ITW Building Components Group, Inc.

Permit # 26936

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TI58228Z0206134440

Truss Fabricator:	Anderson Truss Company
Job Identification:	8-140--OWNER BUILDER Baker -- , **
Truss Count:	30
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.24, 7.36, 7.37.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 06/06/2008

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

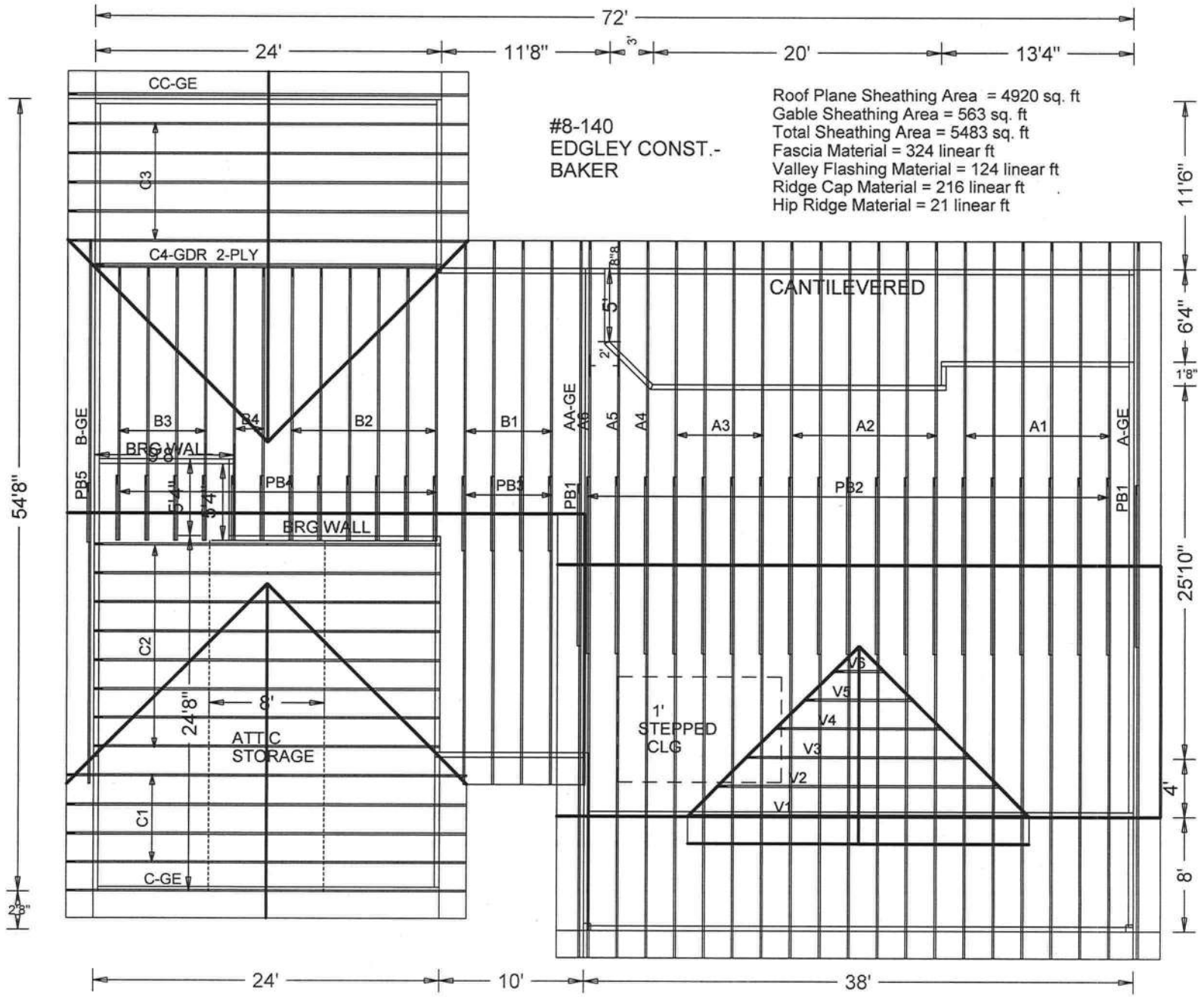
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-TCFILLER-BCFILLER-140GS-VALTRUSS-140GC-160TL-A11015EE-GBLLETIN-PIGBACKB-

#	Ref Description	Drawing#	Date
1	09152--A2	08158008	06/06/08
2	09153--A3	08158022	06/06/08
3	09154--A4	08158023	06/06/08
4	09155--A5	08158024	06/06/08
5	09156--A6	08158009	06/06/08
6	09157--AA-GE	08158010	06/06/08
7	09158--A1	08158025	06/06/08
8	09159--A-GE	08158026	06/06/08
9	09160--V1	08158011	06/06/08
10	09161--V2	08158003	06/06/08
11	09162--V3	08158004	06/06/08
12	09163--V4	08158005	06/06/08
13	09164--V5	08158006	06/06/08
14	09165--V6	08158007	06/06/08
15	09166--B-GE	08158027	06/06/08
16	09167--B3	08158012	06/06/08
17	09168--B1	08158013	06/06/08
18	09169--B2	08158014	06/06/08
19	09170--B4	08158032	06/06/08
20	09171--C4-GDR	08158015	06/06/08
21	09172--C1	08158016	06/06/08
22	09173--C2	08158017	06/06/08
23	09174--C3	08158018	06/06/08
24	09175--C-GE	08158028	06/06/08
25	09176--CC-GE	08158029	06/06/08
26	09177--PB1	08158019	06/06/08
27	09178--PB2	08158020	06/06/08
28	09179--PB3	08158030	06/06/08
29	09180--PB5	08158031	06/06/08
30	09181--PB4	08158021	06/06/08

Repair Charge: $13.75 per Customer Agreement.
Amount to be invoiced separately.



JOB DESCRIPTION:: OWNER BUILDER
/: Baker

JOB NO:
8-140

PAGE NO:
1 OF 1

(8-140--OWNER BUILDER Baker -- , ** - A2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

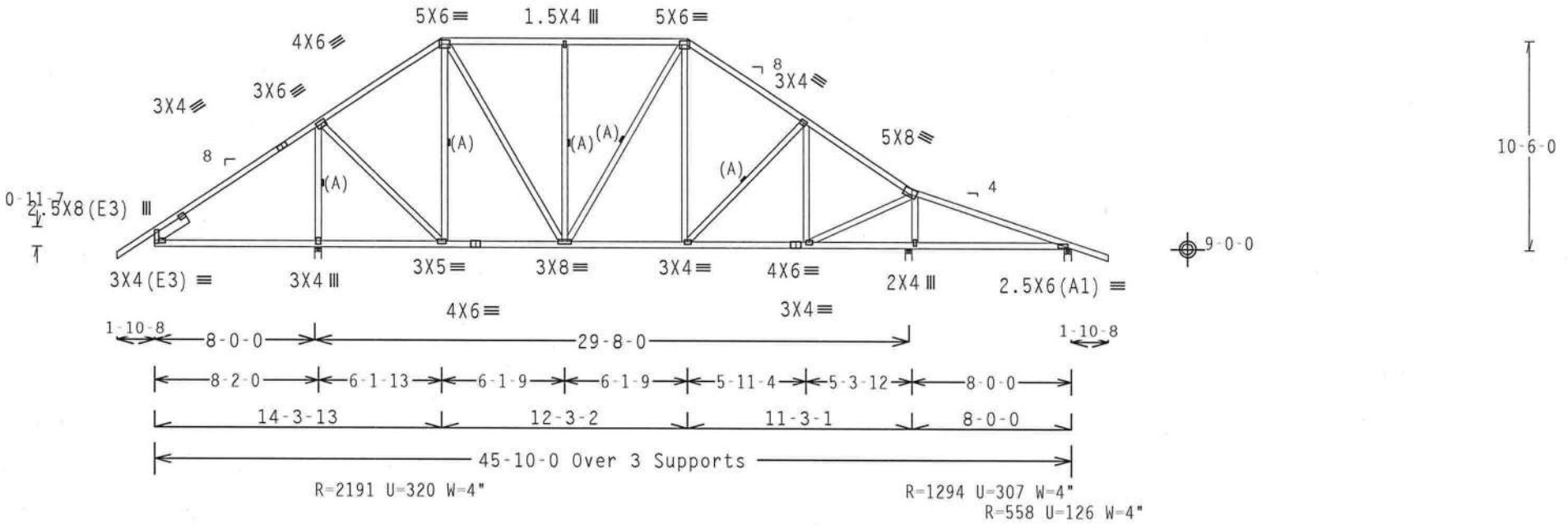
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:6 FL/-/4/-/-/R/- Scale =.125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9152
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158008
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170256
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - A3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C. Including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

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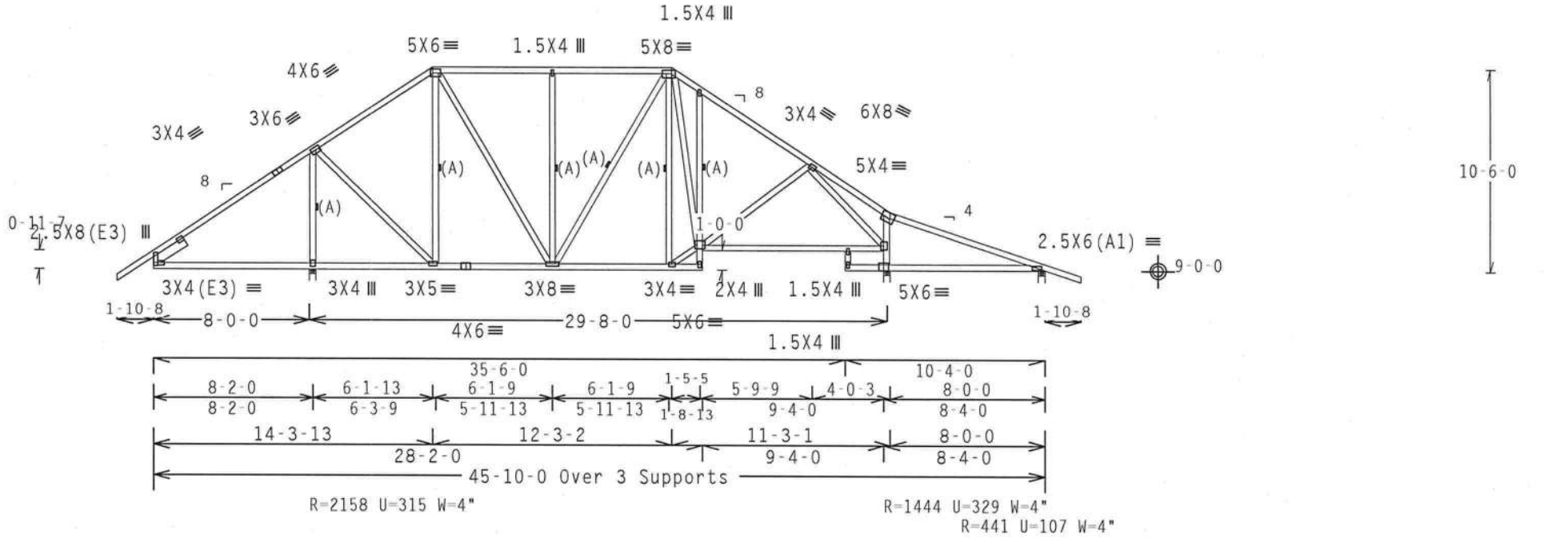

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:4 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9153
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158022
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170279
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - A4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C. Including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

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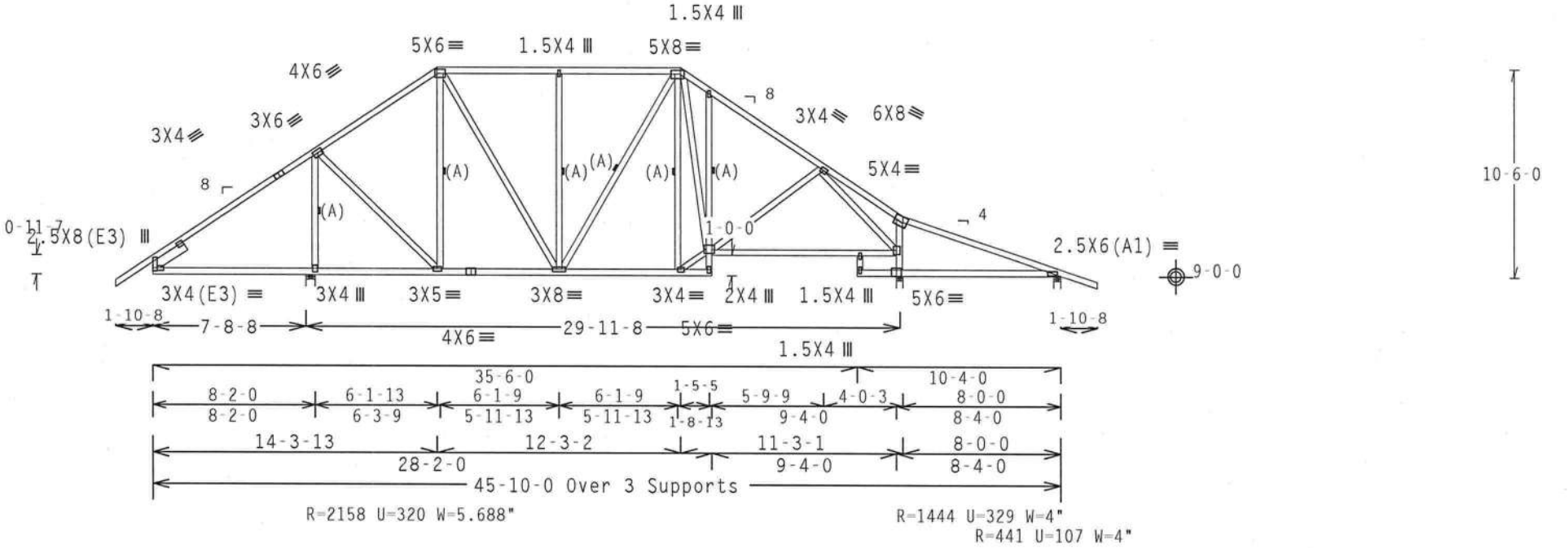

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9154
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158023
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170268
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - A5)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C.
Including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

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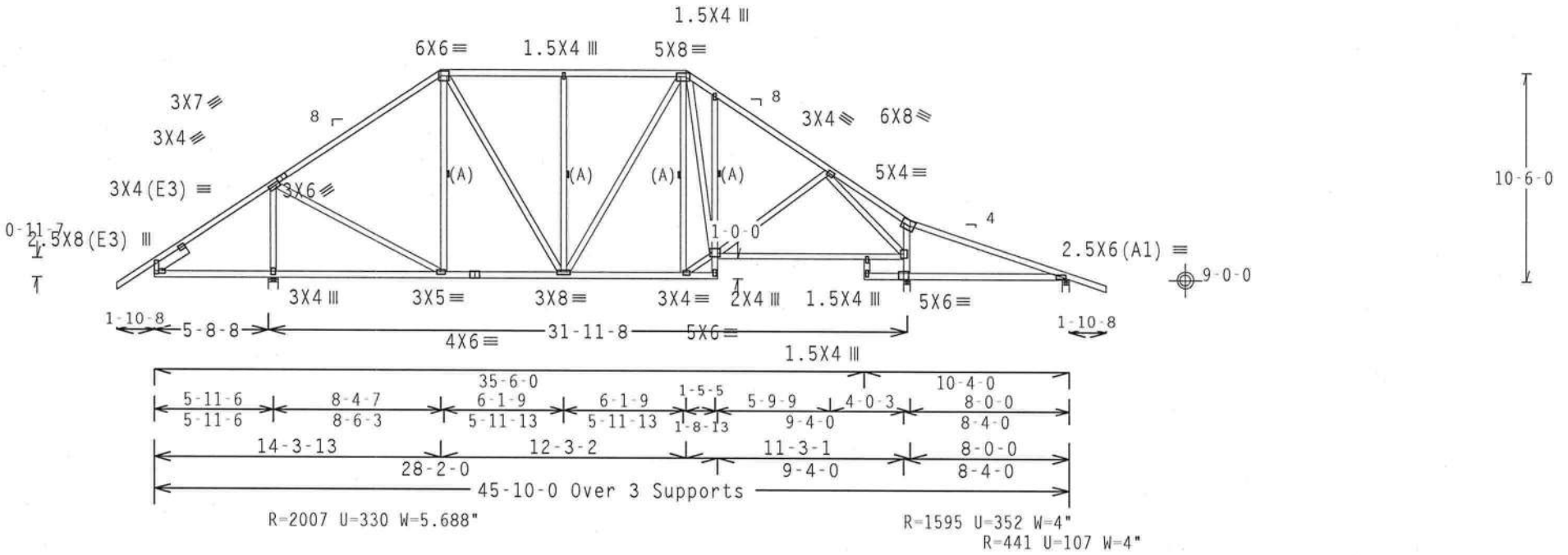

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9155
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158024
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170272
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

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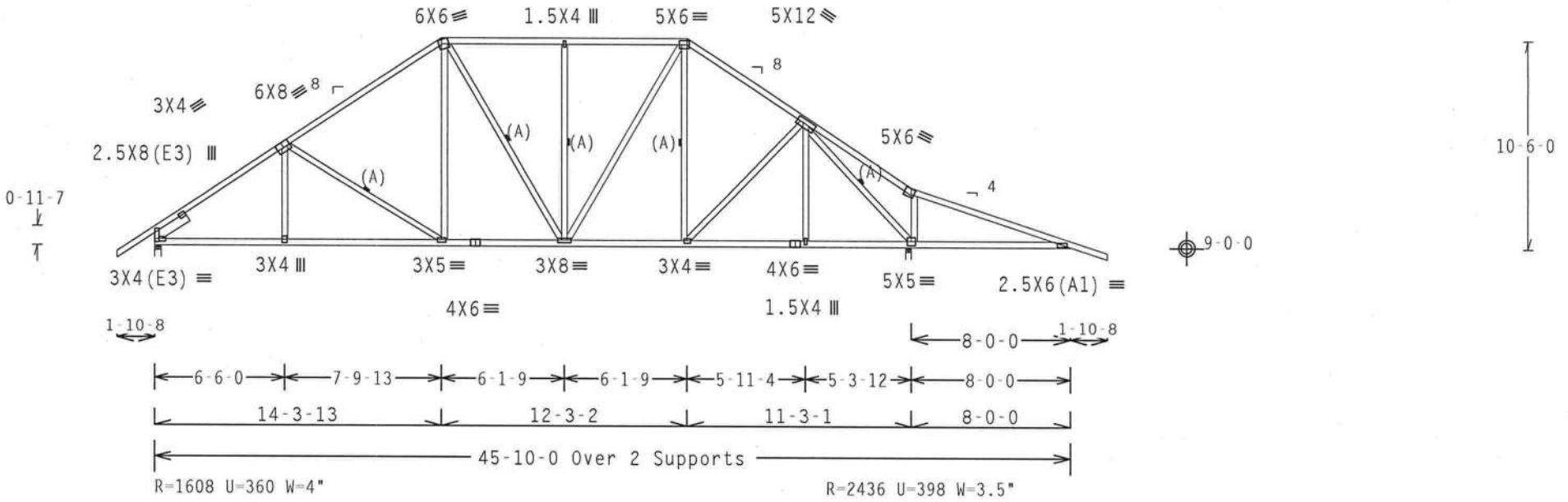
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/0(0) 7.36.042 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9156
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158009
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 33519
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - AA-GE)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2 :B3 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W11 2x4 SP #2 Dense:
:Stack Chord T2 2x4 SP #2 Dense:
:Stack Chord T7 2x4 SP #2 Dense:
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

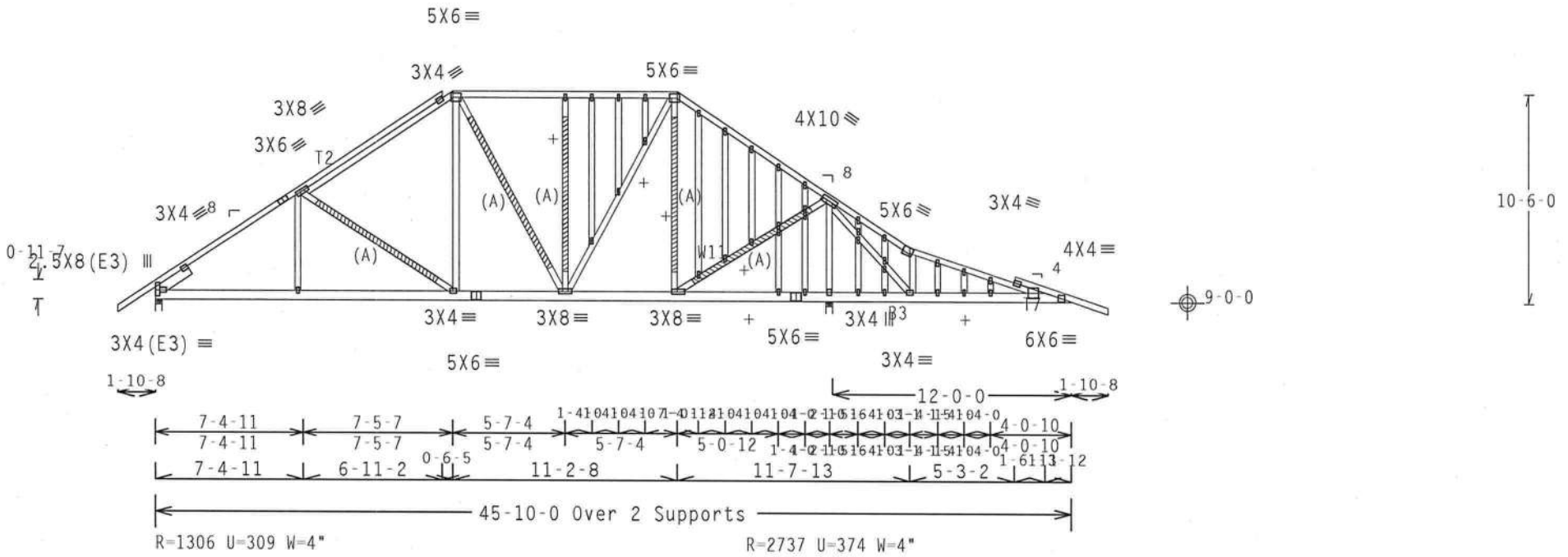
Wind reactions based on MWFRS pressures.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9157
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158010
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170275
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - A1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

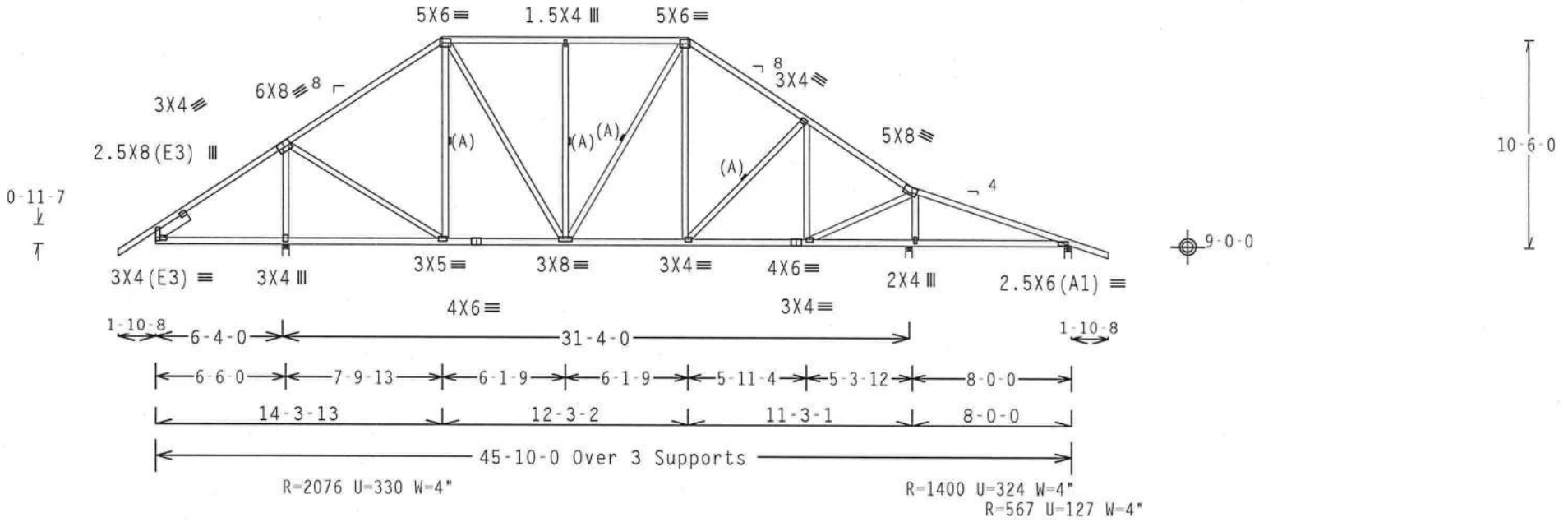
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12[illegible] QTY:6 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
[illegible] 06 '08

TC LL	20.0 PSF	REF R8228- 9158
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158025
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 170250
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - A-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W44 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

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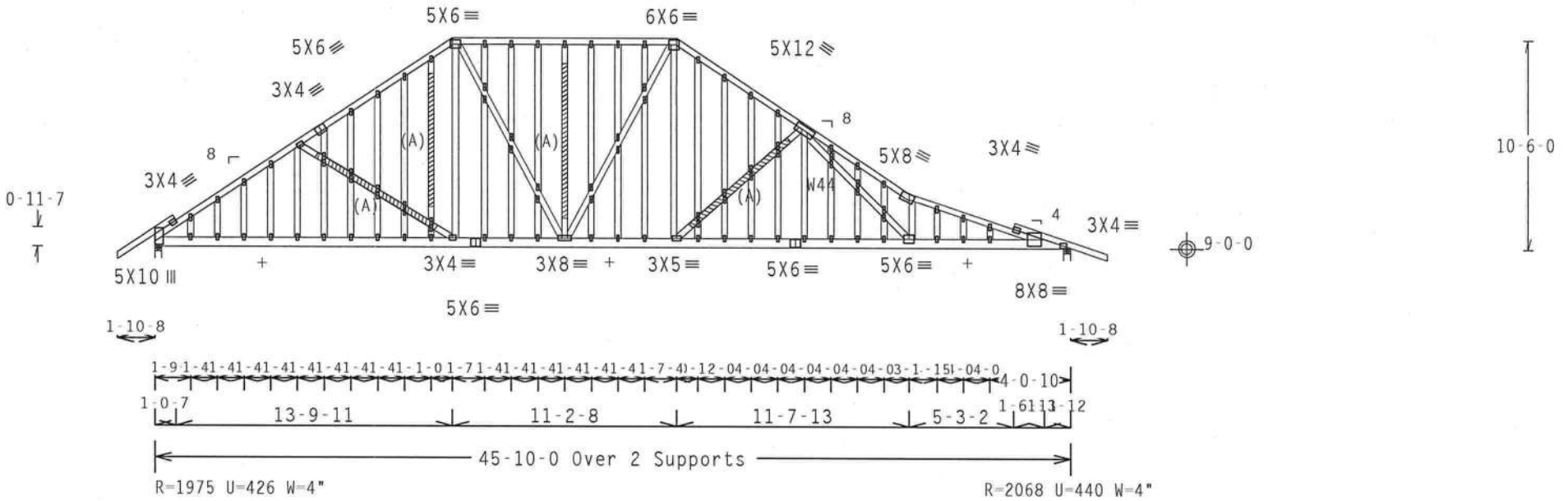

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9159
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	170182
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

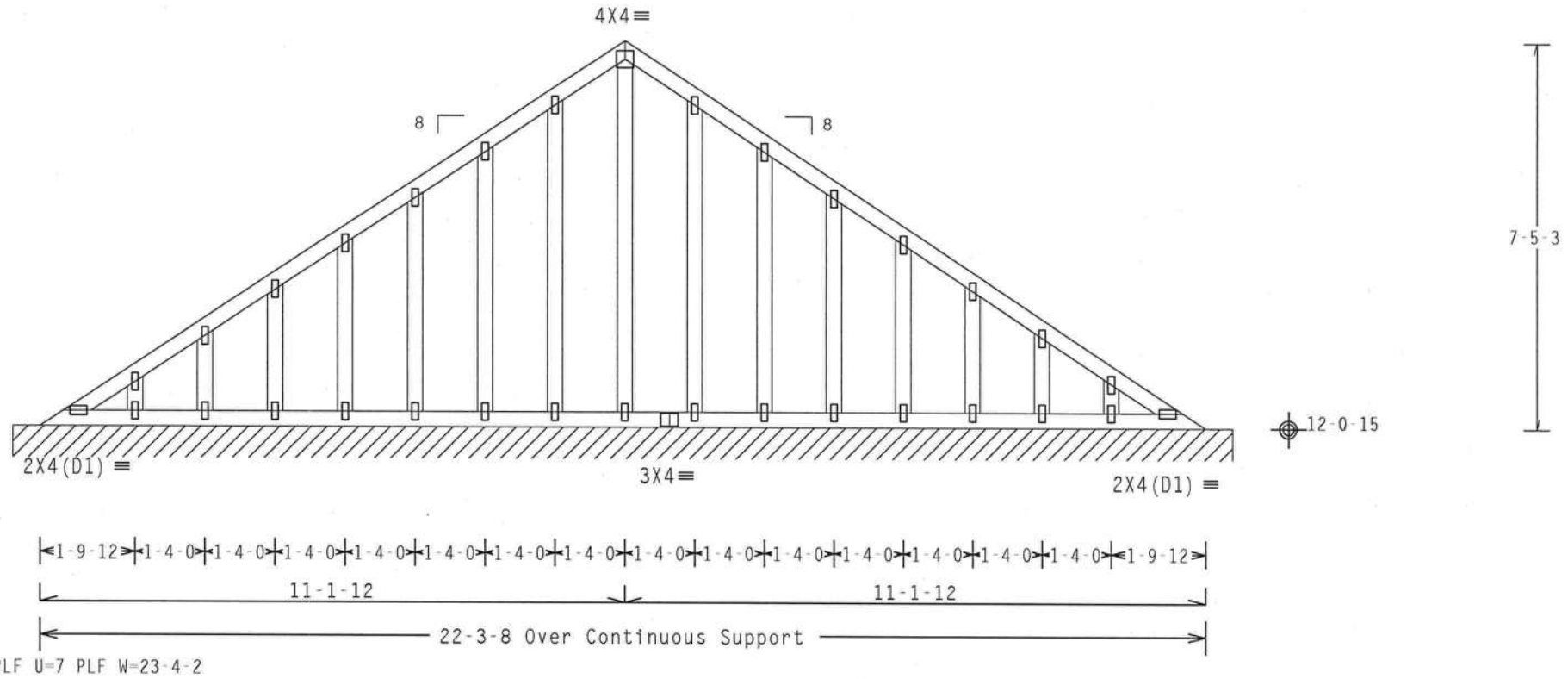
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

110 mph wind, 15.94 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSITITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9160
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	46877
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V2)

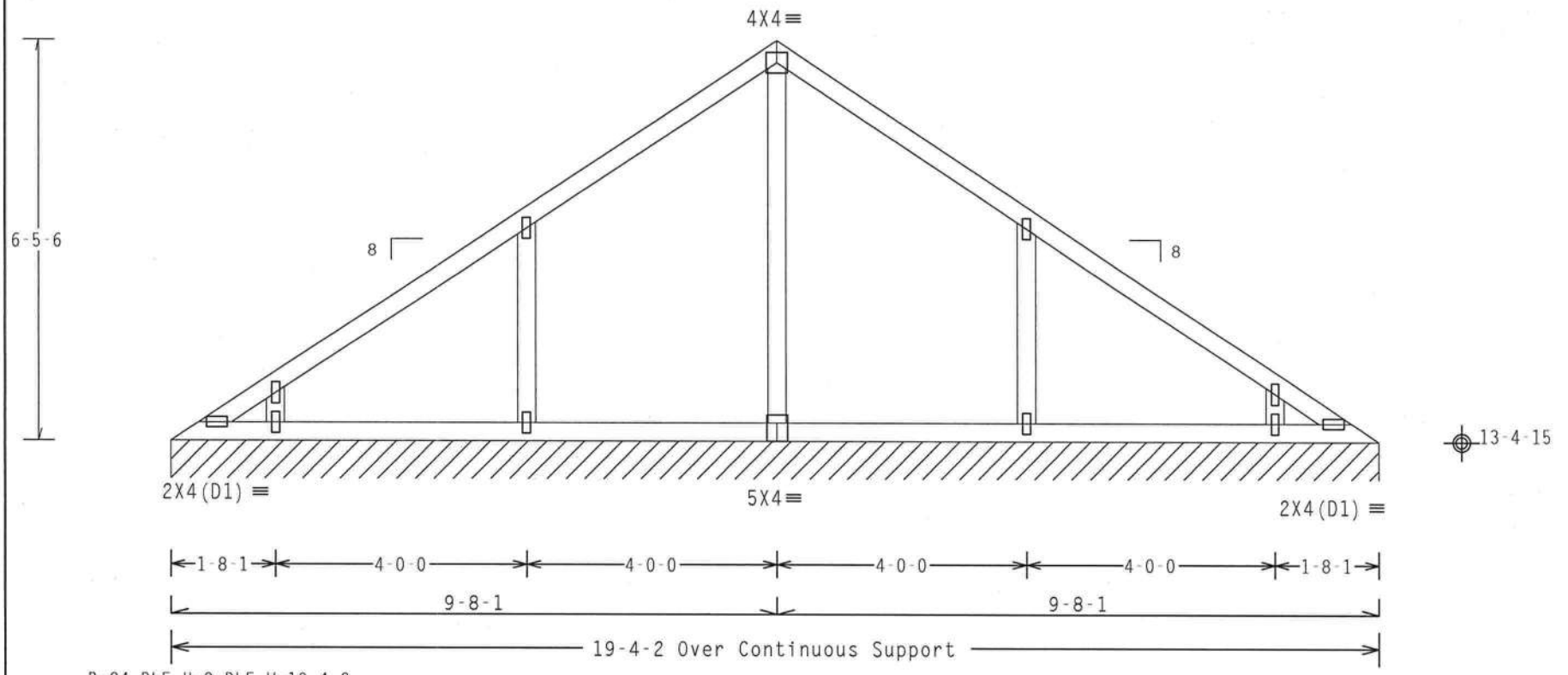
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.78 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUN 06 '08

TC LL	20.0 PSF	REF R8228- 9161
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158003
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 46870
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V3)

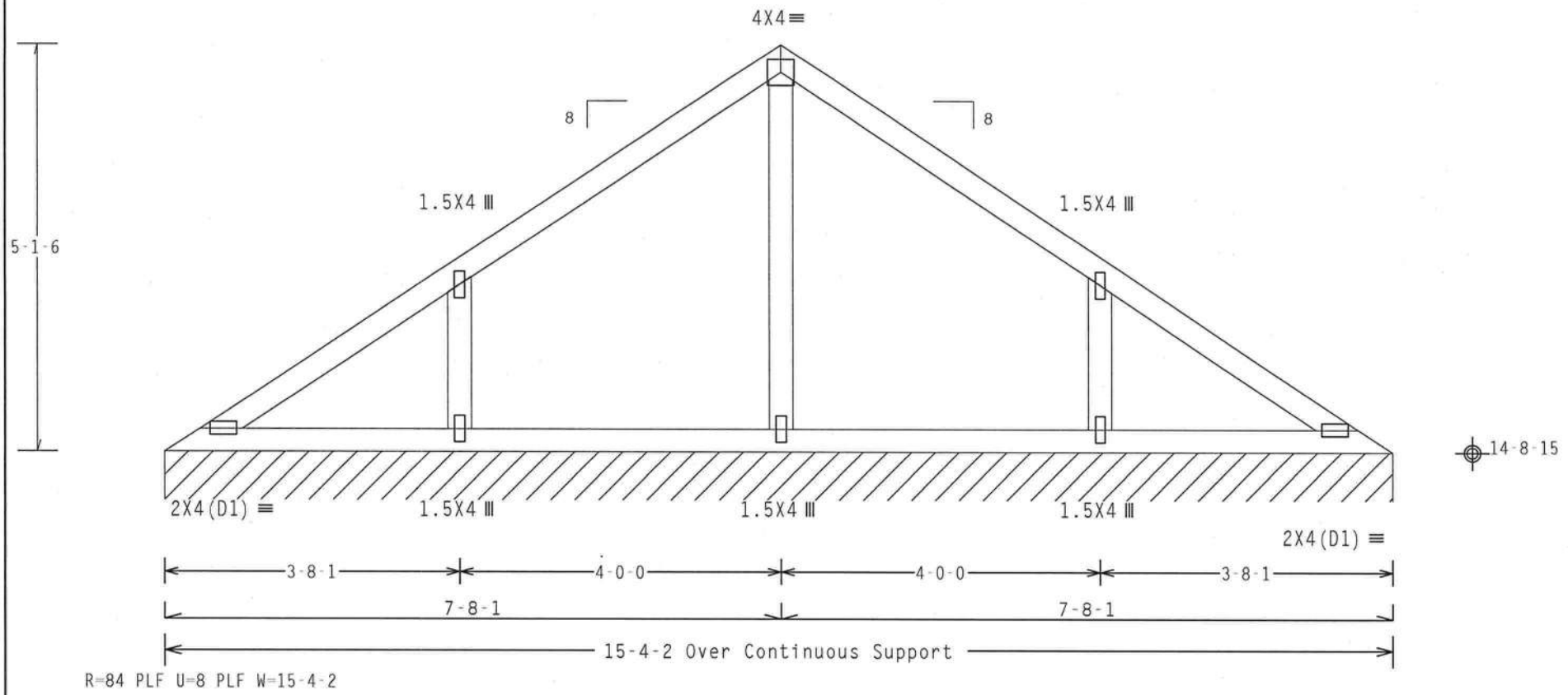
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 17.45 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9162
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158004
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 46864
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V4)

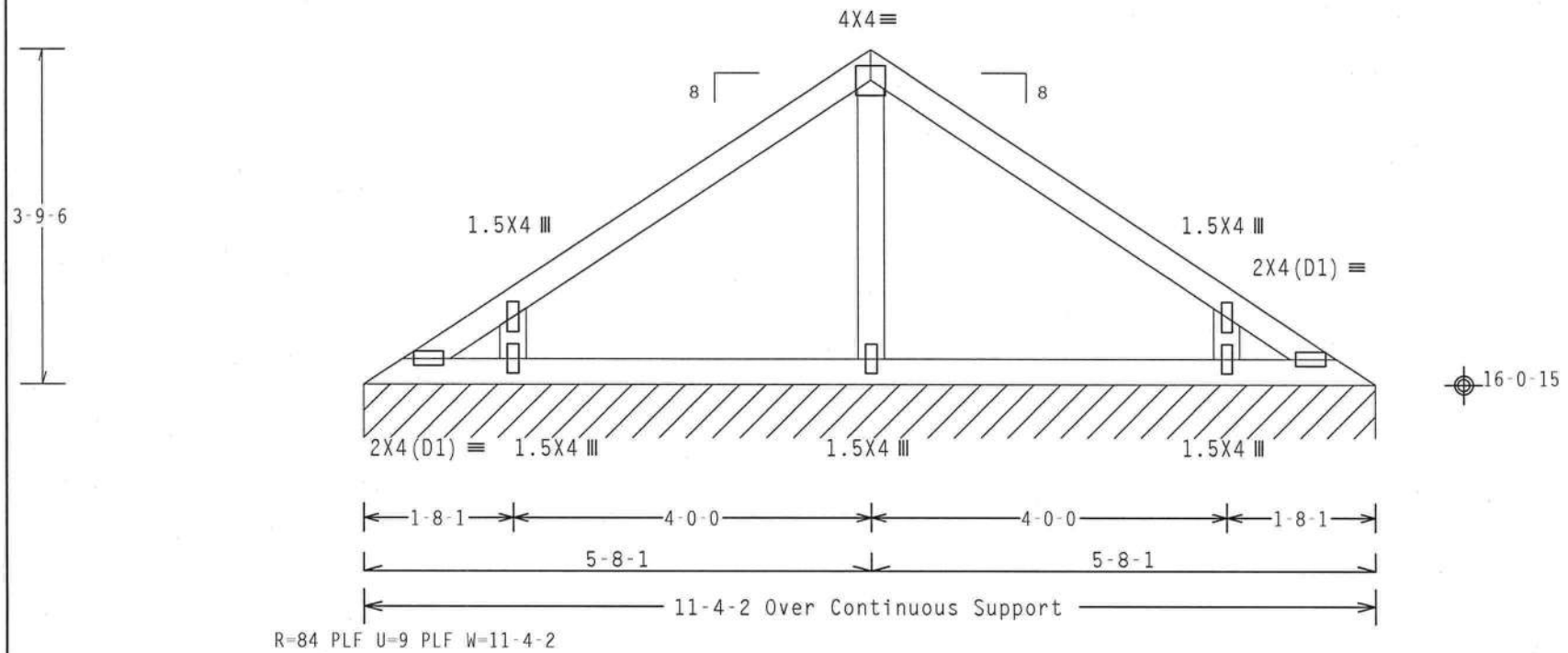
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.11 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9163
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158005
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	46860
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V5)

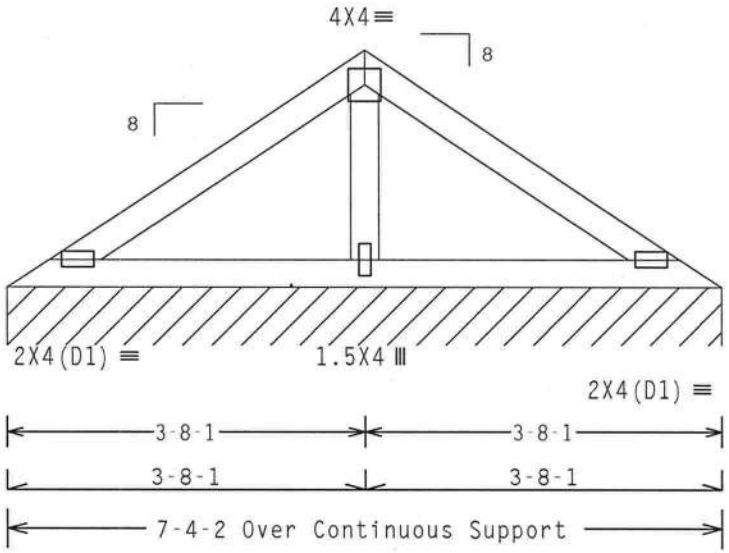
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 18.78 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



17-4-15

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (i) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

06 '08

TC LL	20.0 PSF	REF	R8228- 9164
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158006
BC LL	0.0 PSF		HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN-	46856
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - V6)

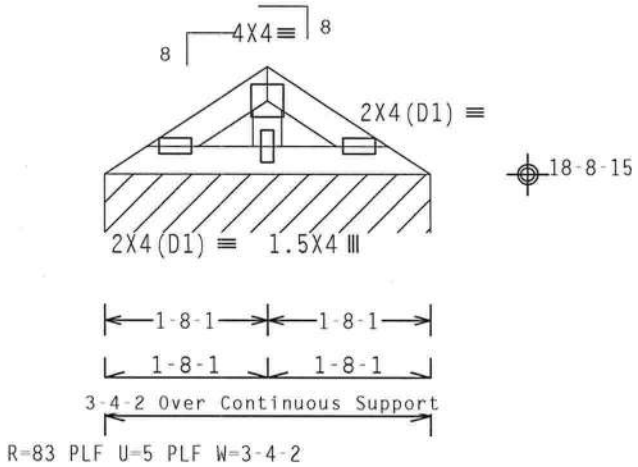
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.45 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



1-1-6

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9165
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158007
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	46852
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - B-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

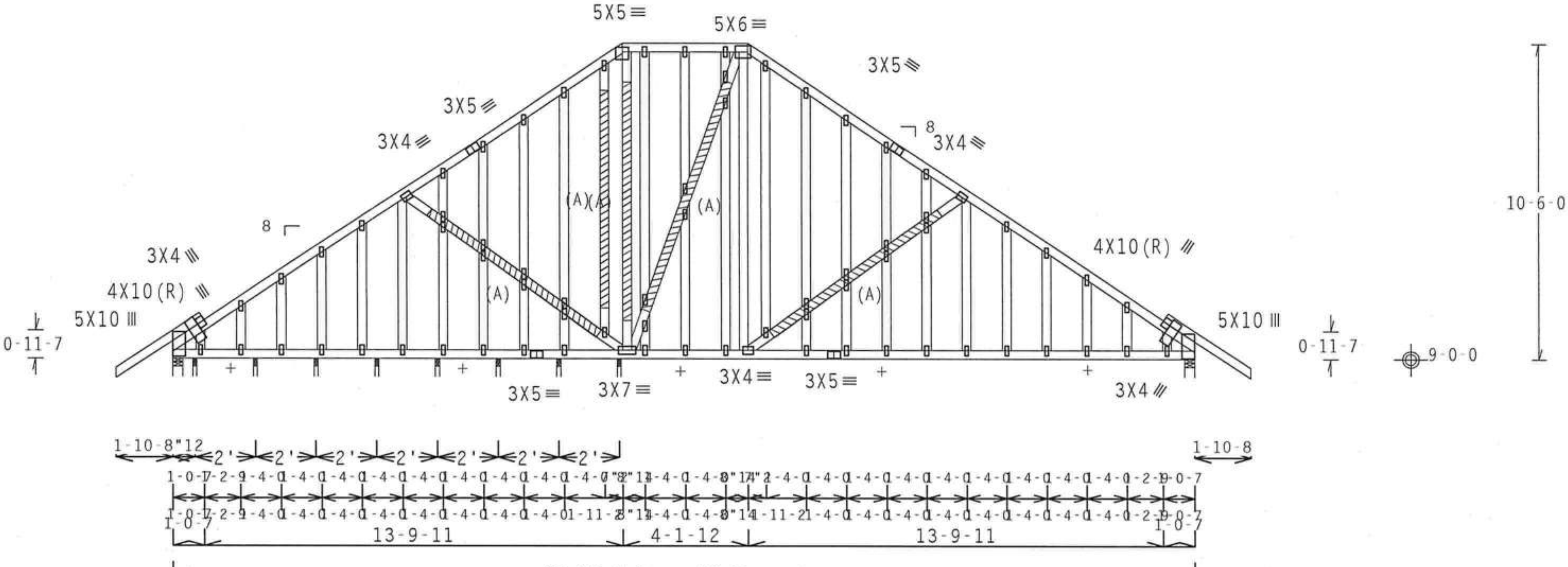
GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



R=226 U=48 W=4" R=152 U=26 W=1.5" R=209 U=4 W=1.5" R=144 U=36 W=1.5" R=904 U=13 W=1.5" R=935 U=93 W=4"
R=38 Rw=56 U=60 W=1.5" R=172 U=39 W=1.5" R=221 U=12 W=1.5" R=100 U=30 W=1.5"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.1230

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9166
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158027
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	46919
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - B3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

(A) Continuous lateral bracing equally spaced on member.

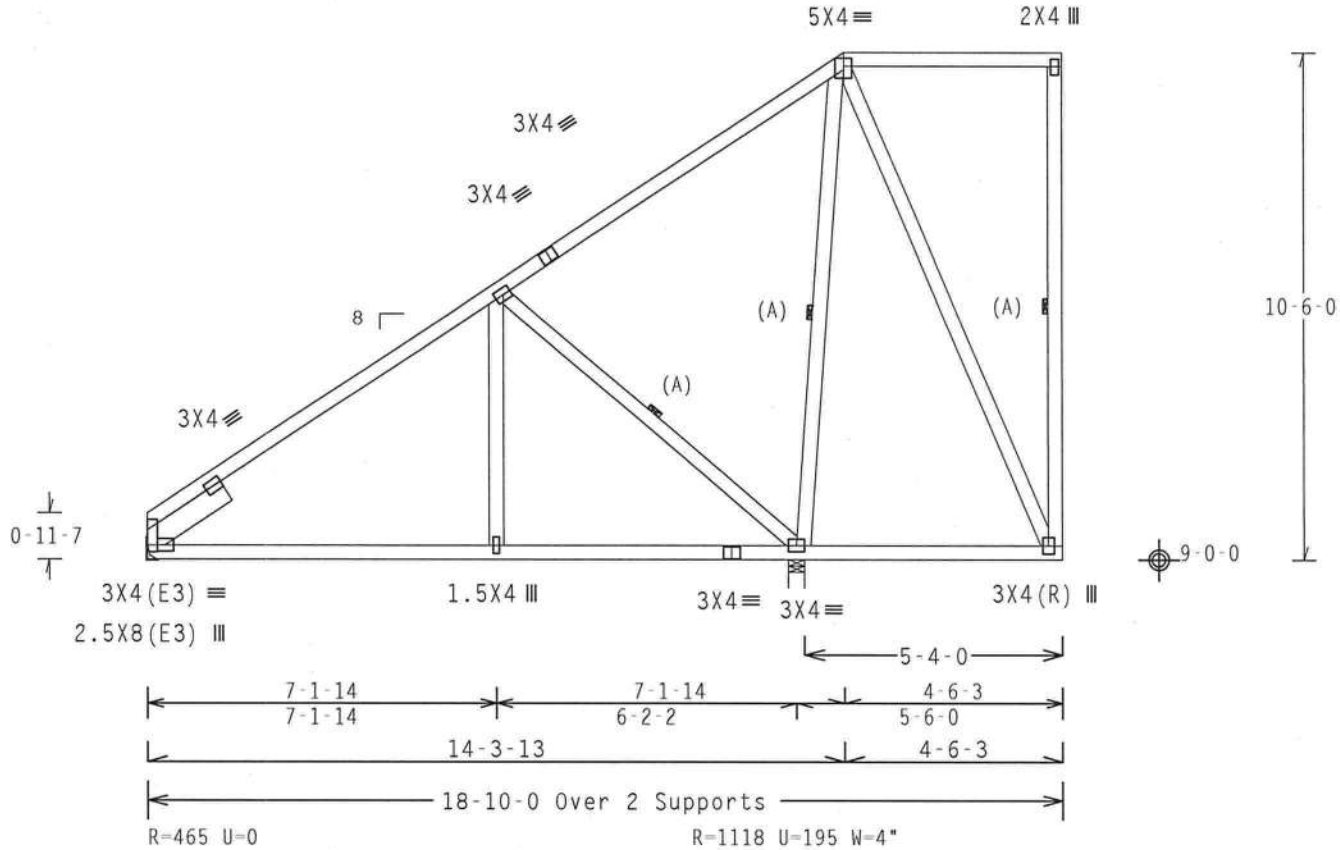
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.12[illegible] QTY:4 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

[illegible] 06 '08

TC LL	20.0 PSF	REF	R8228- 9167
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158012
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	170290
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Roof overhang supports 2.00 psf soffit load.

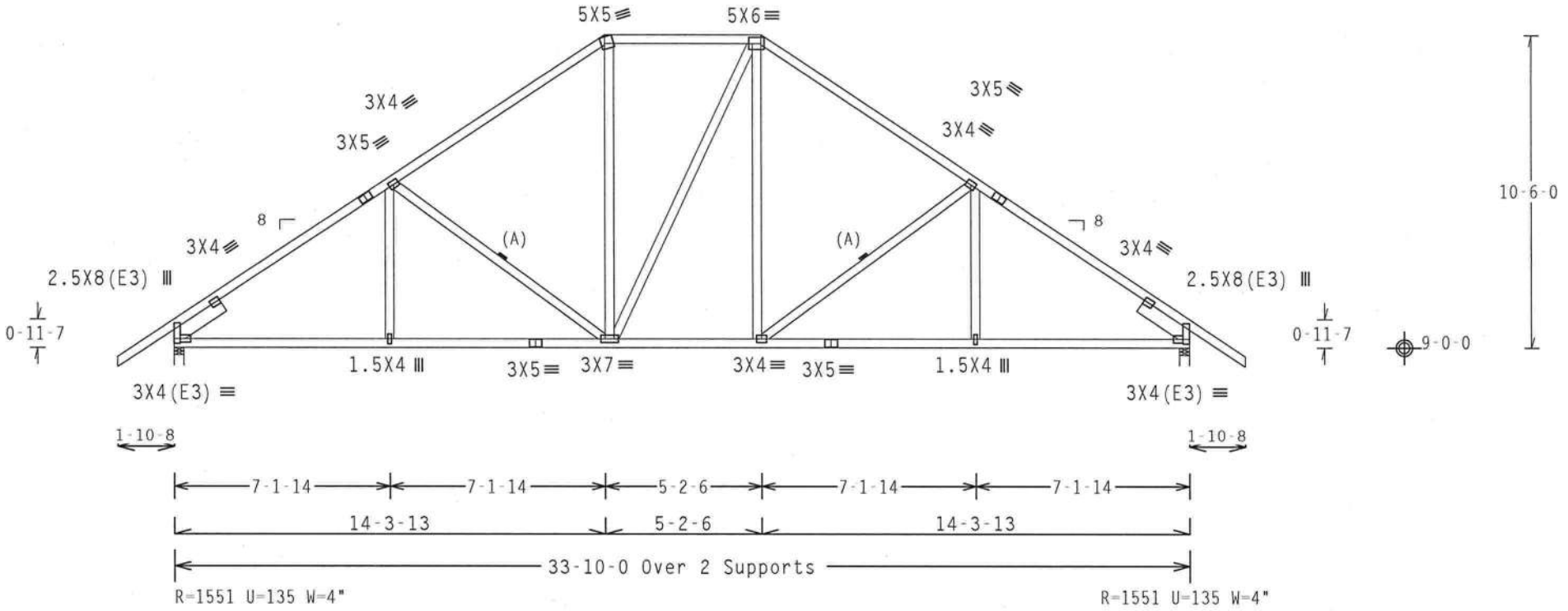
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:4 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9168
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158013
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	170283
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - B2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

(A) Continuous lateral bracing equally spaced on member.

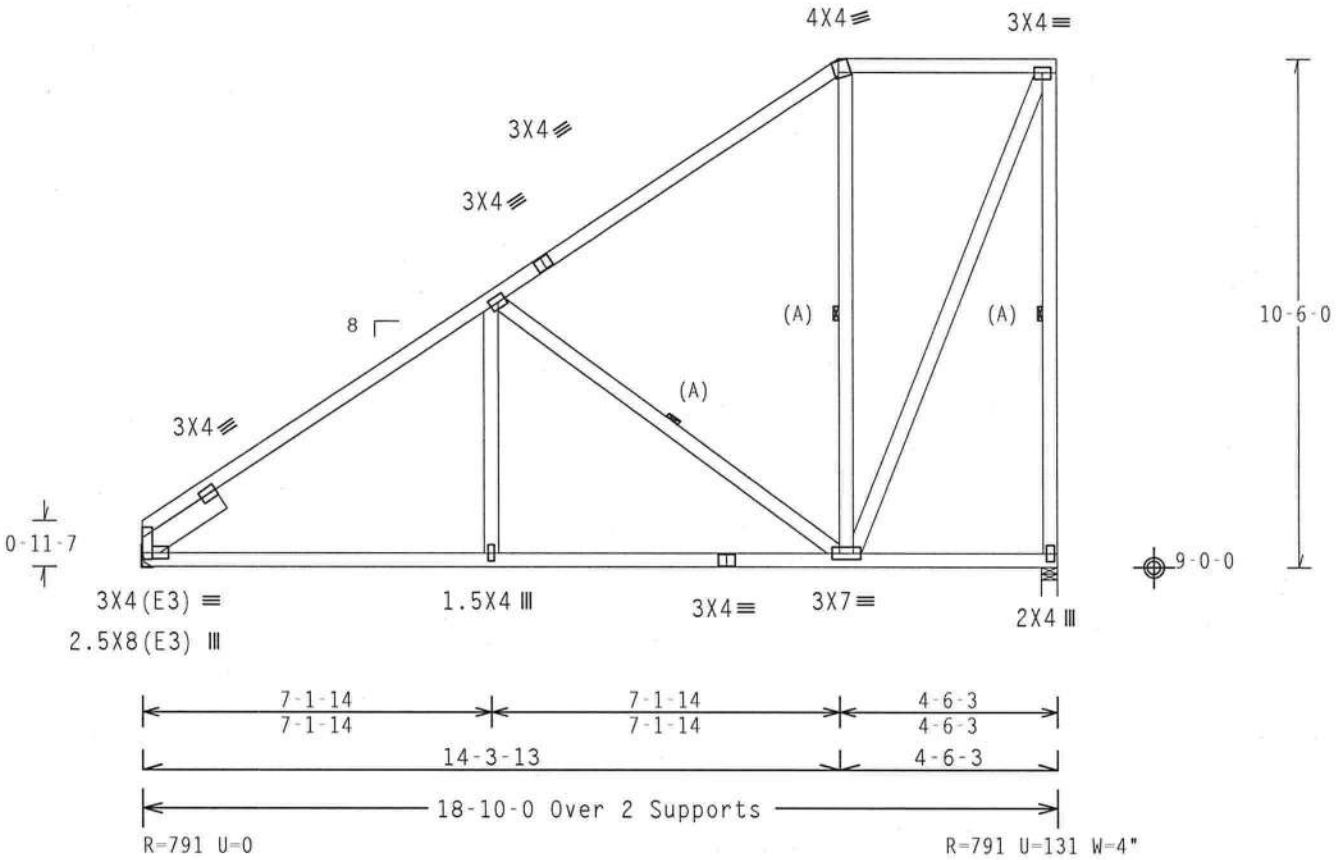
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:8 FL/-/4/-/-/R/- Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9169
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158014
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	170287
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - B4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
+Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.974'

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

This truss is repaired to plate in two new webs (+) as shown.

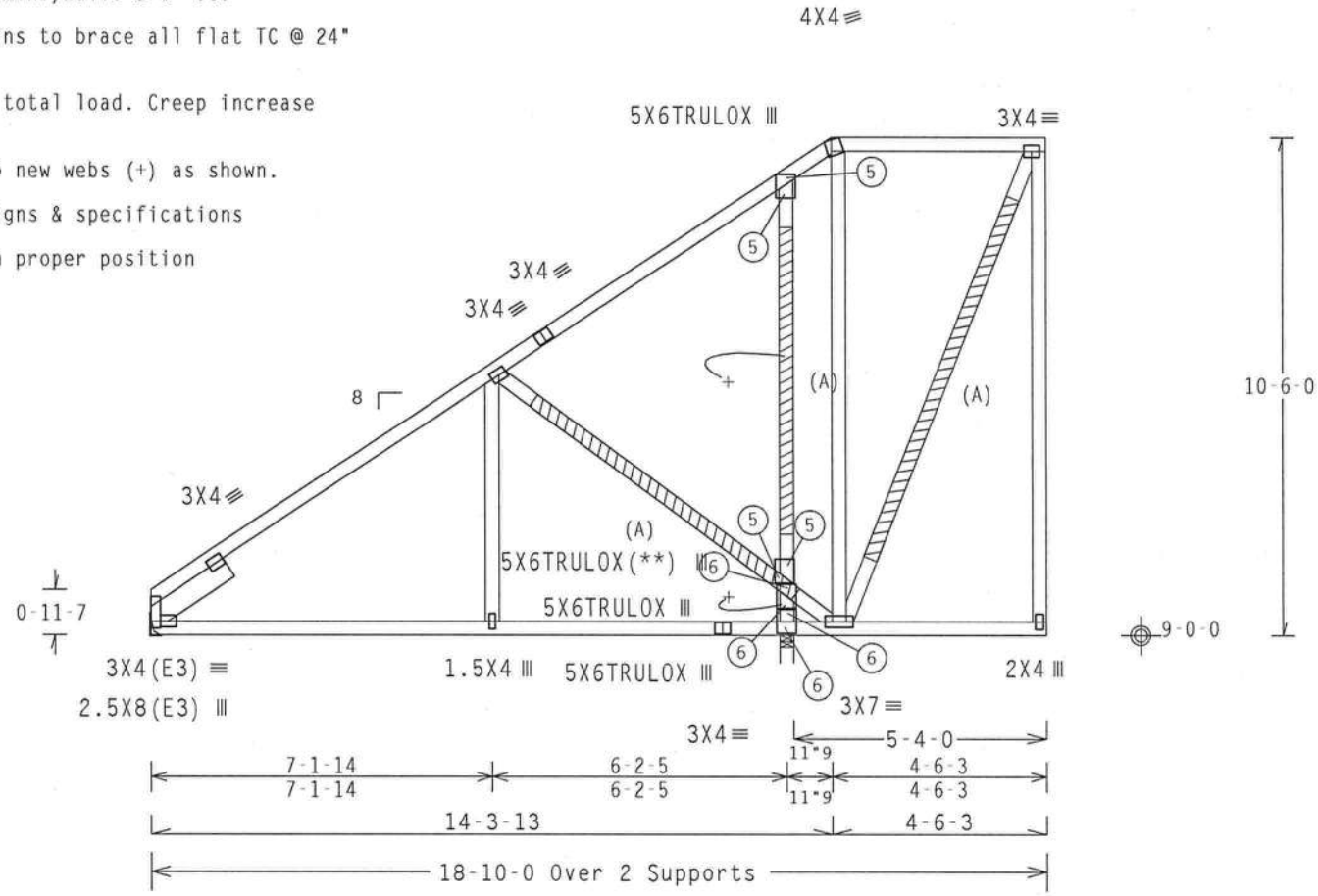
Repair(s) must comply with Alpine designs & specifications

Shore Truss and any supported spans in proper position as repair is being made.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 160TL for nailing and trulox plate requirements.



R=467 U=0

R=1116 U=195 W=3.5"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

PLT TYP. Wave,Trulox

7.36.043 [illegible] QTY:2 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED. THEREFORE, IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD.

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE, IF ANY, AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 9170
DATE	06/06/08
DRW	HCUSR8228 08158032
	HC-ENG DF/DF
SEQN-	33617
FROM	AH
JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - C4-GDR)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at 0.00 to 64 PLF at 12.00
TC - From 64 PLF at 12.00 to 64 PLF at 24.00
BC - From 20 PLF at 0.00 to 20 PLF at 24.00
BC - 465 LB Conc. Load at 1.69, 3.69, 5.69, 7.69
BC - 791 LB Conc. Load at 9.69, 11.69, 13.69, 15.69, 17.69
19.69, 21.69, 23.69

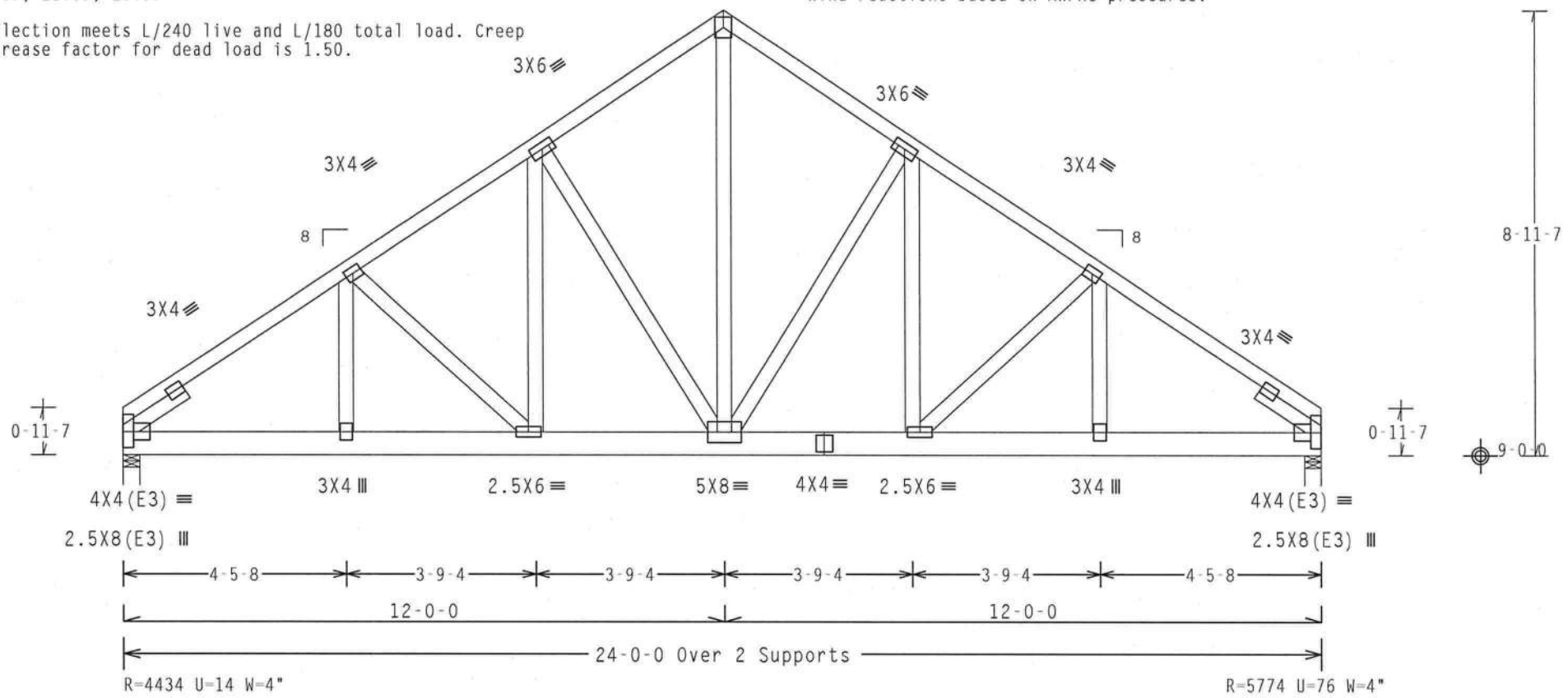
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 5.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9171
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158015
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	46976
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - C1)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
:B3 2x4 SP #2 Dense:
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

Roof overhang supports 2.00 psf soffit load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

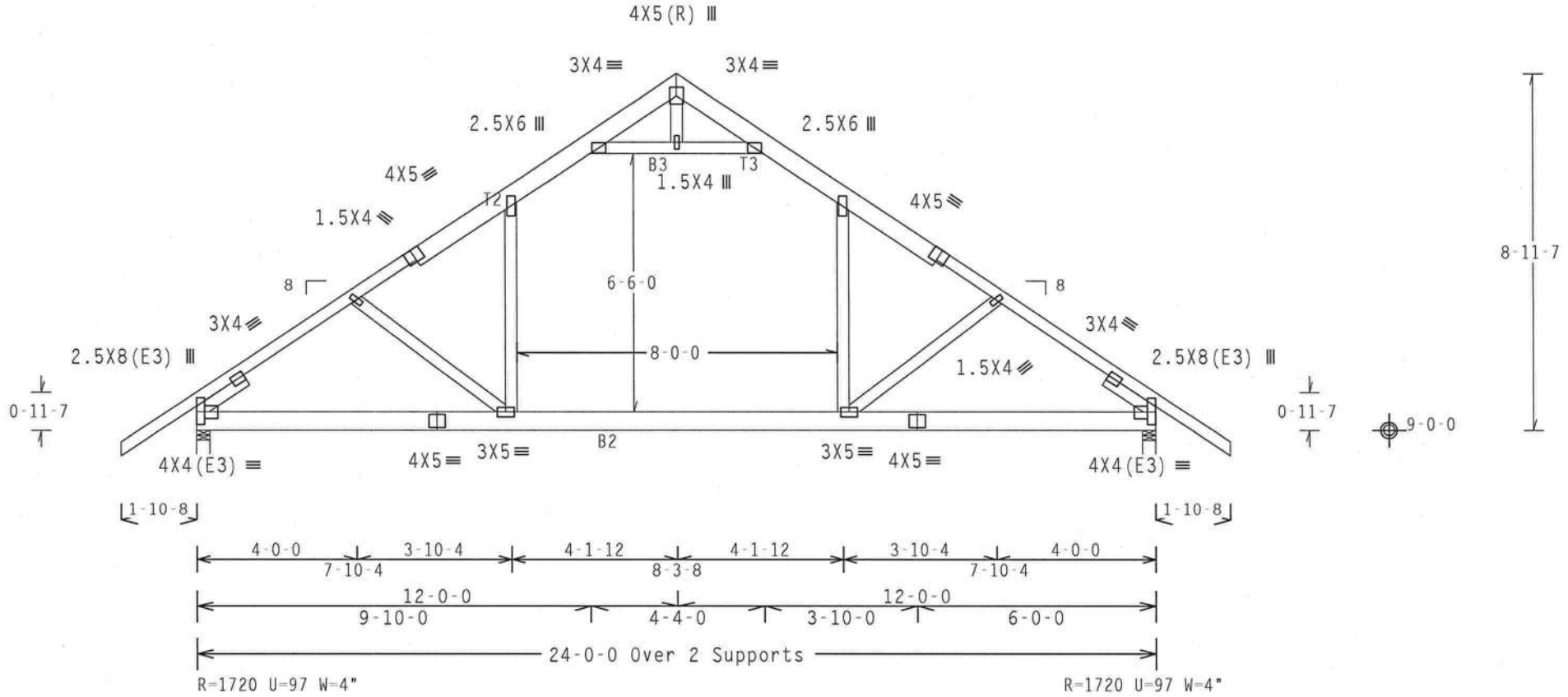
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Calculated horizontal deflection is 0.08" due to live load and 0.17" due to dead load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-0-0 to 16-0-0.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:4 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9172
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158016
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	170239
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
:B3 2x4 SP #2 Dense:
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	64 PLF	at	0.00	to	64 PLF	at 8.00
TC - From	88 PLF	at	8.00	to	88 PLF	at 9.53
TC - From	64 PLF	at	9.53	to	64 PLF	at 12.00
TC - From	64 PLF	at	12.00	to	64 PLF	at 14.47
TC - From	88 PLF	at	14.47	to	88 PLF	at 16.00
TC - From	64 PLF	at	16.00	to	64 PLF	at 25.88
PLT- From	20 PLF	at	10.21	to	20 PLF	at 13.79
BC - From	20 PLF	at	-0.50	to	20 PLF	at 8.00
BC - From	120 PLF	at	8.00	to	120 PLF	at 16.00
BC - From	20 PLF	at	16.00	to	20 PLF	at 24.00
BC - From	5 PLF	at	24.00	to	5 PLF	at 25.88

BC - 110 LB Conc. Load at 8.00, 16.00
PLB- 904 LB Conc. Load at (-0.49,9.04)

Wind reactions based on MWFRS pressures.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS"). TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

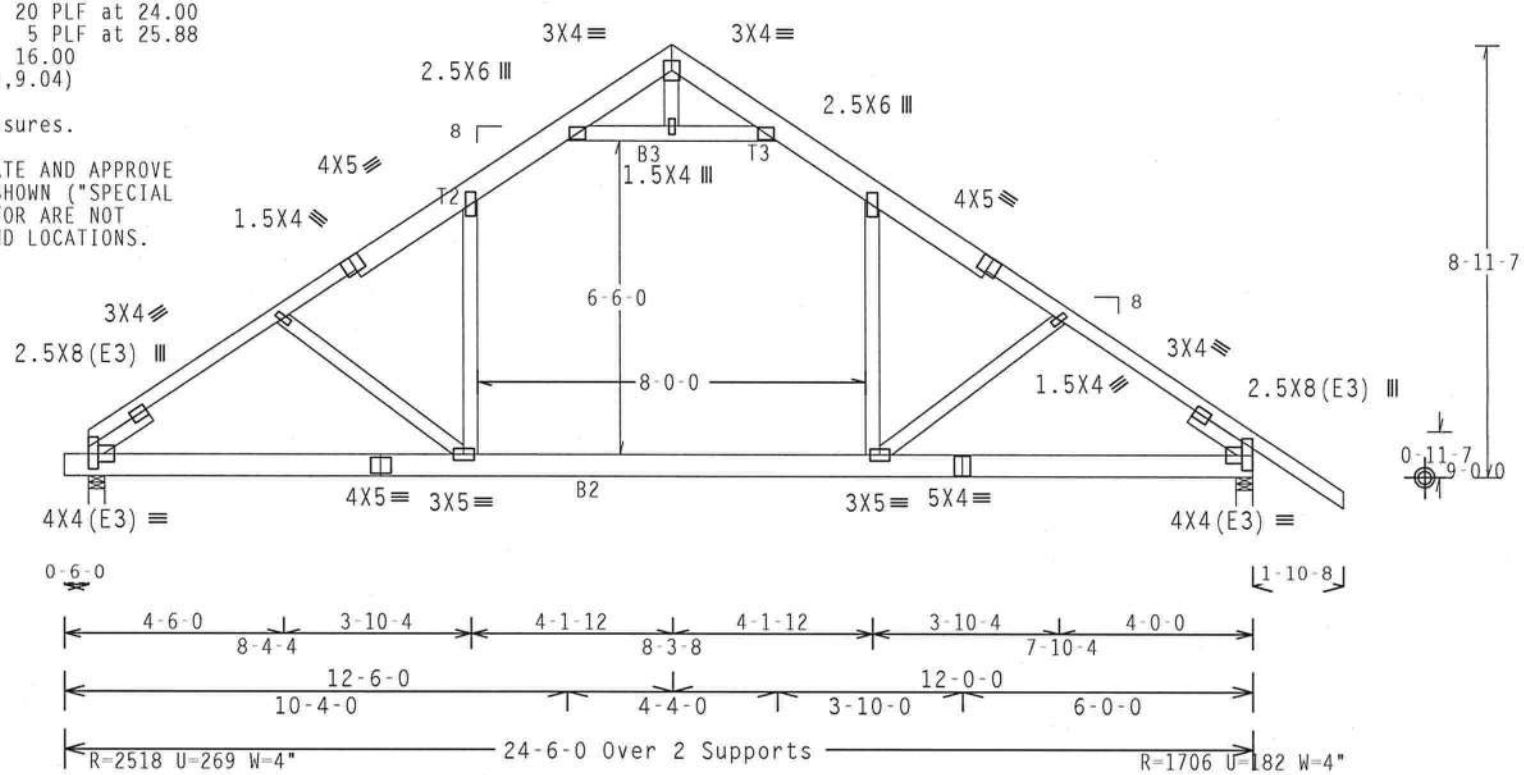
Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.08" due to live load and 0.18" due to dead load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

4X5(R) III



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:8 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
JUN 06 '08

TC LL	20.0 PSF	REF	R8228- 9173
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158017
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	170244
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - C3)

THIS BMG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

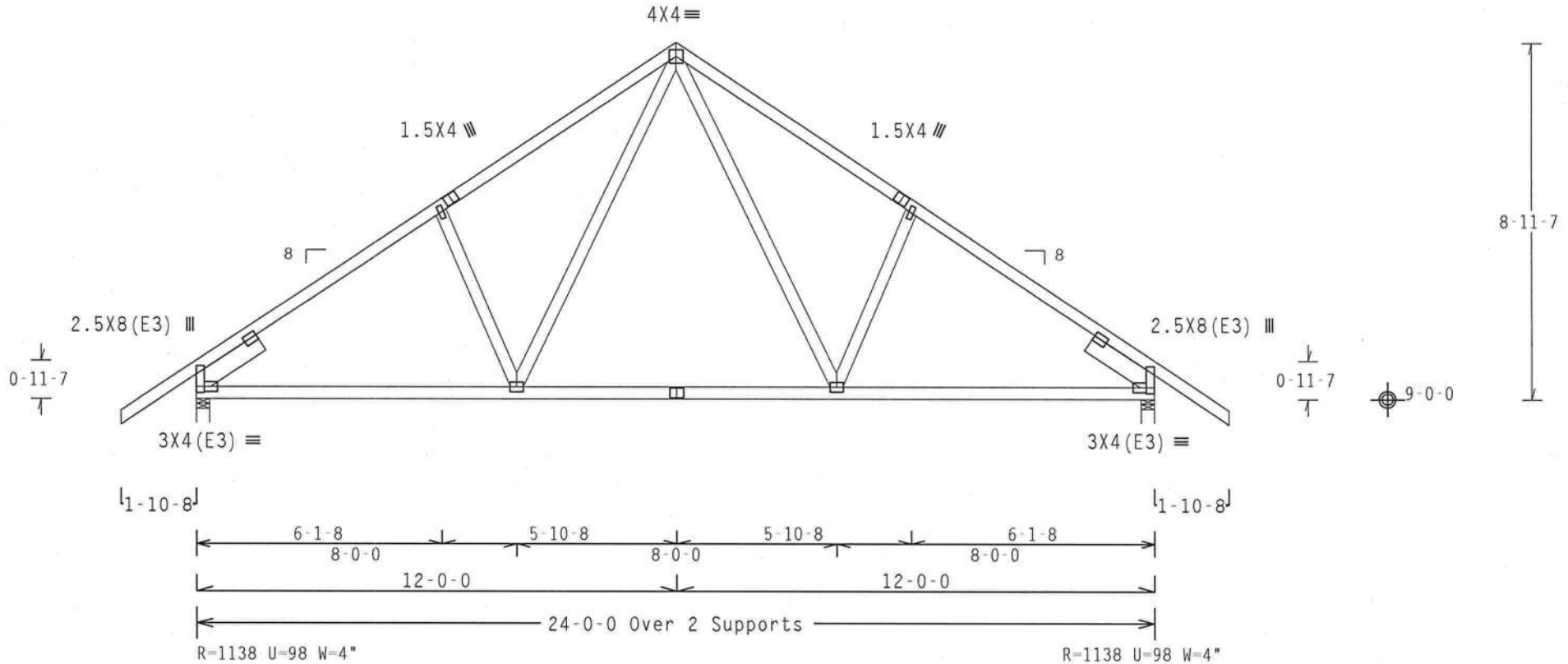
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.975'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.975'

Roof overhang supports 2.00 psf soffit load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.123[illegible] QTY:5 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

[illegible]06 '08

TC LL	20.0 PSF	REF	R8228- 9174
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158018
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	46823
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - C-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

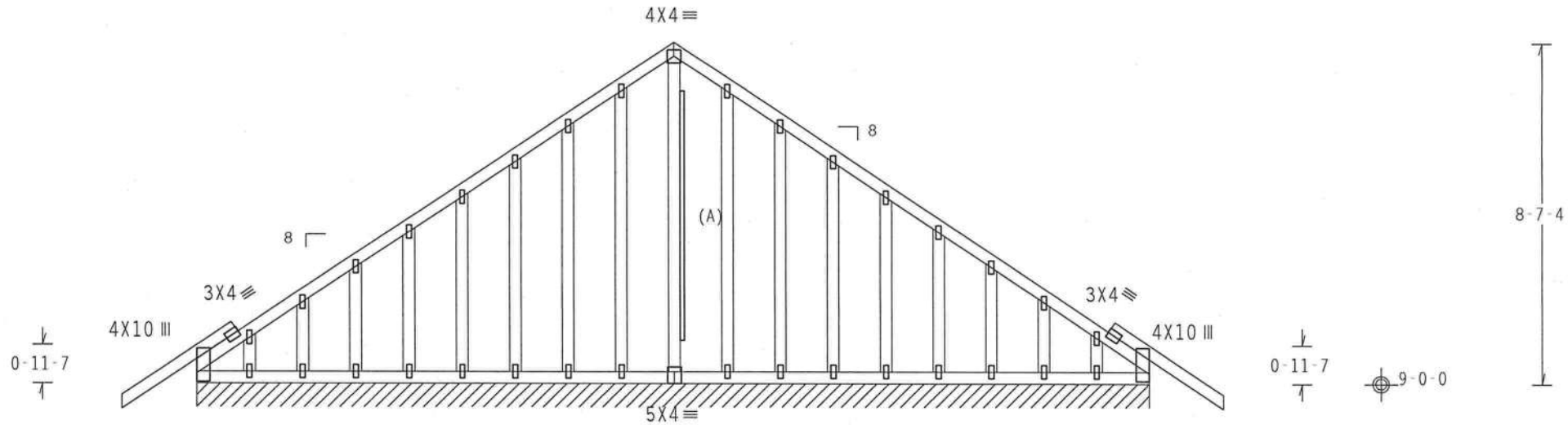
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

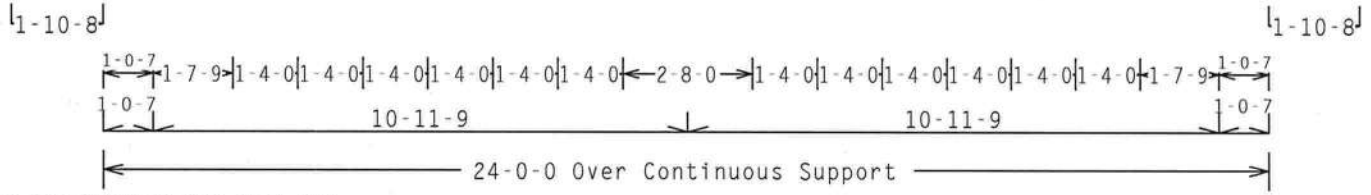
Wind reactions based on MWFRS pressures.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.





R=204 PLF U=30 PLF W=24-0-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9175
TC DL	10.0 PSF	DATE	06/06/08
BC DL	10.0 PSF	DRW	HCUSR8228 08158028
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	47047
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - CC-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord T1 2x4 SP #2 Dense:
:Stack Chord T4 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

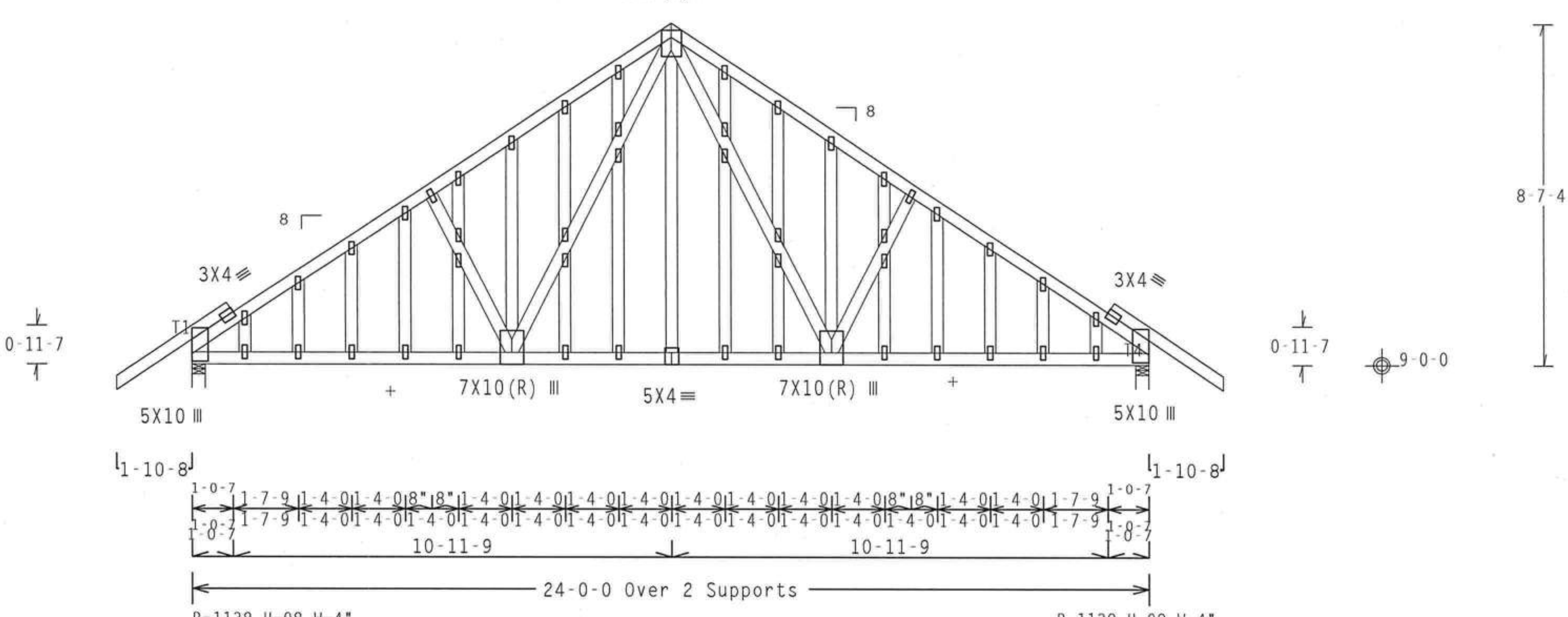
GABLE END IS DESIGNED TO SUPPORT 8" MAX RAKE OVERHANG.

SEE DRW HCUSR001 02086012 FOR GABLE DETAILS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF R8228- 9176
TC DL	10.0 PSF	DATE 06/06/08
BC DL	10.0 PSF	DRW HCUSR8228 08158029
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 46837
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - PB1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

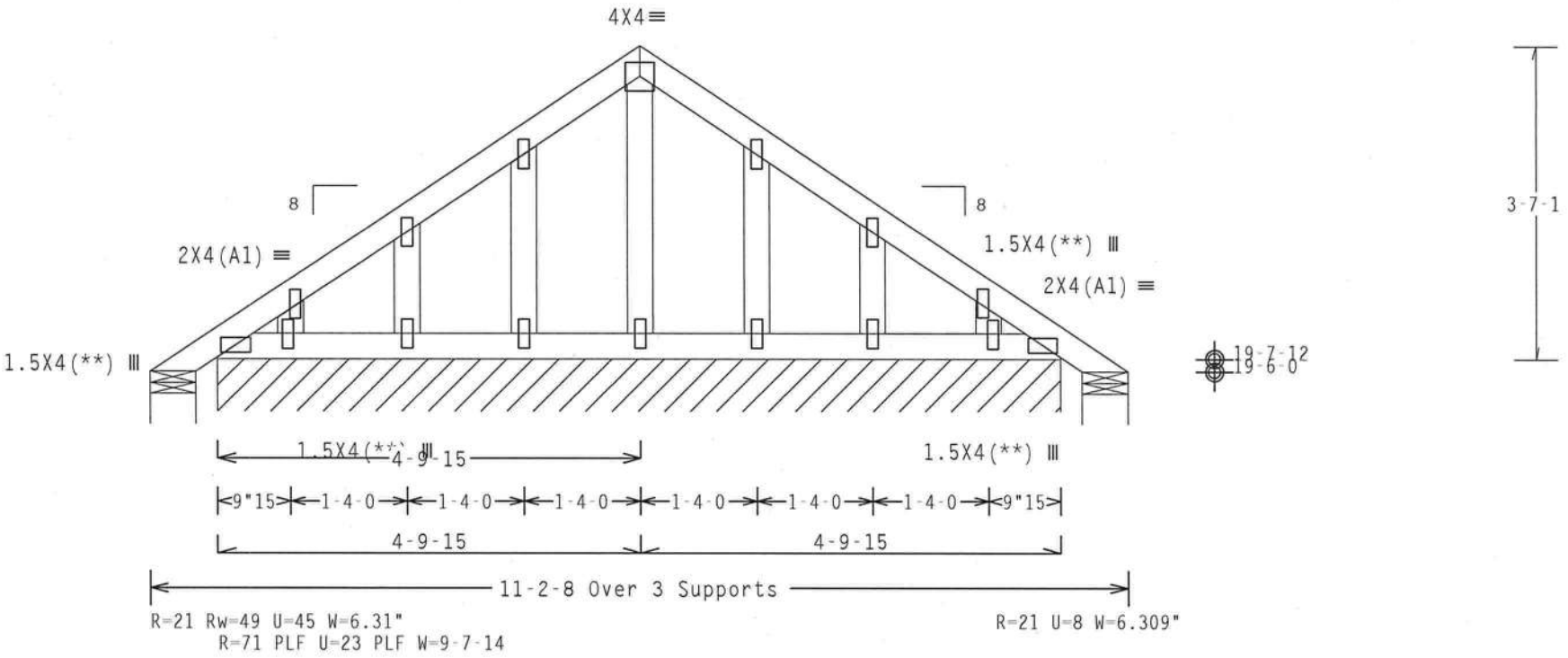
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 21.37 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9177
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158019
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	32.0 PSF	SEQN-	46848
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

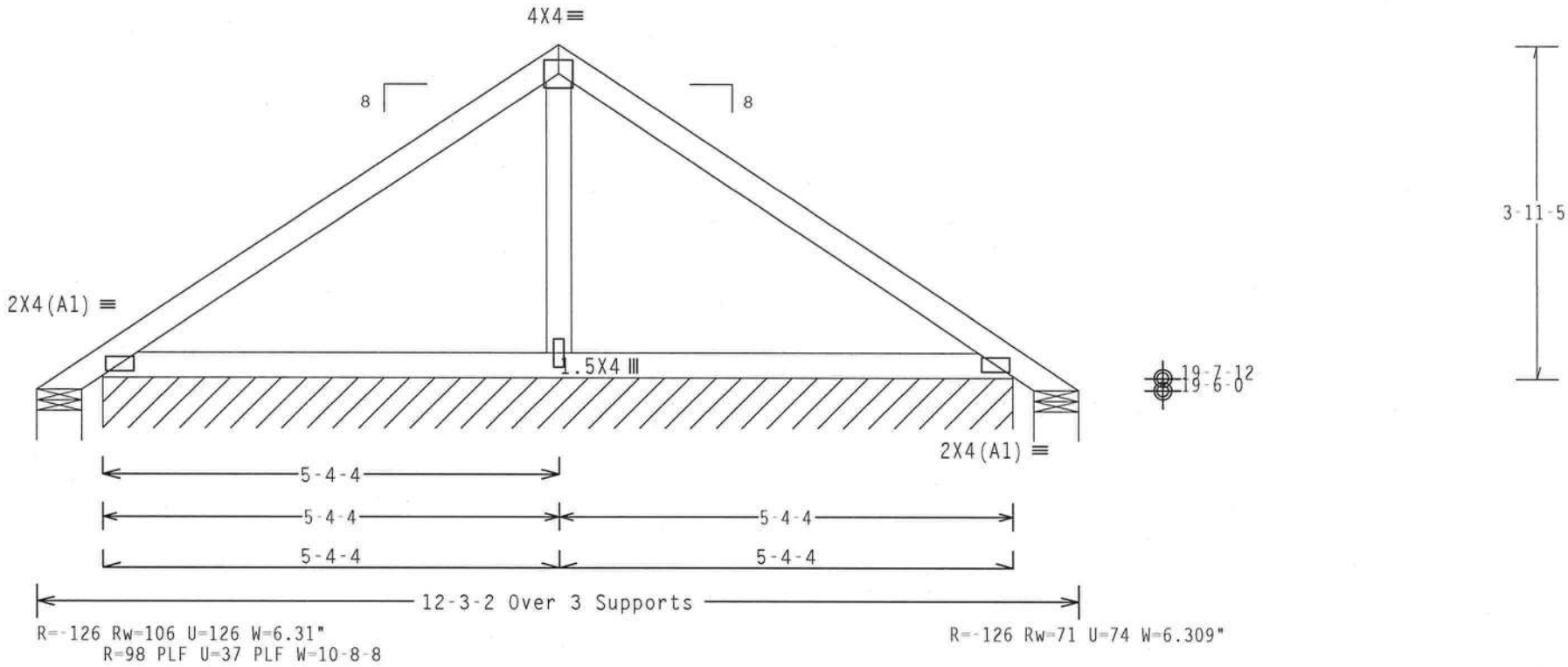
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 21.54 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12

PLT TYP. Wave

QTY:19 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
06 '08

TC LL	20.0 PSF	REF	R8228- 9178
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158020
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	32.0 PSF	SEQN-	170260
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

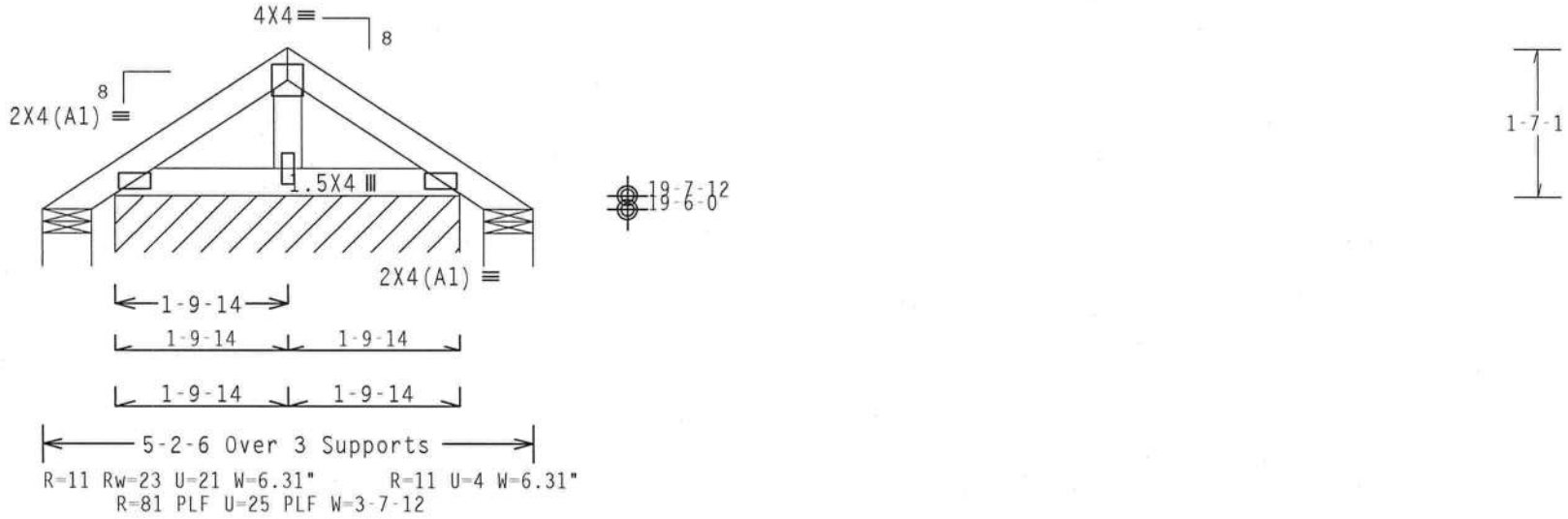
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.37.052[illegible]3

PLT TYP. Wave QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
[illegible]06 '08

TC LL	20.0 PSF	REF	R8228- 9179
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEQN-	30920 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - PB5)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

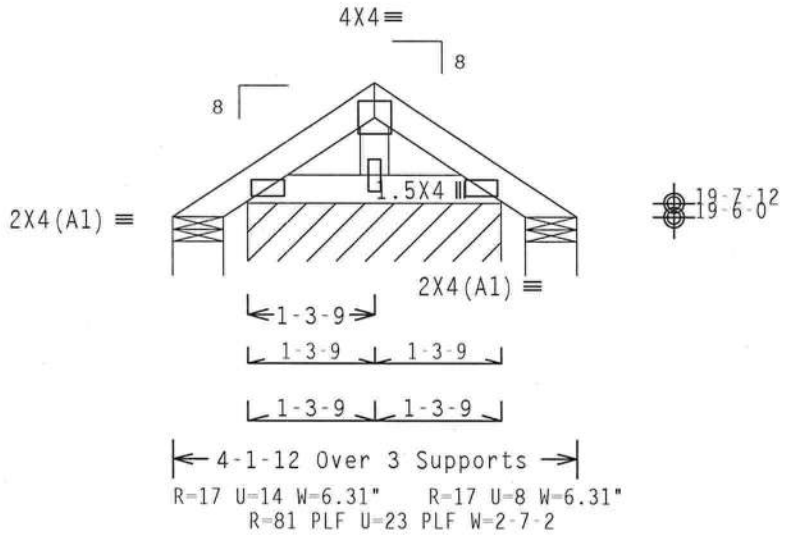
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 20.19 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



1-2-13

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.37.05 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

06 '08

TC LL	20.0 PSF	REF	R8228- 9180
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	32.0 PSF	SEQN-	30923 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

(8-140--OWNER BUILDER Baker -- , ** - PB4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
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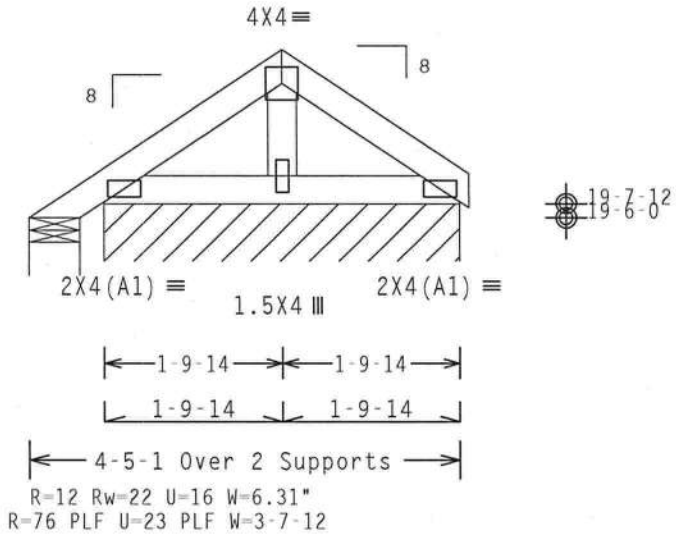
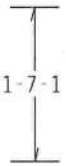
110 mph wind, 20.36 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:12 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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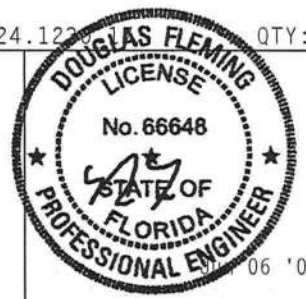


TC LL	20.0 PSF	REF	R8228- 9181
TC DL	10.0 PSF	DATE	06/06/08
BC DL	2.0 PSF	DRW	HCUSR8228 08158021
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	32.0 PSF	SEQN-	46941
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TI58228Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

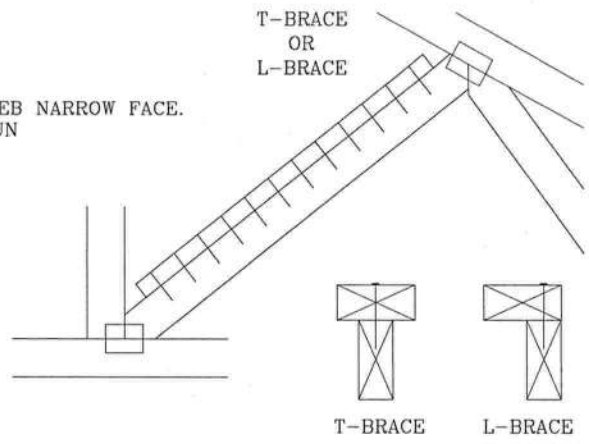
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

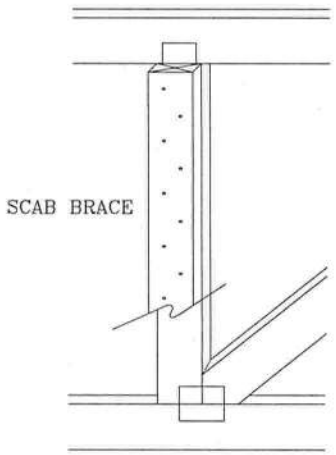
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jun 06

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TOP CHORD FILLER DETAIL

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C. MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH (2) 16d COMMON (0.162"X 3.5",MIN) NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.

* 8/12 MAXIMUM PITCH.

** 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

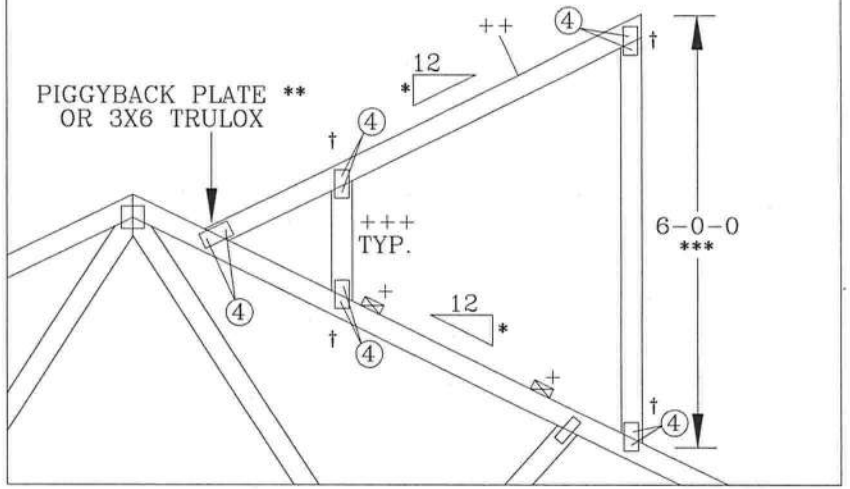
*** 6'0" MAXIMUM HEIGHT.

† W2X4 OR 3X6 TRULOX.

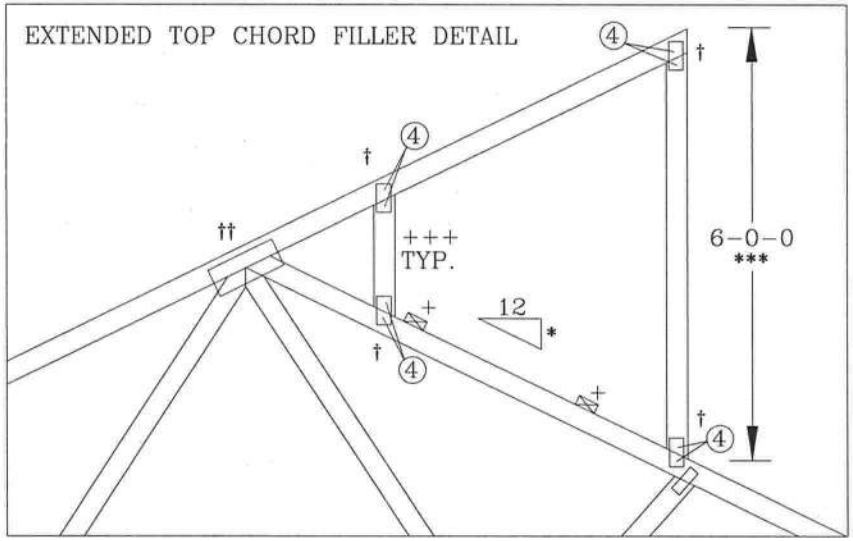
†† REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

0.120"X 1.375" NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS

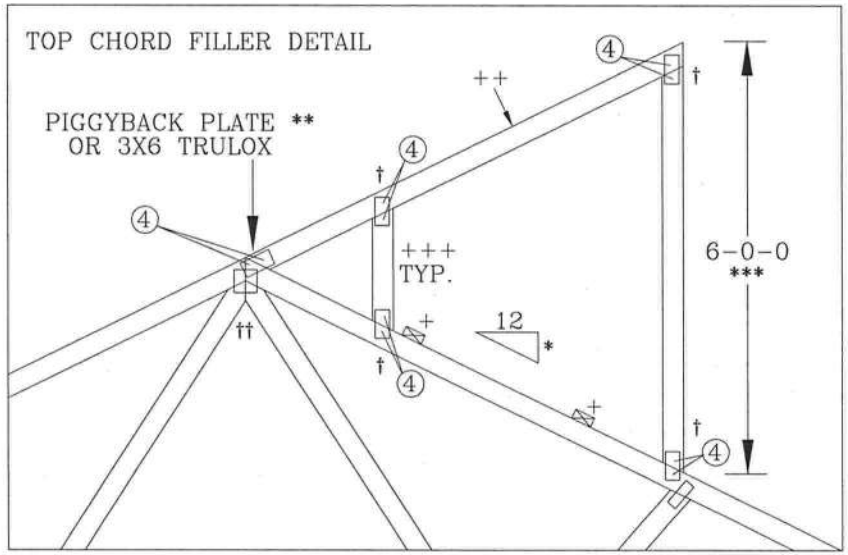
OFFSET FILLER DETAIL



EXTENDED TOP CHORD FILLER DETAIL



TOP CHORD FILLER DETAIL



THIS DRAWING REPLACES DRAWING 884,080

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 06 '08

TC LL	MAX 30 PSF	REF	TC-FILLER
TC DL	MAX 15 PSF	DATE	2/23/07
BC DL	MAX 10 PSF	DRWG	TCFILLER0207
BC LL	0 PSF	-ENG	SJP/KAR
TOT. LD.	MAX 55 PSF		
DUR. FAC.	1.15 OR 1.33		
SPACING	24.0"		

BOTTOM CHORD FILLER DETAIL

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

0.120" X 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS

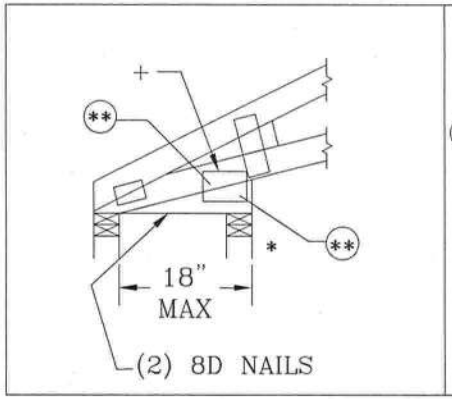
\+ 3X4 WAVE OR 4X8 TRULOX

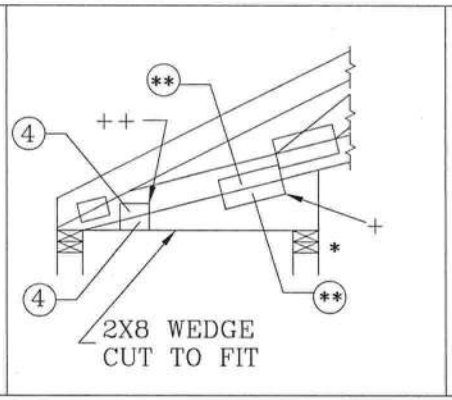
++ 2X4 WAVE OR 3X6 TRULOX

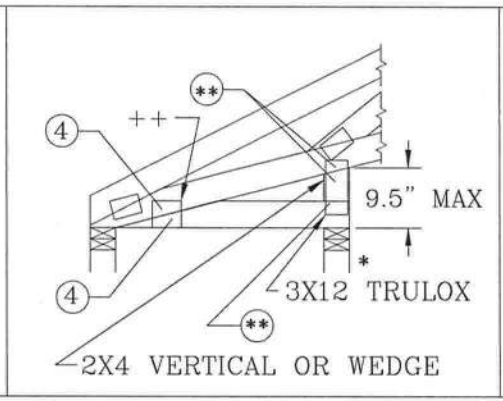
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

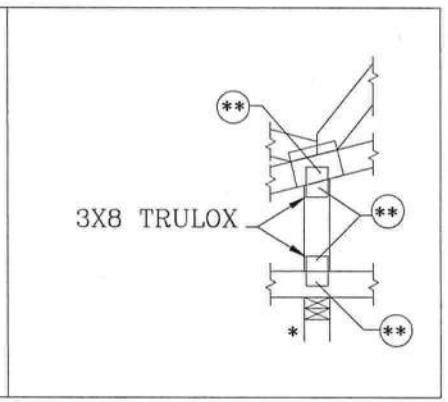
ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM	** REQUIRED NAILS PER FACE WITH TRULOX PLATES				
	DOWNWARD	UPLIFT	BEARING AREA	1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6









THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884,132

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 05 '08

TC LL	—	PSF	REF BC FILLER
TC DL	—	PSF	DATE 2/23/07
BC DL	10.0	PSF	DRWG BCFILLER0207
BC LL	—	PSF	-ENG DLJ/KAR
TOT. LD.	—	PSF	
DUR. FAC. 1.0/1.15/1.25/1.33			
SPACING 24.0"			

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

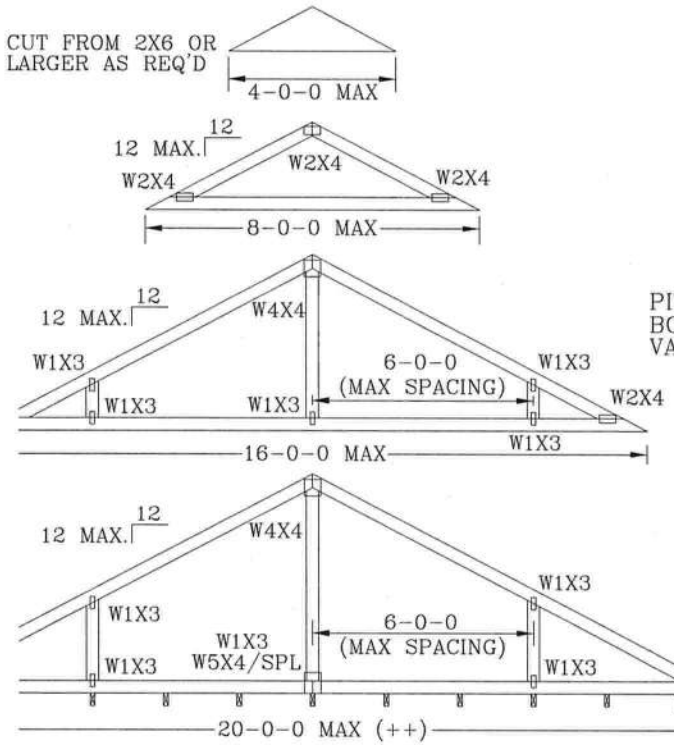
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

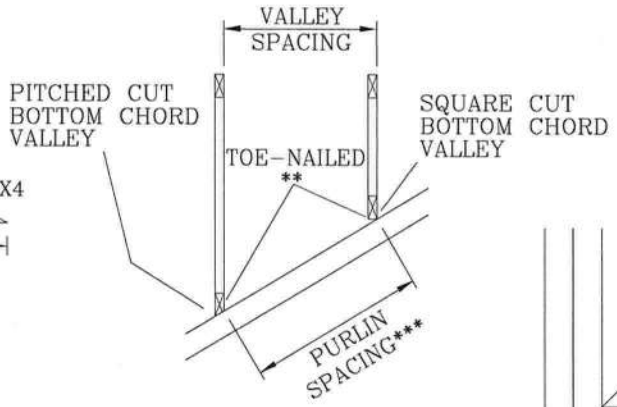
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

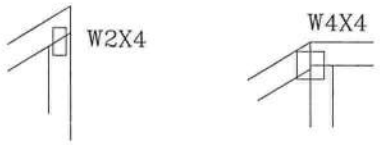
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



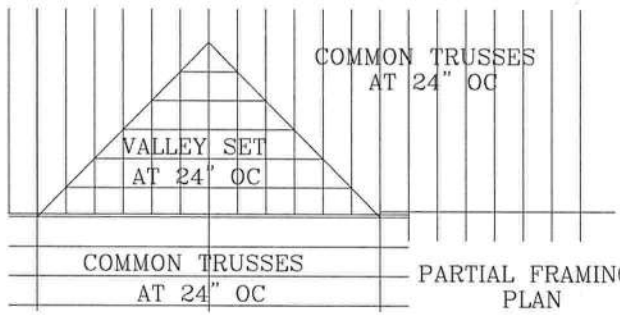
SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.





OPTIONAL STUB END DETAIL

OPTIONAL HIP JOINT DETAIL



PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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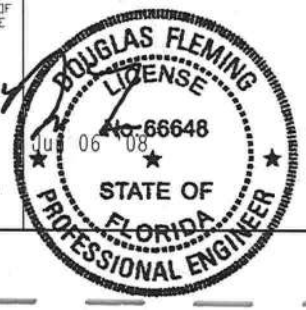


TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24"		

REF	VALLEY DETAIL
DATE	2/23/07
DRWG	VALTRUSS0207
-ENG	MLH/KAR

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

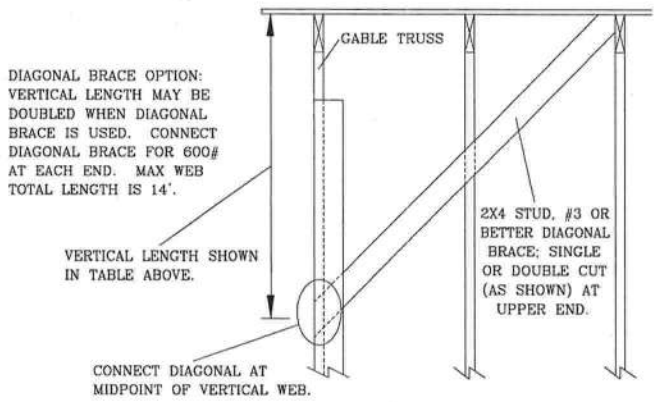
SOUTHERN PINE
#3
STUD
STANDARD

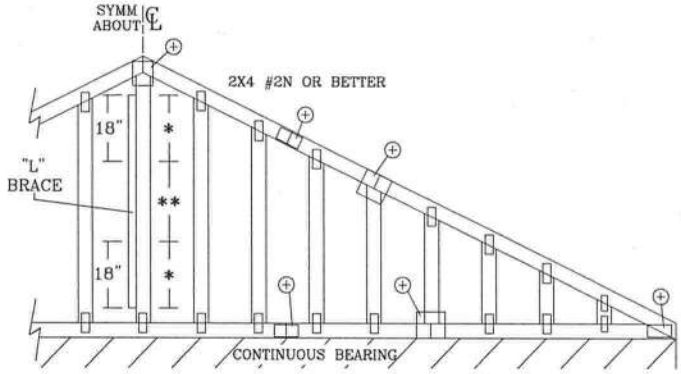
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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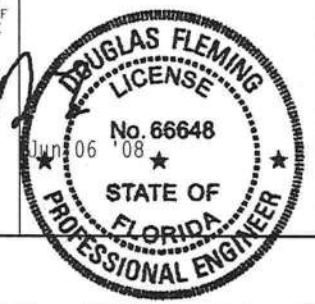
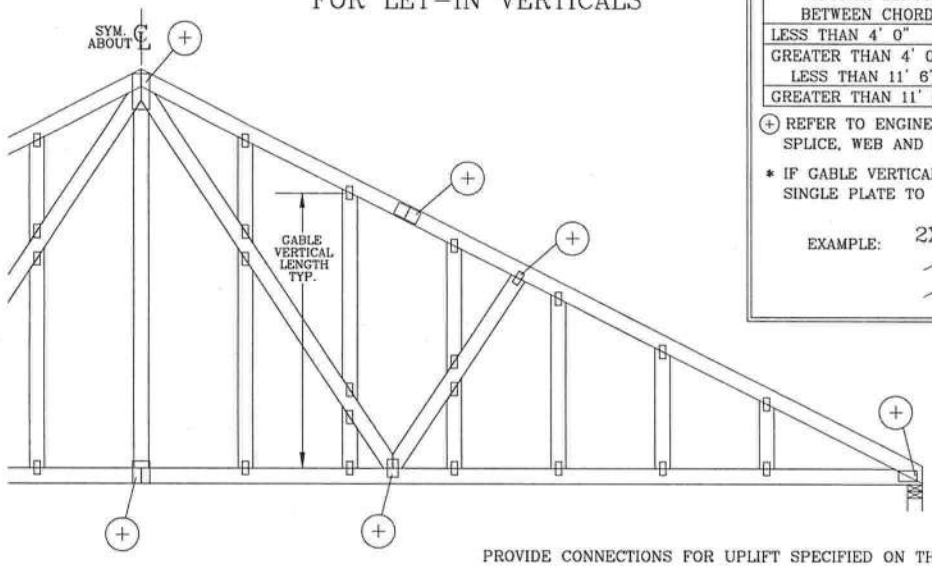


MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS



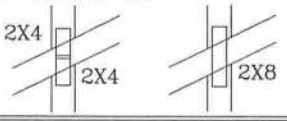
GABLE VERTICAL PLATE SIZES

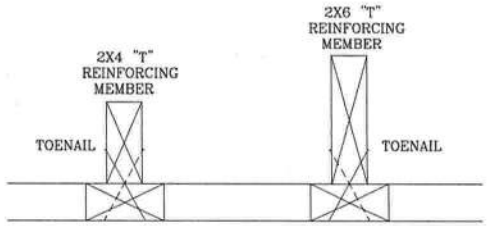
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:





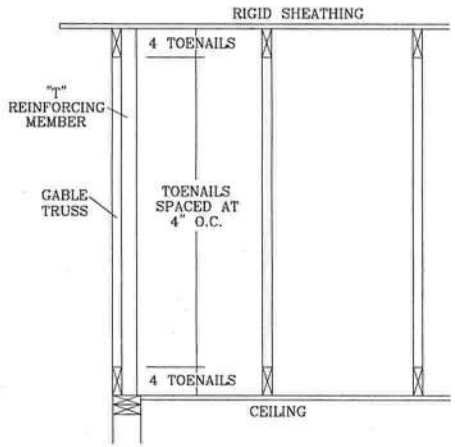
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

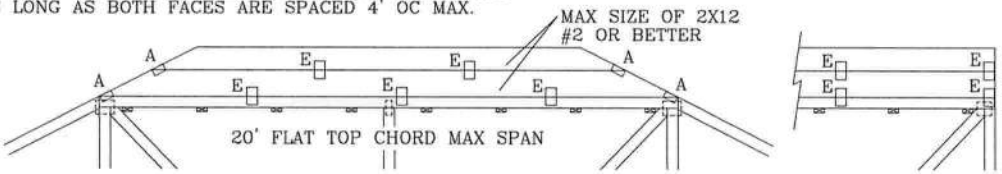
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

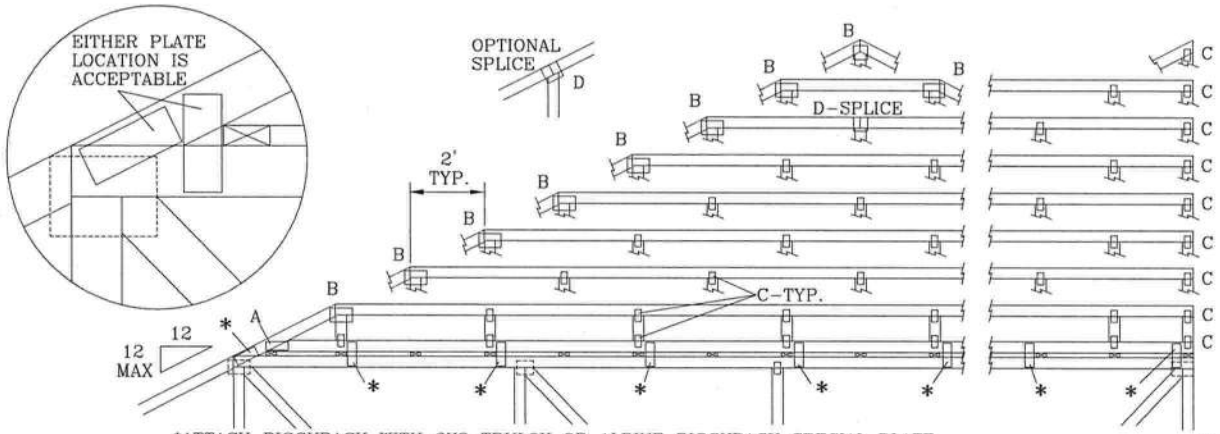
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

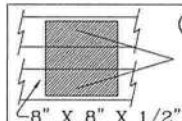
110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

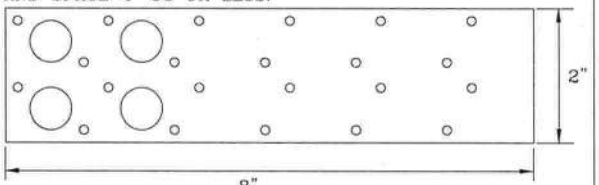
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE

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POMPANO BEACH, FLORIDA

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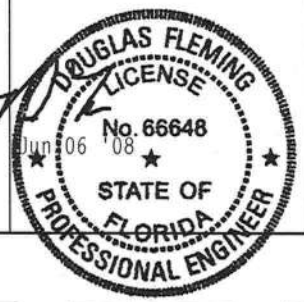


MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF	PIGGYBACK
DATE	2/23/07
DRWG	PIGBACKB0207
-ENG	DLJ/KAR

(ASCE 7-98 GABLE END DETAILS ,140 MPH,EXP C, PART. ENC)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(**) 2X4 SO. PINE #3 GABLE STUDS. ATTACH TO TOP CHORD, DIAGONAL WEBS, AND BOTTOM CHORD WITH W2X4 ALPINE PLATES. ALL (**) GABLE STUDS REQUIRED REINFORCING MEMBER. REINFORCING MEMBER MUST BE TOENAILED TO GABLE STUD WITH 0.131"X3" GUN NAILS AT 4" O.C. PLUS A CLUSTER OF 0.131"X3" TOENAILS AT THE TOP AND BOTTOM CHORD.SEE DETAIL FOR NAILING. SEE CHART FOR STUD BRACING AND SPACING OF VERTICALS.

NOTE:TRUSS ERECTOR IS RESPONSIBLE FOR PERMANENT WEB BRACING. WHEN BRACING IS REQUIRED, FURNISH A COPY OF THIS DRAWING TO TRUSS ERECTOR.

+PLATE AS REQUIRED ON APPROPRIATE DRAWING.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

+++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, PART. ENCLOSED BLDG, CAT II, EXP. C.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02, PART. ENCLOSED BLDG, CAT II, EXP. C.

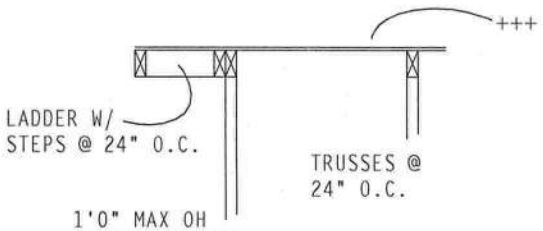
SEE APPROPRIATE ALPINE DRAWING FOR LUMBER,PLATES AND OTHER DATA NOT SHOWN HERE.

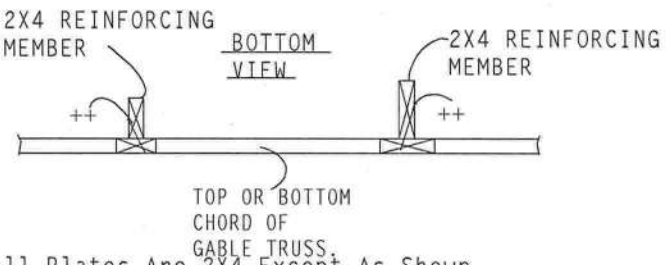
** STUD MUST BE ATTACHED TO CHORDS AND DIAGONAL

REINFORCING MEMBER REQUIRED	SPACING	MAX.LENGTH
2X4 SO. PINE #3	24"O.C.	2'- 1"
2X4 SO. PINE #3	16"O.C.	2'-10"
2X4 SO. PINE #3	12"O.C.	3'- 5"
2X6 SO. PINE #2 N	24"O.C.	5'- 0"
2X6 SO. PINE #2 N	16"O.C.	6'- 2"
2X6 SO. PINE #2 N	12"O.C.	7'- 1"
2X8 SO. PINE #2 N	24"O.C.	7'- 6"
2X8 SO. PINE #2 N	16"O.C.	9'- 1"
2X8 SO. PINE #2 N	12"O.C.	10'- 4"

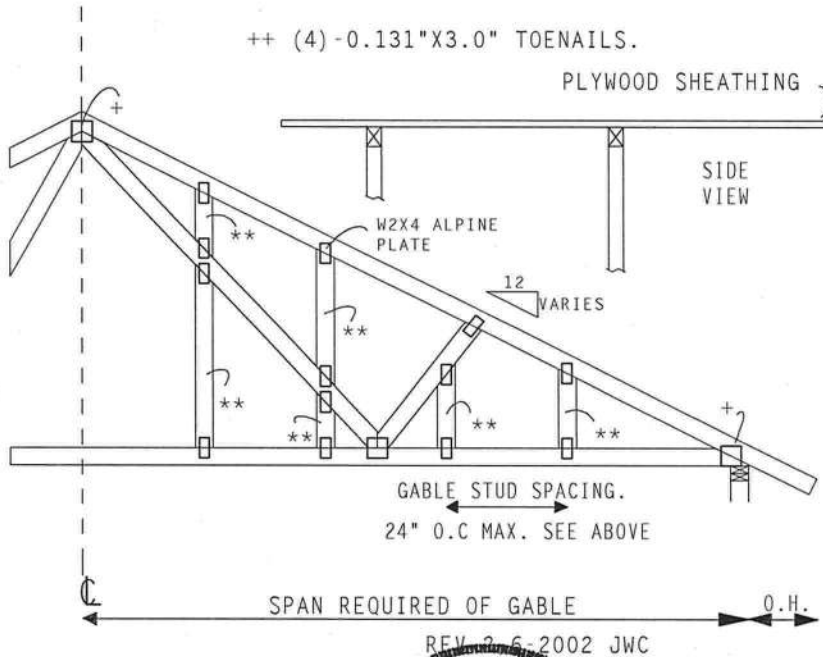
R2: REVISED FOR ASCE 7-02
DLJ 09/30/2005

R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/2006





Note: All Plates Are 2X4 Except As Shown.



DETAIL: 140GS

PLT TYP. Wave TPI-95

Design Criteria: TPI(STD)

REV 2-6-2002 JWC

QTY:1 HI/-/1/-/-/R/-

Scale =.3125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization #

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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER Jun 06 '06

TC LL	30.0 PSF	REF R001-- 0
TC DL	15.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCUSR001 02086012
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	55.0 PSF	SEQN - 24104
DUR.FAC.	1.33	FROM HC
		JREF- 1SV3001_R03

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

+ FOR VERTICAL WEBS LESS THAN 4'0": W1X4
FOR VERTICAL WEBS GREATER THAN 4'0" BUT NO MORE THAN 11'6": W2X4.

* SPLICE, PEAK, AND HEEL PLATES TO MATCH COMMON TRUSS.

** 2X4 OR GREATER CHORDS.

DROP GABLE WILL SUPPORT 4'0" OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) SPACED 24" O.C., OR THE LOAD FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN, CEILING AND ROOF SHEATHING DIAPHRAGM CONNECTIONS, AND ALL TRUSS TO WALL CONNECTIONS.

++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R1 NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

R1 NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

ALT. GABLE SHAPES:

Note: All Plates Are 2X4 Except As Shown.

PLT TYP. Wave TPI-95 Design Crit: TPI-1995(STD)

BRACING DEFINITIONS:
NOTE: "END ZONE" EXISTS 18" AT BOTH ENDS OF VERTICAL WEB.

(A) (1) 2X4 SP #3 "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(B) (2) 2X4 SP #3 "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

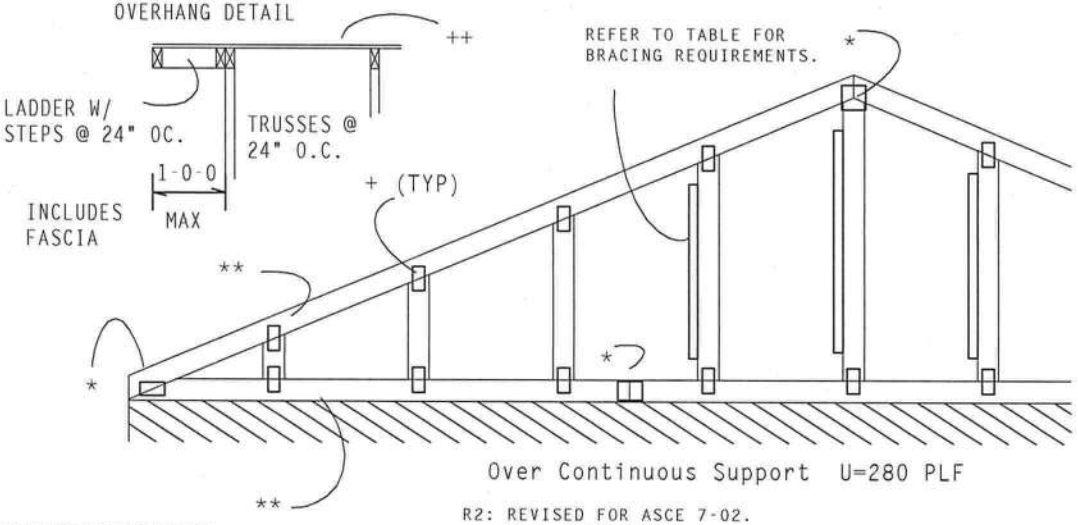
(C) (1) 2X6 SP #2 N "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(D) (2) 2X6 SP #2 N "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

STUD SPACING / BRACING TABLE:

2X4 SP #3 STUD SPACING	DEFLECTION CRITERIA	NO BRACE	(1) 2X4 "L" BRACE TYPE (A)	(2) 2X4 "L" BRACE TYPE (B)	(1) 2X6 "L" BRACE TYPE (C)	(2) 2X6 "L" BRACE TYPE (D)
24"	L/360	-----	3' 1"	4' 2"	6' 3"	8' 0"
24"	L/180	-----	3' 4"	5' 7"	6' 3"	11' 0"
16"	L/360	-----	3'11"	5' 3"	7'10"	9'11"
16"	L/180	-----	4' 9"	7' 4"	9' 6"	11' 0"
12"	L/360	-----	4' 7"	6' 1"	8'11"	11' 0"
12"	L/180	-----	5'11"	8' 5"	11' 0"	11' 0"

OVERHANG DETAIL



R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/200G

R2: REVISED FOR ASCE 7-02.
DLJ 09/30/2005
R1 REV 2-5-02 JWC

HI/-/1/-/-/R/-

DETAIL: 140GC
Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
FL Certificate of Authorization #[illegible]

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER Jun 06'06

TC LL	30.0 PSF	REF	R001-- 0
TC DL	7.0 PSF	DATE	03/27/02
BC DL	10.0 PSF	DRW	HCUSR001 02086015
BC LL	0.0 PSF	HC-ENG	DLJ/DLJ
TOT.LD.	47.0 PSF	SEQN-	24860
DUR.FAC.	1.33		
SPACING	24.0"	JREF-	1SV3001_R03

Return To: Sierra Title, LLC
619 SW Baya Dr., Ste 102
Lake City, FL 32025

THIS INSTRUMENT WAS PREPARED BY:
FIRST FEDERAL SAVINGS BANK OF FLORIDA
4705 WEST U.S. HIGHWAY 90
P.O. BOX 2029
LAKE CITY, FLORIDA 32056

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Feagle Deputy Clerk
Date 04-24-2008

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Inst:200812008024 Date:4/24/2008 Time:11:45 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1148 P:22

TA

PERMIT NO. 26936

#08-0151

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF Columbia

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: See ExhibitA

2. General description of improvement: **Construction of Dwelling**

3. Owner information:
   a. Name and address: George R. Baker & Kristy S. Baker
   200 SW Soundless Ct., Lake City, FL 32024

   b. Interest in property: **Fee Simple**

   c. Name and address of fee simple title holder (if other than Owner): **NONE**

4. Contractor (name and address): EDGLEY CONSTRUCTION CO., a div of CEE-BAS, Inc.

5. Surety:
   a. Name and address: N/A

   b. Amount of bond:

6. Lender: **FIRST FEDERAL SAVINGS BANK OF FLORIDA**
   4705 WEST U.S. HIGHWAY 90
   P. O. BOX 2029
   LAKE CITY, FLORIDA 32056

7. Persons within the State of Florida designated by Owner upon whom notices or other document may be served as provided by Section 713.13 (1) (a) 7., Florida Statutes: NONE

8. In addition to himself, Owner designates **PAULA HACKER of FIRST FEDERAL SAVINGS BANK OF FLORIDA, 4705 West U.S. Highway 90 / P. O. Box 2029, Lake City, Florida 32056** to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

George R. Baker
Borrower Name

Kristy S. Baker
Co-Borrower Name

The foregoing instrument was acknowledged before me this 22 day of APRIL, 2008 by George R. Baker & Kristy S. Baker, who is personally known to me or who has produced driver's license for identification.

Notary Public State of Florida
Matthew Rocco
My Commission DD578349
Expires 09/17/2010

Notary Public
My Commission Expires:

File No. 08-0151/Baker

Exhibit A
Legal Description

Commence at the SW corner of the NW1/4 of the NW 1/4 of Section 28, Township 4 South, Range 16 East, Columbia County, Florida and run N 00°27'17"E, 323.05 feet to the Point of Beginning; thence continue N 00°27'17" E, 324.19 feet; thence N 89°28'27" E, 672.28 feet; thence S 00°04'03" E, 324.15 feet S 89°28'27" W, 675.23 feet to the Point of Beginning.

Subject to a non-exclusive perpetual easement for ingress and egress over and across the west 60 feet thereof.

Together with an easement for ingress and egress purposes, being 60 feet East of and adjacent to the following described line: Commence at the SW corner of the NW 1/4 of the NW 1/4 of Section 28, Township 4 South, Range 16 East, Columbia County, Florida and run N 00°27'17" E, along the West line of said Section 28 a distance of 323.05 feet to the Point of Beginning; thence continue N 00°27'17" E, along said West line of Section 28, a distance of 931.74 feet to a point on the South right-of-way line of County Road 242, said point also being the terminal point of herein described line and easement.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-4S-16-03234-002 **Building permit No.** 000026936

Use Classification SFD,UTILITY **Fire:** 70.62

Permit Holder EDGLEY CONSTRUCTION **Waste:** 184.25

Owner of Building GEORGE & KRISTY BAKER **Total:** 254.87

Location: 235 SW EVA TERR., LAKE CITY, FL

Date: 11/20/2008

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

FEES:

ROAD IMPACT FEE 1046.00 CODE 210 UNIT 1
10100003632400

EMS IMPACT FEE 29.88
10300003632210

FIRE PROTECTION IMPACT FEE 78.63
10200003632220

CORRECTIONS IMPACT FEE 409.16
00100003632200

SCHOOL IMPACT FEE 1500.00
00100003632900

TOTAL FEES CHARGED 3063.67 CHECK NUMBER _____