Alpine, an ITW Company
155 Harlem Ave
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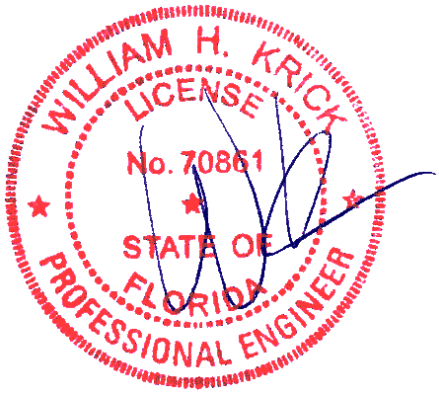

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COA #0 278

Florida Certificate of Product Approval #FL 1999

03/04/2025

Site Information:	**Page 1:**
Customer: Seminole Trusses, Inc.	*Job Number:* B60735a
Job Description: LETNER RESIDENCE	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 8th Ed. 2023 Res.	*IntelliVIEW Version:* 24.02.00C *JRef #:* 1Y808570001
Wind Standard: ASCE 7-22 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 37.00

This package contains general notes pages, 57 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	063.25.1347.46907	A1 43' Common	2	063.25.1347.48503	A1a 43' Common
3	063.25.1348.33320	A2-SG 43' Gable	4	063.25.1348.39837	A3-SG 38'2" Gable
5	063.25.1348.41463	A4 43' Common	6	063.25.1348.43060	A4a 43' Common
7	063.25.1348.44430	A5 34' Common	8	063.25.1348.45930	B1 34' Common
9	063.25.1348.50663	B2-SG 34' Gable	10	063.25.1348.52087	C1 29'2" Common
11	063.25.1348.53310	C1a 29'2" Common	12	063.25.1351.08597	C2 29'2" Common Girder
13	063.25.1348.58910	D1 26' Special	14	063.25.1349.00410	D1a 26' Special
15	063.25.1349.01977	D1b 26' Special	16	063.25.1349.03593	D2 26' Special
17	063.25.1349.05163	D3 26' Stepdown Hip Girder	18	063.25.1351.13407	E1 22'8" Common Girder
19	063.25.1351.14790	E2 22'8" Common	20	063.25.1351.16047	E2a 22'8" Common
21	063.25.1351.17787	E3-G 22'8" Gable	22	063.25.1351.19313	G1 20' Common
23	063.25.1351.20967	G2-G 20' Gable	24	063.25.1351.22303	G3 14'4" Common
25	063.25.1351.27827	H1 18'8" Common Girder	26	063.25.1351.29207	H2 18'8" Common
27	063.25.1351.31080	H3-G 18'8" Gable	28	063.25.1351.32760	JA 5'8" End Jack
29	063.25.1351.33997	JB 5'8" End Jack	30	063.25.1351.35710	K1 12'4" Common Girder
31	063.25.1351.37510	K2 12'4" Common Girder	32	063.25.1351.39107	K3-G 12'4" Gable
33	063.25.1351.40497	L1 9'2" Mono	34	063.25.1351.41603	L2 9'2" Common
35	063.25.1351.42773	L3 9'2" Common	36	063.25.1351.43937	L4 9'2" Common
37	063.25.1351.45297	M1 10'3" Common	38	063.25.1351.46593	M2-G 10'3" Gable
39	063.25.1351.47933	N1 6' Mono	40	063.25.1351.50360	N2-G 6' Gable
41	063.25.1350.55100	PB1 23'10"5 Common	42	063.25.1350.57927	PB2-G 23'10"5 Gable
43	063.25.1350.59163	PB3 22'8"9 Common	44	063.25.1351.06063	PB4 8' Common
45	063.25.1349.20083	PB5-G 8' Gable	46	063.25.1349.21333	V1 7'7"1 Valley
47	063.25.1349.25293	V2 6'2"4 Valley	48	063.25.1349.26160	V3 4'9"7 Valley
49	063.25.1349.26940	V4 3'4"10 Valley	50	063.25.1349.27923	V5 8'7"12 Valley

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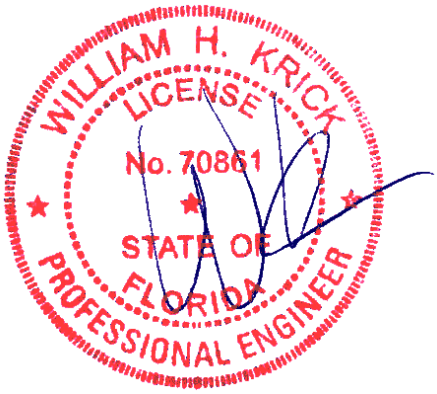

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COA #0 278

Florida Certificate of Product Approval #FL 1999

03/04/2025

Site Information:	Page 2:
Customer: Seminole Trusses, Inc.	*Job Number:* B60735a
Job Description: LETNER RESIDENCE	
Address: FL	

Item	Drawing Number	Truss
51	063.25.1349.28850	V6 7'2"15 Valley
53	063.25.1349.30807	V8 4'5"6 Valley
55	063.25.1349.22270	V10 5'3"12 Valley
57	063.25.1349.24390	V12 3'7"12 Valley
59	PB160160118	
61	REPCHRD1014	
63	VALTN220723	

Item	Drawing Number	Truss
52	063.25.1349.29827	V7 5'10"2 Valley
54	063.25.1349.32000	V9 6'8"9 Valley
56	063.25.1349.23643	V11 5'0"9 Valley
58	BRCLBSUB0119	
60	PB180160118	
62	VAL180220723	
64	GBLDIAG220923	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:
The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:
Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TE = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-BF = Boraflame Fire Retardant Treated lumber
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-ON = OnWood Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

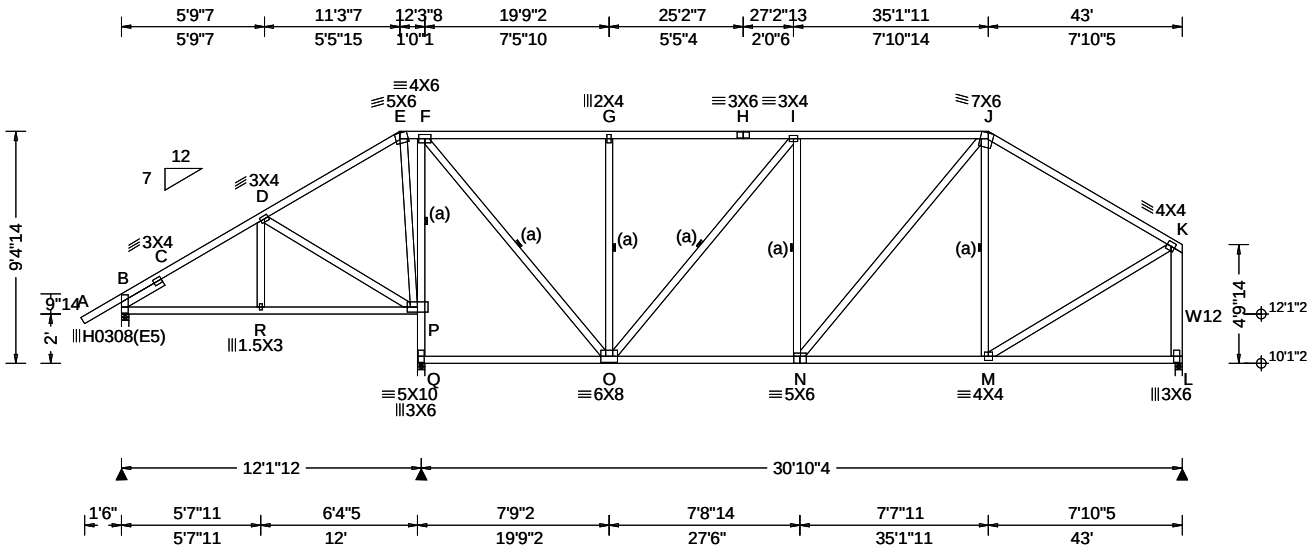
Key to Terms (continued):
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 25281 COMN FROM: RJL	Ply: 1 Qty: 3	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** A1 43' Common		Cust: R 857 JRef:1Y808570001 T20 DrwNo: 063.25.1347.46907 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.73 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.30 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.060	I	999	360
VERT(CL): 0.102	I	999	240
HORZ(LL): -0.019	J	-	-
HORZ(TL): 0.032	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.907
Max BC CSI: 0.580
Max Web CSI: 0.577

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	568	/-	/-	/279	/111	/321
Q	1957	/-	/-	/929	/256	/-
L	1368	/-	/-	/686	/120	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
Q Brg Wid = 3.5 Min Req = 2.3 (Truss)
L Brg Wid = 3.5 Min Req = 1.6 (Truss)
Bearings B, Q, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	361	- 602	G - H	643	- 946
C - D	376	- 533	H - I	643	- 946
E - F	392	- 211	I - J	658	- 1270
F - G	643	- 946	J - K	501	- 1225

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	423	- 165	O - N	1271	- 465
R - P	421	- 167	N - M	980	- 377

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - P	184	- 438	G - O	352	- 430
P - Q	699	- 1789	N - J	453	- 215
P - F	626	- 1470	M - K	1129	- 354
F - O	1440	- 475	K - L	461	- 1314
O - I	244	- 513			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W12 2x6 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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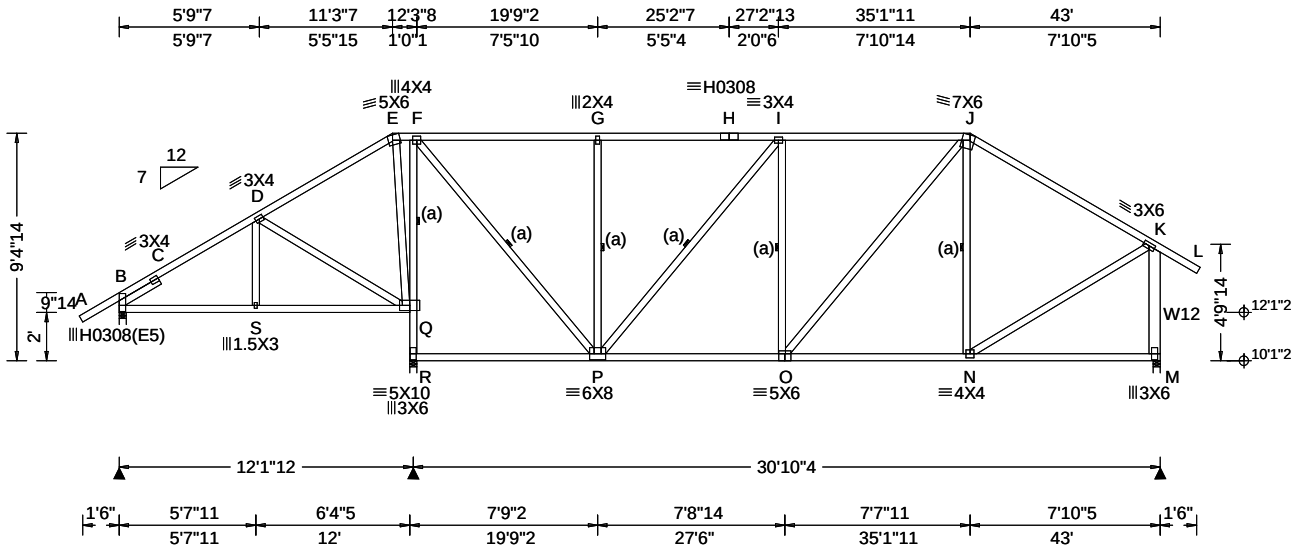
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25284 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** A1a 43' Common	Cust: R 857 JRef:1Y808570001 T45 DrwNo: 063.25.1347.48503 SSB / DF 03/04/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.73 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.30 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.060	I	999	360
VERT(CL): 0.102	I	999	240
HORZ(LL): -0.019	J	-	-
HORZ(TL): 0.032	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.675
Max BC CSI: 0.579
Max Web CSI: 0.969

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	568	/-	/-	/278	/126	/318
R	1955	/-	/-	/944	/240	/-
M	1466	/-	/-	/683	/140	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
R Brg Wid = 3.5 Min Req = 2.3 (Truss)
M Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings B, R, & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	416	- 602	G - H	687	- 943
C - D	431	- 532	H - I	687	- 943
D - E	430	- 265	I - J	695	- 1267
E - F	445	- 208	J - K	537	- 1222
F - G	687	- 943			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - S	423	- 159	P - O	1267	- 451
S - Q	421	- 160	O - N	971	- 353

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - Q	181	- 438	G - P	351	- 429
Q - R	668	- 1787	O - J	460	- 231
Q - F	615	- 1468	N - K	1120	- 326
F - P	1436	- 465	K - M	543	- 1412
P - I	235	- 511			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W12 2x6 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical exposed to wind pressure. Deflection meets L/360.
Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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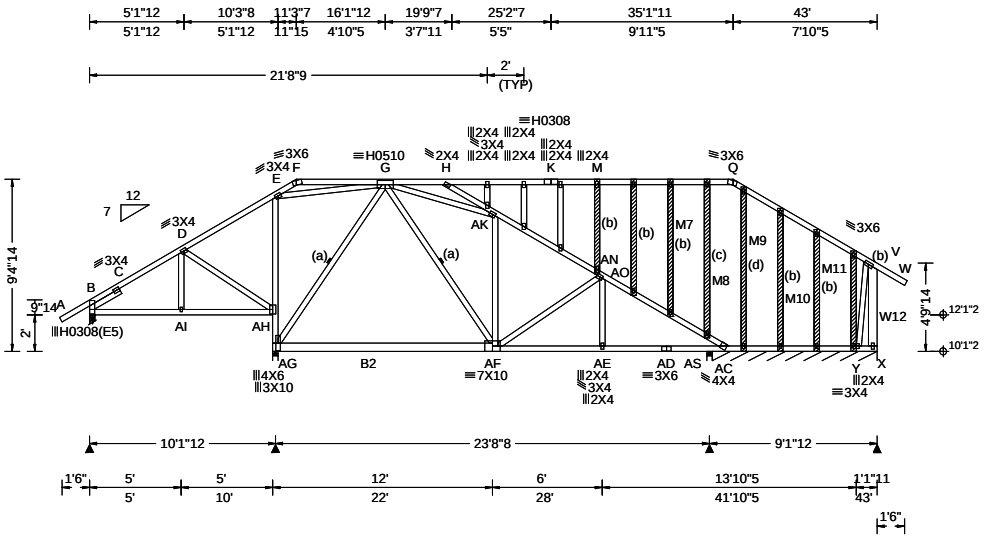
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25216 GABL	Ply: 1	**Job Number:** B60735a		Cust: R 857 JRef:1Y808570001 T11	
FROM: RJL	Qty: 1	LETNER RESIDENCE		DrwNo: 063.25.1348.33320	
Page 1 of 2		**Truss Label:** A2-SG 43' Gable		SSB / DF 03/04/2025	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.084 O 999 360
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.188 O 999 240
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.040 AQ - -
Des Ld:	37.00	EXP: C Kzt: NA		HORZ(TL): 0.090 AQ - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.502
Load Duration:	1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.797
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.884
		C&C Dist a: 4.30 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 12.11 ft	Plate Type(s):	
		GCpi: 0.18	HS, WAVE	VIEW Ver: 24.02.00C.1213.15
		Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	644	/-	/-	/401	/345	/540
AG	2291	/-	/-	/1046	/405	/-
AS	306	/-	/-	/161	/-	/-
X*	295	/-	/-	/101	/65	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
AG Brg Wid = 3.5 Min Req = 2.7 (Truss)
AS Brg Wid = 3.5 Min Req = 1.5 (Truss)
X Brg Wid = 108 Min Req = -
Bearings B, AG, AS, & AS are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	758	- 661	H - K	465	- 222
C - D	774	- 642	K - M	466	- 222
D - E	773	- 518	M - Q	472	- 224
E - F	347	- 528	Q - V	496	- 256
G - H	684	- 403			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B -AI	428	- 146	AF-AE	2038	- 1305
AI-AH	427	- 148	AE-AD	2038	- 1303
AG-AF	784	- 324	AD-AC	4076	- 2606

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D -AH	204	- 525	H -AK	341	- 680
AH- E	212	- 677	AF-AO	771	- 833
AH-AG	274	- 995	AK-AN	1006	- 1615
E - G	460	- 296	AN-AO	1175	- 1875
AG- G	635	- 1477	AO-AC	1555	- 2563
G -AF	1035	- 652	Y - V	405	- 138
G -AK	797	- 978			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
AN- M	408	- 530

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3; W12 2x6 SP #1; M7,M8,M9,M10, M11 2x4 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

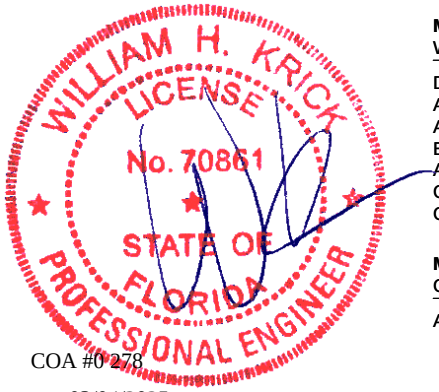
Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/145.

Gable Reinforcement

(b) Scab reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(c) Scab reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(d) HF/SPF #2 or better Scab reinforcement. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.



COA #0278
03/04/2025
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

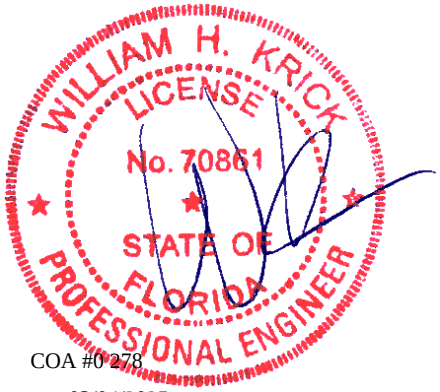
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25216 GABL	Ply: 1	**Job Number:** B60735a	Cust: R 857 JRef:1Y808570001 T11
FROM: RJL	Qty: 1	LETNER RESIDENCE	DrwNo: 063.25.1348.33320
Page 2 of 2		**Truss Label:** A2-SG 43' Gable	SSB / DF 03/04/2025

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)



COA #0 278

03/04/2025

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ALPINE
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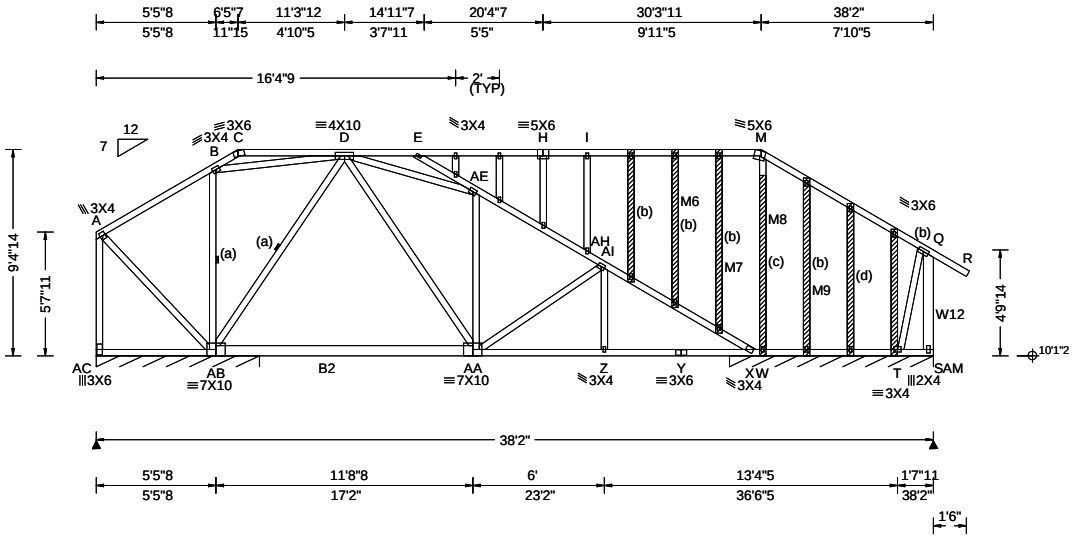
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25212 GABL	Ply: 1	**Job Number:** B60735a	Cust: R 857 JRef:1Y808570001 T14
FROM: RJL	Qty: 1	LETNER RESIDENCE	DrwNo: 063.25.1348.39837
Page 1 of 2		**Truss Label:** A3-SG 38'2" Gable	SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.82 ft
Loc. from endwall: not in 12.11 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.096	K	999	360
VERT(CL): 0.223	K	999	240
HORZ(LL): -0.038	AK	-	-
HORZ(TL): 0.088	AK	-	-

Creep Factor: 2.0
Max TC CSI: 0.824
Max BC CSI: 0.400
Max Web CSI: 0.941

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity		Non-Gravity		
Loc R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC*327	/-	/-	/118	/53	/62
AM*291	/-	/-	/98	/48	/-
AC	/-197				
W	/-125				

Wind reactions based on MWFRS
AC Brg Wid = 89.3 Min Req = -
AM Brg Wid = 111 Min Req = -
Bearings AC & X are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	531	- 143	H - I	391	- 185
D - E	603	- 343	I - M	397	- 186
E - H	391	- 185	M - Q	431	- 216

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	1106	- 725	Y - X	1863	- 820
AA- Z	1862	- 822	X - T	389	- 214
Z - Y	1863	- 820			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A -AB	237	- 408	E -AE	310	- 629
B -AB	364	- 907	AA-AI	395	- 880
B - D	492	- 80	AE-AH	512	- 1416
AB- D	668	- 1573	AH-AI	634	- 1636
D -AA	1066	- 333	AI- X	953	- 2380
D -AE	273	- 817	T - Q	496	- 197

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
AH- I	248	- 448

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3; W12 2x6 SP #1; M6,M7,M8, M9 2x4 SP #1;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

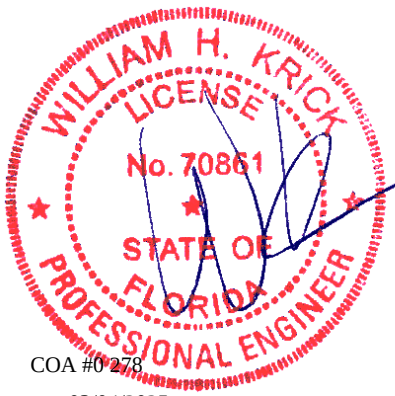
Right end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/155.

Gable Reinforcement

(b) Scab reinforcement. Any species and grade. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(c) HF/SPF #2 or better Scab reinforcement. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.
(d) Scab reinforcement. Same species and grade as web. Full truss height along web member. Attach to the wide face with 10d (0.131"x3",min.) nails @ 4" oc in the web plus (2)10d (0.131"x3",min.) nails in each chord.



COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25212 GABL	Ply: 1	**Job Number:** B60735a	Cust: R 857 JRef:1Y808570001 T14
FROM: RJL	Qty: 1	LETNER RESIDENCE	DrwNo: 063.25.1348.39837
Page 2 of 2		**Truss Label:** A3-SG 38'2" Gable	SSB / DF 03/04/2025

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

COA #0 278

03/04/2025

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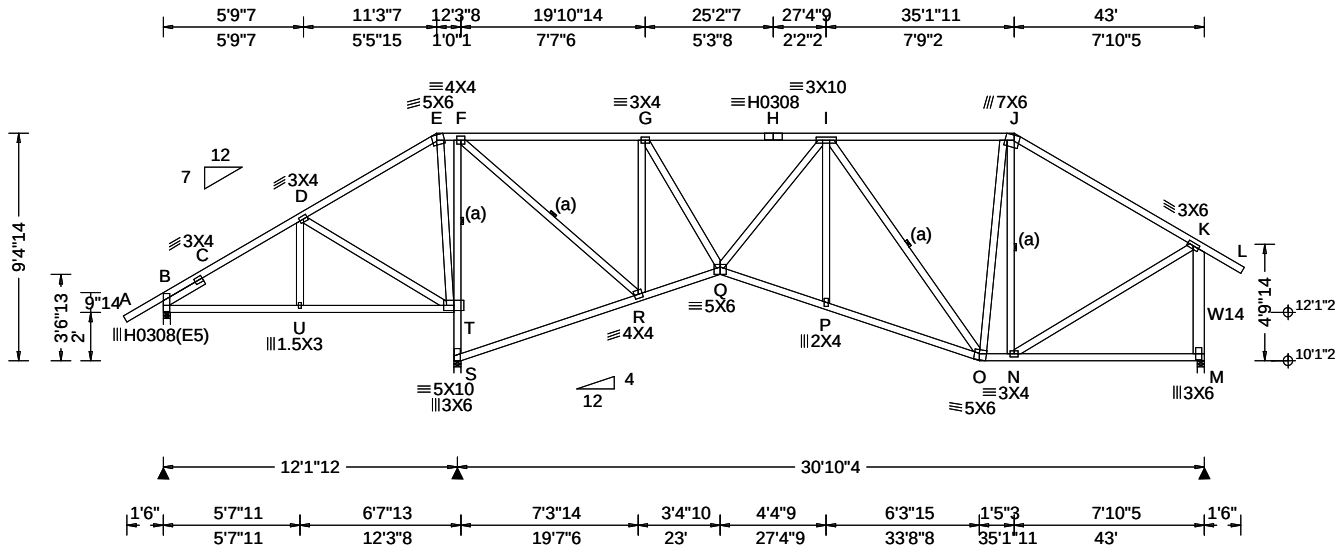
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25273 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** A4 43' Common	Cust: R 857 JRef:1Y808570001 T43 DrwNo: 063.25.1348.41463 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.73 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.30 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.074	Q	999	360
VERT(CL): 0.139	Q	999	240
HORZ(LL): -0.040	S	-	-
HORZ(TL): 0.076	S	-	-

Creep Factor: 2.0
Max TC CSI: 0.622
Max BC CSI: 0.540
Max Web CSI: 0.956

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	554	/-	/-	/271	/100	/319
S	1695	/-	/-	/966	/240	/-
M	1266	/-	/-	/688	/135	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
S Brg Wid = 3.5 Min Req = 1.9 (Truss)
M Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, S, & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	406	- 577	G - H	819	- 1426
C - D	421	- 508	H - I	819	- 1426
D - E	419	- 252	I - J	571	- 835
E - F	438	- 203	J - K	531	- 1014
F - G	748	- 1008			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	402	- 147	Q - P	1396	- 610
U - T	400	- 149	P - O	1409	- 611
R - Q	1105	- 439	O - N	792	- 346

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - T	182	- 442	G - Q	775	- 342
T - S	676	- 1626	I - O	366	- 843
T - F	621	- 1309	N - K	913	- 318
F - R	1370	- 529	K - M	535	- 1204
R - G	571	- 1110			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W14 2x6 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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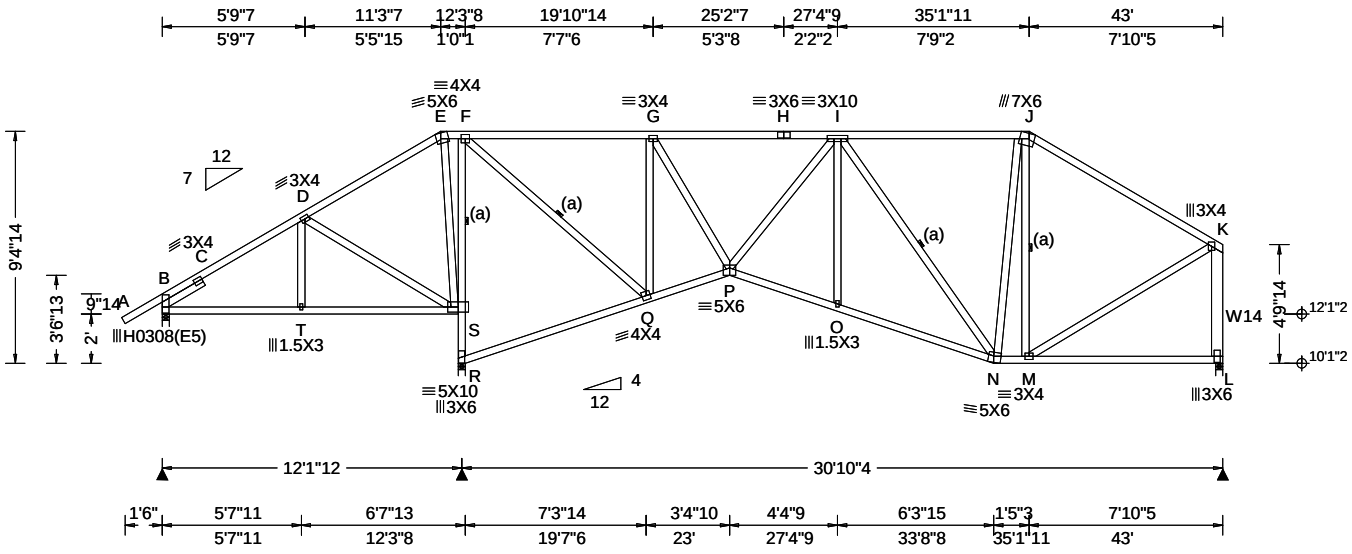
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25277 COMN FROM: RJL	Ply: 1 Qty: 3	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** A4a 43' Common	Cust: R 857 JRef:1Y808570001 T28 DrwNo: 063.25.1348.43060 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.73 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 4.30 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.073	P	999	360
VERT(CL): 0.139	P	999	240
HORZ(LL): -0.039	R	-	-
HORZ(TL): 0.076	R	-	-

Creep Factor: 2.0
Max TC CSI: 0.838
Max BC CSI: 0.540
Max Web CSI: 0.959

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
B	554	/-	/-	/272	/85	/322
R	1697	/-	/-	/950	/257	/-
L	1168	/-	/-	/690	/115	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
R Brg Wid = 3.5 Min Req = 1.9 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, R, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	351	- 577	G - H	786	- 1431
C - D	366	- 508	H - I	786	- 1431
E - F	385	- 205	I - J	543	- 842
F - G	706	- 1011	J - K	495	- 1017

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - T	402	- 153	P - O	1401	- 629
T - S	400	- 155	O - N	1414	- 630
Q - P	1108	- 451	N - M	800	- 370

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - S	184	- 442	G - P	778	- 354
S - R	708	- 1628	I - N	361	- 838
S - F	633	- 1311	M - K	922	- 346
F - Q	1374	- 542	K - L	453	- 1106
Q - G	583	- 1113			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3; W14 2x6 SP #1;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

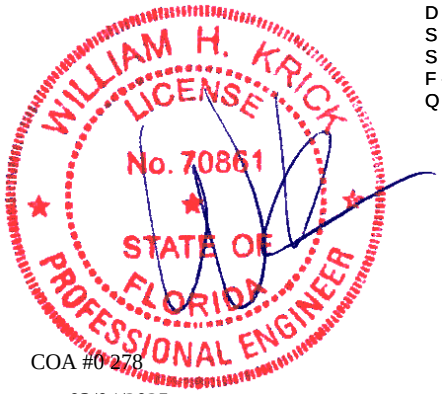
(a) Continuous lateral restraint equally spaced on member.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/360.

Wind loading based on both gable and hip roof types.



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Florida Certificate of Product Approval #FL 1999

03/04/2025

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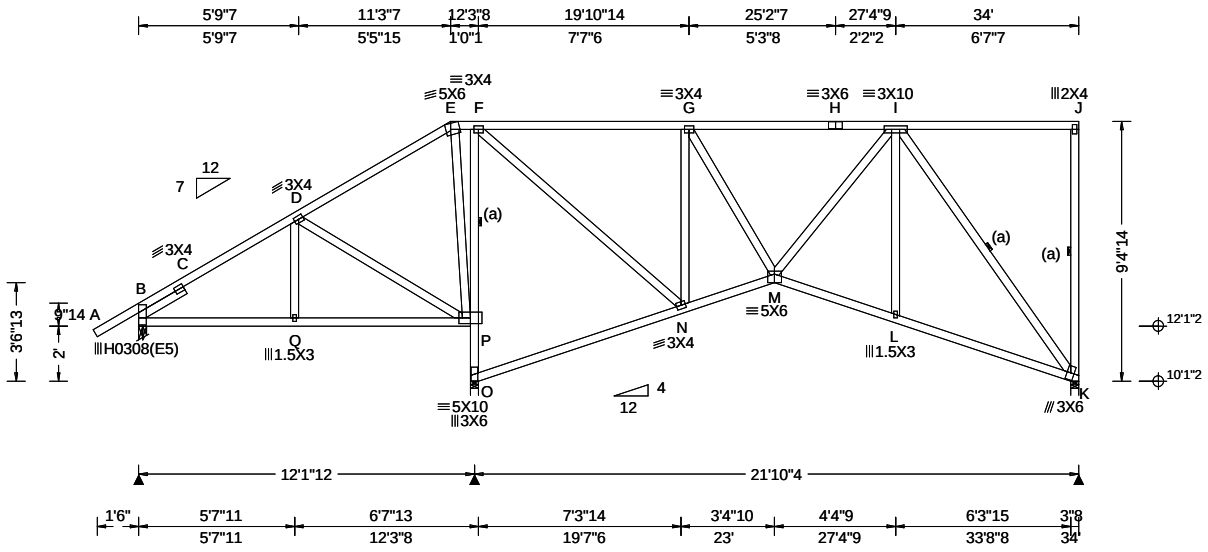
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25188 COMN FROM: RJL	Ply: 1 Qty: 6	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** A5 34' Common	Cust: R 857 JRef:1Y808570001 T44 DrwNo: 063.25.1348.44430 SSB / DF 03/04/2025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.050 M 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.066 M 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.023 O - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.032 O - -
NCBCLL: 10.00	Mean Height: 15.73 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.573
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.492
Spacing: 24.0 "	MWFRS Parallel Dist: > 2h	Rep Fac: No	Max Web CSI: 0.987
	C&C Dist a: 3.40 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 24.02.00C.1213.15
	GCpi: 0.18	HS, WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	560	/-	/-	/266	/1	/190
O	1337	/-	/-	/716	/222	/-
K	830	/-	/-	/385	/173	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
O Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, O, & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	39	- 589	G - H	429	- 783
C - D	0	- 519	H - I	429	- 783
F - G	353	- 643			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Q	411	- 154	M - L	634	- 367
Q - P	409	- 156	L - K	652	- 365
N - M	703	- 403			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - P	181	- 442	F - N	861	- 472
P - O	756	- 1271	N - G	521	- 639
P - F	701	- 969	I - K	580	- 1017

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Wind

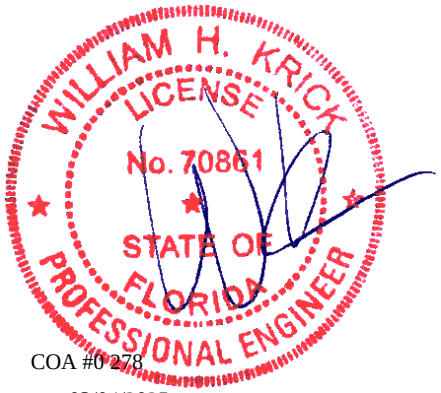
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.



COA #0 278
03/04/2025
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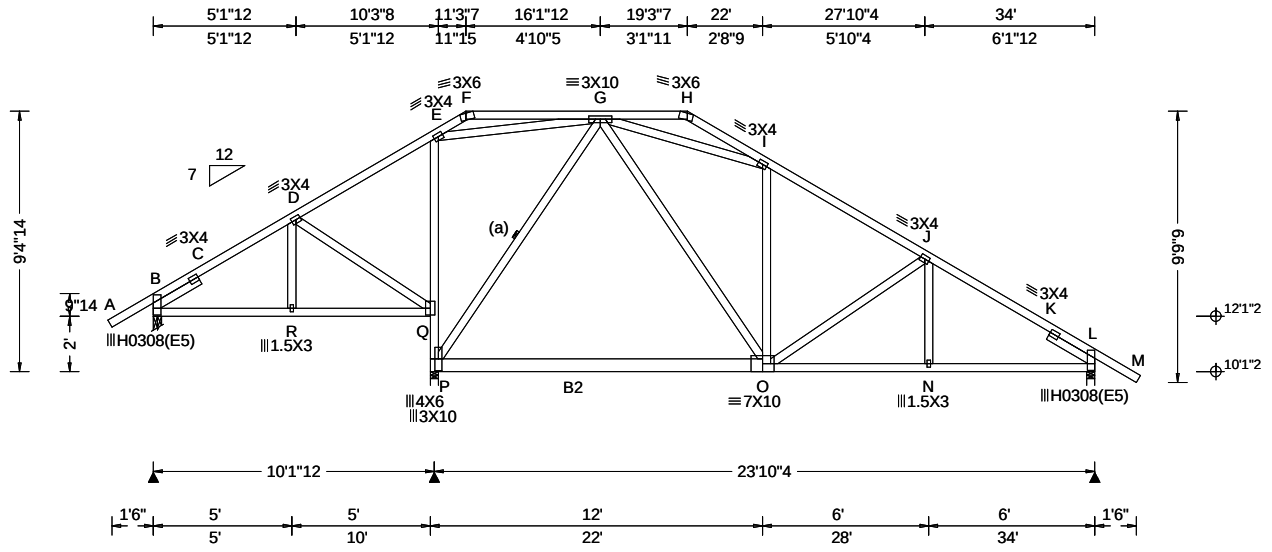
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25090 COMN FROM: RJL	Ply: 1 Qty: 5	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** B1 34' Common	Cust: R 857 JRef:1Y808570001 T4 DrwNo: 063.25.1348.45930 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.40 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.039	I	999	360
VERT(CL): 0.068	I	999	240
HORZ(LL): -0.012	J	-	-
HORZ(TL): 0.021	J	-	-

Creep Factor: 2.0
Max TC CSI: 0.279
Max BC CSI: 0.771
Max Web CSI: 0.984

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	493	/-	/-	/205	/112	/226
P	1493	/-	/-	/774	/0	/-
L	1069	/-	/-	/575	/129	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
P Brg Wid = 3.5 Min Req = 1.8 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, P, & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	354	- 470	J - K	490	- 1375
C - D	328	- 416	K - L	516	- 1426
I - J	486	- 1098			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	477	- 129	N - L	1131	- 309
O - N	1129	- 309			

Maximum Web Forces Per Ply (lbs)

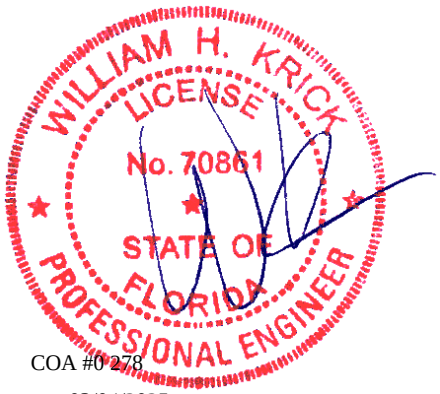
Webs	Tens.	Comp.	Webs	Tens.	Comp.
Q - P	145	- 558	G - O	736	- 92
P - G	263	- 846	G - I	382	- 717

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing
(a) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.



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03/04/2025
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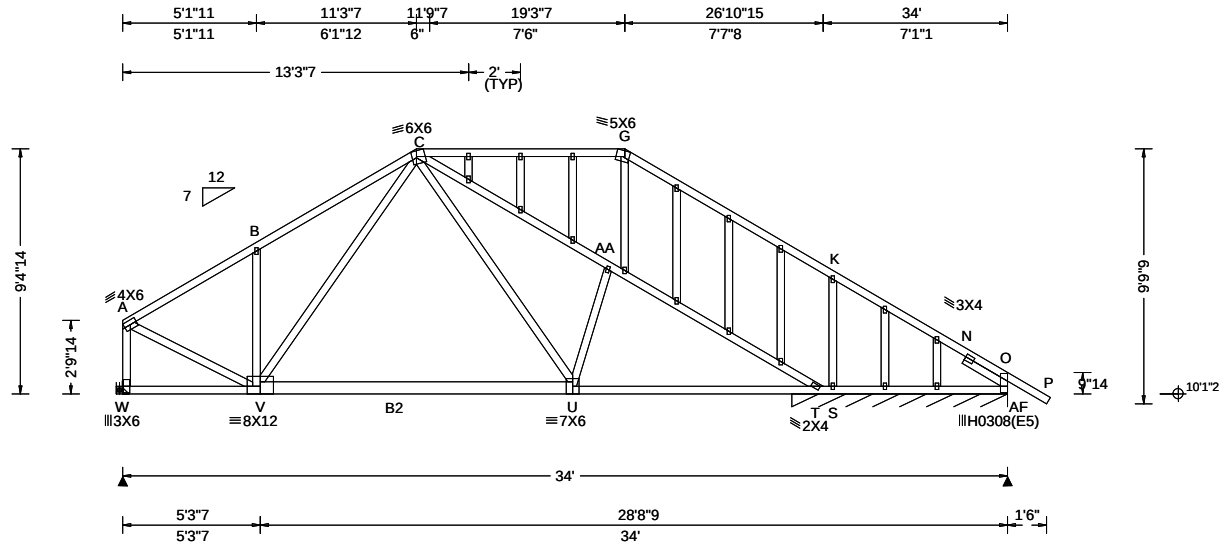
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25098 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** B2-SG 34' Gable		Cust: R 857 JRef:1Y808570001 T21 DrwNo: 063.25.1348.50663 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.132 E 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.281 E 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.056 AD - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.119 AD - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.643
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.734
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.723
	C&C Dist a: 3.40 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 12.11 ft	Plate Type(s):	
	GCpi: 0.18	WAVE, HS	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
W	1759	/-	/-	/709	/303	/338
AF	* 297	/-	/-	/122	/65	/-
S		/-208				

Wind reactions based on MWFRS
W Brg Wid = - Min Req = -
AF Brg Wid = 99.5 Min Req = -
Bearing T is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	570	- 1815	G - K	461	- 983
B - C	771	- 1853	K - N	197	- 938
C - G	468	- 737	N - O	147	- 1013

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
V - U	1437	- 217	T - S	744	0
U - T	1992	- 398	S - O	758	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - W	555	- 1740	C -AA	499	- 1440
A - V	1628	- 442	U -AA	280	- 522
B - V	333	- 633	AA- T	538	- 1557
C - U	736	- 182			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
S - K	248	- 472

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

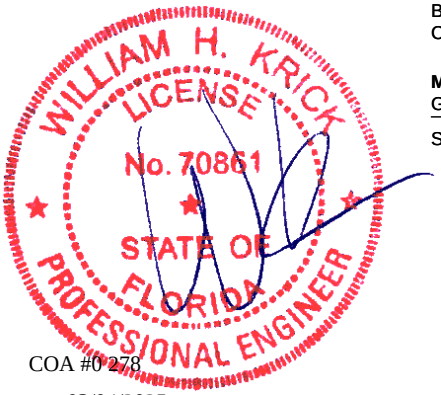
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/265.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0278
03/04/2025
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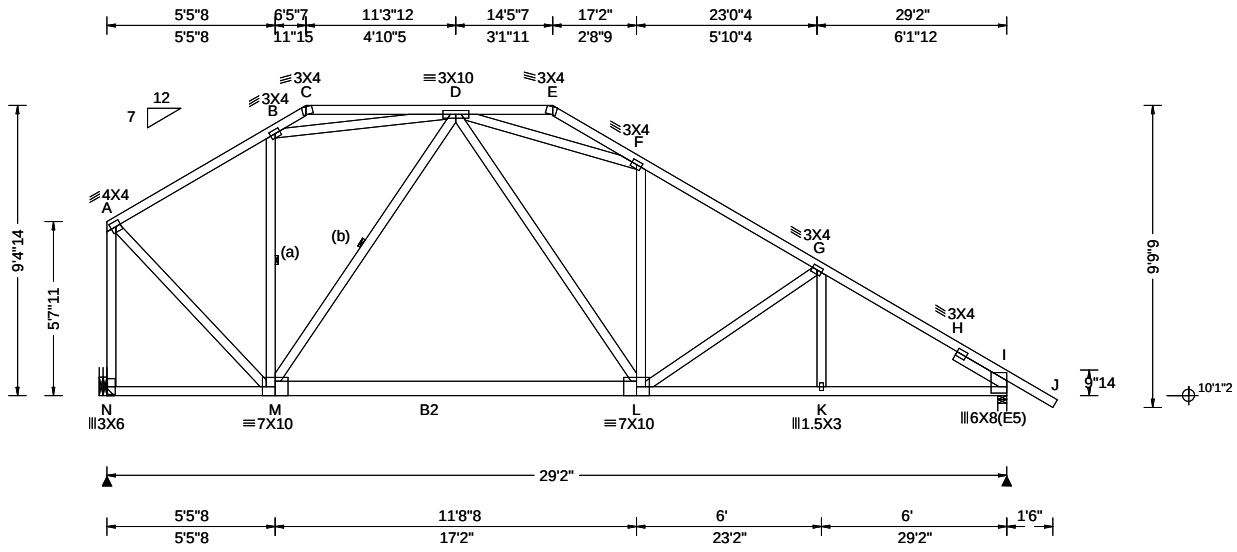
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25286 COMN FROM: RJL	Ply: 1 Qty: 4	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** C1 29'2" Common	Cust: R 857 JRef:1Y808570001 T34 DrwNo: 063.25.1348.52087 SSB / DF 03/04/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.061	L	999	360
VERT(CL): 0.106	L	999	240
HORZ(LL): 0.024	I	-	-
HORZ(TL): 0.042	I	-	-

Creep Factor: 2.0
Max TC CSI: 0.324
Max BC CSI: 0.766
Max Web CSI: 0.820

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
N	1271	/-	/-	/531	/90	/240
I	1312	/-	/-	/625	/97	/-

Wind reactions based on MWFRS
N Brg Wid = - Min Req = -
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearing I is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	353	- 913	G - H	529	- 1803
F - G	529	- 1545	H - I	559	- 1855

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	961	- 173	K - I	1491	- 343
L - K	1489	- 344			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - N	471	- 1264	M - D	262	- 419
A - M	1022	- 332	D - L	555	- 84
B - D	201	- 555	D - F	400	- 1095

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

(b) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

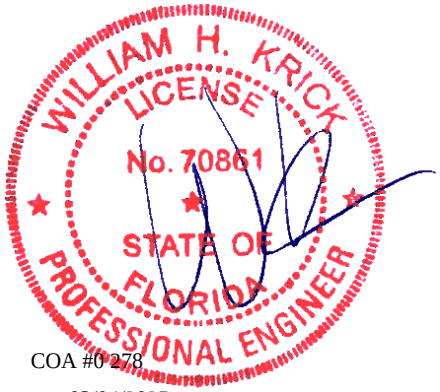
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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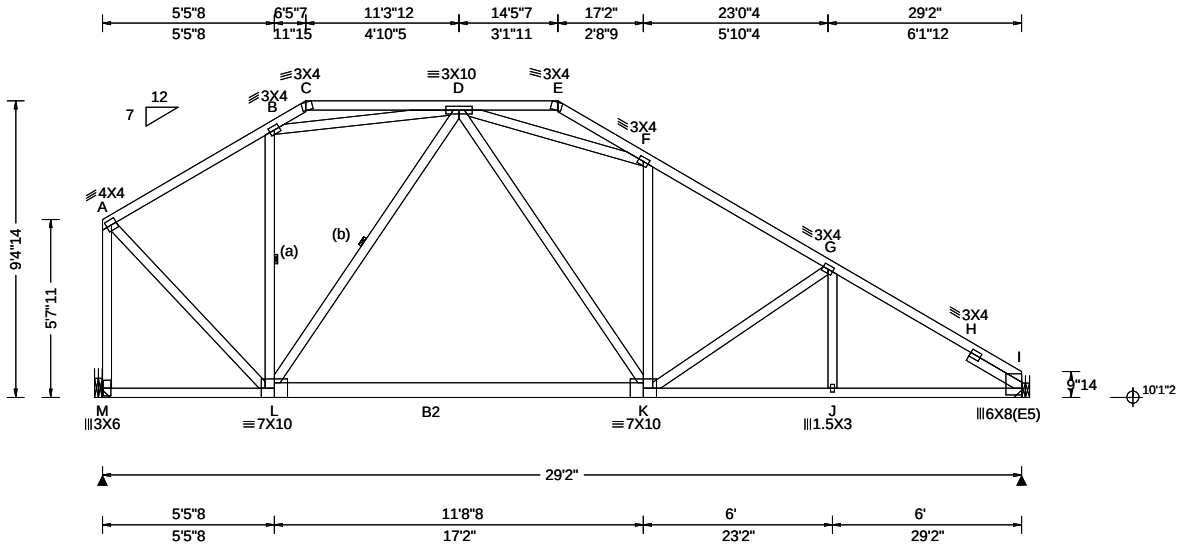
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25288 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** C1a 29'2" Common	Cust: R 857 JRef:1Y808570001 T32 DrwNo: 063.25.1348.53310 SSB / DF 03/04/2025	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.060 K 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.104 K 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.023 I - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.040 I - -
NCBCLL: 10.00	Mean Height: 15.21 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.345
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.761
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: No	Max Web CSI: 0.821
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
M	1274	/-	/-	/532	/93	/222
I	1214	/-	/-	/631	/82	/-

Wind reactions based on MWFRS
M Brg Wid = - Min Req = -
I Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	356	- 915	G - H	542	- 1820
F - G	535	- 1553	H - I	540	- 1872

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	965	- 210	J - I	1511	- 388
K - J	1509	- 389			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - M	474	- 1266	L - D	264	- 422
A - L	1024	- 336	D - K	559	- 85
B - D	203	- 556	D - F	403	- 1096

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

(b) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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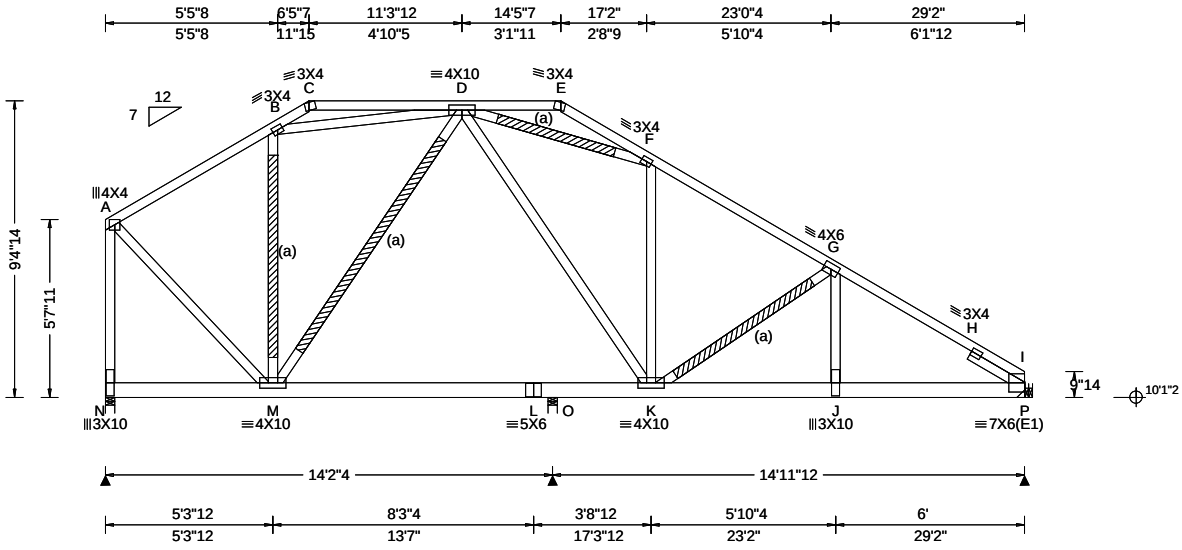
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25162 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** C2 29'2" Common Girder	Cust: R 857 JRef:1Y808570001 T23 DrwNo: 063.25.1351.08597 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.21 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.117	J	999	360
VERT(CL): 0.153	J	999	240
HORZ(LL): 0.032	C	-	-
HORZ(TL): 0.044	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.511
Max BC CSI: 0.859
Max Web CSI: 0.880

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
N	1431 /-	/-	/-	/402	/-
O	835 /-	/0	/-	/77	/0
P	2406 /-	/-	/-	/724	/-

Wind reactions based on MWFRS
N Brg Wid = 3.5 Min Req = 1.7 (Truss)
O Brg Wid = 3.5 Min Req = 1.5 (Truss)
P Brg Wid = - Min Req = -
Bearing O is a rigid surface.
Bearing N Fcperp = 565psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	288	- 994	G - H	1114	- 3601
F - G	767	- 2254	H - I	1138	- 3650

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
M - L	1235	- 407	K - J	3024	- 933
L - K	2470	- 814	J - I	3058	- 940

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - N	409	- 1388	D - K	1229	- 394
A - M	1130	- 313	F - K	428	- 94
M - D	331	- 778	K - G	373	- 1408
B - D	153	- 613	G - J	1198	- 246
D - F	564	- 1728			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing
(a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	56 plf at	0.00 to	56 plf at	14.45
TC: From	28 plf at	14.45 to	28 plf at	19.10
TC: From	56 plf at	19.10 to	56 plf at	29.17
BC: From	20 plf at	0.00 to	20 plf at	13.58
BC: From	10 plf at	13.58 to	10 plf at	29.17
PLB: From	40 plf at	8.00 to	40 plf at	14.62

BC: 353 lb Conc. Load at 15.10,17.10,19.10,21.10 23.10,25.10,27.10

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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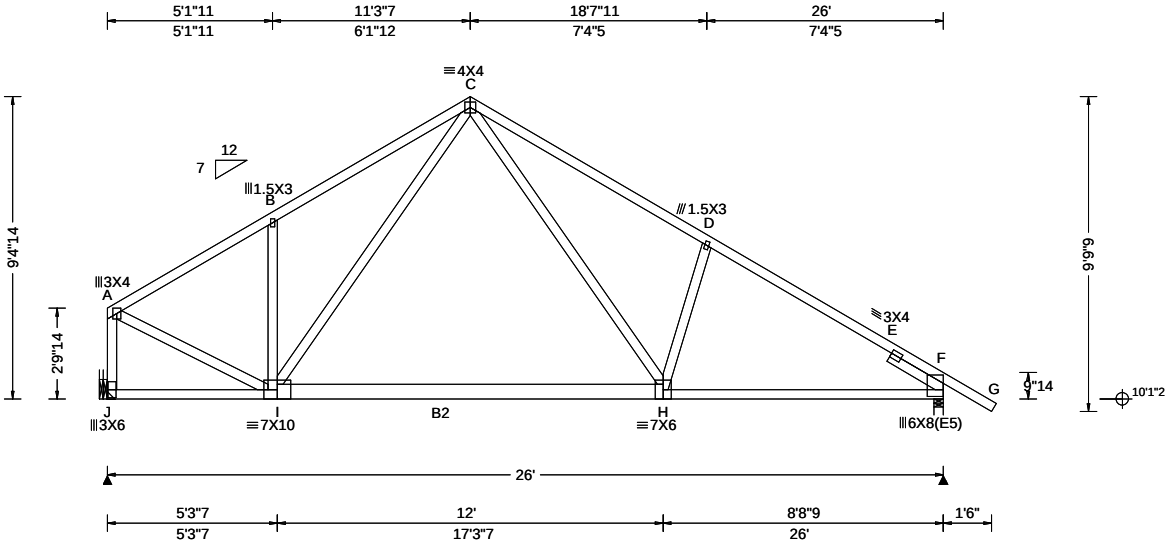
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25265 SPEC FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** D1 26' Special	Cust: R 857 JRef:1Y808570001 T10 DrwNo: 063.25.1348.58910 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.058	H	999	360
VERT(CL): 0.099	H	999	240
HORZ(LL): 0.020	B	-	-
HORZ(TL): 0.034	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.550
Max BC CSI: 0.718
Max Web CSI: 0.410

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J	1141	/-	/-	/497	/35	/240
F	1207	/-	/-	/535	/41	/-

Wind reactions based on MWFRS
J Brg Wid = - Min Req = -
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearing F is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	254	- 1191	D - E	338	- 1650
B - C	397	- 1201	E - F	365	- 1814
C - D	438	- 1563			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
I - H	811	0	H - F	1355	- 170

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - J	257	- 1140	C - H	810	- 174
A - I	1075	- 166			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Hangers / Ties

(J) Hanger Support Required, by others

Loading

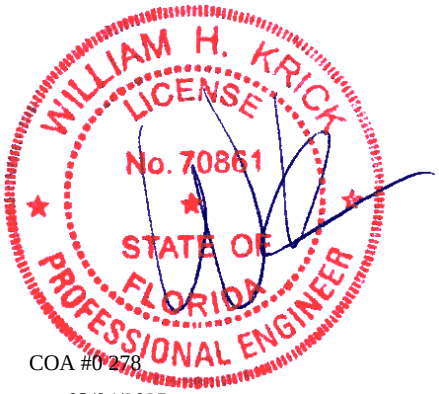
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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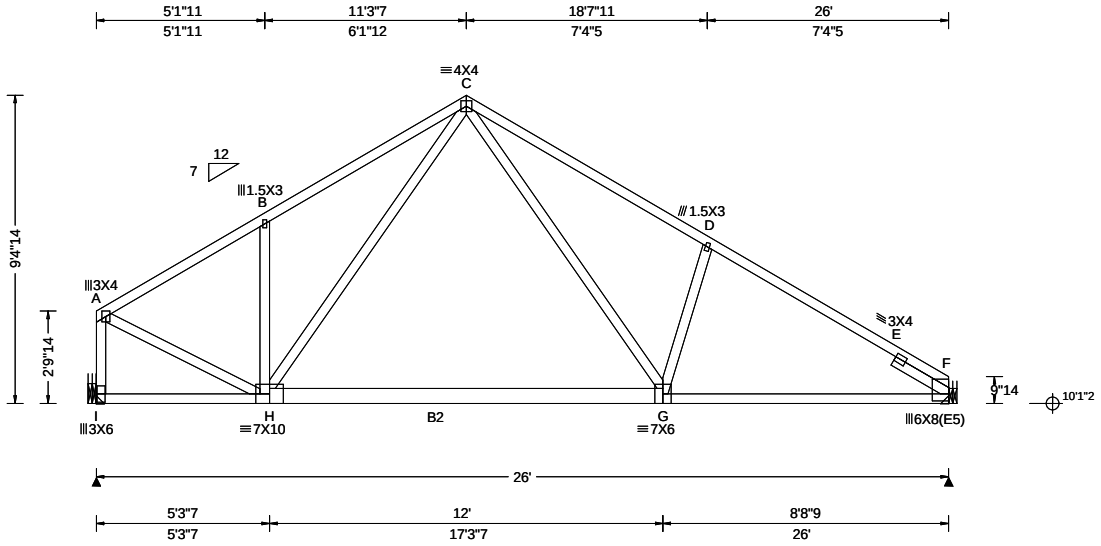
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25267 SPEC FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** D1a 26' Special	Cust: R 857 JRef:1Y808570001 T9 DrwNo: 063.25.1349.00410 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.21 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.056	G	999	360
VERT(CL): 0.096	G	999	240
HORZ(LL): 0.019	B	-	-
HORZ(TL): 0.040	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.584
Max BC CSI: 0.716
Max Web CSI: 0.414

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	1144	/-	/-	/497	/202	/222
F	1108	/-	/-	/541	/193	/-

Wind reactions based on MWFRS
I Brg Wid = - Min Req = -
F Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	257	- 1194	D - E	356	- 1663
B - C	401	- 1205	E - F	455	- 1858
C - D	459	- 1579			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
H - G	814	- 34	G - F	1371	- 213

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - I	260	- 1143	C - G	825	- 184
A - H	1078	- 169			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Hangers / Ties

(J) Hanger Support Required, by others

Loading

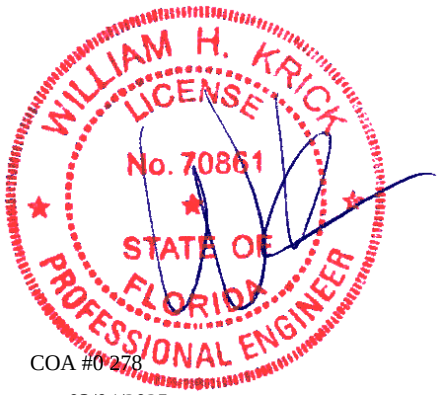
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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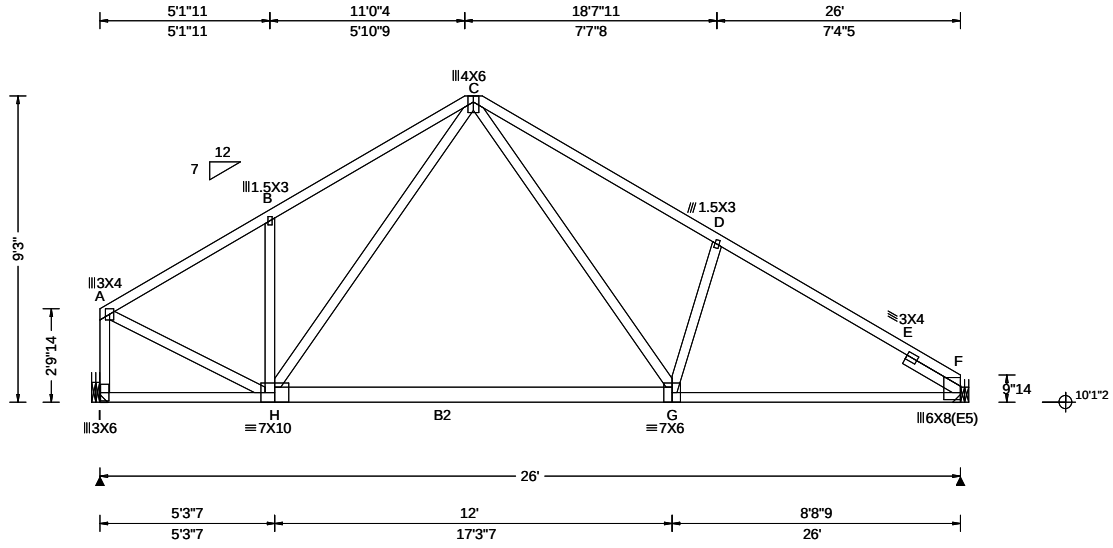
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25269 SPEC FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** D1b 26' Special	Cust: R 857 JRef:1Y808570001 T8 DrwNo: 063.25.1349.01977 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.13 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.056	G	999	360
VERT(CL): 0.096	G	999	240
HORZ(LL): 0.019	B	-	-
HORZ(TL): 0.041	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.584
Max BC CSI: 0.716
Max Web CSI: 0.414

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	1144	/-	/-	/493	/197	/219
F	1108	/-	/-	/538	/189	/-

Wind reactions based on MWFRS
I Brg Wid = - Min Req = -
F Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	251	- 1194	D - E	349	- 1663
B - C	393	- 1205	E - F	448	- 1858
C - D	450	- 1579			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
H - G	814	- 28	G - F	1371	- 208

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - I	255	- 1143	C - G	825	- 184
A - H	1078	- 165			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Hangers / Ties

(J) Hanger Support Required, by others

Loading

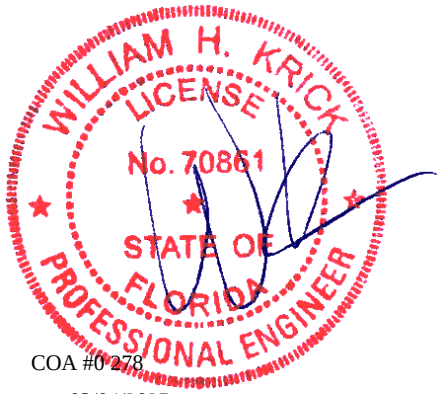
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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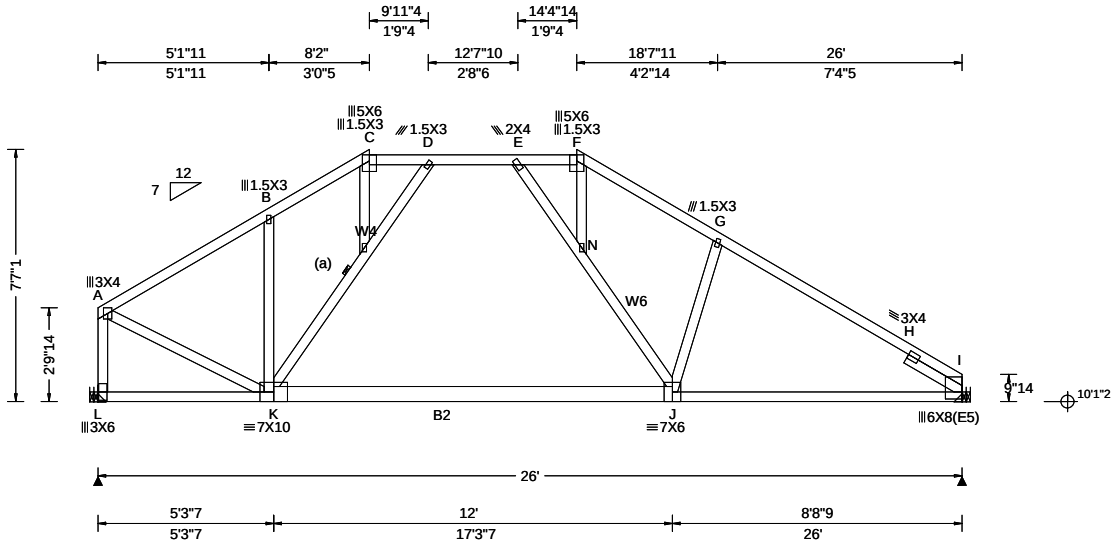
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25060 SPEC FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** D2 26' Special		Cust: R 857 JRef:1Y808570001 T7 DrwNo: 063.25.1349.03593 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.137	E	999	360
VERT(CL): 0.266	E	999	240
HORZ(LL): -0.052	G	-	-
HORZ(TL): 0.098	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.764
Max BC CSI: 0.771
Max Web CSI: 0.553

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
L	1144	/-	/-	/501	/205	/174
I	1108	/-	/-	/546	/196	/-

Wind reactions based on MWFRS
L Brg Wid = - Min Req = -
I Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	390	-1174	E - F	550	-1265
B - C	471	-1080	F - G	590	-1510
C - D	447	-939	G - H	520	-1632
D - E	522	-975	H - I	617	-1861

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	975	-268	J - I	1333	-356

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - L	389	-1142	E - N	544	-88
A - K	1036	-291	N - J	659	-94

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1; B2 2x6 SP #1;
Webs: 2x4 SP #3; W4,W6 2x4 SP #1;
Rt Slider: 2x4 SP #3; block length = 1.958'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

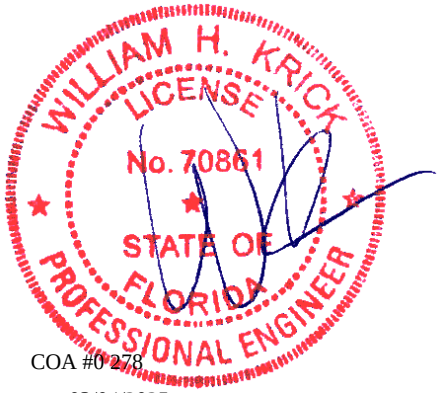
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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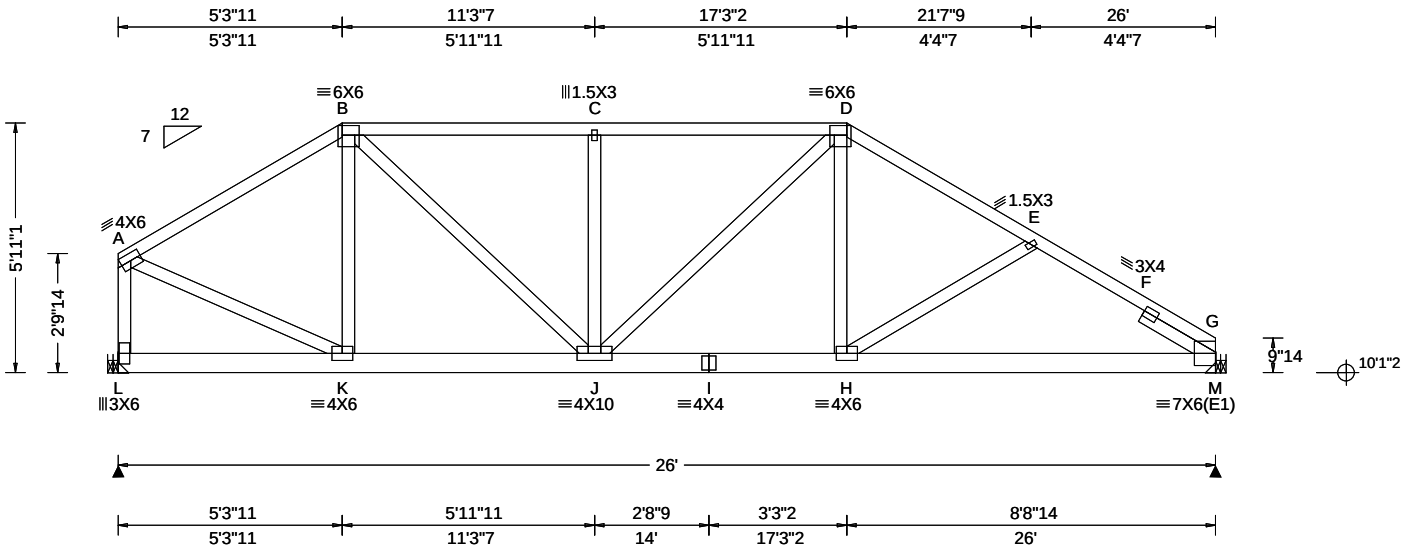
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25073 HIPS FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** D3 26' Stepdown Hip Girder	Cust: R 857 JRef:1Y808570001 T24 DrwNo: 063.25.1349.05163 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.084	C	999	360
VERT(CL): 0.156	C	999	240
HORZ(LL): 0.023	B	-	-
HORZ(TL): 0.043	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.665
Max BC CSI: 0.933
Max Web CSI: 0.712

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
L	2139	/-	/-	/-	/383	/-
M	1910	/-	/-	/-	/430	/-

Wind reactions based on MWFRS
L Brg Wid = - Min Req = -
M Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	195	-2003	D - E	392	-2728
B - C	99	-2552	E - F	443	-2779
C - D	99	-2552	F - G	676	-2803

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	1709	-154	I - H	2339	-322
J - I	2339	-322	H - G	2373	-390

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - L	191	-1822	C - J	0	-711
A - K	1869	-163	D - H	832	-366
B - J	1148	0			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Special Loads

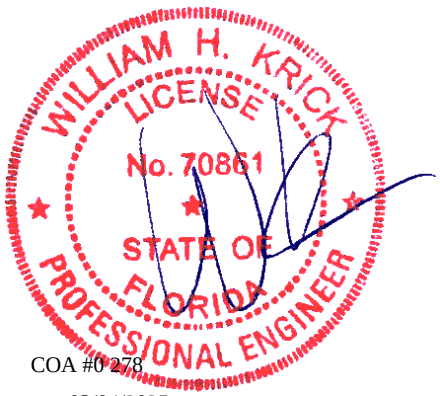
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 28 plf at 0.00 to 28 plf at 26.00
BC: From 10 plf at 0.00 to 10 plf at 26.00
TC: 151 lb Conc. Load at 6.31, 8.31,10.31,11.94 13.94,15.94
BC: 211 lb Conc. Load at 0.40, 2.40, 4.40,17.94 19.94,21.94,23.94
BC: 113 lb Conc. Load at 6.31, 8.31,10.31,11.94 13.94,15.94

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads and reactions based on MWFRS.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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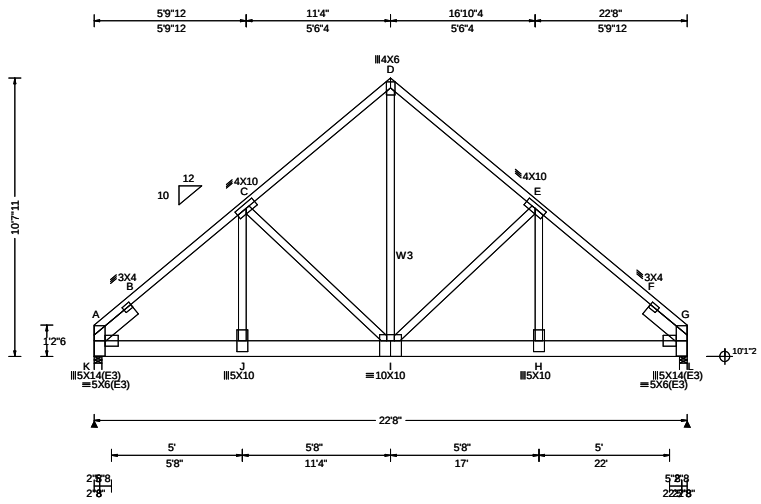
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25132 COMN FROM: RJL	Ply: 2 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** E1 22'8" Common Girder		Cust: R 857 JRef:1Y808570001 T33 DrwNo: 063.25.1351.13407 SSB / DF 03/04/2025	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 16.01 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.079	J	999	360
VERT(CL): 0.146	J	999	240
HORZ(LL): -0.035	B	-	-
HORZ(TL): 0.064	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.488
Max BC CSI: 0.357
Max Web CSI: 0.696

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
K	6264	/-	/-	/-	/616	/-
L	3026	/-	/-	/-	/350	/-

Wind reactions based on MWFRS
K Brg Wid = 3.5 Min Req = 3.2 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings K & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	355	- 3418	D - E	220	- 1979
B - C	342	- 3385	E - F	225	- 1952
C - D	219	- 1975	F - G	238	- 1983

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - J	2545	- 249	I - H	1437	- 160
J - I	2489	- 246	H - G	1441	- 161

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - C	1826	- 109	D - I	2278	- 175
C - I	134	- 1398			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W3 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.958'
Rt Slider: 2x6 SP #1; block length = 1.958'

Nailnote
Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 58 plf at 0.00 to 58 plf at 22.67
BC: From 10 plf at 0.00 to 10 plf at 11.33
BC: From 20 plf at 11.33 to 20 plf at 22.67
BC: 1271 lb Conc. Load at 1.44, 3.44, 5.44, 7.44
BC: 1274 lb Conc. Load at 9.44,11.44

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Blocking
Apply additional nailing over the following bearings with fasteners at 9" oc perpendicular to grain and 4" oc parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 5.50" if used)
Bearing 2 located at 22.4' (blocking >= 5.50" if used)

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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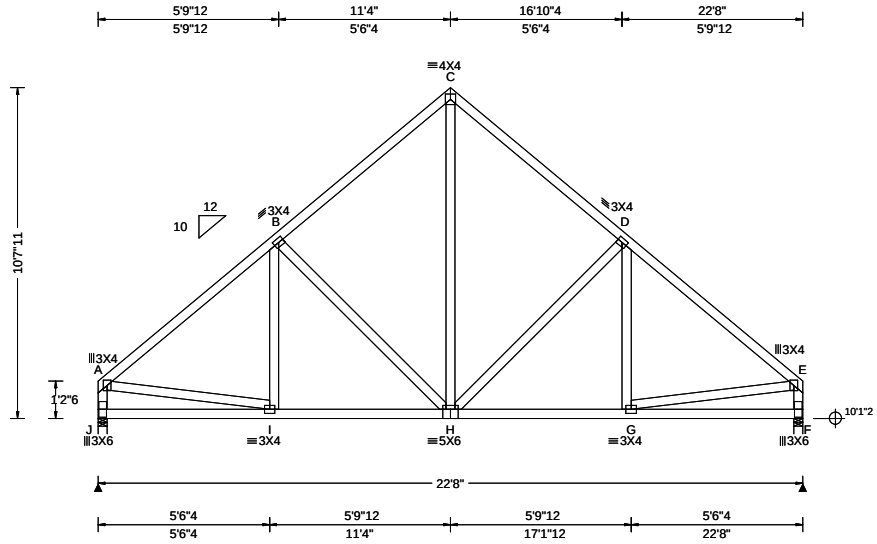
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25123 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** E2 22'8" Common		Cust: R 857 JRef:1Y808570001 T27 DrwNo: 063.25.1351.14790 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 16.01 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.023	H	999	360
VERT(CL): 0.045	H	999	240
HORZ(LL): 0.009	B	-	-
HORZ(TL): 0.018	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.328
Max BC CSI: 0.280
Max Web CSI: 0.463

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J	887	/-	/-	/477	/172	/285
F	887	/-	/-	/477	/172	/-

Wind reactions based on MWFRS
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings J & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	229	- 1020	C - D	302	- 769
B - C	302	- 769	D - E	229	- 1020

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
I - H	715	- 135	H - G	715	- 90

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - J	207	- 840	G - E	678	- 64
A - I	678	- 64	E - F	207	- 840
C - H	537	- 227			

Lumber

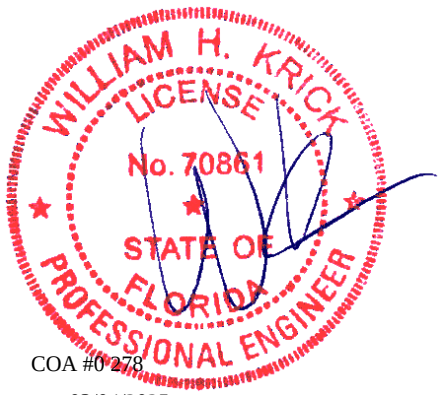
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
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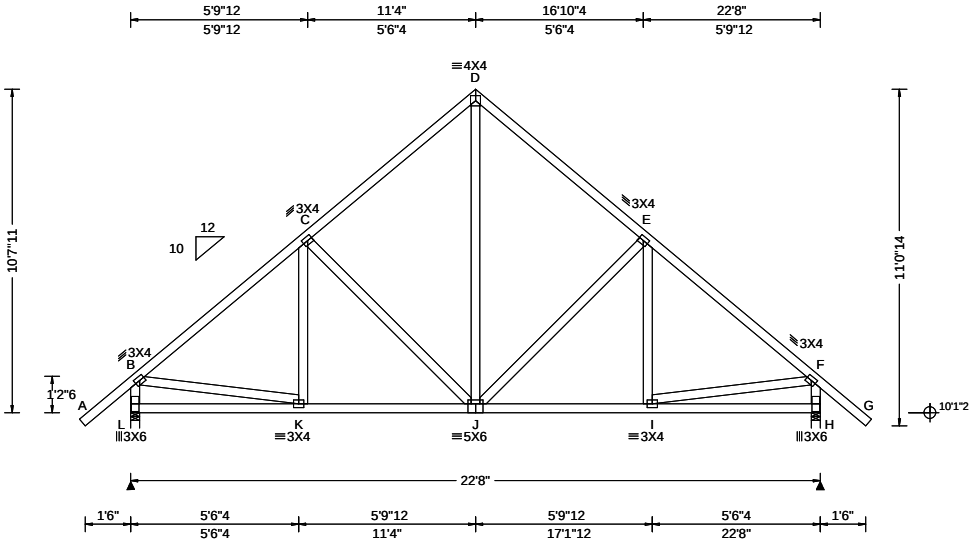
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25118 COMN FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** E2a 22'8" Common	Cust: R 857 JRef:1Y808570001 T13 DrwNo: 063.25.1351.16047 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.31 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.023	J	999	360
VERT(CL): 0.045	J	999	240
HORZ(LL): 0.009	C	-	-
HORZ(TL): 0.018	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.285
Max BC CSI: 0.279
Max Web CSI: 0.628

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
L	988	/-	/-	/482	/194	/291
H	988	/-	/-	/482	/194	/-

Wind reactions based on MWFRS
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings L & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	342	- 1007	D - E	408	- 756
C - D	408	- 756	E - F	342	- 1007

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - J	698	- 110	J - I	698	- 85

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - L	406	- 941	I - F	676	- 67
B - K	676	- 67	F - H	406	- 941
D - J	522	- 308			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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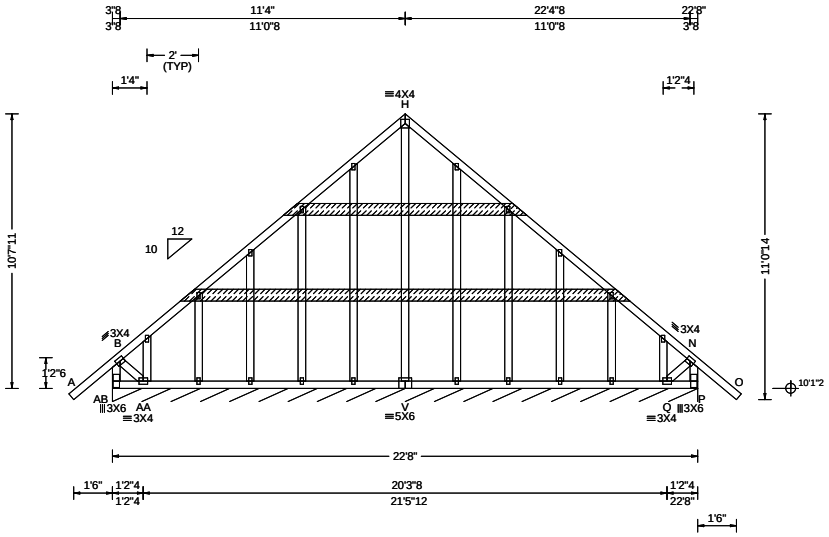
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25121 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** E3-G 22'8" Gable	Cust: R 857 JRef:1Y808570001 T25 DrwNo: 063.25.1351.17787 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**	**▲Maximum Reactions (lbs), or *=PLF**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.31 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 I 999 360 VERT(CL): 0.004 I 999 240 HORZ(LL): -0.001 C - - HORZ(TL): 0.008 G - - Creep Factor: 2.0 Max TC CSI: 0.269 Max BC CSI: 0.038 Max Web CSI: 0.534 VIEW Ver: 24.02.00C.1213.15	Gravity / Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P* 115 /- /- /44 /21 /14 Wind reactions based on MWFRS P Brg Wid = 271 Min Req = - Bearing AB is a rigid surface. Members not listed have forces less than 375# **Maximum Web Forces Per Ply (lbs)** Webs Tens.Comp. Webs Tens. Comp. B -AA 410 -148 Q - N 410 -148

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

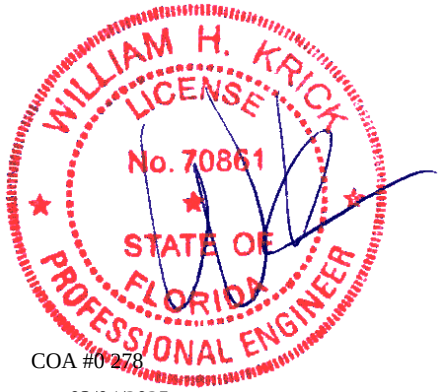
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/662.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278

03/04/2025

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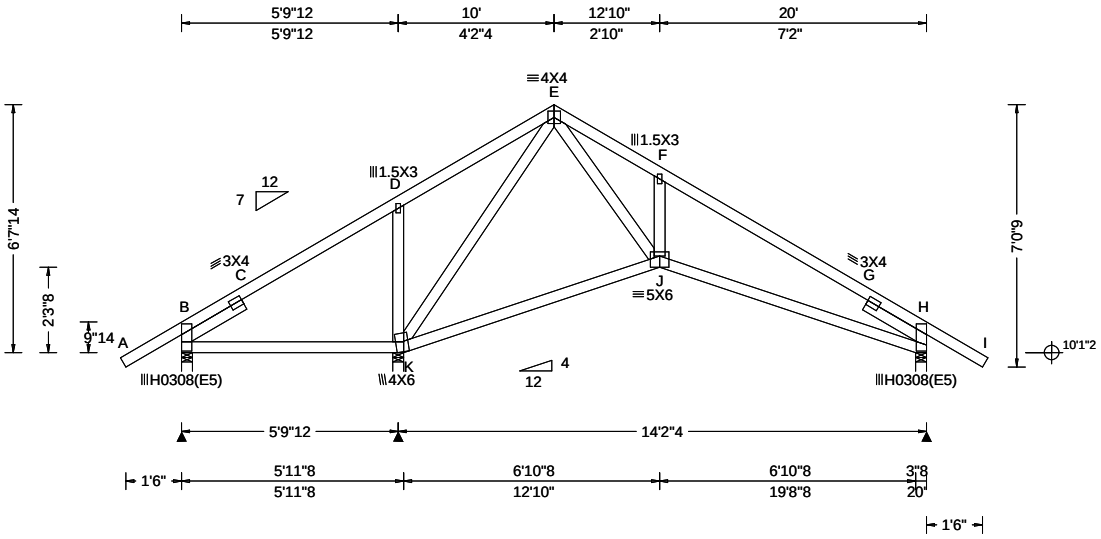
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25134 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** G1 20' Common		Cust: R 857 JRef:1Y808570001 T36 DrwNo: 063.25.1351.19313 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.042	G	999	360
VERT(CL): 0.081	G	999	240
HORZ(LL): 0.025	C	-	-
HORZ(TL): 0.049	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.335
Max BC CSI: 0.390
Max Web CSI: 0.771

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	273	/-32	/-	/40	/163	/150
K	960	/-	/-	/578	/24	/-
H	582	/-	/-	/276	/174	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
K Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B, K, & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	413	0	F - G	430	-882
E - F	609	-866	G - H	458	-975

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
J - H	757	-231

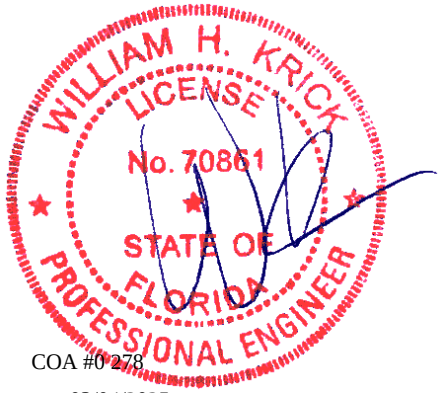
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
K - E	0	-700	E - J	893	-395

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
Shim all supports to solid bearing.



COA #0 278
03/04/2025
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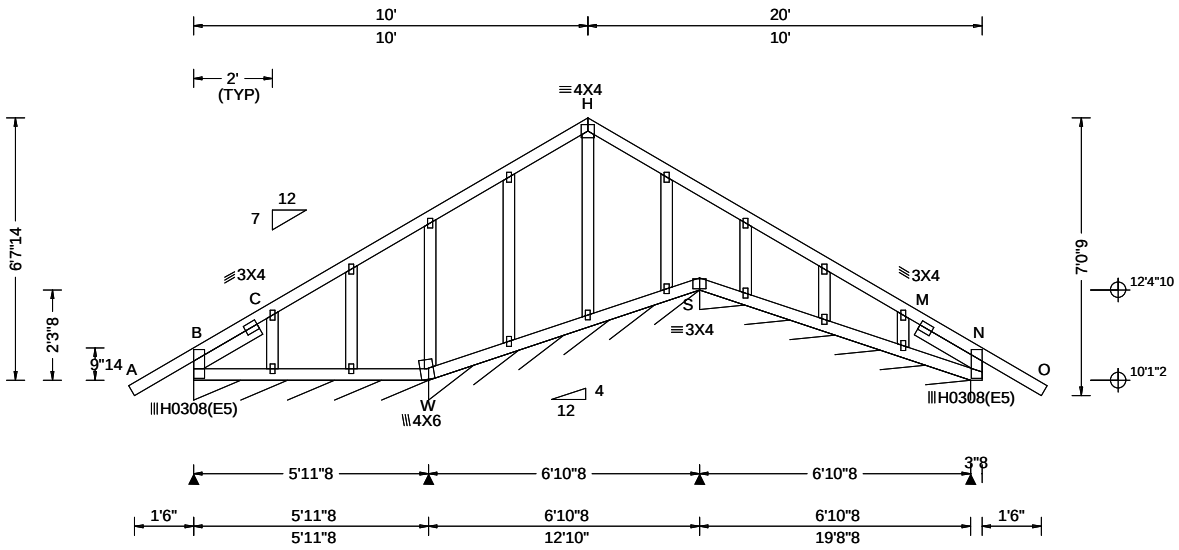
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25138 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** G2-G 20' Gable		Cust: R 857 JRef:1Y808570001 T38 DrwNo: 063.25.1351.20967 SSB / DF 03/04/2025	



Loading Criteria (psf)			
TCLL:	20.00		
TCDL:	7.00		
BCLL:	0.00		
BCDL:	10.00		
Des Ld:	37.00		
NCBCLL:	10.00		
Soffit:	2.00		
Load Duration:	1.25		
Spacing:	24.0 "		

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
HS, WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	N	999	360
VERT(CL): 0.001	N	999	240
HORZ(LL): 0.001	M	-	-
HORZ(TL): 0.004	N	-	-

Creep Factor: 2.0
Max TC CSI: 0.194
Max BC CSI: 0.028
Max Web CSI: 0.883

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	119	/-	/-	/51	/42	/28
W*	87	/-	/-	/48	/-	/-
S*	98	/-	/-	/53	/37	/-

Wind reactions based on MWFRS
B Brg Wid = 71.5 Min Req = -
W Brg Wid = 82.5 Min Req = -
S Brg Wid = 82.5 Min Req = -
Bearings B, W, & S are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.958'
Rt Slider: 2x4 SP #3; block length = 1.958'

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

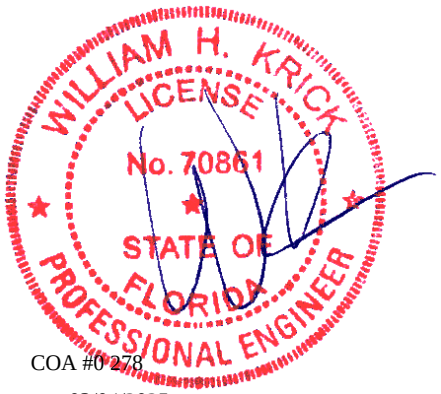
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/225.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Shim all supports to solid bearing.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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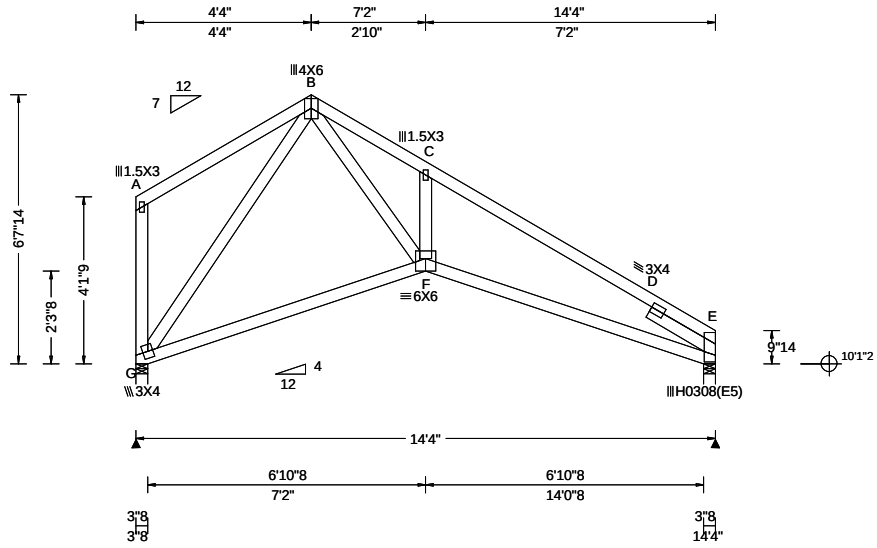
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25136 COMN FROM: RJL	Ply: 1 Qty: 5	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** G3 14'4" Common		Cust: R 857 JRef:1Y808570001 T37 DrwNo: 063.25.1351.22303 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.046	D	999	360
VERT(CL): 0.089	D	999	240
HORZ(LL): 0.029	E	-	-
HORZ(TL): 0.064	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.375
Max BC CSI: 0.435
Max Web CSI: 0.617

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	554	/-	/-	/293	/125	/150
E	554	/-	/-	/309	/88	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings G & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	391	- 1142	D - E	260	- 1272
C - D	230	- 1162			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
F - E	1008	- 125

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
G - B	108	- 561	B - F	1055	- 288

Lumber

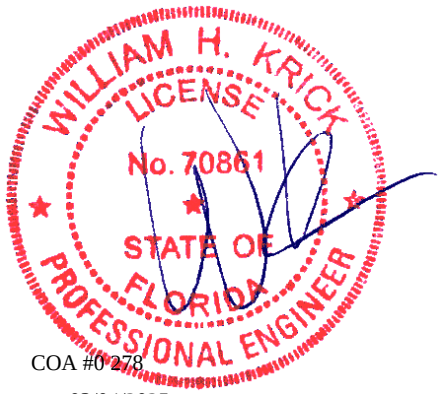
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Slider: 2x4 SP #3; block length = 1.958'

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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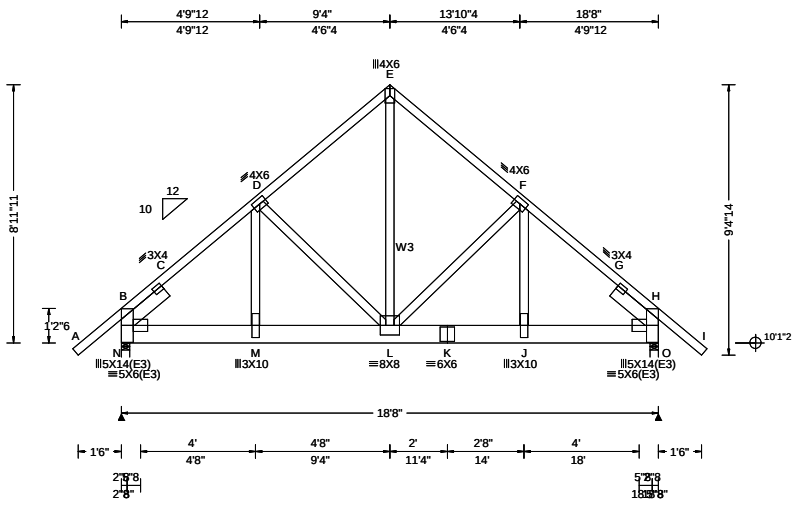
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25104 COMN FROM: RJL	Ply: 2 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** H1 18'8" Common Girder		Cust: R 857 JRef:1Y808570001 T6 DrwNo: 063.25.1351.27827 SSB / DF 03/04/2025	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.072	L	999	360
VERT(CL): 0.132	L	999	240
HORZ(LL): 0.037	G	-	-
HORZ(TL): 0.068	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.496
Max BC CSI: 0.345
Max Web CSI: 0.457

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity	
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
N	4483 /-	/-	/-	/828	/-
O	6665 /-	/-	/-	/1118	/-

Wind reactions based on MWFRS
N Brg Wid = 3.5 Min Req = 2.3 (Truss)
O Brg Wid = 3.5 Min Req = 3.4 (Truss)
Bearings N & O are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	538	-2940	E - F	417	-2285
C - D	525	-2907	F - G	541	-3203
D - E	417	-2285	G - H	553	-3236

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - M	2135	-383	K - J	2340	-392
M - L	2120	-380	J - H	2414	-402
L - K	2340	-392			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
M - D	785	-109	L - F	118	-850
D - L	100	-539	F - J	1200	-131
E - L	2692	-445			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x8 SP SS Dense;
Webs: 2x4 SP #3; W3 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.958'
Rt Slider: 2x6 SP #1; block length = 1.958'

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	58 plf at	-1.69 to	58 plf at	20.35
BC: From	5 plf at	-1.69 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	5.73
BC: From	10 plf at	5.73 to	10 plf at	18.67
BC: From	5 plf at	18.67 to	5 plf at	20.35

BC: 2139 lb Conc. Load at 5.73
BC: 1144 lb Conc. Load at 7.73, 9.73,11.73,13.73
BC: 1141 lb Conc. Load at 15.73
BC: 1759 lb Conc. Load at 17.73

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Blocking

Apply additional nailing over the following bearings with fasteners at 9" oc perpendicular to grain and 4" oc parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 5.50" if used)
Bearing 2 located at 18.4' (blocking >= 5.50" if used)

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data,including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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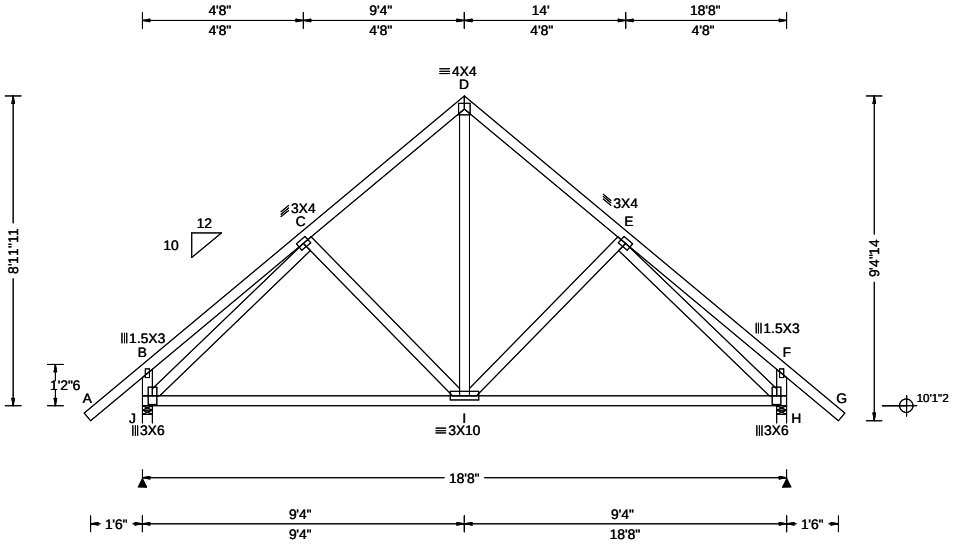
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25064 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** H2 18'8" Common	Cust: R 857 JRef:1Y808570001 T1 DrwNo: 063.25.1351.29207 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.016	I	999	360
VERT(CL): 0.030	I	999	240
HORZ(LL): 0.010	F	-	-
HORZ(TL): 0.019	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.209
Max BC CSI: 0.728
Max Web CSI: 0.563

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

		Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh		/ Rw	/ U	/ RL
J	831	/-	/-		/396	/160	/240
H	831	/-	/-		/396	/160	/-

Wind reactions based on MWFRS
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
H Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings J & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	364	-635	D - E	363	-635

Maximum Bot Chord Forces Per Ply (lbs)

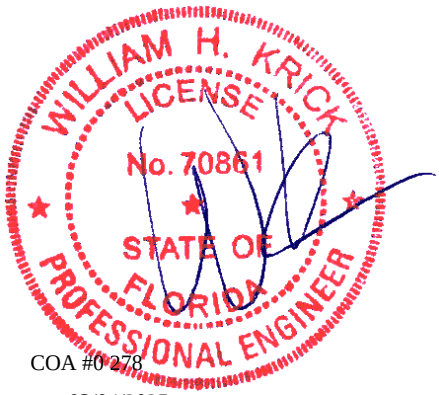
Chords	Tens.	Comp.	Chords	Tens.	Comp.
J - I	522	-106	I - H	522	-92

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - C	147	-683	E - H	147	-683
D - I	478	-256			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0278
Florida Certificate of Product Approval #FL 1999

03/04/2025

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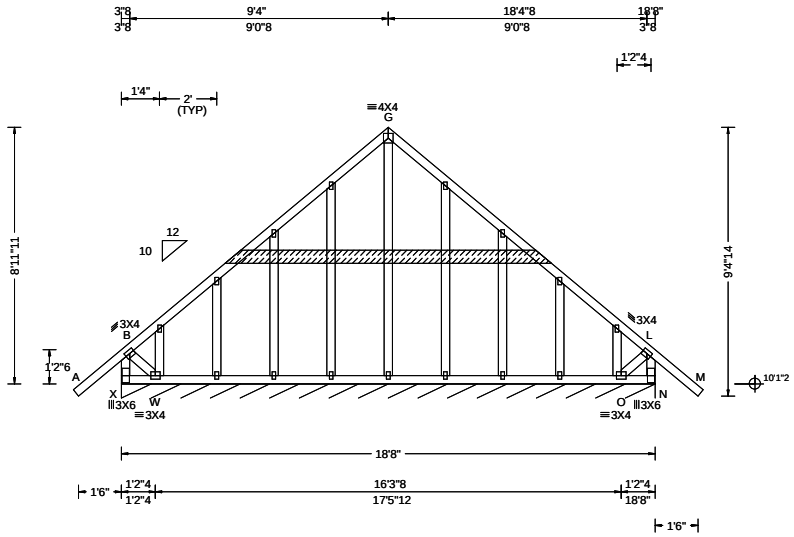
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25093 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** H3-G 18'8" Gable	Cust: R 857 JRef:1Y808570001 T2 DrwNo: 063.25.1351.31080 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 F 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 F 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.008 F - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.303
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.036
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.877
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
N*	113	/-	/-	/44	/21	/14

Wind reactions based on MWFRS
N Brg Wid = 224 Min Req = -
Bearing X is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - W	405	- 94	O - L	413	- 141

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

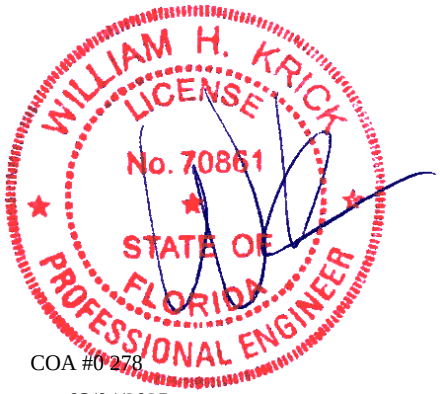
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/671.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278

03/04/2025

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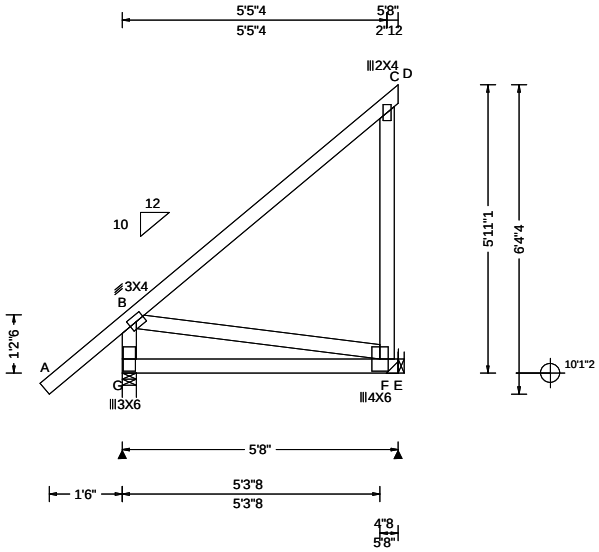
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25071 EJAC FROM: RJL	Ply: 1 Qty: 7	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** JA 5'8" End Jack	Cust: R 857 JRef:1Y808570001 T22 DrwNo: 063.25.1351.32760 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.003	D	310	360
VERT(CL): -0.006	D	999	240
HORZ(LL): -0.002	D	-	-
HORZ(TL): 0.006	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.583
Max BC CSI: 0.301
Max Web CSI: 0.172

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
G	334	/-	/-	/129	/-	/205
E	211	/-	/-	/208	/135	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = - Min Req = -
Bearing G is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
G - F	116	-519

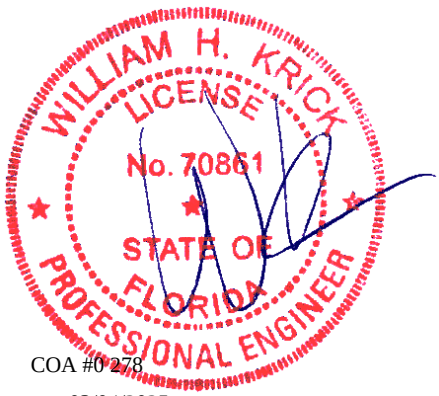
Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - F	525	-117

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
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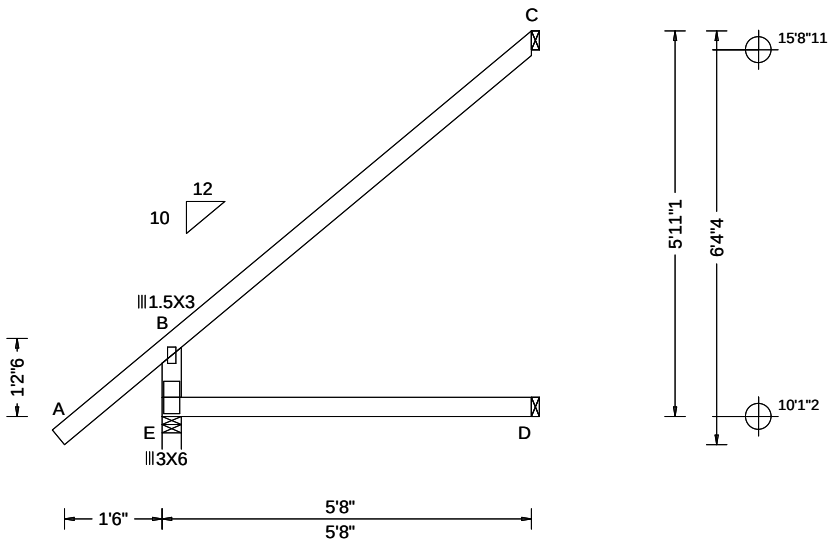
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25069 EJAC FROM: RJL	Ply: 1 Qty: 6	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** JB 5'8" End Jack		Cust: R 857 JRef:1Y808570001 T19 DrwNo: 063.25.1351.33997 SSB / DF 03/04/2025	



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	360
VERT(CL): 0.000	B	999	240
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.003	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.629
Max BC CSI: 0.329
Max Web CSI: 0.204

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	337	/-	/-	/178	/178	/-
D	113	/-	/-	/57	/-	/-
C	151	/-	/-	/95	/-	/205

Wind reactions based on MWFRS
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

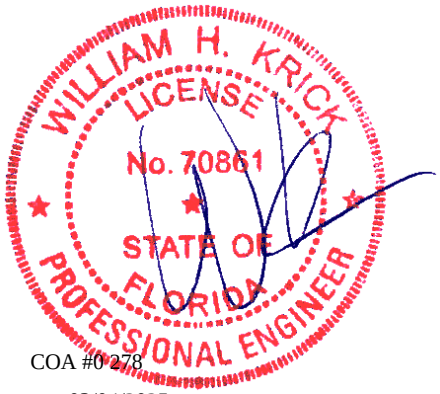
Chords	Tens.	Comp.
B - C	489	- 115

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - E	685	- 280

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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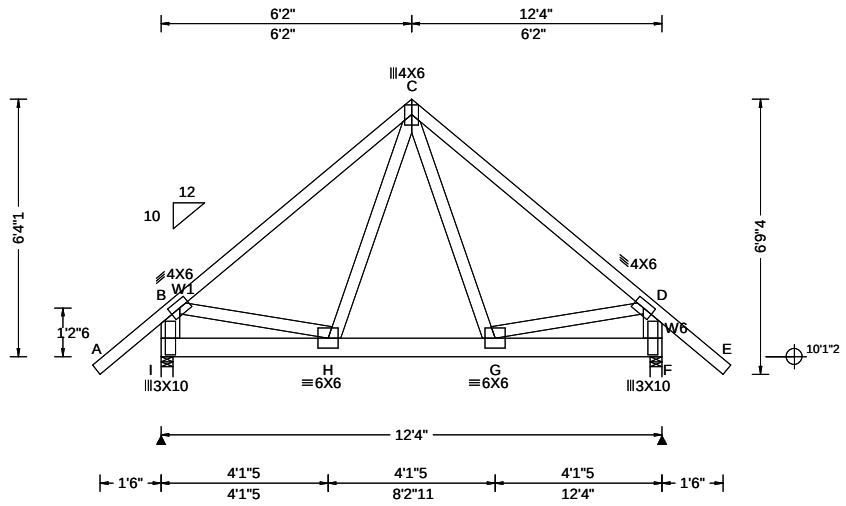
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25076 COMN FROM: RJL	Ply: 2 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** K1 12'4" Common Girder	Cust: R 857 JRef:1Y808570001 T12 DrwNo: 063.25.1351.35710 SSB / DF 03/04/2025	

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.026 G 999 360 VERT(CL): 0.047 G 999 240 HORZ(LL): 0.002 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.191 Max BC CSI: 0.643 Max Web CSI: 0.513 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	2290	/-	/-	/-	/491	/-
F	4045	/-	/-	/-	/811	/-

Wind reactions based on MWFRS
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 2.4 (Truss)
Bearings I & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	298	-1409	C - D	360	-1758

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
H - G	866	-181

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - I	270	-1193	C - G	1346	-235
B - H	1009	-198	G - D	1196	-230
H - C	530	-92	D - F	316	-1450

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W1,W6 2x6 SP #1;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

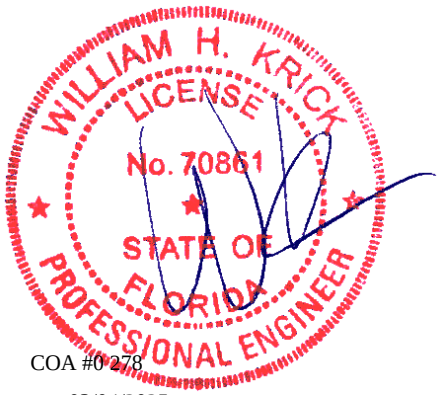
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	58 plf at	-1.69 to	58 plf at	14.02
BC: From	5 plf at	-1.69 to	5 plf at	0.00
BC: From	20 plf at	0.00 to	20 plf at	5.73
BC: From	10 plf at	5.73 to	10 plf at	12.33
BC: From	5 plf at	12.33 to	5 plf at	14.02

BC: 1910 lb Conc. Load at 5.73
BC: 1108 lb Conc. Load at 7.73, 9.73,11.73

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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03/04/2025
Florida Certificate of Product Approval #FL 1999

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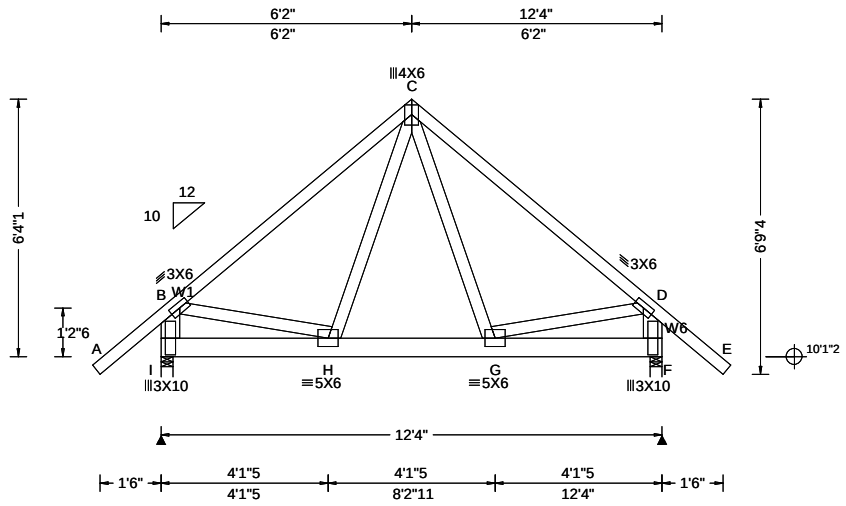
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25165 COMN FROM: RJL	Ply: 2 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** K2 12'4" Common Girder	Cust: R 857 JRef:1Y808570001 T26 DrwNo: 063.25.1351.37510 SSB / DF 03/04/2025	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.018	H	999	360
VERT(CL): 0.034	H	999	240
HORZ(LL): 0.002	D	-	-
HORZ(TL): 0.003	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.163
Max BC CSI: 0.608
Max Web CSI: 0.471

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	3462	/-	/-	/-	/760	/-
F	1294	/-	/-	/-	/321	/-

Wind reactions based on MWFRS
I Brg Wid = 3.5 Min Req = 2.0 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings I & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	324	-1276	C - D	165	-667

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
H - G	532	-135

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - I	285	-1061	G - D	421	-91
B - H	767	-189	D - F	165	-607
H - C	1237	-293			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x6 SP #1;
Webs: 2x4 SP #3; W1,W6 2x6 SP #1;

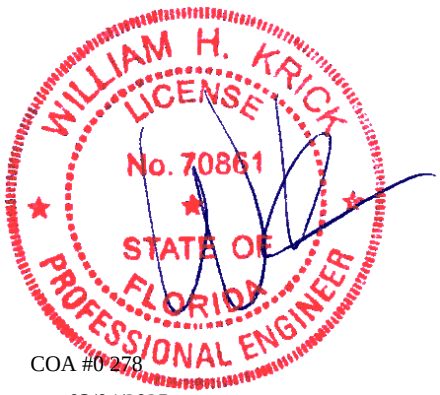
Nailnote
Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 5.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	58 plf at	-1.69 to	58 plf at	14.02
BC: From	5 plf at	-1.69 to	5 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	3.10
BC: From	20 plf at	3.10 to	20 plf at	12.33
BC: From	5 plf at	12.33 to	5 plf at	14.02

BC: 1214 lb Conc. Load at 1.10
BC: 2406 lb Conc. Load at 3.10

Wind
Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
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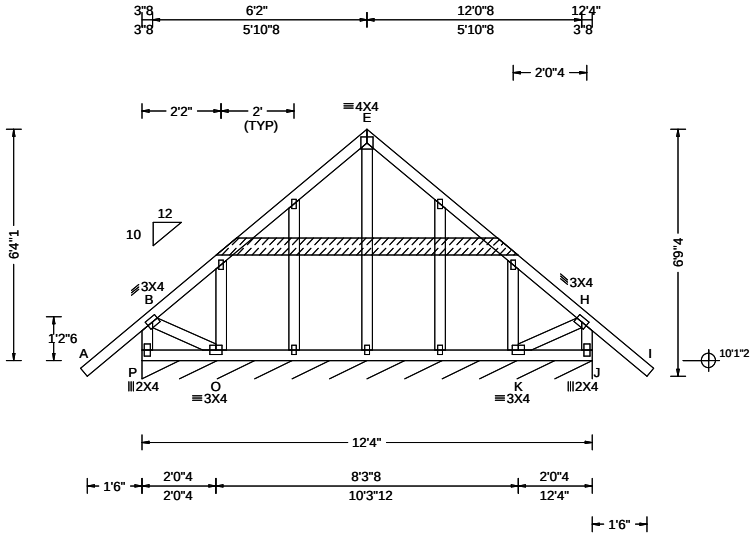
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25263 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** K3-G 12'4" Gable	Cust: R 857 JRef:1Y808570001 T5 DrwNo: 063.25.1351.39107 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 F 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 F 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.004 D - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.379
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.031
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.314
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J*	113	/-	/-	/45	/34	/15

Wind reactions based on MWFRS
J Brg Wid = 147 Min Req = -
Bearing P is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

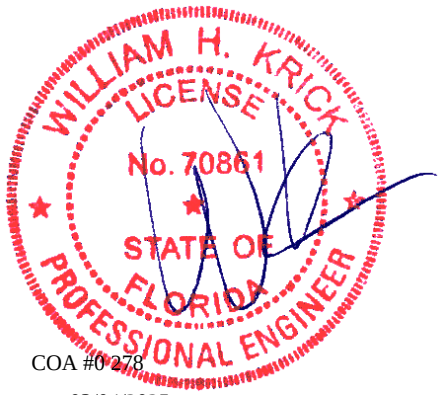
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278

03/04/2025

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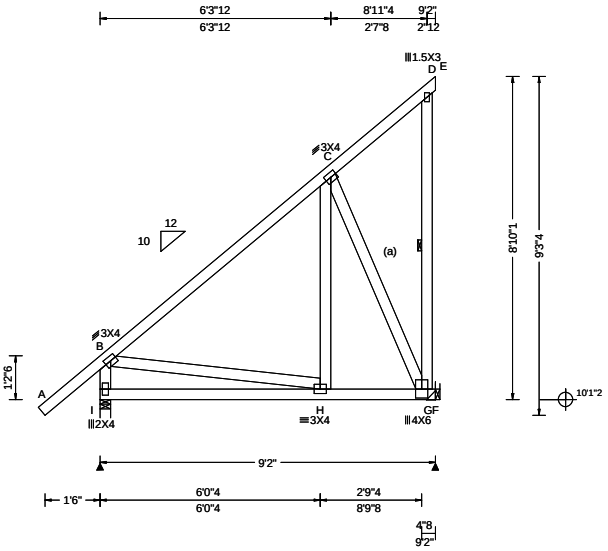
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25149 MONO FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** L1 9'2" Mono		Cust: R 857 JRef:1Y808570001 T46 DrwNo: 063.25.1351.40497 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	H	999	360
VERT(CL): 0.006	H	999	240
HORZ(LL): -0.002	D	-	-
HORZ(TL): 0.005	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.380
Max BC CSI: 0.239
Max Web CSI: 0.291

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	465	/-	/-	/195	/-	/223
F	353	/-	/-	/329	/123	/-

Wind reactions based on MWFRS
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
Bearing I is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
I - H	162	-570

Maximum Web Forces Per Ply (lbs)

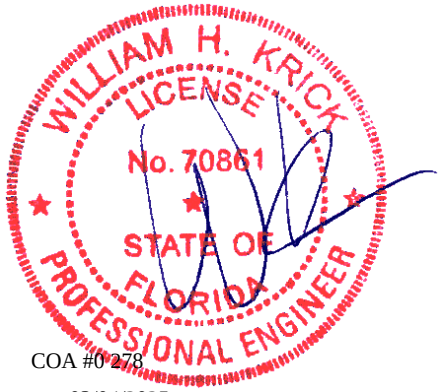
Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - I	136	-413	C - G	394	-345
B - H	409	-41			

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing
(a) Continuous lateral restraint equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" reinforcement. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
03/04/2025
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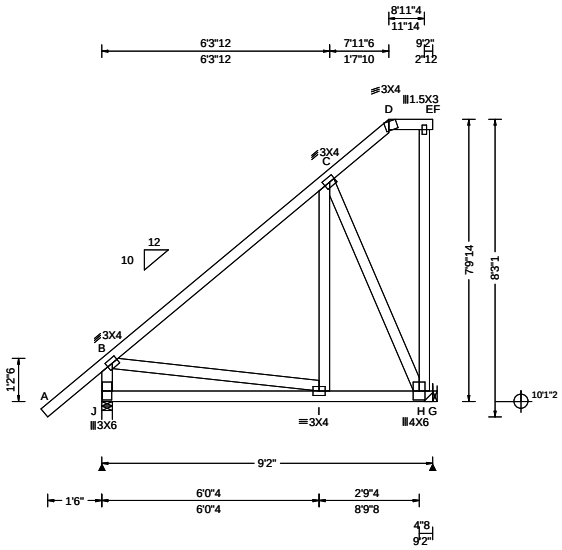
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25152 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** L2 9'2" Common	Cust: R 857 JRef:1Y808570001 T35 DrwNo: 063.25.1351.41603 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 I 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): -0.011 D 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.004 D - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.008 D - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.380
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.239
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: No	Max Web CSI: 0.290
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J	465	/-	/-	/203	/-	/197
G	353	/-	/-	/289	/114	/-

Wind reactions based on MWFRS
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
G Brg Wid = - Min Req = -
Bearing J is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
J - I	144	-513

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - J	184	-413	C - H	390	-344

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Hangers / Ties

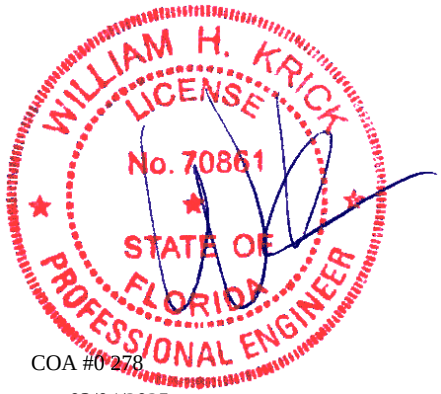
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278

03/04/2025

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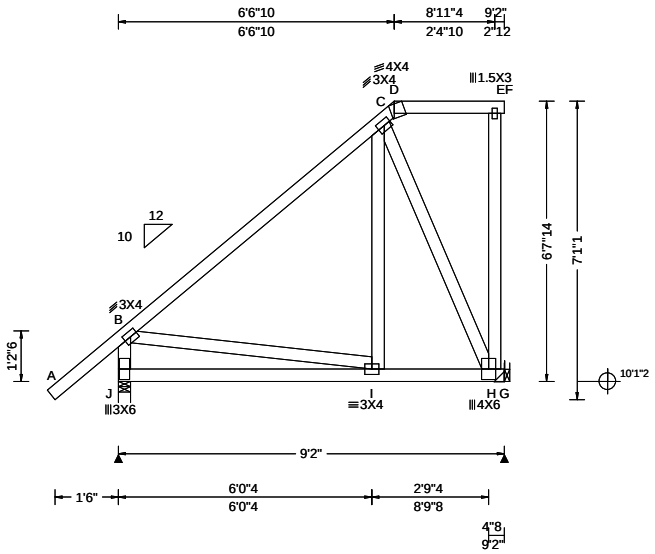
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25154 COMN FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** L3 9'2" Common	Cust: R 857 JRef:1Y808570001 T41 DrwNo: 063.25.1351.42773 SSB / DF 03/04/2025	



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	I	999	360
VERT(CL): 0.006	I	999	240
HORZ(LL): -0.004	D	-	-
HORZ(TL): 0.008	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.382
Max BC CSI: 0.239
Max Web CSI: 0.286

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J	465	/-	/-	/213	/18	/232
G	353	/-	/-	/249	/147	/-

Wind reactions based on MWFRS
J Brg Wid = 3.5 Min Req = 1.5 (Truss)
G Brg Wid = - Min Req = -
Bearing J is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
J - I	126	-449

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - J	246	-413	C - H	385	-339

Lumber
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

COA #0 278
03/04/2025
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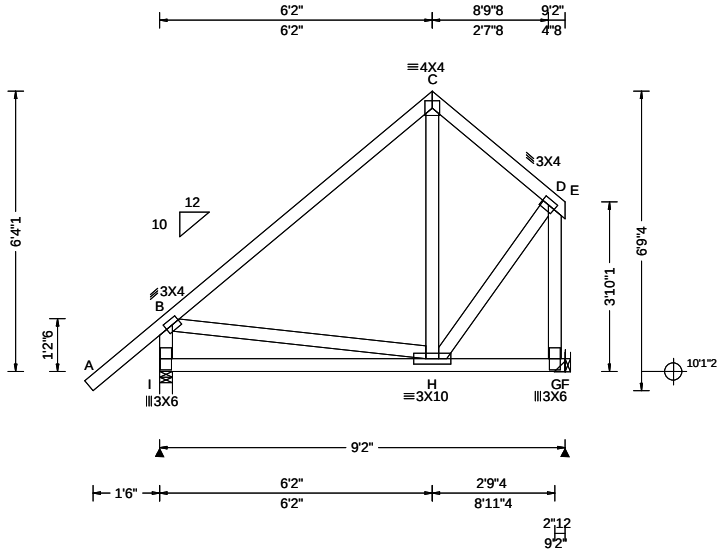
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25140 COMN FROM: RJL	Ply: 1 Qty: 4	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** L4 9'2" Common	Cust: R 857 JRef:1Y808570001 T31 DrwNo: 063.25.1351.43937 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 360 VERT(CL): 0.005 C 999 240 HORZ(LL): 0.001 D - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.376 Max BC CSI: 0.251 Max Web CSI: 0.175 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	465	/-	/-	/199	/70	/169
F	353	/-	/-	/233	/84	/-

Wind reactions based on MWFRS
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
Bearing I is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - I	316	-412

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Hangers / Ties

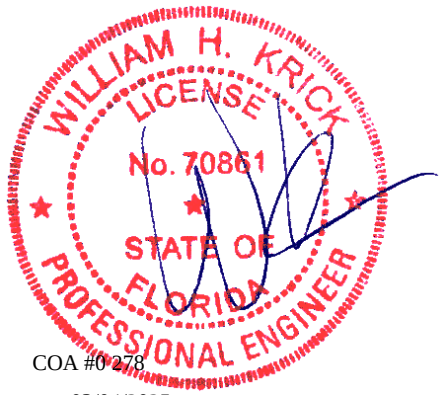
(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278

03/04/2025

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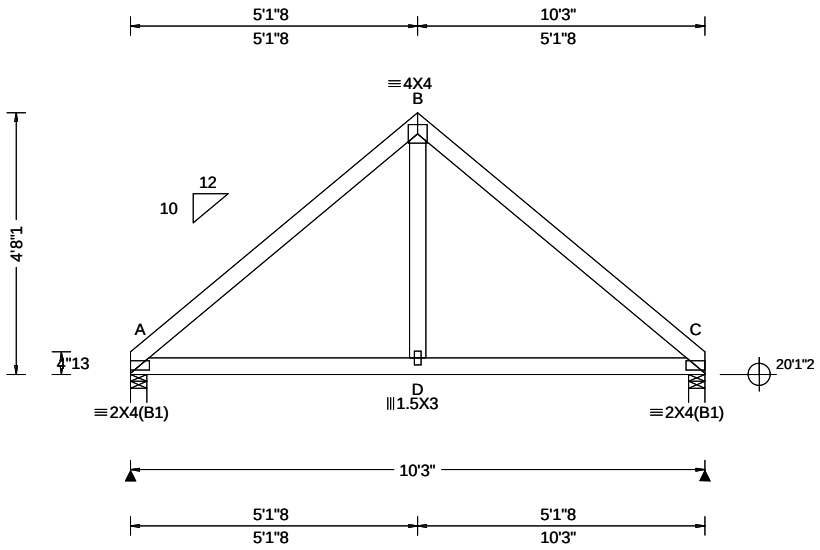
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25110 COMN FROM: RJL	Ply: 1 Qty: 4	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** M1 10'3" Common	Cust: R 857 JRef:1Y808570001 T16 DrwNo: 063.25.1351.45297 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 22.63 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	D	999	360
VERT(CL): 0.005	D	999	240
HORZ(LL): 0.002	A	-	-
HORZ(TL): 0.005	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.275
Max BC CSI: 0.199
Max Web CSI: 0.087

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	401	/-	/-	/217	/120	/131
C	401	/-	/-	/217	/120	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
C Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A & C are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	217	-430	B - C	218	-430

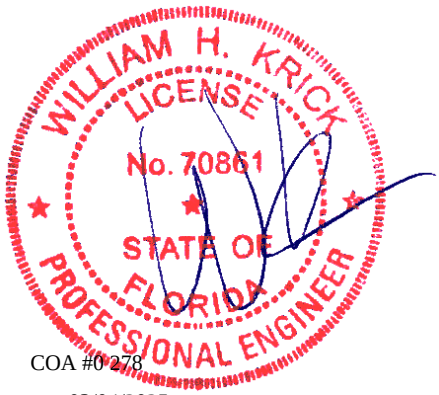
Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0278

03/04/2025

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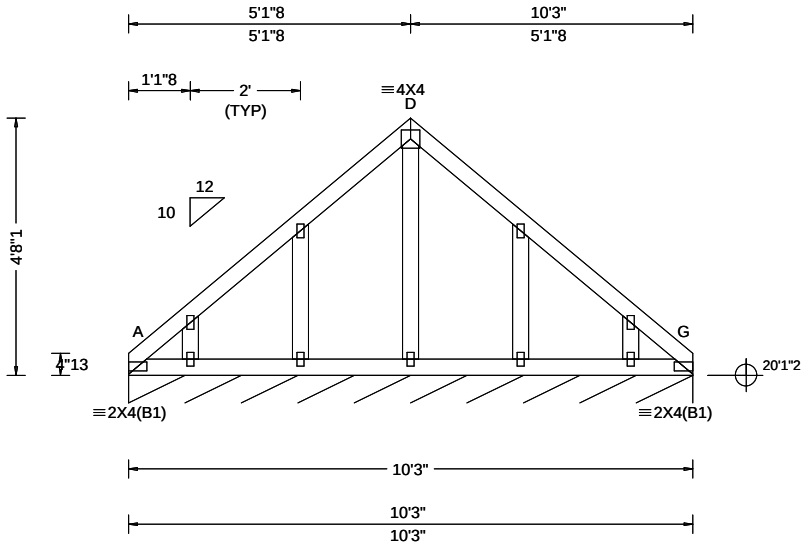
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25112 GABL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** M2-G 10'3" Gable	Cust: R 857 JRef:1Y808570001 T40 DrwNo: 063.25.1351.46593 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 22.63 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 5.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 360 VERT(CL): 0.001 D 999 240 HORZ(LL): -0.000 A - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.062 Max BC CSI: 0.024 Max Web CSI: 0.620 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A*	88	/-	/-	/43	/37	/14

Wind reactions based on MWFRS
A Brg Wid = 123 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

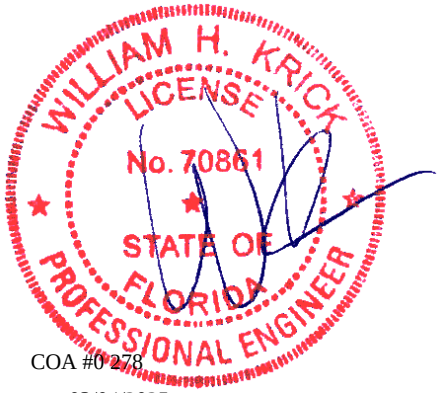
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/378.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.



COA #0 278

03/04/2025

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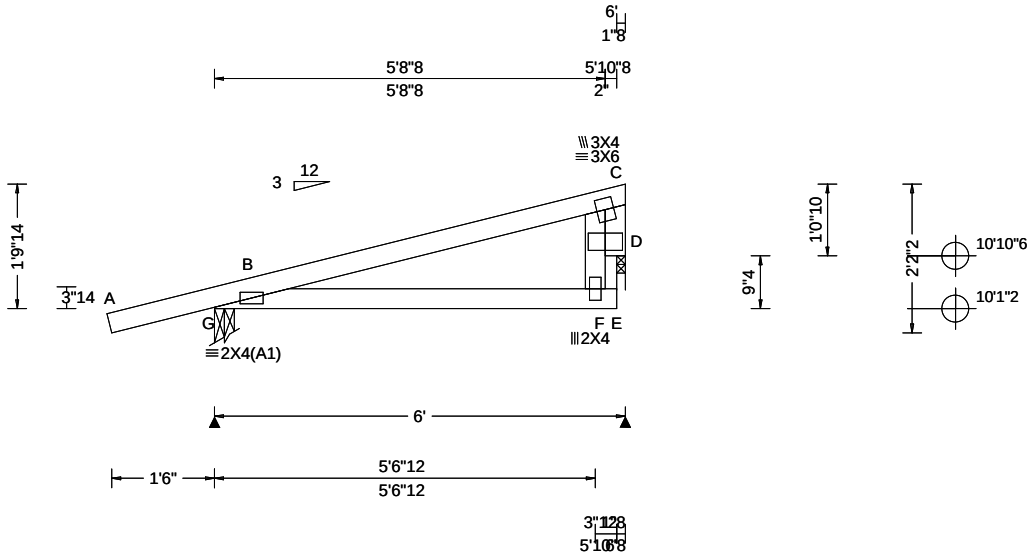
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25106 MONO FROM: RJL	Ply: 1 Qty: 16	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** N1 6' Mono		Cust: R 857 JRef:1Y808570001 T17 DrwNo: 063.25.1351.47933 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.020	B	999	360
VERT(CL): 0.029	B	999	240
HORZ(LL): 0.006	B	-	-
HORZ(TL): 0.008	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.322
Max BC CSI: 0.216
Max Web CSI: 0.364

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	331	/-	/-	/123	/101	/65
D	203	/-	/-	/107	/52	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = 1.5 (Support)
Bearings G & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
C - D	1106	-565

Lumber

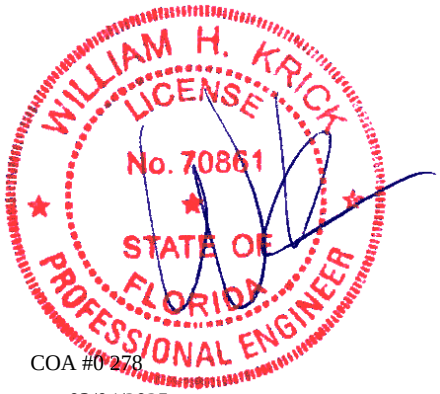
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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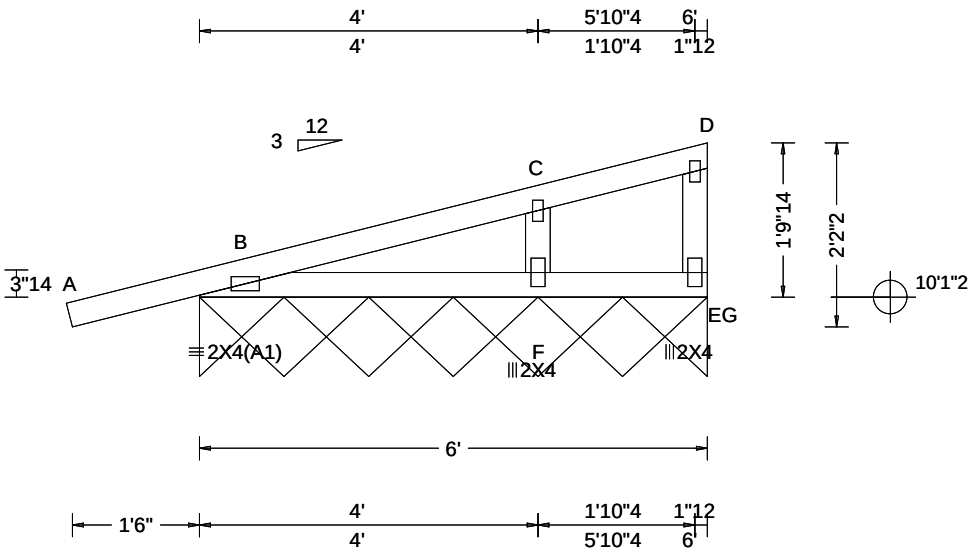
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25108 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** N2-G 6' Gable	Cust: R 857 JRef:1Y808570001 T18 DrwNo: 063.25.1351.50360 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 360 VERT(CL): 0.006 B 999 240 HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.232 Max BC CSI: 0.078 Max Web CSI: 0.125 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	93	/-	/-	/40	/78	/25

Wind reactions based on MWFRS
G Brg Wid = 72.0 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
C - F	421 - 207

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

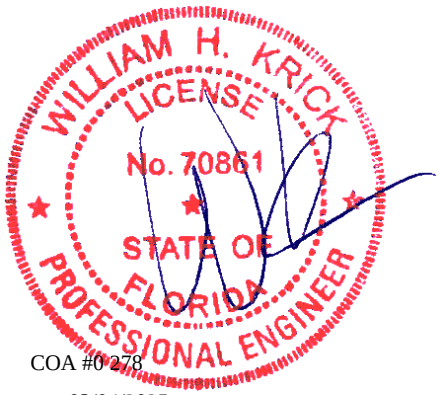
Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0278

03/04/2025

Florida Certificate of Product Approval #FL 1999

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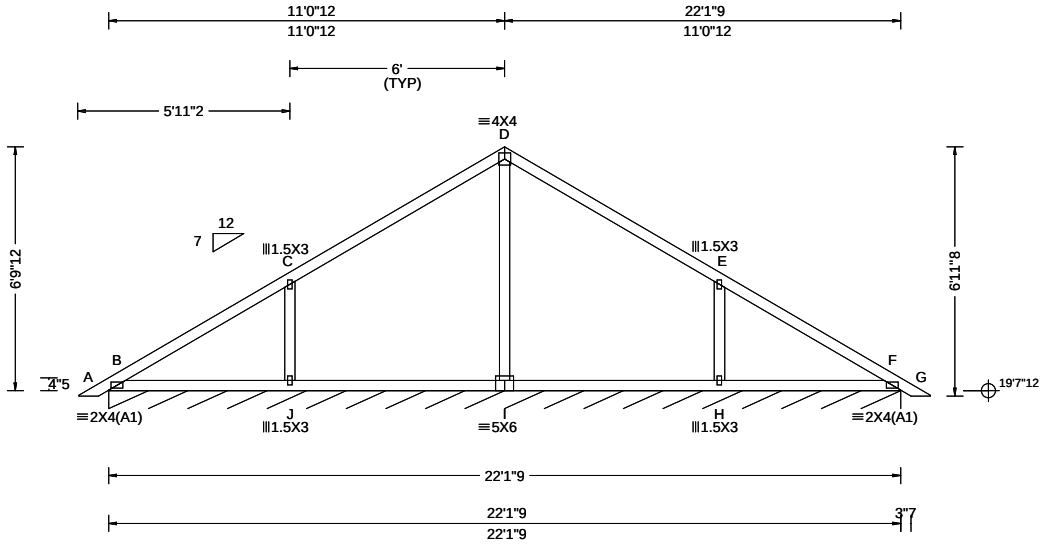
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25218 COMN FROM: RJL	Ply: 1 Qty: 10	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** PB1 23'10"5 Common	Cust: R 857 JRef:1Y808570001 T55 DrwNo: 063.25.1350.55100 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 22.99 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	F	999	360
VERT(CL): 0.005	F	999	240
HORZ(LL): -0.002	B	-	-
HORZ(TL): 0.003	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.390
Max BC CSI: 0.312
Max Web CSI: 0.192

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	95	/-	/-	/38	/9	/8

Wind reactions based on MWFRS
B Brg Wid = 265 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

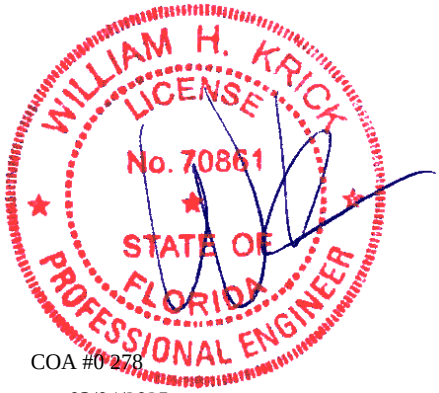
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160220723 for piggyback details.



COA #0 278
03/04/2025
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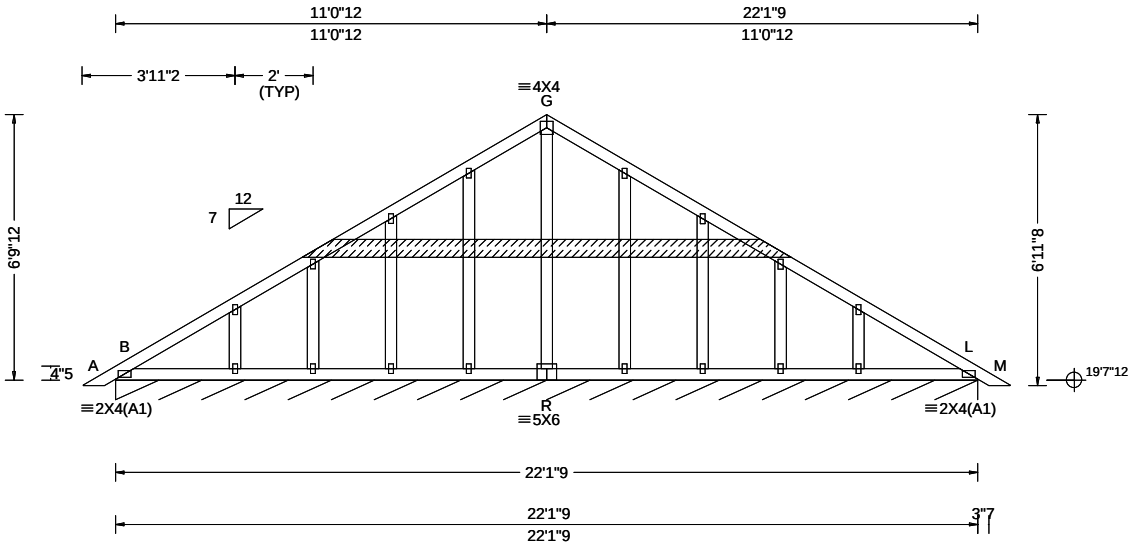
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25223 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** PB2-G 23'10"5 Gable	Cust: R 857 JRef:1Y808570001 T54 DrwNo: 063.25.1350.57927 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 H 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 H 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 B - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.003 F - -
NCBCLL: 10.00	Mean Height: 22.99 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.066
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.050
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.493
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 10.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	95	/-	/-	/40	/30	/9

Wind reactions based on MWFRS
B Brg Wid = 265 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/956.

Additional Notes

See DWG GBLDIAG220923 for gable stiffback and diagonal bracing details.

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

COA #0 278

03/04/2025

Florida Certificate of Product Approval #FL 1999

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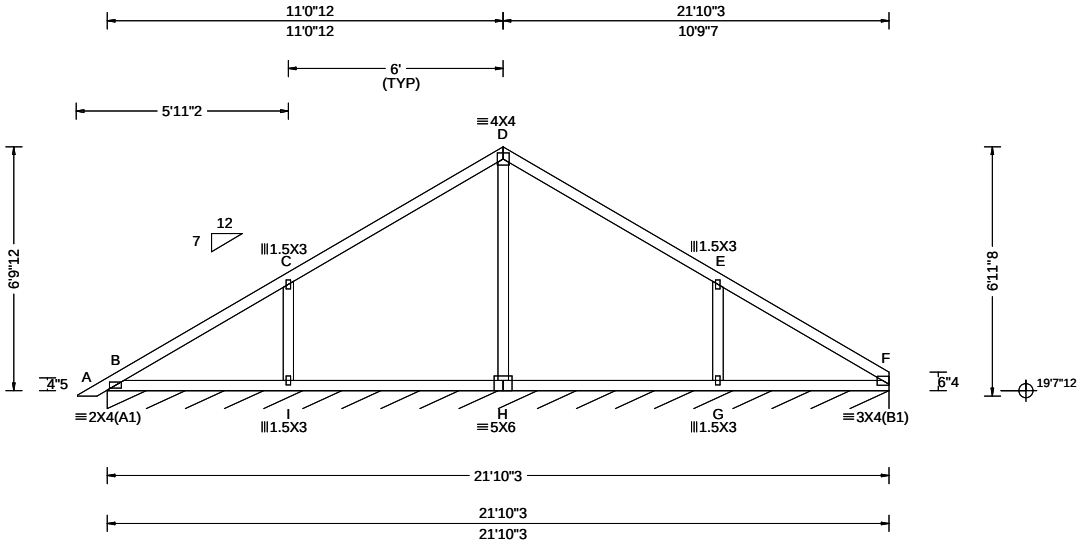
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25227 COMN FROM: RJL	Ply: 1 Qty: 6	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** PB3 22'8"9 Common	Cust: R 857 JRef:1Y808570001 T57 DrwNo: 063.25.1350.59163 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 B 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.004 B 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 B - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.004 E - -
NCBCLL: 10.00	Mean Height: 22.99 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.391
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.315
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: No	Max Web CSI: 0.202
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	94	/-	/-	/38	/9	/9

Wind reactions based on MWFRS
B Brg Wid = 262 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160220723 for piggyback details.

COA #0278

03/04/2025

Florida Certificate of Product Approval #FL 1999

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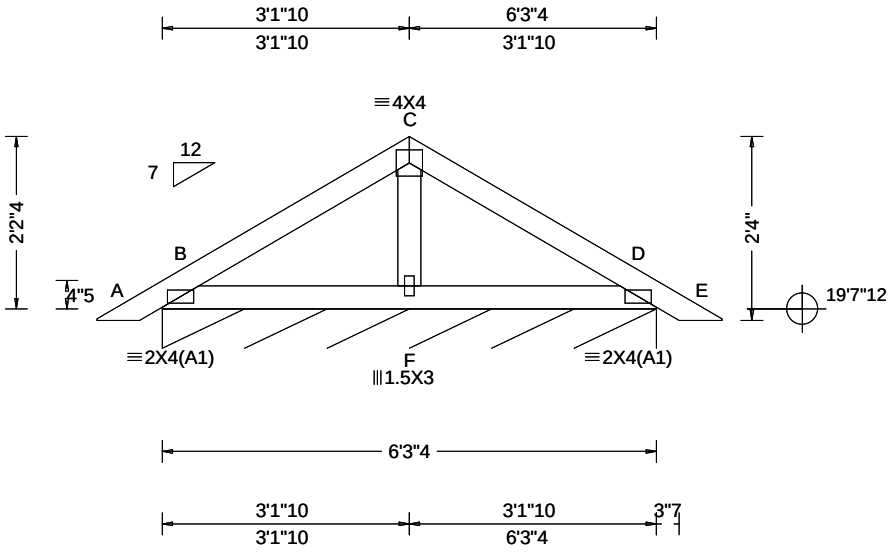
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25229 COMN FROM: RJL	Ply: 1 Qty: 11	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** PB4 8' Common		Cust: R 857 JRef:1Y808570001 T58 DrwNo: 063.25.1351.06063 SSB / DF 03/04/2025	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 20.68 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	360
VERT(CL): 0.002	B	999	240
HORZ(LL): -0.001	D	-	-
HORZ(TL): 0.001	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.113
Max BC CSI: 0.077
Max Web CSI: 0.012

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	87	/-	/-	/39	/18	/8

Wind reactions based on MWFRS
B Brg Wid = 75.3 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

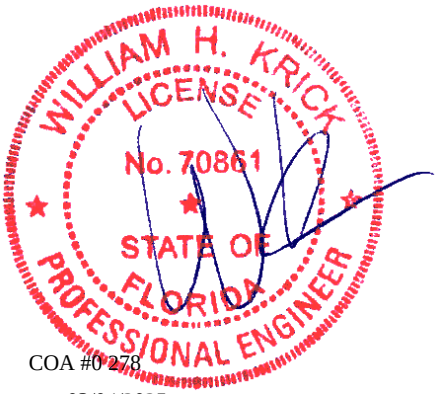
Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160220723 for piggyback details.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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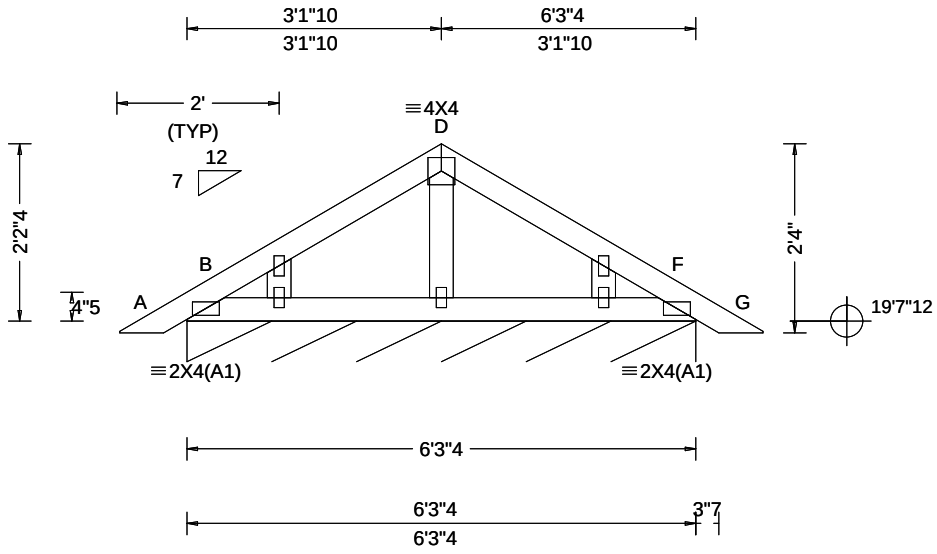
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25234 GABL FROM: RJL	Ply: 1 Qty: 2	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** PB5-G 8' Gable		Cust: R 857 JRef:1Y808570001 T56 DrwNo: 063.25.1349.20083 SSB / DF 03/04/2025	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.68 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 360 VERT(CL): 0.000 D 999 240 HORZ(LL): -0.000 B - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.059 Max BC CSI: 0.024 Max Web CSI: 0.053 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	92	/-	/-	/41	/45	/9

Wind reactions based on MWFRS
B Brg Wid = 75.3 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 1.5X3 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

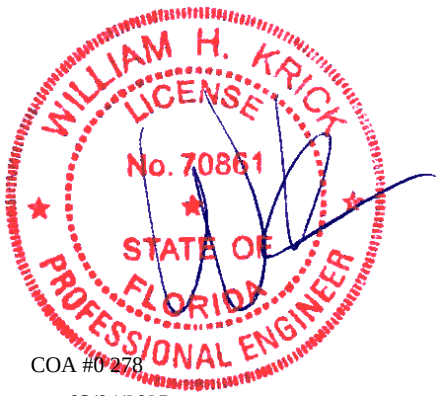
Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/999.

Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Refer to DWG PB160220723 for piggyback details.



COA #0 278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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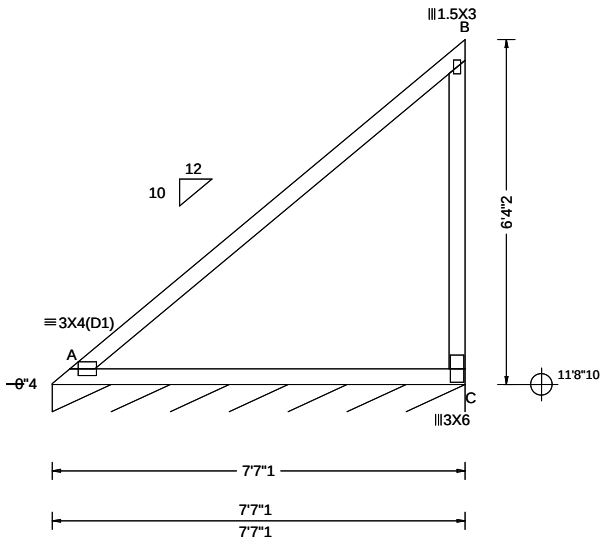
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25194 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V1 7'7"1 Valley	Cust: R 857 JRef:1Y808570001 T29 DrwNo: 063.25.1349.21333 SSB / DF 03/04/2025



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.021 A - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.040 A - -
NCBCLL: 10.00	Mean Height: 15.04 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 4.2 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.784
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.587
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: No	Max Web CSI: 0.134
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 24.02.00C.1213.15
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/54	/4	/20

Wind reactions based on MWFRS
C Brg Wid = 91.1 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
A - C	383	-106

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0278

03/04/2025

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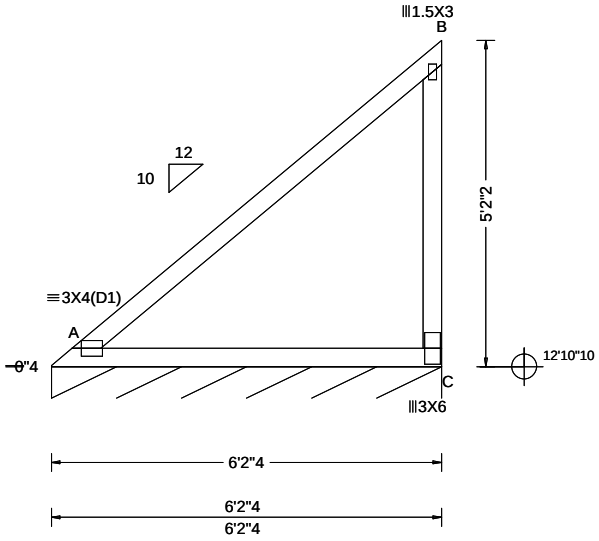
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25196 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V2 6'2"4 Valley	Cust: R 857 JRef:1Y808570001 T39 DrwNo: 063.25.1349.25293 SSB / DF 03/04/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.62 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): NA
VERT(CL): NA
HORZ(LL): 0.011 A - -
HORZ(TL): 0.022 A - -
Creep Factor: 2.0
Max TC CSI: 0.571
Max BC CSI: 0.344
Max Web CSI: 0.080

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/54	/3	/20

Wind reactions based on MWFRS
C Brg Wid = 74.3 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0278

03/04/2025

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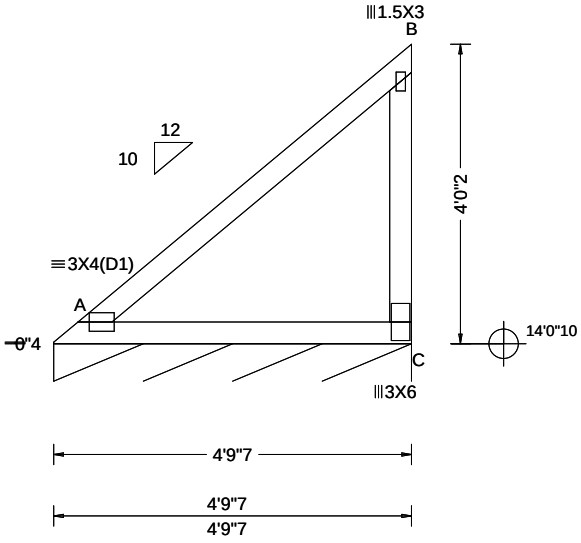
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25198 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V3 4'9"7 Valley	Cust: R 857 JRef:1Y808570001 T48 DrwNo: 063.25.1349.26160 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.20 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.005 A - - HORZ(TL): 0.009 A - - Creep Factor: 2.0 Max TC CSI: 0.357 Max BC CSI: 0.216 Max Web CSI: 0.176 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/53	/3	/20

Wind reactions based on MWFRS
C Brg Wid = 57.5 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

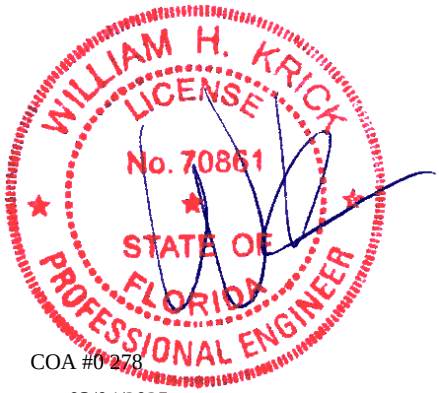
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0278

03/04/2025

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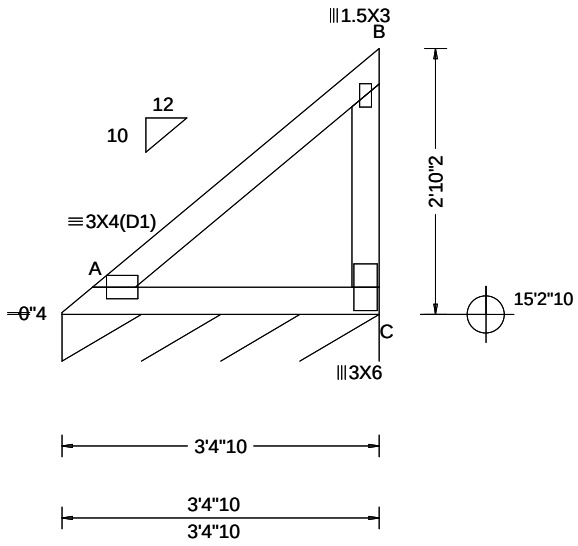
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25200 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V4 3'4"10 Valley	Cust: R 857 JRef:1Y808570001 T49 DrwNo: 063.25.1349.26940 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.79 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 A - - HORZ(TL): 0.003 A - - Creep Factor: 2.0 Max TC CSI: 0.170 Max BC CSI: 0.103 Max Web CSI: 0.096 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/52	/3	/19

Wind reactions based on MWFRS
C Brg Wid = 40.7 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

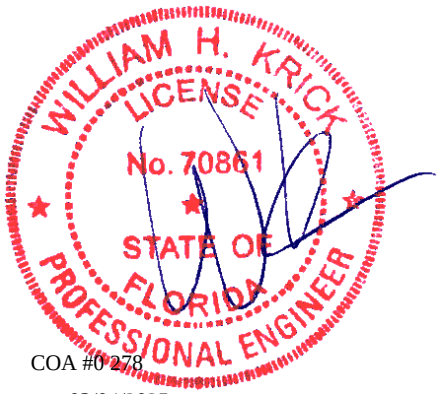
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0278
Florida Certificate of Product Approval #FL 1999
03/04/2025

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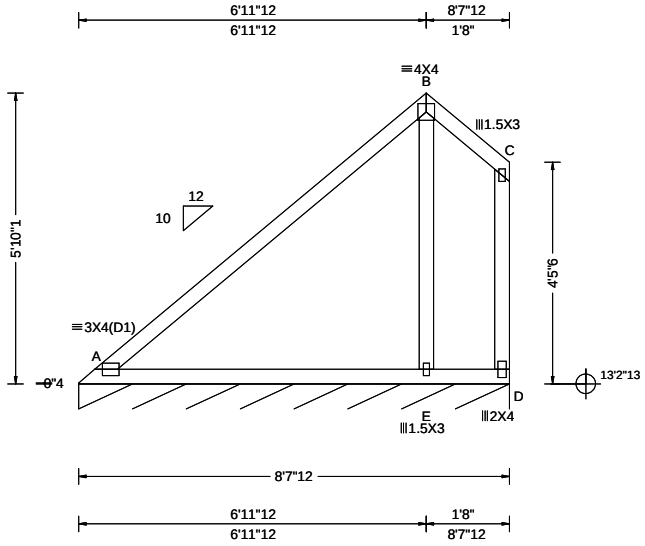
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25082 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V5 8'7"12 Valley	Cust: R 857 JRef:1Y808570001 T50 DrwNo: 063.25.1349.27923 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 16.30 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.046	A	999	360
VERT(CL): 0.062	A	999	240
HORZ(LL): 0.019	A	-	-
HORZ(TL): 0.027	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.597
Max BC CSI: 0.416
Max Web CSI: 0.129

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	78	/-	/-	/49	/14	/18

Wind reactions based on MWFRS
D Brg Wid = 103 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

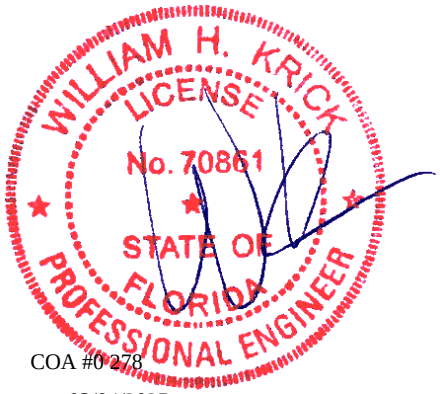
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278

03/04/2025

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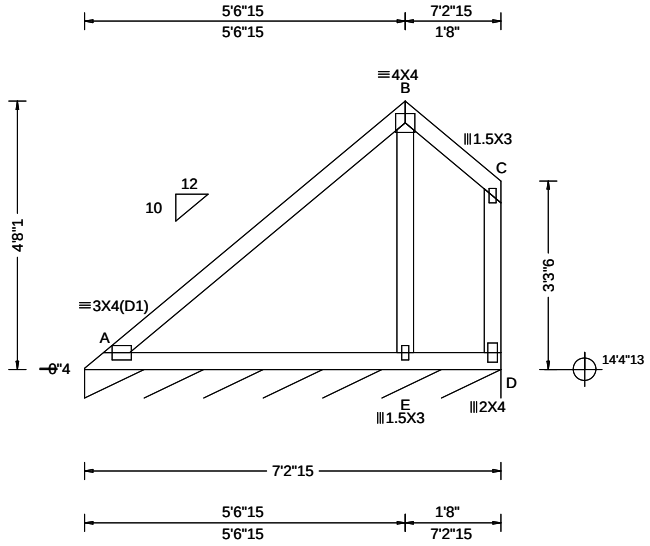
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25084 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V6 7'2"15 Valley		Cust: R 857 JRef:1Y808570001 T51 DrwNo: 063.25.1349.28850 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 16.88 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.017 A 999 360 VERT(CL): 0.031 A 999 240 HORZ(LL): 0.007 A - - HORZ(TL): 0.017 A - - Creep Factor: 2.0 Max TC CSI: 0.435 Max BC CSI: 0.283 Max Web CSI: 0.071 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	78	/-	/-	/47	/14	/16

Wind reactions based on MWFRS
D Brg Wid = 87.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0278

03/04/2025

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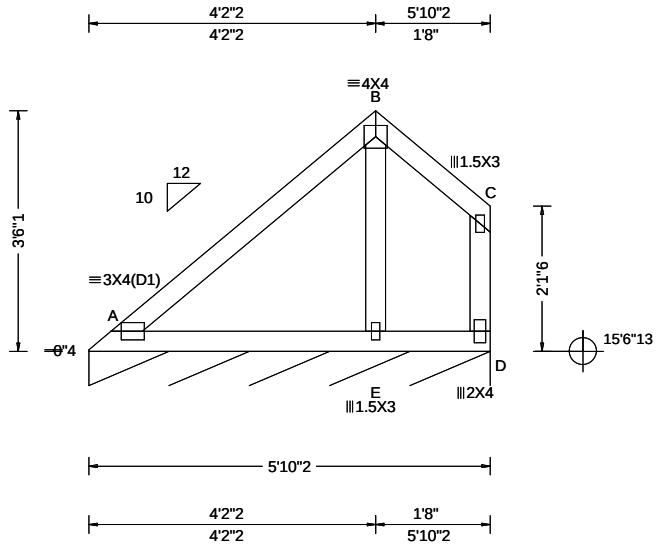
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25086 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V7 5'10"2 Valley	Cust: R 857 JRef:1Y808570001 T52 DrwNo: 063.25.1349.29827 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 17.46 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.007 A 999 360 VERT(CL): 0.013 A 999 240 HORZ(LL): 0.003 A - - HORZ(TL): 0.006 A - - Creep Factor: 2.0 Max TC CSI: 0.282 Max BC CSI: 0.181 Max Web CSI: 0.039 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	78	/-	/-	/45	/11	/15

Wind reactions based on MWFRS
D Brg Wid = 70.2 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

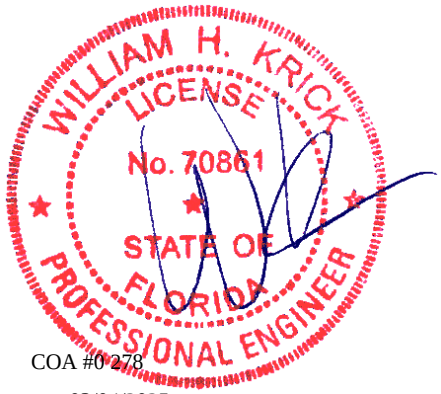
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0278

03/04/2025

Florida Certificate of Product Approval #FL 1999

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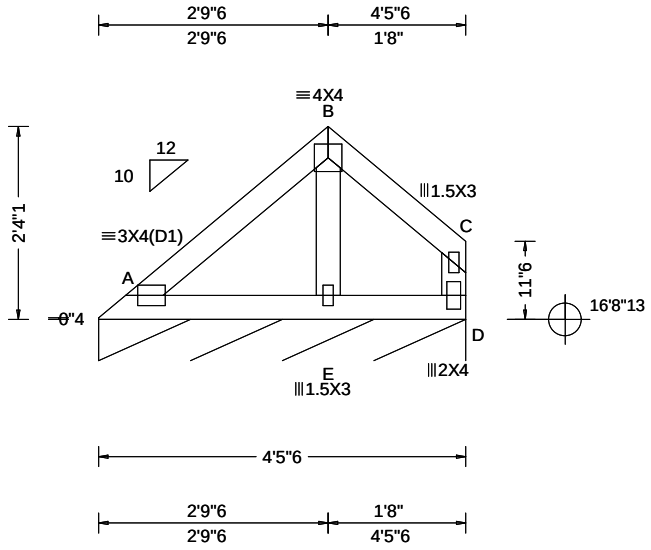
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25088 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V8 4'5"6 Valley		Cust: R 857 JRef:1Y808570001 T53 DrwNo: 063.25.1349.30807 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 18.05 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 A 999 360 VERT(CL): 0.004 A 999 240 HORZ(LL): 0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.112 Max BC CSI: 0.076 Max Web CSI: 0.039 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
D*	78	/-	/-	/42	/11	/13

Wind reactions based on MWFRS
D Brg Wid = 53.4 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0278

03/04/2025

Florida Certificate of Product Approval #FL 1999

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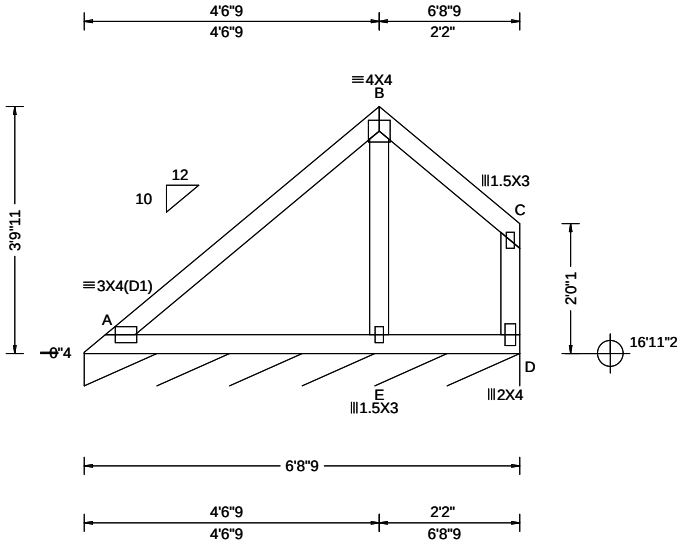
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25190 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V9 6'8"9 Valley	Cust: R 857 JRef:1Y808570001 T42 DrwNo: 063.25.1349.32000 SSB / DF 03/04/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 18.98 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.009	A	999	360
VERT(CL): 0.017	A	999	240
HORZ(LL): 0.004	A	-	-
HORZ(TL): 0.009	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.319
Max BC CSI: 0.207
Max Web CSI: 0.055

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	78	/-	/-	/44	/12	/15

Wind reactions based on MWFRS
D Brg Wid = 80.6 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

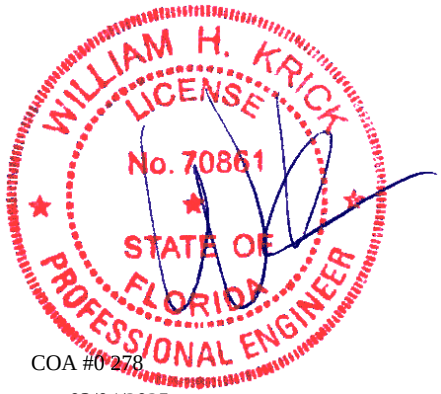
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
03/04/2025
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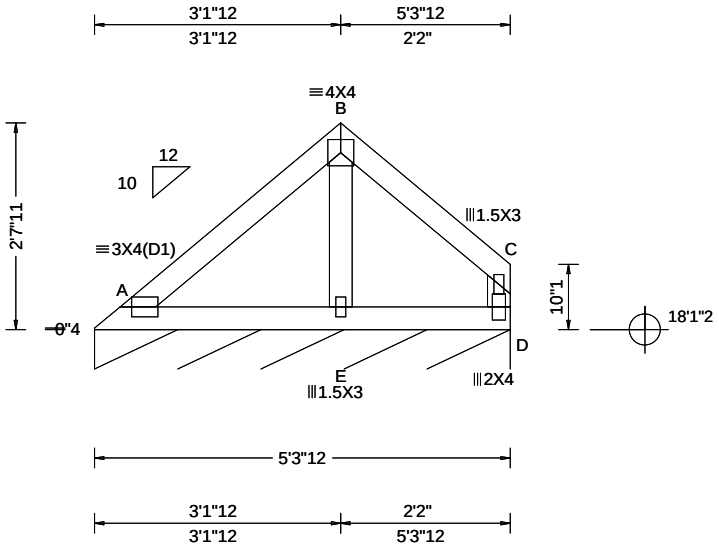
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25192 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V10 5'3"12 Valley	Cust: R 857 JRef:1Y808570001 T47 DrwNo: 063.25.1349.22270 SSB / DF 03/04/2025



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 19.56 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.003 A	999	360
VERT(CL):	0.006 A	999	240
HORZ(LL):	0.001 A	-	-
HORZ(TL):	0.002 A	-	-

Creep Factor: 2.0
Max TC CSI: 0.157
Max BC CSI: 0.106
Max Web CSI: 0.058

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D*	78	/-	/-	/41	/11	/13

Wind reactions based on MWFRS
D Brg Wid = 63.8 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

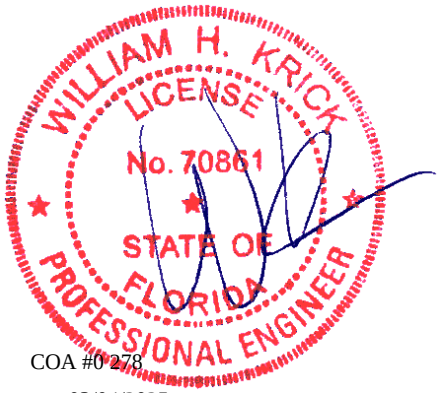
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0278
03/04/2025
Florida Certificate of Product Approval #FL 1999

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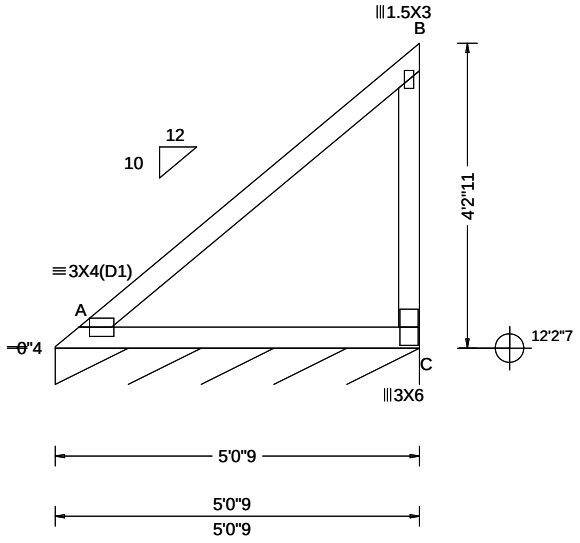
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25078 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V11 5'0"9 Valley	Cust: R 857 JRef:1Y808570001 T3 DrwNo: 063.25.1349.23643 SSB / DF 03/04/2025



Loading Criteria (psf)
TCLL: 20.00
TCDL: 7.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 37.00
NCBCLL: 10.00
Soffit: 2.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-22
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 8th Ed. 2023 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.008 A	-	-
HORZ(TL):	0.011 A	-	-

Creep Factor: 2.0
Max TC CSI: 0.387
Max BC CSI: 0.235
Max Web CSI: 0.192

VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/53	/12	/27

Wind reactions based on MWFRS
C Brg Wid = 60.6 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

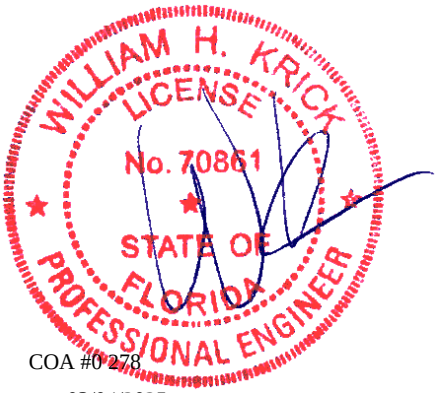
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL 1999
03/04/2025

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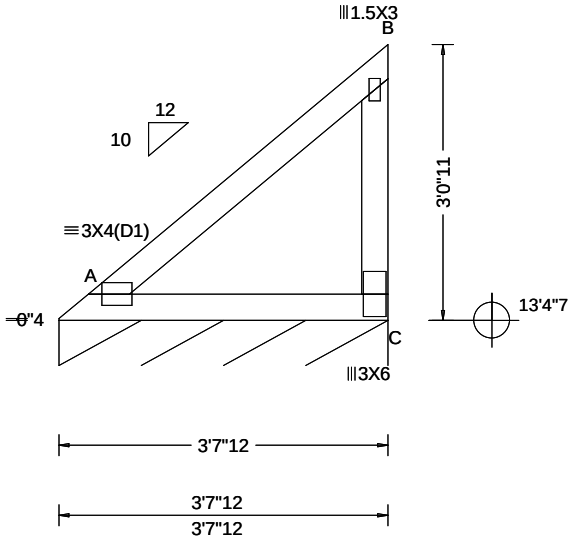
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 25080 VAL FROM: RJL	Ply: 1 Qty: 1	**Job Number:** B60735a LETNER RESIDENCE **Truss Label:** V12 3'7"12 Valley	Cust: R 857 JRef:1Y808570001 T15 DrwNo: 063.25.1349.24390 SSB / DF 03/04/2025	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.05 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.003 A - - HORZ(TL): 0.004 A - - Creep Factor: 2.0 Max TC CSI: 0.198 Max BC CSI: 0.119 Max Web CSI: 0.106 VIEW Ver: 24.02.00C.1213.15

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	78	/-	/-	/52	/11	/26

Wind reactions based on MWFRS
C Brg Wid = 43.8 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS VALTN220723 and VAL180220723 for valley details.

COA #0278

03/04/2025

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ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

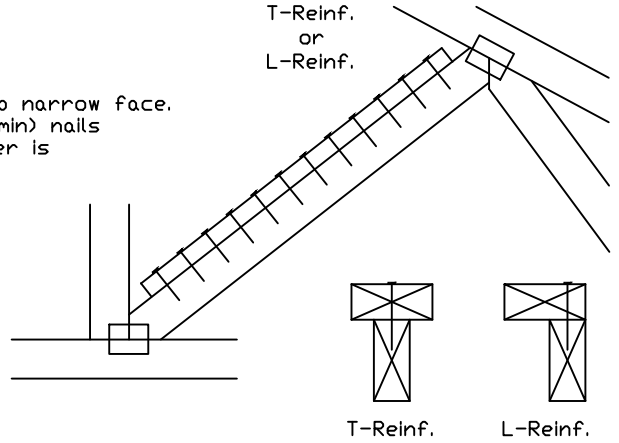
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

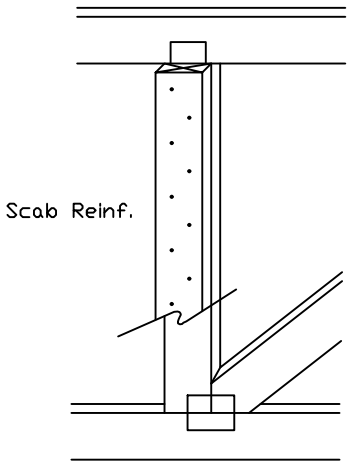
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/04/2025

~~TC~~ LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

COA #0 278 DUR. FAC.
Florida Certificate of Product Approval #FL 1999

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

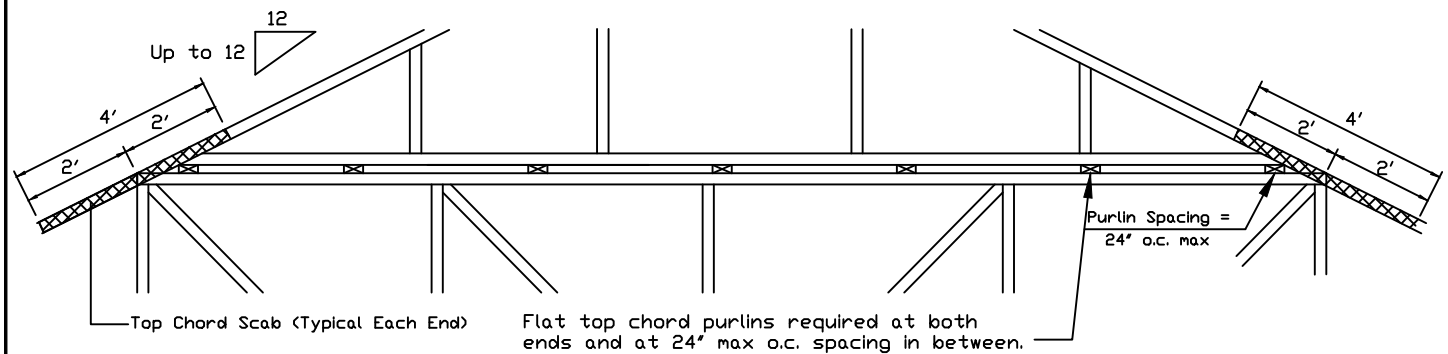
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

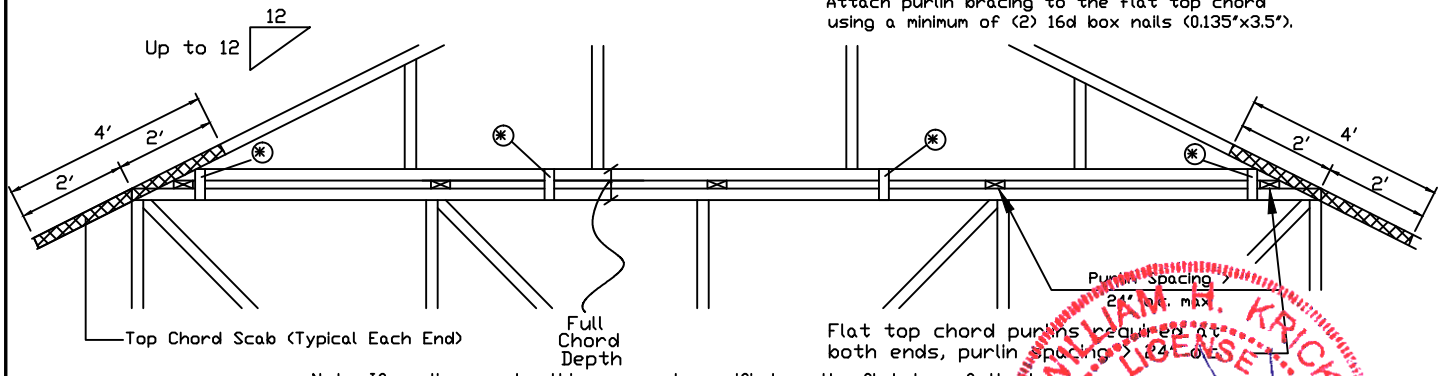
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:
Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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WILLIAM E. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/04/2025

COA #0 278
Florida Certificate of Product Approval #FL 1999

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

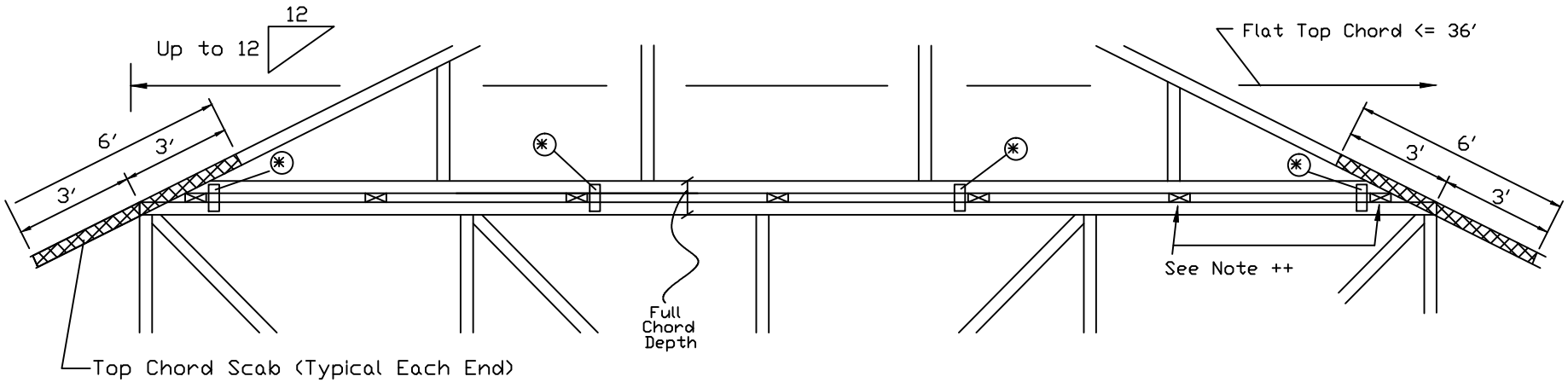
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24" intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods:	
Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

03/04/2025

COA #0 278
Florida Certificate of Product Approval #FL 1999

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118
SPACING	24.0

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section

(L) = Minimum nailing distance on each side of damaged area (B)

(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

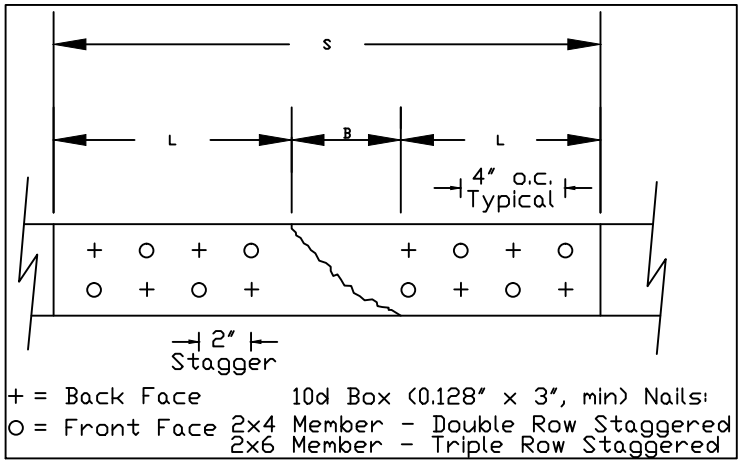
This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

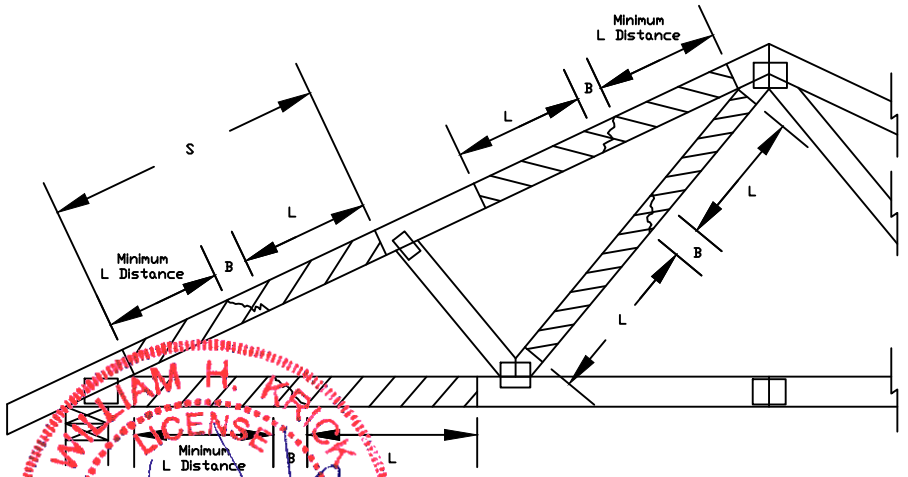
Broken chord may not support any tie-in loads.

Load Duration = 0%
Member forces may be increased for Duration of Load

Member	Size	L	Maximum Member Axial Force SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



Nail Spacing Detail



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AN ITW COMPANY

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Glenview, IL 60025

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03/04/2025

COA #0 278
Florida Certificate of Product Approval #FL 999

REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014
SPACING	24.0" MAX

Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or equivalent connector for
ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
Or
ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

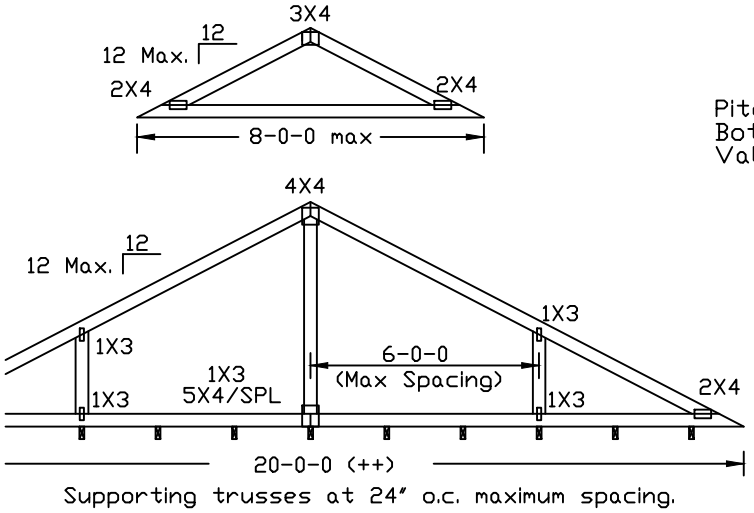
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

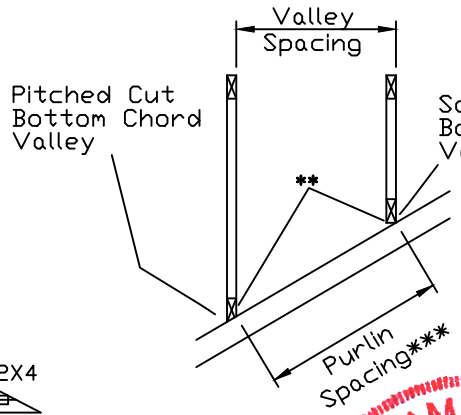
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

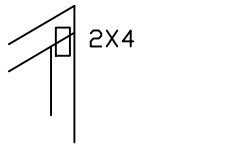
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".

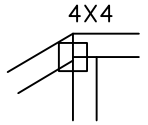


Supporting trusses at 24" o.c. maximum spacing.



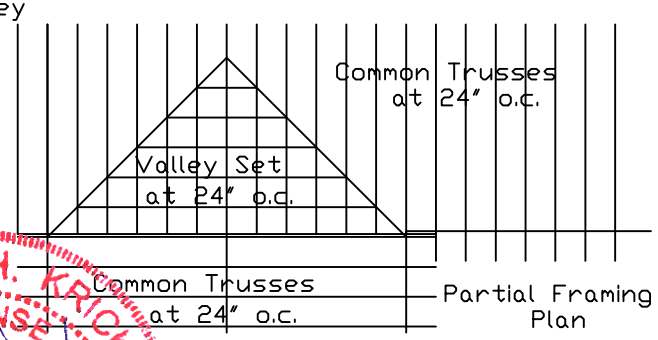


Stubbed Valley End Detail



Optional Hip Joint Detail

Square Cut
Bottom Chord
Valley



Partial Framing Plan

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TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25	1.33	1.15	1.15
SPACING			24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VAL180220723

03/04/2025
COA #0 278
Florida Certificate of Product Approval #FL 1999

Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location:
140 mph for SP (G = 0.55, min.),
125 mph for DF-L (G = 0.50, min.), or
105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

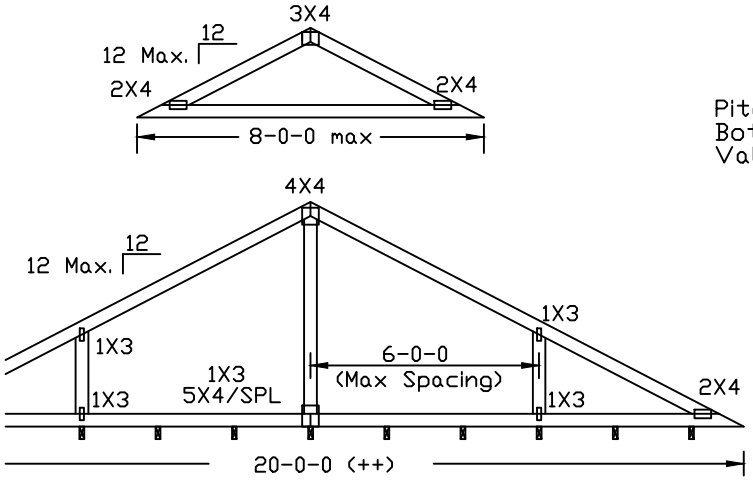
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

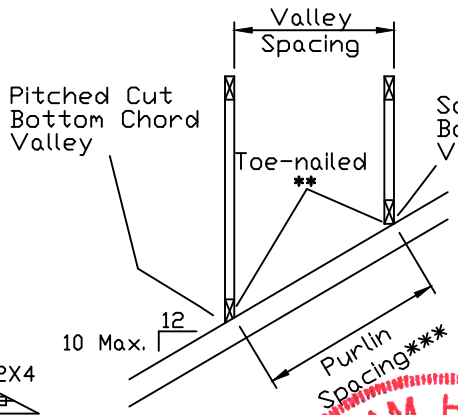
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

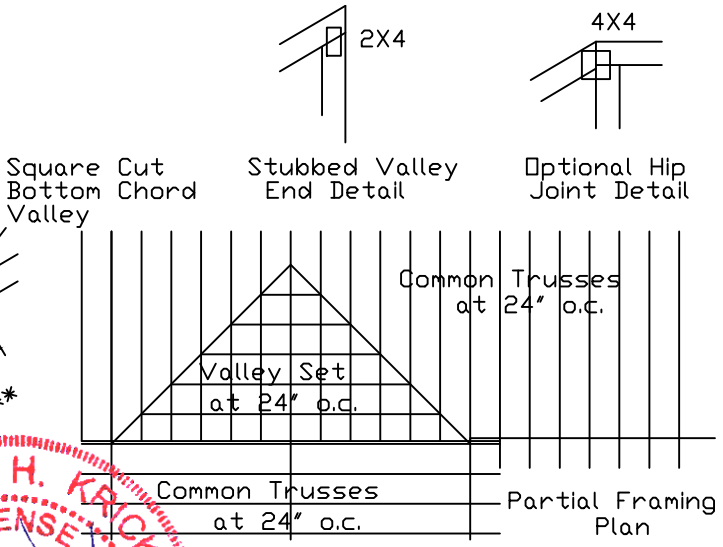
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





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155 Harlem Ave
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Glenview, IL 60025

WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

03/04/2025

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25	1.33	1.15	1.15
SPACING			24.0"	

REF	VALLEY DETAIL
DATE	07/03/2023
DRWG	VALTN220723

COA #0 278
Florida Certificate of Product Approval #FL 1999

Gable End Wind Bracing Details - Stiffback w/ Diagonal Bracing

Apply single or double stiffback as per Engineer's sealed truss design referencing this detail.

Refer to Engineer's sealed truss design for additional information not provided on this detail.

The required locations for lateral restraint or bracing depicted on this detail are for the permanent lateral transfer and support to transfer load and reduce buckling lengths. Details shall be specified by the Building Designer or other Registered Design Professional. This Detail does not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Refer to Building Designer for conditions not addressed by this detail.

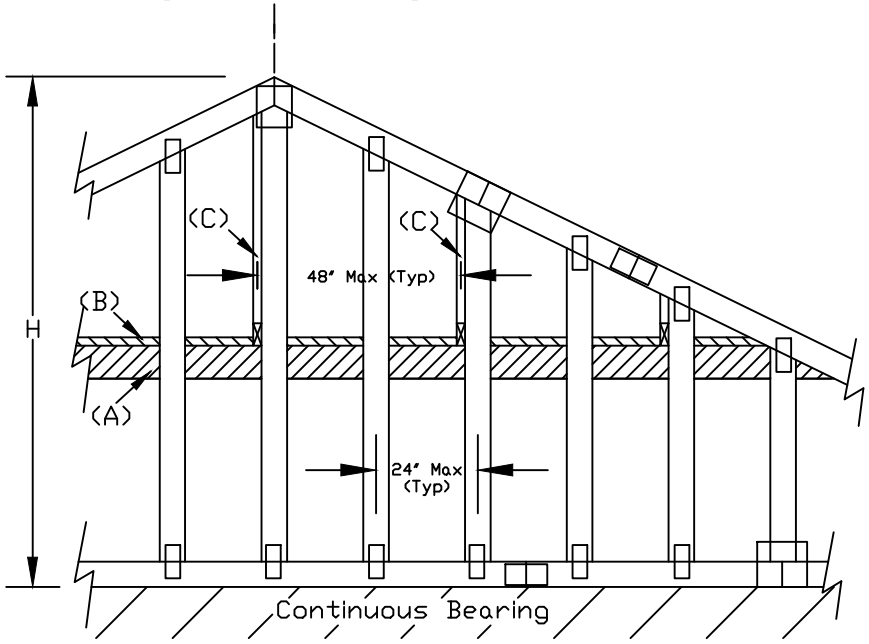
Gable Lateral Bracing Components

(A) Stiffback. Provide connection to each intersecting stud and chord.

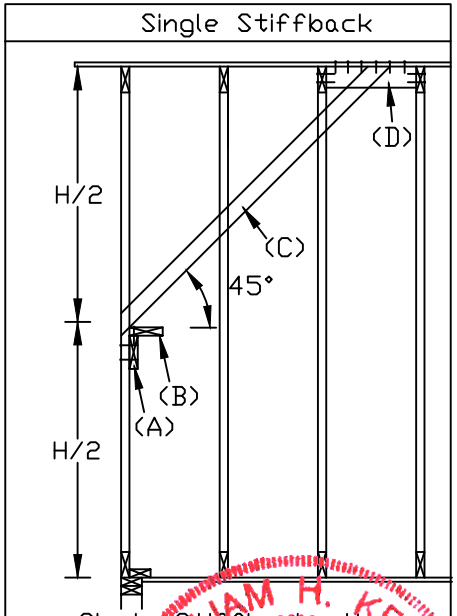
(B) L-reinforcement. Provide connection to narrow edge of stiffback.

(C) Diagonal brace. Provide connection to gable stud at bottom end and to blocking at top end.

(D) Blocking, cut to fit tight between trusses. Attach blocking to trusses at each end and to roof sheathing.

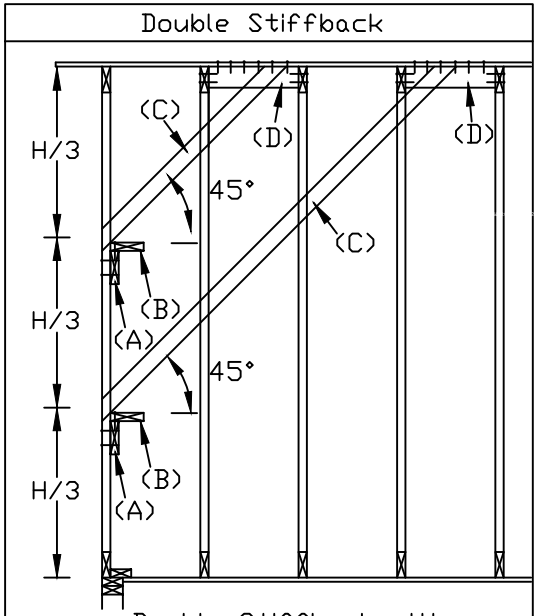


Single Stiffback



Single Stiffback with Diagonal Bracing Spaced at 1/2 Truss Height

Double Stiffback



Double Stiffback with Diagonal Bracing Spaced at 1/3 Truss Height

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MAX. TOT. LD.
03/04/2025
COA #0 278
Florida Certificate of Product Approval #FL 1999
MAX. SPACING

REF	GE STIFFBACK
DATE	09/27/2023
DRWG	GBLDIAG220923