

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d	-	
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB	

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

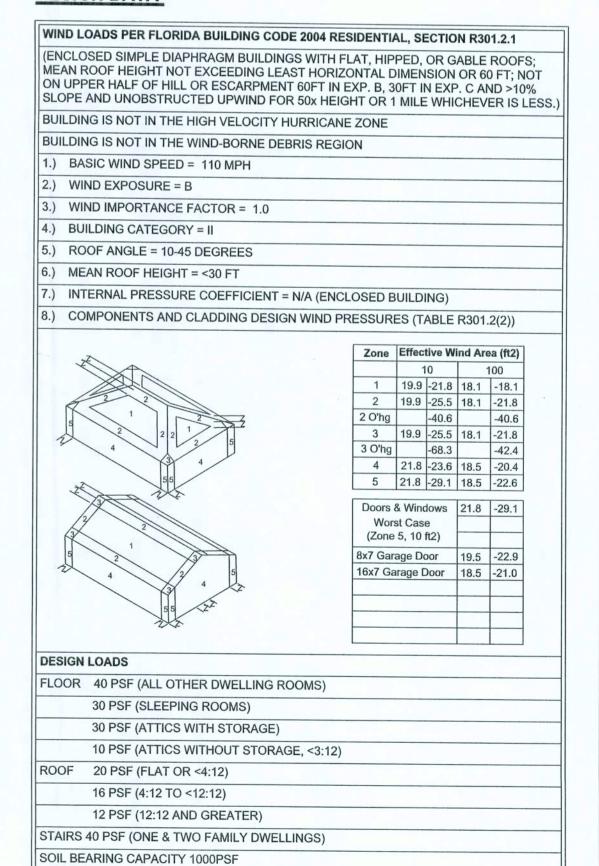
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA



NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB368, Lake City, FL 32056, 386-754-549 DIMENSIONS tated dimensions upercede scaled dimensions. Refer a questions to Mark Disosway, P.E for resolution. Do not proceed without clarification. lark Disosway, P.E hereby expressly reser s common law copyrights and property right hese instruments a service. This document i ot to be reproduce, altered or copied in any orm or manner without first the express writt mission and conent of Mark Disosway. ERTIFICATION: I iereby certify that I have mined this plan, and that the applicable ions of the plan, relating to wind engineer emply with sectionR301.2.1, florida building ode residential 200, to the best of my IMITATION: This design is valid for one ouilding, at specified location. PE. 53915

REVISONS

SOFTPLAN

Milton Builders

Rowand Residence

AIDRESS: Lot 7 Lakevood Estates S/D

ColumbiaCounty, Florida Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056

Fax: (386) 269 - 4871 PRIN'ED DATE: March 2, 2007

Phone: (316) 754 - 5419

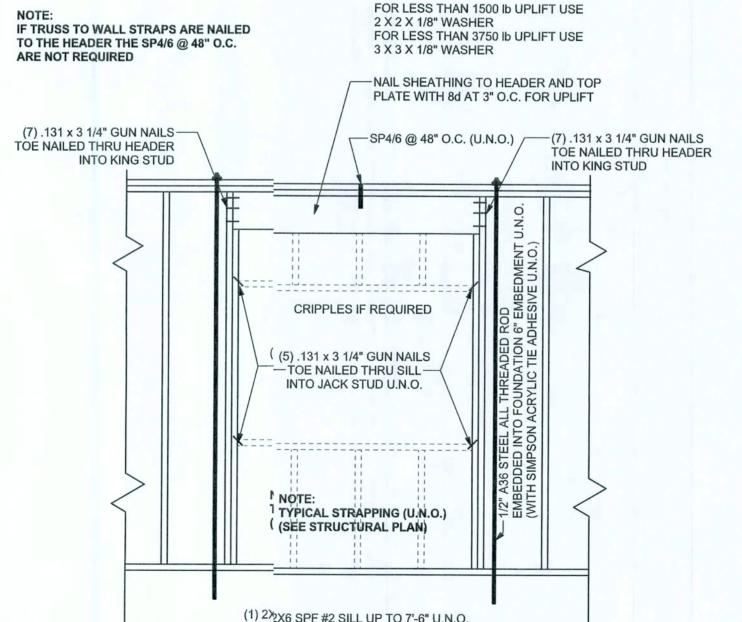
STRUCTURAL BY David Disoswa

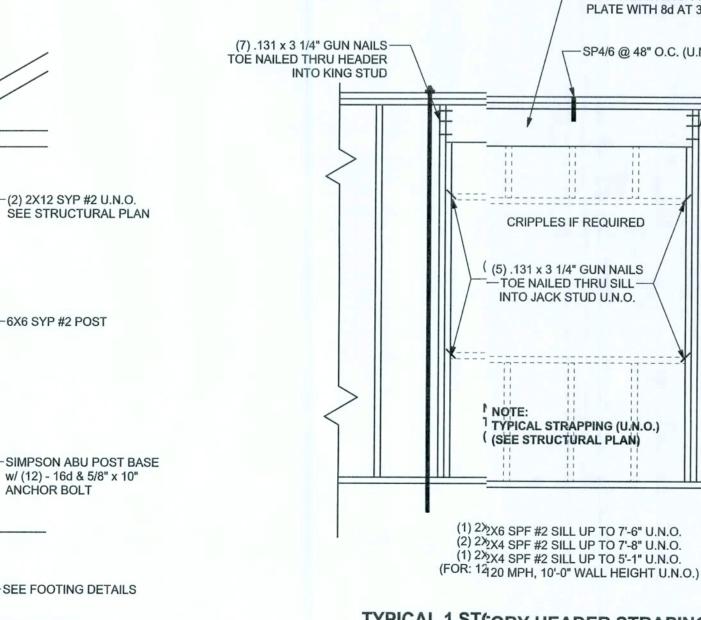
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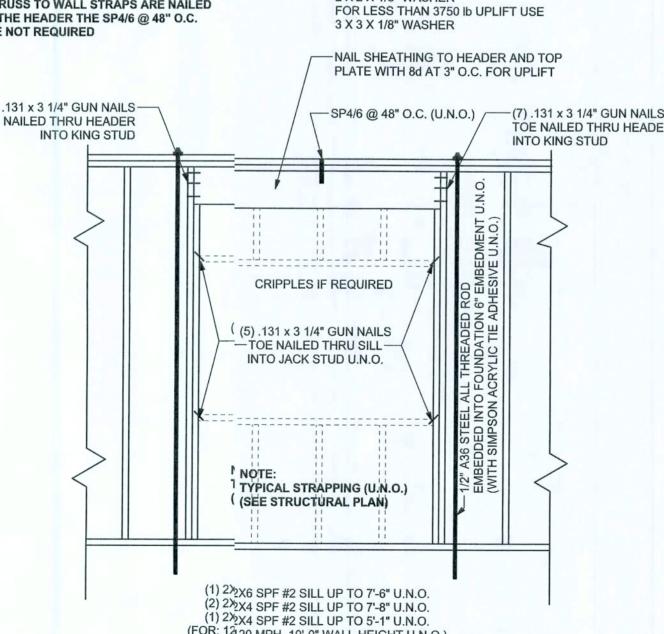
DRAWING NUMBER

OF 3SHEETS

7(3091







TYPICAL 1 STCORY HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

SYP #2

SYP #2

SYP #2

24F-V3 SP

LSL TIMBERSTRAND 1700

MICROLAM

PARALAM

Fb (psi) E (10⁶ psi

1.6

1.6

2.0

1200

1050

975

2400

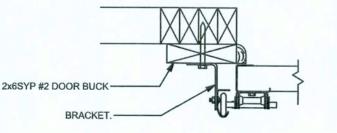
2900

2900

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT RANSFER LOAD, CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW:

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN	
8' - 10'	24" O.C.	5" O.C.	5" O.C.	
11' - 15'	18" O.C.	4" O.C.	4" O.C.	
16' - 18'	16" O.C.	3" O.C.	3" O.C.	



GARAGE DOOR BUCK INSTALLATION DETAIL

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN SIMPSON H2.5A U.N.O. SEE STRUCTURAL PLAN SEE STRUCTURAL PLAN (1-ONE SIDE, 2-ON -OPPOSITE SIDE) EA NAILED WITH 14-10d COLUMN TOGETHER W/2-16d NAILS AT 16" O.C. MIN. (SEE STRUCTURAL PLAN) SUPPORTIVE POST TO BEAM **BEAM MID-WALL CONNECTION DETAIL DETAIL FOR SINGLE BEAM** (2) SIMPSON LSTA21w/ (8) -16d TO HEADER SCALE: N.T.S. SCALE: N.T.S. AND (8) -16d TO POST

IF BEAM JOINT IS AT -

POST CONNECTION. INSTALL ONE SIMPSON

LSTA18 ON ONE SIDE

SUPPORTIVE -

ALL STUDS TO BE 2x4 ———

CONTINUOUS FRAME TO

CEILING DIAPHRAGM DETAI

- NON-SUPPORTIVE

2X4 LADDER BEAM

SPF NAILED TO TOP

AND BOTTOM PLATES

WITH 2-16d NAILS

SCALE: N.T.S.

SEE STRUCTURAL PLAN BEAM W/4-16d SEE STRUCTURAL PLAN BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

BEAM CORNER CONNECTION. DETAIL

(2-ONE SIDE,2-ON

OTHER SIDE)

SUPPORTIVE CENTER POST TO BEAM ETAIL

SUPPORTIVE BEAM -

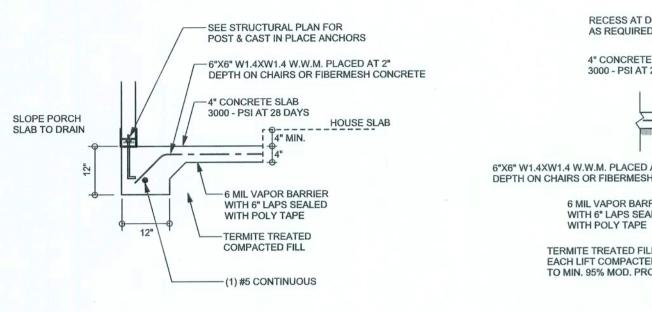
TYPICAL PORCH POST DETAIL

-(2) 2X12 SYP #2 U.N.O.

--- 6X6 SYP #2 POST

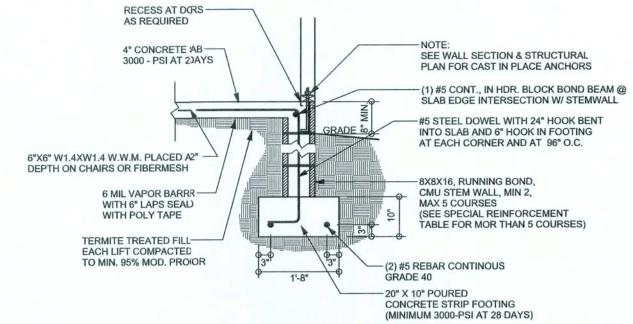
ANCHOR BOLT

-SEE FOOTING DETAILS



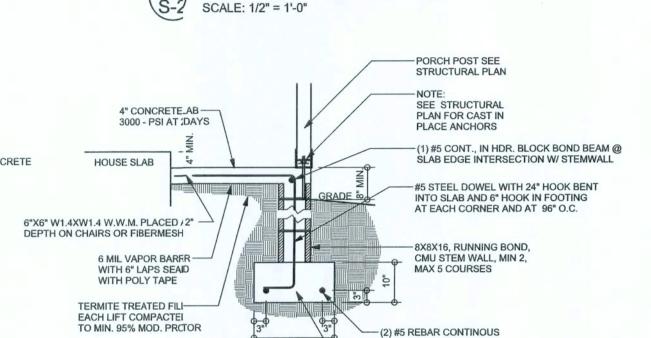
SEE INTERIOR WALL SECTION

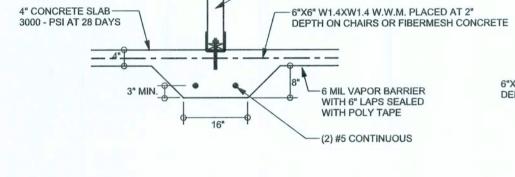
& STRUCTURAL PLAN FOR ANCHORS



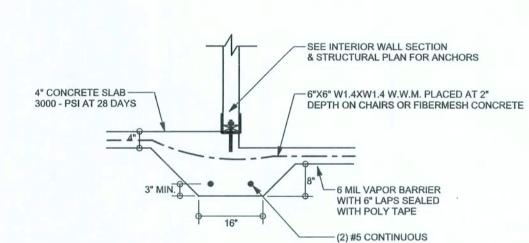
STEM WALL FOOTING

F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

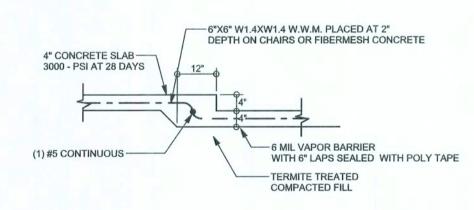








F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



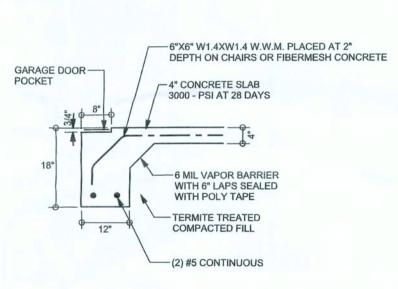
F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"

F1 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

GRADE 40

— 20" X 10" POURED CONCRETE STRIP FOOTING

(MINIMUM 3000-PSI AT 28 DAYS)

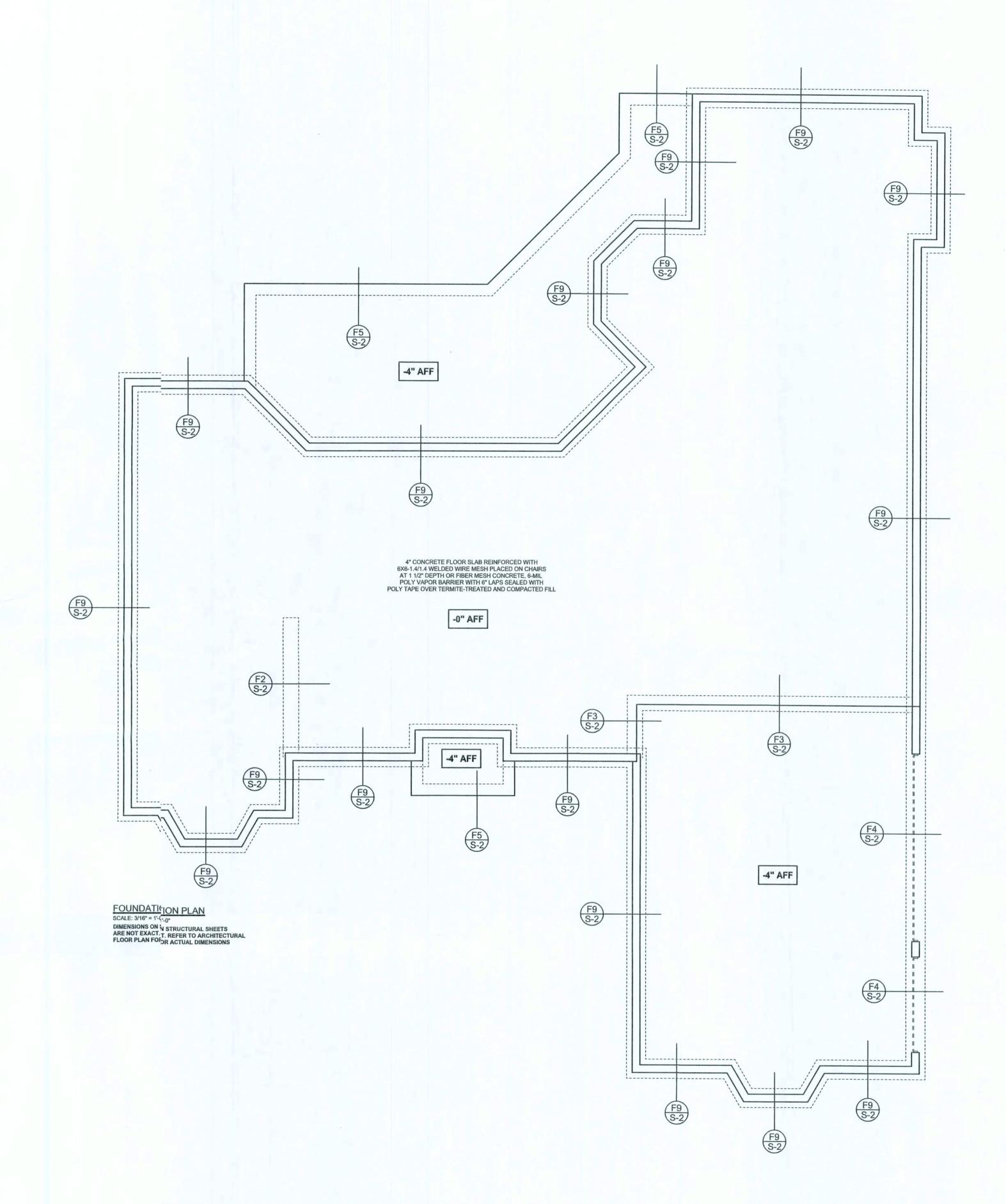


F4 ALT. STEM WALL GARAGE DOOR FOOTING
S-2 CALE: 1/2" = 1'-0"

TALL STM WALL TABLE

The table assumes) ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at tltop. The vertical steel is to be placed toward the tension side of the CMU wall (away fix the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, addurowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 conuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement shown in the table below.

STEMWALL HEIGHT (FEET)	UNEANCED B/KFILL HGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	1.7	96	96	96	96	96	96
4.7	1.3	88	96	96	96	96	96
5.3	i.0	56	96	96	96	96	96
6.0	i.7	40	80	96	80	96	96
6.7	1.3	32	56	80	56	96	96
7.3	'.0	24	40	56	40	80	96
8.0	'.7	16	32	48	32	64	80
8.7	1.3	8	24	32	24	48	64
9.3	1.0	8	16	24	16	40	48



REVISIONS

SOFTPIXN

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, IOB 868, Lake City, FL 32056, 386-7545419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Reer all questions to Mark Disosway P.E. for resolution. Do not proceedwithout clarification.

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permission and onsent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this ran, and that the applicable portions of the lan, relating to wind engineering comply with section R301.2.1, florida building code residentia 2004, to the best of my

LIMITATION: Tils design is valid for one building, at speified location.

MARK DISOSWAY
P.E. 53915

WARO

SEAL

Milton Builders

Rowind Residence

ADDRESS: Lot 7 Likewood Estates S/D Colunbia County, Florida

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Mach 12, 2007

DRAWN BY STRUCTURAL BY:
David Disosway

FINALS DA'E: 12 / Mar / 17

> JOB NUMBER: 703091 DRAWING NUMBER

> > **S-2** (F 3 SHEETS

