DATE 04/06/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024354

APPLICANT HUGO ESCALANTE PHONE 288-8666

ADDRESS 6210 SW CR 18 FORT WHITE FL 32038

OWNER HUGO ESCALANTE SR PHONE 786-295-6916

ADDRESS 235 SW BLUE JAY COURT FORT WHITE FL 32038

CONTRACTOR HUGO ESCALANTE PHONE 288-8666

LOCATION OF PROPERTY 47 S, L 27, L BLUE JAY COURT, LOT 7 ON RIGHT
ABOUT 1/4

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 85900.00

HEATED FLOOR AREA 1718.00 TOTAL AREA 2296.00 HEIGHT 18.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING FORT WHITE MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT REAR SIDE

NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO.

PARCEL ID 03-7S-16-04060-107 SUBDIVISION FORT WHITE PARK

LOT 7 BLOCK PHASE UNIT TOTAL ACRES 0.50

		CRC1326967	Hugo Escalante
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

FORT WHITE	06-0246-N	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: TOWN OF FORT WHITE LETTER ATTACHED

Check # or Cash 4057

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 430.00 CERTIFICATION FEE $ 11.48 SURCHARGE FEE $ 11.48

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ CULVERT FEE $ **TOTAL FEE** 502.96

INSPECTORS OFFICE L. H CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0603-87 Date Received 3/23/06 By CG Permit # 24354

Application Approved by - Zoning Official _____ Date _____ Plans Examiner OK JTH Date 4-6-06

Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____

Comments TOWN OF FORT white LETTER ATTACHED

Applicants Name Hugo Escalande Phone 386-288-8666

Address 6210 S.W. CR 18, Fort White, FL 32038

Owners Name Hugo Escalande SR. Phone 786-295-6916

911 Address 235 S.W. Blue Jay Court

Contractors Name Hugo Escalande Phone 386-288-8666

Address 6210 S.W. CR 18, FT White, FL 32038

Fee Simple Owner Name & Address None

Bonding Co. Name & Address None

Architect/Engineer Name & Address Daniel Shaheen, Lake City, FL

Mortgage Lenders Name & Address None

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 03-7S-16-04060-107 Estimated Cost of Construction 135,000-

Subdivision Name Fort White Park Lot 7 Block ____ Unit ____ Phase ____

Driving Directions 47 South, to US27 T/L, 2miles T/L on Blue Jay Court Lot 7 on right 1/4 mile.

Type of Construction New Single Family Number of Existing Dwellings on Property 0

Total Acreage .5 Lot Size 1/2 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 65 Side 15 Side 15 Rear 70

Total Building Height 18'-0" Number of Stories 1 Heated Floor Area 1718SF Roof Pitch 6-12

Porch 92 CACAg 486 TOTAL 2296

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Hugo Escalante
Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

LAURIE HODSON
MY COMMISSION # DD 333508
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Hugo Escalante
Contractor Signature

Contractors License Number CRC1326967

Competency Card Number _____

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me

this 23 day of March 20 06.

Personally known ✓ or Produced Identification _____

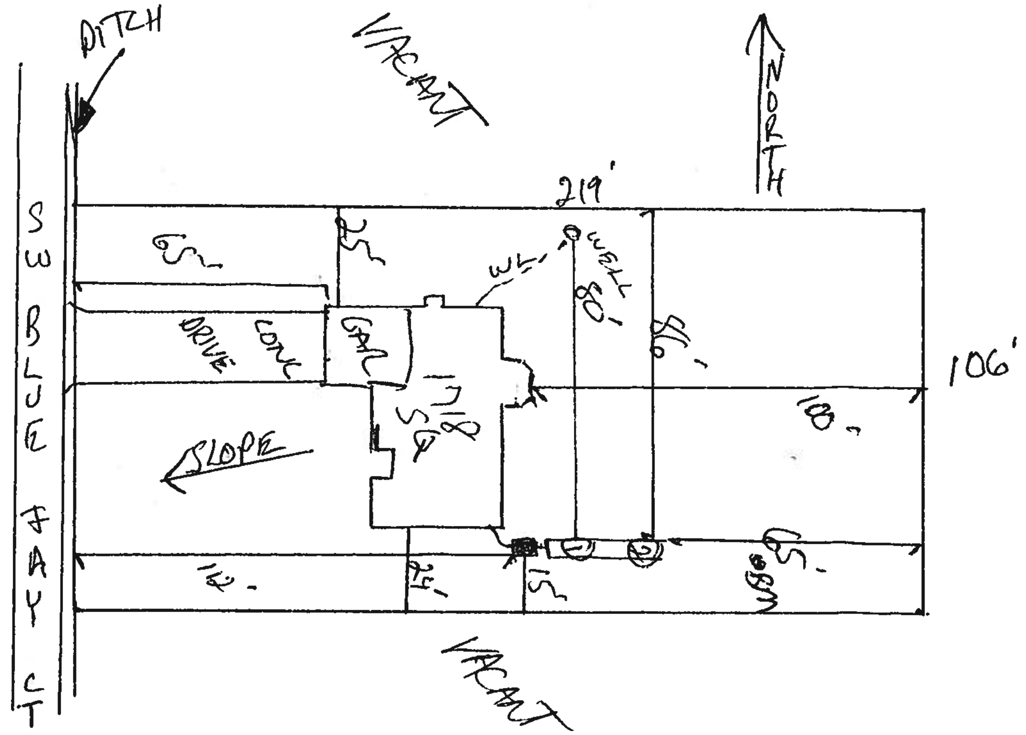
Laurie Hodson
Notary Signature

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0246N

---------------------------- PART II - SITEPLAN ----------------------------

Scale: 1 inch = 50 feet.



Notes: ____

Site Plan submitted by: [signature] MASTER CONTRACTOR

Plan Approved ✓ Not Approved_____ Date 3/20/06

By [signature] Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 03-7S-16-04060-107

1. **Description of property: (legal description of the property and street address or 911 address)**
   Lot 7 Ford White Park, ORB 753-864, 831-1602 WD 1006-421
   911 Address: 235 S.W. Blue Jay, Ford White, FL 32038

2. **General description of improvement:** New Single Family Dwelling

3. **Owner Name & Address** Hugo Escalante, SR, 6210 S.W. CR18, Ford White, FL
   Interest in Property 100%

4. **Name & Address of Fee Simple Owner (If other than owner):** N/A

5. **Contractor Name** Hugo Escalante (Empl FLC) **Phone Number** 386-288-8666
   Address 6210 S.W. CR 18 Ft White, FL 32038

6. **Surety Holders Name** None **Phone Number**
   Address None
   Amount of Bond None

   Inst:2006007230 Date:03/23/2006 Time:10:17
   DC,P.DeWitt Cason,Columbia County B:1078 P:817

7. **Lender Name** None
   Address None

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name Hugo Escalante **Phone Number** 386-288-8666
   Address 6210 S.W. CR 18, Ford White, FL 32038

9. **In addition to himself/herself the owner designates** Marleen Escalante **of** Ford White, **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** 386-497-2628

10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Hugo Escalante
Signature of Owner

Sworn to (or affirmed) and subscribed before day of March 23, 20 06

NOTARY STAMP/SEAL

LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Laurie Hodson
Signature of Notary

Town of Fort White

Post Office Box 129 Fort White, Florida 32038-0129
Town Hall - (386) 497-2321 • Public Works - (386) 497-3345
Email: townofftwhite@alltel.com • Web site: Townoffortwhitefl.com

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

OWNER'S NAME: Hugo Escalante

ADDRESS: 194 S.W. CR 18 Fort White, FL 32038

PROPERTY DESCRIPTION: Lot #7 Fort White Park
(parcel number if possible) parcel #4060-107 .50acres

DEVELOPMENT: single family dwelling

You are hereby authorized to issue the appropriate building permits.

03/03/06
DATE

Janis E. Revels (KA)
LAND DEVELOPMENT REGULATION
ADMINISTRATOR
TOWN OF FORT WHITE

Columbia County Property Appraiser

DB Last Updated: 2/10/2006

Parcel: 03-7S-16-04060-107

2006 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

<< Prev Search Result: 2 of 11 Next >>

Owner & Property Info

Owner's Name	ESCALANTE HUGO
Site Address	FORT WHITE PARK
Mailing Address	P O BOX 280 FT WHITE, FL 32038
Brief Legal	LOT 7 FORT WHITE PARK. ORB 753-864, 831-1602 WD 1006-421.

Use Desc. (code)	VACANT (000000)
Neighborhood	16.00
Tax District	4
UD Codes	MKTA02
Market Area	02
Total Land Area	0.500 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$10,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$10,500.00

Just Value	$10,500.00
Class Value	$0.00
Assessed Value	$10,500.00
Exempt Value	$0.00
Total Taxable Value	$10,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
1/30/2004	1006/421	WD	V	Q		$19,000.00
11/25/1996	831/1602	WD	V	U	14	$12,900.00
11/8/1991	753/864	WD	V	U	35	$60,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.500AC)	1.00/1.00/1.00/1.00	$10,500.00	$10,500.00

Columbia County Property Appraiser

DB Last Updated: 2/10/2006

<< Prev 2 of 11 Next >>

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 3/2/2006 **DATE ISSUED:** 3/3/2006

ENHANCED 9-1-1 ADDRESS:

235 SW BLUE JAY CT

FORT WHITE FL 32038

PROPERTY APPRAISER PARCEL NUMBER:

03-7S-16-04060-107

Remarks:

LOT 7 FORT WHITE PARK S/D

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

95

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit #_____ Owner's Name: EWPL - Ft. White Park Lot 7

Well Depth_____ Ft. Casing Depth _____Ft. Water Level _____Ft.

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model S20-100 HP 1

System Pressure (PSI) On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20(GPM)

Tank Installation: Bladder /Galvanized Make Challenger

Model PC 244 Size 81 gallon

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

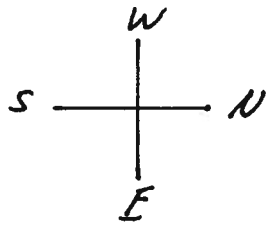
Linda Newcomb
Signature

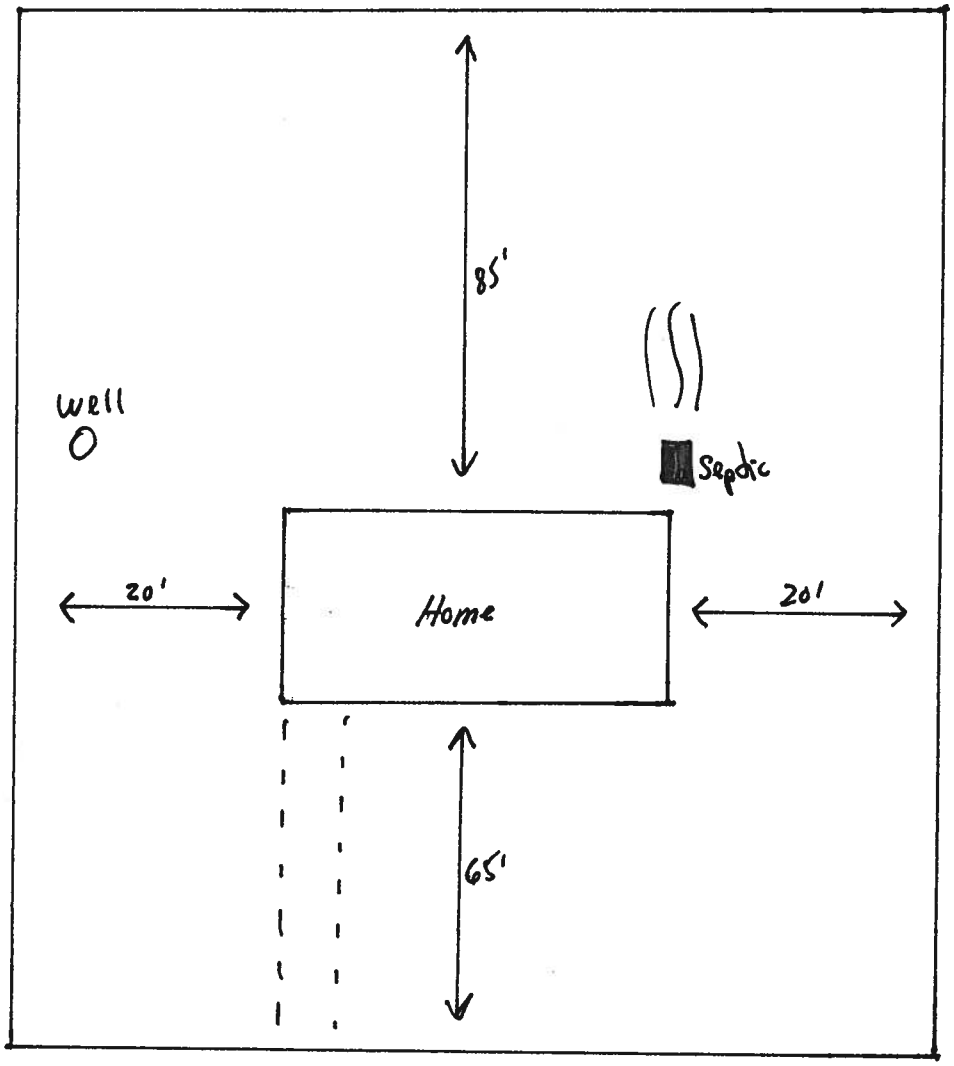
Linda Newcomb
Print Name

2609
License Number

3/23/06
Date

Lot 7 Ford White Park
Parcel # 03-7S-16-04060-107





SW Blue Jay Ct

Notice of Treatment 12095

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYAVE
City Lake City Phone 752 1705

Site Location: Subdivision Ft White Park
Lot # 7 Block# Permit # 24354
Address 173 SW Blue Jay

Product used	Active Ingredient	% concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☐ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2296		4

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

7/27/06 Date | 1030 Time | F254 Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	THE NICOLAS +	Builder:	EWPL INC
Address:	Lot: 7, Sub: FW Park, Plat:	Permitting Office:	Columbia Co.
City, State:	Fort White, FL 32038-	Permit Number:	24354
Owner:	EWPL INC	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	
2. Single family or multi-family	Single family	
3. Number of units, if multi-family	1	
4. Number of Bedrooms	3	
5. Is this a worst case?	No	
6. Conditioned floor area (ft²)	1718 ft²	
7. Glass area & type		
a. Clear - single pane	0.0 ft²	
b. Clear - double pane	351.7 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	
d. Tint/other SHGC - double pane	0.0 ft²	
8. Floor types		
a. Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft	
b. N/A		
c. N/A		
9. Wall types		
a. Frame, Wood, Adjacent	R=13.0, 197.0 ft²	
b. Frame, Wood, Exterior	R=13.0, 1554.0 ft²	
c. N/A		
d. N/A		
e. N/A		
10. Ceiling types		
a. Under Attic	R=30.0, 1718.0 ft²	
b. N/A		
c. N/A		
11. Ducts		
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 115.0 ft	
b. N/A		
12. Cooling systems		
a. Central Unit	Cap: 30.0 kBtu/hr SEER: 10.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 30.0 kBtu/hr HSPF: 6.80	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons EF: 0.88	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.20

Total as-built points: 27223

Total base points: 27515

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: 0

DATE: _____

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2746.00	8238.0	50.0	0.88	3		1.00	2746.00	1.00	8238.0
				As-Built Total:							**8238.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
9643	**9634**	**8238**	**27515**	**9798**	**9186**	**8238**	**27223**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points				Area X	WPM	=	Points
	1718.0	-0.59	-1013.6				1718.0	-0.59		-1013.6
Winter Base Points:			**15355.6**	**Winter As-Built Points:**						**15762.5**
Total Winter Points	X System Multiplier		= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
15355.6	**0.6274**		**9634.1**	15762.5 **15762.5**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.501 **0.501**	1.000 **1.000**		9186.4 **9186.4**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1718.0	12.74	3939.7	Double, Clear	N	1.5	7.5	42.0	14.30	1.00	601.4
				Double, Clear	N	9.0	10.0	13.3	14.30	1.02	194.0
				Double, Clear	N	9.0	4.0	9.3	14.30	1.03	136.9
				Double, Clear	N	1.5	5.5	17.5	14.30	1.00	251.1
				Double, Clear	E	1.5	5.5	30.0	9.09	1.04	284.0
				Double, Clear	S	1.5	5.5	17.5	4.03	1.15	80.9
				Double, Clear	S	1.5	6.5	72.0	4.03	1.09	317.5
				Double, Clear	SW	1.5	6.5	16.0	7.17	1.05	120.5
				Double, Clear	S	1.5	6.5	36.0	4.03	1.09	158.8
				Double, Clear	SE	1.5	6.5	16.0	5.33	1.08	92.3
				Double, Clear	W	1.5	6.5	16.0	10.77	1.02	175.6
				Double, Clear	S	1.5	5.5	30.0	4.03	1.15	138.7
				Double, Clear	W	1.5	5.5	20.0	10.77	1.03	221.4
				Double, Clear	W	1.5	5.0	16.0	10.77	1.03	178.2
				As-Built Total:				**351.7**			**2951.4**

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	197.0	3.60	709.2	Frame, Wood, Adjacent	13.0	197.0	3.30	650.1
Exterior	1554.0	3.70	5749.8	Frame, Wood, Exterior	13.0	1554.0	3.40	5283.6
Base Total:	**1751.0**		**6459.0**	**As-Built Total:**		**1751.0**		**5933.7**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	20.0	11.50	230.0	Exterior Wood	40.0	12.30	492.0
Exterior	40.0	12.30	492.0	Adjacent Wood	20.0	11.50	230.0
Base Total:	**60.0**		**722.0**	**As-Built Total:**	**60.0**		**722.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X	WCM =	Points
Under Attic	1718.0	2.05	3521.9	Under Attic	30.0	1718.0	2.05 X	1.00	3521.9
Base Total:	**1718.0**		**3521.9**	**As-Built Total:**		**1718.0**			**3521.9**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	194.0(p)	8.9	1726.6	Slab-On-Grade Edge Insulation	0.0	194.0(p	18.80	3647.2
Raised	0.0	0.00	0.0					
Base Total:			**1726.6**	**As-Built Total:**		**194.0**		**3647.2**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points				Area X	SPM	=	Points
	1718.0	10.21	17540.8				1718.0	10.21		17540.8
Summer Base Points:			**22603.8**	**Summer As-Built Points:**						**25234.2**
Total Summer Points	X System Multiplier		= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
22603.8	**0.4266**		**9642.8**	25234.2 **25234.2**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.341 **0.341**	1.000 **1.000**		9798.4 **9798.4**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1718.0	20.04	6197.2	Double, Clear	N	1.5	7.5	42.0	19.22	0.96	776.1	
				Double, Clear	N	9.0	10.0	13.3	19.22	0.73	186.2	
				Double, Clear	N	9.0	4.0	9.3	19.22	0.61	110.0	
				Double, Clear	N	1.5	5.5	17.5	19.22	0.93	312.2	
				Double, Clear	E	1.5	5.5	30.0	40.22	0.90	1081.5	
				Double, Clear	S	1.5	5.5	17.5	34.50	0.83	502.4	
				Double, Clear	S	1.5	6.5	72.0	34.50	0.88	2177.9	
				Double, Clear	SW	1.5	6.5	16.0	38.46	0.90	556.2	
				Double, Clear	S	1.5	6.5	36.0	34.50	0.88	1089.0	
				Double, Clear	SE	1.5	6.5	16.0	40.86	0.90	589.9	
				Double, Clear	W	1.5	6.5	16.0	36.99	0.93	548.7	
				Double, Clear	S	1.5	5.5	30.0	34.50	0.83	861.3	
				Double, Clear	W	1.5	5.5	20.0	36.99	0.90	663.5	
				Double, Clear	W	1.5	5.0	16.0	36.99	0.88	518.1	
				As-Built Total:				**351.7**			**9972.9**	
WALL TYPES	Area X	BSPM =	Points	Type		R-Value		Area X	SPM =		Points	
Adjacent	197.0	0.70	137.9	Frame, Wood, Adjacent		13.0		197.0	0.60		118.2	
Exterior	1554.0	1.70	2641.8	Frame, Wood, Exterior		13.0		1554.0	1.50		2331.0	
Base Total:	**1751.0**		**2779.7**	**As-Built Total:**				**1751.0**			**2449.2**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	Exterior Wood				40.0	6.10		244.0	
Exterior	40.0	6.10	244.0	Adjacent Wood				20.0	2.40		48.0	
Base Total:	**60.0**		**292.0**	**As-Built Total:**				**60.0**			**292.0**	
CEILING TYPES	Area X	BSPM =	Points	Type		R-Value		Area X	SPM X SCM =		Points	
Under Attic	1718.0	1.73	2972.1	Under Attic		30.0		1718.0	1.73 X 1.00		2972.1	
Base Total:	**1718.0**		**2972.1**	**As-Built Total:**				**1718.0**			**2972.1**	
FLOOR TYPES	Area X	BSPM =	Points	Type		R-Value		Area X	SPM =		Points	
Slab	194.0(p)	-37.0	-7178.0	Slab-On-Grade Edge Insulation		0.0		194.0(p	-41.20		-7992.8	
Raised	0.0	0.00	0.0									
Base Total:			**-7178.0**	**As-Built Total:**				**194.0**			**-7992.8**	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 82.2

The higher the score, the more efficient the home.

EWPL INC, Lot: 7, Sub: FW Park, Plat: , Fort White, FL, 32038-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	3	b.	N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1718 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	351.7 ft²	a.	Electric Heat Pump	Cap: 30.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 194.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 197.0 ft²			EF: 0.88
b.	Frame, Wood, Exterior	R=13.0, 1554.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1718.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 115.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-1824.

EnergyGauge® (Version: FLRCPB v3.2)

Residential System Sizing Calculation

Summary

EWPL INC

Fort White, FL 32038-

Project Title:
THE NICOLAS +

Code Only
Professional Version
Climate: North

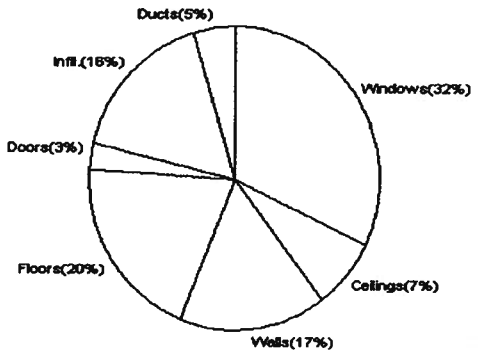
2/22/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**30737**	**Btuh**	**Total cooling load calculation**	**31071**	**Btuh**
Submitted heating capacity	30000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	97.6	%	Submitted as % of calculated	96.6	%

WINTER CALCULATIONS

Winter Heating Load (for 1718 sqft)

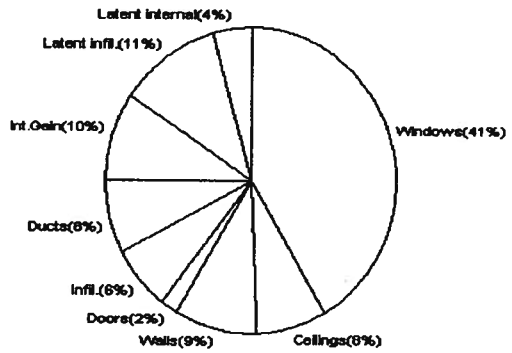
Load component			Load	
Window total	352	sqft	9952	Btuh
Wall total	1751	sqft	5133	Btuh
Door total	60	sqft	902	Btuh
Ceiling total	1718	sqft	2233	Btuh
Floor total	194	ft	6130	Btuh
Infiltration	115	cfm	4923	Btuh
Subtotal			**29273**	**Btuh**
Duct loss			1464	Btuh
TOTAL HEAT LOSS			**30737**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1718 sqft)

Load component			Load	
Window total	352	sqft	12891	Btuh
Wall total	1751	sqft	2909	Btuh
Door total	60	sqft	599	Btuh
Ceiling total	1718	sqft	2440	Btuh
Floor total			0	Btuh
Infiltration	100	cfm	1988	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**23826**	**Btuh**
Duct gain			2383	Btuh
Total sensible gain			**26209**	**Btuh**
Latent gain(infiltration)			3482	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4862**	**Btuh**
TOTAL HEAT GAIN			**31071**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: _____
DATE: 2-21-06

Manual J Winter Calculations

Residential Load - Component Details (continued)

EWPL INC

Fort White, FL 32038-

Project Title:
THE NICOLAS +

Code Only
Professional Version
Climate: North

2/22/2006

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Winter

Residential Load - Component Details

EWPL INC

Fort White, FL 32038-

Project Title:
THE NICOLAS +

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

2/22/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	42.0	28.3	1189 Btuh
2	2, Clear, Metal, DEF	N	13.3	28.3	377 Btuh
3	2, Clear, Metal, DEF	N	9.3	28.3	264 Btuh
4	2, Clear, Metal, DEF	N	17.5	28.3	495 Btuh
5	2, Clear, Metal, DEF	E	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	S	17.5	28.3	495 Btuh
7	2, Clear, Metal, DEF	S	72.0	28.3	2038 Btuh
8	2, Clear, Metal, DEF	SW	16.0	28.3	453 Btuh
9	2, Clear, Metal, DEF	S	36.0	28.3	1019 Btuh
10	2, Clear, Metal, DEF	SE	16.0	28.3	453 Btuh
11	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
12	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
13	2, Clear, Metal, DEF	W	20.0	28.3	566 Btuh
14	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
	Window Total		352		9952 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	197	1.6	315 Btuh
2	Frame - Exterior	13.0	1554	3.1	4817 Btuh
	Wall Total		1751		5133 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		40	17.9	718 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
	Door Total		60		902Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1718	1.3	2233 Btuh
	Ceiling Total		1718		2233Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	194.0 ft(p)	31.6	6130 Btuh
	Floor Total		194		6130 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	17180(sqft)	115	4923 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			115	4923 Btuh

Totals for Heating	**Subtotal**	**29273 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1464 Btuh**
	Total Btuh Loss	**30737 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

EWPL INC

Fort White, FL 32038-

Project Title:
THE NICOLAS +

Code Only
Professional Version
Climate: North

2/22/2006

Totals for Cooling	**Subtotal**	**23826**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**2383**	**Btuh**
	Total sensible gain	**26209**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3482**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**31071**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

EWPL INC

Project Title:
THE NICOLAS +

Code Only
Professional Version
Climate: North

Fort White, FL 32038-

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 2/22/2006

Window	Type		Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	7.5	42.0	0.0	42.0	22	22	924	Btuh
2	2, Clear, DEF, N, N	N	9	10	13.3	0.0	13.3	22	22	293	Btuh
3	2, Clear, DEF, N, N	N	9	4	9.3	0.0	9.3	22	22	205	Btuh
4	2, Clear, DEF, N, N	N	1.5	5.5	17.5	0.0	17.5	22	22	385	Btuh
5	2, Clear, DEF, N, N	E	1.5	5.5	30.0	2.2	27.8	22	72	2048	Btuh
6	2, Clear, DEF, N, N	S	1.5	5.5	17.5	17.5	0.0	22	37	385	Btuh
7	2, Clear, DEF, N, N	S	1.5	6.5	72.0	36.0	36.0	22	37	2124	Btuh
8	2, Clear, DEF, N, N	SW	1.5	6.5	16.0	5.4	10.6	22	62	776	Btuh
9	2, Clear, DEF, N, N	S	1.5	6.5	36.0	36.0	0.0	22	37	792	Btuh
10	2, Clear, DEF, N, N	SE	1.5	6.5	16.0	5.4	10.6	22	62	776	Btuh
11	2, Clear, DEF, N, N	W	1.5	6.5	16.0	2.0	14.0	22	72	1053	Btuh
12	2, Clear, DEF, N, N	S	1.5	5.5	30.0	30.0	0.0	22	37	660	Btuh
13	2, Clear, DEF, N, N	W	1.5	5.5	20.0	1.5	18.5	22	72	1366	Btuh
14	2, Clear, DEF, N, N	W	1.5	5	16.0	1.0	15.0	22	72	1103	Btuh
	Window Total				352					12891	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Adjacent	13.0	197.0	1.0	205	Btuh
2	Frame - Exterior	13.0	1554.0	1.7	2704	Btuh
	Wall Total		1751.0		2909	Btuh

Doors	Type	Area	HTM	Load	
1	Wood - Exter	40.0	10.0	399	Btuh
2	Wood - Adjac	20.0	10.0	200	Btuh
	Door Total	60.0		599	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1718.0	1.4	2440	Btuh
	Ceiling Total		1718.0		2440	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	194.0 ft(p)	0.0	0	Btuh
	Floor Total		194.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	17180	100.4	1988	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			100	1988	Btuh

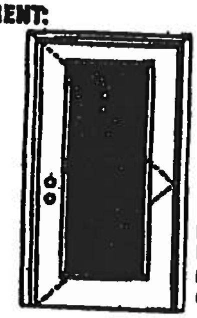
Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	1200	3000	Btuh

X
Glazed Inswing Unit

COP-WL-EN4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #30204470 and COP/Test Report Validation Matrix #30204470-001 provide additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+50.5/-60.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.
Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

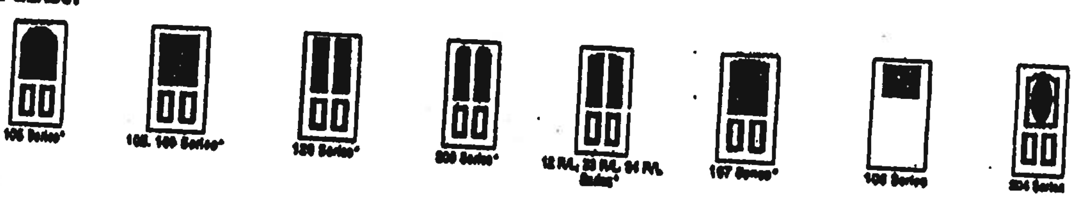
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:
1/4 GLASS:

1/2 GLASS:



*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Entergy
Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR Premium Quality Doors
Exclusively from Masonite
Masonite International Corporation

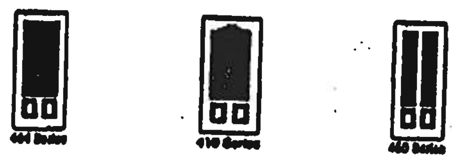
X
Glazed Inswing Unit

COP-WL-FN4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:



FULL GLASS:

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portnoy, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top and rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #0023447C and COP/Test Report Validation Matrix #0023447C-001 provide additional information - available from the ITS/WH website (www.etl-semko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Entergy
Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

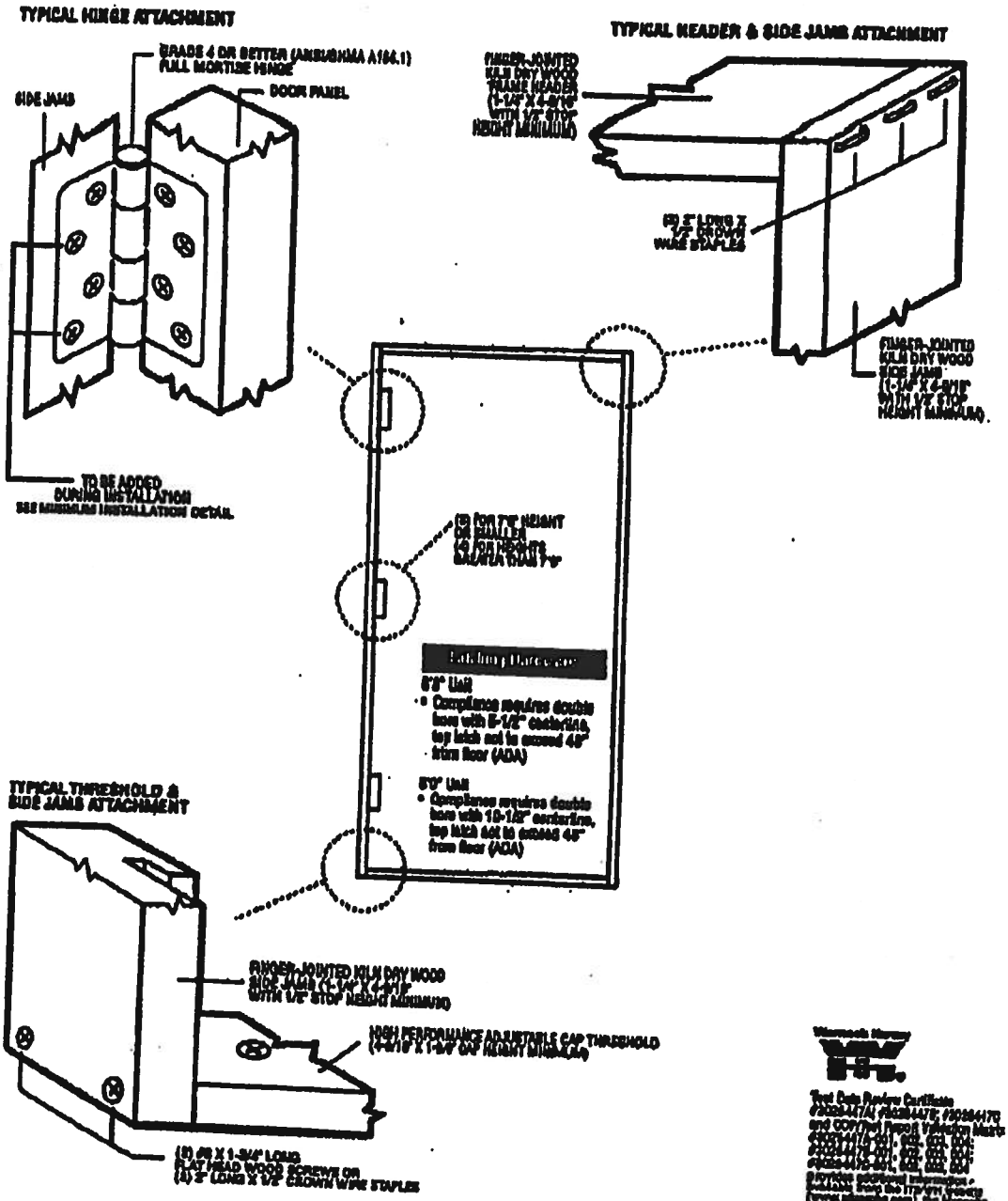
PREMDOOR Entries
Premium Quality Doors

Exclusively from
Masonite
Masonite International Corporation

X
Unit

MAD-WI-MA0001-02

INSWING UNIT WITH SINGLE DOOR



Test Data Review Certificate

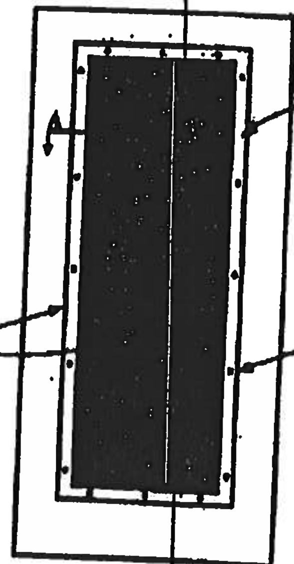
1

October 14, 2003

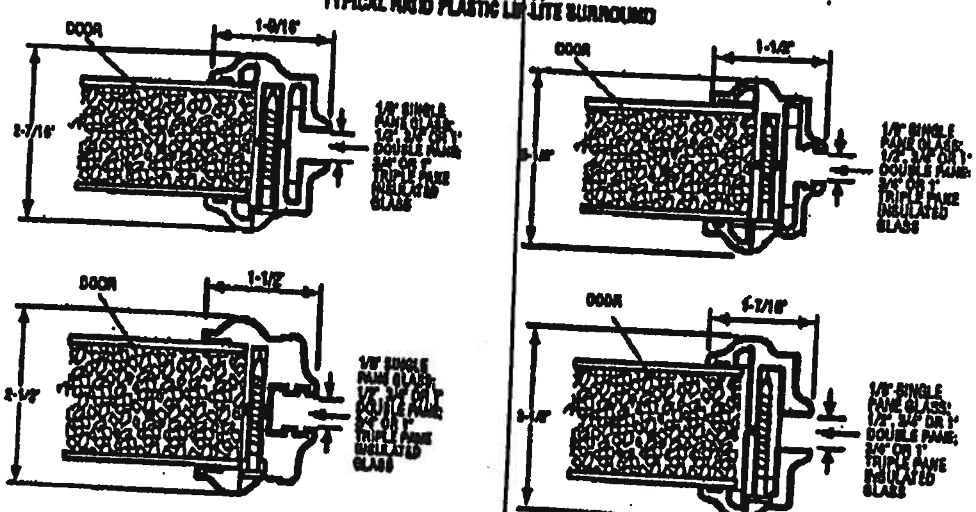
Masonite

MAD-WL-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND

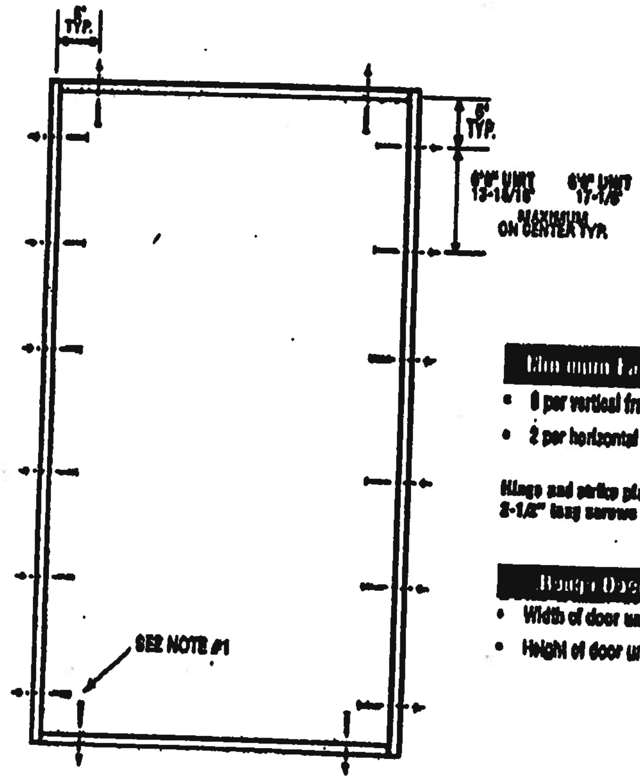


*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3026447A, #3026447B, #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8245*, 8255*, 8241*, 8248, 8251* or 8258
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

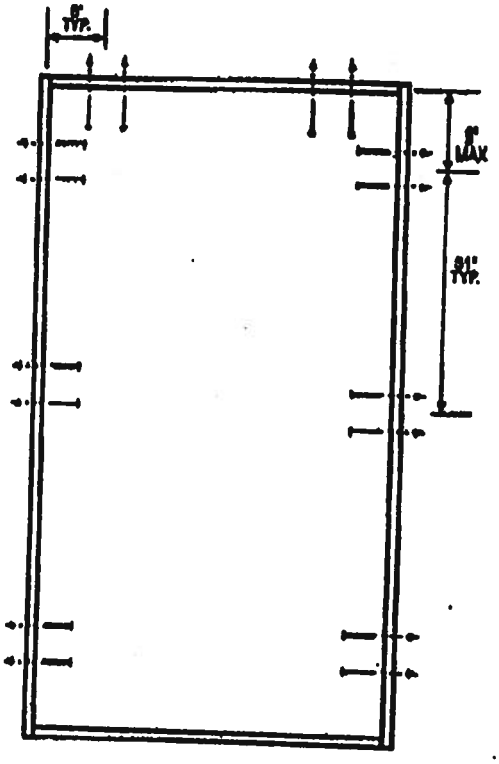
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite®

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 8 per vertical framing member for 70" height and smaller
- 8 per vertical framing member for heights greater than 70"
- 4 per horizontal framing member

Hinge and strike plates require two 3-1/2" long screws per location.

Rough Opening (RO):

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey Test Data Review Certificate [illegible] and CGP/Test Report Validation Matrix [illegible] - available from the ITS/WH website (www.[illegible].com), the Masonite website (www.masonite.com) or the Warnock Hersey system.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 6249*, 6288*, 3241*, 3248, 3291* or 3296
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 400 (or equal structural adhesive).
2. The common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

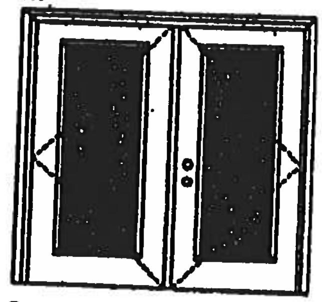
Masonite

XX
Glazed Outswing Unit

COP-WL-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447C and COP/Test Report Validation Matrix #3026447C-001 provide additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure
+50.5/-50.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.
Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

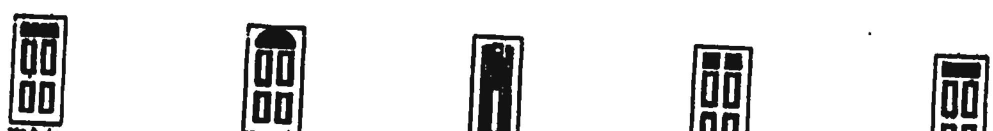
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed -- see MAD-WL-MA0012-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed -- see MID-WL-MA0002-02.

APPROVED DOOR STYLES:
1/4 GLASS:



100 Series | 133, 136 Series | 138 Series | 880 Series | 822 Series

1/2 GLASS:

103 Series* | 106, 160 Series* | 120 Series* | 200 Series* | 12 RL, 23 RL, 24 RL Series* | 167 Series* | 108 Series | 304 Series

*This glass kit may also be used in the following door styles: 6-panel, 6-panel with scroll, Eyebrow 6-panel, Eyebrow 6-panel with scroll.

Entergy
Entry Systems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Premdor — Exclusively from Masonite®
Masonite International Corporation

XX

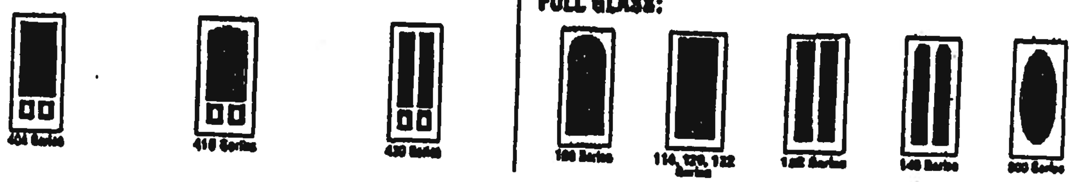
Glazed Outswing Unit

COP-WI-FN4162-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS: **FULL GLASS:**



404 Series | 416 Series | 430 Series | 100 Series | 114, 120, 132 Series | [illegible] Series | 140 Series | 300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Door panels constructed from 24-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.032" steel. Bottom end rails constructed of 0.032" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum bumper threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH MIAMI-DADE BCCO PA202

COMPANY NAME

CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L. Balthazor (signature)

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

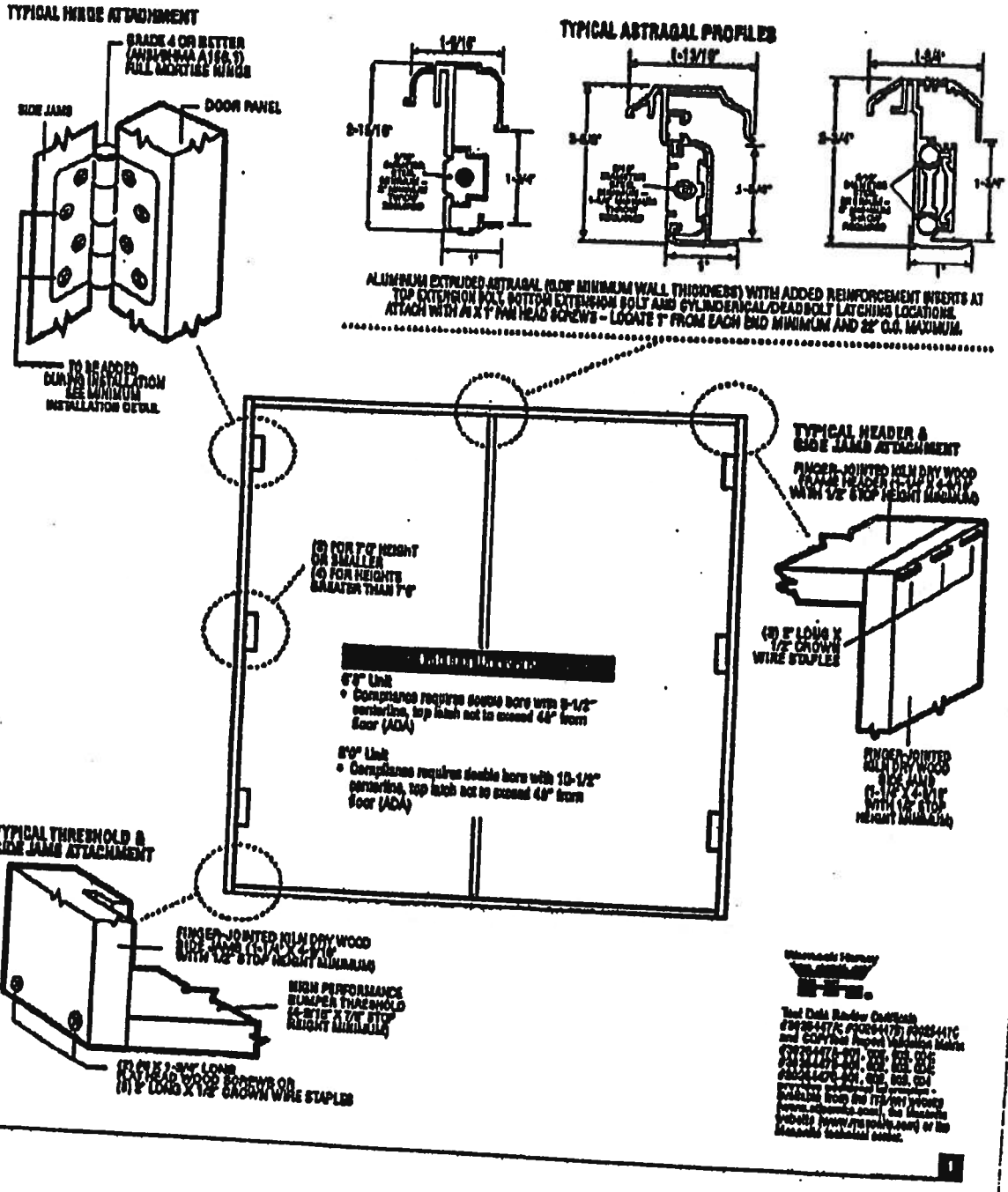
Entergy
Entry Systems
June 17, 2002

PREMDOR Collection
Exclusively from Masonite
Masonite International Corporation

XX
Unit

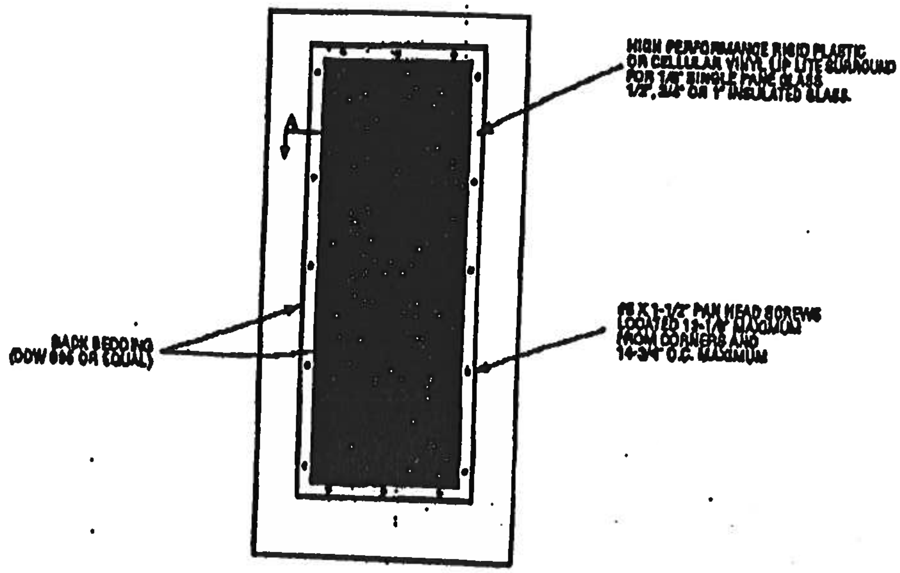
MAD WL-MA0012-02

OUTSWING UNITS WITH DOUBLE DOOR

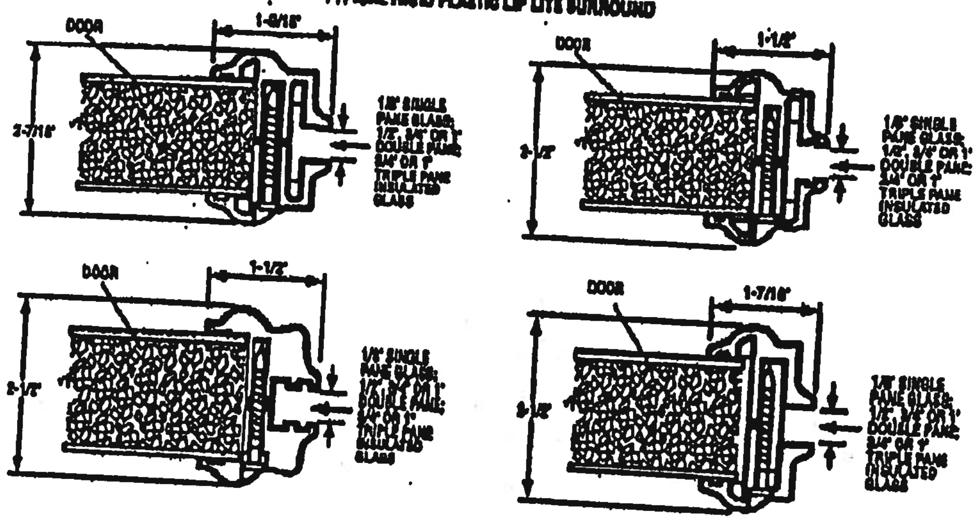


MAD-WI-MA0041-02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



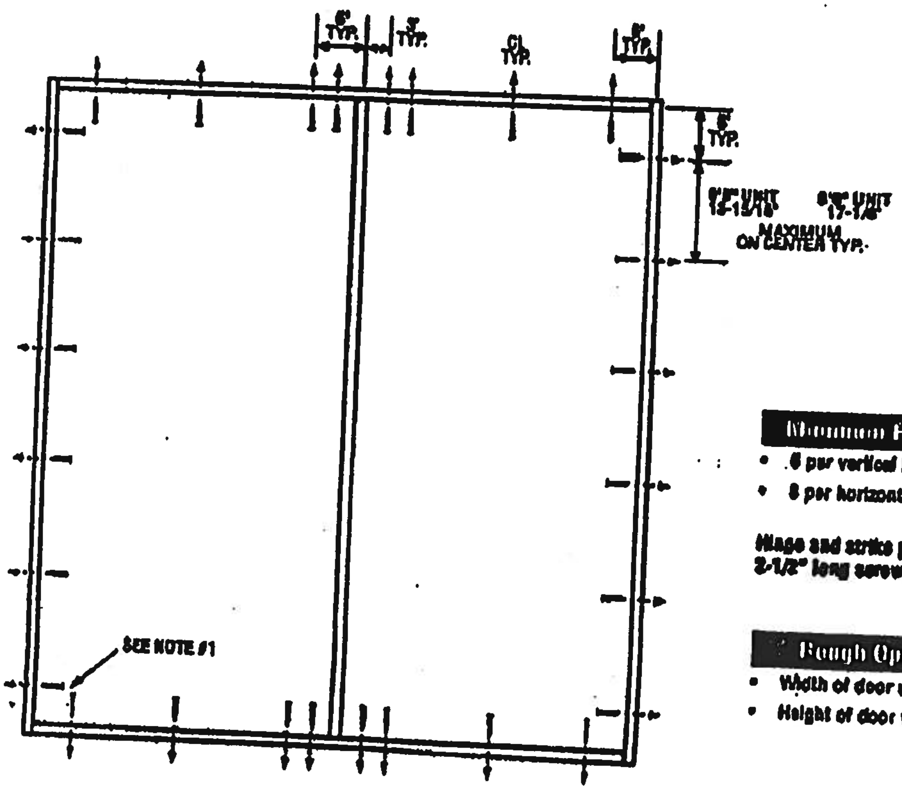
*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

June

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Maximum Fastener Count
- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)
- Width of door unit plus 1/2"
- Height of door unit plus 1/4".

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix [illegible] additional information - available from the [illegible] website [illegible], the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8287*, 8242*, 8247, 8282* or 8287
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

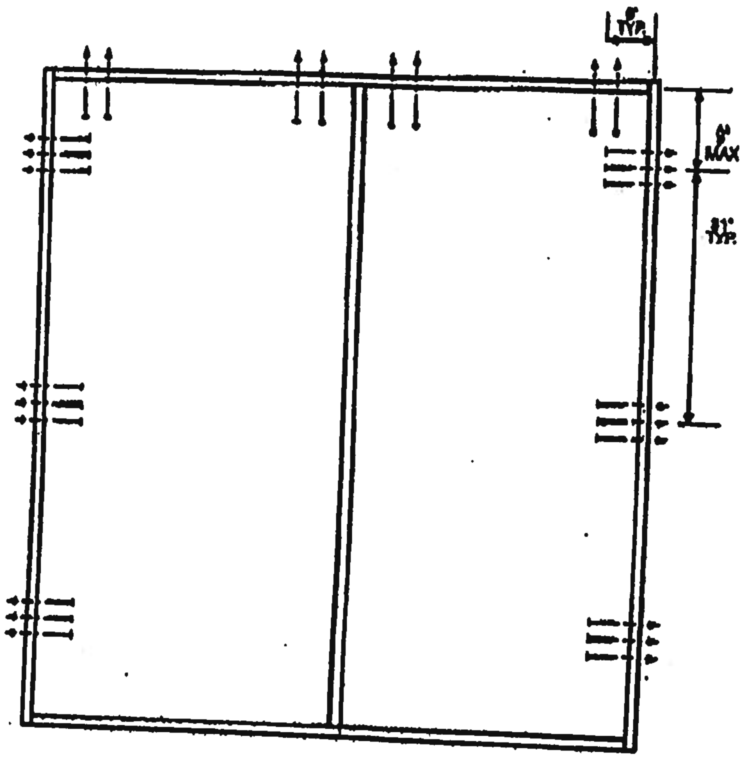
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Masonite

XX
Unit

MID WL MA0002 02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member for 7'0" heights and smaller
- 8 per vertical framing member for heights greater than 7'0"
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #30264474-001, 002, 003, 004; #30264478-001, 002, 003, 004; #30264470-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- UNITS COVERED BY COP DOCUMENT 8247*, 8257*, 3242*, 3247, 3252* or 3257
Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 wood screws and 10d common nails. Threshold fasteners analyzed for this unit include Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw and common nail single shear design values come from ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment of 1-1/4".
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 18, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite

MI Home Products, Inc.
650 West Market St.
P.O. Box 370
Gratz, PA 17030-0370

(717) 365-3300
(717) 362-7025 Fax

740/744 Single Hung (Fin & Flange)
165 Single Hung (Fin & Flange)
BB165/740/744 Fixed (Fin & Flange)

- Test Reports
  - 165 Single Hung
    - #CTLA-787W (Fin)
    - #CTLA-787W-1 (Flange)
  - 740/744 Single Hung
    - #01-40351.03 (Fin)
    - #01-40351.04 (Flange)
  - 165/740/744 Fixed
    - #NCTL-310-0005-2.1 (Fin)
    - # NCTL-310-0005-5.1 (Flange)
    - #01-40486.03 (2-Panel Fixed)
- Installation Instructions
- Sample 110/120/140 MPH Labels

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 740/744
TYPE: Aluminum Single Hung Window with Nail Fin

Title of Test	Results
Rating	H R45 52 x 72
Overall Design Pressure	45 psf
Operating Force	24 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance	6.75 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-40351.03 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:baw

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF FLORIDA
PROFESSIONAL ENGINEER

THIS FENESTRATION PRODUCT COMPLIES* WITH THE

NEW FLORIDA BUILDING CODE

FOR RESIDENTIAL BUILDINGS WITH A MEAN ROOF HEIGHT OF 30 FT. OR LESS, *EXPOSURE "B"* (WHICH IS INLAND OF A LINE THAT IS 1500 FT. FROM THE COAST), AND *WALL ZONE "5"* (INSTALLED NEAR THE CORNER OF THE BUILDING).

PER *ASTM E1300*, THE CORRECT GLASS THICKNESS, BASED ON THE *NEGATIVE* DESIGN PRESSURE (DP) LISTED BELOW, HAS BEEN INSTALLED IN THIS UNIT. THE GLASS THICKNESS IS BASED ON ITS' WIDTH, HEIGHT, AND ASPECT RATIO.

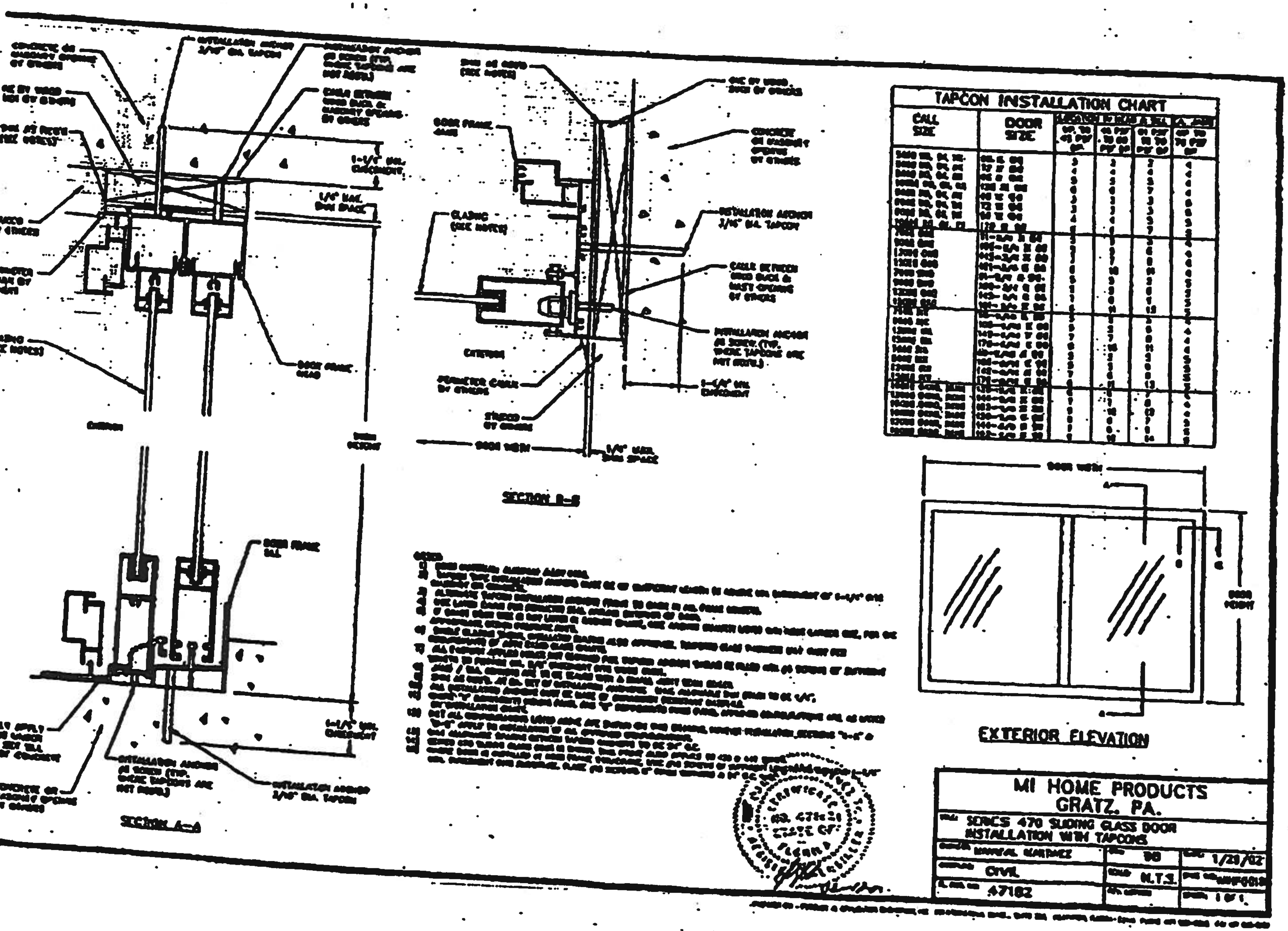
Series 470HP SLIDING GLASS DOOR – all 6'- 8" High Panels

- 2'- 6" WIDE DP +40.0 / -55.4
- 3'- 0" WIDE DP +40.0 / -48.5
- 4'- 0" WIDE DP +40.0 / -40.3

THIS PRODUCT MEETS THE REQUIREMENTS FOR STRUCTURAL LOADS, WATER AND AIR INFILTRATION PER ATTACHED *AAMA* PERFORMANCE LABEL. BE ADVISED THAT IF LOADS ARE PLACED UP TO OR EXCEEDING THE TESTED LEVELS, THIS PRODUCT MAY BE ALTERED IN SUCH A WAY THAT FUTURE PERFORMANCE WILL BE REDUCED.

* COMPLIANCE MUST INCLUDE INSTALLATION ACCORDING TO MANUFACTURER'S INSTRUCTIONS AND FLORIDA CODE REQUIREMENTS.

MIP-686


TAPCON INSTALLATION CHART
CALL SIZE
DOOR SIZE
SECTION A-A
SECTION B-B
EXTERIOR ELEVATION
MI HOME PRODUCTS
GRATZ, PA.
SERIES 470 SLIDING GLASS DOOR
INSTALLATION WITH TAPCONS
CIVIL
47182
N.T.S.
1/29/02
1 OF 1

TOTAL P.04

DOCUMENT CONTROL ADDENDUM #01-40351.00

Current Issue Date: 02/15/02

Report No.: 01-40351.01

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 744 aluminum single hung window with flange.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

Report No.: 01-40351.02

Requested by: William Emley, MI Home Products, Inc.
Purpose: Change of glass type.
Issued Date: 12/28/01
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories.

Report No.: 01-40351.03

Requested by: William Emley, MI Home Products, Inc.
Purpose: AAMA/NWWDA 101/I.S.2-97 testing of Series/Model 740/744 aluminum single hung window with nail fin.
Issued Date: 02/15/02
Comments: Florida P.E. seal required on report.
Certification copy to John Smith at Associated Laboratories, Inc.

ALLEN N. REEVES
CERTIFICATE NO. 13354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
15 FEBRUARY 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meting rail) (Loads were held for 52 seconds)		
	@ 45.0 psf (positive)	0.91"*	0.29" max.
	@ 45.0 psf (negative)	0.97"*	0.29" max.
4.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads held for 10 seconds)		
	@ 67.5 psf (positive)	0.14"	0.20" max.
	@ 67.5 psf (negative)	0.19"	0.20" max.
4.4.2	@ 70.8 psf (negative)	0.20"	0.20" max.

** Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:baw
01-40351.03

Allen N. Reeves, P.E.
Director - Engineering Services
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into the #2 2 x 8 Spruce-Pine-Fir wood buck with 1" galvanized roofing nails through the nail fin every 8" on center. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.30 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-96) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.86"*	0.29" max.
	@ 15.0 psf (negative)	0.81"*	0.29" max.

*Note: * Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.20" max.
	@ 22.5 psf (negative)	<0.01"	0.20" max.
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction at 70 lbs		
	Top rail	0.06"/12%	0.50"/100%
	Bottom rail	0.06"/12%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.03"/6%	0.50"/100%
	Right stile	0.03"/6%	0.50"/100%

Allen N. Reeves
15 FEBRUARY 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.330" high by 0.187" backed polypile with center fin	1 Row	Fixed meeting rail interlock
0.170" high by 0.187" backed polypile with center fin	1 Row	Fixed lite, stiles and top rail
3/8" diameter hollow bulb gasket	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash stiles
0.150" high by 0.187" wide polypile	1 Row	Active sash stiles

Frame Construction: All frame members were constructed of extruded aluminum with coped, butted and sealed corners fastened with two screws each. Fixed meeting rail was secured utilizing one screw in each end directly through exterior face into jamb. Silicone was utilized around exterior meeting rail/jamb joinery.

Sash Construction: All sash members were constructed of extruded aluminum with coped and butted corners fastened with one screw each.

Screen Construction: The screen frame was constructed from roll-formed aluminum members with plastic keyed corners. The screening consisted of a fiberglass mesh and was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Plastic tilt latch	2	One each end of the interior Meeting rail
Metal sweep lock	2	13" from meeting rail ends
Balance assembly	2	One per jamb
Screen tension spring	2	One per end of screen stile
Tilt pin	2	One each end of bottom rail

Allen N. Reeves
15 FEBRUARY 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19356
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-40351.03
Test Dates: 10/22/01
And: 10/23/01
Report Date: 02/15/02
Expiration Date: 10/23/05

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness performance testing on a Series/Model 740/744, aluminum single hung window at MI Home Products, Inc.'s test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R45 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 740/744

Type: Aluminum Single Hung Window With Nail Fin

Overall Size: 4' 4-1/8" wide by 5' 11-5/8" high

Active Sash Size: 4' 2-3/4" wide by 2' 11-5/8" high

Fixed Daylight Opening Size: 4' 1-1/8" wide by 2' 9" high

Screen Size: 4' 1-7/8" wide by 2' 11-5/16" high

Finish: All aluminum was polished.

Glazing Details: The active sash and fixed lite were glazed with one sheet of 1/8" thick clear tempered glass. Each sash was channel glazed using a flexible vinyl gasket.

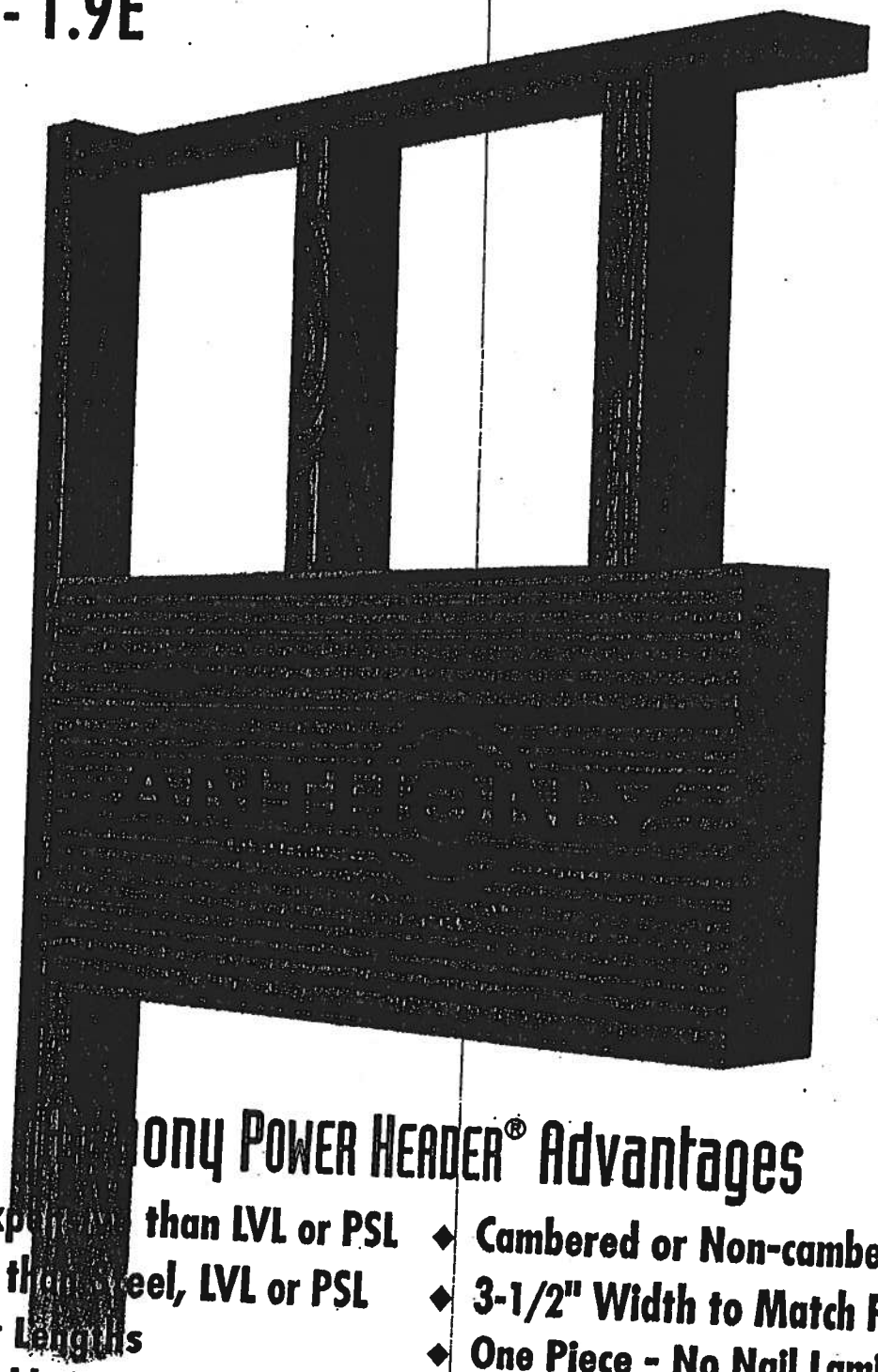
ALLEN M. REEVES ★ CERTIFICATE NO. 1935 ★ STATE OF FLORIDA ★ PROFESSIONAL ENGINEER ★

Allen M. Reeves

130 Derry Court
York, PA 17402-9405
Phone: 717.764.7700
fax: 717.764.4129
www.testati.com

Anthony POWER HEADER®

2600F_b - 1.9E



Anthony POWER HEADER® Advantages

- Less Expensive than LVL or PSL
- Lighter than Steel, LVL or PSL
- Pre-Cut Lengths
- Renewable Resource
- Cambered or Non-cambered
- 3-1/2" Width to Match Framing
- One Piece - No Nail Laminating
- Lifetime Warranty

Garage Header Sizing Tables

ANTHONY®
ANTHONY FOREST PRODUCTS CO.

Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER APPLICATION - SINGLE STORY

HEADER SUPPORTING: 1/2 ROOF SPAN

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	16-3/4
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	9-3/4	15-3/8	
8-3/8	12-5/8	14	8-3/8	12-5/8	15-3/8	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8		9-3/4		
8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	16-3/4	9-3/4	15-3/8		9-3/4		
8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4		
8-3/8	14	15-3/8	8-3/8	15-3/8		8-3/8	15-3/8		9-3/4			9-3/4			11-1/4		
8-3/8	14	16-3/4	8-3/8	15-3/8		9-3/4	15-3/8		9-3/4			9-3/4			11-1/4		

9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"	9'-3"	16'-3"	18'-3"
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14
8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	11-1/4	12-5/8	8-3/8	12-5/8	14	8-3/8	12-5/8	14
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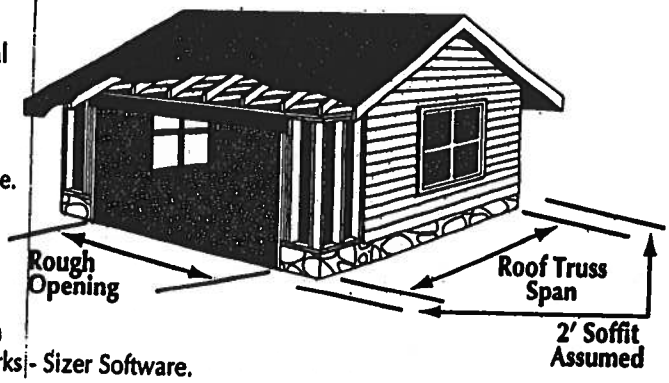
8-3/8	12-5/8	14	8-3/8	12-5/8	14	8-3/8	14	15-3/8	8-3/8	14	15-3/8	8-3/8	15-3/8	

NOTES:

1. Table assumes a simple span header supporting a uniform load transferred from 1/2 the roof span plus a 2' soffit.
2. Roof live and dead loads shown are applied vertically to the horizontal projection. No reductions in roof live loads or snow loads were considered. The header weight is accounted for in the table.
3. Deflection is limited to L/240 for live load and L/180 for total load.
4. Headers are assumed to have continuous lateral support along top edge.
5. Bearing length based on full width bearing is indicated as follows:
   Non-shaded sizes require two trimmers (3" bearing).
   Shaded sizes require three trimmers (4.5" bearing).
   Shaded & outlined sizes require four trimmers (6" bearing).
6. ** Applications where load carrying capacity of 16-3/4" depth has been exceeded. See AFP $30F_b$ POWER BEAM® literature or AFP's WoodWorks - Sizer Software.



Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582					
107	153	169	245	260	380	368	540	501	715	664	864	840
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Anthony Power Header®

$26F_b$ - 1.9E

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v as outlined in AITC 117 - Design 1993 and the NDS for Wood Construction 1997.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for Power Header®.

GARAGE HEADER COMPARISONS

810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"***
990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"***
640 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"**
765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
750 / 480	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	-----

For more information on Power Header®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com

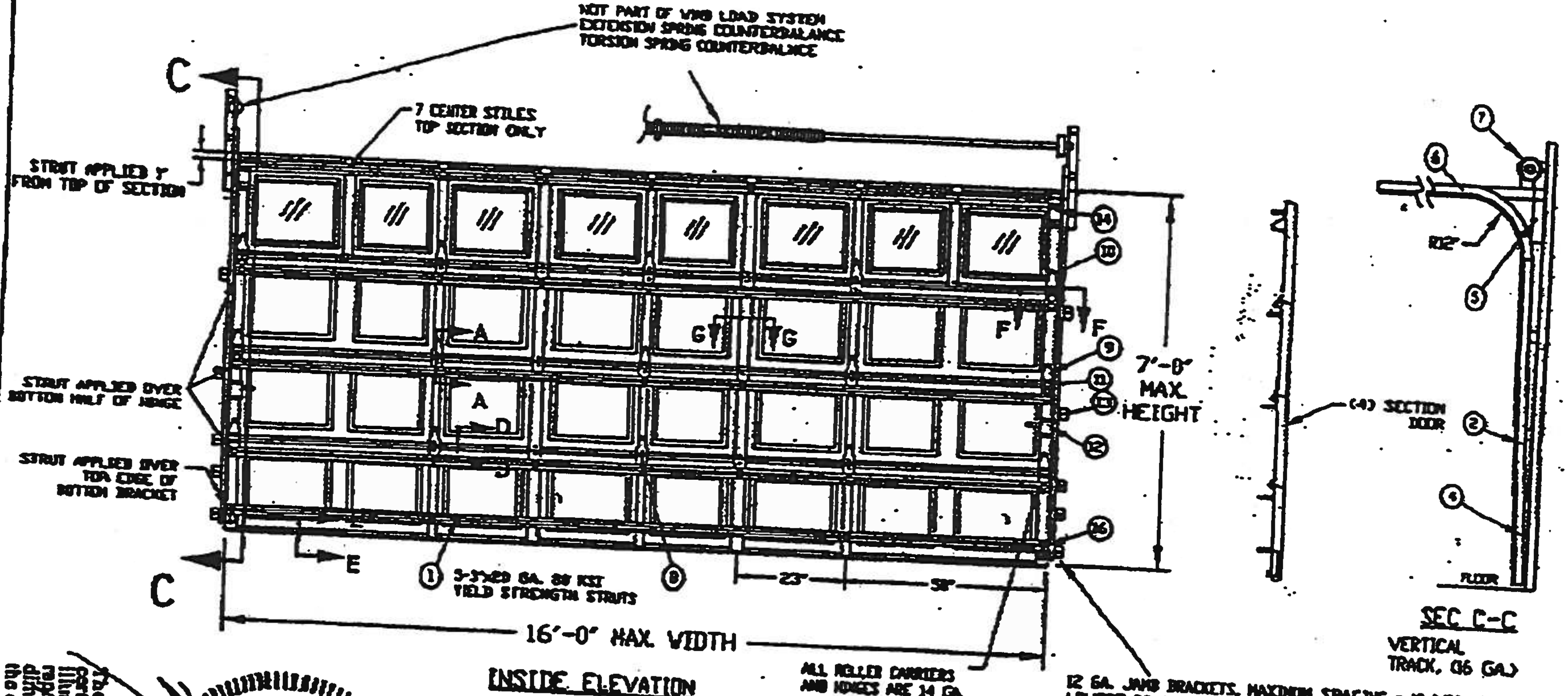
Distributed by:

FROM :Columbia Door Company FAX NO. :386-754-9993 Apr. 28 2004 01:02PM P3

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 21"
3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 5½" (7" AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR SHOWN.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR (2) AS TESTED
△ 9. DROP IN TYPE OF INSULATION IS OPTIONAL.

REV.	DATE	BY	DESCRIPTION
A - A	01-10-00	DW	SEE E.C.N. 031



NORTH CAROLINA PROFESSIONAL ENGINEER SEAL PE No. 024280 KASER R. ARYVAN

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

LISTED SBCCI PST&ESS REPORT No. 2202

TEST REPORTS ON FILE VIDED 10/19/00 (00293)

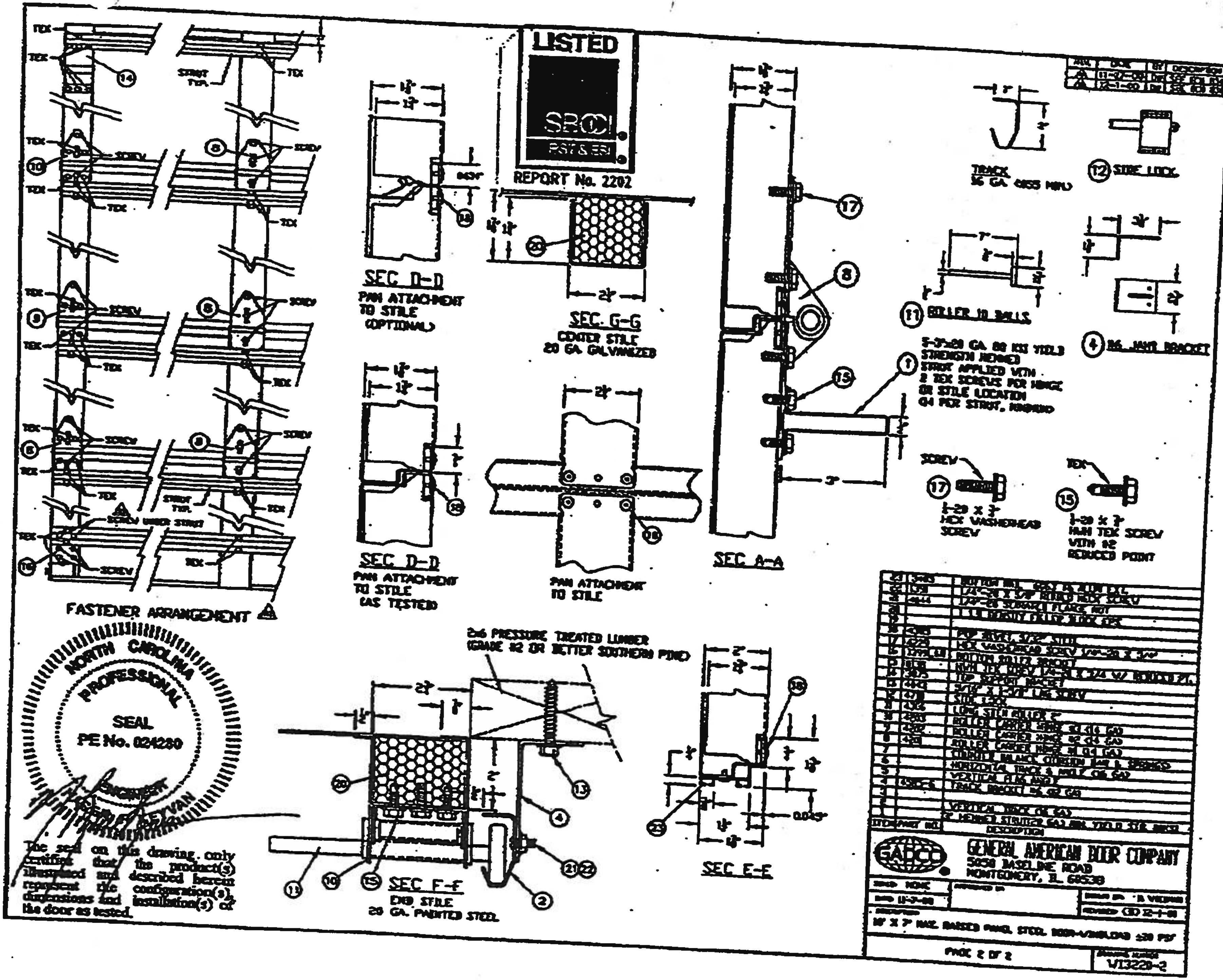
DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

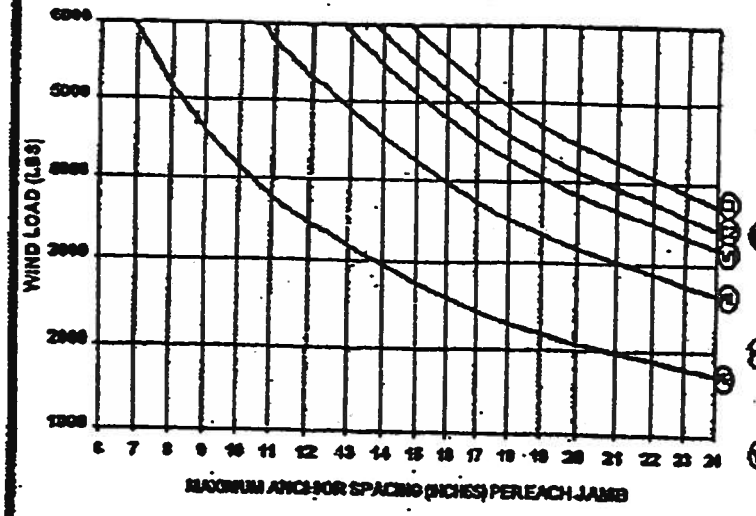
GADCO DOORS
SERIES 7400, EXTERIOR STEEL =.017 MIN (AS TESTED)
SERIES 7825, EXTERIOR STEEL =.019" MIN △
SERIES 7524, EXTERIOR STEEL =.024" MIN △
(TESTED WITH WINDOWS)

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	STRUTS 80 KSI SIZE	STRUTS 80 KSI QTY.	VERTICAL TRACK
16'	7'	23"	3"	5	2 IN.

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: NTS	APPROVED BY	DRAWN BY: S. WICKMAN
DATE: 10-20-00		REVISED (A) 11-10-00
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR -WINDLOAD ±20 PSF		
PART NUMBER	PAGE 1 OF 2	DRAWING NUMBER: V13220-1


FASTENER ARRANGEMENT
LISTED
SBCCI
PST & ESI
REPORT No. 2202
SEC D-D
PAN ATTACHMENT TO STILE (OPTIONAL)
SEC D-D
PAN ATTACHMENT TO STILE (AS TESTED)
SEC G-G
CENTER STILE
20 GA. GALVANIZED
PAN ATTACHMENT TO STILE
SEC A-A
TRACK
16 GA. (.055 MIN)
SIDE LOCK
ROLLER 10 BALLS
16. JAMB BRACKET
1-20 x 3/4" HEX WASHERHEAD SCREW
1-20 x 3/4" HWH TEK SCREW WITH #2 REDUCED POINT
2x6 PRESSURE TREATED LUMBER (GRADE #2 OR BETTER SOUTHERN PINE)
SEC F-F
END STILE
20 GA. PAINTED STEEL
SEC E-E
NORTH CAROLINA PROFESSIONAL SEAL
PE No. 024280
ENGINEER
The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.
GADCO
GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538
PAGE 2 OF 2
VI3220-2



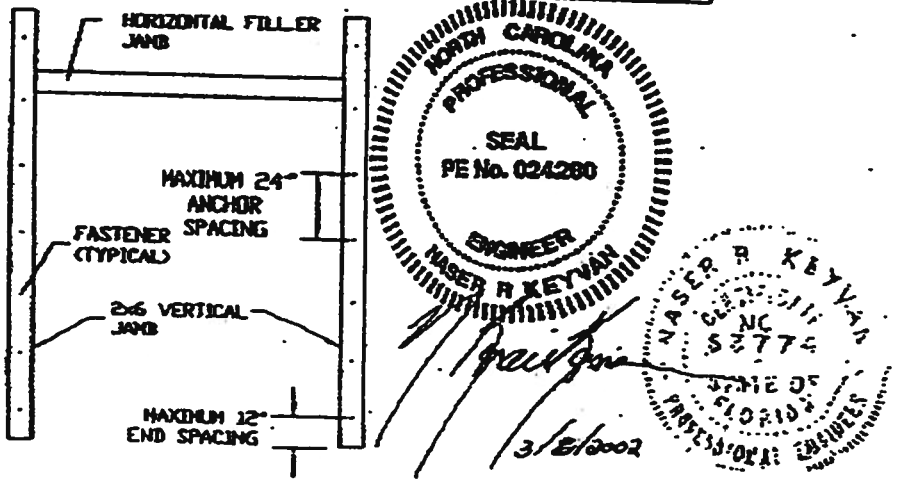
DESIGN LOAD (LBS/FT²) X GARAGE DOOR AREA(WIDTH-FT X HEIGHT-FT) = WIND LOAD(LBS)

EXAMPLE

30 LBS/FT² X 16 FT WIDE X 8 FT HIGH = 3840 LBS

(21) USE 22" SPACING (24) USE 16" SPACING
(22) USE 21" SPACING (25) USE 10" SPACING
(23) USE 19" SPACING

SEE NOTE 8 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2x6 PRESSURE TREATED (GRADE #2 OR BETTER SOUTHERN PINE) WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES:

1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTD 10," CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) WOOD FRAME BUILDINGS: STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE #2 GRADE OR BETTER) WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.

5) REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI; GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI; REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

6) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

7) ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4".

8) LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.

9) WASHERS ARE REQUIRED ON ALL FASTENERS.

10) THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.

11) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

ISSUED NONE	APPROVED BY	DRAWN BY DV
DATE 9-30-99		CHECKED
DESCRIPTION	JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS	
	DRAWING NUMBER A10560	REVISION NUMBER

ELK

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size 13¼"x 39¾"
Exposure 5⅝"
Pieces/Bundle 16
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 11

50-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Raised Profile

Product size 13¼"x 38¾"
Exposure 5⅝"
Pieces/Bundle 22
Bundles/Square 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique I *High Definition*

Product size 13¼"x 39¾"
Exposure 5⅝"
Pieces/Bundle 16
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 14

40-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

Prestique *High Definition*

Product size 13¼"x 38¾"
Exposure 5⅝"
Pieces/Bundle 22
Bundles/Square 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: non-prorated coverage for shingles and application labor for the initial 5 years, plus an option for transferability*; prorated coverage for application labor and shingles for balance of limited warranty period; 5-year limited wind warranty*.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6¼"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors: Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood**, Forest Green, Wedgewood**, Birchwood**, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge roofing products contain Elk WindGuard® sealant. WindGuard activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae. Not available in Sablewood.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Metro Dade building code requirements.

*See actual limited warranty for conditions and limitations.
**Check for product availability.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color).

MATERIALS: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For low-

warranties are contingent upon the correct installation as shown on the instructions. These instructions are the

Lot 7 FWP

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan Including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations Including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

a) Manual J sizing equipment or equivalent computation
b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
   (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
   A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

NOTICE:

ADDRESSES BY APPOINTMENT ONLY!

TO OBTAIN A 9-1-1 ADDRESS THE REQUESTER MUST CONTACT THE COLUMBIA COUNTY 9-1-1 ADDRESSING DEPARTMENT AT (386) 752-8787 FOR AN APPOINTMENT TIME AND DATE:

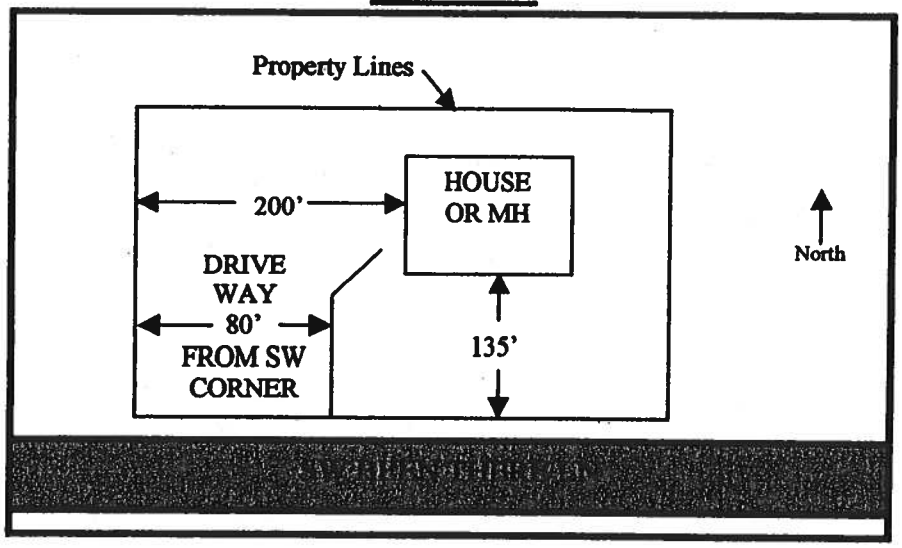
YOU CAN NOT OBTAIN A NEW ADDRESS OVER THE TELEPHONE. MUST MAKE AN APPOINTMENT!

THE ADDRESSING DEPARTMENT IS LOCATED AT 263 NW LAKE CITY AVENUE (OFF OF WEST U.S. HIGHWAY 90 WEST OF INTERSTATE 75 AT THE COLUMBIA COUNTY EMERGENCY OPERATIONS CENTER).

THE REQUESTER WILL NEED THE FOLLOWING:

1. THE PARCEL OR TAX ID NUMBER (SAMPLE: "25-4S-17-12345-123" OR "R12345-123) FOR THE PROPERTY.
2. A PLAT, PLAN, SITE PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
   a. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
   b. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
   c. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



NOTE: 5 TO 7 WORKING DAYS MAY BE REQUIRED IF ADDRESSING DEPARTMENT NEEDS TO CONDUCT AN ON SITE SURVEY.

Lot 7 Fort White Park

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

August 11, 2006

Permit # 24354

Building Department

Re: Job #603071, Ewpl Inc, Spec House Lot 7 Fort White Park S/D, Columbia County, Florida

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to use ½" AB with 7" embedment at 4'oc, SP4 stud straps top and bottom on same stud at 4'oc, and header straps per Simpson table in lieu of ½" A307 steel threaded rods with 6" embedment in foundation with Simpson AT and 2" x 2" x 1/8" washer on top plate, each side of each corner, each side of each opening, and 5'4"oc.

14 AUG 06

Mark Disosway, PE
Florida Registered Professional Engineer

Cc Builder

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 03-7S-16-04060-107 **Building permit No.** 000024354

Use Classification SFD,UTILITY **Fire:** 55.80

Permit Holder HUGO ESCALANTE **Waste:** 0.00

Owner of Building HUGO ESCALANTE SR **Total:** 55.80

Location: 235 SW BLUE JAY CT(FT WHITE PARK,LOT 7)

Date: 12/07/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L153960		
Builder:	HUGO ESCALANTE	Date:	3/6/2006
Lot:	LOT 7 FORT WHITE PARK	Start Number:	**1278**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	46		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**120**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ESCALANTE, HUGO CRC 1326967
Address: P.O. BOX 280
FORT WHITE, FL. 32038 Designer: **33**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0306061278	3/6/2006	41	T25	0306061318	3/6/2006
2	CJ3	0306061279	3/6/2006	42	T26	0306061319	3/6/2006
3	CJ4	0306061280	3/6/2006	43	T26G	0306061320	3/6/2006
4	CJ5	0306061281	3/6/2006	44	T27	0306061321	3/6/2006
5	EJ6	0306061282	3/6/2006	45	T27G	0306061322	3/6/2006
6	EJ7	0306061283	3/6/2006	46	T28	0306061323	3/6/2006
7	EJ7A	0306061284	3/6/2006				
8	EJ7B	0306061285	3/6/2006				
9	EJ7G	0306061286	3/6/2006				
10	EJ7T	0306061287	3/6/2006				
11	HJ4	0306061288	3/6/2006				
12	HJ7	0306061289	3/6/2006				
13	HJ9	0306061290	3/6/2006				
14	T01	0306061291	3/6/2006				
15	T02	0306061292	3/6/2006				
16	T03	0306061293	3/6/2006				
17	T03A	0306061294	3/6/2006				
18	T03G	0306061295	3/6/2006				
19	T05	0306061296	3/6/2006				
20	T06	0306061297	3/6/2006				
21	T07	0306061298	3/6/2006				
22	T08	0306061299	3/6/2006				
23	T09	0306061300	3/6/2006				
24	T10	0306061301	3/6/2006				
25	T11	0306061302	3/6/2006				
26	T12	0306061303	3/6/2006				
27	T13	0306061304	3/6/2006				
28	T13A	0306061305	3/6/2006				
29	T14	0306061306	3/6/2006				
30	T15	0306061307	3/6/2006				
31	T16	0306061308	3/6/2006				
32	T17	0306061309	3/6/2006				
33	T18	0306061310	3/6/2006				
34	T19	0306061311	3/6/2006				
35	T19A	0306061312	3/6/2006				
36	T20	0306061313	3/6/2006				
37	T21	0306061314	3/6/2006				
38	T22	0306061315	3/6/2006				
39	T23	0306061316	3/6/2006				
40	T24	0306061317	3/6/2006				

MAR 06 2006

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02:00:39 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**ESCALANTE, HUGO (Primary Name)**
	EWPL INC (DBA Name)
Main Address:	**P.O. BOX 280** **FORT WHITE, Florida 32038**

License Information

License Type:	**Certified Residential Contractor**
Rank:	**Cert Residental**
License Number:	**CRC1326967**
Status:	**Current, Active**
Licensure Date:	**11/24/2003**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Qualified Business License Required	11/24/2003

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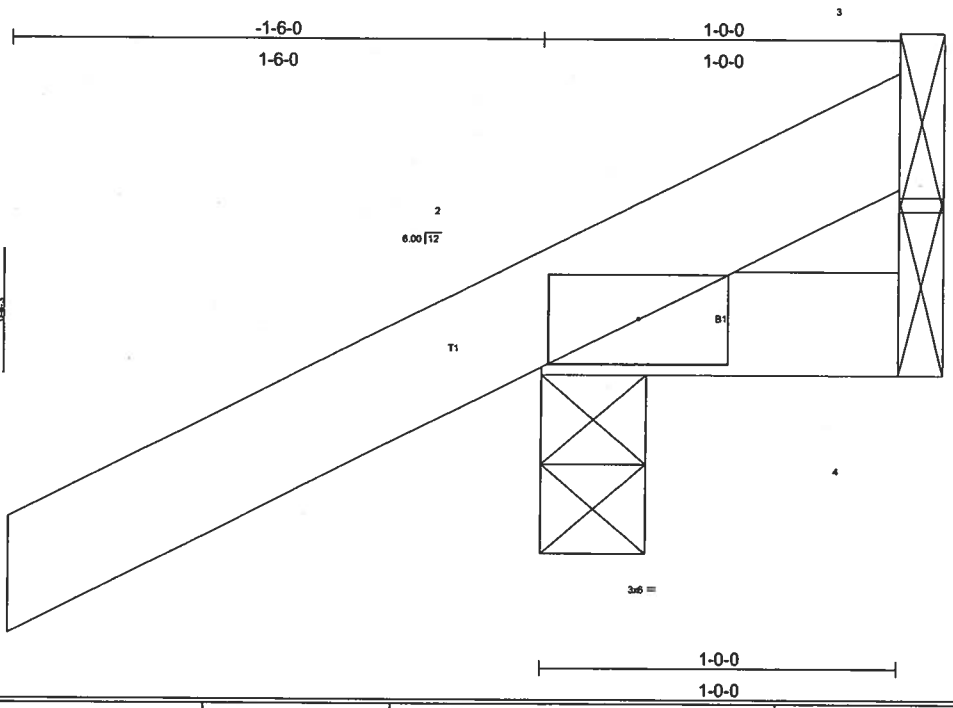
MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING [illegible]
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061278

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	CJ1	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:17 2006 Page 1



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TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 6 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=189/0-3-8, 4=14/Mechanical, 3=-40/Mechanical
Max Horz 2=84(load case 5)
Max Uplift2=-220(load case 5), 3=-40(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=73(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 2 and 40 lb uplift at joint 3.

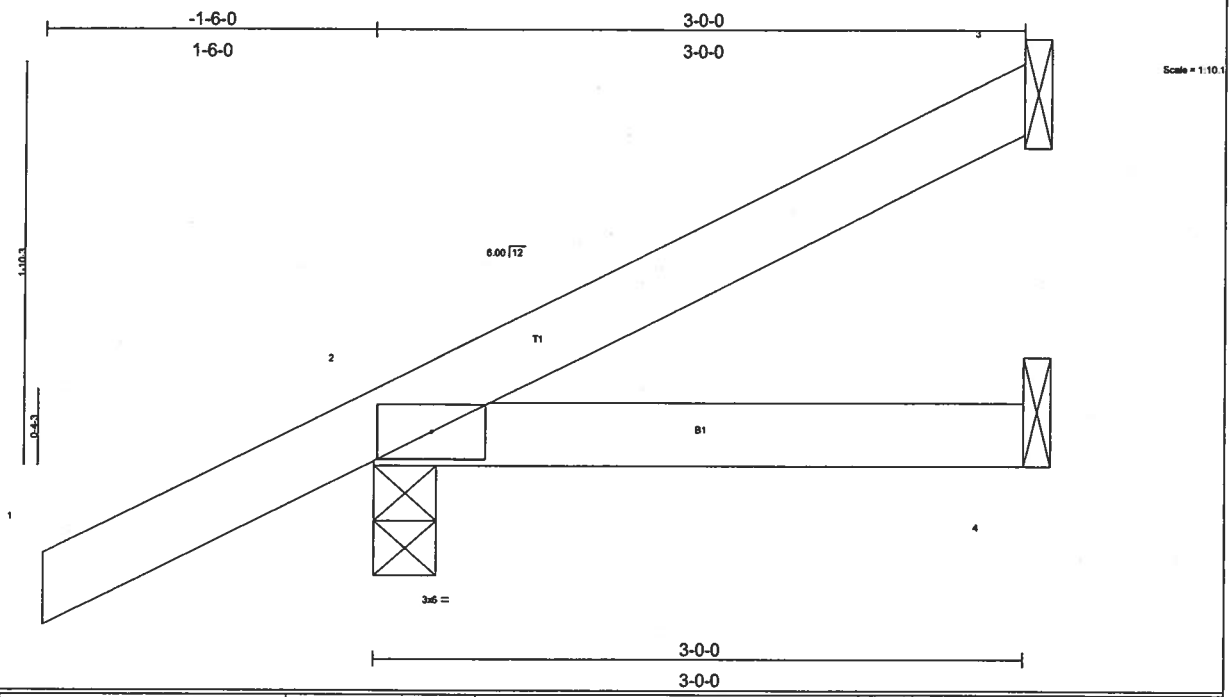
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061279

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	CJ3	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:17 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=137(load case 5)
Max Uplift3=-47(load case 5), 2=-187(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-52/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 3 and 187 lb uplift at joint 2.

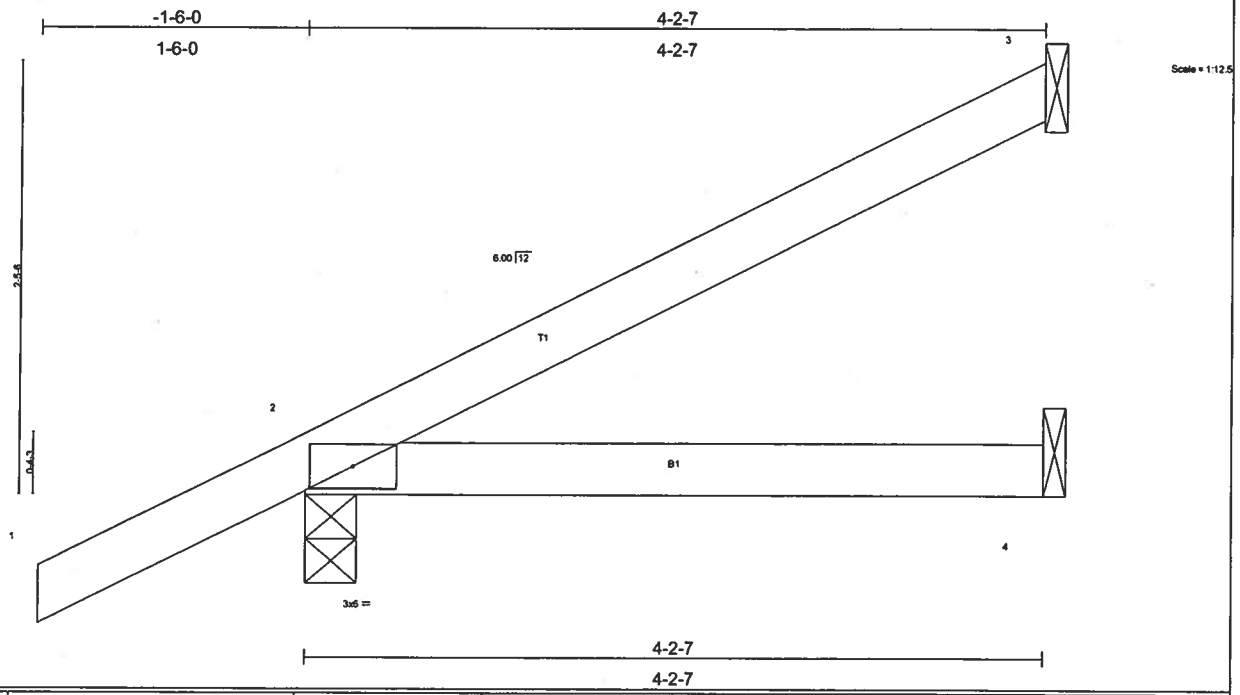
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061280

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	CJ4	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:18 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=90/Mechanical, 2=275/0-3-8, 4=60/Mechanical
Max Horz 2=170(load case 5)
Max Uplift3=-95(load case 5), 2=-191(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-89/32
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3 and 191 lb uplift at joint 2.

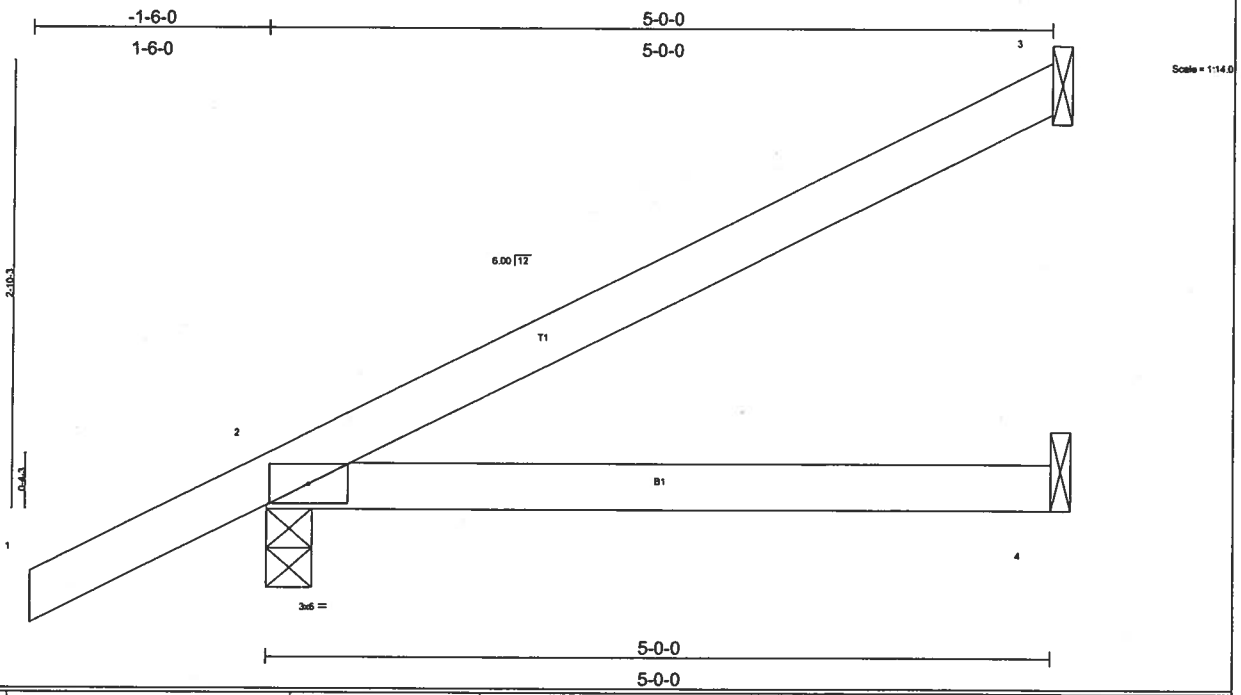
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061281

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	CJ5	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:18 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=192(load case 5)
Max Uplift3=-124(load case 5), 2=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-112/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 197 lb uplift at joint 2.

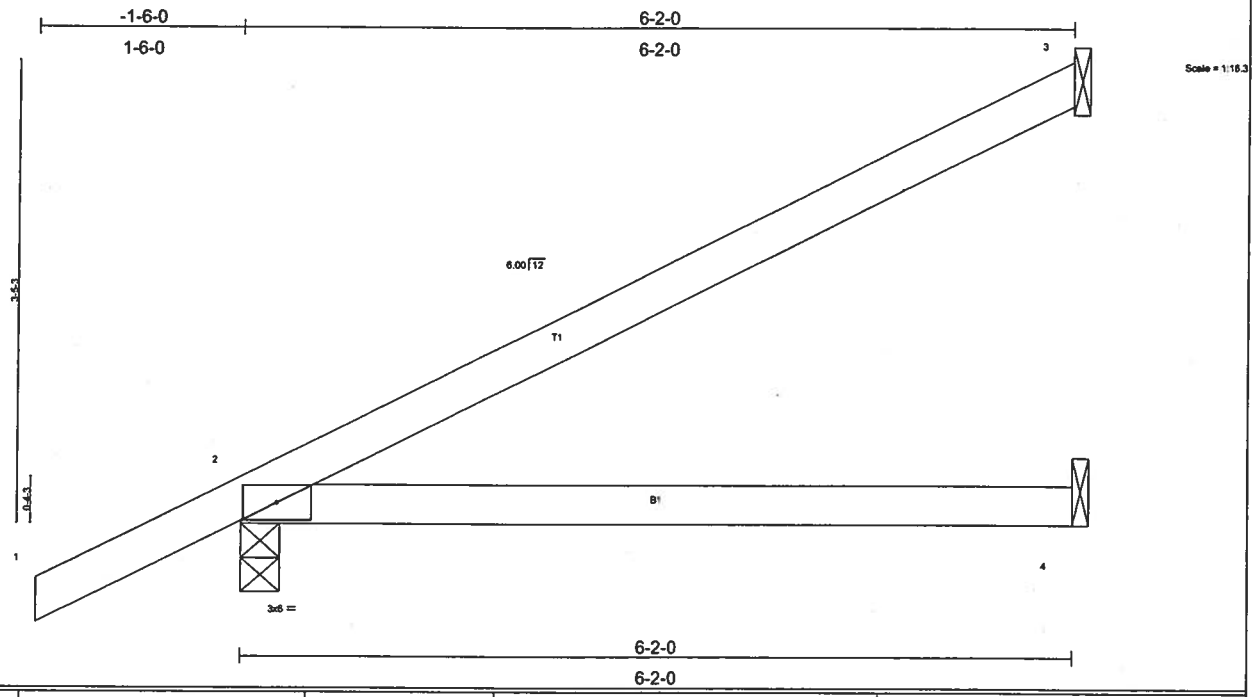
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061282

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ6	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:19 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.12	2-4	>618	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=149/Mechanical, 2=351/0-3-8, 4=89/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-164(load case 5), 2=-208(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-140/54
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 3 and 208 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061283

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ7	MONO TRUSS	20	1	Job Reference (optional)

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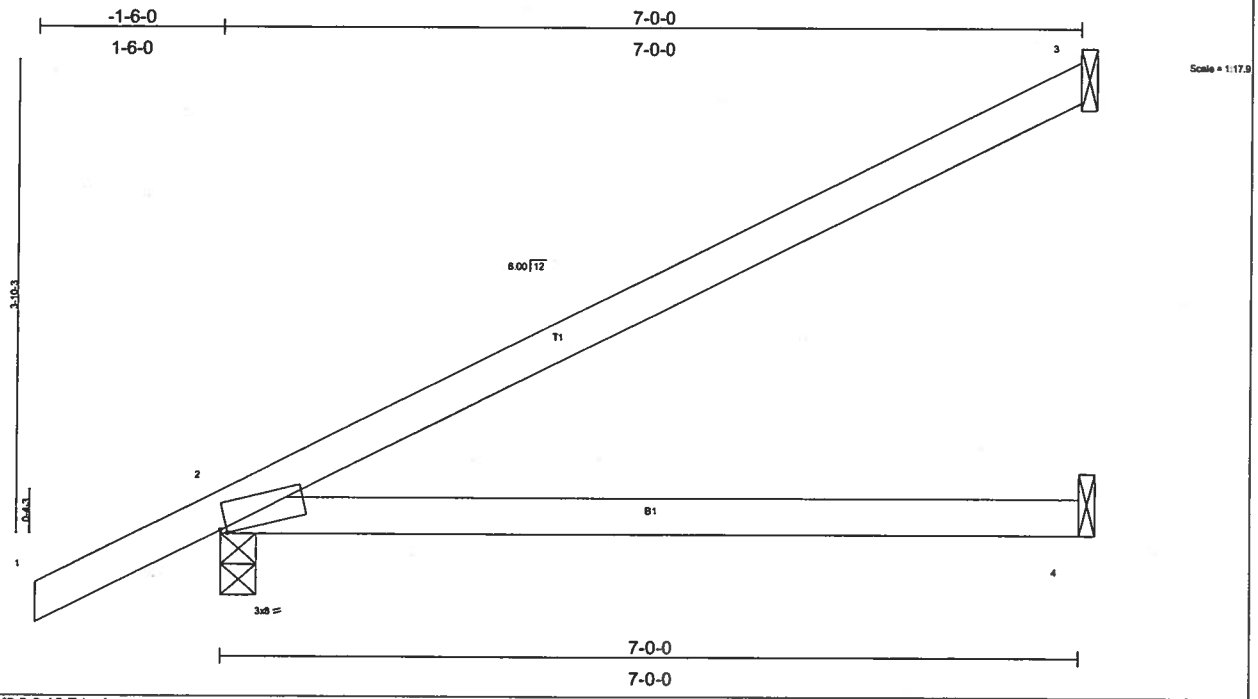

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.13	2-4	>606	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.22	2-4	>365	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=166/Mechanical, 2=385/0-3-8, 4=108/Mechanical
Max Horz 2=247(load case 5)
Max Uplift3=-170(load case 5), 2=-217(load case 5), 4=-1(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-143/59
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 3, 217 lb uplift at joint 2 and 1 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061284

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ7A	MONO HIP	2	1	Job Reference (optional)

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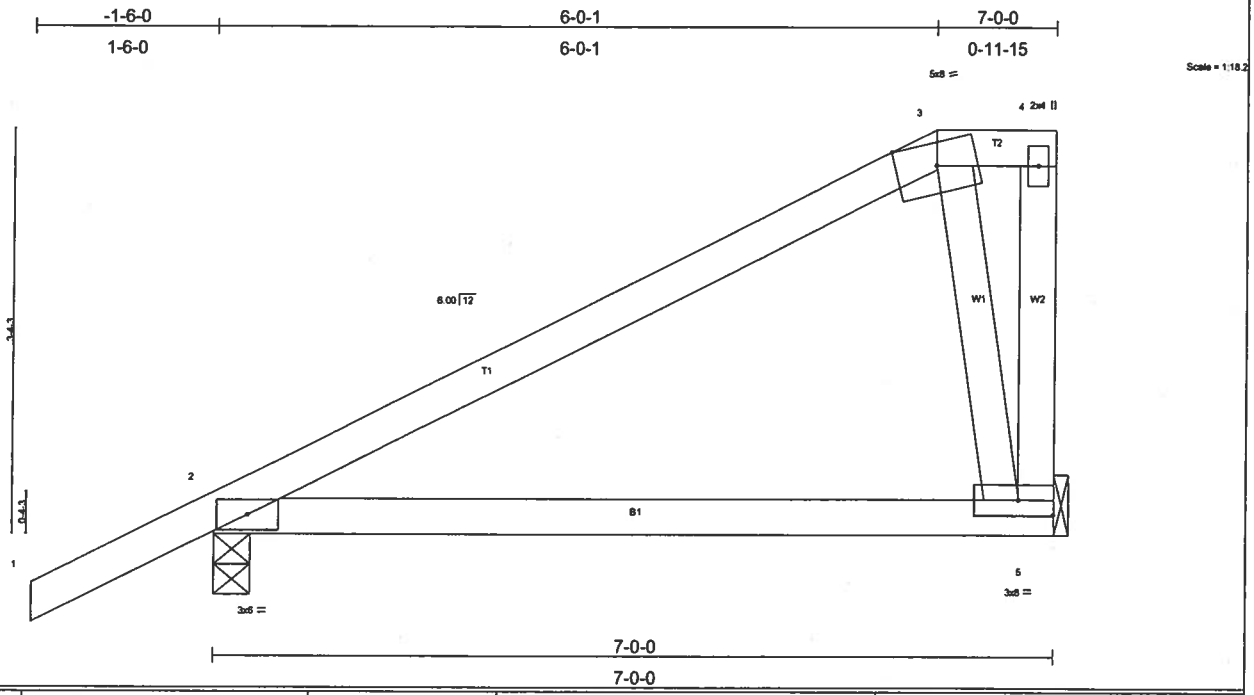

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.06	2-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.10	2-5	>811	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=271/Mechanical, 2=382/0-3-8
Max Horz 2=222(load case 5)
Max Uplift5=-144(load case 5), 2=-226(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-210/35, 3-4=-50/3, 4-5=-250/138
BOT CHORD 2-5=-141/130
WEBS 3-5=-307/524

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 5 and 226 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061285

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ7B	MONO TRUSS	3	1	Job Reference (optional)

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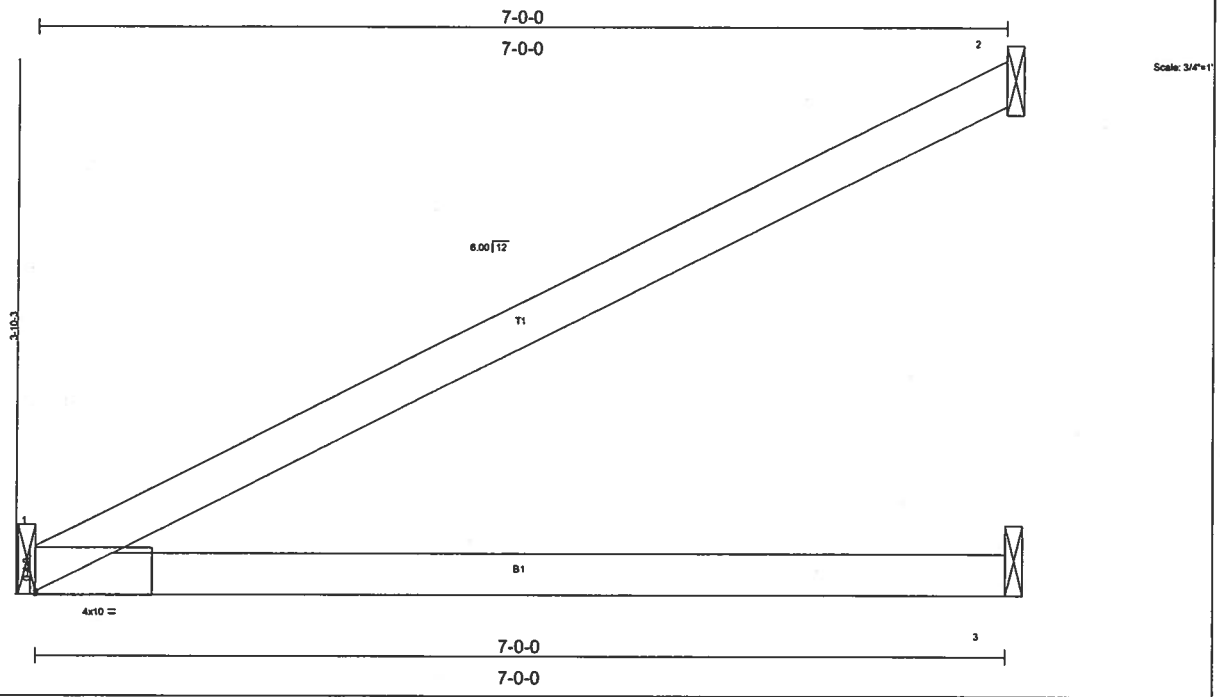

Plate Offsets (X,Y): [1:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.19	1-3	>440	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.26	1-3	>316	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=289/Mechanical, 2=173/Mechanical, 3=116/Mechanical
Max Horz 1=192(load case 5)
Max Uplift1=-90(load case 5), 2=-179(load case 5), 3=-9(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-147/62
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 90 lb uplift at joint 1, 179 lb uplift at joint 2 and 9 lb uplift at joint 3.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061286

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ7G	MONO HIP	2	1	Job Reference (optional)

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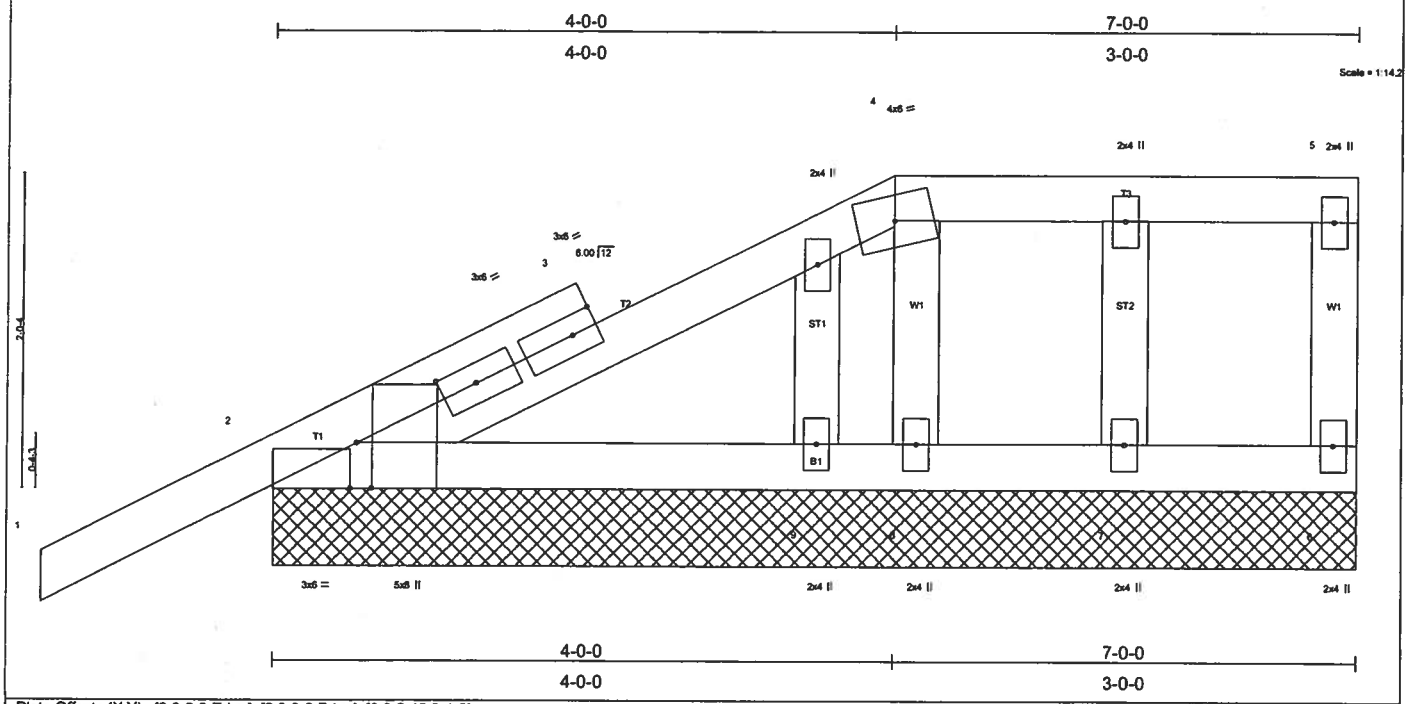

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.00	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.09	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=364/7-0-0, 6=115/7-0-0, 8=264/7-0-0, 9=138/7-0-0, 7=51/7-0-0
Max Horz 2=149(load case 5)
Max Uplift2=-243(load case 5), 6=-91(load case 3), 8=-197(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/56, 2-3=-104/24, 3-4=-40/34, 4-5=-15/21, 5-6=-98/112
BOT CHORD 2-9=-48/41, 8-9=-48/41, 7-8=-21/15, 6-7=-21/15
WEBS 4-8=-308/349

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 2, 91 lb uplift at joint 6 and 197 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-5=-87(F=-33), 2-6=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061287

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	EJ7T	SPECIAL	5	1	Job Reference (optional)

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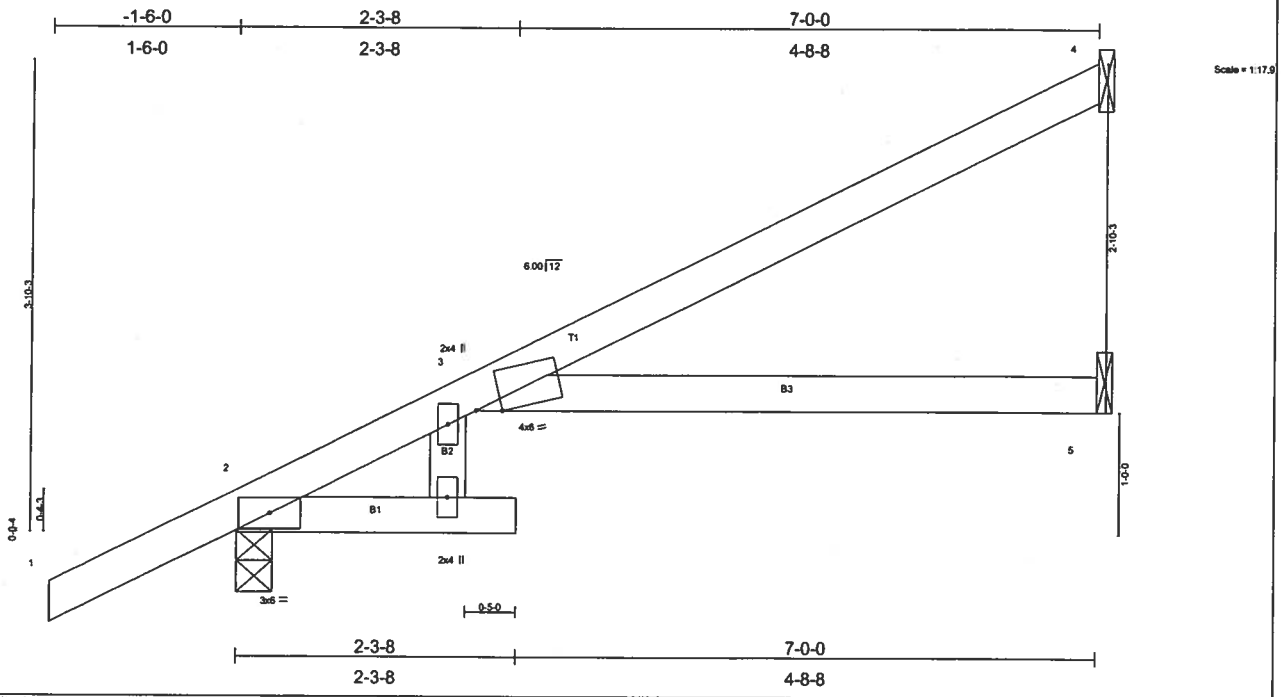

Plate Offsets (X,Y): [3:0-2-9,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.22	3-5	>370	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.25	3-5	>321	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.10	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=153/Mechanical, 2=358/0-3-8, 5=114/Mechanical
Max Horz 2=250(load case 5)
Max Uplift4=-142(load case 5), 2=-225(load case 5), 5=-21(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-160/0, 3-4=-113/57
BOT CHORD 3-5=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 4, 225 lb uplift at joint 2 and 21 lb uplift at joint 5.
4) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061288

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	HJ4	MONO TRUSS	4	1	Job Reference (optional)

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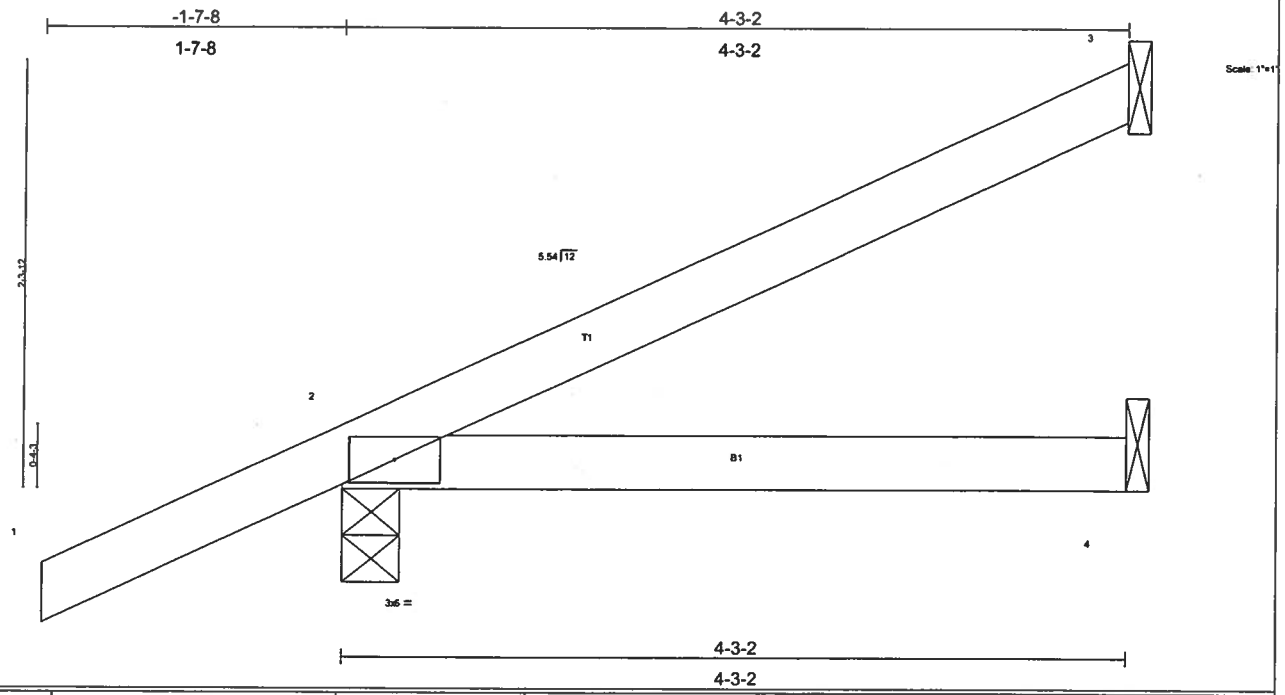

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.02	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=88/Mechanical, 2=287/0-3-13, 4=61/Mechanical
Max Horz 2=163(load case 5)
Max Uplift3=-89(load case 5), 2=-209(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-81/29
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 3 and 209 lb uplift at joint 2.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061289

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	HJ7	PORCH TRUSS	2	1	Job Reference (optional)

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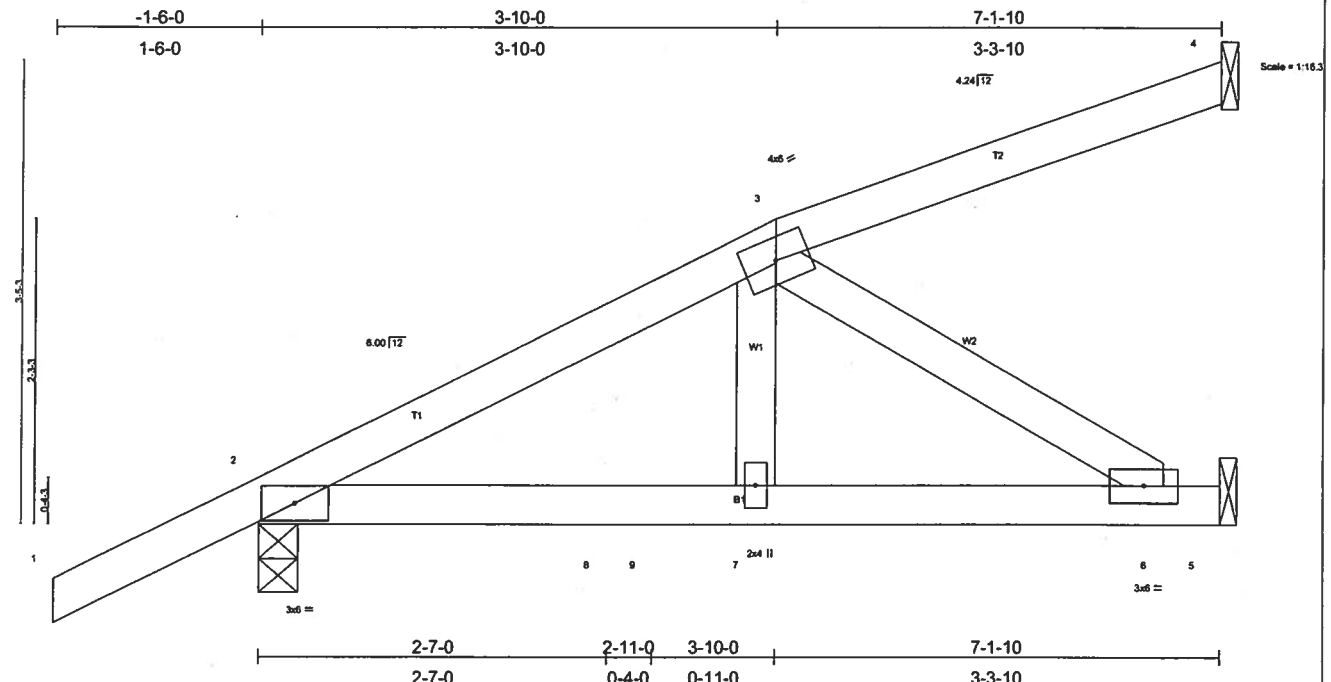

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	0.02	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.03	2-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-8 oc bracing.

REACTIONS (lb/size) 4=69/Mechanical, 2=541/0-3-8, 5=303/Mechanical
Max Horz 2=225(load case 5)
Max Uplift4=-76(load case 3), 2=-288(load case 5), 5=-129(load case 5)
Max Grav 4=70(load case 9), 2=541(load case 1), 5=303(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-604/292, 3-4=-42/18
BOT CHORD 2-8=-415/496, 8-9=-415/496, 7-9=-415/496, 6-7=-404/469, 5-6=0/0
WEBS 3-7=-158/366, 3-6=-567/488

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 4, 288 lb uplift at joint 2 and 129 lb uplift at joint 5.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 61 lb down and 46 lb up at 2-7-0, 60 lb down and 46 lb up at 2-11-0, and 60 lb down and 46 lb up at 2-11-0, and 61 lb down and 46 lb up at 2-7-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 2-5=-30
Concentrated Loads (lb)
Vert: 8=-122(F=-61, B=-61) 9=-120(F=-60, B=-60)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061290

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	HJ9	MONO TRUSS	3	1	Job Reference (optional)

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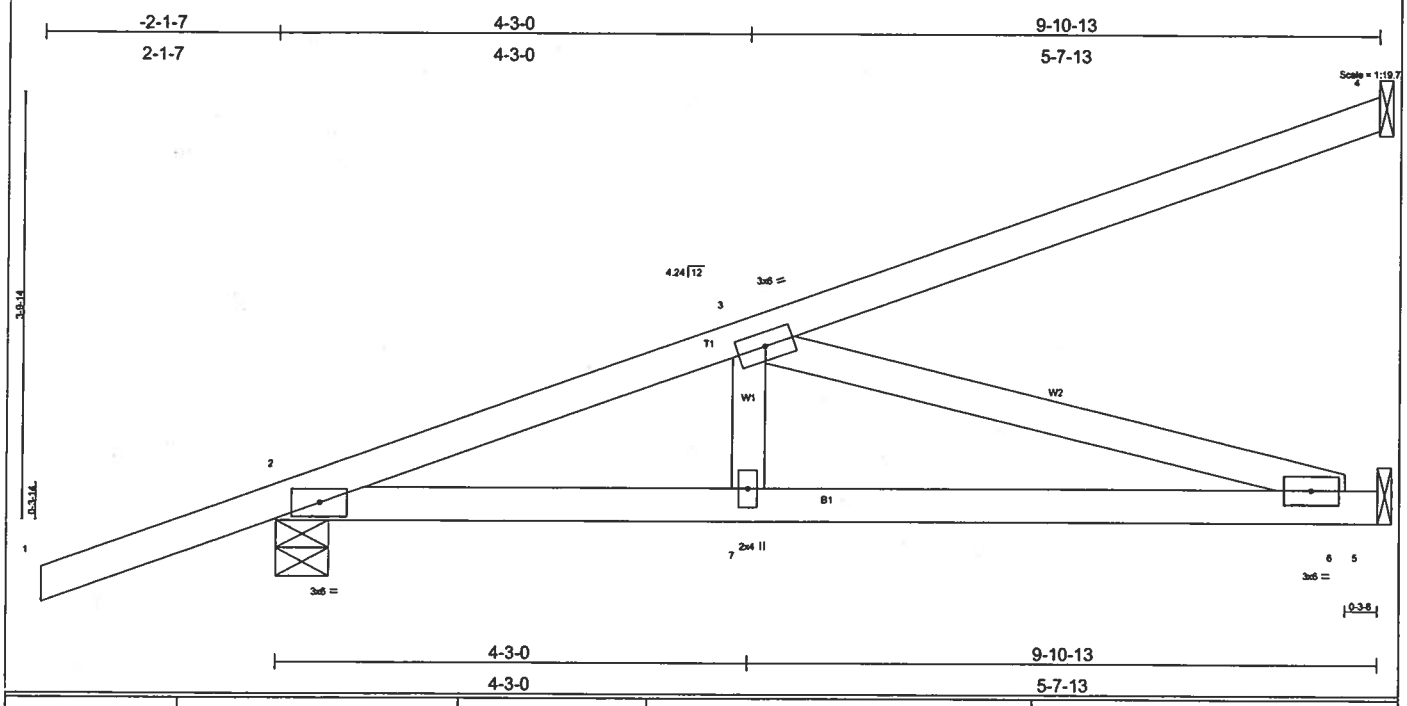

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.17	6-7	>693	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.52	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-12 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=486/0-5-11, 5=387/Mechanical
Max Horz 2=301(load case 2)
Max Uplift4=-281(load case 2), 2=-287(load case 2), 5=-105(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-933/240, 3-4=-121/65
BOT CHORD 2-7=-466/869, 6-7=-466/869, 5-6=0/0
WEBS 3-7=0/197, 3-6=-904/485

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 4, 287 lb uplift at joint 2 and 105 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061291

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T01	HIP	1	1	Job Reference (optional)

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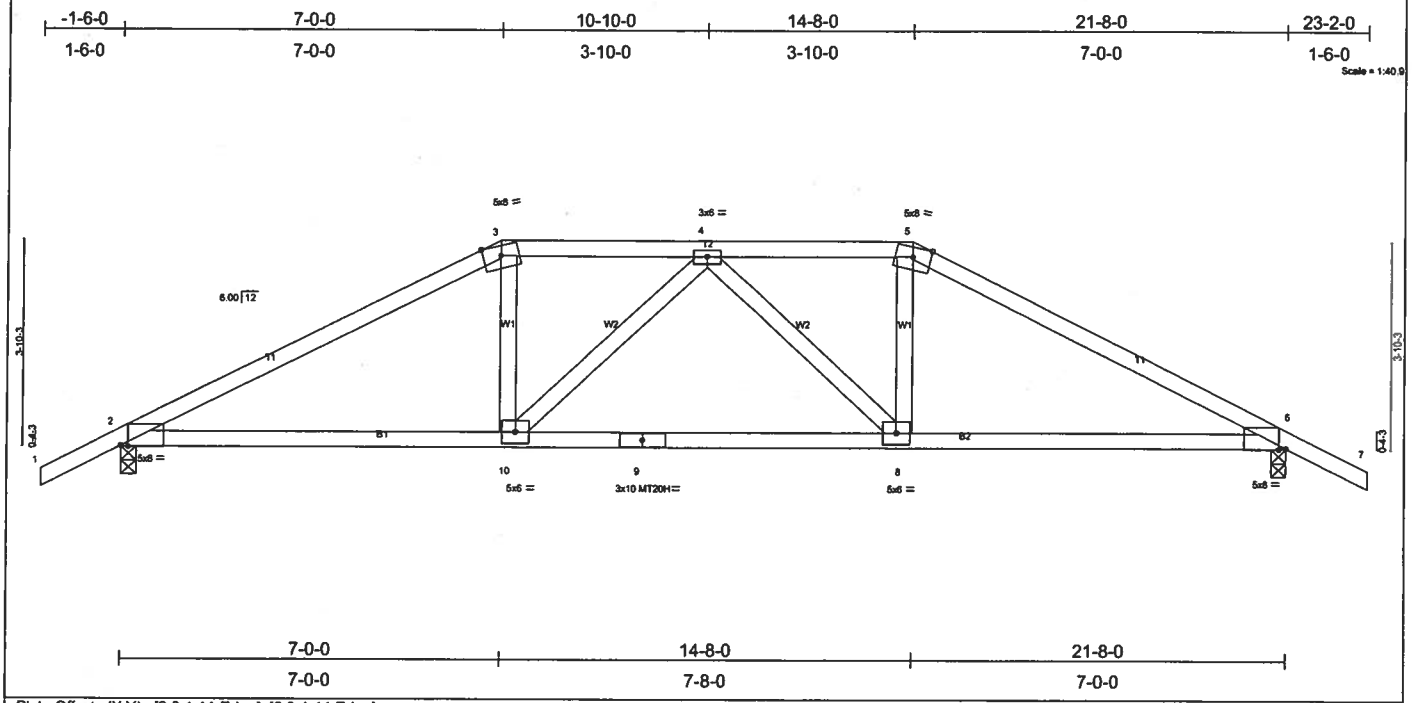

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.26	8-10	>972	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.44	8-10	>584	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-10 oc bracing.

REACTIONS (lb/size) 2=1923/0-3-8, 6=1923/0-3-8
Max Horz 2=-92(load case 5)
Max Uplift2=-1070(load case 4), 6=-1070(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-3555/1811, 3-4=-3134/1713, 4-5=-3134/1713, 5-6=-3555/1811, 6-7=0/35
BOT CHORD 2-10=-1547/3087, 9-10=-1780/3337, 8-9=-1780/3337, 6-8=-1495/3087
WEBS 3-10=-542/1199, 4-10=-403/402, 4-8=-403/402, 5-8=-542/1199

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1070 lb uplift at joint 2 and 1070 lb uplift at joint 6.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 14-8-0, and 539 lb down and 348 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-122(F=-68), 5-7=-54, 2-10=-30, 8-10=-68(F=-38), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061292

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T02	HIP	1	1	Job Reference (optional)

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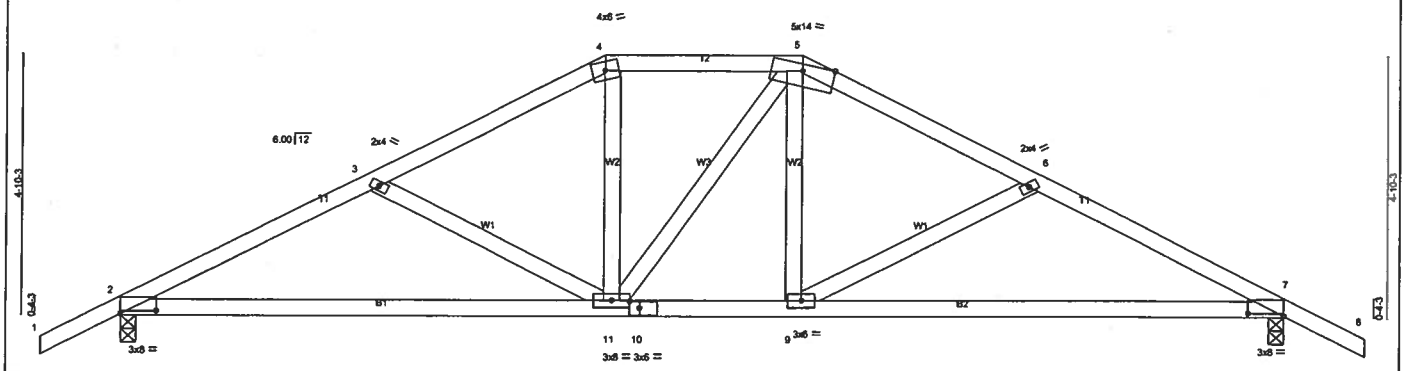
-1-6-0 | 4-9-4 | 9-0-0 | 12-8-0 | 16-10-12 | 21-8-0 | 23-2-0

1-6-0 | 4-9-4 | 4-2-12 | 3-8-0 | 4-2-12 | 4-9-4 | 1-6-0

Scale = 1:40.9



9-0-0 | 12-8-0 | 21-8-0

9-0-0 | 3-8-0 | 9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10], [10:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.18	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.31	7-9	>829	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-4 oc bracing.

REACTIONS (lb/size) 2=987/0-3-8, 7=987/0-3-8
Max Horz 2=-109(load case 6)
Max Uplift2=-480(load case 5), 7=-480(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1497/830, 3-4=-1230/673, 4-5=-1058/663, 5-6=-1229/673, 6-7=-1497/830, 7-8=0/35
BOT CHORD 2-11=-581/1302, 10-11=-318/1056, 9-10=-318/1056, 7-9=-581/1302
WEBS 3-11=-289/298, 4-11=-98/318, 5-11=-103/106, 5-9=-98/319, 6-9=-290/298

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 480 lb uplift at joint 2 and 480 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061293

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T03	COMMON	4	1	Job Reference (optional)

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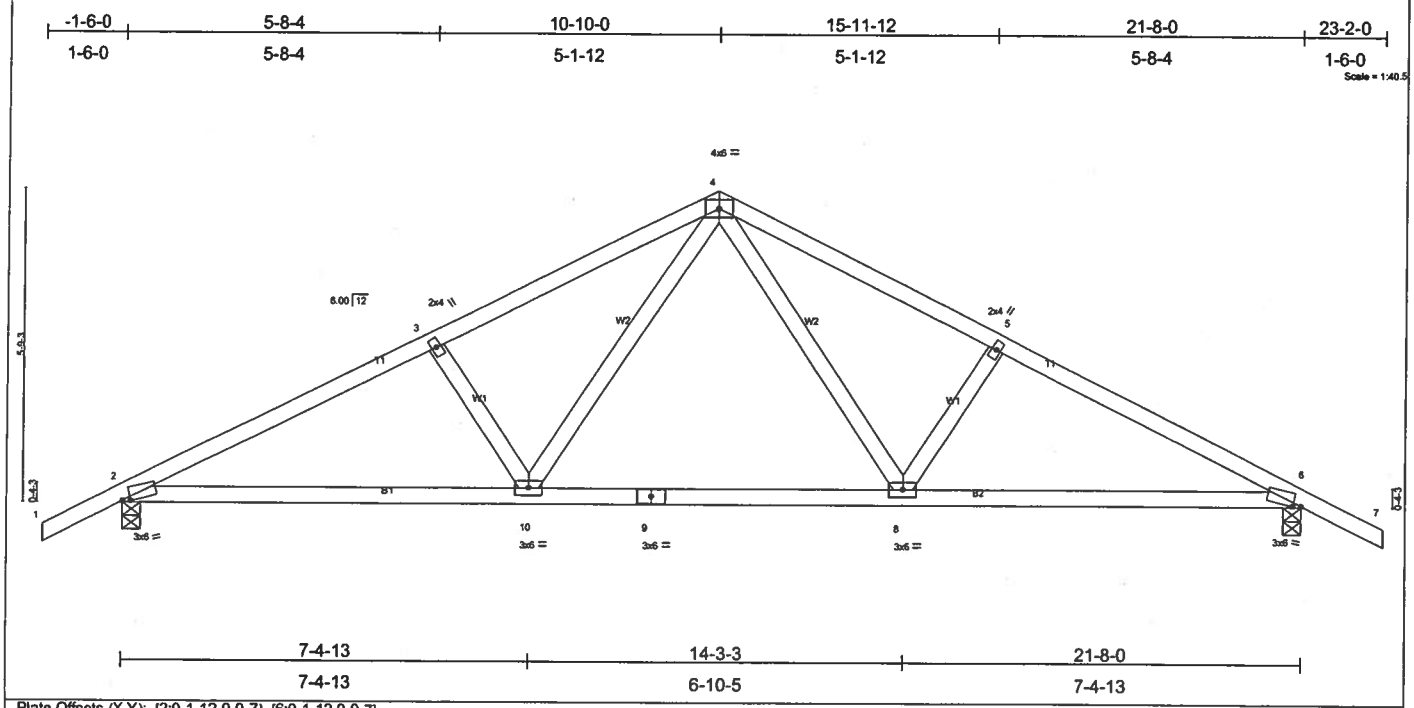

Plate Offsets (X,Y): [2:0-1-12,0-0-7], [6:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.20	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.32	8-10	>793	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-0 oc bracing.

REACTIONS (lb/size) 2=1158/0-4-0, 6=1158/0-4-0
Max Horz 2=124(load case 5)
Max Uplift2=-576(load case 5), 6=-576(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1911/1027, 3-4=-1754/1020, 4-5=-1754/1020, 5-6=-1911/1027, 6-7=0/35
BOT CHORD 2-10=-742/1637, 9-10=-388/1126, 8-9=-388/1126, 6-8=-742/1637
WEBS 3-10=-251/294, 4-10=-369/743, 4-8=-369/743, 5-8=-251/294

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 576 lb uplift at joint 2 and 576 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MARCH 6,2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061294

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T03A	COMMON	2	1	Job Reference (optional)

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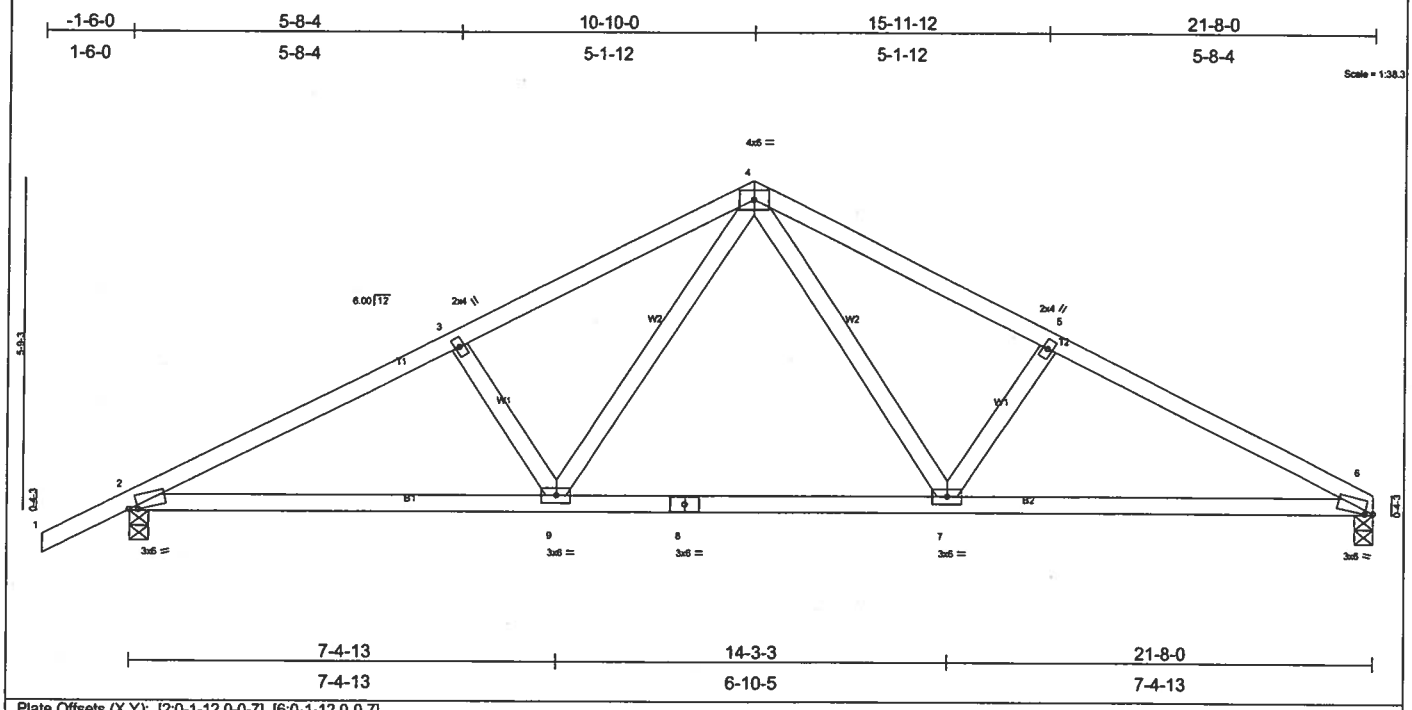

Plate Offsets (X,Y): [2:0-1-12,0-0-7], [6:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.20	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.32	7-9	>802	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.30	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-5 oc bracing.

REACTIONS (lb/size) 6=1064/0-4-0, 2=1161/0-4-0
Max Horz 2=146(load case 5)
Max Uplift6=-457(load case 6), 2=-578(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1918/1040, 3-4=-1761/1033, 4-5=-1774/1060, 5-6=-1919/1068
BOT CHORD 2-9=-823/1644, 8-9=-471/1133, 7-8=-471/1133, 6-7=-855/1660
WEBS 3-9=-251/295, 4-9=-365/742, 4-7=-407/762, 5-7=-262/314

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 6 and 578 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-9=-30, 7-9=-80(F=-50), 6-7=-30

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061295

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T03G	COMMON	1	1	Job Reference (optional)

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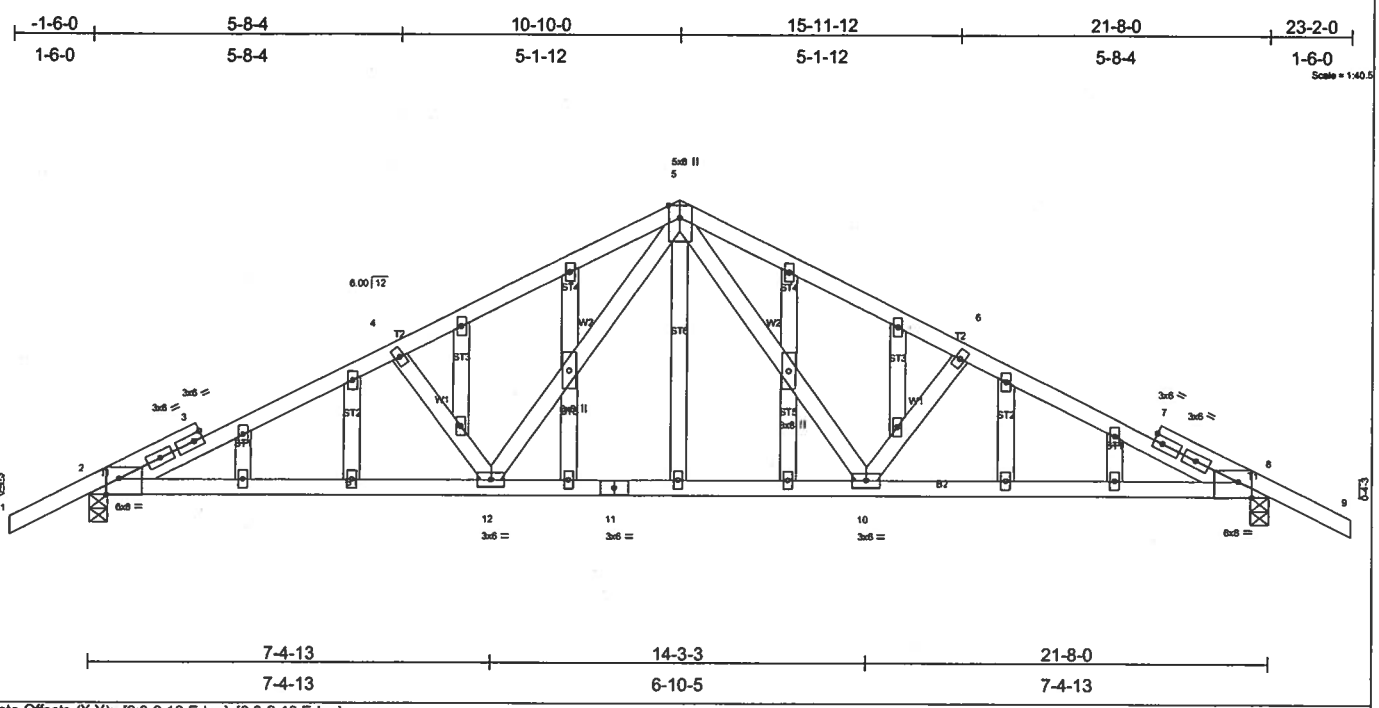

Plate Offsets (X,Y): [2:0-2-13,Edge], [8:0-2-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.14	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.23	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-11 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=119(load case 5)
Max Uplift2=-554(load case 5), 8=-554(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-1866/978, 3-4=-1823/989, 4-5=-1661/936, 5-6=-1661/936, 6-7=-1823/989, 7-8=-1866/978, 8-9=0/42
BOT CHORD 2-12=-751/1646, 11-12=-353/1039, 10-11=-353/1039, 8-10=-751/1646
WEBS 4-12=-406/373, 5-12=-324/671, 5-10=-324/671, 6-10=-406/373

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 2 and 554 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061296

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T05	SPECIAL	1	1	Job Reference (optional)

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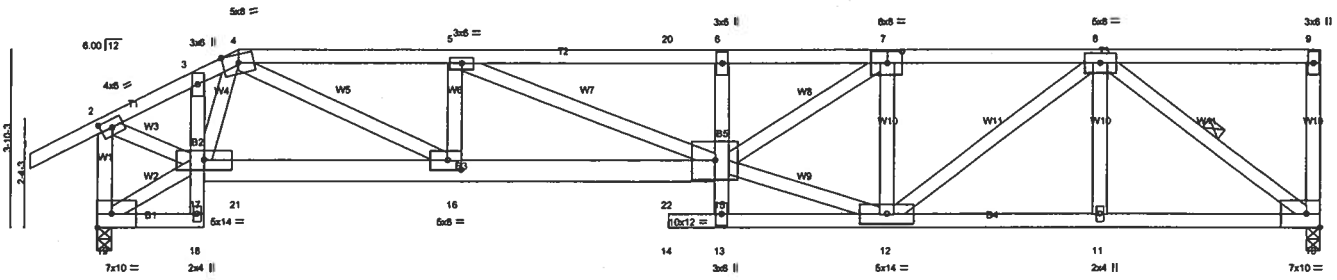
-1-6-0	2-3-8	2-11-15	7-8-0	12-4-0	17-1-1	21-8-7	26-5-8
1-6-0	2-3-8	0-8-7	4-8-0	4-8-0	4-9-1	4-7-5	4-9-1

Scale: 1/4"=1'



2-3-8	7-8-0	12-4-0	13-4-0	17-1-1	21-8-7	26-5-8
2-3-8	5-4-8	4-8-0	1-0-0	3-9-1	4-7-5	4-9-1

Plate Offsets (X,Y): [2:0-2-15,0-2-0], [7:0-3-12,0-3-0], [16:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.41	14	>765	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.66	14	>477	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(TL)	0.21	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5 2 X 4 SYP No.2, W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-11-4 oc bracing.
WEBS 1 Row at midpt 8-10

REACTIONS (lb/size) 10=2402/0-3-8, 19=2462/0-3-8
Max Horz 19=402(load case 4)
Max Uplift10=-1318(load case 5), 19=-1147(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2388/1317, 3-4=-2371/1388, 4-5=-5292/2797, 5-20=-6383/3362, 6-20=-6383/3362, 6-7=-6251/3298, 7-8=-4100/2203, 8-9=-77/43, 9-10=-258/246, 2-19=-2320/1243
BOT CHORD 18-19=-85/32, 17-18=-57/118, 3-17=-71/101, 17-21=-1410/2451, 16-21=-1410/2451, 16-22=-2797/5292, 15-22=-2797/5292, 13-15=0/224, 6-15=-503/450, 13-14=0/0, 12-13=-290/500, 11-12=-1449/2657, 10-11=-1449/2657
WEBS 4-16=-1591/3215, 5-16=-1020/635, 5-15=-643/1185, 12-15=-1997/3759, 7-15=-1327/2607, 7-12=-1966/1233, 8-12=-952/1823, 8-11=0/308, 8-10=-3258/1775, 2-17=-1208/2314, 4-17=-859/435, 17-19=-312/5

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1318 lb uplift at joint 10 and 1147 lb uplift at joint 19.
5) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 3-0-0; 4-3-0 from 3-0-0 to 12-4-0; 4-7-5 from 3-0-0 to 12-4-0
6) Girder carries hip end with 0-0-0 right side setback, 12-4-0 left side setback, and 7-0-0 end setback.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-54, 4-20=-103(F=-49), 9-20=-118(F=-64), 18-19=-129(F=-99), 17-21=-129(F=-99), 21-22=-71(F=-41), 15-22=-65(F=-35), 13-14=-65(F=-35), 10-13=-65(F=-35)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

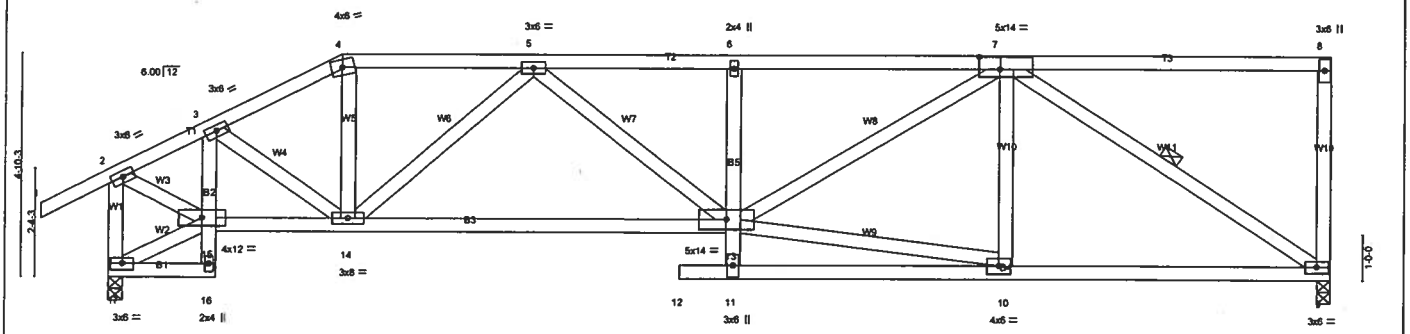
Dwg.#0306061297

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:26 2006 Page 1

-1-6-0	2-3-8	4-11-15	9-1-15	13-4-0	19-4-12	26-5-8
1-6-0	2-3-8	2-8-7	4-2-0	4-2-1	6-0-12	7-0-12

Scale: 1/4"=1'



2-3-8	4-11-15	12-4-0	13-4-0	19-4-12	26-5-8
2-3-8	2-8-7	7-4-1	1-0-0	6-0-12	7-0-12

Plate Offsets (X,Y): [7:0-5-7,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.19	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.31	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.09	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-13 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 9=1113/0-3-8, 17=1208/0-3-8
Max Horz 17=256(load case 5)
Max Uplift9=-530(load case 4), 17=-474(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1083/701, 3-4=-1473/811, 4-5=-1318/778, 5-6=-2141/1091, 6-7=-2104/1087, 7-8=-62/32, 8-9=-183/158, 2-17=-1141/810
BOT CHORD 16-17=-56/74, 15-16=-17/31, 3-15=-504/161, 14-15=-747/949, 13-14=-1043/1842, 11-13=0/124, 6-13=-256/225, 11-12=0/0, 10-11=-91/59, 9-10=-685/1336
WEBS 3-14=-174/430, 4-14=-171/438, 5-13=-116/387, 10-13=-608/1297, 7-13=-465/901, 7-10=-28/160, 7-9=-1517/778, 2-15=-540/1053, 15-17=-208/0, 5-14=-699/412

NOTES
1) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 530 lb uplift at joint 9 and 474 lb uplift at joint 17.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061298

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:27 2006 Page 1

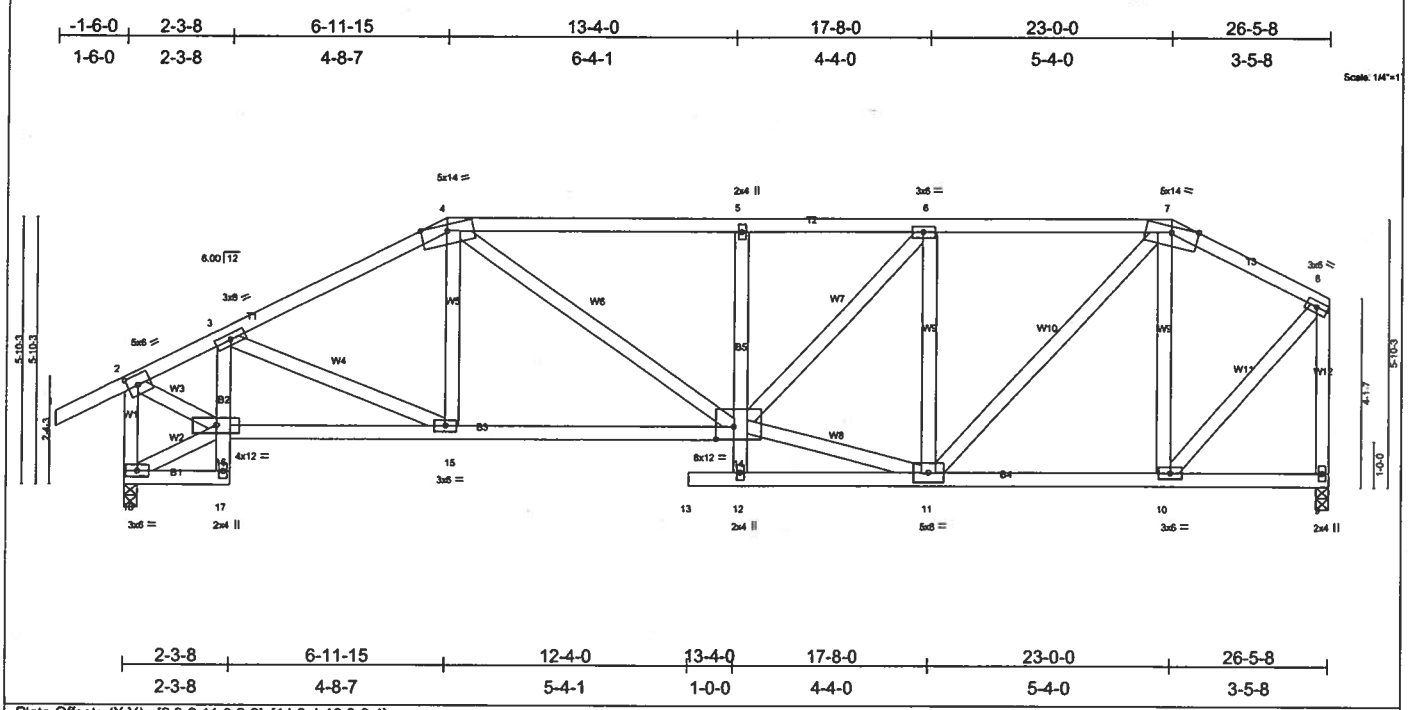


Plate Offsets (X,Y): [2:0-2-11,0-2-8], [14:0-4-13,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.10	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.17	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 184 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-8-0 oc bracing: 15-16
7-7-3 oc bracing: 14-15.

REACTIONS (lb/size) 9=1113/0-3-8, 18=1208/0-3-8
Max Horz 18=188(load case 5)
Max Uplift9=-424(load case 3), 18=-503(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1110/647, 3-4=-1499/806, 4-5=-1670/930, 5-6=-1637/913, 6-7=-1194/692, 7-8=-731/396, 2-18=-1160/770, 8-9=-1071/578
BOT CHORD 17-18=-61/31, 16-17=-10/29, 3-16=-426/214, 15-16=-686/1012, 14-15=-691/1298, 12-14=0/100, 5-14=-294/256, 12-13=0/0, 11-12=-80/59, 10-11=-299/613, 9-10=-9/12
WEBS 3-15=-183/370, 4-15=0/106, 4-14=-265/527, 11-14=-525/1167, 6-14=-327/656, 6-11=-746/463, 7-11=-422/855, 7-10=-551/367, 2-16=-559/1089, 8-10=-450/926, 16-18=-115/5

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 424 lb uplift at joint 9 and 503 lb uplift at joint 18.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061299

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T08	SPECIAL	1	1	Job Reference (optional)

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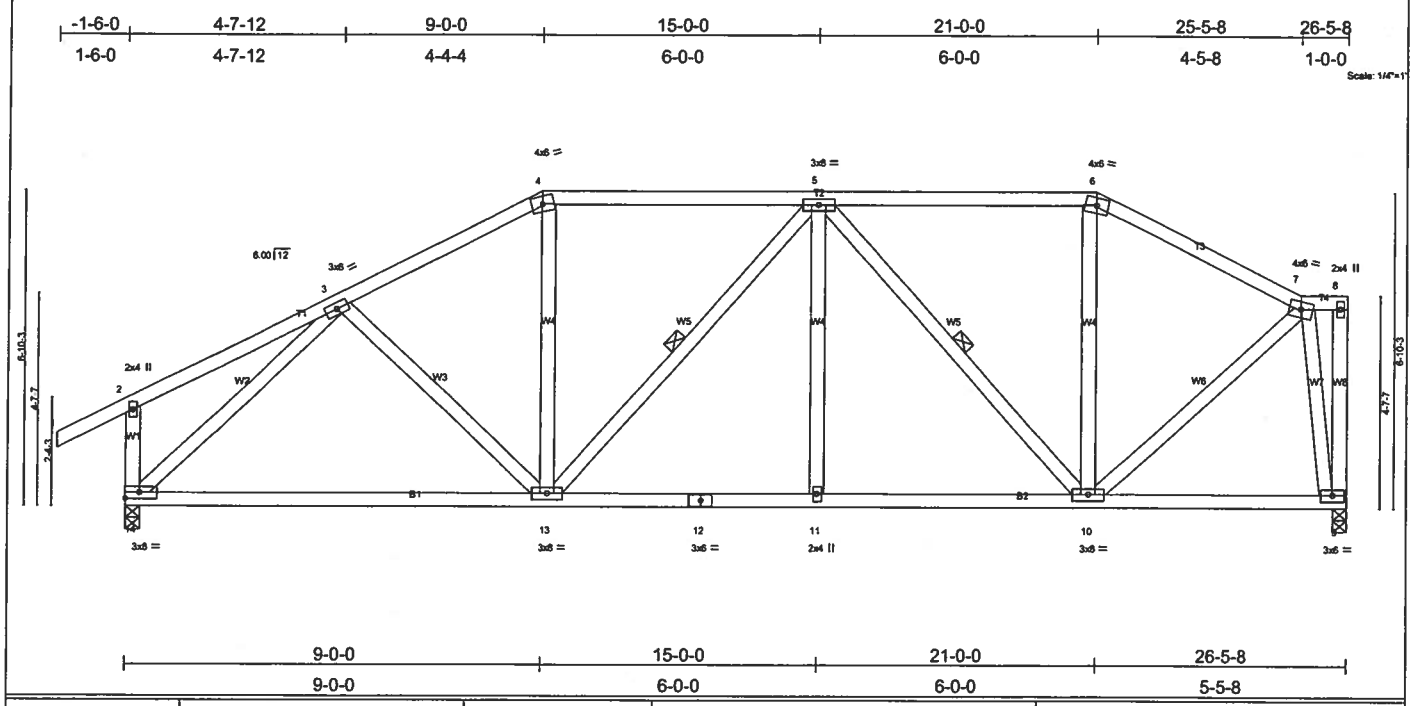

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.13	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.22	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 178 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-0-4 oc bracing.
WEBS 1 Row at midpt 5-13, 5-10

REACTIONS (lb/size) 9=1096/0-3-8, 14=1191/0-3-8
Max Horz 14=280(load case 5)
Max Uplift9=-397(load case 6), 14=-521(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-194/198, 3-4=-1188/702, 4-5=-1026/692, 5-6=-754/517, 6-7=-884/506, 7-8=-21/2, 8-9=-85/47, 2-14=-285/352
BOT CHORD 13-14=-622/901, 12-13=-585/1106, 11-12=-585/1106, 10-11=-585/1106, 9-10=-131/217
WEBS 3-13=-87/247, 4-13=-45/225, 5-13=-208/157, 5-11=0/153, 5-10=-567/304, 6-10=0/125, 7-10=-346/735, 7-9=-1011/666, 3-14=-1096/544

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 397 lb uplift at joint 9 and 521 lb uplift at joint 14.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061300

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T09	SPECIAL	1	1	Job Reference (optional)

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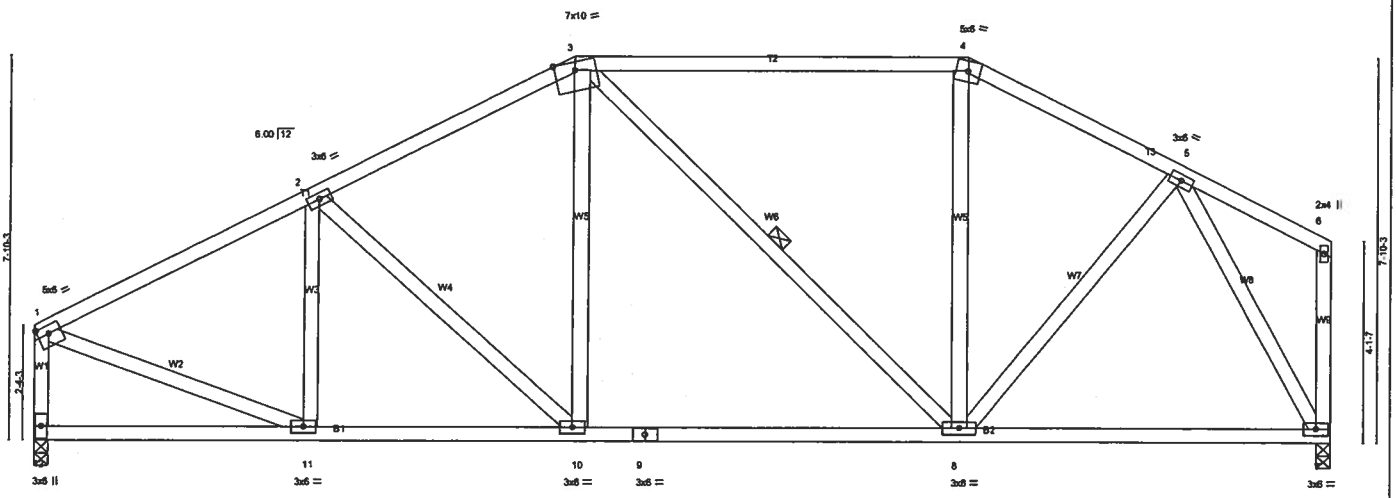
5-7-12 | 10-11-15 | 19-0-0 | 23-2-12 | 26-5-8
5-7-12 | 5-4-4 | 8-0-1 | 4-2-12 | 3-2-12

Scale = 1:44.8



5-7-12 | 10-11-15 | 19-0-0 | 26-5-8
5-7-12 | 5-4-4 | 8-0-1 | 7-5-8

Plate Offsets (X,Y): [1:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.09	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.14	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-2 oc bracing.
WEBS 1 Row at midpt 3-8

REACTIONS (lb/size) 12=1099/0-3-8, 7=1099/0-3-8
Max Horz 12=217(load case 5)
Max Uplift12=-428(load case 5), 7=-413(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1202/666, 2-3=-1137/711, 3-4=-825/611, 4-5=-941/603, 5-6=-89/57, 1-12=-1010/583, 6-7=-118/94
BOT CHORD 11-12=-245/104, 10-11=-612/1013, 9-10=-492/974, 8-9=-492/974, 7-8=-296/528
WEBS 2-11=-202/188, 2-10=-81/184, 3-10=-84/252, 3-8=-270/162, 4-8=-25/142, 5-8=-185/485, 1-11=-414/974, 5-7=-1004/600

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 428 lb uplift at joint 12 and 413 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061301

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:29 2006 Page 1

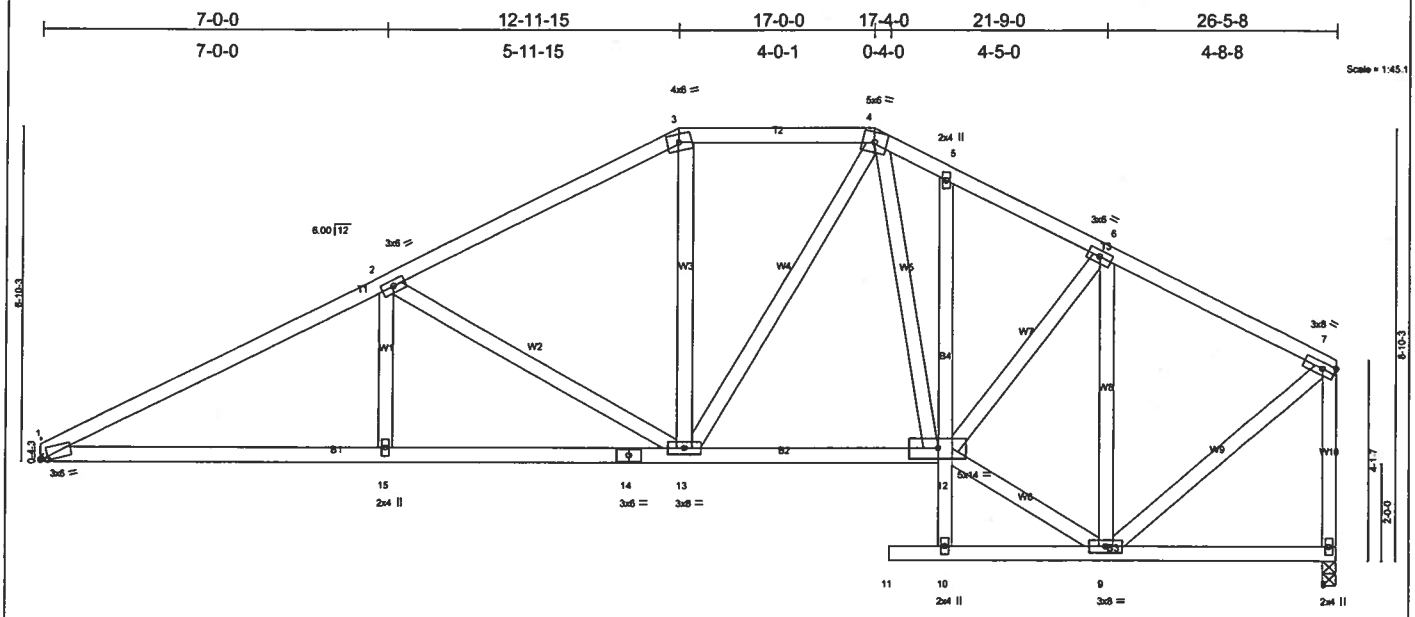


Plate Offsets (X,Y): [1:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.14	1-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.22	1-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 176 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1114/Mechanical, 8=1126/0-3-8
Max Horz 1=181(load case 5)
Max Uplift1=-457(load case 5), 8=-426(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1998/1045, 2-3=-1364/804, 3-4=-1154/793, 4-5=-1225/825, 5-6=-1260/764, 6-7=-849/475, 7-8=-1054/601
BOT CHORD 1-15=-932/1714, 14-15=-932/1714, 13-14=-932/1714, 12-13=-445/1040, 10-12=0/80, 5-12=-90/124, 10-11=0/0, 9-10=-15/0, 8-9=-23/33
WEBS 2-15=0/235, 2-13=-658/490, 3-13=-95/305, 4-13=-157/300, 6-12=-205/600, 7-9=-431/891, 6-9=-896/526, 9-12=-395/836, 4-12=-163/231

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 1 and 426 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061302

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T11	SPECIAL	1	1	Job Reference (optional)

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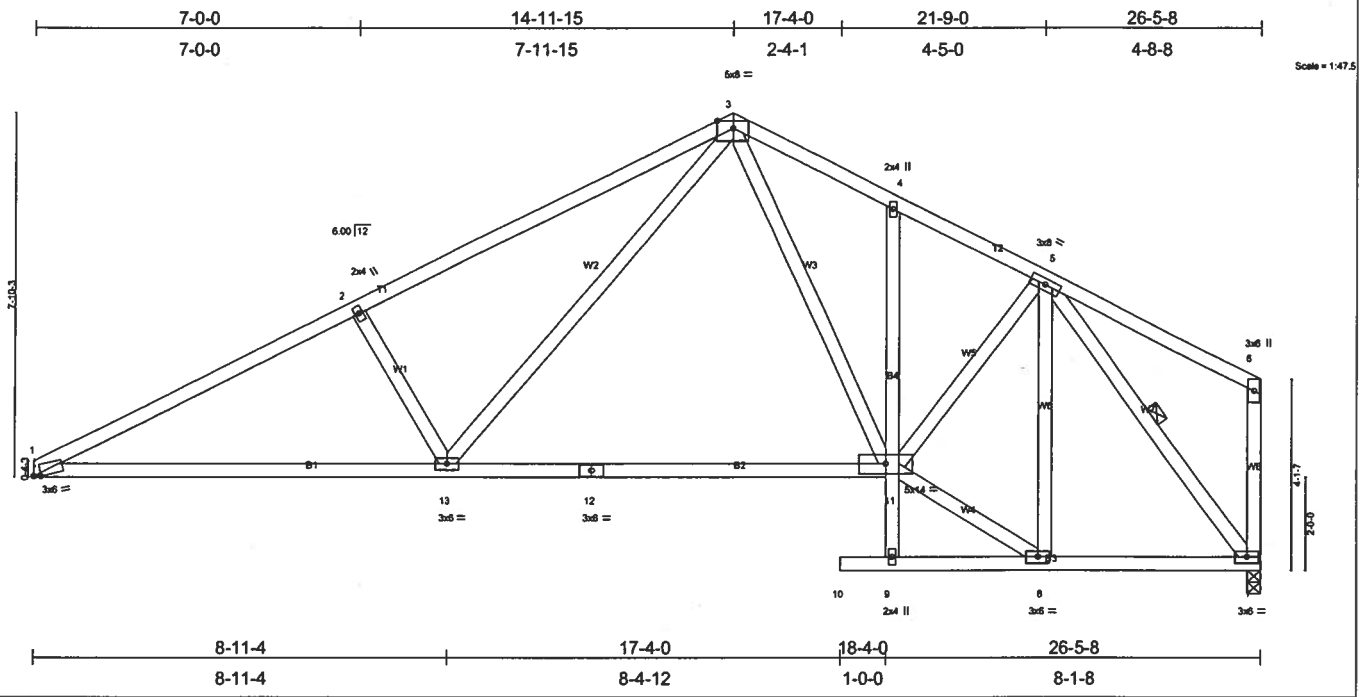

Plate Offsets (X,Y): [1:0-1-12,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.33	1-13	>947	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 1=1114/Mechanical, 7=1126/0-3-8
Max Horz 1=198(load case 5)
Max Uplift1=-468(load case 5), 7=-441(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1958/1081, 2-3=-1785/1086, 3-4=-1241/826, 4-5=-1280/774, 5-6=-92/108, 6-7=-138/156
BOT CHORD 1-13=-973/1702, 12-13=-432/979, 11-12=-432/979, 9-11=0/73, 4-11=-160/124, 9-10=0/0, 8-9=-73/0, 7-8=-360/712
WEBS 2-13=-400/480, 3-13=-481/846, 3-11=-166/442, 5-11=-205/623, 5-7=-1140/566, 5-8=-351/247, 8-11=-390/911

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 1 and 441 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061303

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:30 2006 Page 1

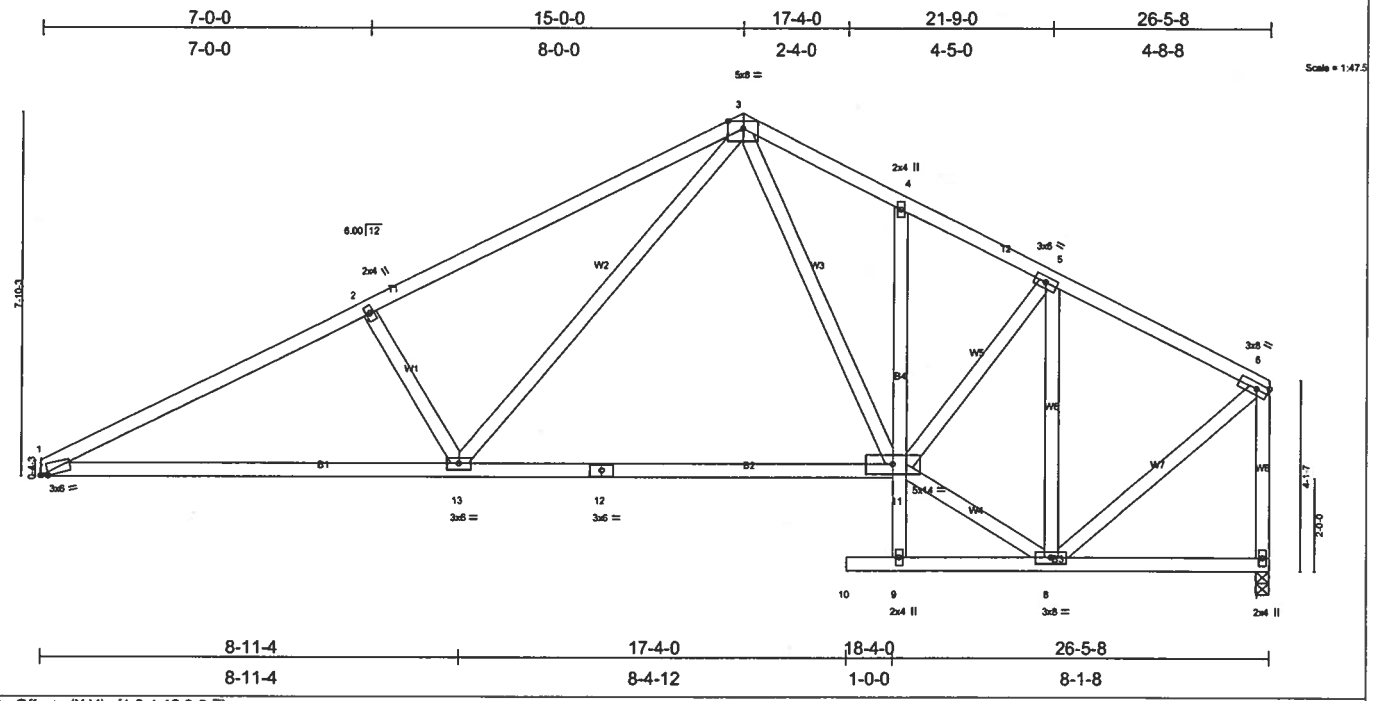

Plate Offsets (X,Y): [1:0-1-13,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.20	1-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.33	1-13	>943	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1114/Mechanical, 7=1126/0-3-8
Max Horz 1=198(load case 5)
Max Uplift1=-468(load case 5), 7=-441(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1959/1082, 2-3=-1786/1086, 3-4=-1241/825, 4-5=-1279/774, 5-6=-850/484, 6-7=-1055/610
BOT CHORD 1-13=-974/1703, 12-13=-432/979, 11-12=-432/979, 9-11=0/72, 4-11=-159/124, 9-10=0/0, 8-9=-71/0, 7-8=-23/32
WEBS 2-13=-401/481, 3-13=-482/847, 3-11=-166/441, 5-11=-204/620, 5-8=-927/532, 6-8=-442/894, 8-11=-389/909

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 1 and 441 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061304

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T13	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:30 2006 Page 1

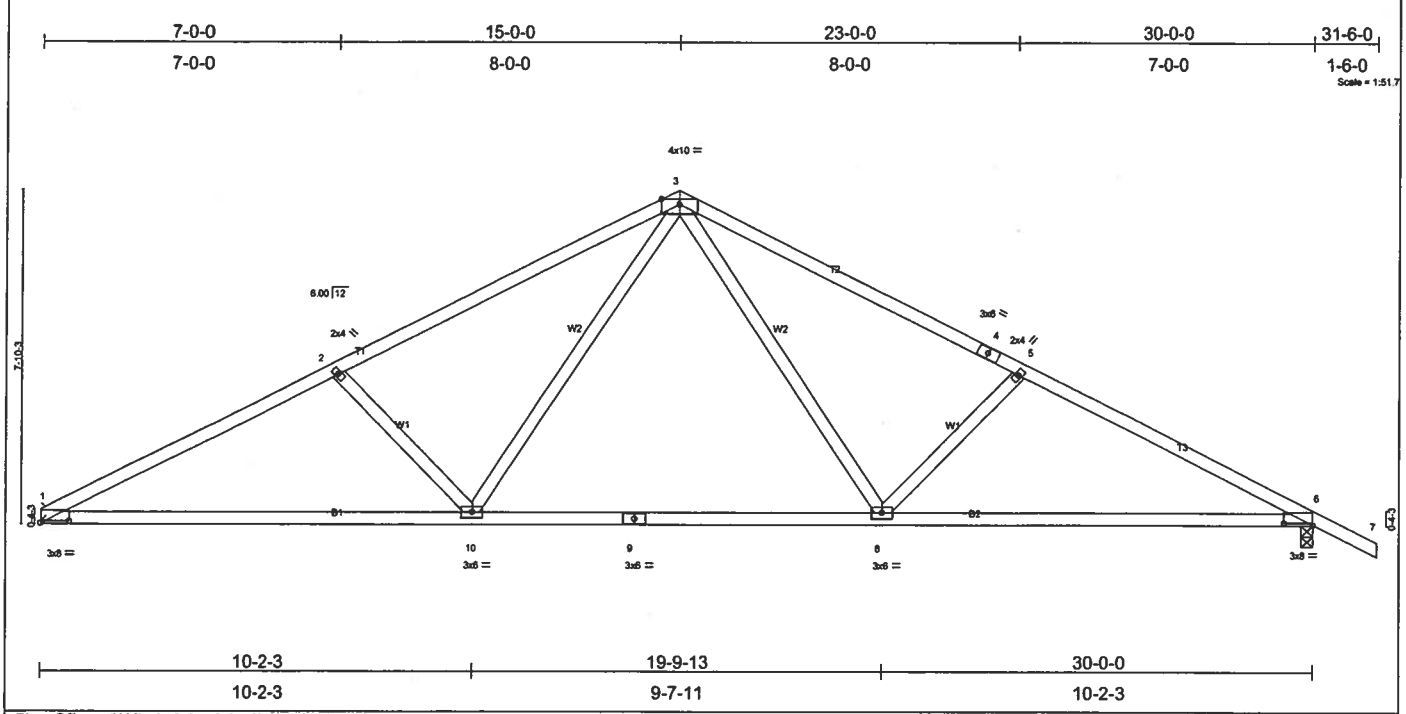

Plate Offsets (X,Y): [1:0-8-4,0-0-10], [6:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.32	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.53	1-10	>669	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.08	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-15 oc bracing.

REACTIONS (lb/size) 1=1245/Mechanical, 6=1339/0-3-8
Max Horz 1=-180(load case 6)
Max Uplift1=-522(load case 5), 6=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2192/1251, 2-3=-1930/1149, 3-4=-1823/1130, 4-5=-1920/1103, 5-6=-2180/1229, 6-7=0/35
BOT CHORD 1-10=-938/1910, 9-10=-425/1252, 8-9=-425/1252, 6-8=-912/1896
WEBS 2-10=-423/472, 3-10=-352/740, 3-8=-323/727, 5-8=-415/457

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 522 lb uplift at joint 1 and 640 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061305

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T13A	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:31 2006 Page 1

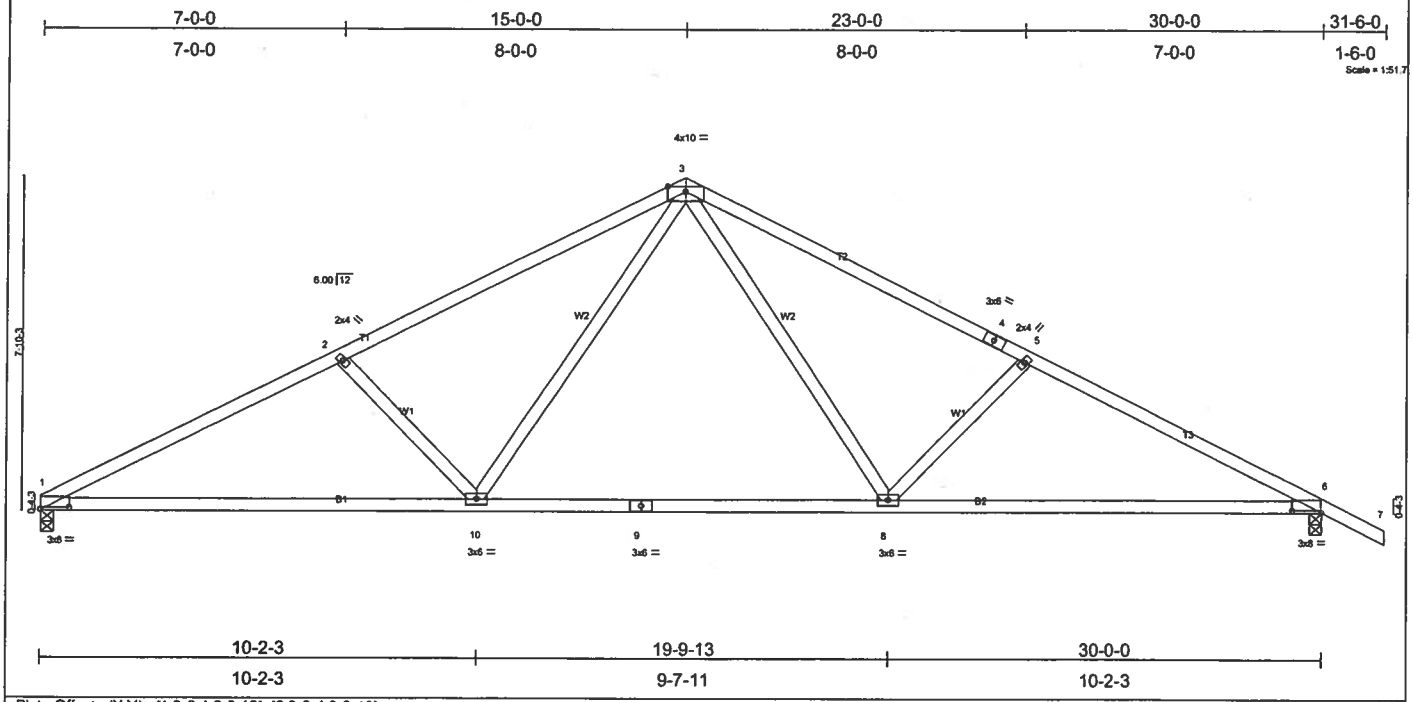

Plate Offsets (X,Y): [1:0-8-4,0-0-10], [6:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.32	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.53	1-10	>669	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.08	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-15 oc bracing.

REACTIONS (lb/size) 1=1245/0-3-8, 6=1339/0-3-8
Max Horz 1=-180(load case 6)
Max Uplift1=-522(load case 5), 6=-640(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2192/1251, 2-3=-1930/1149, 3-4=-1823/1130, 4-5=-1920/1103, 5-6=-2180/1229, 6-7=0/35
BOT CHORD 1-10=-938/1910, 9-10=-425/1252, 8-9=-425/1252, 6-8=-912/1896
WEBS 2-10=-423/472, 3-10=-352/740, 3-8=-323/727, 5-8=-415/457

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 522 lb uplift at joint 1 and 640 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061306

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T14	HIP	1	1	Job Reference (optional)

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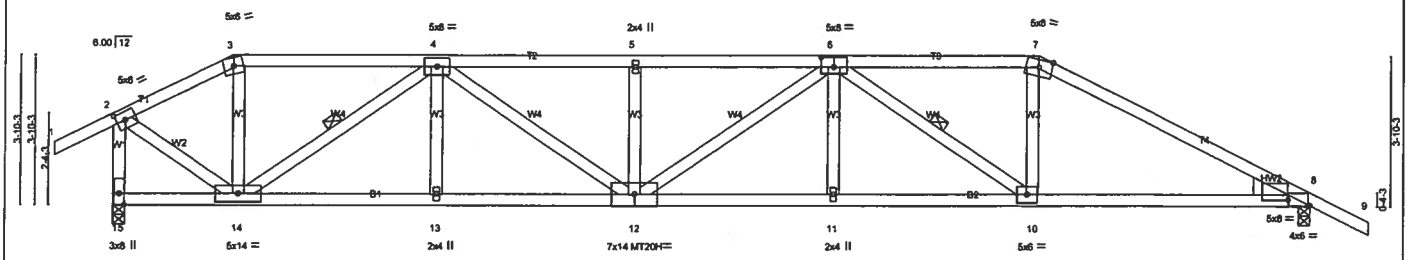
-1-6-0	2-11-15	8-2-13	13-4-0	18-5-2	23-8-0	30-8-0	32-2-0
1-6-0	2-11-15	5-2-14	5-1-2	5-1-2	5-2-14	7-0-0	1-6-0

Scale = 1:56.4



2-11-15	8-2-13	13-4-0	18-5-2	23-8-0	30-8-0
2-11-15	5-2-14	5-1-2	5-1-2	5-2-14	7-0-0

Plate Offsets (X,Y): [2:0-2-11,0-2-8], [6:0-4-0,0-3-0], [8:0-0-8,Edge], [8:0-6-11,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.45	11-12	>809	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.72	11-12	>504	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(TL)	0.21	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2, W4 2 X 4 SYP No.2
WEDGE
Right: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 3-8-3 oc bracing.
WEBS 1 Row at midpt 4-14, 6-10

REACTIONS (lb/size) 15=3129/0-3-8, 8=2699/0-3-8
Max Horz 15=-130(load case 2)
Max Uplift15=-1648(load case 3), 8=-1438(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-2651/1481, 3-4=-2388/1378, 4-5=-6186/3493, 5-6=-6185/3493, 6-7=-4644/2588, 7-8=-5217/2811, 8-9=0/35, 2-15=-2950/1570
BOT CHORD 14-15=-21/132, 13-14=-2740/4977, 12-13=-2740/4977, 11-12=-3305/6082, 10-11=-3305/6082, 8-10=-2414/4568
WEBS 3-14=-296/797, 4-14=-3166/1813, 4-13=0/334, 4-12=-829/1479, 5-12=-584/576, 6-12=-97/162, 6-11=0/335, 6-10=-1859/1123, 7-10=-921/1881, 2-14=-1577/2853

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1648 lb uplift at joint 15 and 1438 lb uplift at joint 8.
6) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 2-11-15
7) Girder carries hip end with 7-0-0 right side setback, 2-11-15 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 348 lb up at 23-8-0, and 231 lb down and 149 lb up at 2-11-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-7=-118(F=-64), 7-9=-54, 14-15=-129(F=-99), 10-14=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 14=-231(F) 10=-539(F)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

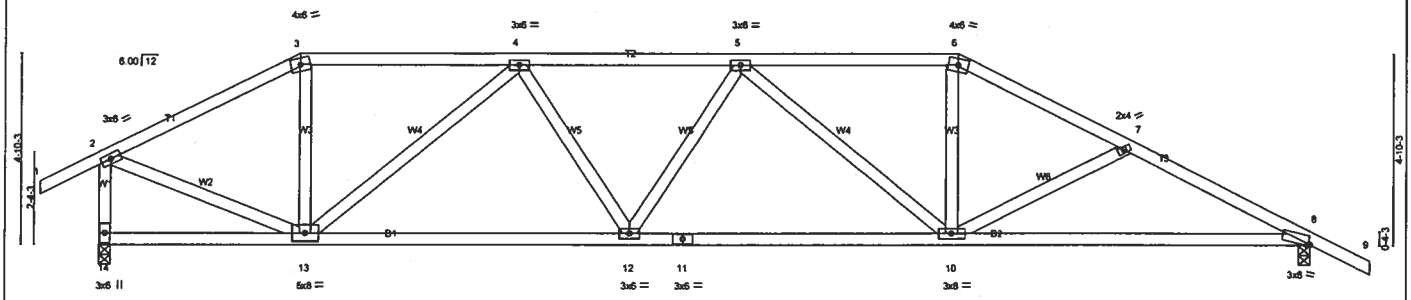
Dwg.#0306061307

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:33 2006 Page 1

-1-6-0	4-11-15	10-6-10	16-1-5	21-8-0	25-10-12	30-8-0	32-2-0
1-6-0	4-11-15	5-6-11	5-6-11	5-6-11	4-2-12	4-9-4	1-6-0

Scale = 1:55.8



4-11-15	13-4-0	21-8-0	30-8-0
4-11-15	8-4-0	8-4-0	9-0-0

Plate Offsets (X,Y): [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.20	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.33	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-14 oc bracing.

REACTIONS (lb/size) 14=1365/0-3-8, 8=1365/0-3-8
Max Horz 14=-147(load case 3)
Max Uplift14=-543(load case 5), 8=-598(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1394/758, 3-4=-1205/743, 4-5=-2092/1162, 5-6=-1825/1041, 6-7=-2069/1085, 7-8=-2307/1226, 8-9=0/35, 2-14=-1307/811
BOT CHORD 13-14=-42/140, 12-13=-782/1912, 11-12=-882/2122, 10-11=-882/2122, 8-10=-930/2015
WEBS 3-13=-82/369, 4-13=-959/500, 4-12=-68/359, 5-12=-72/125, 5-10=-479/326, 6-10=-225/631, 7-10=-238/271, 2-13=-508/1261

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 543 lb uplift at joint 14 and 598 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

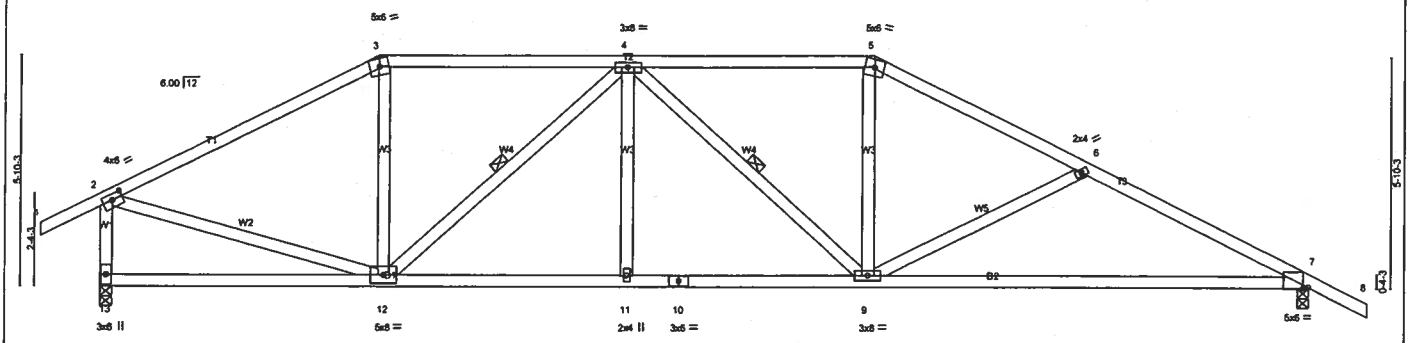
Dwg.#0306061308

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:34 2006 Page 1

-1-6-0	6-11-15	13-4-0	19-8-0	24-10-12	30-8-0	32-2-0
1-6-0	6-11-15	6-4-0	6-4-0	5-2-12	5-9-4	1-6-0

Scale = 1:55.8



6-11-15	13-4-0	19-8-0	30-8-0
6-11-15	6-4-0	6-4-0	11-0-0

Plate Offsets (X,Y): [2:0-3-0,0-1-12], [7:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.38	7-9	>970	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.64	7-9	>568	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-12 oc bracing.
WEBS 1 Row at midpt 4-12, 4-9

REACTIONS (lb/size) 13=1365/0-3-8, 7=1365/0-3-8
Max Horz 13=-165(load case 3)
Max Uplift13=-567(load case 5), 7=-618(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1504/827, 3-4=-1277/830, 4-5=-1670/1007, 5-6=-1920/1038, 6-7=-2243/1232, 7-8=0/35, 2-13=-1259/827
BOT CHORD 12-13=-101/157, 11-12=-656/1739, 10-11=-656/1739, 9-10=-656/1739, 7-9=-924/1962
WEBS 3-12=-51/352, 4-12=-690/334, 4-11=0/138, 4-9=-221/201, 5-9=-158/520, 6-9=-347/367, 2-12=-462/1200

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 567 lb uplift at joint 13 and 618 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061309

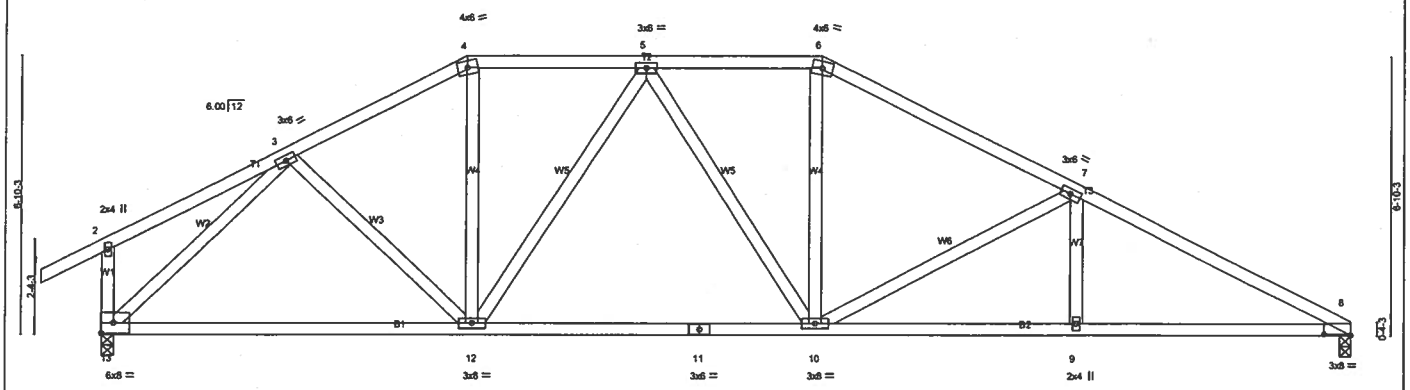
Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T17	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:34 2006 Page 1

-1-6-0 | 4-7-12 | 8-11-15 | 13-4-0 | 17-8-0 | 23-10-12 | 30-8-0

1-6-0 | 4-7-12 | 4-4-4 | 4-4-0 | 4-4-0 | 6-2-12 | 6-9-4

Scale = 1:54.0



8-11-15 | 17-8-0 | 23-10-12 | 30-8-0

8-11-15 | 8-8-1 | 6-2-12 | 6-9-4

Plate Offsets (X,Y): [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.16	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.26	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-1 oc bracing.

REACTIONS (lb/size) 8=1273/0-3-8, 13=1367/0-3-8
Max Horz 13=-164(load case 3)
Max Uplift8=-518(load case 6), 13=-590(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-185/200, 3-4=-1461/881, 4-5=-1270/852, 5-6=-1481/977, 6-7=-1727/1002, 7-8=-2346/1249, 2-13=-280/352
BOT CHORD 12-13=-483/1084, 11-12=-570/1440, 10-11=-570/1440, 9-10=-1002/2021, 8-9=-1002/2021
WEBS 3-12=-95/320, 4-12=-154/383, 5-12=-388/231, 5-10=-81/133, 6-10=-165/445, 7-10=-628/483, 7-9=0/218, 3-13=-1363/714

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 518 lb uplift at joint 8 and 590 lb uplift at joint 13.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061310

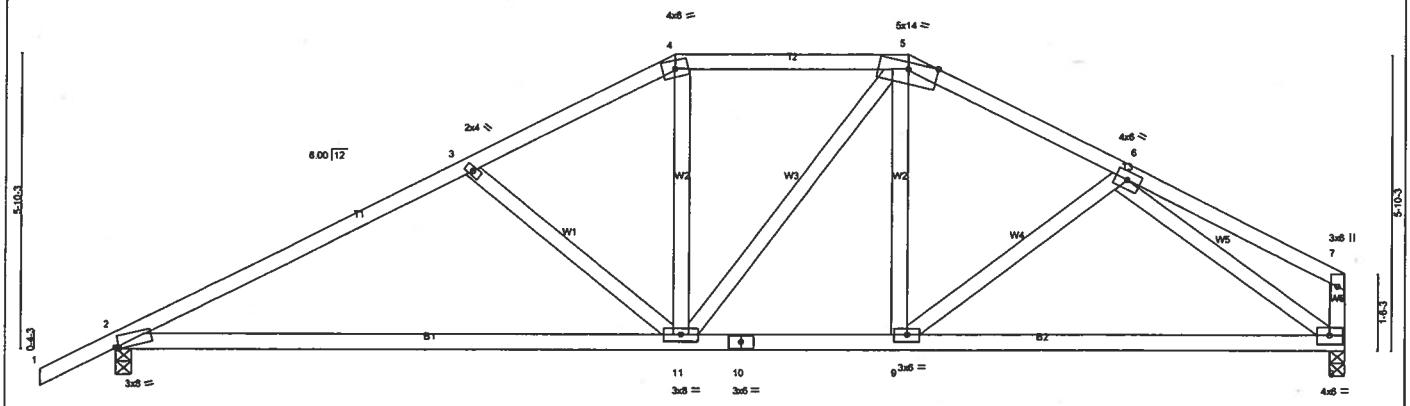
Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T18	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:35 2006 Page 1

-1-6-0 | 7-0-0 | 10-11-15 | 15-8-0 | 19-10-4 | 24-4-0

1-6-0 | 7-0-0 | 3-11-15 | 4-8-1 | 4-2-4 | 4-5-12

Scale = 1:43.6



10-11-15 | 15-8-0 | 24-4-0

10-11-15 | 4-8-1 | 8-8-0

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.39	2-11	>742	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.66	2-11	>435	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 129 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-12 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 8=1007/0-3-8
Max Horz 2=196(load case 5)
Max Uplift2=-532(load case 5), 8=-393(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1619/901, 3-4=-1327/766, 4-5=-1134/731, 5-6=-1198/715, 6-7=-346/132, 7-8=-256/163
BOT CHORD 2-11=-758/1397, 10-11=-440/1032, 9-10=-440/1032, 8-9=-548/1012
WEBS 3-11=-352/373, 4-11=-156/384, 5-11=-92/248, 5-9=-58/151, 6-9=-32/159, 6-8=-966/642

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 2 and 393 lb uplift at joint 8.

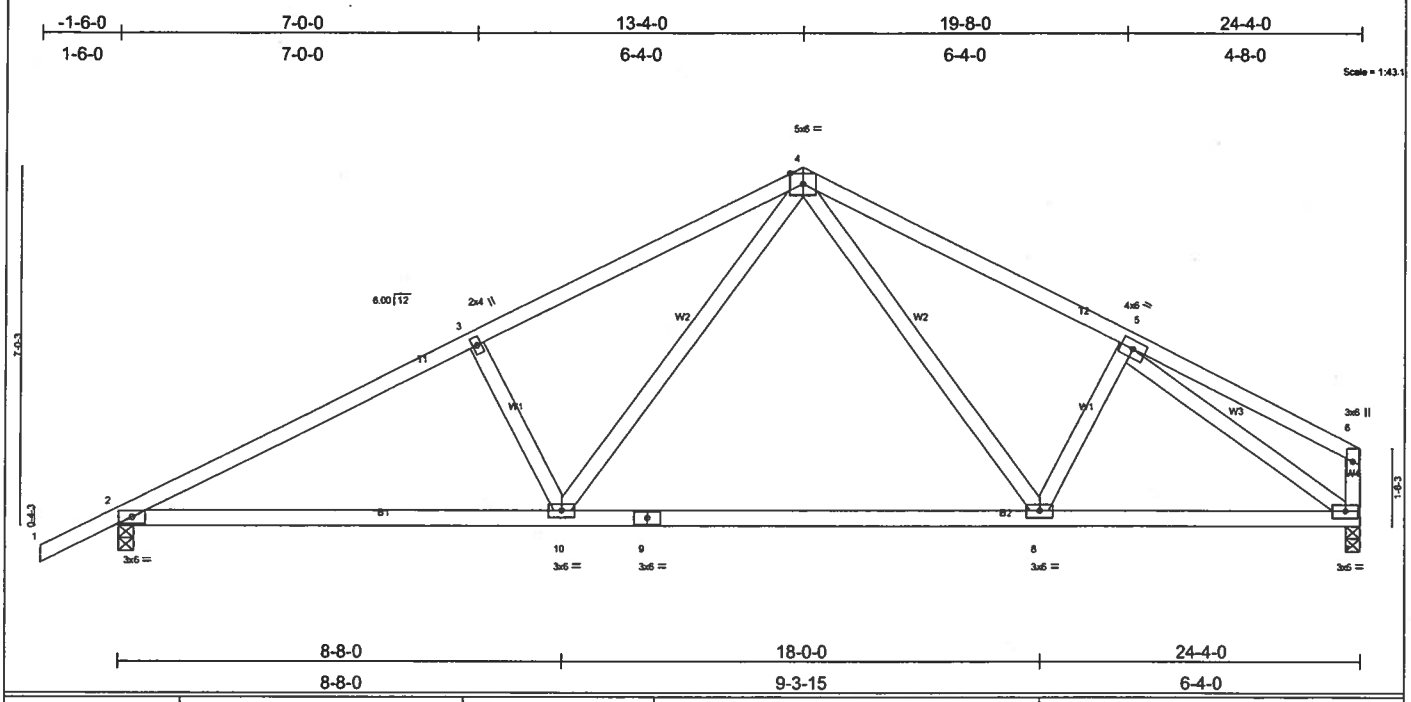
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061311

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T19	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:35 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.17	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.29	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-6 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 7=1007/0-3-8
Max Horz 2=215(load case 5)
Max Uplift2=-546(load case 5), 7=-411(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1712/918, 3-4=-1547/946, 4-5=-1277/807, 5-6=-193/147, 6-7=-175/160
BOT CHORD 2-10=-776/1456, 9-10=-393/901, 8-9=-393/901, 7-8=-576/1084
WEBS 3-10=-342/390, 4-10=-368/741, 4-8=-172/377, 5-8=-78/220, 5-7=-1216/649

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 546 lb uplift at joint 2 and 411 lb uplift at joint 7.

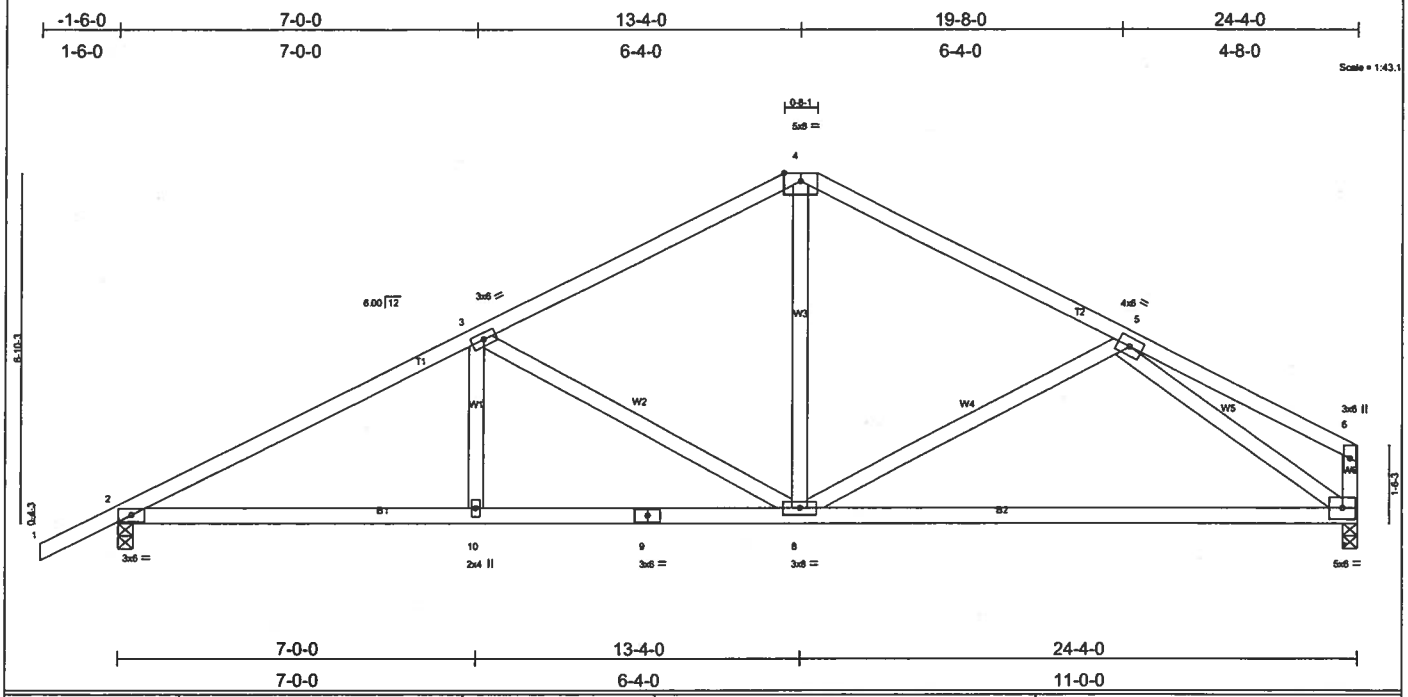
LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061312

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T19A	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:36 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.25	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.43	7-8	>673	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 124 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-5 oc bracing.

REACTIONS (lb/size) 2=1102/0-3-8, 7=1007/0-3-8
Max Horz 2=215(load case 5)
Max Uplift2=-546(load case 5), 7=-411(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1731/915, 3-4=-1142/699, 4-5=-1139/692, 5-6=-399/83, 6-7=-265/125
BOT CHORD 2-10=-772/1470, 9-10=-772/1470, 8-9=-772/1470, 7-8=-590/1032
WEBS 3-10=0/195, 3-8=-616/448, 5-8=-166/252, 5-7=-918/738, 4-8=-270/600

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 546 lb uplift at joint 2 and 411 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

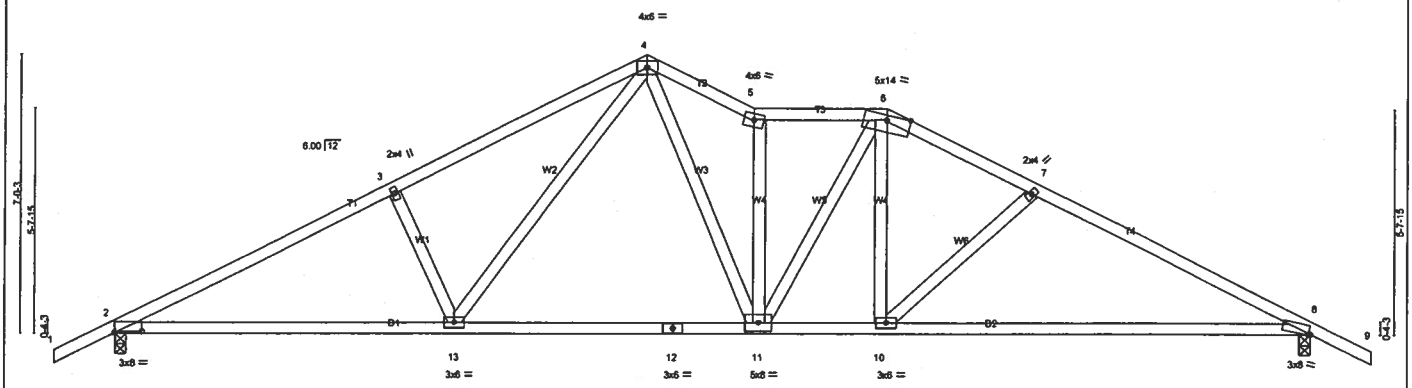
Dwg.#0306061313

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:36 2006 Page 1

-1-6-0	7-0-0	13-4-0	16-0-8	19-4-8	23-0-0	30-0-0	31-6-0
1-6-0	7-0-0	6-4-0	2-8-8	3-4-0	3-7-8	7-0-0	1-6-0

Scale = 1:55.3



8-6-5	16-0-8	19-4-8	30-0-0
8-6-5	7-6-3	3-4-0	10-7-8

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.38	8-10	>943	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.64	8-10	>558	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 158 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 2=1337/0-3-8, 8=1337/0-3-8
Max Horz 2=145(load case 5)
Max Uplift2=-627(load case 5), 8=-655(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2223/1185, 3-4=-2069/1227, 4-5=-1935/1213, 5-6=-1707/1054, 6-7=-1871/1062, 7-8=-2140/1176, 8-9=0/35
BOT CHORD 2-13=-868/1910, 12-13=-475/1361, 11-12=-475/1361, 10-11=-596/1626, 8-10=-856/1857
WEBS 3-13=-334/399, 4-13=-393/723, 4-11=-516/968, 5-11=-946/610, 6-11=-129/176, 6-10=-200/517, 7-10=-336/355

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 2 and 655 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

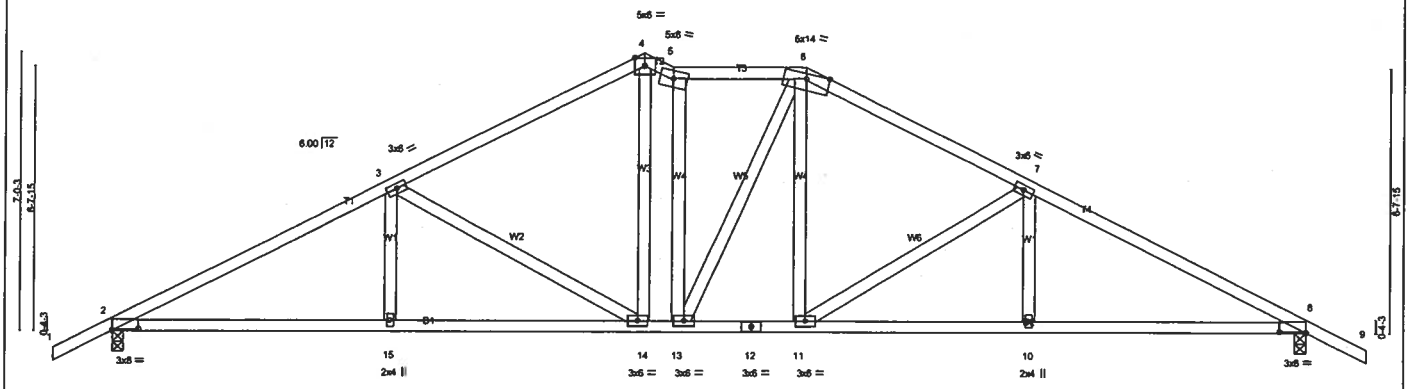
Dwg.#0306061314

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T21	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:37 2006 Page 1

-1-6-0	7-0-0	13-4-0	14-0-8	17-4-8	23-0-0	30-0-0	31-6-0
1-6-0	7-0-0	6-4-0	0-8-8	3-4-0	5-7-8	7-0-0	1-6-0

Scale = 1:55.3



7-0-0	14-0-8	17-4-8	23-0-0	30-0-0
7-0-0	7-0-8	3-4-0	5-7-8	7-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.15	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.24	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-4 oc bracing.

REACTIONS (lb/size) 2=1337/0-3-8, 8=1337/0-3-8
Max Horz 2=145(load case 5)
Max Uplift2=-627(load case 5), 8=-655(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2246/1172, 3-4=-1647/972, 4-5=-1332/851, 5-6=-1403/932, 6-7=-1673/987, 7-8=-2248/1171, 8-9=0/35
BOT CHORD 2-15=-855/1927, 14-15=-855/1927, 13-14=-479/1407, 12-13=-502/1441, 11-12=-502/1441, 10-11=-853/1928, 8-10=-853/1928
WEBS 3-15=0/229, 3-14=-630/431, 5-13=-159/78, 6-13=-235/128, 6-11=-205/492, 7-11=-587/418, 7-10=0/220, 4-14=-252/642

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 627 lb uplift at joint 2 and 655 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061315

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:37 2006 Page 1

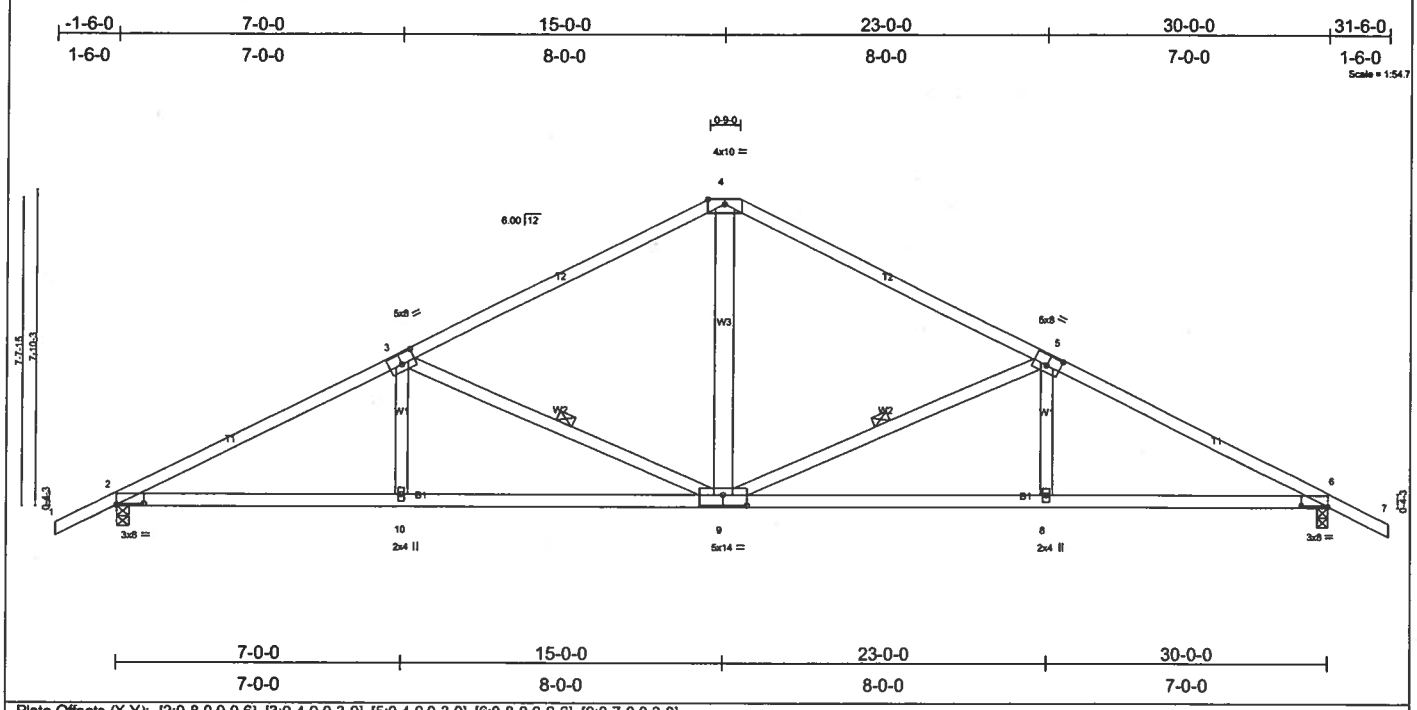


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-4-0,0-3-0], [5:0-4-0,0-3-0], [6:0-8-0,0-0-6], [9:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.17	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.28	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.09	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-11 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS (lb/size) 2=1337/0-3-8, 6=1337/0-3-8
Max Horz 2=159(load case 5)
Max Uplift2=-639(load case 5), 6=-639(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2273/1204, 3-4=-1529/917, 4-5=-1529/917, 5-6=-2273/1204, 6-7=0/35
BOT CHORD 2-10=-890/1956, 9-10=-889/1960, 8-9=-889/1960, 6-8=-890/1956
WEBS 3-10=0/242, 3-9=-772/526, 5-9=-772/526, 5-8=0/242, 4-9=-371/835

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 639 lb uplift at joint 2 and 639 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061316

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T23	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 03 17:23:38 2006 Page 1

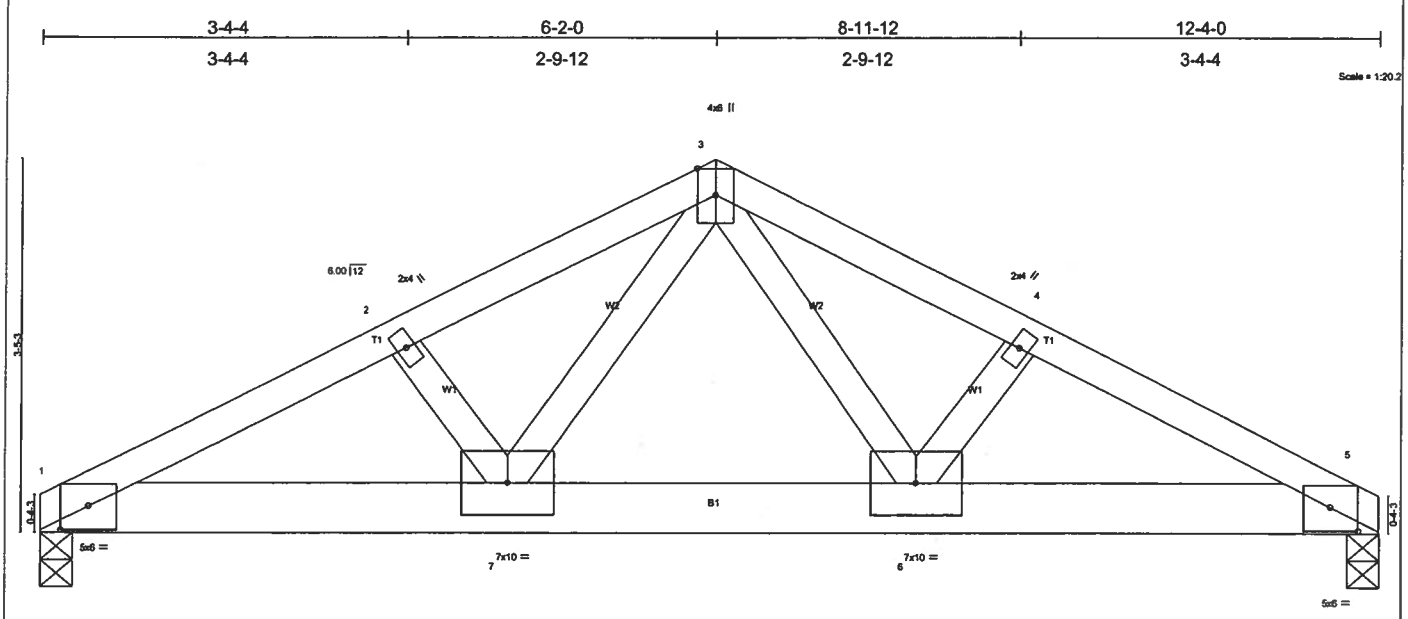


Plate Offsets (X,Y): [1:0-3-0,0-2-9], [5:0-3-0,0-2-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.08	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.12	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 128 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4009/0-3-8, 5=4009/0-3-8
Max Horz 1=-53(load case 2)
Max Uplift1=-1901(load case 4), 5=-1901(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6202/2933, 2-3=-6117/2931, 3-4=-6117/2931, 4-5=-6202/2934
BOT CHORD 1-7=-2623/5511, 6-7=-1772/3856, 5-6=-2572/5511
WEBS 2-7=-95/138, 3-7=-1477/3072, 3-6=-1477/3071, 4-6=-95/138

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1901 lb uplift at joint 1 and 1901 lb uplift at joint 5.
6) Girder carries tie-in span(s): 30-0-0 from 0-0-0 to 12-4-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-612(F=-582)

MARCH 6,2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061317

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T24	COMMON	1	1	Job Reference (optional)

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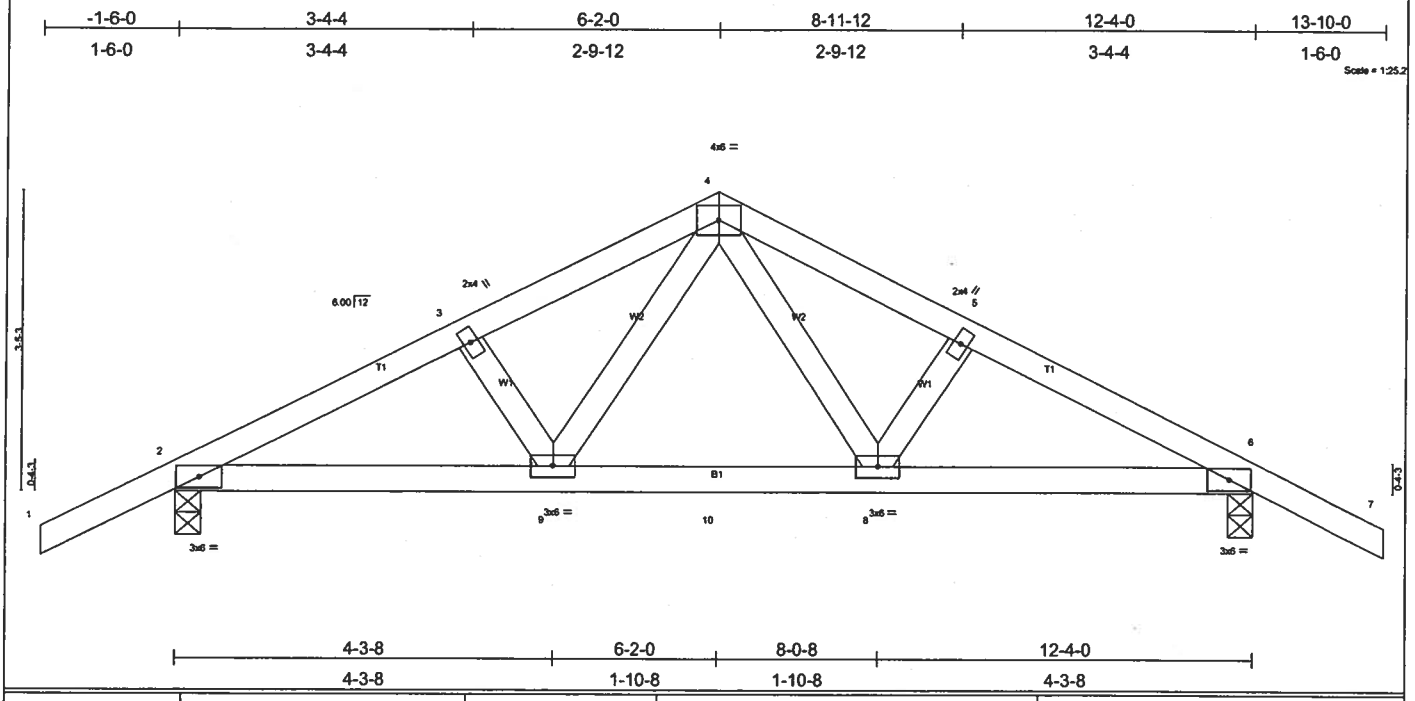

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	0.12	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.17	8-9	>838	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 59 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-9 oc bracing.

REACTIONS (lb/size) 2=997/0-3-8, 6=998/0-3-8
Max Horz 2=-85(load case 5)
Max Uplift2=-591(load case 4), 6=-591(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1734/894, 3-4=-1644/900, 4-5=-1644/901, 5-6=-1734/895, 6-7=0/35
BOT CHORD 2-9=-752/1477, 9-10=-501/1083, 8-10=-501/1083, 6-8=-709/1477
WEBS 3-9=-51/87, 4-9=-399/735, 4-8=-399/735, 5-8=-51/88

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 591 lb uplift at joint 2 and 591 lb uplift at joint 6.
4) Girder carries hip end with 6-2-0 end setback.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 806 lb down and 520 lb up at 6-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-6=-30
Concentrated Loads (lb)
Vert: 10=-806(F)

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Dwg.#0306061318

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T25	COMMON	1	1	Job Reference (optional)

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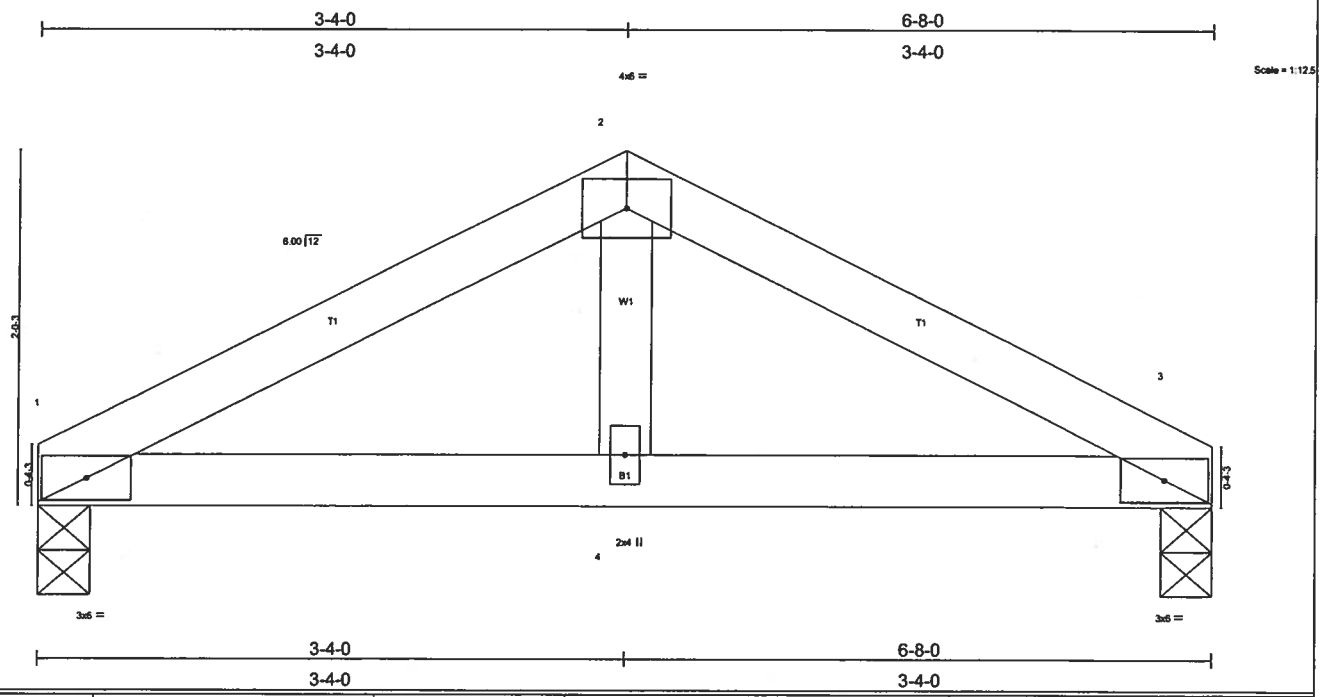

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	-0.02	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.03	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 24 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=583/0-3-8, 3=583/0-3-8
Max Horz 1=-29(load case 2)
Max Uplift1=-264(load case 4), 3=-264(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-745/313, 2-3=-745/312
BOT CHORD 1-4=-238/628, 3-4=-238/628
WEBS 2-4=-162/501

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 1 and 264 lb uplift at joint 3.
4) Girder carries tie-in span(s): 7-0-0 from 0-0-0 to 6-8-0
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 1-3=-129(F=-99)

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Dwg.#0306061319

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T26	COMMON	1	1	Job Reference (optional)

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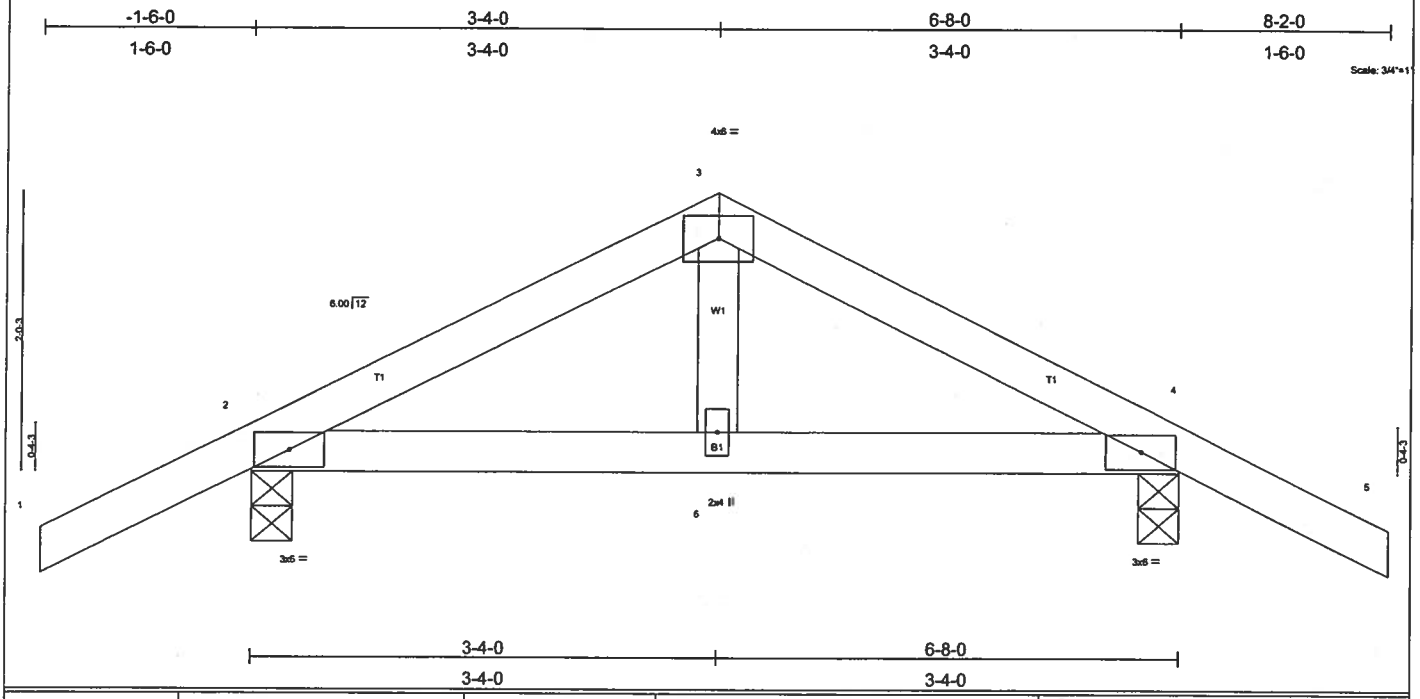

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.00	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=357/0-3-8, 4=357/0-3-8
Max Horz 2=62(load case 5)
Max Uplift2=-236(load case 5), 4=-236(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-307/142, 3-4=-307/142, 4-5=0/35
BOT CHORD 2-6=0/225, 4-6=0/225
WEBS 3-6=0/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 236 lb uplift at joint 2 and 236 lb uplift at joint 4.

LOAD CASE(S) Standard

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Dwg.#0306061320

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T26G	COMMON	1	1	Job Reference (optional)

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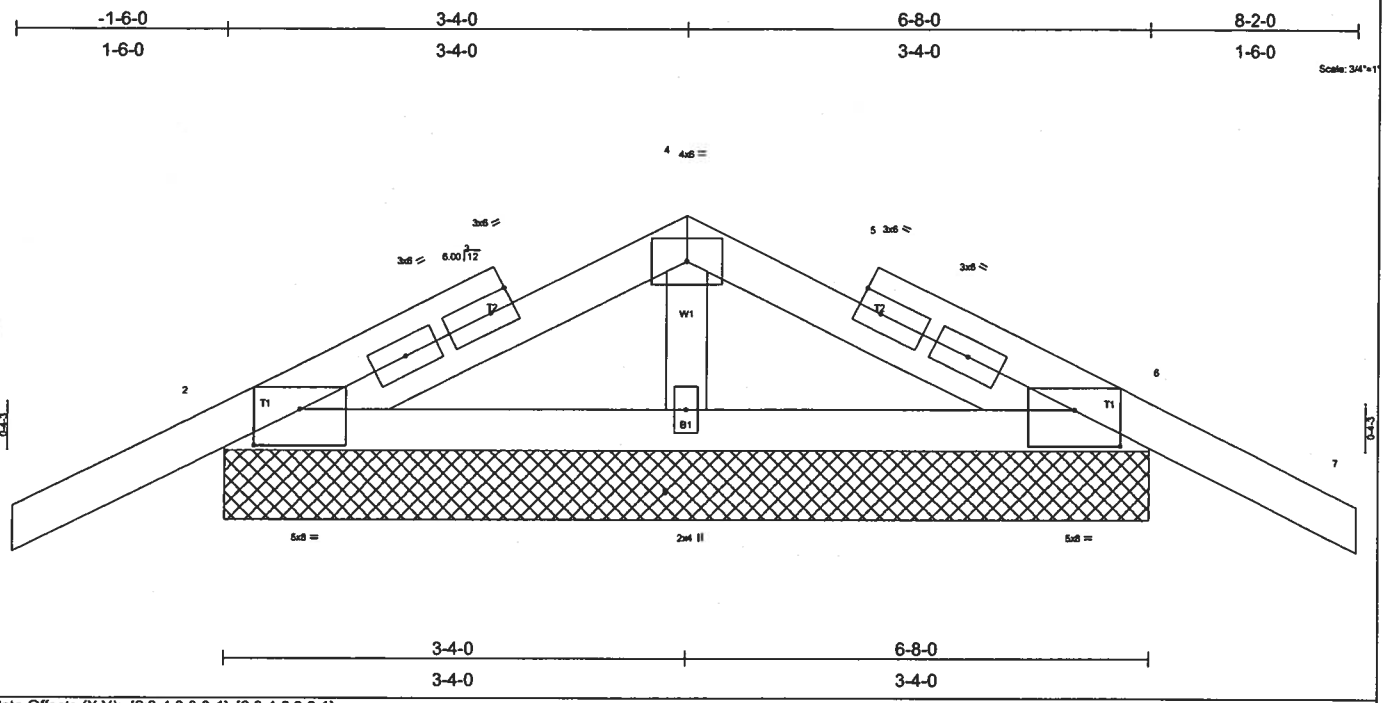

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [6:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.25	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=309/6-8-0, 6=310/6-8-0, 8=422/6-8-0
Max Horz 2=56(load case 5)
Max Uplift2=-240(load case 5), 6=-250(load case 6), 8=-137(load case 5)
Max Grav 2=317(load case 9), 6=317(load case 10), 8=422(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-11/56, 2-3=-35/85, 3-4=-44/125, 4-5=-44/125, 5-6=-37/85, 6-7=-11/56
BOT CHORD 2-8=-79/164, 6-8=-79/164
WEBS 4-8=-297/261

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 2, 250 lb uplift at joint 6 and 137 lb uplift at joint 8.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 6.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-30

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061321

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T27	COMMON	1	1	Job Reference (optional)

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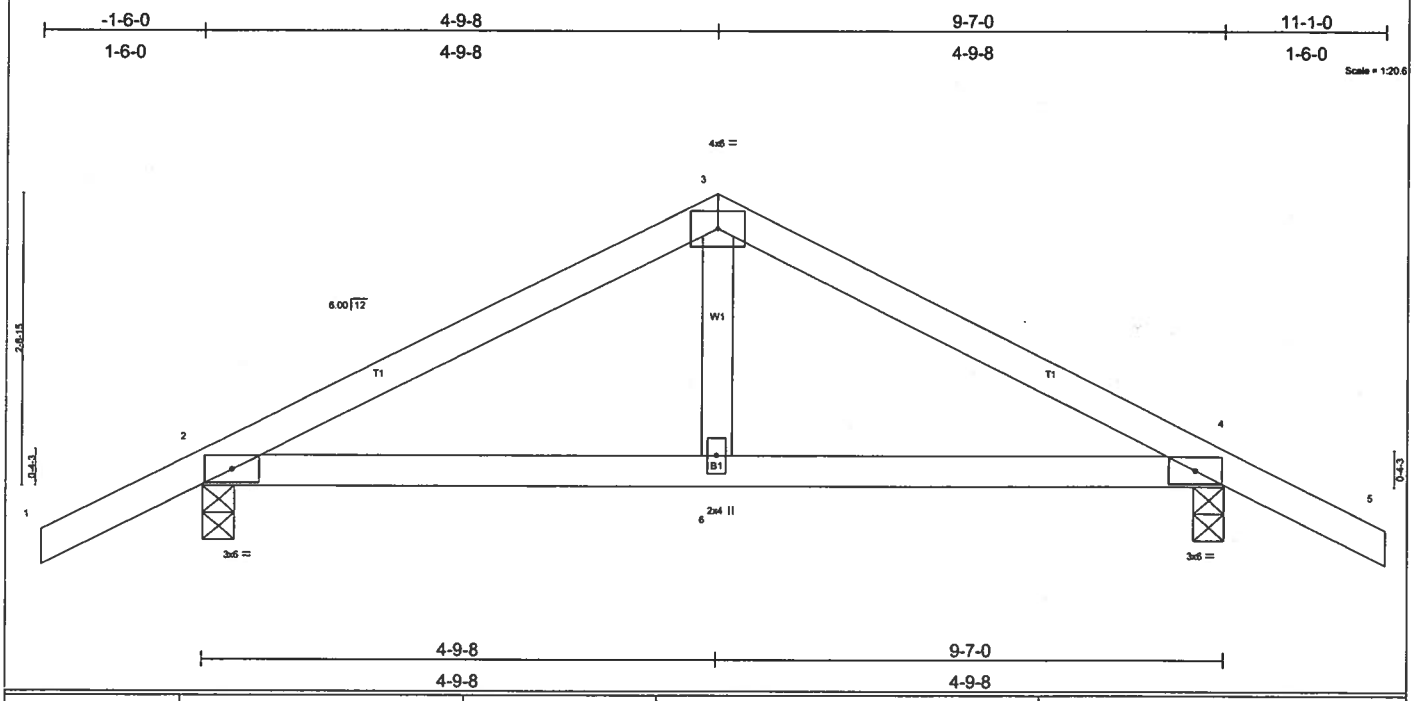

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	0.04	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-9-15 oc bracing.

REACTIONS (lb/size) 2=479/0-3-8, 4=479/0-3-8
Max Horz 2=-74(load case 6)
Max Uplift2=-423(load case 5), 4=-423(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-520/728, 3-4=-520/728, 4-5=0/35
BOT CHORD 2-6=-488/414, 4-6=-488/414
WEBS 3-6=-345/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 2 and 423 lb uplift at joint 4.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0306061322

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T27G	COMMON	1	1	Job Reference (optional)

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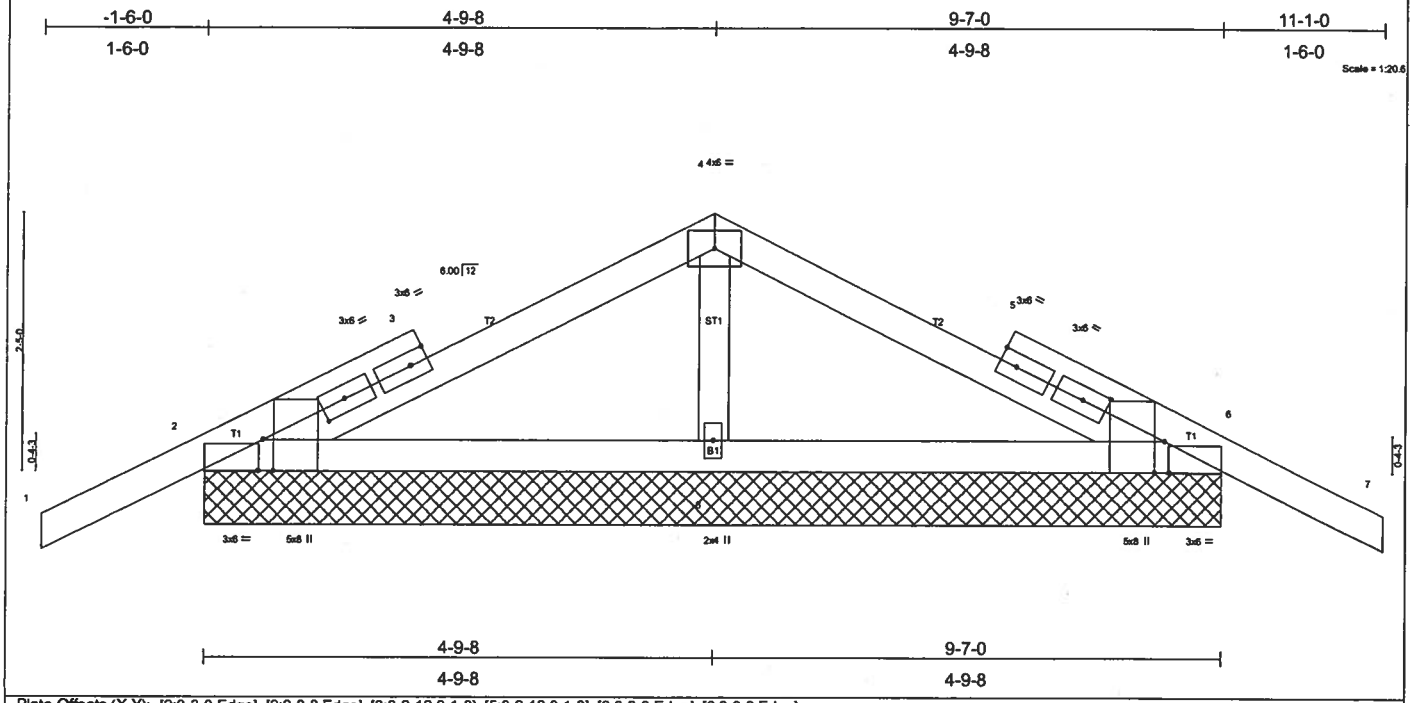

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-12,0-1-8], [5:0-2-12,0-1-8], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.02	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	0.03	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=590/9-7-0, 6=590/9-7-0, 8=203/9-7-0
Max Horz 2=-68(load case 6)
Max Uplift2=-380(load case 5), 6=-380(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-9/56, 2-3=-618/498, 3-4=-555/491, 4-5=-555/491, 5-6=-618/498, 6-7=-9/56
BOT CHORD 2-8=-311/506, 6-8=-311/506

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 2 and 380 lb uplift at joint 6.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-87(F=-33), 4-7=-87(F=-33), 2-6=-30

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Dwg.#0306061323

Job	Truss	Truss Type	Qty	Ply	HUGO- LOT 7 FORT WHITE PARK
L153960	T28	COMMON	4	1	Job Reference (optional)

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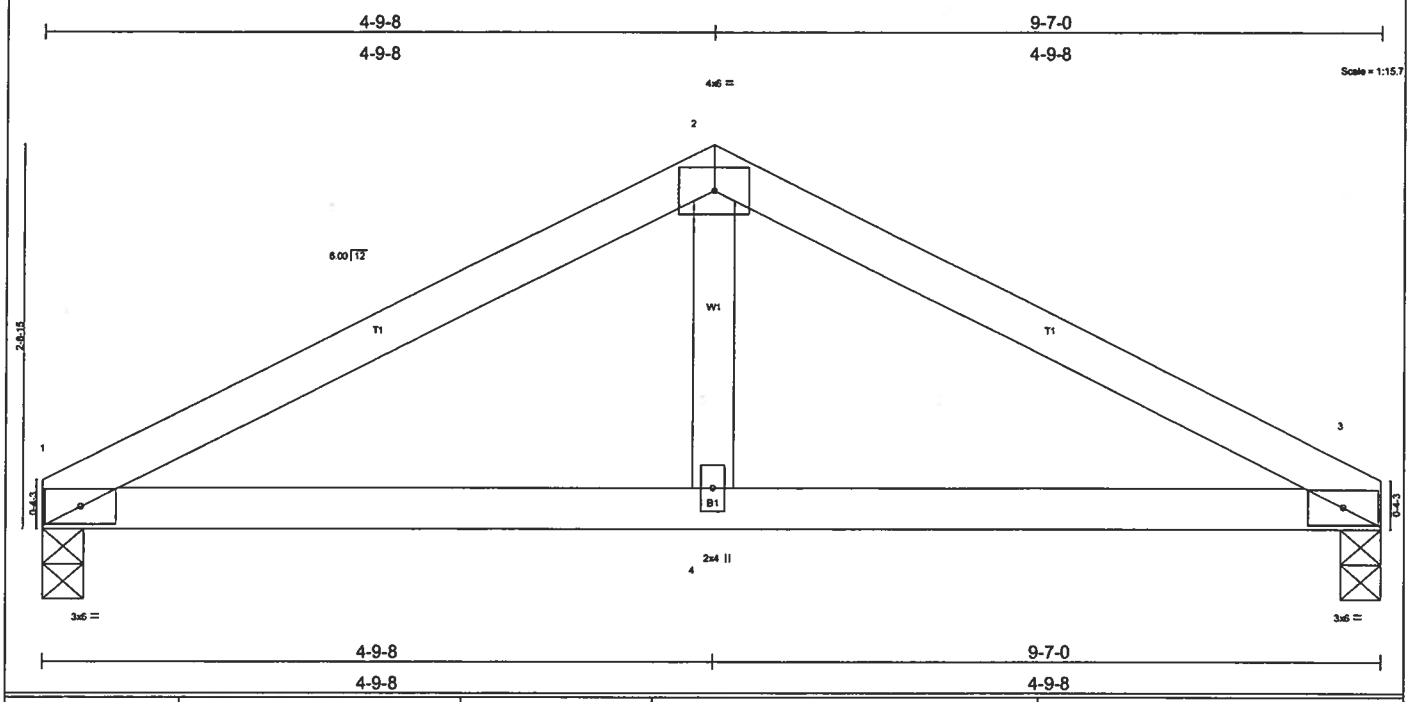

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.06	1-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	0.05	1-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-11 oc bracing.

REACTIONS (lb/size) 1=390/0-3-8, 3=390/0-3-8
Max Horz 1=-42(load case 3)
Max Uplift1=-302(load case 5), 3=-302(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-565/816, 2-3=-565/816
BOT CHORD 1-4=-644/458, 3-4=-644/458
WEBS 2-4=-393/186

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 1 and 302 lb uplift at joint 3.

LOAD CASE(S) Standard

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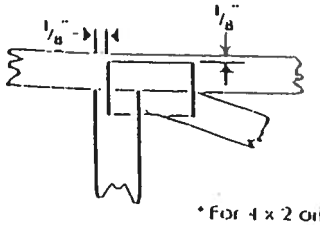
Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

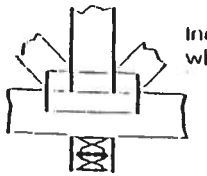
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

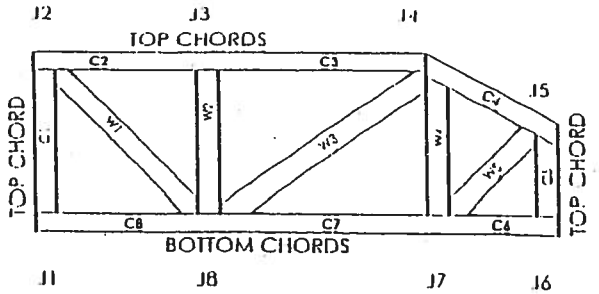
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

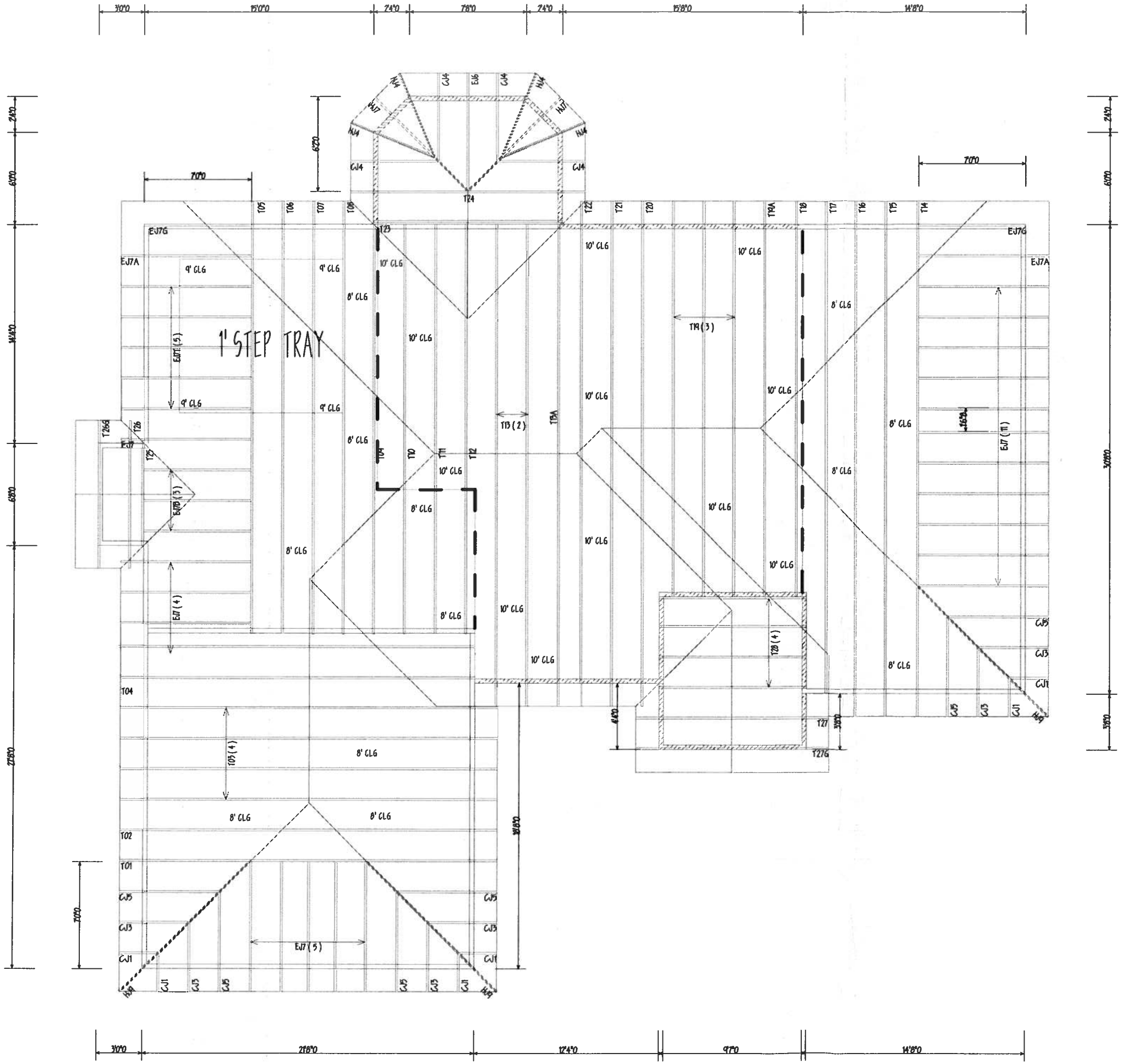
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

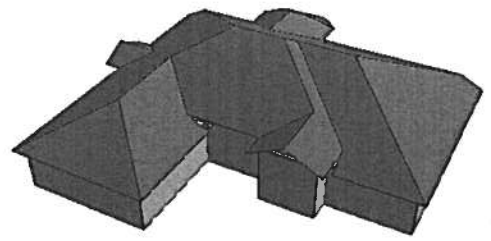
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.





BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

6/12 PITCH

1'-6" OH

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904 772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407 322-0059 FAX: 407 322-5553

BUILDER:	HUGO ESCALANTE
LEGAL ADDRESS:	LOT 7 FORT WHITE PARK
MODEL: NICOLAS	REVISION:
	SCALE: NTS
DATE: 3-3-06	DRAWN BY: JRD
	JOB #: L153960