

DESIGN SPECIFICATIONS

DESIGN CODE:
2017 FLORIDA BUILDING CODE – RESIDENTIAL
DESIGN IS VOID ONE YEAR AFTER THE DATE OF THE ORIGINAL PLANS,
UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.

DESIGN LOADS: ACTUAL AND UNIFORM
ROOF LOADING (cd=1.25) FLOOR (cd=1.00)
TOP CHORD DEAD LOAD 20 psf 40 psf
TOP CHORD DEAD LOAD 7 psf (ARCH SHINGLES) 10 psf
TOP CHORD DEAD LOAD 20 psf (TILE SHINGLES) 10 psf
BOTTOM CHORD DEAD LOAD 10 psf 0 psf
BOTTOM CHORD DEAD LOAD 5 psf 0 psf

DEFLECTION CRITERIA:
ROOF FRAMING: LIVE LOAD L/240 TOTAL LOAD L/180
FLOOR FRAMING: LIVE LOAD L/360 & TOTAL LOAD L/240
0.75" MAX ANY CASE

WIND LOADING:
ASCE 7/10 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A
MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD AND 5 PSF
BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS
OF ROOF TRUSSES SHALL BE REDUCED SPECIFICALLY ATTIC FLOOR
LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED
BY 0.75 WHEN COMBINED W/ DEAD LOADS.

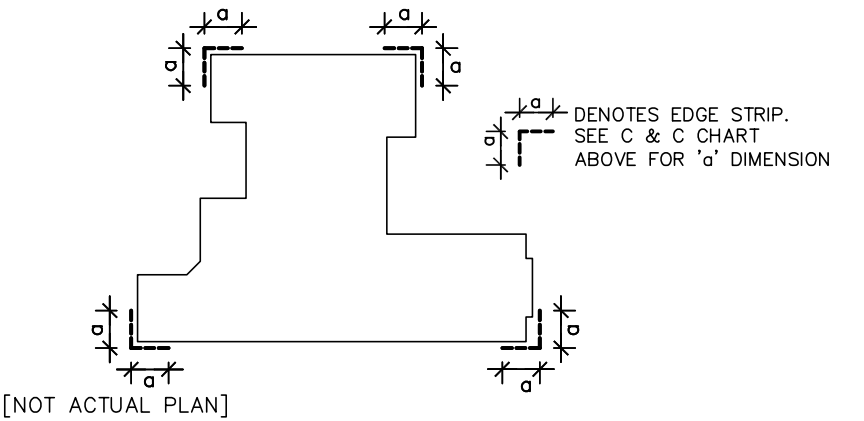
BASIC WIND SPEED (ASCE 7-10) 130 MPH
IMPORTANCE FACTOR 1.00
MEAN ROOF HEIGHT 20.0 FT
ROOF PITCH 6/12
BUILDING CATEGORY C
EXPOSURE CATEGORY C
ENCLOSURE CLASSIFICATION ENCLOSED
INTERNAL PRESSURE COEFFICIENT ± .18

MATERIAL SPECIFICATIONS

HARDWARE AND ANCHORS:
ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH
ASTM A 307 OR ASTM F 1554 GRADE 36
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE 5
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO
EXTERIOR SHALL BE GALVANIZED.
REBAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR
DOVELLS SHALL BE 12 BAR DIAMETER MINIMUM. HOLES SHALL BE 1/4"
LARGER THAN REBAR SIX AND 1/8" LARGER THAN THREADED ROD SIZE.
(U.O.N.)
ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS
(QUAL CARTRIDGE INSTALLATION ONLY):
EPOXY: ITW RED HEAD AT
REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60.
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.
WELDED WIRE FABRIC (WWF): SHALL BE ASTM A185.
LAMINATED VENEER LUMBER (LVL): ALL LAMINATED VENEER LUMBER
SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC
MODULUS (E) 1,900ksi, BENDING STRESS (Fb) 2600psi

TRIBUTARY AREA (sf)	COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES		GARAGE DOOR PRESSURES (PSF)
	INTERIOR ZONE (PSF)	EDGE STRIP (PSF): "a" = 4'-6"	
10	+25.6 –27.7	+25.6 –34.2	+22.9 (8"x7")
50	+22.9 –25.0	+22.9 –28.8	+21.8 (16"x7")
100	+21.8 –23.9	+21.8 –26.6	+21.8 (16"x7")

- THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE
ABOVE WIND PRESSURES HAVE BEEN REDUCED BY 0.60 AS PERMITTED BY
THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION
SHALL BE PERMITTED
- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH
POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE
RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET
THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



SCOPE OF SERVICE

MEANS AND METHODS:
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE
FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR
SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY
OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF
THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT
DOCUMENTS.
LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE
LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT,
WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND
HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED
PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT
SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY
ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

FLOOR SHEATHING SPECIFICATIONS:
23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD
ROOF SHEATHING SPECIFICATIONS:
SHINGLE- MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED W/ 0.113x2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
TILE - MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.113x2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
METAL - MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.113x2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).
WALL SHEATHING SPECIFICATIONS:
FLEXIBLE FINISH-MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS
INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.
STUCCO FINISH-MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE
3/4" STRUCTURAL 1 GRADE SHEATHING OR 1/2" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:
MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF
2000 psi PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/8" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER.
CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL
REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STENWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE
USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH
- #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR
DIAMETERS INTO EACH ELEMENT. AT STEINWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCING AT 16" O.C. VERTICALLY. (EVERY OTHER COURSE), AND VERTICAL REINF. SHALL BE INCREASED AS NOTED ON
1/51.0, UNLESS NOTED OTHERWISE. LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS
CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:
FOOTING AND FOUNDATIONS:
FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE
SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL
SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUTS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE
FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT
DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS.
PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48
BAR DIAMETERS

CONCRETE SLABS ON GRADE:
SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED WITH #4 AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL TREATMENT FOR PREVENTION OF SUBTERRANEAN
TERMITES. SAWCUTS: FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12'x12' GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

WOOD FRAMING SPECIFICATIONS:
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR
SOIL SHALL BE PRESURE-TREATED. IF, ACC OR NON-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF AZZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE
STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BRACING IS INSTALLED AT ALL TRUSSES AS
INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER COMMENTARY. AND
RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSSES. WHEN USING (2) STRAPS ON
SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

ROOF COVERING SPECIFICATIONS:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY
AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL." AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED
ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE

MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONT. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

NAIL SPECIFICATIONS
3"x0.131" = GUN NAILS
2"x0.113" = 8d
2"x0.113" = 6d
3"x0.148" = 10d
1 1/2"x0.148" = 10d x 1 1/2"
2"x0.113" = RINK SHANK
2"x0.131" = 8d
3 1/2"x0.162" = 16d
1 1/2"x0.131" = 8d x 1 1/2"

BRICK NOTES / LINTEL SCHD

LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
13 1/2"x3 1/2"x 1/4"	4"	6'-0"
14x3 1/2"x 1/4"	6"	8'-0"
15x3 1/2"x 1/4"	6"	10'-0"
16x3 1/2"x 1/4"	6"	12'-0"
17x3 1/2"x 1/4"	6"	16'-0"

1. STEEL LINTELS TO BE MINIMAL 36"
LINTEL MUST HAVE CORROSION
RESISTANT COATING OF EPOXY BASED
PAINT.

2. LINTEL MORE THAN 8'-0". SHOULD
BE LATERALLY SUPPORTED NOT TO
EXCEED 6 FT. O.C. W/ 2-1/2"x3" WD.
SCREWS INTO HEADER PROVIDE A 1/2"
VERTICAL SLOTTED HOLE FOR SCREW.

3. BRICK VENEER ATTACHMENT:
HORIZONTAL TIES @ 24" O.C., VERT.
TIES @ 12" O.C. (FOR 110mph
WIND-ZONE VERT. TIES @ 16" O.C.).
AT ALL OPENINGS SPACE TIES WITHIN
12" OF OPENINGS. PROVIDE 3/8" WEEP
HOLES @ 33" O.C. IMMEDIATELY ABOVE
FLASHING.

BRICK VENEER
WEATHER
BARRIER
LINTEL
ATTACHMENT
FLASHING
BRICK LINTEL
SECTION VIEW
OF BRICK LINTEL

PLAN LEGEND AND ABBREVIATIONS

INTERIOR LOAD BEARING WALL	BUILT-UP POST IN THE WALL
GABLE X-BRACE, SEE DETAIL 10/SO.1	HEADER SIZE, JACK AND KING STUD QUANTITY.
DESIGNATES SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL THE SHEARWALL SHEATHING TO BE APPLIED 8d @ 3/8" DESIGNATES 8d COMMONS @ 3" O.C. EDGE & 6" O.C. IN THE FIELD.	
ADJ. = ADJACENT BM = BEAM BOT = BOTTOM BRG = BEARING CMU = CONCRETE MASONRY UNIT DBL = DOUBLE DIA = DIAMETER EA = EACH EQ = EACH END ENG = ENGINEER OF RECORD EQ = EQUAL EXT = EXTERIOR FBC = FLORIDA BUILDING CODE FND = FOUNDATION FT = FOOT WIND-ZONE VERT. TIES @ 16" O.C. HORIZ = HORIZONTAL LBS = POUNDS	LC = Long MANUF. = Manufacture MONO = Monolithic OC = On Center OSB = Oriented Strand Board PERP = Perpendicular PRE ENG = Pre Engineered PSF = Pounds per Square Foot PSI = Pounds per Square Inch QT = Quick Tie REINF = Reinforce SF = Square Foot SPF = Spruce Pine Fir SWP = Southern Yellow Pine THRU = Through TYP = Typical UN = Unless Otherwise Noted VERT = Vertical WFT = Welded Wire Fabric

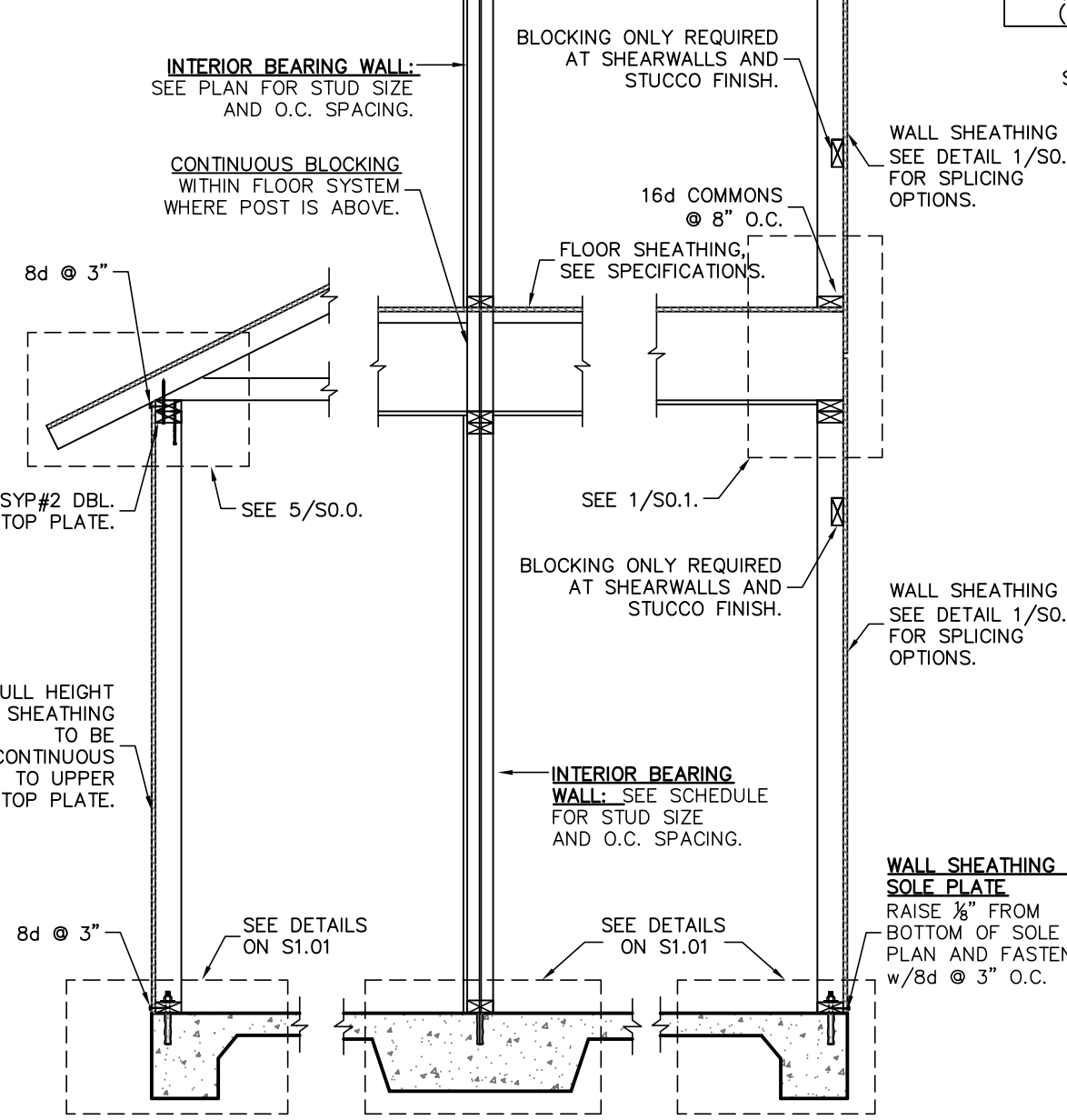
USP CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d1 1/2"	
USP RT7	585	495	(5)8d EA. END	
USP RT8A	775	650	(5)10d1 1/2" EA. END	
USP MTW12	1195	860	(7)10d1 1/2" EA. END	
USP HTW20	1450	1245	(12)10d1 1/2" EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LST20B	1105	1105	1/2" ROD TO FTG.	
USP JUS28	1305	1305	(6)10d TO HEADER	
USP HTT16	4290	4290	3/4" ROD TO FTG.	
USP HTT22	5370	5370	3/4" ROD TO FTG.	
USP PAU44	2535		3/4" ROD W/ (12)16d	
USP PAU66	2535		3/4" ROD W/ (12)16d	
USP MSTM24	1545	1455	(5)1/4"x2-1/4" TAPCONS	

SIMPSON CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
A35	450	450	12-8d1 1/2"	10446.4
H25T	600	520	5-8d EA. END	11478.3
HTS16	1150	1085	16-10d EA. END	10456.6
MTS12	1000	860	7-10d1 1/2" EA. END	10456.3
LTS20	1450	1245	24-10d1 1/2" EA. END	13872.3
MSTA24	1765	1270	9-10d EA. END	13872.4
MSTA36	2050	1870	13-10d EA. END	13872.8
HTT4	3480	3080	18-16d TO TRUSS/BEAM 1-3/4" ROD TO FTG.	11496.2
HTT5	5250	4670	32-16d TO TRUSS/BEAM 1-3/4" ROD TO FTG.	11496.2
LUS28	930	780	6-10d TO HEADER 4-10d TO JOIST	10655.113
HU410	905	785	14-16d TO HEADER 6-16d TO JOIST	10531.36
ABU44	2200		3/4" ROD EPOKED 6" MIN	10849.6
ABU66	2300		3/4" ROD EPOKED 6" MIN	10849.6
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LT20B	1675	1675	10-16d TO STUD/BEAM/POST	11496.3
LSTA12	805	695	10-10d	13872.5
CS16	1705	1705	13-8d	10852.1

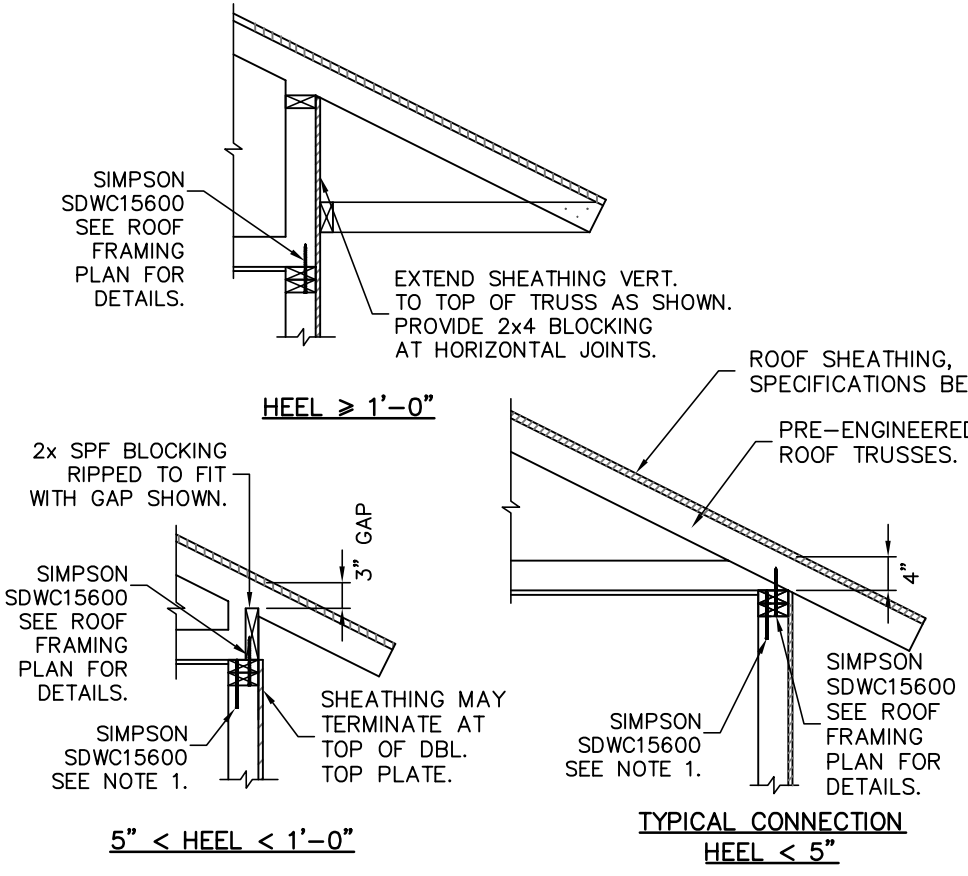
TYPICAL WALL FRAMING NOTES:
1. USE SYP#2 OR BETTER FOR ALL WALL STUDS..
2. USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.
3. USE SYP#2 FOR ALL HEADERS
4. ALL WALLS SHALL BE BALLOON FRAMED FULL HEIGHT TO ROOF OR FLOOR BEARING ELEVATION, U.O.N. ON PLAN.
5.) FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO CONCRETE SLAB TYPICALLY 3/4" FULL HEIGHT THREADED RODS @ 6'-0" O.C. MINIMUM. SEE FOUNDATION PLAN ADDITIONAL ANCHORS AT SHEARWALLS



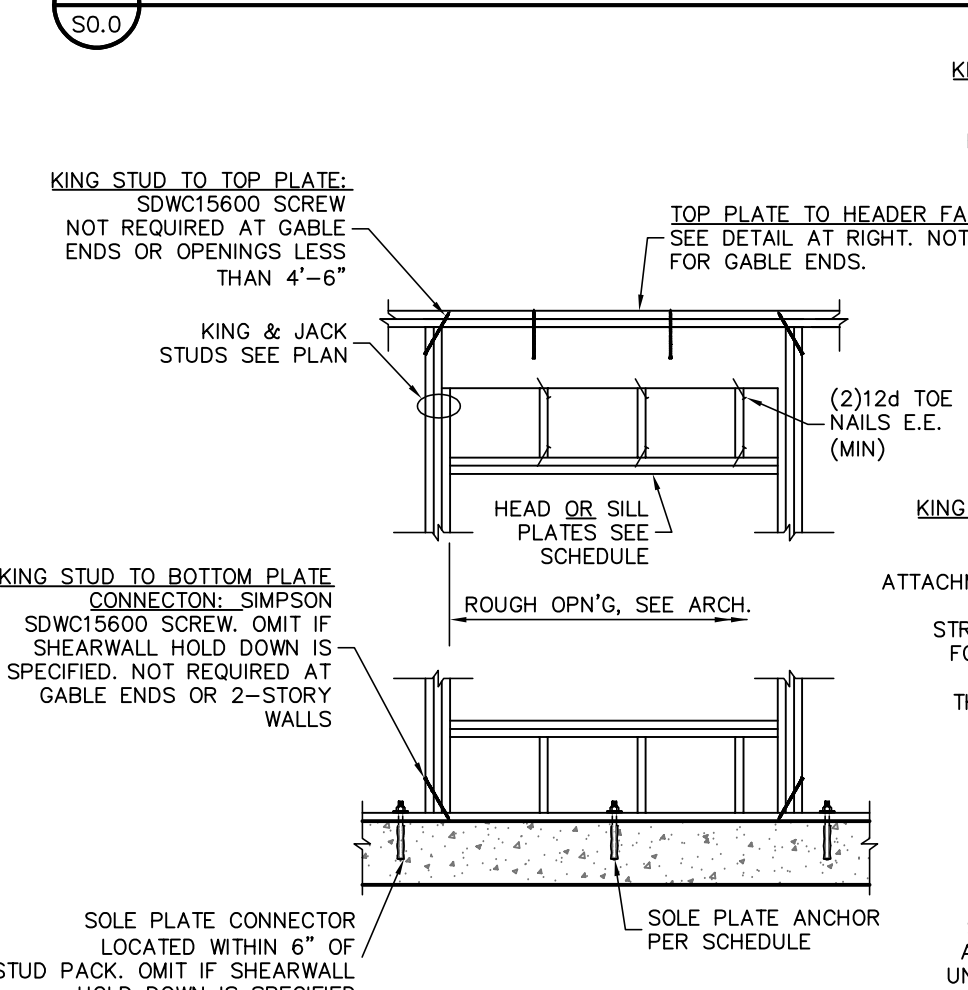
SINGLE STORY

MULTY STORY

1 TYP. WALL SECTIONS

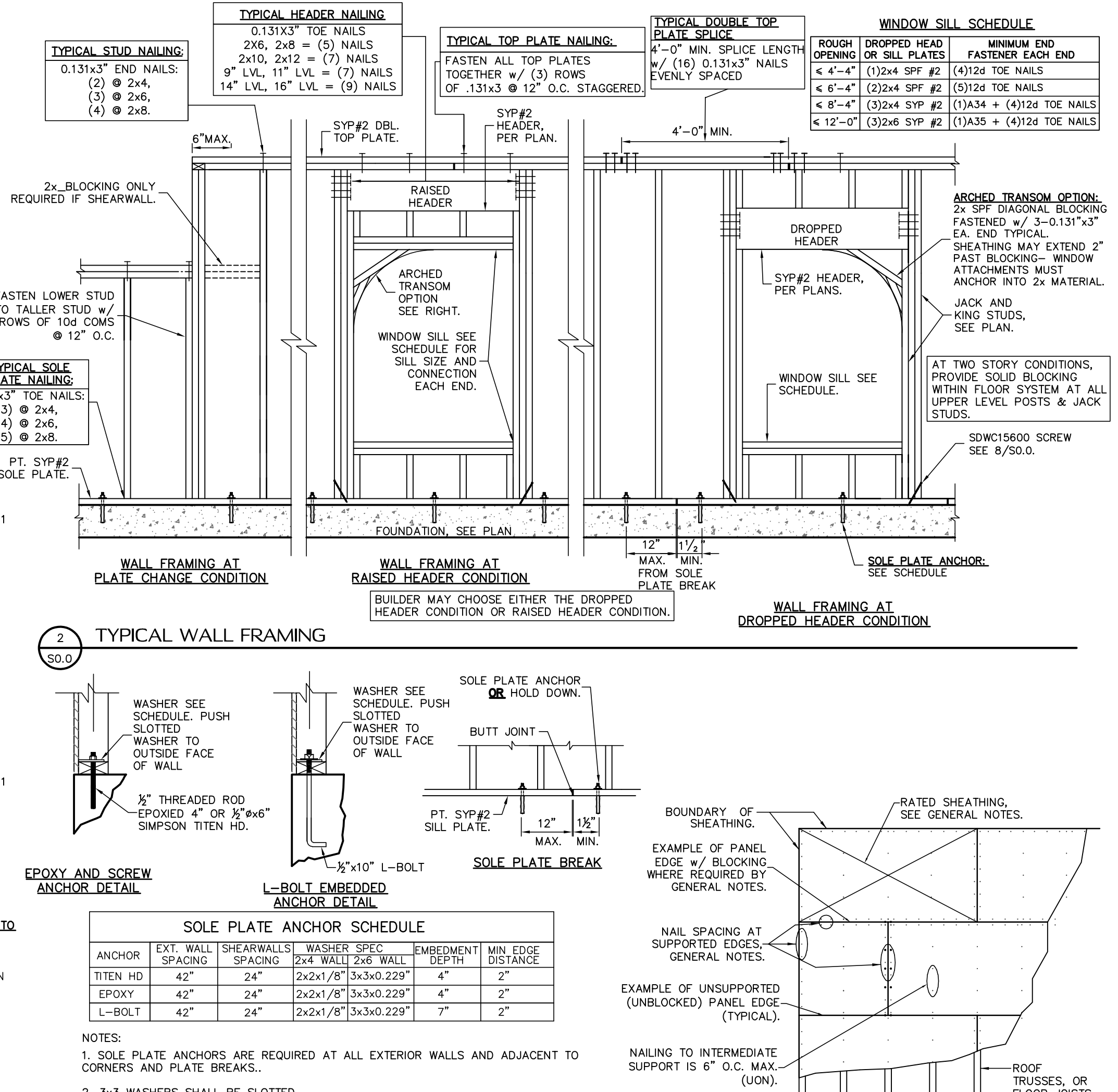


5 ROOF TRUSS CONNECTION



8 TYPICAL HEADER FASTENING

SOLE PLATE CONNECTOR LOCATED WITHIN 6" OF STUD PACK. OMIT IF SHEARWALL HOLD DOWN IS SPECIFIED



3 SOLE PLATE ANCHOR DETAIL & SCHEDULE

SCALE: 3/4" = 1'-0"

Diagram illustrating the connection of a 2x4 beam to a 3x4 beam using 12d nails. The connection is shown from the side, with the 2x4 beam on the left and the 3x4 beam on the right. The 2x4 beam is labeled "2 PLY BEAM." and the 3x4 beam is labeled "3 PLY BEAM." The connection is secured with "4) ROWS 12d @ 12" O.C. EA. SIDE." The nails are shown in cross-section, with the 2x4 beam having a width of "WS 12d 2" O.C. SIDE." and the 3x4 beam having a width of "WS 12d 2" O.C. SIDE." The connection is labeled "2 & 3 PLY BEAMS."

2 & 3 PLY BEAMS

Diagram illustrating the connection of a 2x4 or 2x6 post to a 3x4 beam using 12d nails. The connection is shown from the side, with the 2x4 or 2x6 post on the left and the 3x4 beam on the right. The post is labeled "2x4 OR 2x6 POST" and the beam is labeled "3x4 BEAM." The connection is secured with "2x4 MEMBERS: (1) ROW OF 10d NAILS @ 4" O.C. STAGGERED" and "2x6 MEMBERS: (2) ROW OF 10d NAILS @ 4" O.C. STAGGERED." The nails are shown in cross-section, with the post having a width of "WS 12d 2" O.C. SIDE." and the beam having a width of "WS 12d 2" O.C. SIDE." The connection is labeled "2x4 OR 2x6 POST."

2x4 OR 2x6 POST

Diagram illustrating the connection of a 3-ply beam to a 4-ply beam using 5 inch trusslock engineered wood fasteners. The connection is shown from the side, with the 3-ply beam on the left and the 4-ply beam on the right. The 3-ply beam is labeled "3-PLY" and the 4-ply beam is labeled "4-PLY." The connection is secured with "3) ROWS OF 5" TRUSSLOCK ENGINEERED WOOD FASTENERS @ 24" O.C. AS SHOWN." The fasteners are shown in cross-section, with the 3-ply beam having a width of "WS 12d 2" O.C. SIDE." and the 4-ply beam having a width of "WS 12d 2" O.C. SIDE." The connection is labeled "3-PLY."

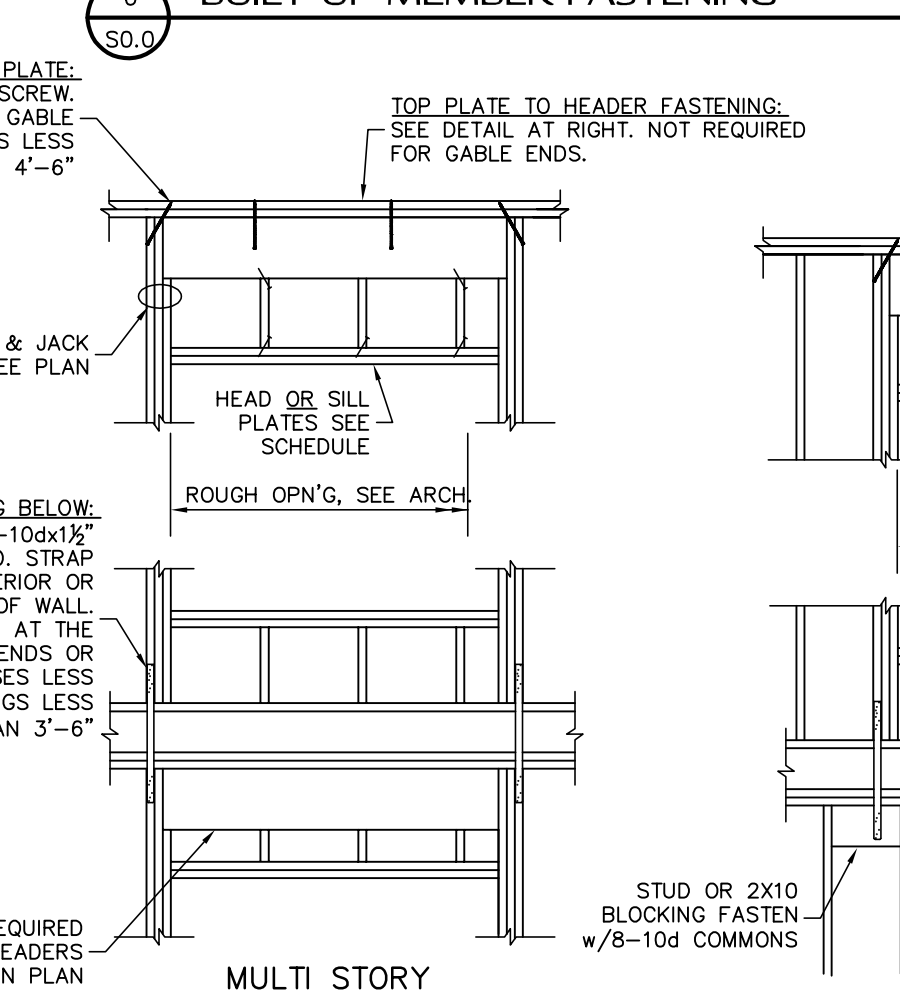
14" LVL AND GREATER

Diagram illustrating the connection of a 4-ply beam to a 3x4 beam using 6 3/4 inch trusslock engineered wood fasteners. The connection is shown from the side, with the 4-ply beam on the left and the 3x4 beam on the right. The 4-ply beam is labeled "4-PLY" and the 3x4 beam is labeled "3x4 BEAM." The connection is secured with "3) ROWS OF 6 3/4" TRUSSLOCK ENGINEERED WOOD FASTENERS @ 24" O.C. AS SHOWN." The fasteners are shown in cross-section, with the 4-ply beam having a width of "WS 12d 2" O.C. SIDE." and the 3x4 beam having a width of "WS 12d 2" O.C. SIDE." The connection is labeled "4-PLY."

NOTES:

1. TYP. CONNECTION AT STUD COLUMNS,

6 BUILT-UP MEMBER FASTENING

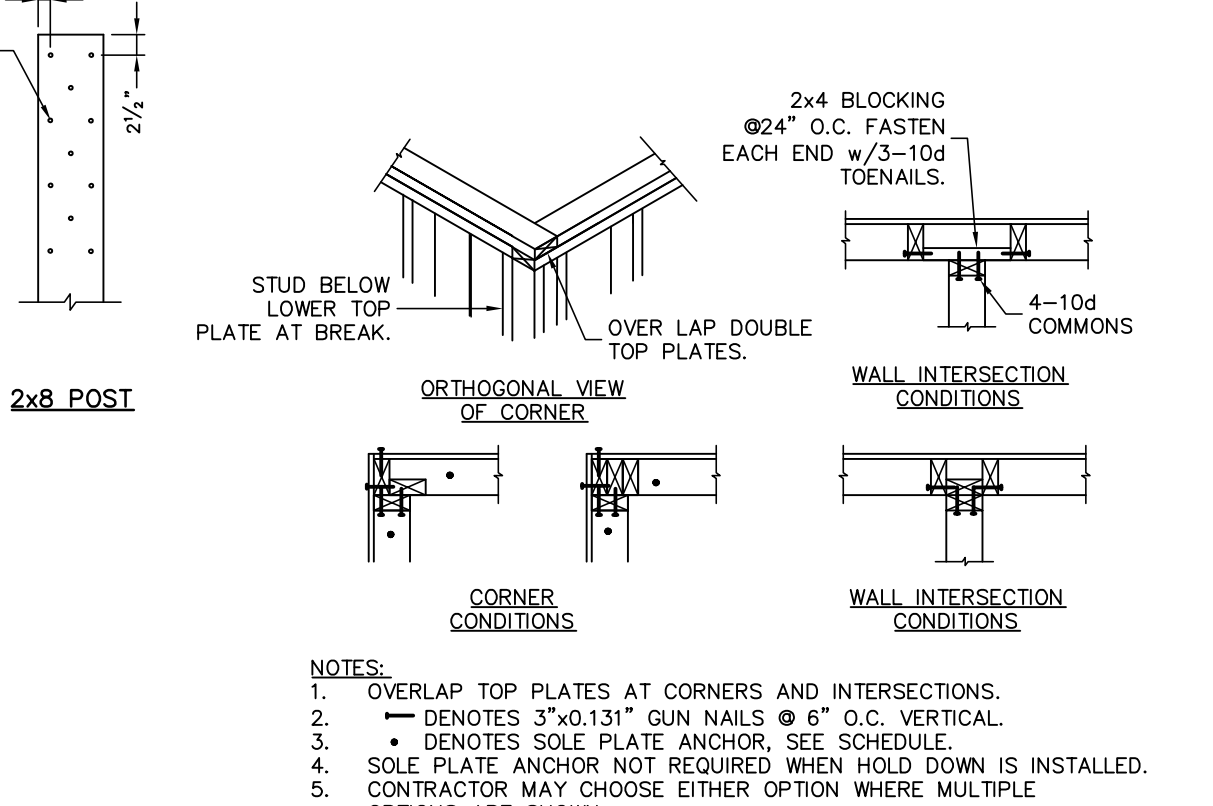


7 FRAMED WALL CORNER AND INTERSECTIONS STUDS CONFIGURATIONS

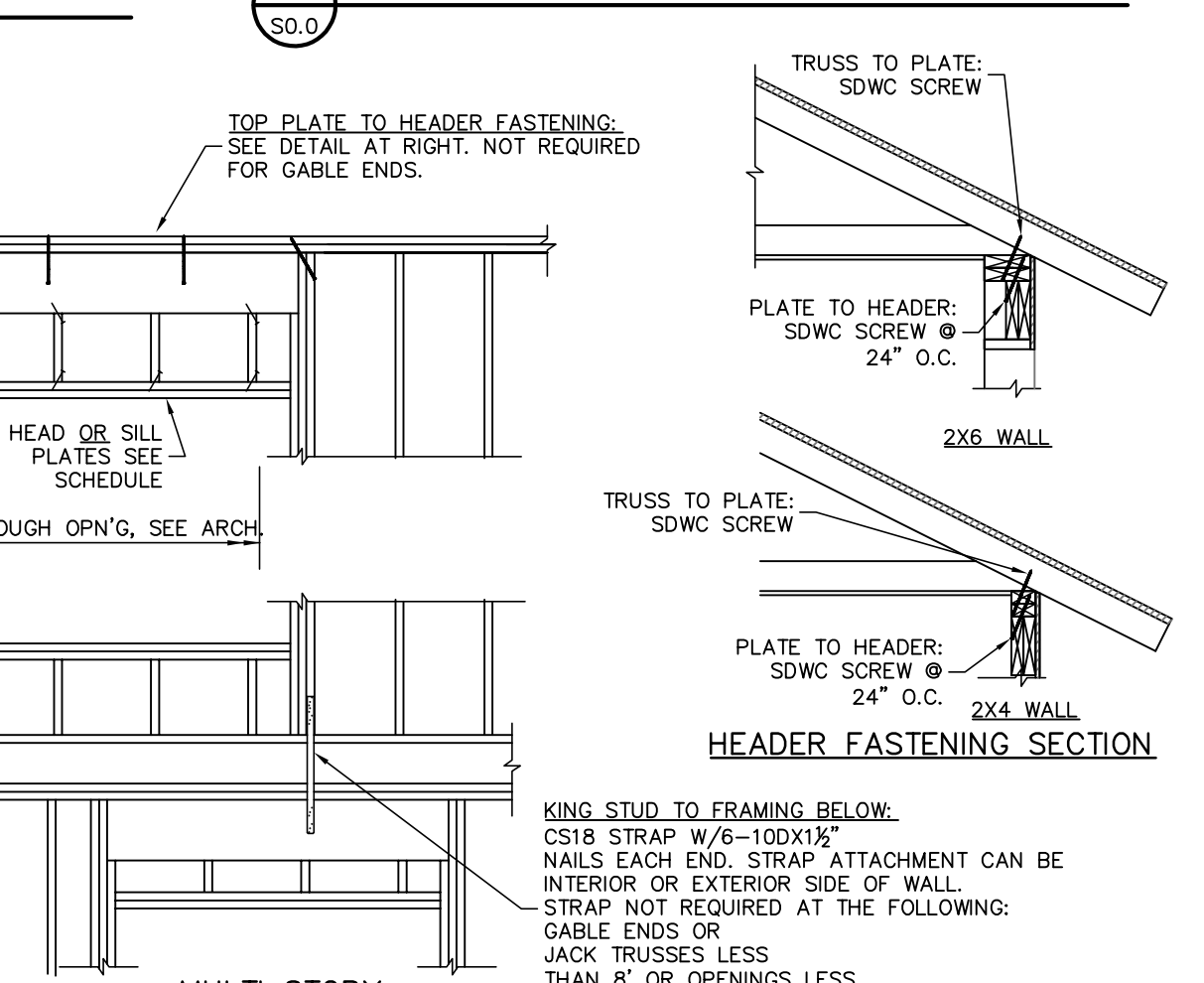
SCALE: 3/4" = 1'-0"

4 ROOF AND FLOOR SHEATHING NAILING

SCALE: 3/4" = 1'-0"



7 FRAMED WALL CORNER AND INTERSECTIONS STUDS CONFIGURATIONS



8 TYPICAL HEADER FASTENING

SOLE PLATE CONNECTOR LOCATED WITHIN 6" OF STUD PACK. OMIT IF SHEARWALL HOLD DOWN IS SPECIFIED



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FIELD ALTERATION

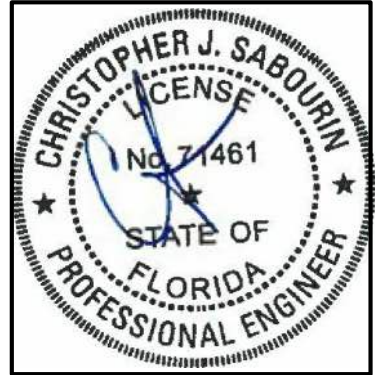
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SCALING

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DESIGN CRITERIA AND GENERAL NOTES

SHEET
S0.0
SHEET 1 OF 7



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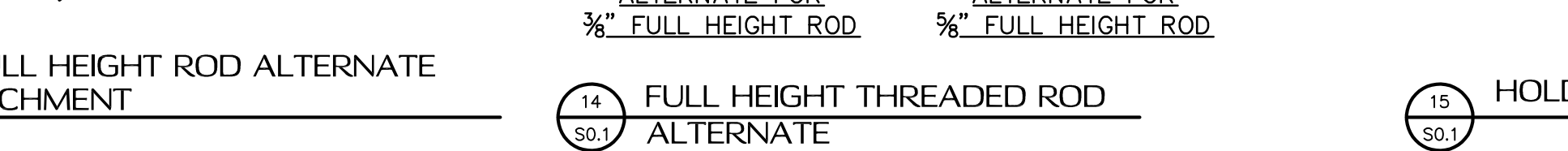
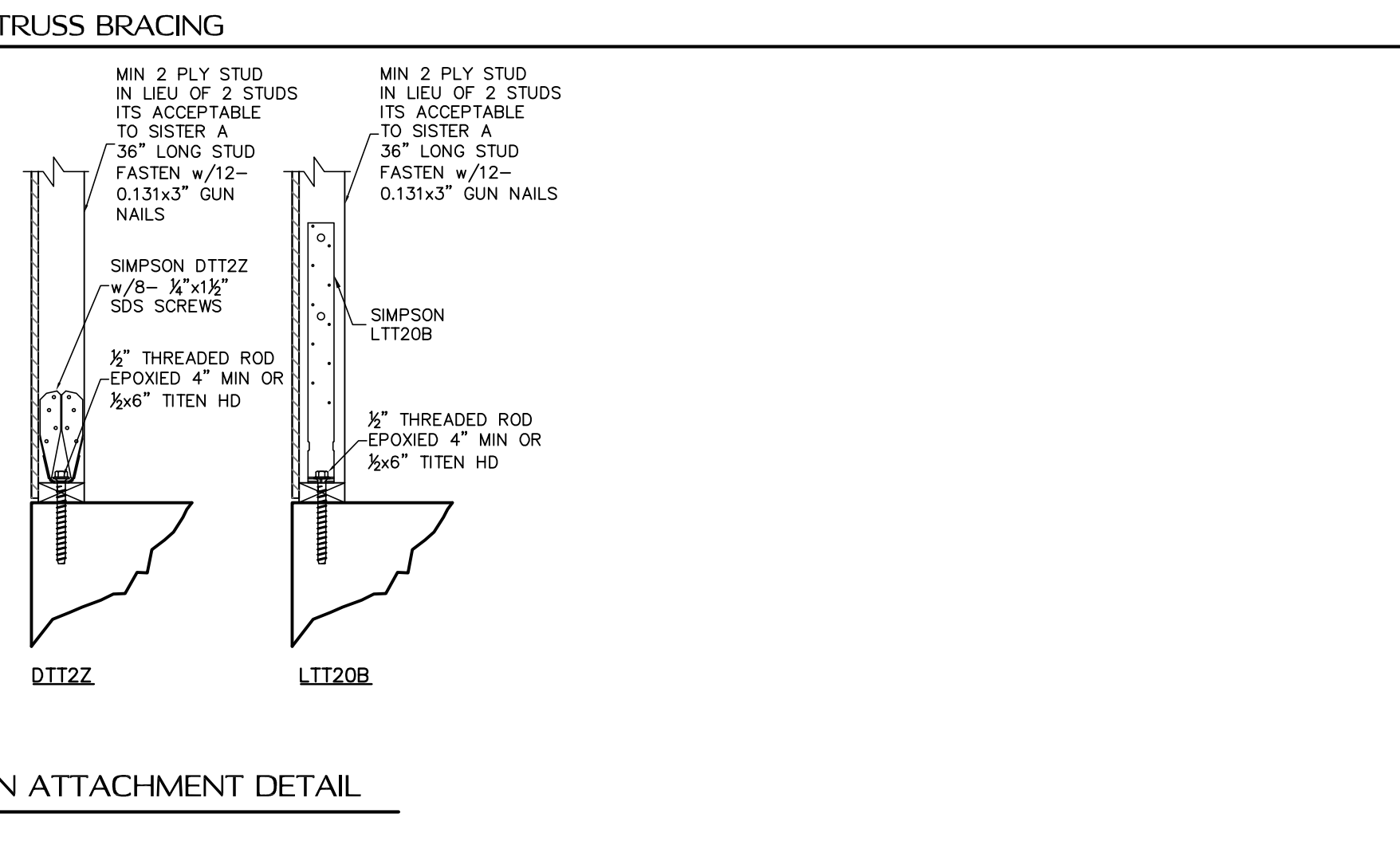
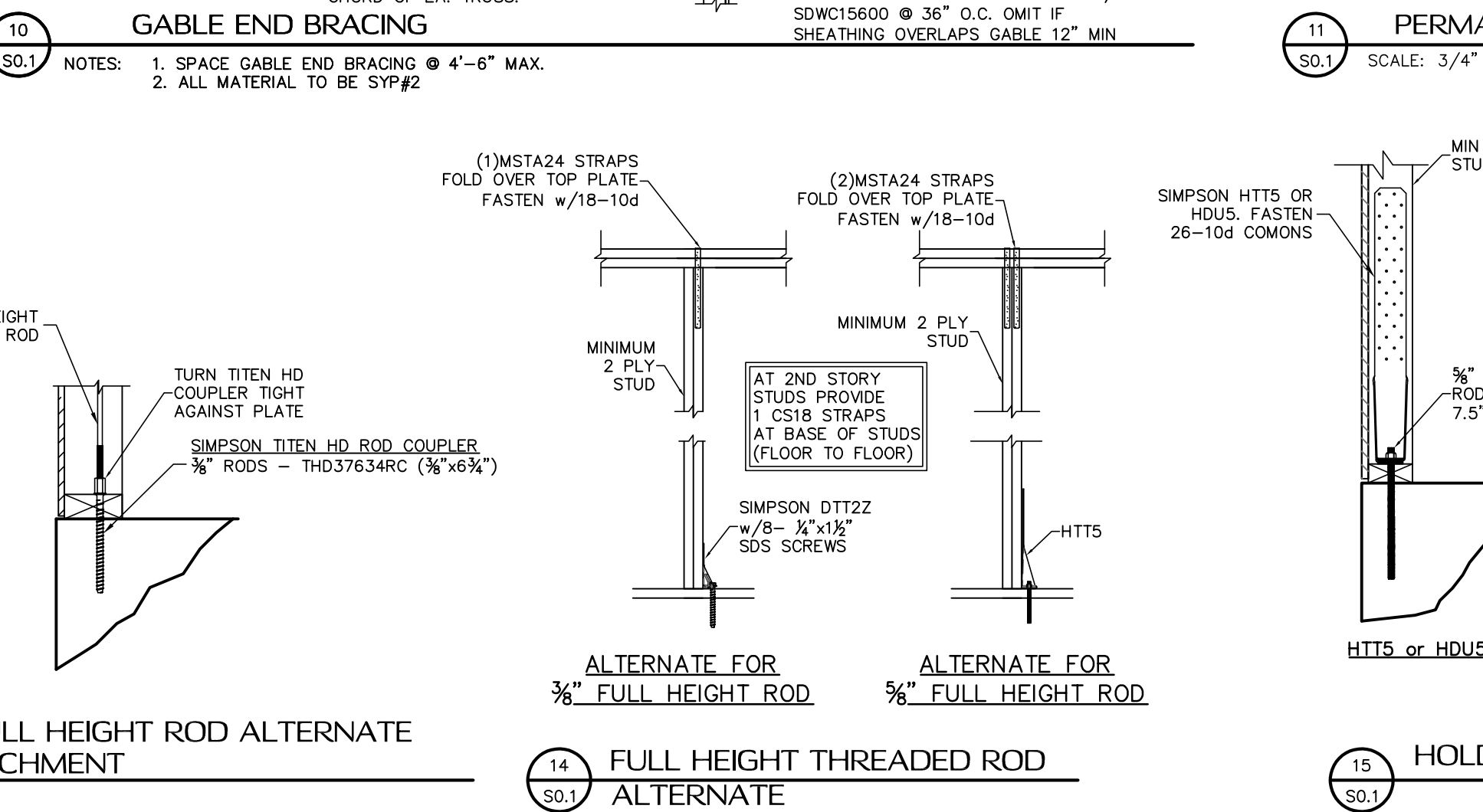
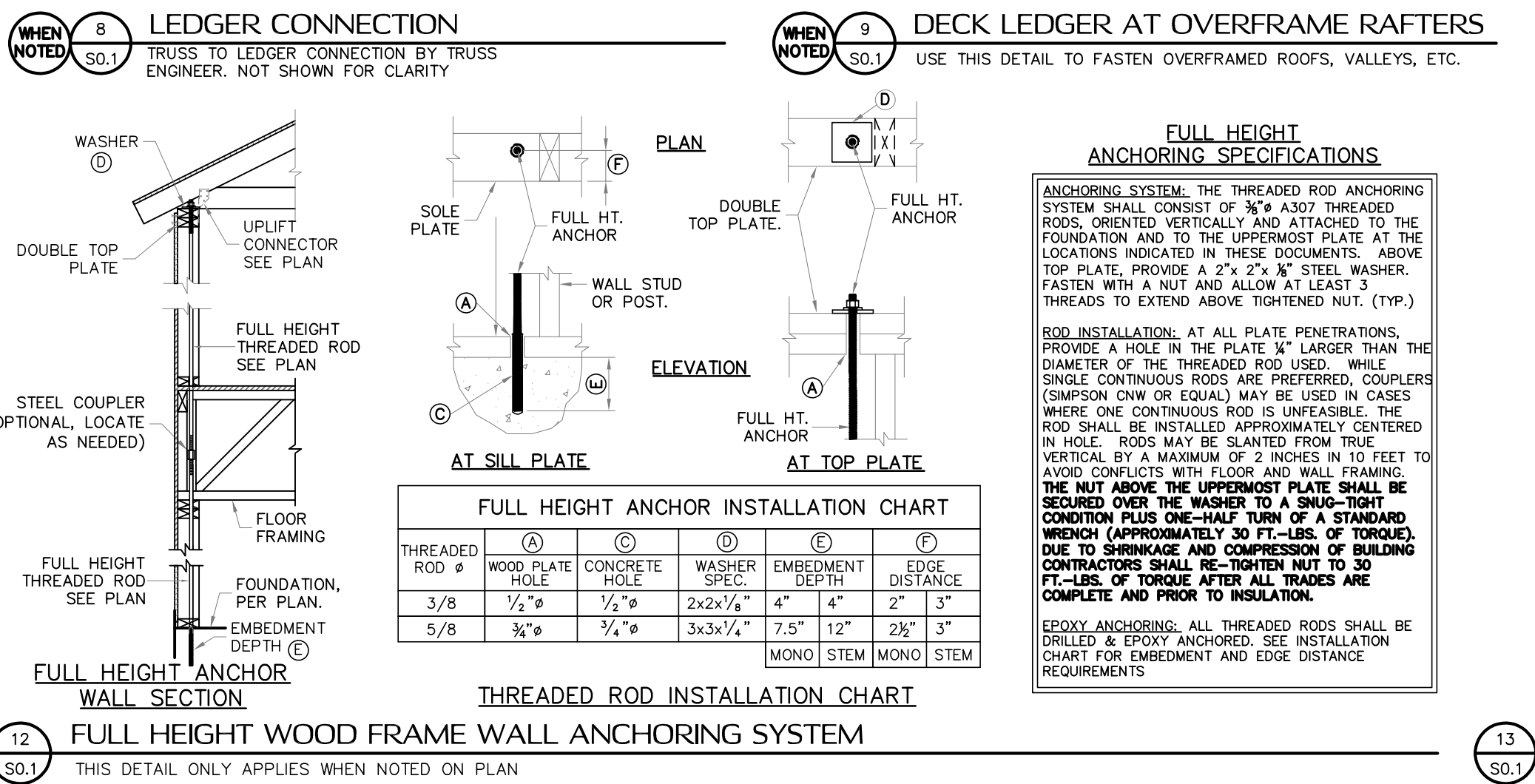
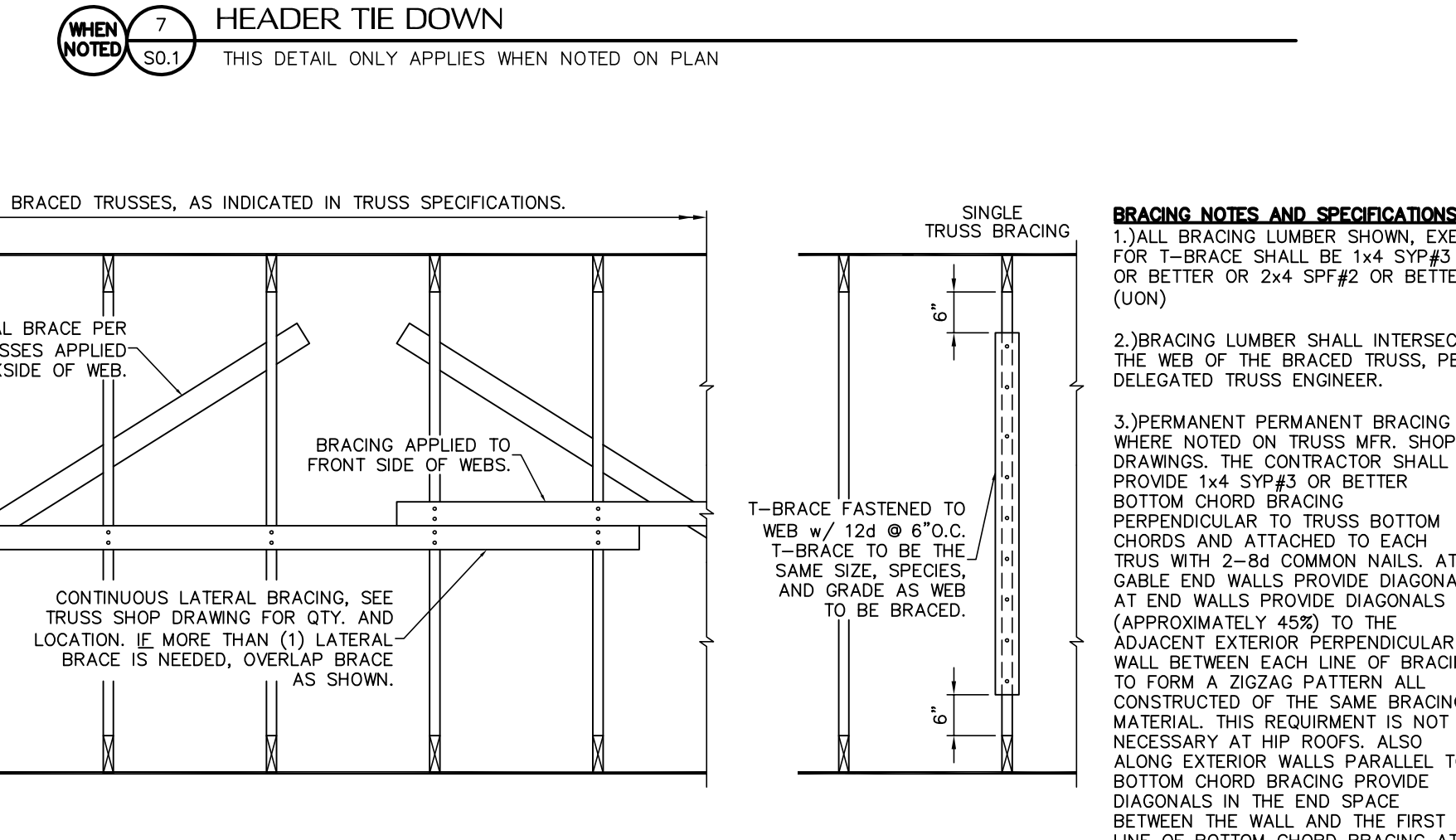
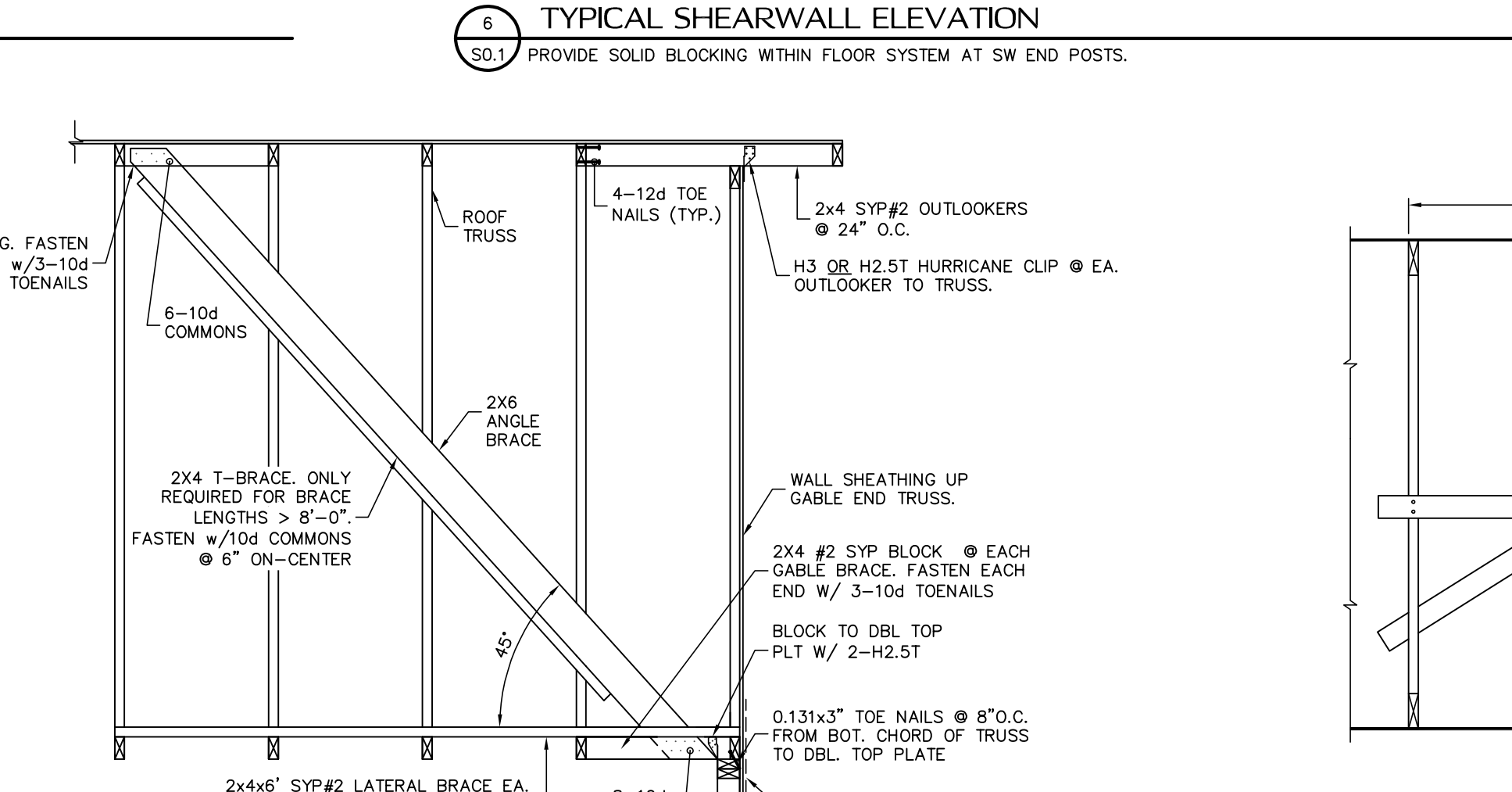
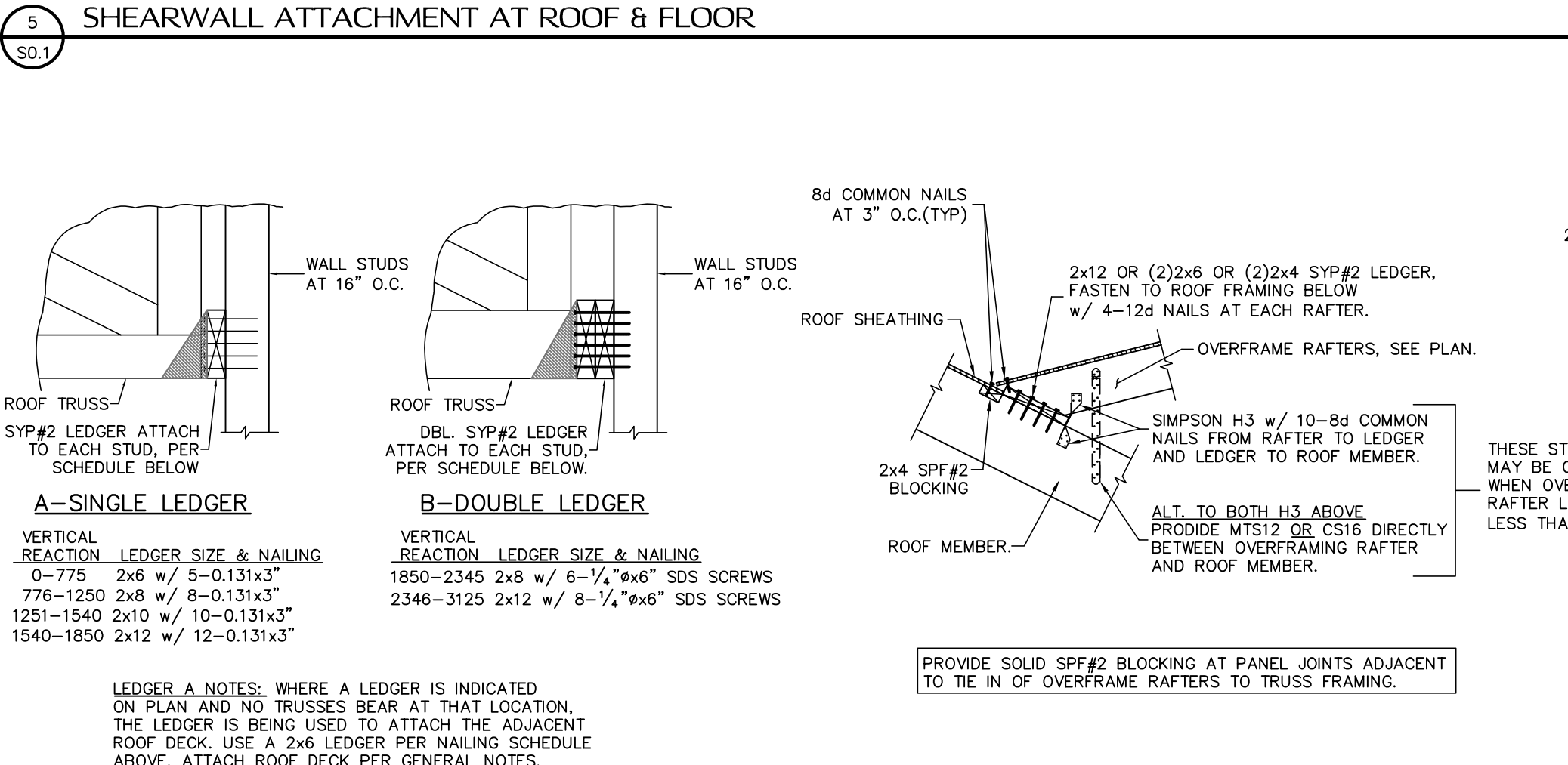
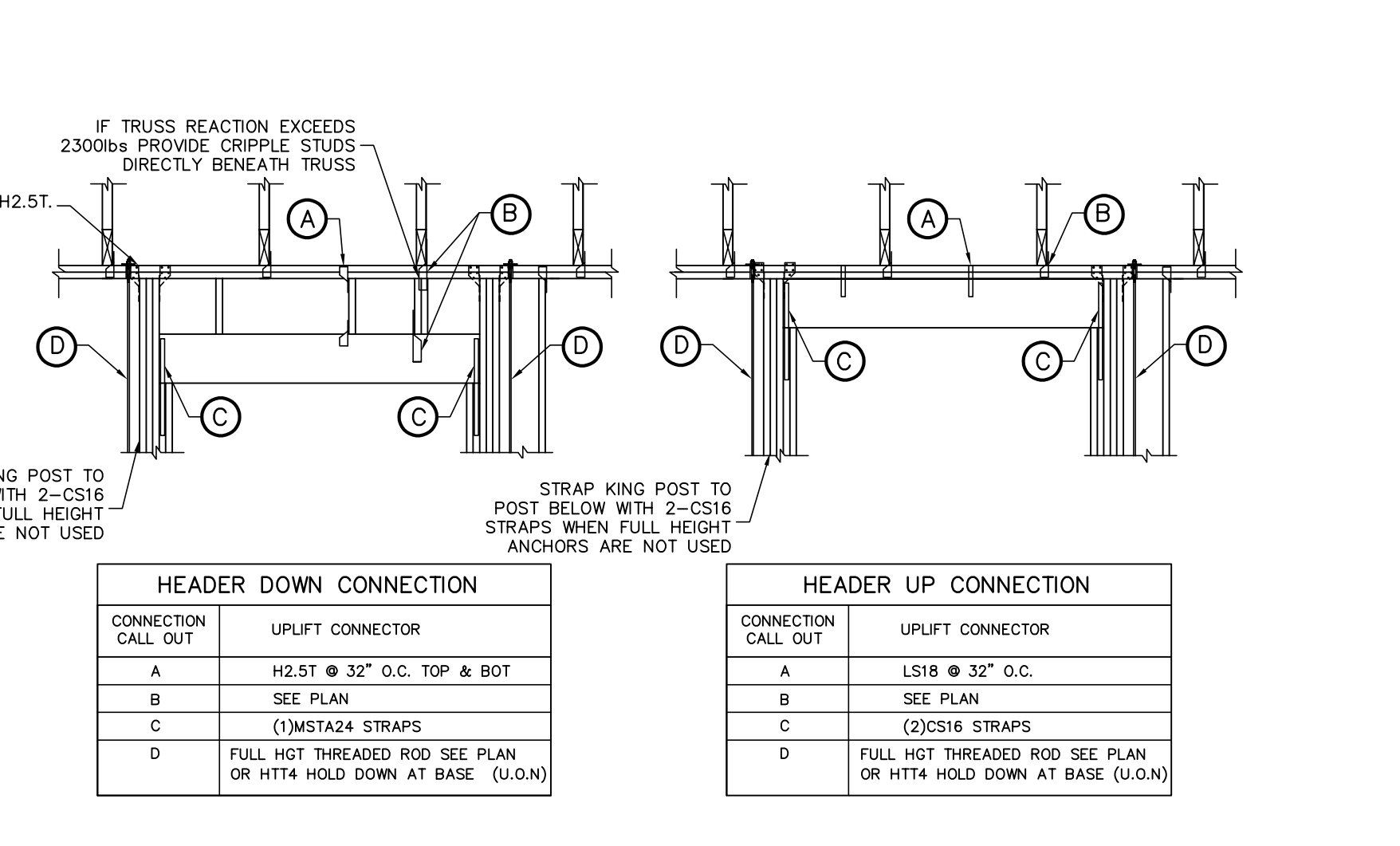
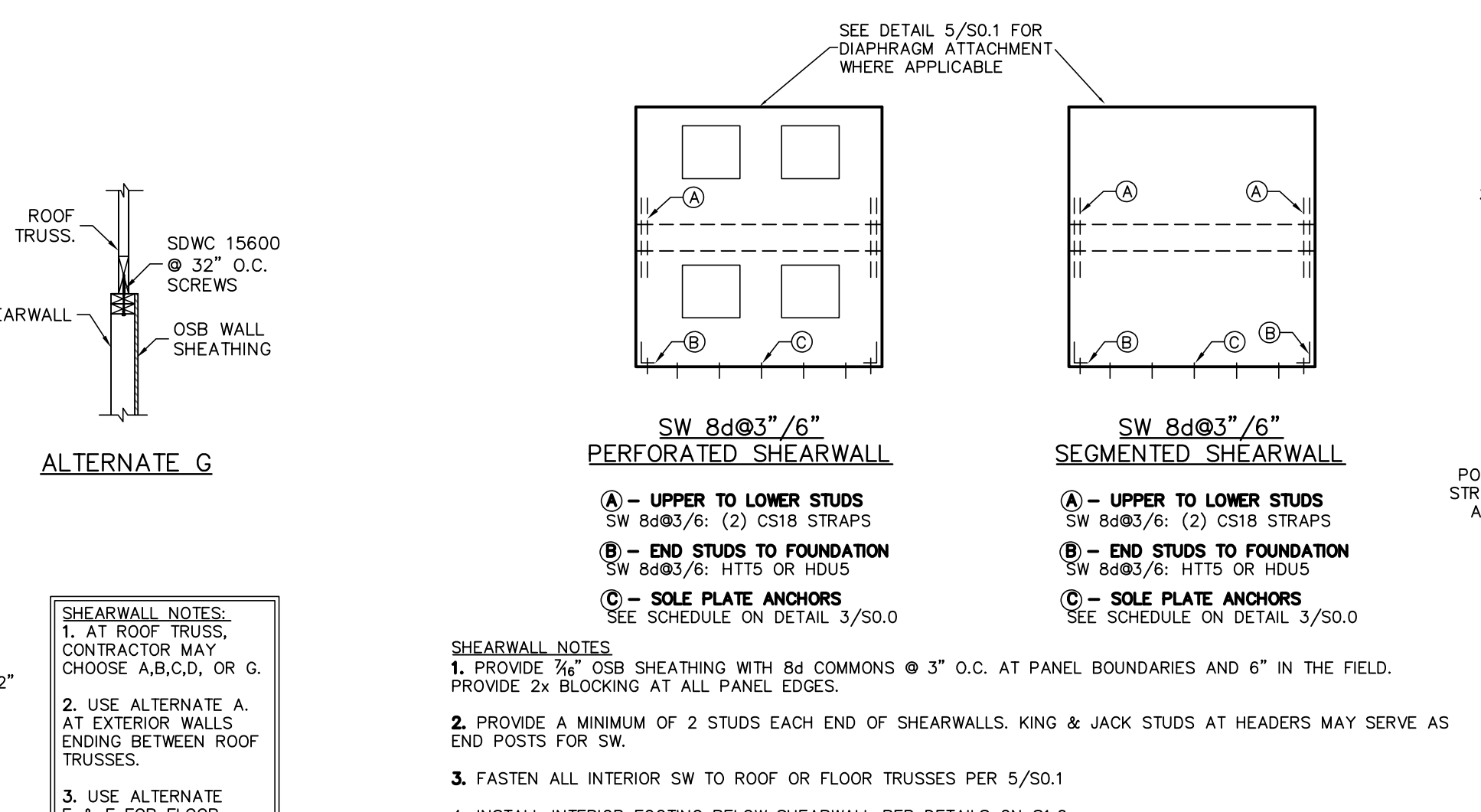
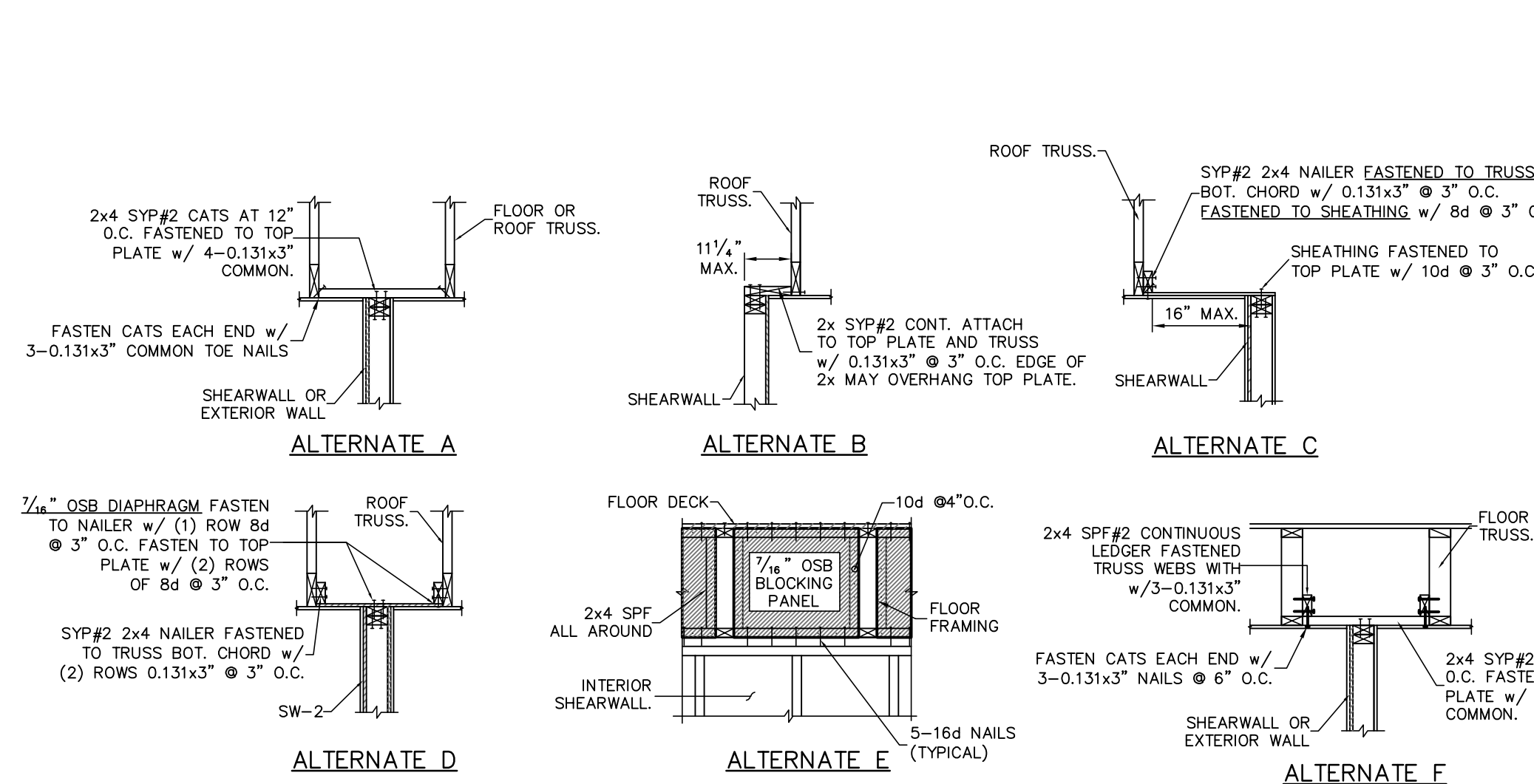
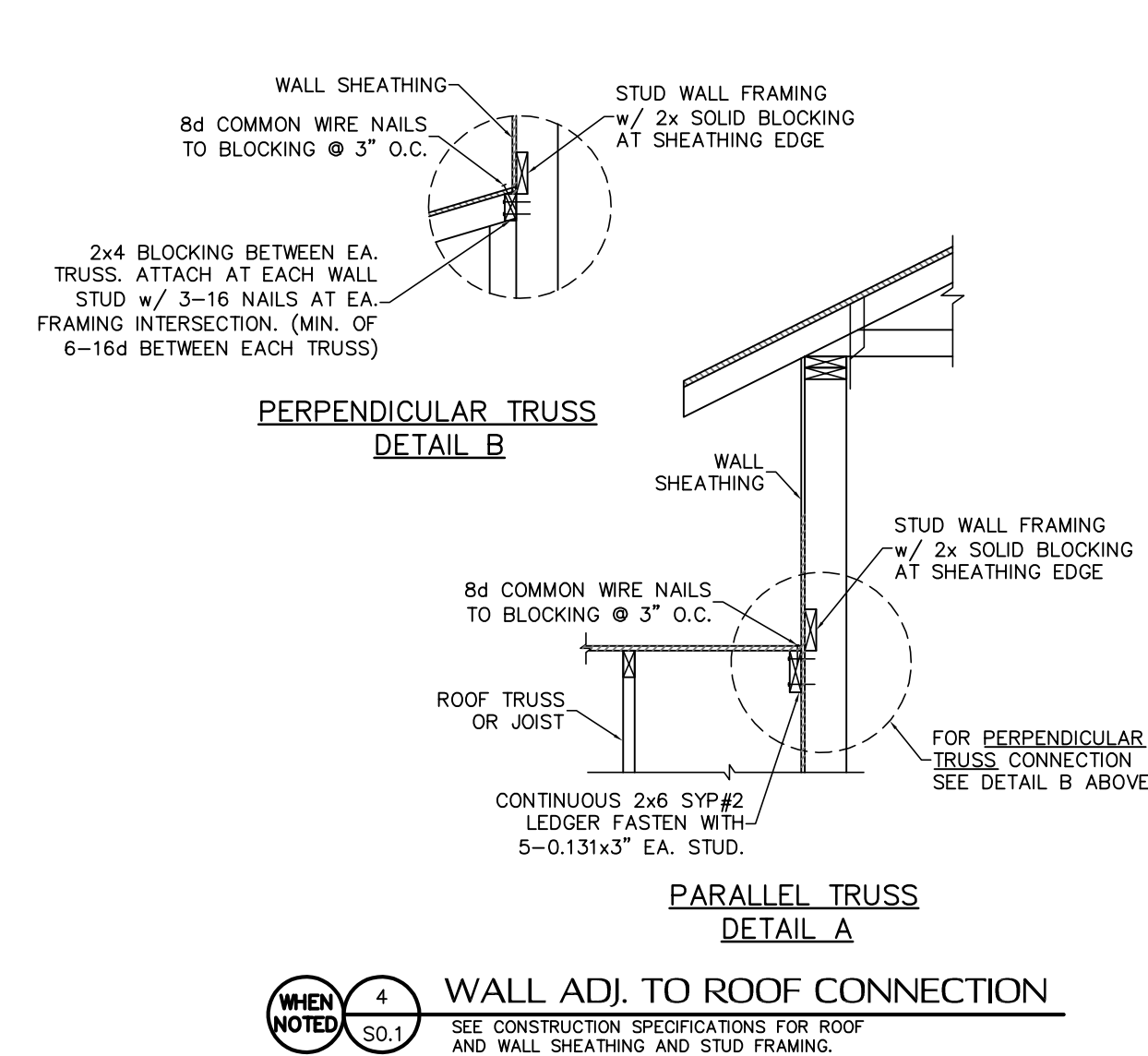
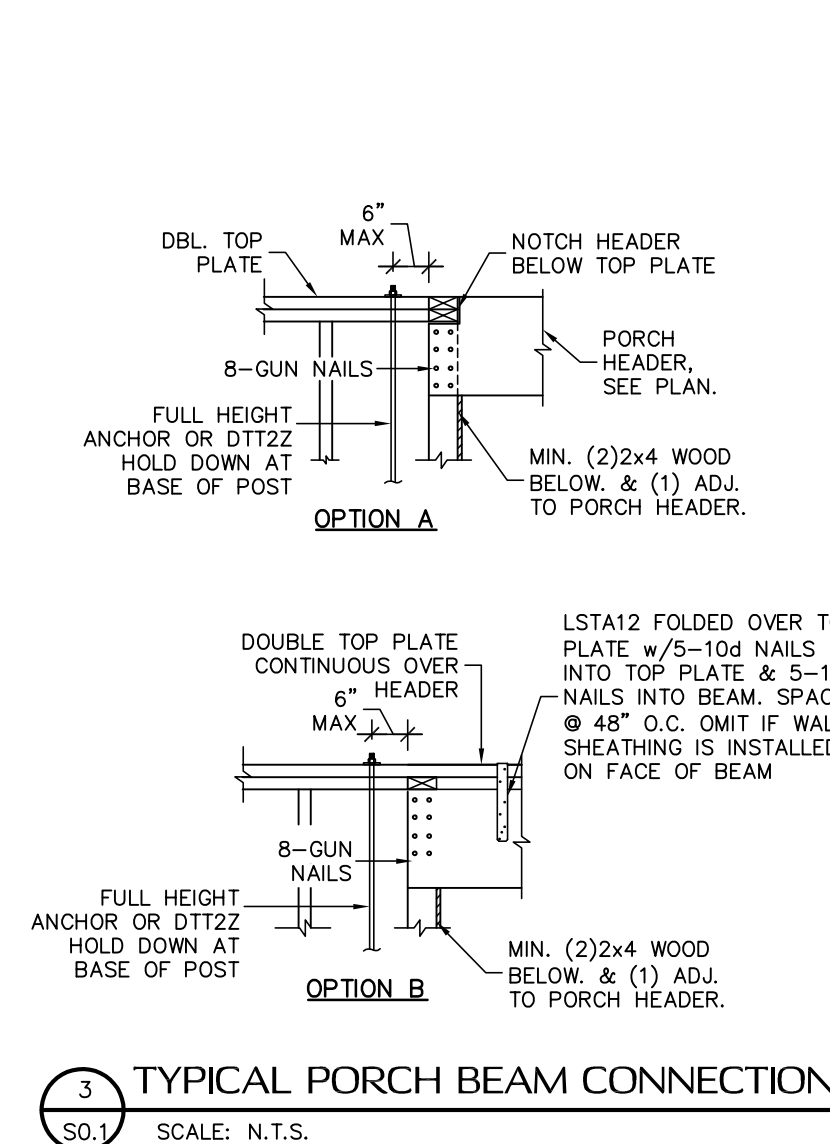
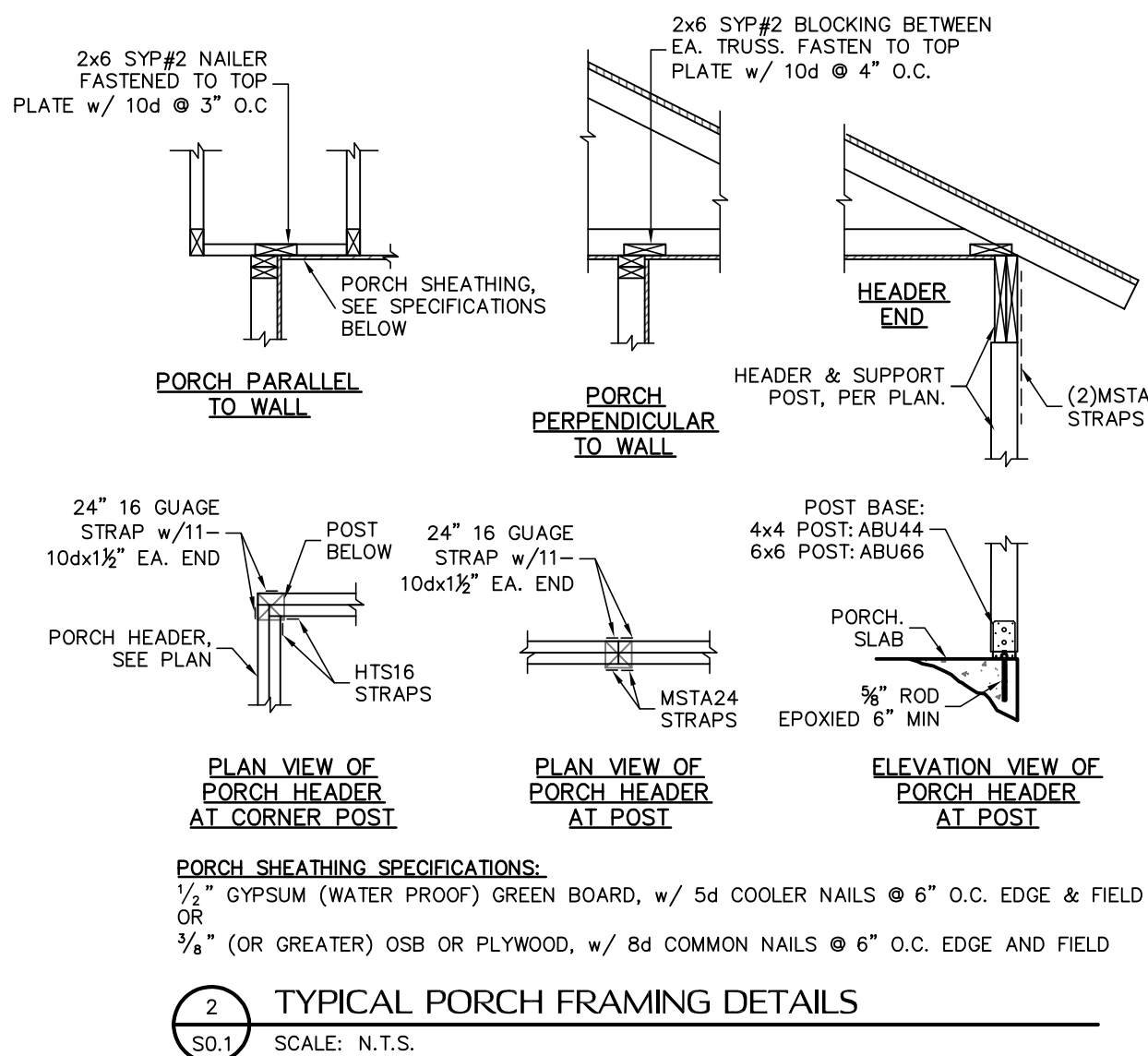
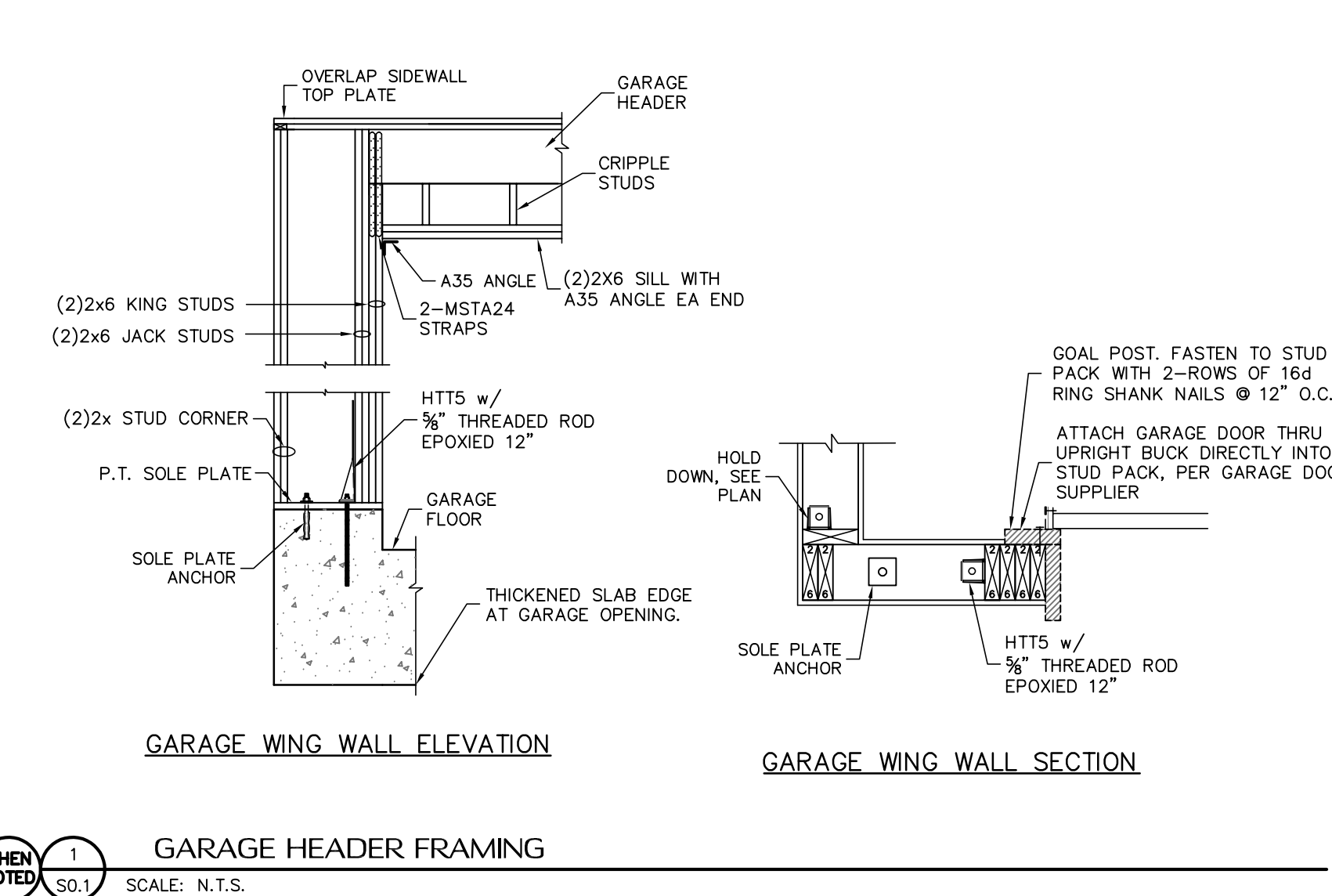
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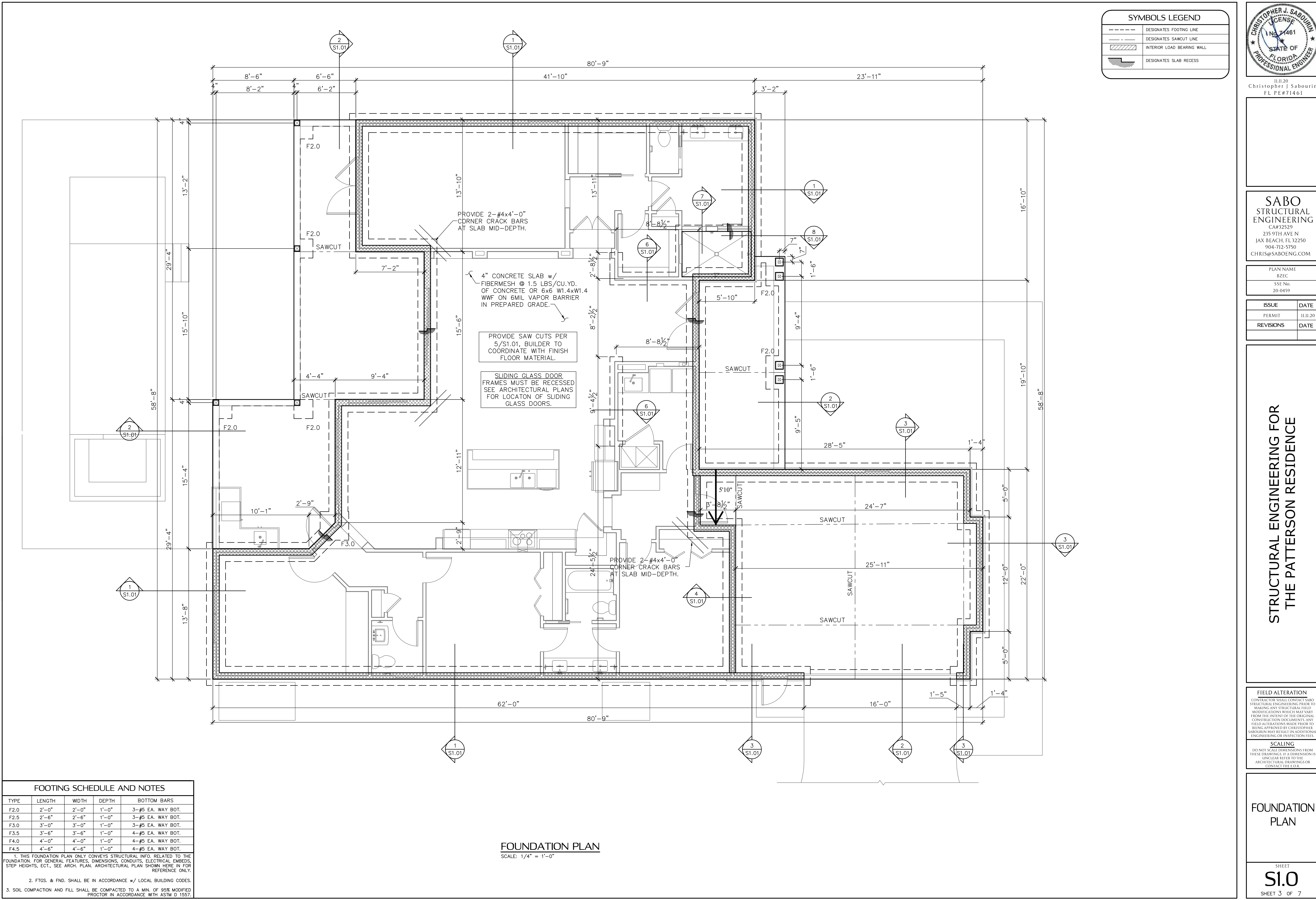
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TYPICAL
FRAMING
DETAILS

SHEET
SO.1
SHEET 2 OF 7





FOOTING SCHEDULE AND NOTES					
TYPE	LENGTH	WIDTH	DEPTH	BOTTOM BARS	
F2.0	2'-0"	2'-0"	1'-0"	3-#5 EA. WAY BOT.	
F2.5	2'-6"	2'-6"	1'-0"	3-#5 EA. WAY BOT.	
F3.0	3'-0"	3'-0"	1'-0"	3-#5 EA. WAY BOT.	
F3.5	3'-6"	3'-6"	1'-0"	4-#5 EA. WAY BOT.	
F4.0	4'-0"	4'-0"	1'-0"	4-#5 EA. WAY BOT.	
F4.5	4'-6"	4'-6"	1'-0"	4-#5 EA. WAY BOT.	
1. THIS FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFO. RELATED TO THE FOUNDATION. FOR GENERAL FEATURES, DIMENSIONS, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ECT., SEE ARCH. PLAN. ARCHITECTURAL PLAN SHOWN HERE IN FOR REFERENCE ONLY.					
2. FTGS. & FND. SHALL BE IN ACCORDANCE W/ LOCAL BUILDING CODES.					
3. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.					

SYMBOLS LEGEND	
	DESIGNATES FOOTING LINE
	DESIGNATES SAWCUT LINE
	INTERIOR LOAD BEARING WALL
	DESIGNATES SLAB RECESS

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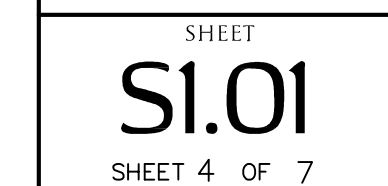
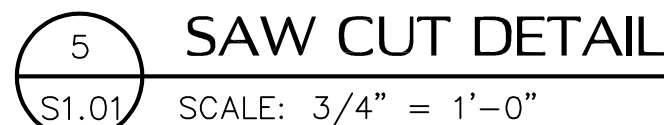
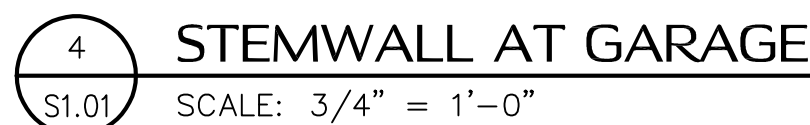
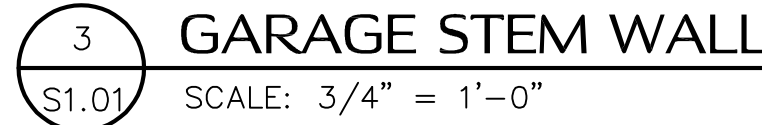
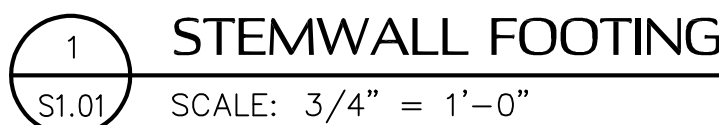
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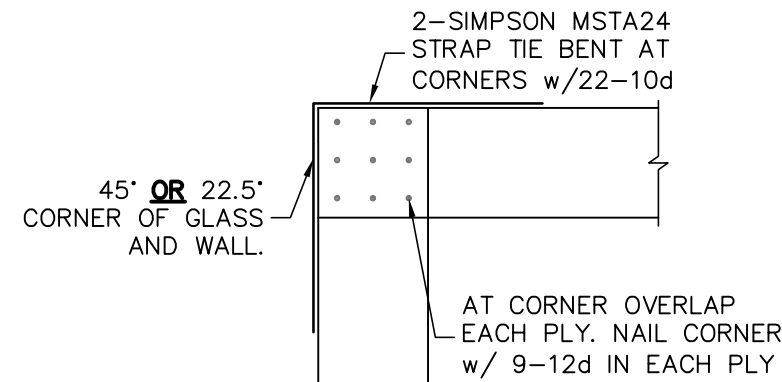
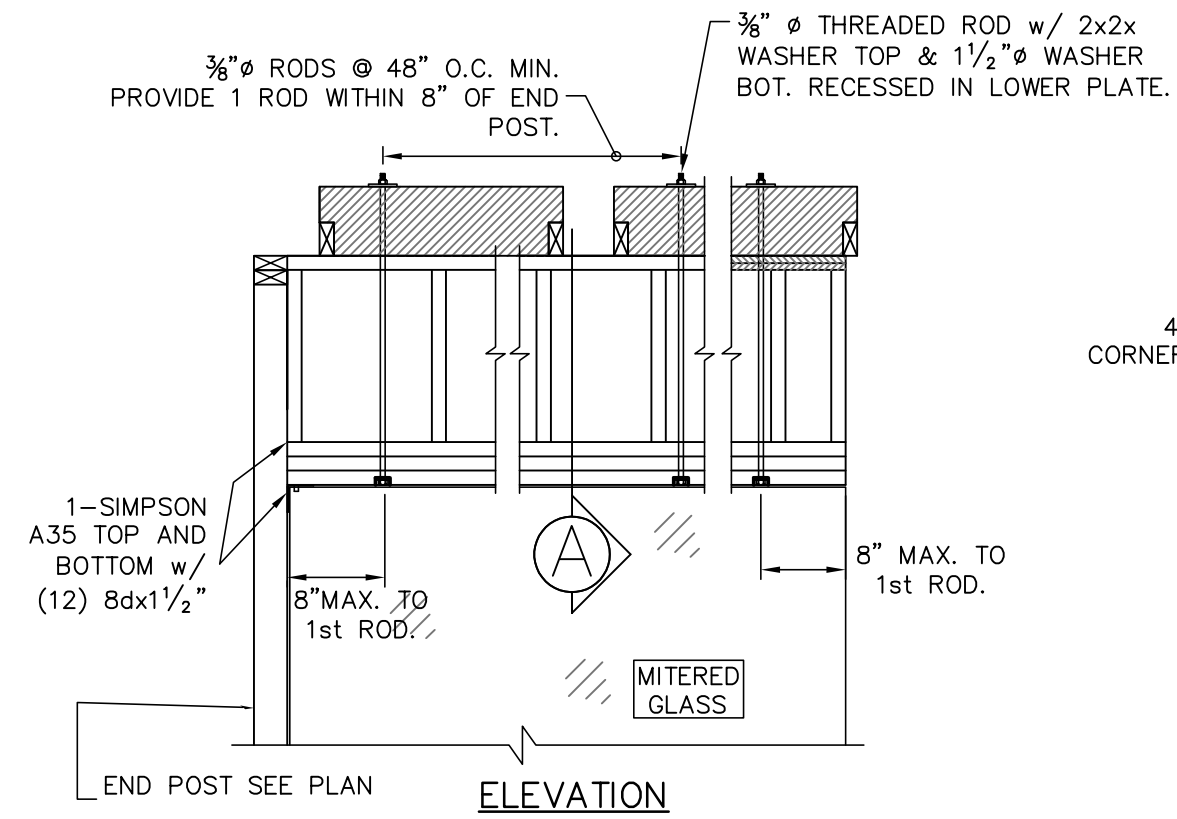
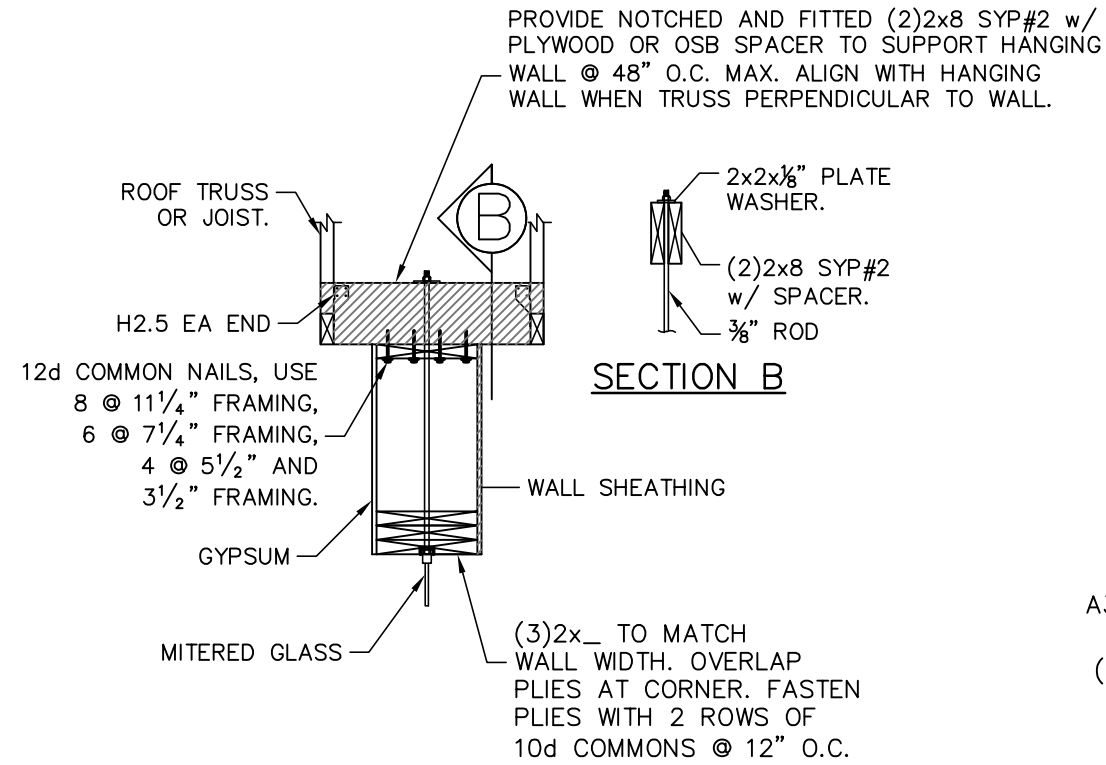
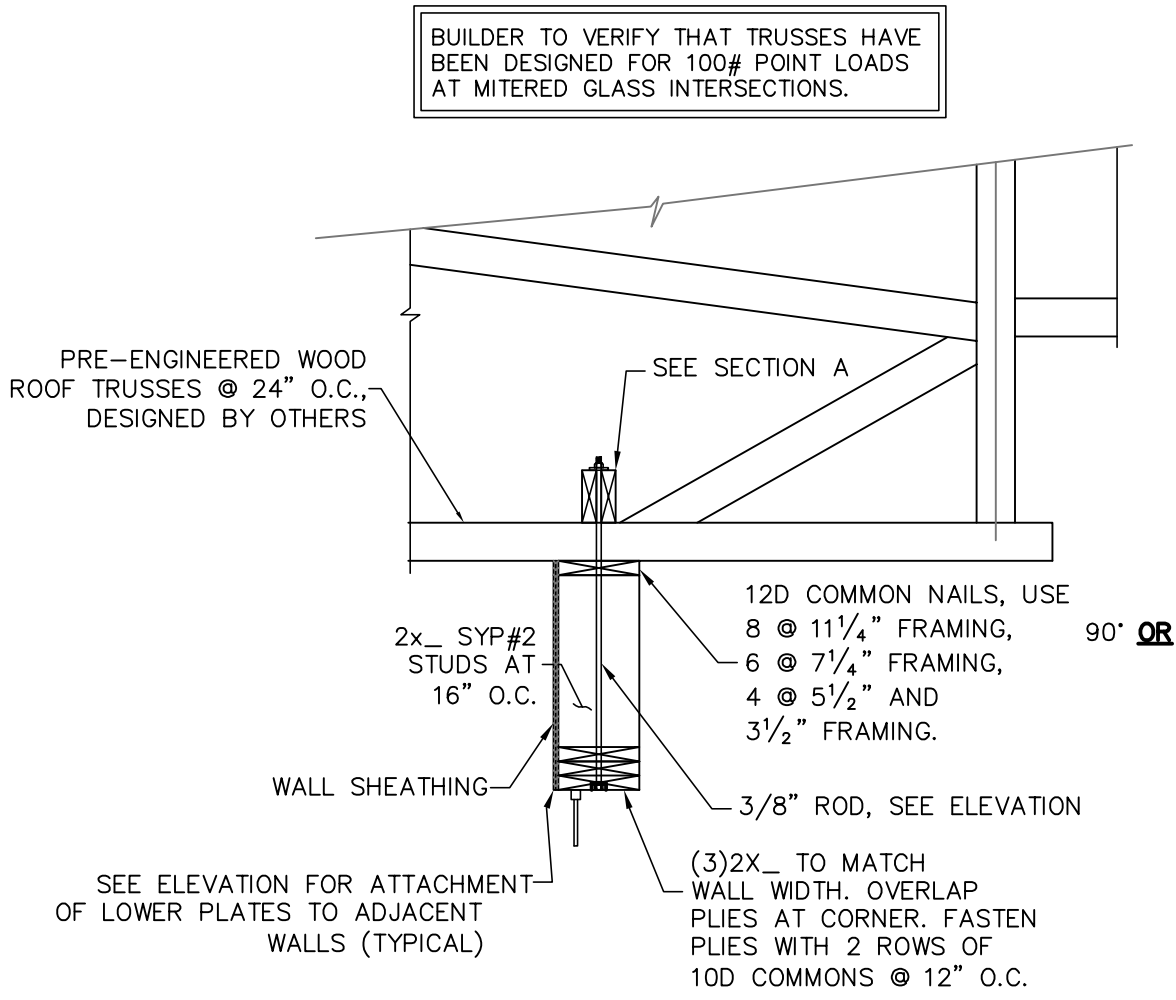
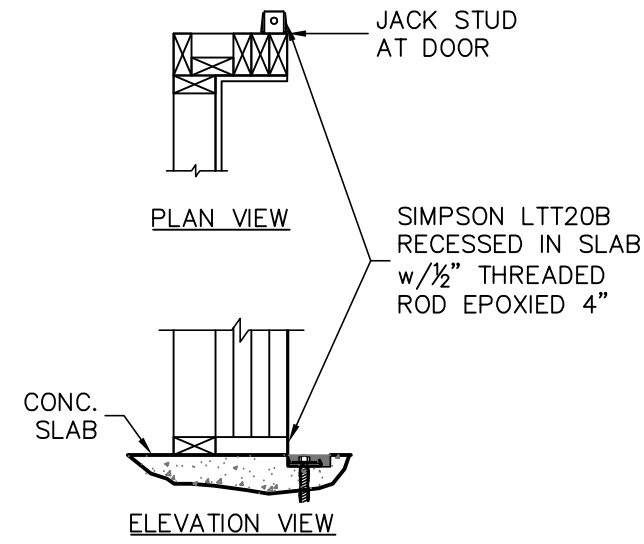
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FOUNDATION
PLAN

SHEET
S1.0

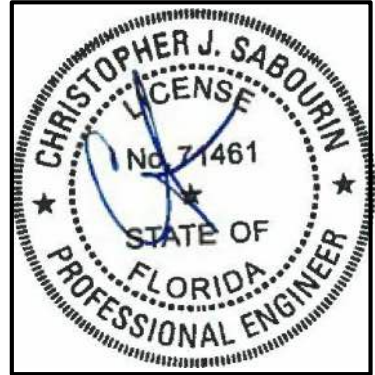
SHEET 3 OF 7





WHEN NOTED 1 S2.0 DOOR JAMB FASTENING THIS DETAIL ONLY APPLIES WHEN NOTED ON PLAN

2 S2.0 MITERED WINDOW HEAD FRAMING SCALE: N.T.S.



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MISC
DETAILS

SHEET
S2.0
SHEET 7 OF 7