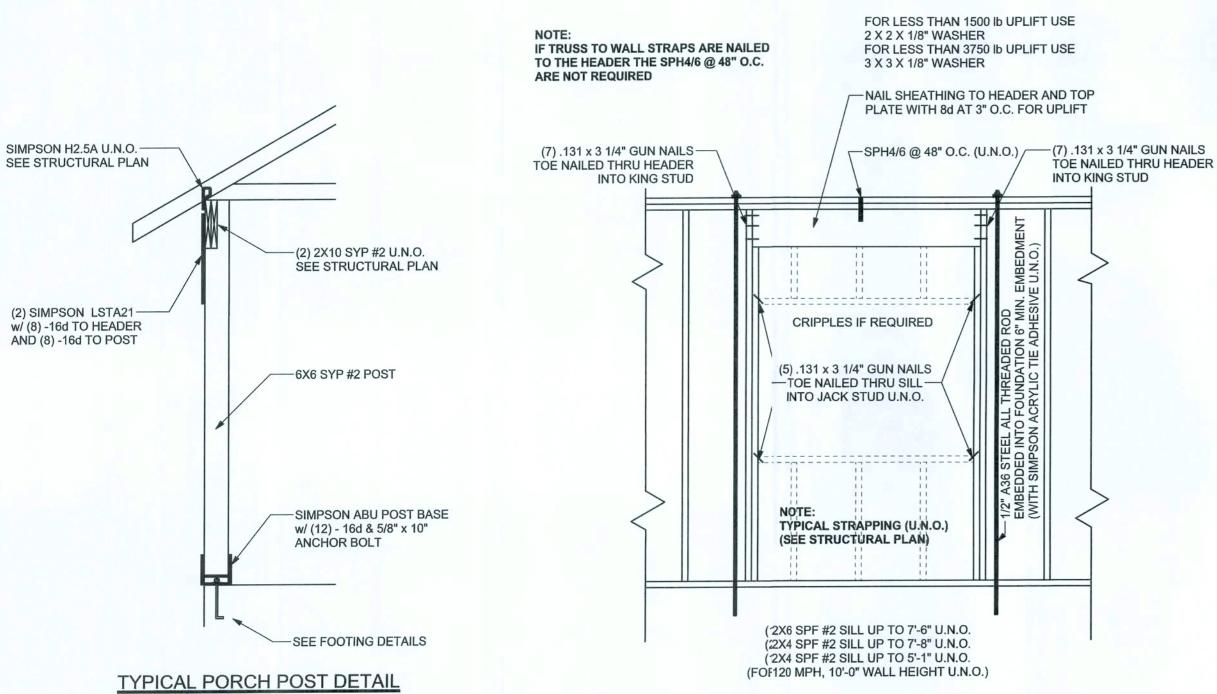


ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



TYPICAL HEADER STRPING DETAIL (FOR OPENINGS OVER 4'-0")

NAILSB TO UPPER TOP PLATE

OR NAIL OSB TO LOWER TOP PLATE &

-SPLICE MAY BE OMITTED IF FULL

1/2" MINIMUM EDGE

DISTANCE FOR ALL

HORIZONTAL EDGES

- 2X4/6 PT PLATE SYP #2 w/ 1/2"x10" -

ANCHOR BOLTS w/ 3x3x.25" STEEL WASHER

8" FROM CORNERS EACH WAY & 32" O.C.

W6 - SHEATHING NAILING FOR TRUSS UPLIFT

-2X4 OUTRIGGER @ 24" O.C.

7/16" OSB ROOF SHEATHING 8d 6" O.C

EDGE, 12" O.C. FIELD, & 4" O.C. GABLES

- BLOCKING REQUIRED BETWEEN OUT RIGGERS

HEIGHT SHEATHING COVERS

UPPER TOP PLATE & BOTTOM

STRAP TRUSS TO LOWER TOP PLATE

LTS16 ----

12-10d X 1 1/2"

(6) NAILS IN-

LOWER TOP

PLATE 1" MIN.

OUNDATION

INSTALL 2X4 SPF #2 DIAGONAL BRACE -AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND RAT RUN @ 6' O.C.

-(4) 12dS

- 2X4 SPF #2 BLOCKING

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.

- H3 INSTALLED HORIZONTALLY

FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

(8) 12dS

GABLE BRACING DETAIL SCALE: 1/2" = 1'-0"

PLATE

(IF PSSIBLE)

H2.5A 13d -

8d, @ 31.C.-

8d, @ 31.C.-

2X4 SP#2-

8d, @ 31.C.-

1" MAX (P @

HORIZO AL

SPLICE

7/16" OI -

8d, @ 31.C.-

SEE

DET/S

SCALE/2"=1'-0"

ATTACH RAZUN TO:

BLOCKING v4) 12dS

w/ (8) -16d T(RUSS

@ 48" O.C. U.O.

TO TOP PLA 12d @ 6" O.C

BLOCKI3

FOR GIRDER TRUSS

OVER 600 LB UPLIFT

MTS16 ----

└─8d, @ 3" O.C. OFFSET-

14-10d X 1 1/2"

STUD &

TO STUD

7/16" OSB-MSTA24-

FOUNDATIO)

DIAGONAL BRACE MUST BE NAILED O TRUSS WEBS FOR LENGTH

OVER 12' IT MAY BE "T" BRACED UP

12' AND UNBRACED UP TO 7'

-2X4X8' RAT RUN NAIL EACH

OR .135" X 3.125"

OR .131 X 3.25"

-8d, @ 3" O.C. DETAILS

NAIL STRAP

NOTE:

NOTE:

ADD 2ND STUD

FOR > 2500 LB

1/2" MINIMUM EDGE

HORIZONTAL EDGES

DISTANCE FOR AL

REACTION

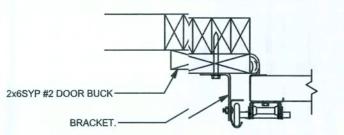
USE (2) MST16

└─8d, @ 3" O.C.

FOR > 860 LB

2x6 SYP #2 GARAGGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK ATTACH ATTACH GARAGE DOO!OR BUCK TO STUD PACK AT EACH SIDE OF DOOR O OPENING WITH 3/8"x4" LAG SCREWS W/ 1" WASHEFER LAG SCREWS MAY BE COUNTERSUNK. HORIZ!ZONTAL JAMBS DO NOT TRANSFER LOAD. CEN'NTER LAG SCREWS OR STAGGER 16d NAILS O!OR (2) ROWS OF .131 x 3 1/4" GN PER TABLE BELOW.W:

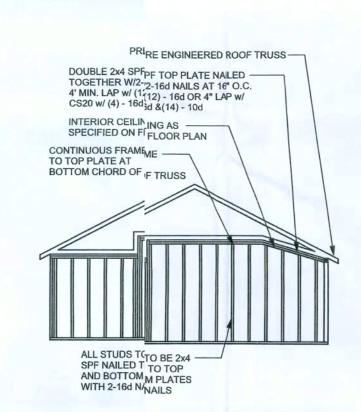
DOOR WIDTH	3/8" > x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24 ¹ 24" O.C.	5* O.C.	5" O.C.
11' - 15'	¹⁸ 18" O.C.	4" O.C.	4" O.C.
16' - 18'	16' _{6"} O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL

GRADDE & SPECIES TABLE

			Fb (psi)	E (10 ⁶ psi)
2x8		SYP #2	1200	1.6
2x10		SYP #2	1050	1.6
2x12		SYP#2	975	1.6
GLB		24F-V3 SP	2400	1.8
LSL	T	TIMBERSTRAND	1700	1.7
LVL		MICROLAM	1600	1.9
PSL	100	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING; DIAPHRAGM DETAIL SCALE: N.T.S.

-SPH4/6 ALL OOPENINGS (U.N.O.)-

IF TRUSS TO WALL STRAPS ARE NAILILED

TO THE HEADER THE SPH4/6 @ 48" O.D.C.

ARE NOT REQUIRED

(6) .131 x 3 1/4" GUN NAILS-

INTO KING STUD

TOE NAILED THRU HEADER

MASONRY NOTES:

	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approval			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metaties not completely embedded in mortar o grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.			

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER CUNCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING STREET PIDER REINFORCEME FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	JNDATION BEARING CAPACITY, GRADE AND AND DEBRIS ZONE, AND FLOOD ZONE.
	TRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 D WIND VELOCITY AND DESIGN PRESSURES.
	PATH FROM TRUSSES TO FOUNDATION. IF YOU ITINUOUS LOAD PATH CONNECTION, CALL EDIATELY.
DESIGN, PLACEMENT PLANS, TEM	RER'S SEALED ENGINEERING INCLUDES TRUSS MPORARY AND PERMANENT BRACING DETAILS, S, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

DESIGN DATA

< 420	UPLIFT LBS. SPF < 245		TO PLATES	TO RAFTER/TRUSS	TO STUD
		H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	4
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*	T		TO FOUNDATE
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADEI 12" EMBEDME
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED 12" EMBEDME
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED 12" EMBEDME
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2
< 1240	< 1065	SPH4			10-10d, 1 1/2
< 885	< 760	SP6			6-10d, 1 1/2
< 1240	< 1065	SPH6			10-10d, 1 1/2
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		JIO AD
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/0" AD
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88			1/2" AB
LULU	- 2020	ADUOO	18 - 16d		2-5/8" AB

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

1 | 19.9 | -21.8 | 18.1 | -18.1

2 19.9 -25.5 18.1 -21.8

2 O'hg -40.6 -40.6

3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

Doors & Windows 21.8 -29.1

8x7 Garage Door 19.5 -22.5

16x7 Garage Door 18.5 -21.0

Worst Case

(Zone 5, 10 ft2)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

BASIC WIND SPEED = 110 MPH

5.) ROOF ANGLE = 10-45 DEGREES

MEAN ROOF HEIGHT = <30 FT

3.) WIND IMPORTANCE FACTOR = 1.0

2.) WIND EXPOSURE = B

4.) BUILDING CATEGORY = II

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS:

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

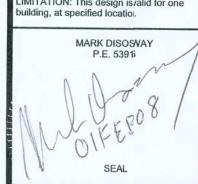
REVISIONS

PE No.53915, POB 868, Lae City, FL 32056, 386-754-5419 Stated dimensions supercee scaled imensions. Refer all questions to Mark Disosway, P.E. for resolution.

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CERTIFICATION: I hereby ertify that I have examined this plan, and thathe applicable ortions of the plan, relatingto wind engine comply with section R301.21, florida building ode residential 2004, to thebest of my

LIMITATION: This design is/alid for one building, at specified location.



26644

Brad Barney Garage

ADDRESS: Lot 14 Blk. B Oakhaven S/D Columbia coutny, Florida

Mark Disosway P.E. P.O. Box868 Lake City, Florda 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> February 01,2008 CHECKED BY:

FINALS DATE: 01 / Feb / 08

> JOB NUMBER: 711202 DRAWING NUMBER

> > **S-1** OF 3 SHEETS

INTO KING STUD

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

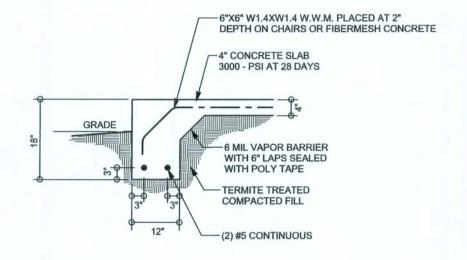
		SPH4/q/6 @ 48" O.C. (U.N.O.) CRIPPLES:S IF REQUIRED		STRUCTUI MUST IMM ANY CONF	TO ALL REQUIREMENTS OF "SPI RES" (ACI 530.1/ASCE 6/TMS 602). EDIATELY, BEFORE PROCEDING, FLICTS BETWEEN ACI 530.1-02 AN PTIONS TO ACI 530.1-02 MUST BE G.	THE CONTRACTOR AND MASO NOTIFY THE ENGINEER OF ID THESE DESIGN DRAWINGS.
					ACI530.1-02 Section	Specific Requirements
		(4) .131 x 3 1/1/4" GUN NAILS /		1.4A	Compressive strength	8" block bearing walls F'm = 150
		TOE NAILEIED THRU SILL —		2.1	Mortar	ASTM C 270, Type N, UNO
		INTO JACK K STUD U.N.O.		2.2	Grout	ASTM C 476, admixtures require
				2.3	CMU standard	ASTM C 90-02, Normal weight, I medium surface finish, 8"x8"x16 bond and 12"x12" or 16"x16" co block
				2.3	Clay brick standard	ASTM C 216-02, Grade SW, Typ 5.5"x2.75"x11.5"
		NOTE: TYPICAL STITRAPPING (U.N.O.)		2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ks splices min 48 bar dia. (30" for #
		(SEE STRUC CTURAL PLAN)		2.4F	Coating for corrosion protection	Anchors, sheet metal ties comple embedded in mortar or grout, AS A525, Class G60, 0.60 oz/ft2 or
		SPH4/6 ALL Olopenings (U.N.O.)		2.4F	Coating for corrosion protection	Joint reinforcement in walls expormoisture or wire ties, anchors, sl ties not completely embedded in grout, ASTM A153, Class B2, 1.3 or 304SS
		(1) 2X6 SPF #2 SIL _{ILL} UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SII _{SILL} UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-y-0" WALL HIGHT U.N.O.)		3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project dra require engineering approval.
TYPIC SCALE:	CAL HEADER ST	RAPING DETAAIL (FOR OPENINGS	6 4'-0" OR LESS)	3.3.E.7	Movement joints	Contractor assumes responsibiliand location of movement joints detailed on project drawings.

(6) .131 x 3 1/4" GUN NAILS

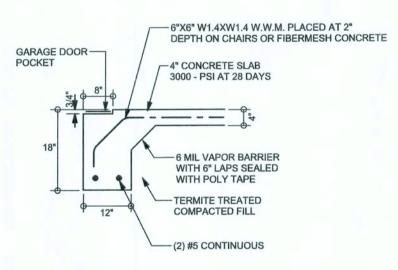
TOE NAILED THRU HEADER

REVISIONS

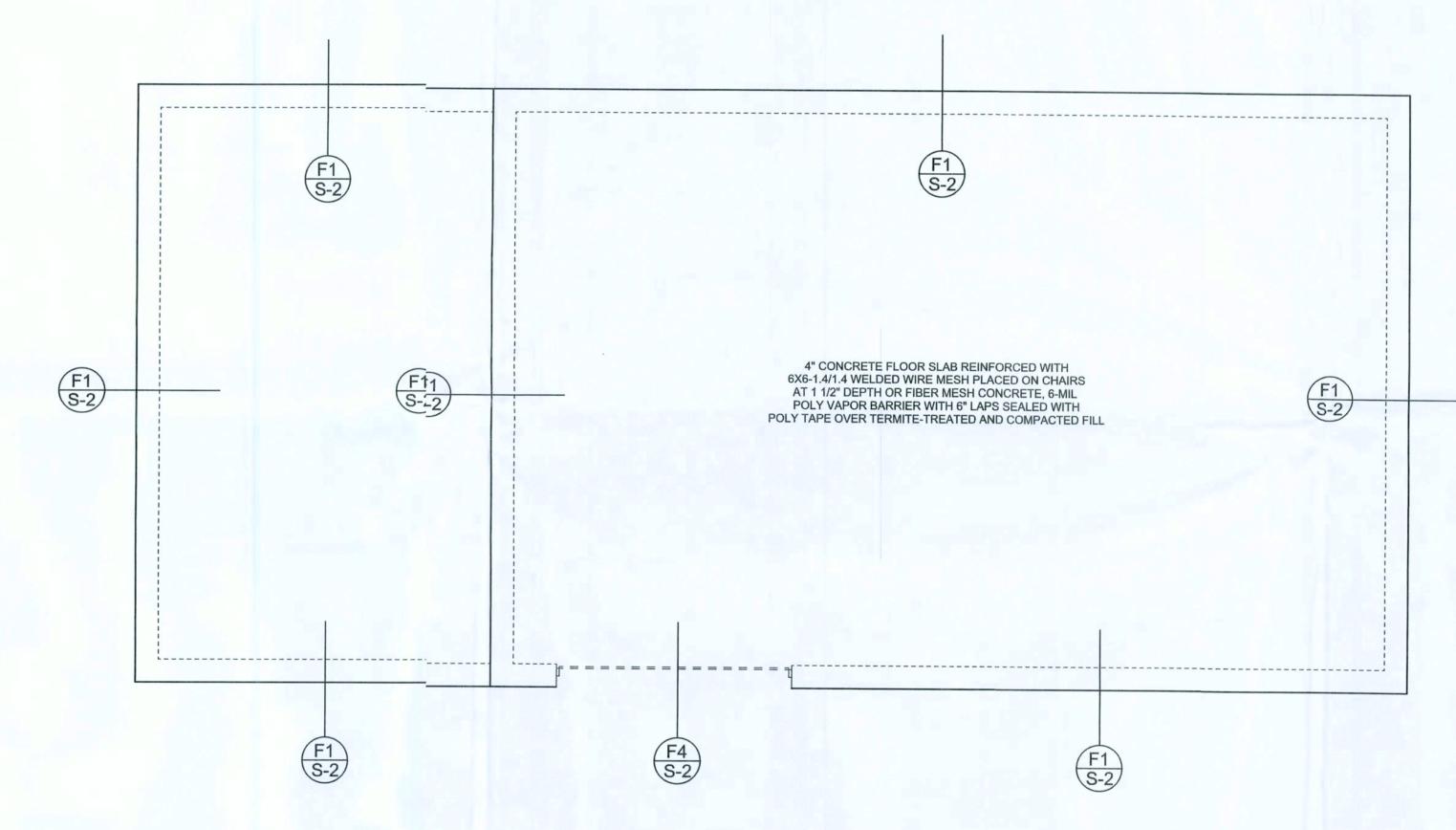
SOFTPIXN ARCHITECTURAL DESIGNOFTWARE



F1 MONOLITHIC FOOTING
S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS

ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Max Disosway, PE No.53915, POB 868, LakeCity, FL 32056, 386-754-5419

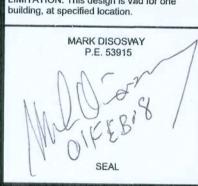
DIMENSIONS: Stated dimensions supercedescaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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not to be reproduced, altered in copied in any form or manner without first the express written permission and consent of Mak Disosway.

CERTIFICATION: I hereby cetify that I have examined this plan, and that the applicable portions of the plan, relating twind engineering comply with section R301.2.1 florida building code residential 2004, to the lest of my knowledge.

LIMITATION: This design is vilid for one building, at specified location.



Brad Barney Garage

ADDRESS
Lot 14 Blk. B Oakhiven S/D
Columbia coutny,Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269- 4871

PRINTED DATE:
February 01, 2108

DRAWN BY: CIECKED BY:

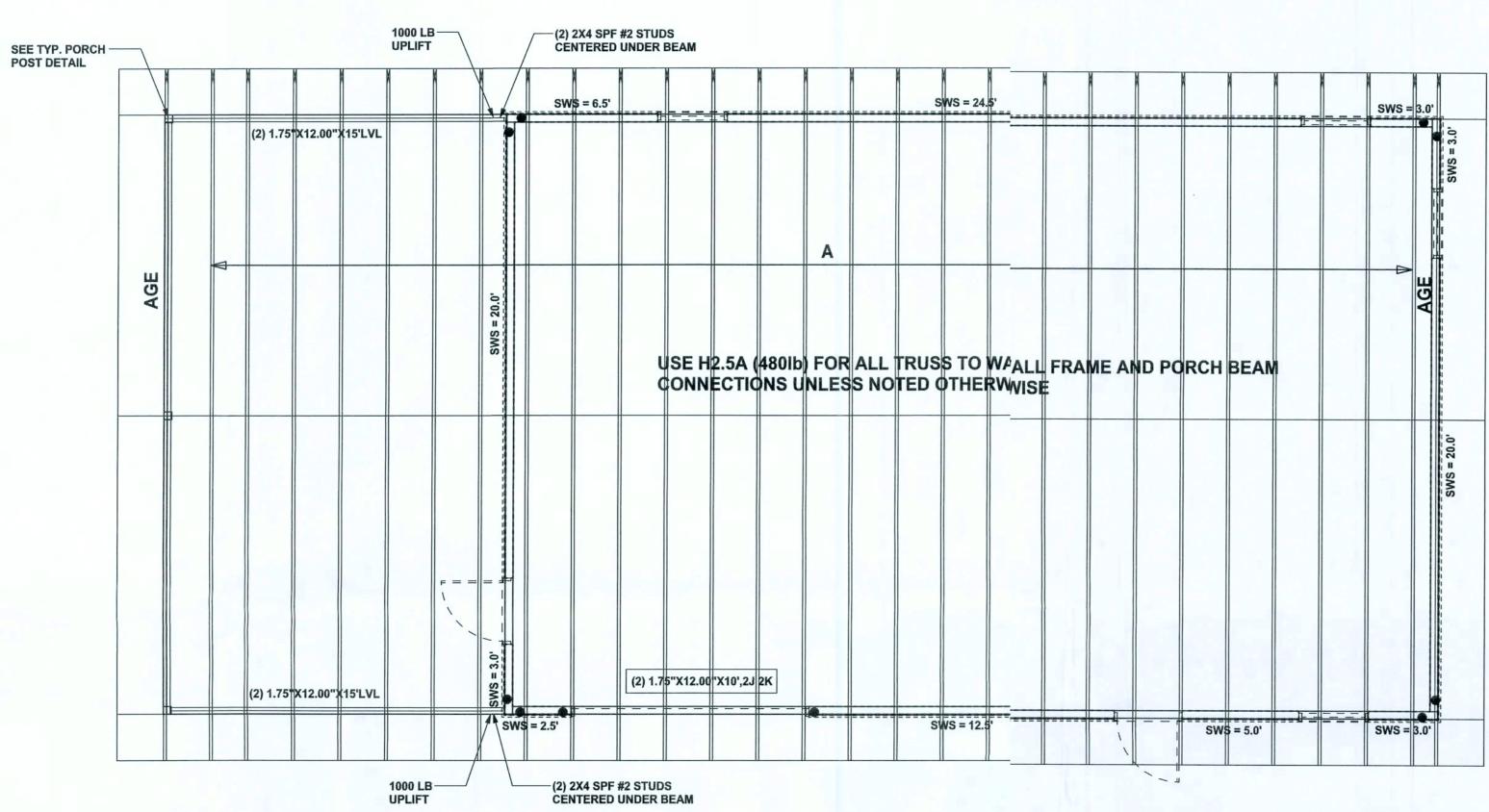
FINALS DATE: 01 / Feb / 08

JOB NUMEER: 711202 DRAWING NUMBER

S-2OF 3 SHEETS

REVISIONS

SOFTPIAN DESIGN SOFTMAN



STRUCTURALPLAN
SCALE: 1/4" = 1'-0"

STRUCTURALPLAN NOTES

SN-1 ALL LOADEARING FRAME WALL & PORCH HEADERS SHALL BE MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOADEARING FRAME WALL HEADERS SHALL HA! (1) JACK STUD & (1) KING STUD EACH SIDIU.N.O.)

SN-3
DIMENSIOS ON STRUCTURAL SHEETS
ARE NOT FACT. REFER TO ARCHITECTURAL
FLOOR PLI FOR ACTUAL DIMENSIONS

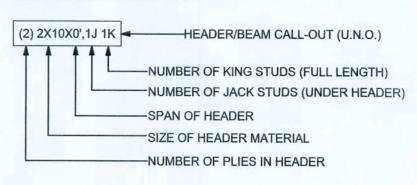
PERMANEI TRUSS BRACING IS TO BE INSTALLED AT LOCATIONAS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL LACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, LSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PAKAGE

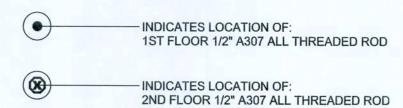
WALL LEGEND

SWS = 0.0' !====================================	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW = = = = 100000001	1ST FLOOR INTERIOR BEARING V, WALLS
IBW	2ND FLOOR INTERIOR BEARING \ WALLS

HEADER LEGEND



THREADED ROD LEGEND



TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	31.5'	46.0'
LONGITUDINAL	9.0	57.0

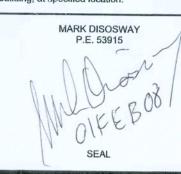
WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, IL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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permission and consent of Mark Dissway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the appcable portions of the plan, relating to wind ingineering comply with section R301.2.1, floridabuilding code residential 2004, to the best of ny knowledge.

LIMITATION: This design is valid forme building, at specified location.



Brad Barney Gange

ADDRESS: Lot 14 Blk. B Oakhaven 3/D Columbia coutny, Florda

Mark Disosway PE. P.O. Box 868 Lake City, Florida 3:056 Phone: (386) 754 - \$419 Fax: (386) 269 - 4871

PRINTED DATE:
February 01, 2008

DRAWN BY: CHECK:D BY:

FINALS DATE: 01 / Feb / 08

> JOB NUMBEF: 711202 DRAWING NUMBER

> > **S-3**OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS COMPANY JOB #7-333