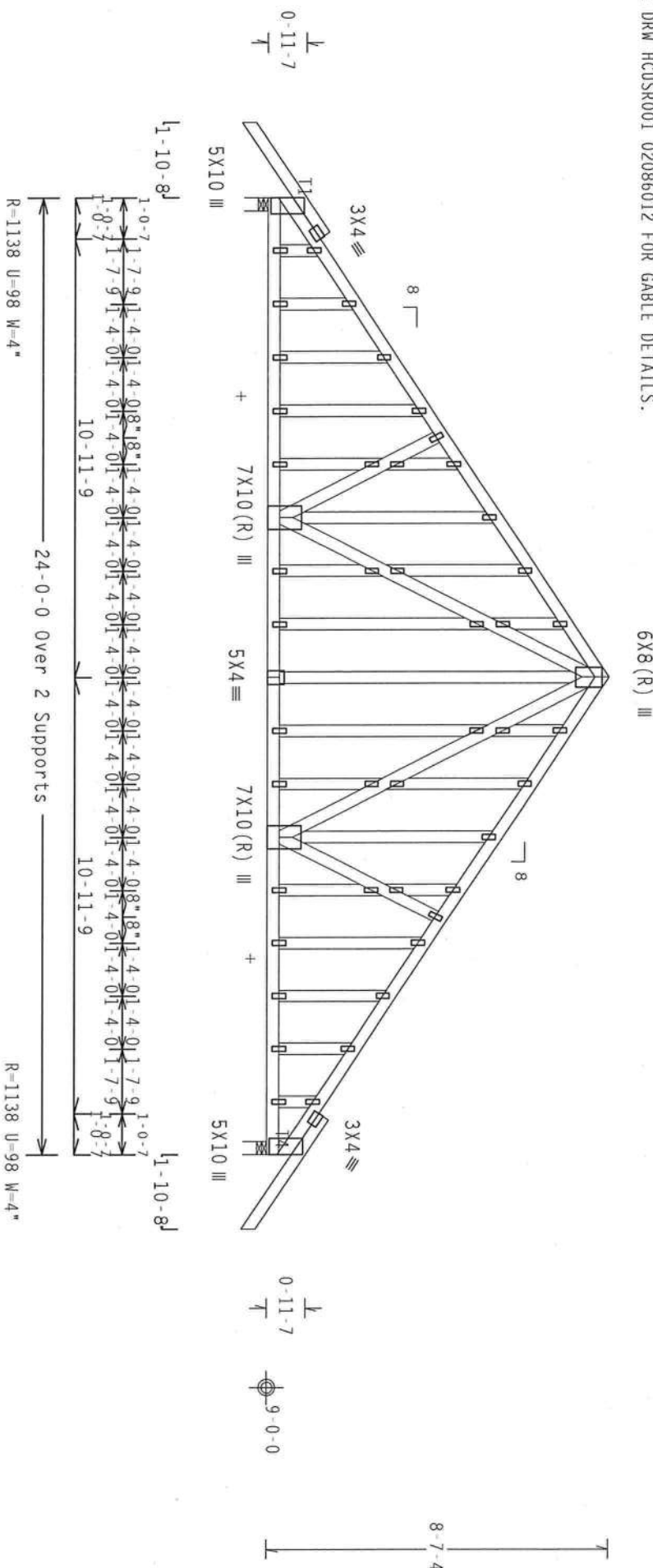
110 mph wind, 15.00 ft. mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-) = -0.18

Wind reactions based on MWERS pressures.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

Wind reactions based on MWFRS pressures.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

7.24.1236
GLAS FLEA
QTY:1

QTY:1 FL/-/4/-/-/R/-/

Scale = .25"/Ft.

[illegible]

****IMPORTANT**** TURN IN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING BRACKET OR TRUSSES.

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 9176
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158029
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	46837
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1T158228202

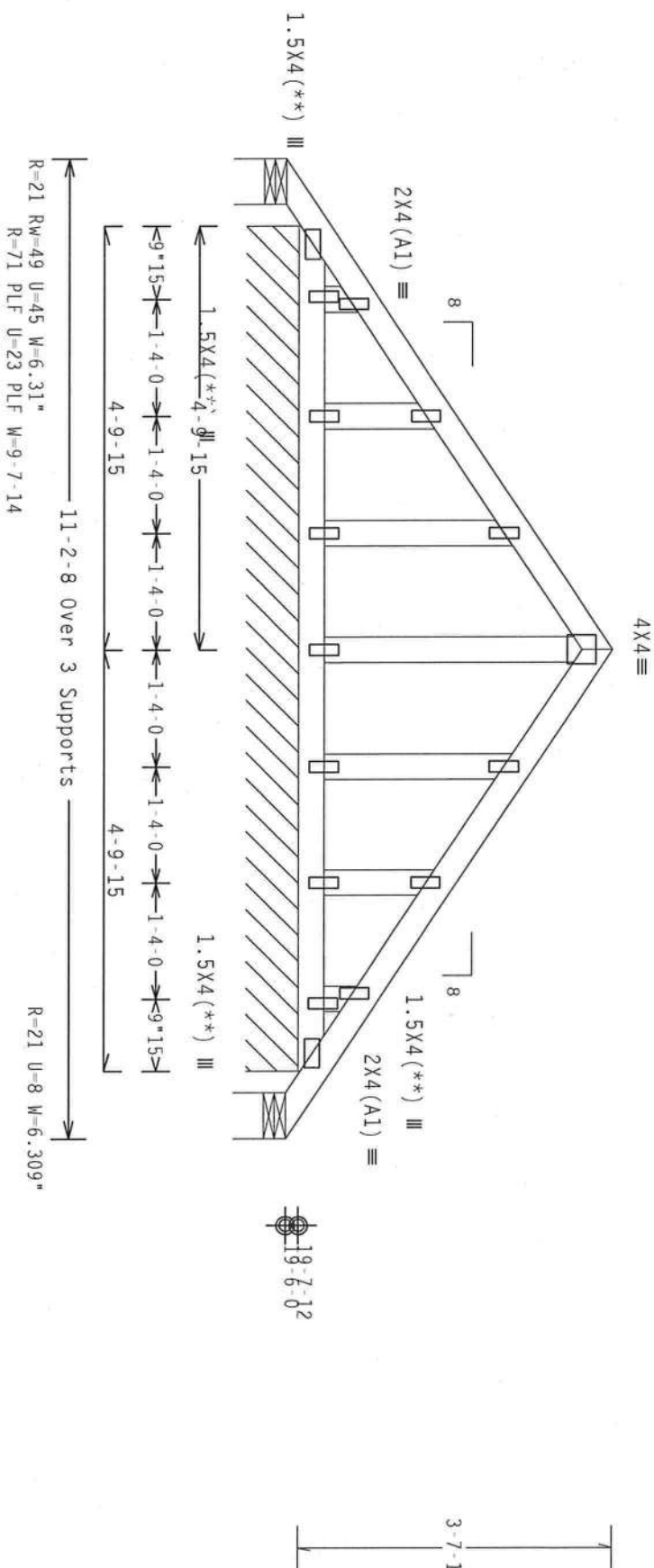
	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	Web	2x4	SP	#3		

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.123

QTY:2

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

WARNING: THESE RESIDUE EXTREMELY CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND DRAGGING REFER TO RCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE EGRESS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND NETA, 6000 TROSS CENTER OF AMERICA, 65000 ENTERPRISE LANE, MADISON, WI, 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED FIELD CELLING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FI #0278



TC LL	20.0 PSF	REF	R8228- 9177
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158019
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEQN-	46848
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

110 mph wind, 21.54 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf, lw=1.00 gcpl(+/-)0.18

Wind reactions based on MFRS pressures.



Scale = .5" / Ft.

DOUGLAS FLEMING
LICENSE
No. 66648

ITW Building Components Group Inc

Haines City, FL 33844
FL COA #0278

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CODE, INC., SHALL MAKE IT RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TOWNS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMBINES WITH APPLICABLE PROVISIONS OF AISC (ADDITIONAL DESIGN SPEC. BY AREA) AND TPI. THE RECORD DRAWINGS SHOULD BE 2016/1606 (C/H/S/S/P) ASH ASSY DRAFT, 4/9/06 (D), 4/7/08 GALL. STEEL, APPLY PROTECTIVE COATINGS TO EXPOSED SURFACES OF ALL STEEL MEMBERS.

AFTER INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF TPI-1-2002 SEC.2.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLID FOR THE TOWNS COMPONENTS OF THE BUILDING SHOWING THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 9178
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 0815802
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEQN -	170260
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

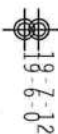
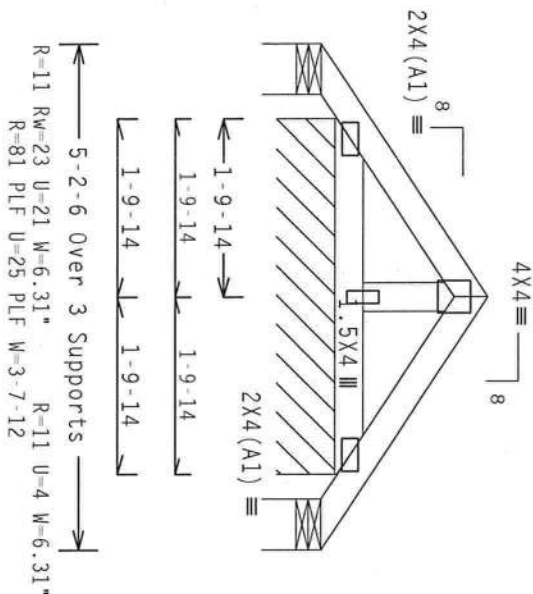
In lieu of rigid ceiling use purlins to brace BC @ 24" OC

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 Gcpi(+/-)0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.37.052

QTY: 4

FL/-/4/-/-/R/-

Scale = .5" / Ft.

[illegible]

ALPIN

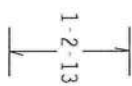
ITW Building Components Group Inc

Haines City, FL 33844
 FL COA #0278



TC LL	20.0 PSF	REF	R8228- 9179
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEON-	30920 REI
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

Wind reactions based on MWFRS pressures.



Scale = .5"/Ft.

Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228- 9180
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEQN-	30923 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T158228202

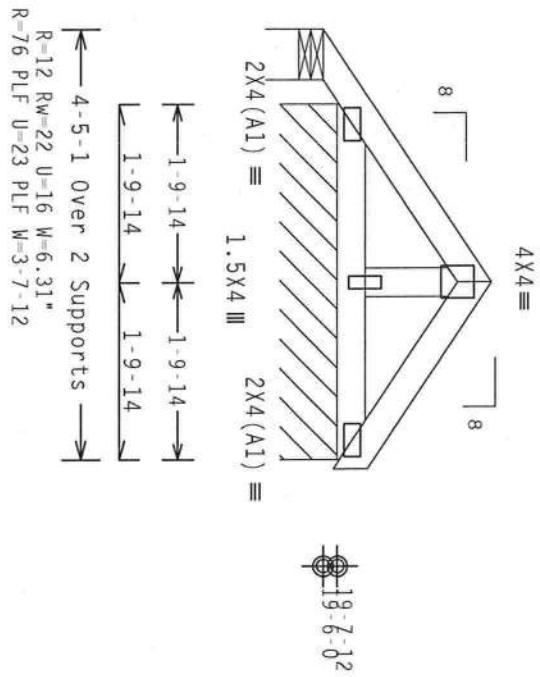
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf, IW=1.00 GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

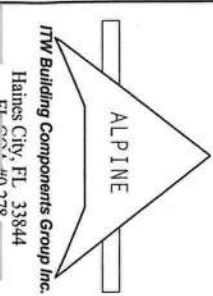
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0.0)

QTY:12 FL/-/4/-/R/-

Scale =.5"/Ft.

WARNING TRUSSES REQUIRE EXACTING CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO THE FOLLOWING BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSSES, INC., 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND IBCA (GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. TPI BCG PLACES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2, 1604-3, 1604-4, 1604-5, 1604-6, 1604-7, 1604-8, 1604-9, 1604-10, 1604-11, 1604-12, 1604-13, 1604-14, 1604-15, 1604-16, 1604-17, 1604-18, 1604-19, 1604-20, 1604-21, 1604-22, 1604-23, 1604-24, 1604-25, 1604-26, 1604-27, 1604-28, 1604-29, 1604-30, 1604-31, 1604-32, 1604-33, 1604-34, 1604-35, 1604-36, 1604-37, 1604-38, 1604-39, 1604-40, 1604-41, 1604-42, 1604-43, 1604-44, 1604-45, 1604-46, 1604-47, 1604-48, 1604-49, 1604-50, 1604-51, 1604-52, 1604-53, 1604-54, 1604-55, 1604-56, 1604-57, 1604-58, 1604-59, 1604-60, 1604-61, 1604-62, 1604-63, 1604-64, 1604-65, 1604-66, 1604-67, 1604-68, 1604-69, 1604-70, 1604-71, 1604-72, 1604-73, 1604-74, 1604-75, 1604-76, 1604-77, 1604-78, 1604-79, 1604-80, 1604-81, 1604-82, 1604-83, 1604-84, 1604-85, 1604-86, 1604-87, 1604-88, 1604-89, 1604-90, 1604-91, 1604-92, 1604-93, 1604-94, 1604-95, 1604-96, 1604-97, 1604-98, 1604-99, 1604-100. THE BCG DESIGN SHOWN, THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 9181
TC DL	10.0 PSF	DATE 06/06/08
BC DL	2.0 PSF	DRW HCUR8228 08158021
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	32.0 PSF	SEQN- 46941
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1T158228Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

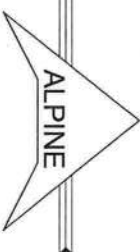
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW 2 ROWS	2X4 2X6	1-2X4 2-2X4
2X6	1 ROW 2 ROWS	2X4 2X6	1-2X6 2-2X4(*)
2X8	1 ROW 2 ROWS	2X6 2X6	1-2X8 2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



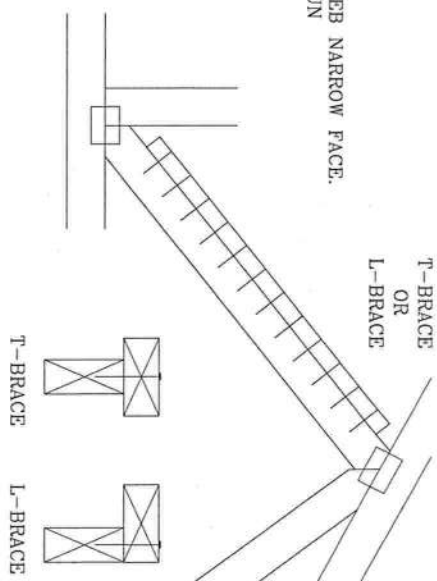
TRUSSING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE MANUFACTURERS ASSOCIATION, INC., 1000 W. 10TH AVE., SUITE 100, DENVER, CO 80202-1500, FOR THE LATEST DESIGN PRACTICES. THESE FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN AND FAILURE TO BUILD THE TRUSS IN ACCORDANCE WITH THE DESIGN. THE TRUSSING CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSSING DESIGN. THE TRUSSING CONTRACTOR SHALL DESIGN CONNECTIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASD) AND ITV, BCG CONNECTOR PLATES ARE MADE OF 2018/1654 (A/H/SS/CO) ASTM A653 GRADE 40/60 (A/H/SS/CO) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED BY CD SHALL BE PER DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND STRENGTH OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

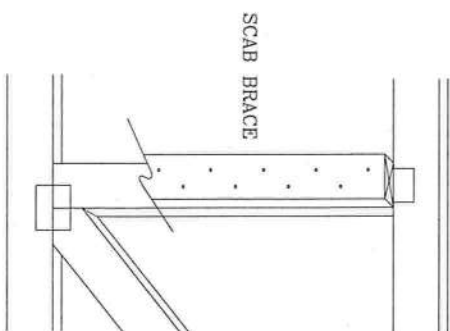
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579.640



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCBLSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TOP CHORD FILLER DETAIL

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C.
MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH

(2) 16d COMMON (0.162"X 3.5",MIN) NAILS.

BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED
48" OC MAXIMUM.

* 8/12 MAXIMUM PITCH.

**** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.**

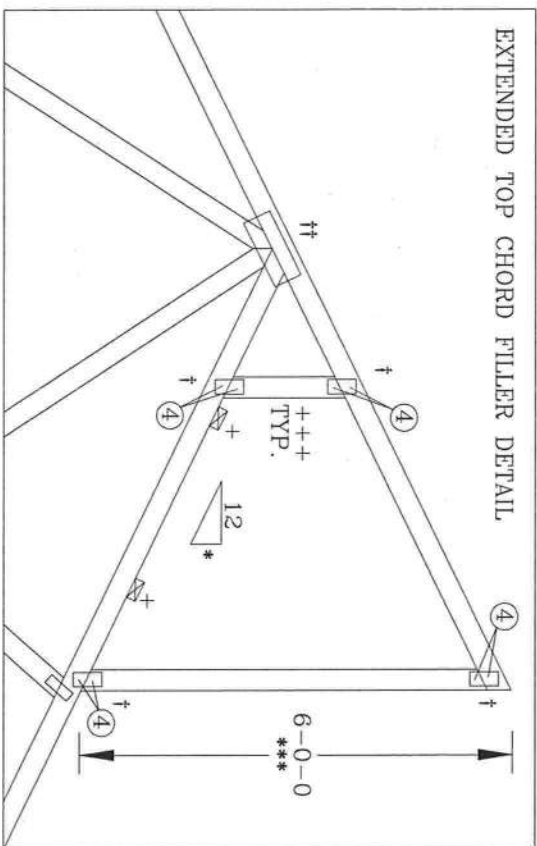
*** 6'0" MAXIMUM HEIGHT.

† W2X4 OR 3X6 TRULOX.

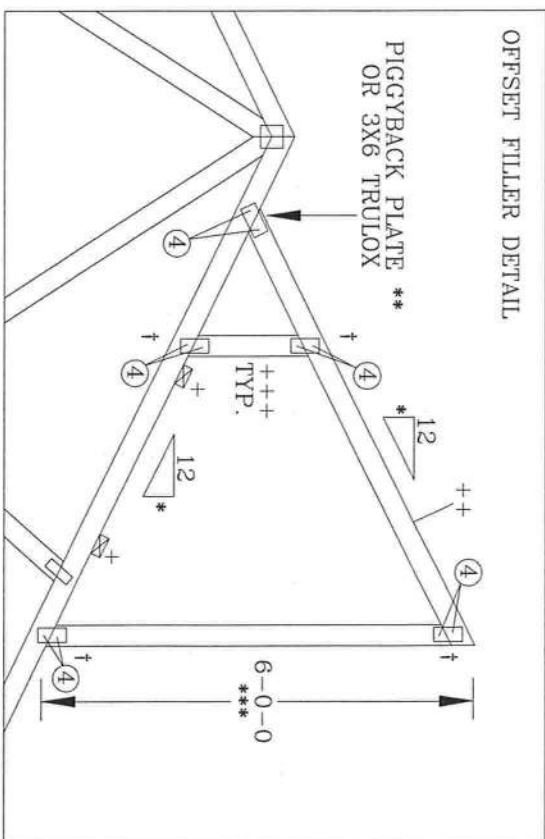
IF REFER TO ENGINEERS SEALED DESIGN REFERENCING THIS
DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT
SHOWN.

0.120"X 1.375" NAILS REQUIRED
FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED
IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY
SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS

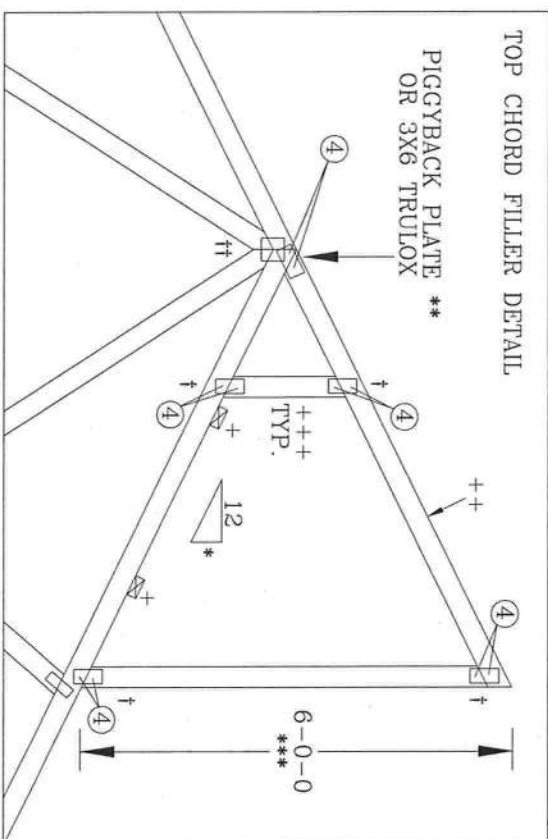
EXTENDED TOP CHORD FILLER DETAIL



OFFSET FILLER DETAIL



TOP CHORD FILLER DETAIL



THIS DRAWING REPLACES DRAWING 884,080



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

*****WARNING***** TESTS REQUIRE EXTREME CARE FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE STEEL PLATE INSTITUTE, 218 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA 22304 AND AISC/CES TRUSS COUNCIL, 1000 AUSTIN AVENUE, SUITE 100, WESTPORT, CT 06880. FOR SAFETY PRACTICES BEFORE TO PERFORMING THESE TESTS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

FUNCTIONS, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING

***IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN

CONFORMANCE WITH TPI, OR FABRICATING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AIA/P) AND TPI.

11W, BCU CONCRETE PLATES ARE MADE UP 20/18/1604 (W/H/SS/CK) ASTM A653 GRADE 40/60 (C/X/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 1604-2 ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER

DESIGNED BY JOHN L. BERRY, P.E., A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS. THIS DRAWING INDICATES ACCEPTANCE BY SEP. 08/06, SEE PLAN ANEX A-3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND

USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

[illegible][illegible]

DOUGLAS FLEMING
LICENSE
No. 66648
JUN 06 '08

TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	2/23/07
BC DL	MAX 10 PSF	DRWG	TCFILLER0207
BC LL	0 PSF	-ENG	SJP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

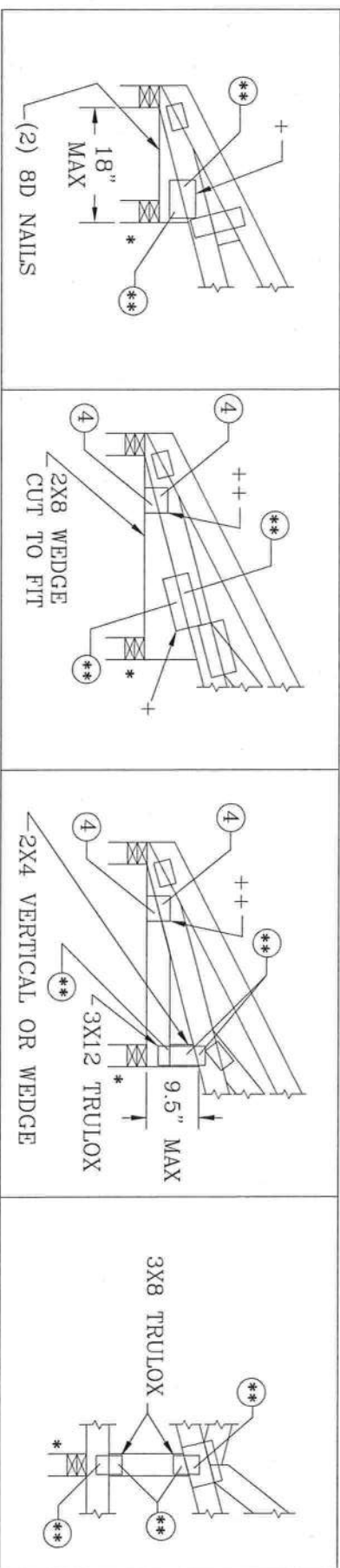
+ 3X4 WAVE OR 4X8 TRULOX
++ 2X4 WAVE OR 3X6 TRULOX

0.120" X 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 1607L FOR NAILING AND TRULOX PLATE REQUIREMENTS

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL, FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA		** REQUIRED NAILS PER FACE WITH TRULOX PLATES					
	DOWNWARD	UPLIFT			1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.	
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"		12	11		10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"		9	8		7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"		10	9		8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"		12	11		10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"		10	9		8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"		9	8		7	7	6



THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884.132



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314 AND VITA C/DOO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. TPI BCS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. TPI, BCS CONNECTOR PLATES ARE MADE OF 2018/1616GA (V/A/SS/CO) ASTM A653 GRADE 40/60 (V/A/H/SS) PLATE. STITCHED AND PLATED TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE Labeled ON THIS PER DESIGNER'S DRAWING PER TPI. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



TC LL	—	PSF	REF	BC FILLER
TC DL	—	PSF	DATE	2/23/07
BC DL	10.0	PSF	DRWG	BCFILLER0207
BC LL	—	PSF	—	ENG DL/KAR
TOT. LD.	—	PSF		
DUR. FAC. 1.0/1.15/1.25/1.33				
SPACING 24.0"				

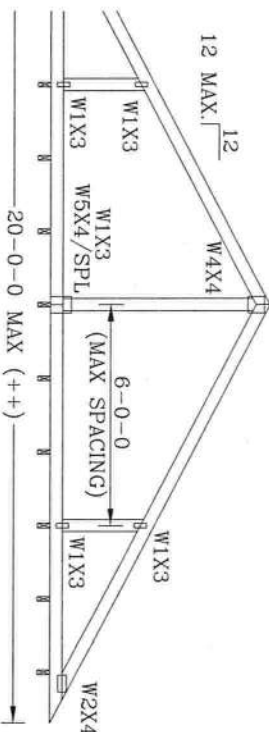
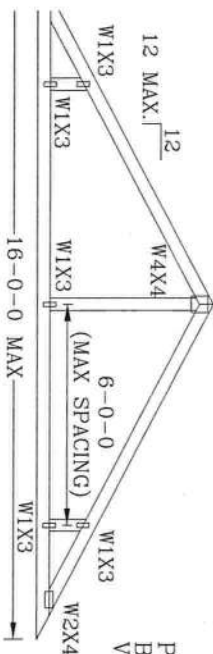
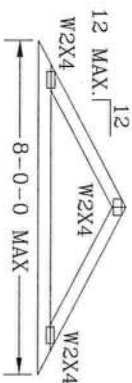
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

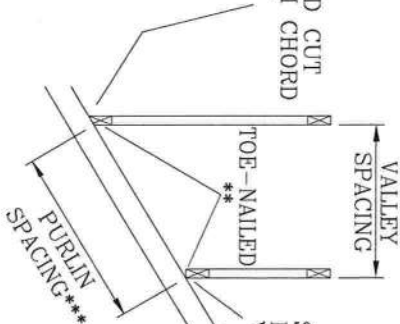
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98,
ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN
HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL,
WIND TC DL=5 PSF



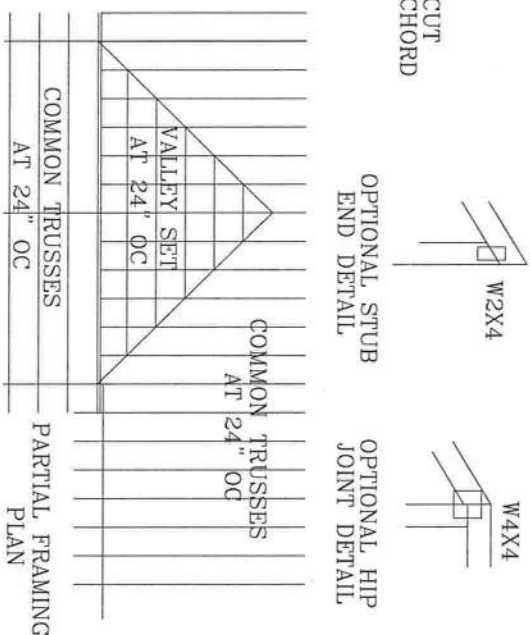
SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.



*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

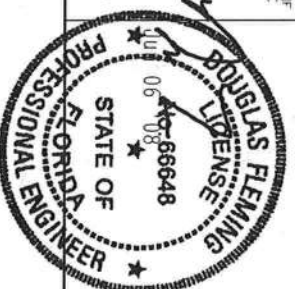


THIS DRAWING REPLACES DRAWING A105



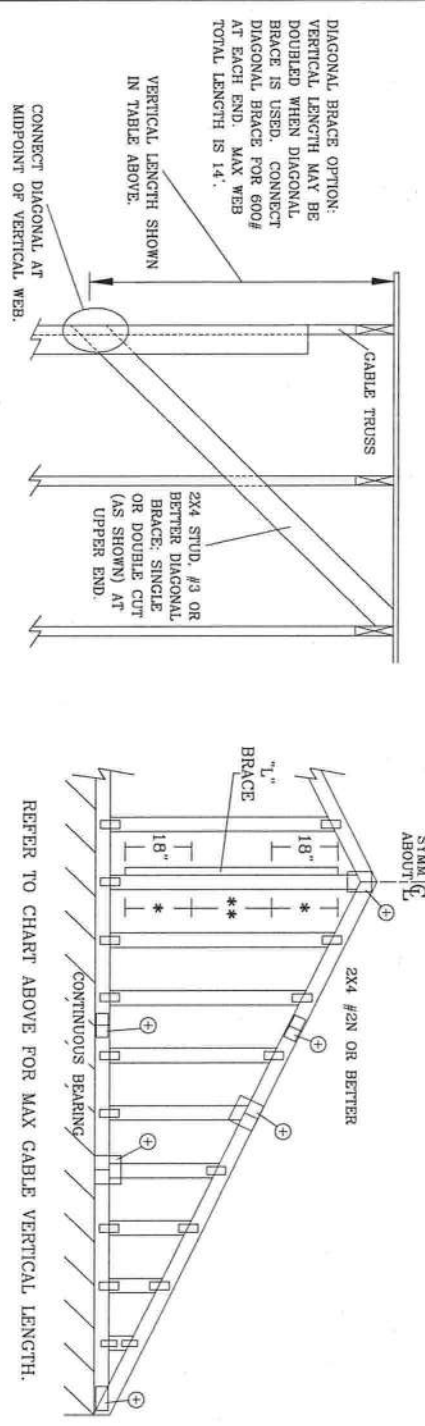
ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA 22314 AND VITA CORD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS, QNTANTIL, DESIGN SPEC. BY ACP&PA AND TPI. CALV. BEG. CONNECTS PLATES ARE MADE OF 60/30/16GA C/A/H/SS/438 WITH 4633 GRADE 40/60 C/A/H/SS/438. G.A.V. BEG. CONNECTS PLATES ARE MADE OF 60/30/16GA C/A/H/SS/438 WITH 4633 GRADE 40/60 C/A/H/SS/438. DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER DESIGN. ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



TC LL	30	30	40 PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	2/23/07
BC DL	10	10	10 PSF	DRWG	VALTRUSS0207
BC LL	0	0	0 PSF	-ENG	MLH/KAR
TOT. LD.	60	55	57 PSF		
DUR.FAC.	1.25/1.33	1.15/1.15			
SPACING	24"				

2x4 GABLE VERTICAL		BRACE		NO BRACES		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE *		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **	
SPACING	SPECIES	GRADE				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	STUD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
				3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
				3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
				3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
				3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	STUD	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
				4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	STUD	4' 5"	6' 1"	6' 1"	6' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"
				4' 5"	6' 1"	6' 1"	6' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"
				4' 5"	6' 1"	6' 1"	6' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"
				4' 5"	6' 1"	6' 1"	6' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"
				4' 5"	6' 1"	6' 1"	6' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"



BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCES-PINE-FIR	HEM-FIR	SPRUCES-PINE-FIR	HEM-FIR
#1 / #2	#2	#1 / #2	#2
STUD	STUD	STUD	STUD
STANDARD	STANDARD	STANDARD	STANDARD

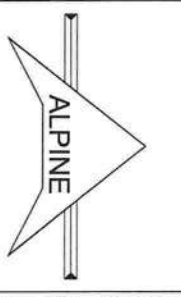
GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.
PROVIDE UPLIFT CONNECTIONS FOR 80 PSF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C. IN 18" END ZONES AND 4' 0" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 18" END ZONES AND 6' 0" O.C. BETWEEN ZONES.
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2x4
GREATER THAN 11' 6"	2.5x4

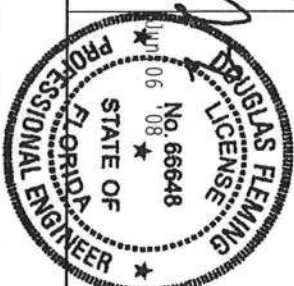
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EEO207
ENG	
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

Diagram illustrating the cable length type (CABLE LENGTH TYPE) and the vertical length type (VERTICAL LENGTH TYPE) for a cable-stayed bridge. The diagram shows a cross-section of the bridge deck with a cable attached to the top. The cable length is labeled 'CABLE LENGTH TYPE' and the vertical length is labeled 'VERTICAL LENGTH TYPE'. The diagram also shows the cable length type (CABLE LENGTH TYPE) and the vertical length type (VERTICAL LENGTH TYPE).

EXAMPLE: 2

* IF CABLE VERTICAL SINGLE PLATE TO

REFER TO ENGINEER

SPLICE, WEB AND

LESS THAN 11' 6"

GREATER THAN 11' 6"

LESS THAN 4' 0"

GREATER THAN 4' 0"

BETWEEN CHORDS

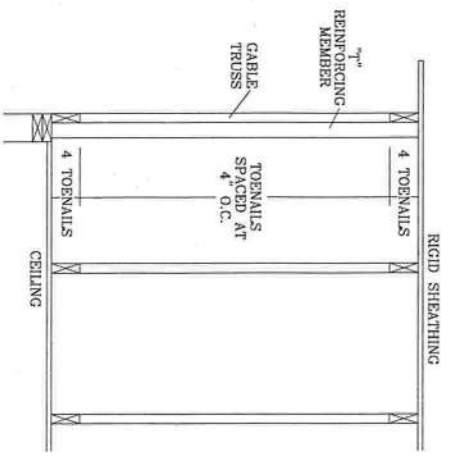
④ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

10d COMMON (0.148" X 3." MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE
OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS
 A11015EN0207, A10015EN0207, A08015EN0207, A07015EN0207
 A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
 ASCE 7-98 GABLE DETAIL DRAWINGS
 A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08015EC0207,
 A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08030EC0207
 ASCE 7-02 GABLE DETAIL DRAWINGS
 A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08015EE0207,
 A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08030EE0207
 ASCE 7-05 GABLE DETAIL DRAWINGS
 A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08015E50207,
 A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08030E50207
 SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI
 WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE
 VERTICAL LENGTH.



THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

WIND SPEED AND MRH	T _{air} REHF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	15 FT	20 %	40 %
30 FT	2x6	20 %	10 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	0 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"I" REINFORCING MEMBER SIZE = 2x4
"I" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2x4 "I" BRACE LENGTH = 6' 7"
MAXIMUM "I" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER SECTION 1.1, SEC. 2.



REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR
MAX TOT. LD. 60 PSF	
DUR. FAC.	ANY
MAX SPACING 24.0"	

TOP	CHORD	2X4	#2	OR	BETTER
BOT	CHORD	2X4	#2	OR	BETTER
	WEBS	2X4	#3	OR	BETTER

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

ATTACH VERTICAL WEBS TO
BOTTOM CHORD MAY BE OMITTED.
TRUSS TOP CHORD WITH 1.5X3 PLATE.

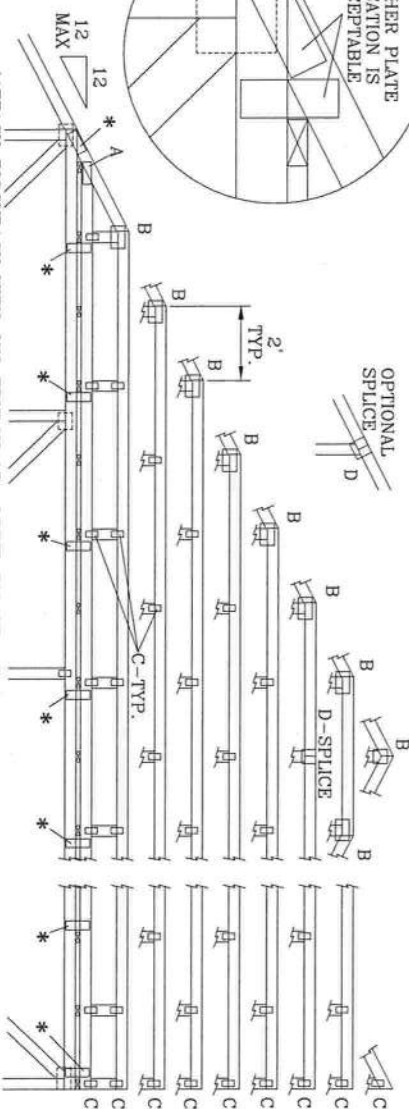
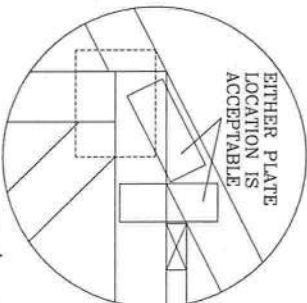
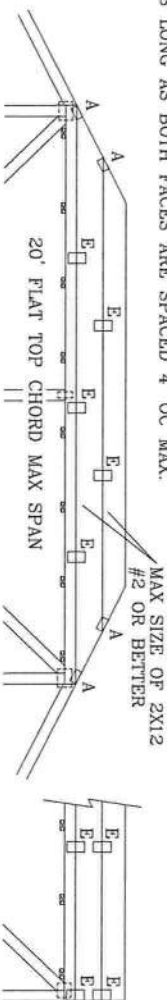
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

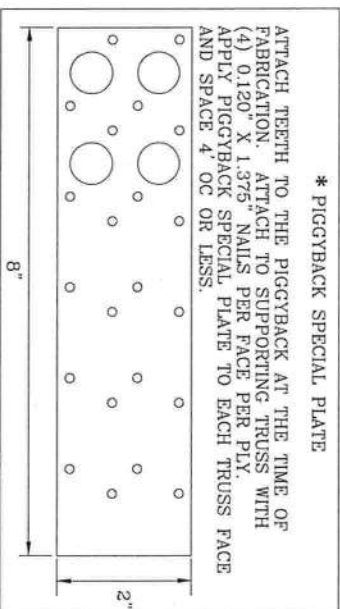
130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR
ASCE 7-05, CLOSD BLDG, LOCATED ANYWHERE IN ROOF, CAT II
EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, SBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL. PLATE

THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045



JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7' 9"	NO BRACING
7' 9" TO 10'	1x4 "t" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113" X 2.5" MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "t" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135" X 3.5" MIN) NAILS AT 4" OC.

ATTACH TRULOX PLATES WITH (3) 0.120" X 1.375" NAILS OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

* PIGGYBACK SPECIAL. PLATE

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

*BRAINING** THESE REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHING BY THE STEEL PLATE INSTITUTE, 218 NORTH LE STE, SUITE 312, ALEXANDRIA VA 22314 AND VICA (VOID) TROSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, UT 57749 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIONS.

*BRACING** OVERLAP INDICATED.

*CLADDING** CLADDING SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

*EARTHQUAKE** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1, OR EARTHQUAKING, HANDLING, SHIPPING, INSTALLING, DESIGN & BRACING OF TRUSSES. DESIGN CONTRACTS WITH TP1, OR APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY ARCA AND TP1.

*FLOOR CONNECTOR PLATES ARE MADE OF 20X18X16GA (W/SH/5X) ASTM A653 GRADE 49/60 (C/KH/SS) TYPE 304 STAINLESS STEEL. EACH FACE OF OVERLAP SHALL BE FULLY WELDED TO THE OTHER SIDE OF THE PLATE POSITION PER DRAWING.

*GIRDS** GIRDS SHALL BE DESIGNED TO SUPPORT ALL LOADS INCLUDING DEAD, LIVE AND WIND LOADS PER ANNEX A3 OF TP1-1-2002 SEC. 3, A SEA, ON THIS BRAVING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TP1 SEC. 2.



MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	2/23/07
1.33 DUR. FAC.	DRWG	PIGBACKB0207
50 PSF AT	-ENG	DLJ/KAR
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING		24.0"

(**) 2X4 SO. PINE #3 GABLE STUDS. ATTACH TO TOP CHORD. DIAGONAL MEMBERS AND BOTTOM CHORD WITH W2X4 ALPINE PLATES. ALL (**) GABLE STUDS REQUIRED REINFORCING MEMBER. REINFORCING MEMBER MUST BE TOENAILLED TO GABLE STUD WITH 0.131"x3" GUN NAILS AT 4" O.C. PLUS A CLUSTER OF 0.131"x3" TOENAILS AT THE TOP AND BOTTOM CHORD. SEE DETAIL FOR NAILING. SEE CHART FOR STUD BRACING AND SPACING OF VERTICALS.

NOTE: TRUSS ERECTOR IS RESPONSIBLE FOR PERMANENT WEB BRACING. WHEN BRACING IS REQUIRED. FURNISH A COPY OF THIS DRAWING TO TRUSS ERECTOR.

+PLATE AS REQUIRED ON APPROPRIATE DRAWING.

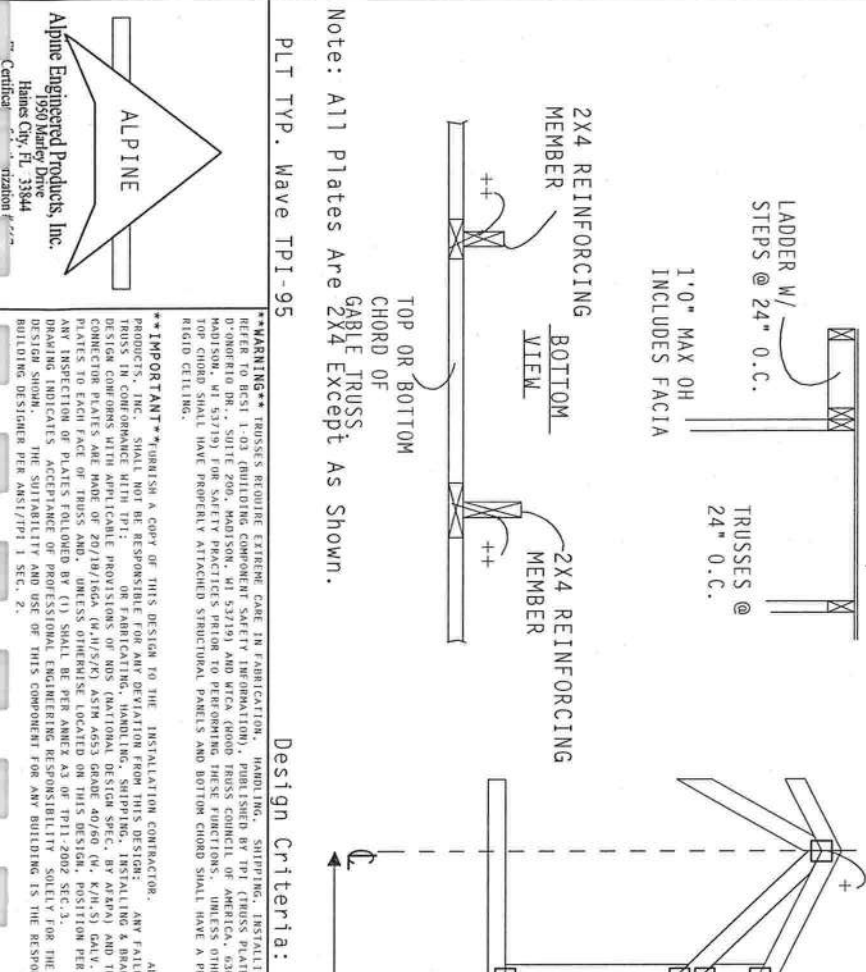
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

NOTE: MAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

+++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

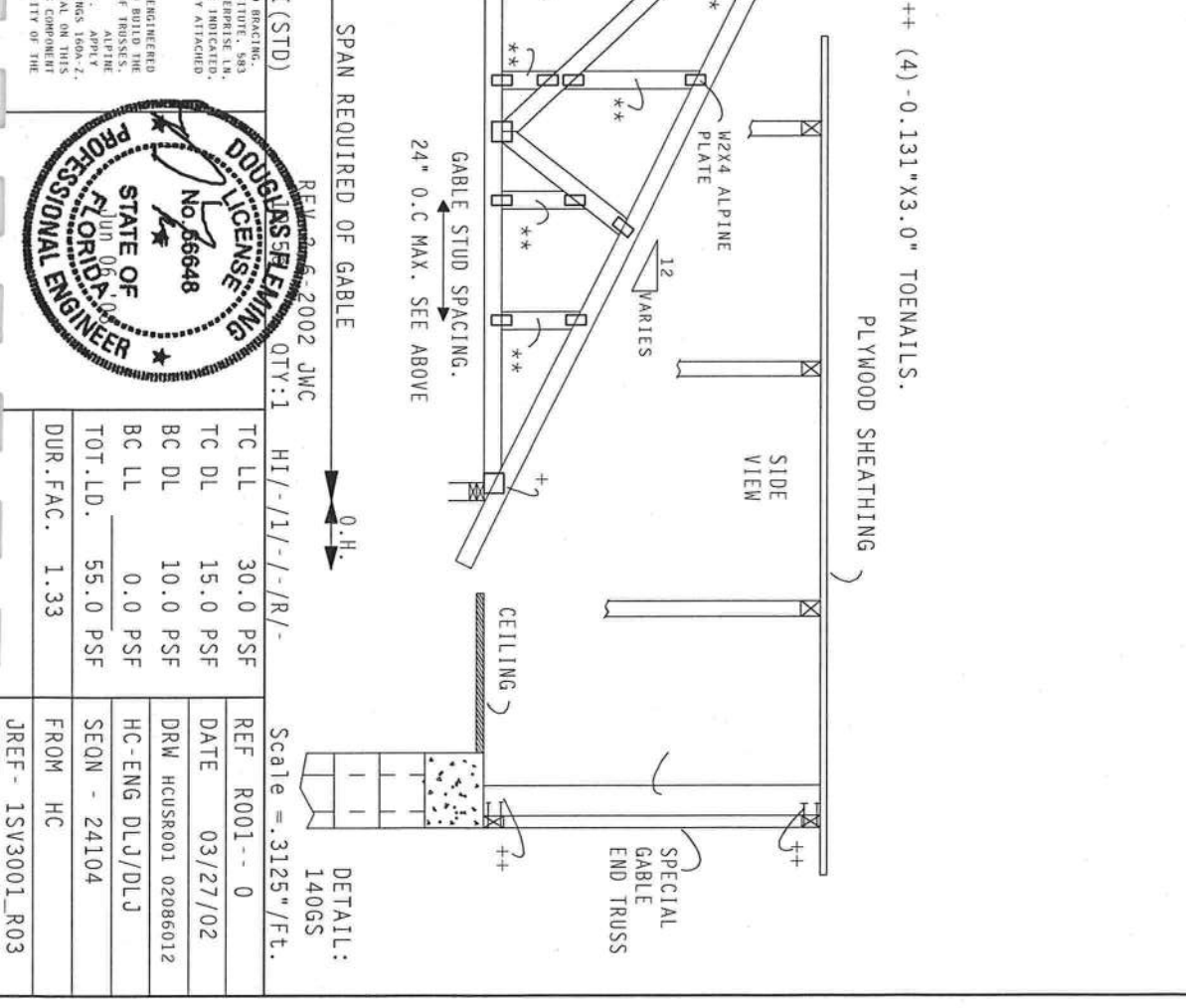
R2: REVISED FOR ASCE 7-02
DLJ 09/30/2005
R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/2006



140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, PART. ENCLOSED BLDG.
CAT II, EXP. C.
140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02, PART. ENCLOSED BLDG.
CAT II, EXP. C.
SEE APPROPRIATE ALPINE DRAWING FOR LUMBER, PLATES AND OTHER DATA NOT SHOWN HERE.

** STUD MUST BE ATTACHED TO CHORDS AND DIAGONAL REINFORCING MEMBER REQUIRED

2X4 SO. PINE #3	SPACING	MAX. LENGTH
2X4 SO. PINE #3	24" O.C.	2'-10"
2X4 SO. PINE #3	16" O.C.	3'-10"
2X4 SO. PINE #3	12" O.C.	5'-0"
2X6 SO. PINE #2 N	24" O.C.	6'-2"
2X6 SO. PINE #2 N	16" O.C.	7'-1"
2X6 SO. PINE #2 N	12" O.C.	7'-6"
2X8 SO. PINE #2 N	16" O.C.	9'-1"
2X8 SO. PINE #2 N	12" O.C.	10'-4"



Design Criteria: TPI(STD)

REV 2-6-2002 JMC
QTY: 1

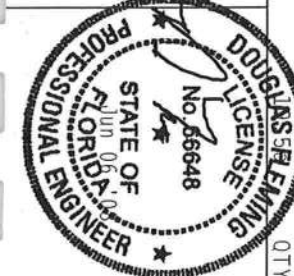
Scale = .3125"/ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certified Engineering Firm

****WARNING**** TRUSSES ROUTED EXTERIOR GABLE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 563 DORRIS RD., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN COMPLIANCE WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AIA/PS) AND TPI. ALPINE CONNECTION PLATES ARE MADE OF 20/10/160A (W/15/15) ASH 6063 GRADE 40/60 (W/ 6/18/53) GALV. STEEL. ANY INSPECTION OF PLATES FOLLOWED BY REPAIRS SHALL BE PERFORMED AS OF THE ORIGINAL DESIGN. SET OUT THE DRAWING, BRACING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	30.0 PSF	REF R001 - 0
TC DL	15.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCURS001 02086012
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	55.0 PSF	SEQN - 24104
DUR.FAC.	1.33	FROM HC
JREF - 1SV3001_R03		

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCOL=5.0 PSF, WIND BCDL=5.0 PSF.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCOL=5.0 PSF, WIND BCDL=5.0 PSF.

+ FOR VERTICAL WEBS LESS THAN 4'0": MIX4 FOR VERTICAL WEBS GREATER THAN 4'0" BUT NO MORE THAN 11'6": W2X4.

* SPLICE, PEAK, AND HEEL PLATES TO MATCH COMMON TRUSS.

** 2X4 OR GREATER CHORDS.

DROP GABLE WILL SUPPORT 4'0" OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) SPACED 24" O.C., OR THE LOAD FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN, CEILING AND ROOF SHEATHING DIAPHRAGM CONNECTIONS, AND ALL TRUSS TO WALL CONNECTIONS.

++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R1 NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

R1 NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

ALT. GABLE SHAPES:



Note: All Plates Are 2X4 Except As Shown.

PLT TYP. Wave TPI-95

Design Crit: TPI-1995(STD)

R3: REVISED DIAPHRAGM NOTE. DLJ 02/27/2006

R2: REVISED FOR ASCE 7-02. DLJ 09/30/2005

RI REV 2-5-02 JMC

DETAIL: 140GC Scale = .375"/ft.

BRACING DEFINITIONS:
NOTE: "END ZONE" EXISTS 18" AT BOTH ENDS OF VERTICAL WEB.

- (A) (1) 2X4 SP #3 "L" BRACE. ATTACH WITH 0.128"x3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.
- (B) (2) 2X4 SP #3 "L" BRACES. ATTACH EACH WITH 0.128"x3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.
- (C) (1) 2X6 SP #2 N "L" BRACE. ATTACH WITH 0.128"x3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.
- (D) (2) 2X6 SP #2 N "L" BRACES. ATTACH EACH WITH 0.128"x3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

STUD SPACING / BRACING TABLE:

2X4 SP #3 STUD SPACING	DEFLECTION CRITERIA	NO BRACE	(1) 2X4 "L" BRACE TYPE (A)	(2) 2X4 "L" BRACE TYPE (B)	(1) 2X6 "L" BRACE TYPE (C)	(2) 2X6 "L" BRACE TYPE (D)
24"	L/360	-----	3' 1"	4' 2"	6' 3"	8' 0"
24"	L/180	-----	3' 4"	5' 7"	6' 3"	11' 0"
16"	L/360	-----	3' 11"	5' 3"	7' 10"	9' 11"
16"	L/180	-----	4' 9"	7' 4"	9' 6"	11' 0"
12"	L/360	-----	4' 7"	6' 1"	8' 11"	11' 0"
12"	L/180	-----	5' 11"	8' 5"	11' 0"	11' 0"

OVERHANG DETAIL

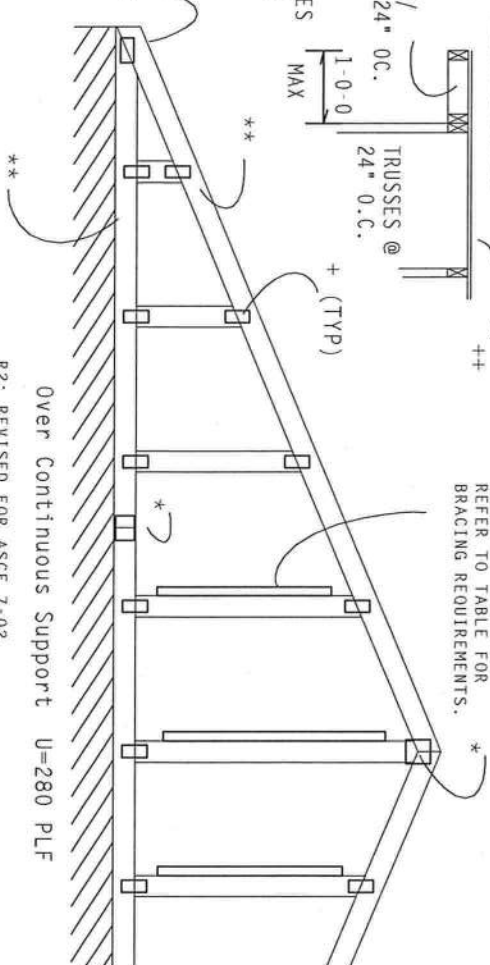
REFER TO TABLE FOR BRACING REQUIREMENTS.

LADDER W/ STEPS @ 24" OC.

TRUSSES @ 24" O.C.

(TYP)

INCLUDES FASCIA



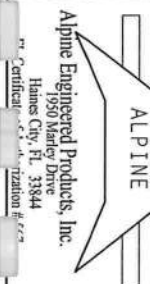
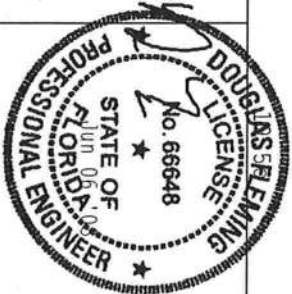
Over Continuous Support U=280 PLF

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DETAIL 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 180 D-ORFORD DR., SUITE 200, MADISON, WI 53719 AND AISC (6000) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH THE DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN OR CONSTRUCTION OF THIS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSSES.

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSSES. DESIGN OR CONSTRUCTION OF THIS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSSES.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002 SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	30.0 PSF	REF	R001-- 0
TC DL	7.0 PSF	DATE	03/27/02
BC DL	10.0 PSF	DRW	HCUSR001 02086015
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT. LD.	47.0 PSF	SECON-	24860
DUR. FAC.	1.33		
SPACING	24.0"	JREF	1SV3001_R03

Return To: Sierra Title, LLC
619 SW Baya Dr., Ste 102
Lake City, FL 32025

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS

By Shera Feagle
Deputy Clerk

Date 04-24-2008 Inst: 200812008024 Date: 4/24/2008 Time: 11:45 AM

DC, P. DeWitt Cason, Columbia County Page 1 of 2



PERMIT NO. 26936

#08-0151

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

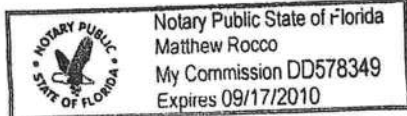
The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: See Exhibit A
2. General description of improvement: Construction of Dwelling
3. Owner information:
 - a. Name and address: George R. Baker & Kristy S. Baker
200 SW Soundless Ct., Lake City, FL 32024
 - b. Interest in property: Fee Simple
 - c. Name and address of fee simple title holder (if other than Owner): NONE
4. Contractor (name and address): EDGLEY CONSTRUCTION CO., a d/b of CEE-BAS, Inc.
5. Surety:
 - a. Name and address: N/A
 - b. Amount of bond: _____
6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
4705 WEST U.S. HIGHWAY 90
P. O. BOX 2029
LAKE CITY, FLORIDA 32056
7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE
8. In addition to himself, Owner designates PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056 to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

George R. Baker
Borrower Name

Kristy S. Baker
Co-Borrower Name

The foregoing instrument was acknowledged before me this 22 day of APRIL, 2008, by George R. Baker & Kristy S. Baker, who is personally known to me or who has produced driver's license for identification.



Notary Public
My Commission Expires: _____

File No. 08-0151/Baker

Exhibit A

Legal Description

Commence at the SW corner of the NW1/4 of the NW 1/4 of Section 28, Township 4 South, Range 16 East, Columbia County, Florida and run N 00°27'17"E, 323.05 feet to the Point of Beginning; thence continue N 00°27'17" E, 324.19 feet; thence N 89°28'27" E, 672.28 feet; thence S 00°04'03" E, 324.15 feet S 89°28'27" W, 675.23 feet to the Point of Beginning.

Subject to a non-exclusive perpetual easement for ingress and egress over and across the west 60 feet thereof.

Together with an easement for ingress and egress purposes, being 60 feet East of and adjacent to the following described line: Commence at the SW corner of the NW 1/4 of the NW 1/4 of Section 28, Township 4 South, Range 16 East, Columbia County, Florida and run N 00°27'17" E, along the West line of said Section 28 a distance of 323.05 feet to the Point of Beginning; thence continue N 00°27'17" E, along said West line of Section 28, a distance of 931.74 feet to a point on the South right-of-way line of County Road 242, said point also being the terminal point of herein described line and easement.

COLUMBIA COUNTY OFFICE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-4S-16-03234-002

Building permit No. 000026936

Use Classification SFD, UTILITY

Fire: 70.62

Permit Holder EDGLEY CONSTRUCTION

Waste: 184.25

Owner of Building GEORGE & KRISTY BAKER

Total: 254.87

Location: 235 SW EVA TERR., LAKE CITY, FL

Date: 11/20/2008

Tanya Dicker

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



FEEs:

ROAD IMPACT FEE 1046.00 CODE 210 UNIT 1
10100003632400

EMS IMPACT FEE 29.88
10300003632210

FIRE PROTECTION IMPACT FEE 98.63
10200003632220

CORRECTIONS IMPACT FEE 409.16
00100003632200

SCHOOL IMPACT FEE 1500.00
00100003632900

TOTAL FEES CHARGED 3063.67 CHECK NUMBER _____