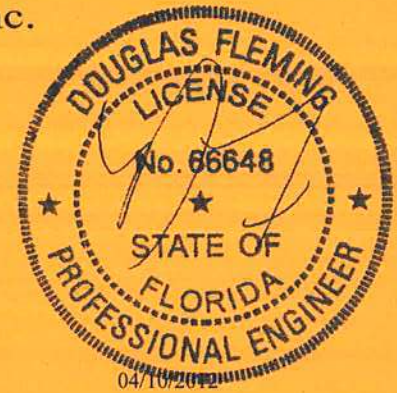


ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IUL5487-Z0110171557



Truss Fabricator: Anderson Truss Company
Job Identification: 12-071--Fill in later BRYAN ZECHER/VICTORES -- , **
Truss Count: 48
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

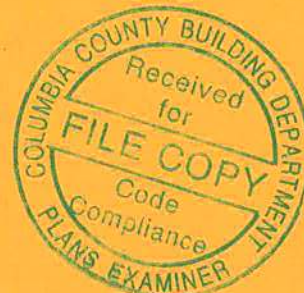
Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-CNNAILSP-PB120-

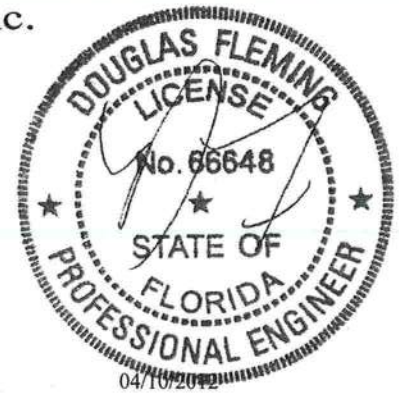
#	Ref	Description	Drawing#	Date
1	03085--H7A		12101001	04/10/12
2	03086--H9A		12101002	04/10/12
3	03087--H11A		12101003	04/10/12
4	03088--H13A		12101004	04/10/12
5	03089--H23A		12101005	04/10/12
6	03090--HT15A		12101006	04/10/12
7	03091--HT17A		12101007	04/10/12
8	03092--HT19A		12101008	04/10/12
9	03093--H21AT		12101009	04/10/12
10	03094--AT		12101010	04/10/12
11	03095--H7B		12101011	04/10/12
12	03096--H9B		12101012	04/10/12
13	03097--H21B		12101013	04/10/12
14	03098--H11BT		12101014	04/10/12
15	03099--H13BT		12101015	04/10/12
16	03100--H15BT		12101016	04/10/12
17	03101--H17BT		12101017	04/10/12
18	03102--H19BT		12101018	04/10/12
19	03103--CGE		12101019	04/10/12
20	03104--C		12101020	04/10/12
21	03105--C1		12101021	04/10/12
22	03106--C2		12101022	04/10/12
23	03107--H9C		12101023	04/10/12
24	03108--H7C		12101024	04/10/12
25	03109--H5C		12101025	04/10/12
26	03110--CS		12101026	04/10/12
27	03111--H3C		12101027	04/10/12
28	03112--D		12101028	04/10/12
29	03113--DGE		12101029	04/10/12
30	03114--D1		12101030	04/10/12
31	03115--D2		12101031	04/10/12
32	03116--H7E		12101032	04/10/12
33	03117--EG		12101033	04/10/12
34	03118--FG		12101034	04/10/12
35	03119--CJ1		12101035	04/10/12
36	03120--CJ3		12101036	04/10/12

#	Ref	Description	Drawing#	Date
37	03121--CJ5		12101037	04/10/12
38	03122--CJ5S		12101038	04/10/12
39	03123--EJ7S		12101039	04/10/12
40	03124--EJ7		12101040	04/10/12
41	03125--HJ3		12101041	04/10/12
42	03126--HJ7		12101042	04/10/12
43	03127--H7M		12101043	04/10/12
44	03128--H9M		12101044	04/10/12
45	03129--M		12101045	04/10/12
46	03130--M3G		12101046	04/10/12
47	03131--H23AP		12101047	04/10/12
48	03132--AP		12101048	04/10/12



ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Page 1 of 1 Document ID:1UL5487-Z0110171557



Truss Fabricator: Anderson Truss Company
Job Identification: 12-071--Fill in later BRYAN ZECHER/VICTORES -- , **
Truss Count: 1
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

-Truss Design Engineer-
Douglas Fleming

1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	03102--H19BT		12101018	04/10/12

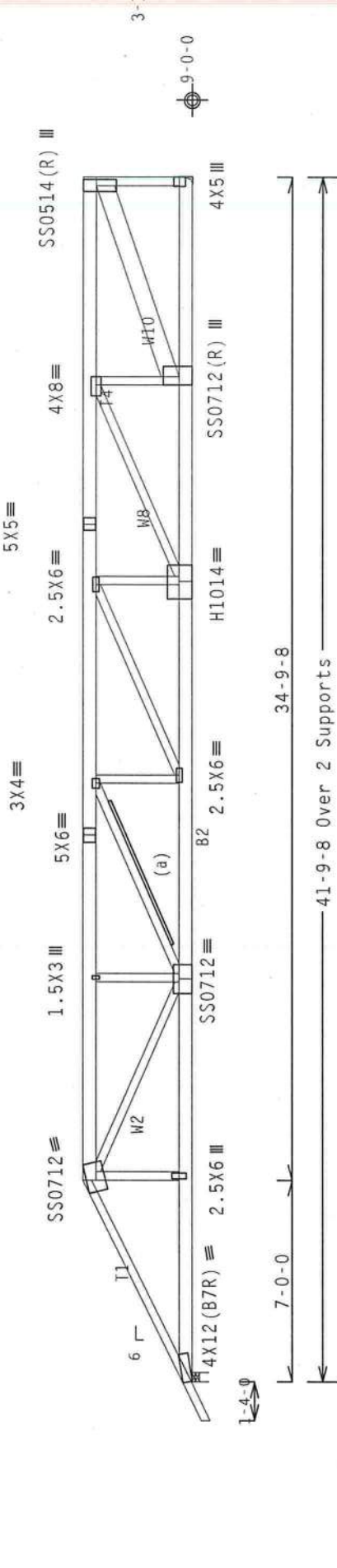
Top chord 2x6 SP M-26 :11 2x4 SP M-30:
:T4 2x6 SP #1 Dense:
Bot chord 2x6 SP #1 Dense :B2 2x6 SP SS:
Webs 2x4 SP #3 :W2, W8 2x4 SP M-30:
:W10 2x8 SP #1 Dense:

Roof overhang supports 2.00 psf soffit load.
Calculated horizontal deflection is 0.12" due to live load and 0.18" due to dead load.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.
#1 hip supports 7-0-0 jacks with no webs.

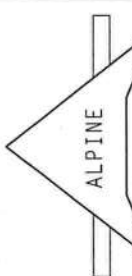
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.60" due to live load and 0.96" due to dead load at X = 21'-1-2.



R-3662 U-0 H-Simpson HGUS26
Supported Member Face: (8) 0.148"x3" nails
Supporting Member Face: (20) 0.148"x3" nails
Supporting Member: (2) 2x6 SP #2

PLT TYP. 20 Gauge HS.18 Gauge HS. Design Crit: FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0)



ALPINE
Haines City, FL 33844
FL COA #0278

PROFESSIONAL ENGINEER
Douglas Fleming
No. 66648
10-03-2010
10-03-2010

FLORIDA
STATE OF
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R487--	3085
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101001
BC LL	0.0 PSF	HC-ENG	TCE/DF	
TOT.LD.	40.0 PSF	SEQN-	62275	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) - (J) Hanger not calculated (2)2x6 SP #2 supporting member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WWFRS loads based on trusses located at least 7.50 ft. from roof edge.

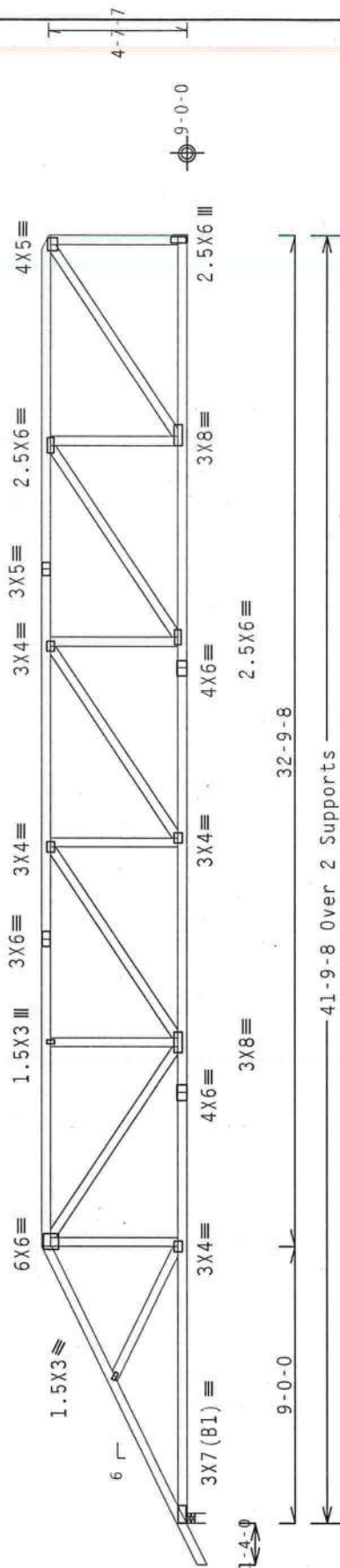
wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1817 U=1 W=4"
RL=85/-34

R-1712 U-4 H-Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (2) 2x6 SP #2

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-
Scale = .1875"/Ft.

TC LL	20.0 PSF	REF R487-- 3086
TC DL	10.0 PSF	DATE 04/10/12
BC DL	10.0 PSF	DRW HCUR487 12101002
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 62282
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UL5487 Z01

The seal is circular with a double-lined border. The outer ring contains the text "FLORIDA PROFESSIONAL ENGINEER" at the top and "STATE OF" at the bottom. In the center, the number "No. 66648" is printed vertically. To the right of the number, the expiration date "04/10/2012" is printed. The seal is stamped over a document that includes the text "I hereby certify that the above is a true and correct copy of the original as filed in my office." and "Witness my hand and the seal of my office this 10th day of April, 2012." The signature of the official is written over the seal.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**

****IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Please follow the latest edition of BCSI Building Components Safety Information, Part 1, which contains detailed instructions regarding the design, erection and bracing of trusses. Trusses must be installed as shown or as otherwise specified. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Conditions shown for permanent lateral restraint shall have bracing installed per BCSI sections 83, 87 or 910, as applicable.

The Building Components Group Inc. (BICG) shall not be responsible for any deviation from the design shown. The contractor shall be responsible for all bracing, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the details, unless noted otherwise. Refer to drawings 160A-2 For standard plate positions. A seal drawing or cover page listing truss design, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/APA 1 Sec.2. For more information see: The Building Components Group Inc., 114-BDO, wabuildg.com, IP:www.ipinst.org; MFCN; www.mfcncanada.com; Tel: 519-550-1534 ext. 100

ALPINE

MTW Building Components Group Inc.
Haines City, FL 33844
FL COA #0378

Top chord	2x4	SP	M-30
Bot chord	2x4	SP	M-30
webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

H – recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (2)2x6 SP #2 supporting member.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpi (+/-) = -0.18

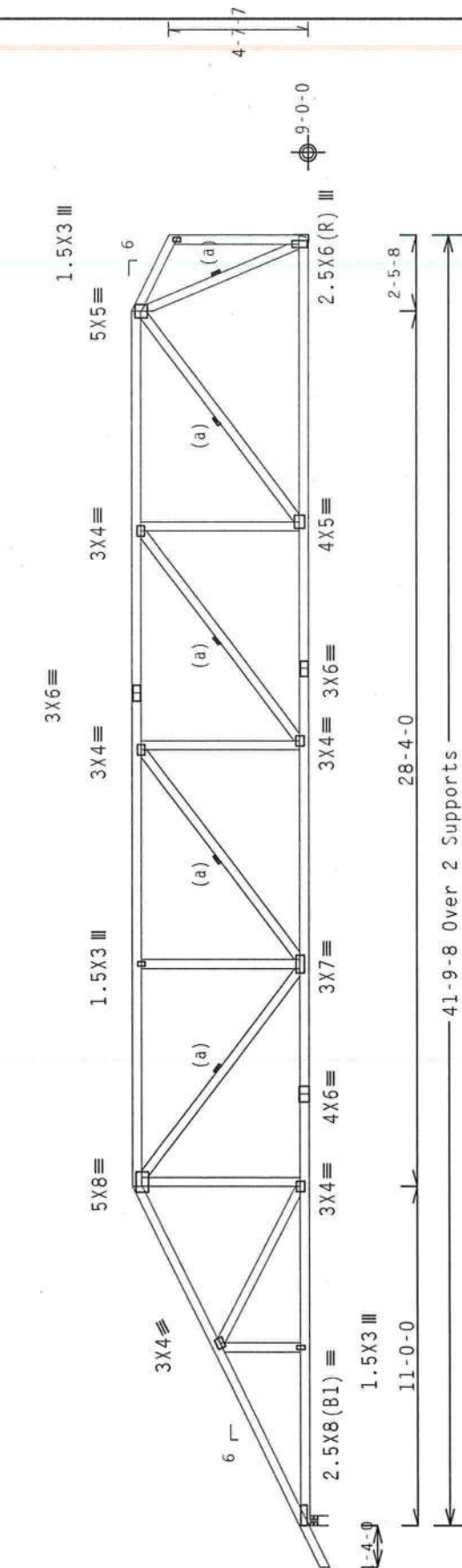
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located at minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1818 U=0 W=4"
RL-82/-53

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-
Scale = .1875"/Ft.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSP (Building Component Safety) Information, by IPPI and VITA for the latest practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint. No webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Job Order Details, unless noted otherwise. Refer to drawings 1609-2 for standard plate positions. A seal on drawing or cover plate listing this drawing, indicates acceptance of professional engineering responsibility by the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; IPPI: www.tpiint.org; VITA: www.vitausa.org; ICC: www.iccsafe.org

(12-071--Fill) in later BRYAN ZECHER/VICTORIES -- ** - H13A)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (2)2x6 SP #2 supporting member.

In lieu of structural panels use purlins to brace all flat TC @ 24"

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind loads and reactions based on MWFRS with additional C&C member design.

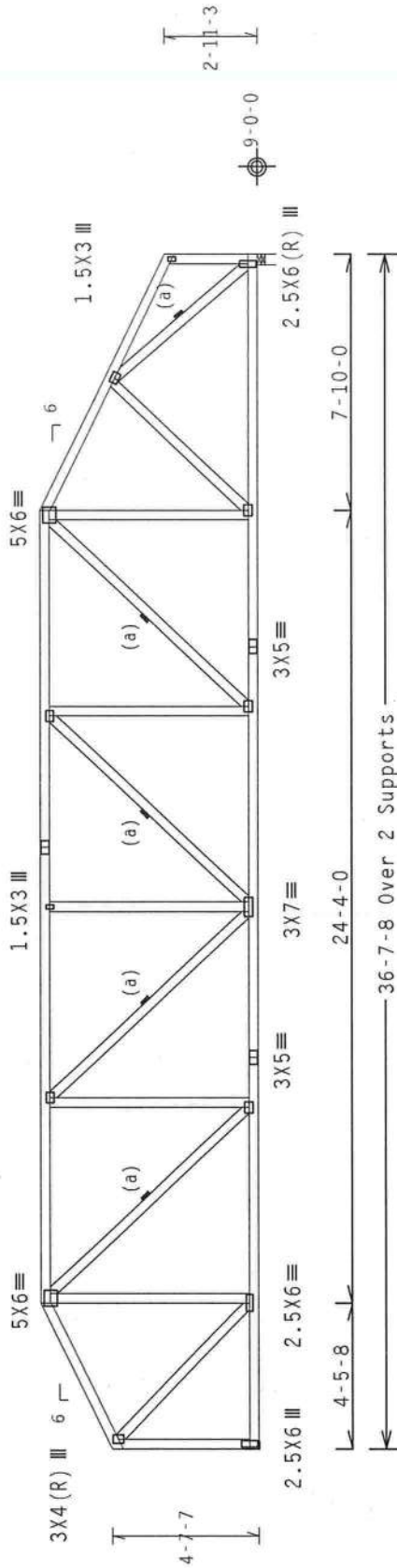
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

3X5=



R-1508 U-3

RL-41/-52 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Note: All Plates, etc. Except (2) 2x6 Hanger

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)

FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

R-1508 U-0 W-4"

FL/-4/-1-/R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

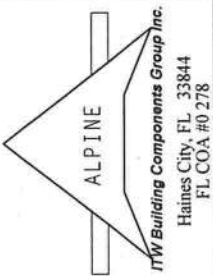
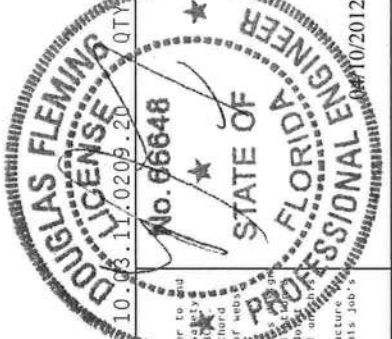
BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"



Top chord	2x4	SP	M-30
Bot chord	2x4	SP	M-30
Web	2x4	SP	#3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) - (J) Hanger not calculated (2)2x6 SP SS

Supporting member.	not calculated	(1) 2x6 SP	#1 Dense
(H2) - (J)			
Supporting member.			

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 16.21 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

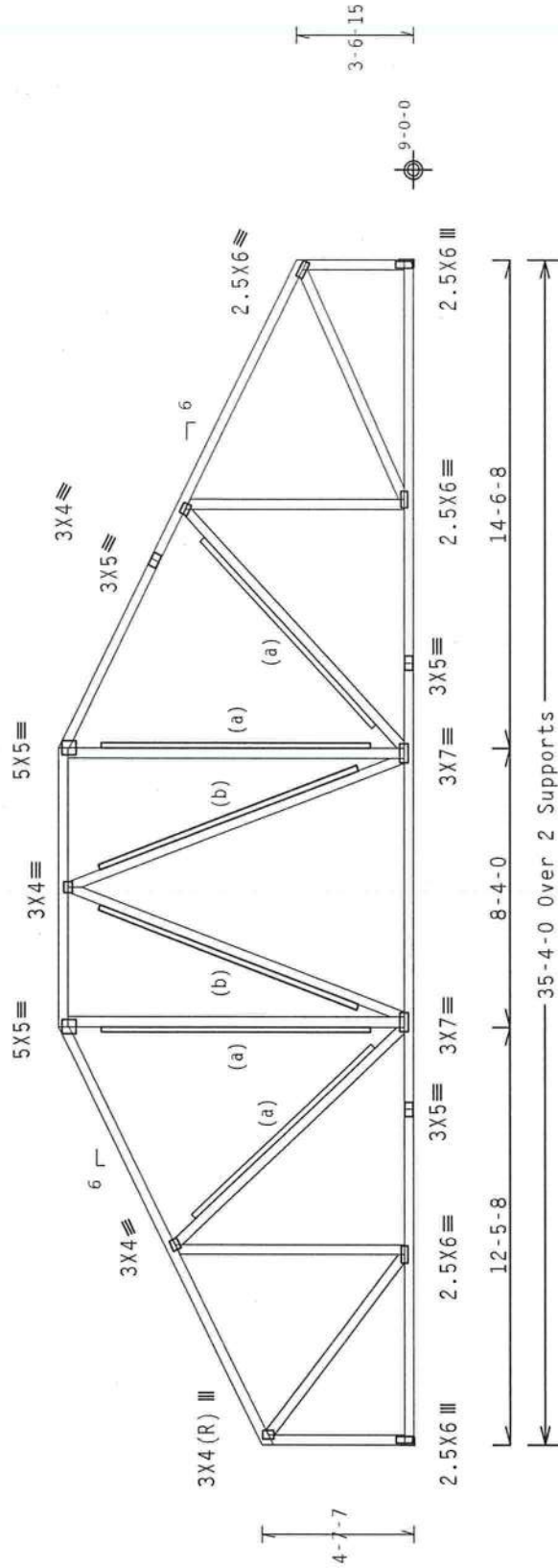
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) 2x4 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5" min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 16.21 ft. from roof edge.



R=1455 U=0

RL=99/-107 H=Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (14) 0.140
Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

3CZOLURES/IFT-2007
FT/RT=10%(0%) / 0(0)

1/0209/20

FL/-14/-1/-1R/-

Scale = .1875"/Ft.

ALPINE

UTW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

* **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
 FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trouses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following latest edition of BCSI (Building Component Safety Information, by IPI and MICA) for more information and practices prior to performing these tasks. Installers should provide temporary bracing per BCSI. The contractor shall have a properly attached rigid ceiling. Locations shall have bracing installed per BCSI sections. B31, B32 and B33, as applicable.

The Building Components Group Inc. (BUCOG) shall not be responsible for any deviation from this specification or drawings by the contractor. The contractor shall conform with ANSI/TPI-1 handling instructions for erecting and installing trusses, apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings I60A-2 for standard plate positions. A seal on truss endplate detailing or cover plate listing this drawing, indicates acceptance of professional engineering responsibility solely for this design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI-1 Sec.2. For more information see: This Job's Specification and BUCOG's website: www.bucog.com; TPI: www.tpi.net.org; NCCA: www.nccaindust.com; EC: ec@ncca.org

[illegible]

SPACING 24.0'

JREF- 1UL5487_Z01

(12-071--Fill) in later BRYAN ZECHER/VICTORES -- ** - HT17A)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (2)2x6 SP SS supporting member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MFERS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCp1(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

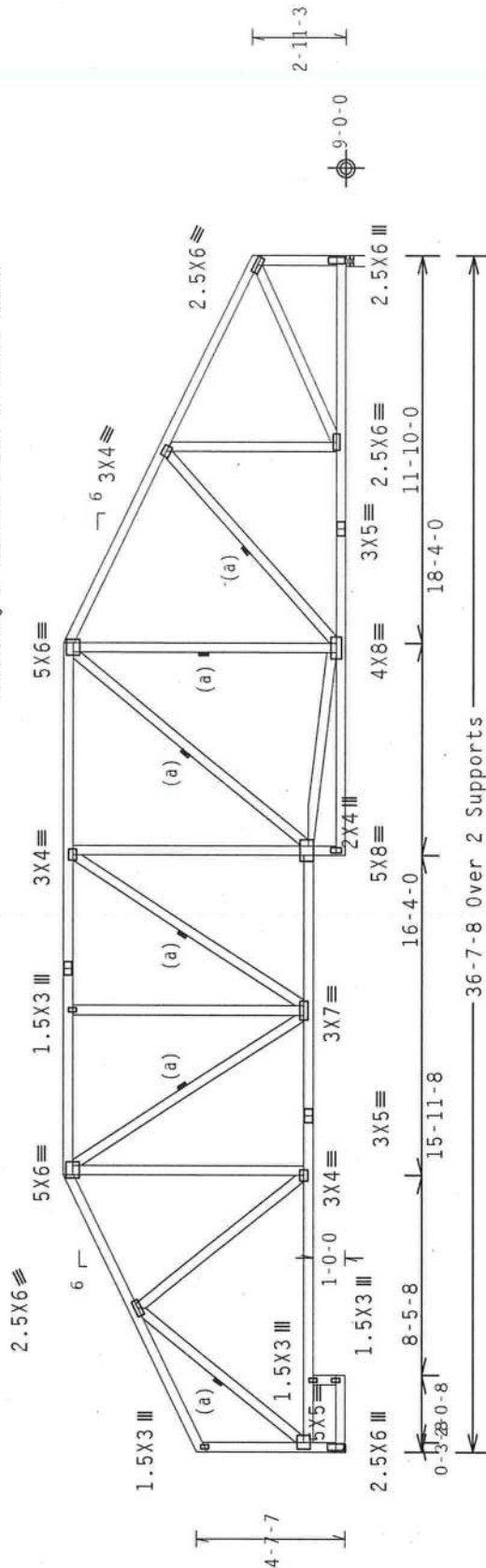
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.

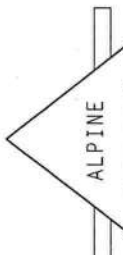


R-1508 U-0
RL-72/-83 H-Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (2) 2x6 SP #2
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

R-1508 U-0 W-4"

Scale = .1875"/Ft.

 ALPINE Haines City, FL 33844 FL COA #0278	Douglas Fleming Professional Engineer No. 66648 State of Florida 10-03-2009		REF R487-- 3091		
	DATE 04/10/12		TC LL 20.0 PSF		
	DRW HCUSR487 12101007		TC DL 10.0 PSF		
	HC-ENG TCE/DF		BC DL 10.0 PSF		
	SEQN- 62336		BC LL 0.0 PSF		
TOT.LD. 40.0 PSF			DUR.FAC. 1.25		
SPACING 24.0"			JREF- 1UL5487_Z01		

(12-071--Fill) in later BRYAN ZECHER/VICTORIES -- , ** - HT19A)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

120 mph wind, 15.39 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpl(+/-)-0.18

End verticals not exposed to wind pressure.

Wind loads and reactions based on MWFRS with additional C&C member design.

H - recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(H1) - (J) Hanger not calculated (2)2x6 SP SS supporting member.

(a) Continuous lateral bracing equally spaced on member.

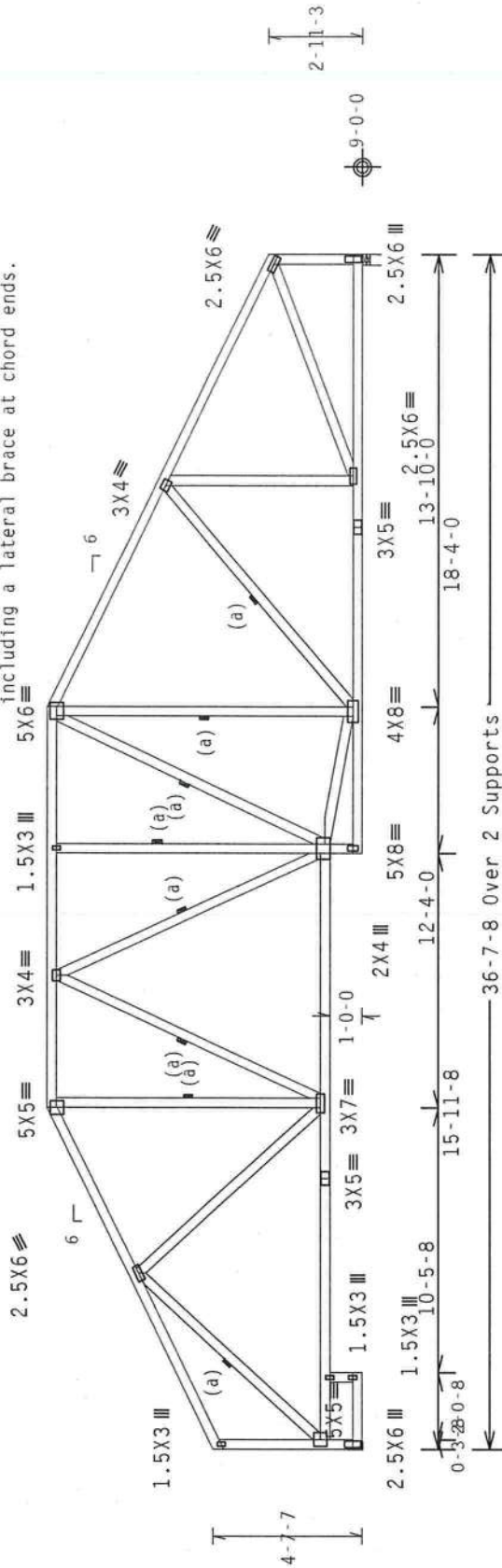
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 15.39 ft. from roof edge.

Laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.



R=1509 U-0

RL-87/-99 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

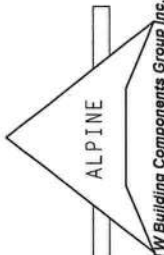
Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

R=1508 U-0 W-4"

Scale = .1875"/Ft.

 ALPINE Haines City, FL 33844 FL COA #0278 nTW Building Components Group Inc.	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.		DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 04/10/2012		TY: FL/-4/-/-R/-		REF R487 -- 3092		
	Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for details and practices prior to performing field connections. Trusses shall be braced in accordance with the BCSI manual. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.		10.00.11/0208.20		TC LL	20.0 PSF	DATE 04/10/12		
	any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. For more information see: This Job's TCE: www.tcesafe.org		10.00.11/0208.20		TC DL	10.0 PSF	DRW HCUSR487 12101008		
					BC DL	10.0 PSF	HC-ENG TCE/DF		
					BC LL	0.0 PSF	SEQN- 62352		
								TOT.LD.	40.0 PSF
							SPACING	24.0"	JREF- 1UL5487_Z01

(12-071--Fill) in later BRYAN ZECHER/VICTORES -- . ** - AT)

Top chord 2x4 SP H-30
Bot chord 2x4 SP H-30
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (U) Hanger not calculated (2)2x6 SP SS

(H2) = (U) Hanger not calculated (1)2x6 SP #1 Dense

supporting member.

(b) 2x6 #3 or better "I" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace Bottom Chord above filler at 24" OC, including a lateral brace at chord ends.

120 mph wind, 16.21 ft mean hgt, ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCPI(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

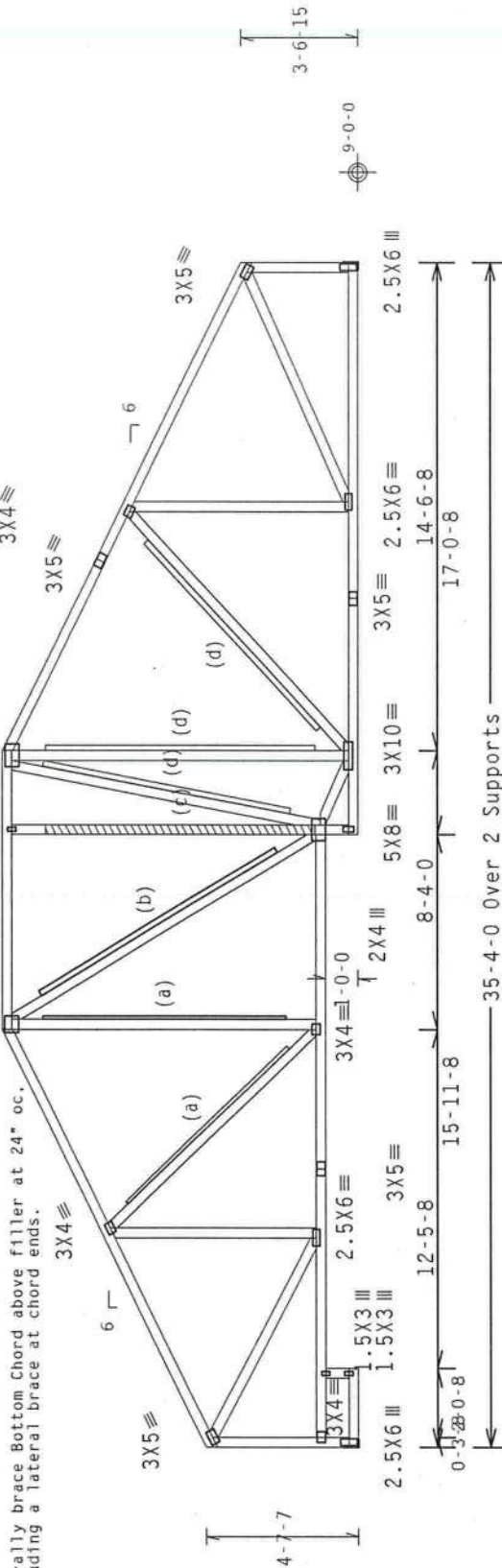
(a) 1x4 #3SR8 SPF-S or better "I" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(d) 2x4 #3 or better "I" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

(c) #3 or better scab brace, same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42'-high x 24'-wide clearance.

MWFRS loads based on trusses located at least 16.21 ft. from roof edge.



R-1637 U-07

RL-99/-107 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

R-1621 U-0 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (1) 2x6 SP #2

PLT TYP. Wave

Scale = .1875"/Ft.

FL/-/4/-/-/R/-

REF R487 -- 3094

DATE 04/10/12

DRW HCUSR487 12101010

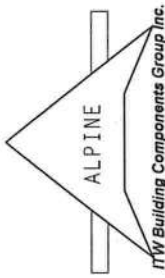
HC-ENG TCE/DF

SEQN- 62393

DUR.FAC. 1.25

SPACING 24.0"

JREF- 1UL5487_Z01



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL COMPANIES INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information, by TPI and WCA for details and instructions. Trusses shall be installed in accordance with the instructions and details provided. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 100-2 for standard plate positions. A seal of the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. ITWBCG: www.itwbcg.com; TPI: www.tpiinc.org; WCA: www.wcaindustry.com; ECC: www.eccsafe.org

(12-071--Fill) in later BRYAN ZECHER/VICTORES -- , ** - H7B)

Top chord 2x6 SP #2 :T1 2x4 SP M-30:
Bot chord 2x6 SP #2 :B1 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W3, W9 2x4 SP M-30:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 6Cpf(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang.
End jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang.
Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

* Negative reaction(s) of -634# MAX. (See below) from a non-wind load case requires uplift connection.

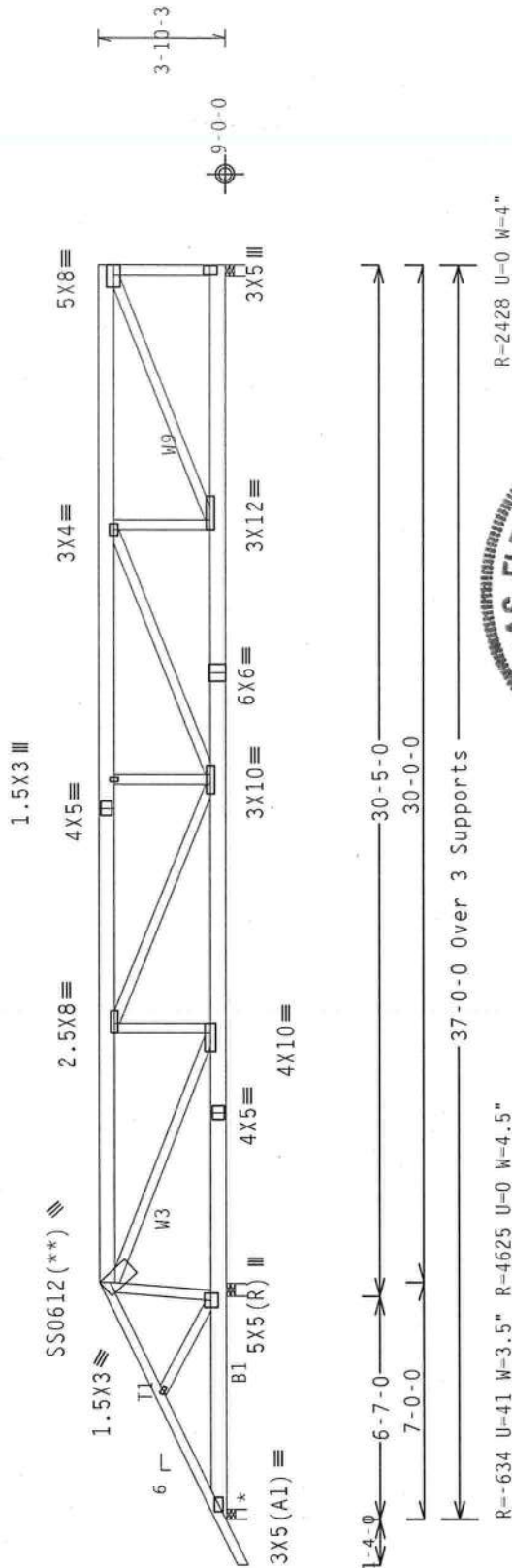
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. 18 Gauge HS Wave

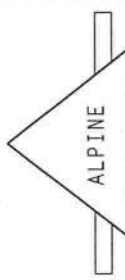
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875" / Ft.

REF	R487--	3095
DATE	04/10/12	
DRW	HCSR487	12101011
HC-ENG	TCE/DF	
SEQN	62088	
DUR.FAC.	1.25	
SPACING	24.0"	
JREF	1UL5487_Z01	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****HPORE/INT**** FURNISH THIS DESIGN TO ALL COMPARTMENTS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by IPI and WICA) for details and practices prior to performing the installation. The truss manufacturer shall be responsible for the design and shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1, Sec. 6. For more information see: This job's ICG: www.icg-safe.org



ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord	2x4	SP	M-30
Bot chord	2x4	SP	M-30
webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS Toads based on trusses located at least 7.50 ft. from roof edge.

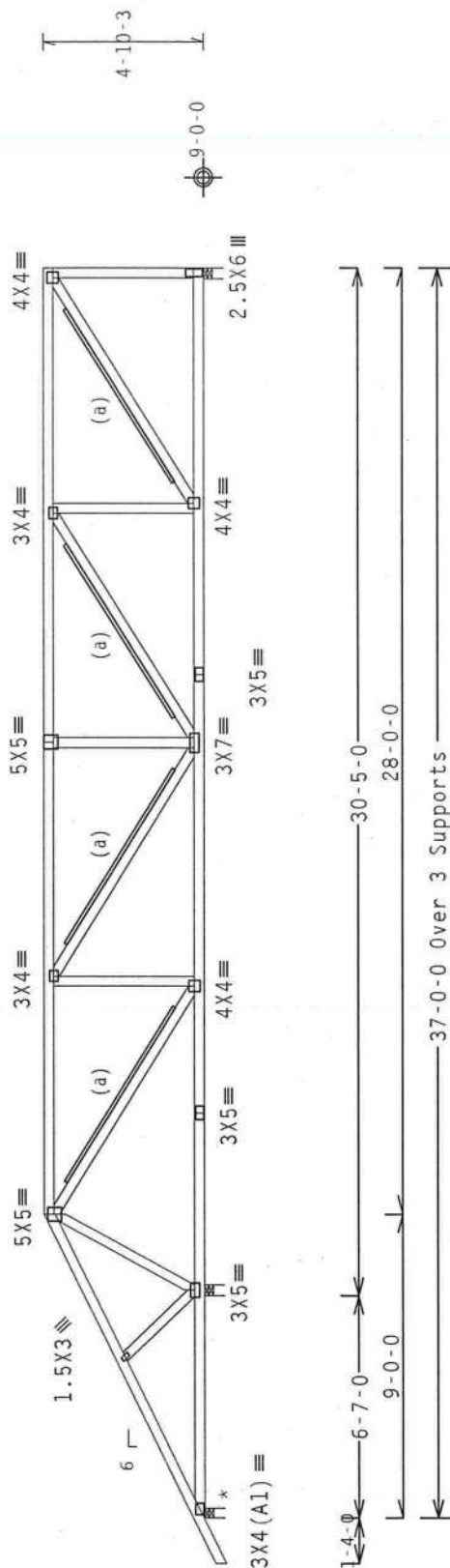
* Negative reaction(s) of -245# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=141/-245 U=68 W=3.5"
RL=85/-32 R=2

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .1875" / Ft.

REF	R487--	3096
DATE	04/10/12	
PRW	HCUSR487	12101012
HC-ENG	TCE/DF	
EEQN-	62104	
REF-	1UL5487	Z01

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.L.D.	40.0 PSF
DOUR.FAC.	1.25
SPACING	24.0"

NO. 66848

STATE OF FLORIDA

PROFESSIONAL ENGINEER

04/10/2012

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of the Building Components Safety Manual for details. Temporary bracing per the notes noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of the truss shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

THE BUILDING COMPONENTS GROUP, INC. (LHQBQS) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/P1.1, or for handling, shipping, installing or placing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Detail. Trusses shall be installed in accordance with the design and bracing shown on this drawing. Responsibility for the design shown, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/P1.1 sec.2. For more information see: The general notes page: LHQBQS-w-lhqbqs.com; LH/P1.1/ansi.org; MCA: www.sdsindustry.com

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	M-30
Bot	chord	2x4	SP	M-30
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "J" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5" min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WFRS loads based on trusses located at least 7.50 ft. from roof edge.

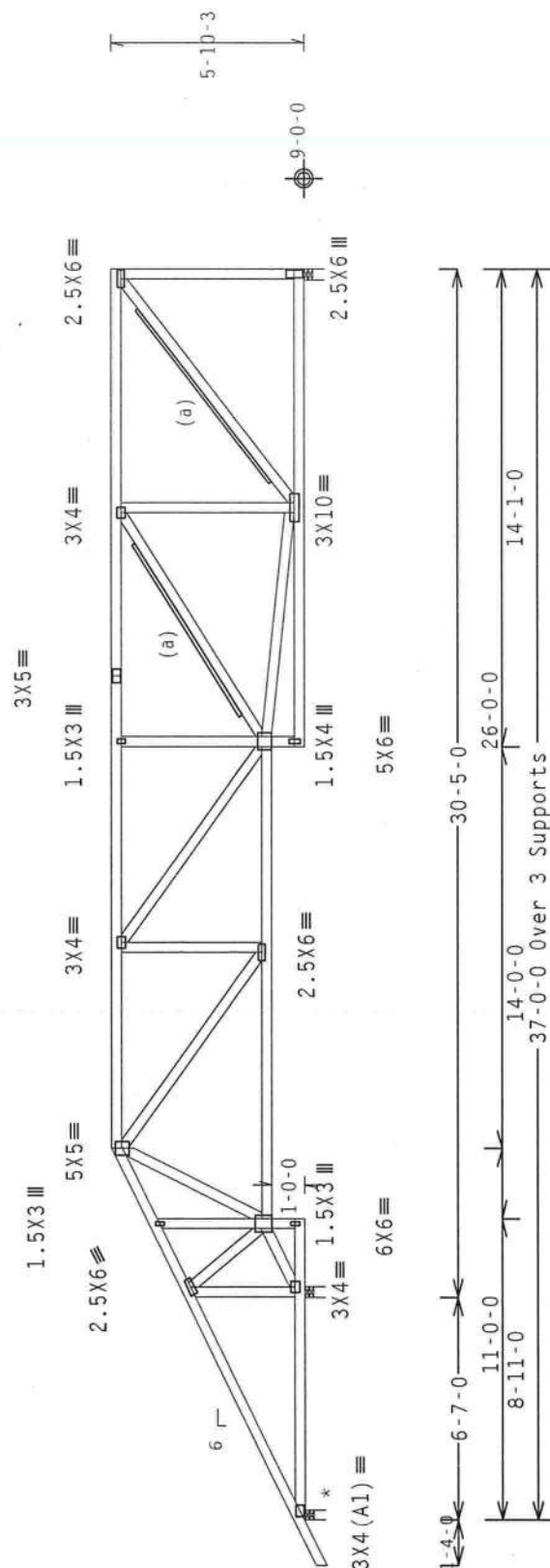
* Negative reaction(s) of -252# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpt (+/-) -0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-183/-252 U=52 W=3.5"
R-2032 U=20 W=4"
RL-102/-37

Design Crit: FBC2010Res/TPI-2007 (STD)
 $FT/RT=10\%(0\%)/0(0\%)$

PLT TYP. Wave

Scale = .1875"/Ft.

ALPINE

nTW Building Components Group Inc.

Haines City, FL 33844

Tel. 888-444-7288

04/10/2012

04/10/2012

04/10/2012

04/10/2012

04/10/2012

04/10/2012

04/10/2012

04/10/2012

04/10/2012

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.		No. 66648		TC LL	20.0 PSF	REF R487 --	3098
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.		★		TC DL	10.0 PSF	DATE	04/10/12
Trusses require extreme care in fabricating, handling, shipping, installation and bracing. Refer to the latest edition of BCSP (Building Component Safety) Information, by API and WCLCA for more information regarding the design and construction of trusses. All trusses shall be installed according to the instructions provided. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.		★		BC DL	10.0 PSF	DRW	HCU5R487 12101014
nTW Building Components Group Inc. (nTWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/APA T & E, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the following details, unless noted otherwise. Refer to drawings 1609-2 for standard plate positions. A seal on the drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/APA T & E Sec.2. For more information see: nTWBCG website: www.nTWbcg.com; Email: info@ntwbcg.com; nTWCA: www.nTWcaindustry.com; ICS: www.ICSUSA.org		STATE OF FLORIDA PROFESSIONAL ENGINEER		BC LL	0.0 PSF	HC-ENG	TCE/DF
				TOT.LD.	40.0 PSF	SEQN-	62112
				DUR.FAC.	1.25		
				SPACING	24.0"	JREF-	IUL5487_Z01
							04/10/2012

(12-071--Fill) in later BRYAN ZECHER/VICTORES -- , ** - H17BT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(b) 2x4 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

Laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpt (+/-)=0.18

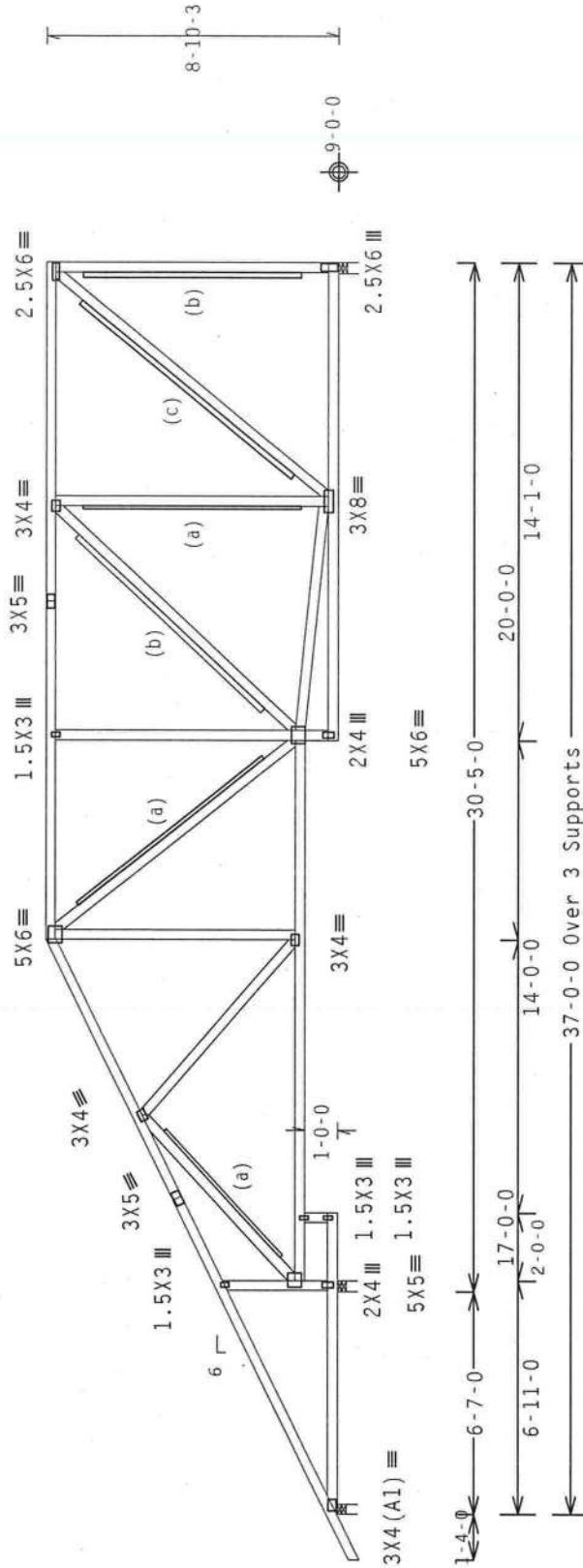
Wind loads and reactions based on MMFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(c) 2x6 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-335 U=0 W=3.5"
RL=101/-50

R=1569 U=0 W=4"

R-1236 U=0 W=4"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487--	3101
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101017
BC LL	0.0 PSF	HC-ENG	TCE/DF	
TOT.L.D.	40.0 PSF	SEQN	62483	
DUR.FAC.	1.25			
SPACING	24.0"	JREF	1UL5487_Z01	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by IPI and UICA) for details. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

LTM Building Components Group Inc. (LTMBCG) shall not be responsible for any deviation from the truss bracing of trusses. Apply plates to each face of truss and position as shown above and on the Job. Details, unless noted otherwise. Refer to drawings 180A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. For more information, contact: LTMBCG, Inc. 1111-800-1111 or 1111-800-1111; www.ltmbsg.com; IPI: www.tpiinst.org; UICA: www.shedindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(b) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(a) 2x4 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

(c) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Laterally brace Bottom Chord above filler at 24" oc, including a lateral brace at chord ends.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 6Cp1 (+/-)-0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

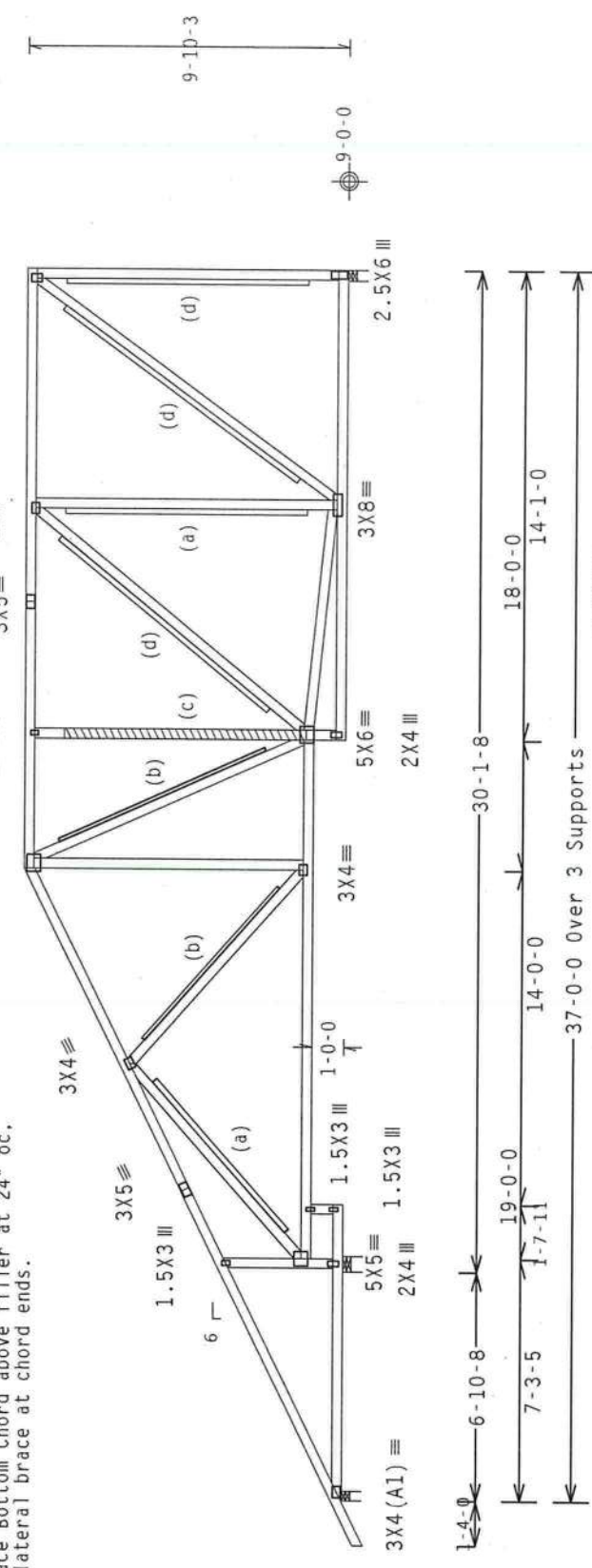
Right end vertical not exposed to wind pressure.

(d) 2x6 #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

5X6 = 1.5X3 III 3X5 = 3X4 = 3X4 (R) III



R-344 U-0 W-3.5"
RL-168/-54

R-1575 U-30 W-5.657"

R-1220 U-16 W-4"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.



TC LL	20.0 PSF	FL/-4/-/-/R/-	REF	R487--	3102
TC DL	10.0 PSF		DATE	04/10/12	
BC DL	10.0 PSF		DRW	HCUSR487	12101018
BC LL	0.0 PSF		HC-ENG	TCE/DF	
TOT.LD.	40.0 PSF		SEQN-	3045	REV
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	1UL5487_Z01	

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following instructions for safety information, by TPI and NICA for details on proper installation practices prior to performing these functions. Install lateral bracing in accordance with the instructions noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this drawing or truss design. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on the drawing cover page listing this drawing, indicates acceptance of professional engineering. The responsibility of the Building Designer per ANSI/TPI 1.5-2.2. This job is the property of ITWBCG. For general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; NICA: www.nicaindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

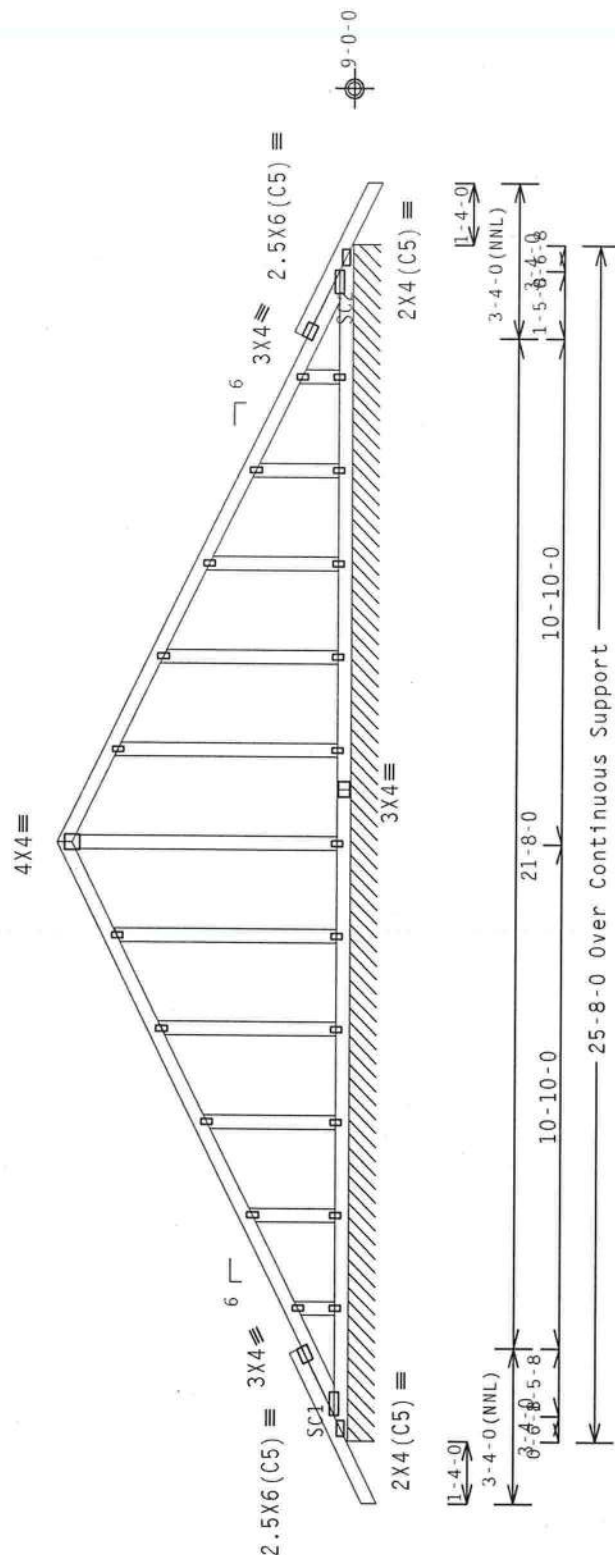
In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Wind loads and reactions based on MWFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



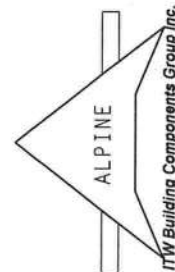
R-122 PLF U=0 PLF W=25-8-0
RL-4/-4 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT-10%(0%) / 0 (0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to all applicable codes and standards for proper bracing. The following are some of the minimum practices to be followed. These practices are not intended to be all-inclusive and are not intended to supersede any applicable codes and standards. The user is responsible for determining the applicability of these practices to the specific project. The user is responsible for obtaining the necessary permits and approvals for the project. The user is responsible for obtaining the necessary insurance coverage for the project. The user is responsible for obtaining the necessary safety training for the personnel involved in the project. The user is responsible for obtaining the necessary safety equipment for the project. The user is responsible for obtaining the necessary safety signage for the project. The user is responsible for obtaining the necessary safety barriers for the project. The user is responsible for obtaining the necessary safety lighting for the project. The user is responsible for obtaining the necessary safety communication system for the project. The user is responsible for obtaining the necessary safety training for the personnel involved in the project. The user is responsible for obtaining the necessary safety equipment for the project. The user is responsible for obtaining the necessary safety signage for the project. The user is responsible for obtaining the necessary safety barriers for the project. The user is responsible for obtaining the necessary safety lighting for the project. The user is responsible for obtaining the necessary safety communication system for the project.



Haines City, FL 33844
FL COA #0 278

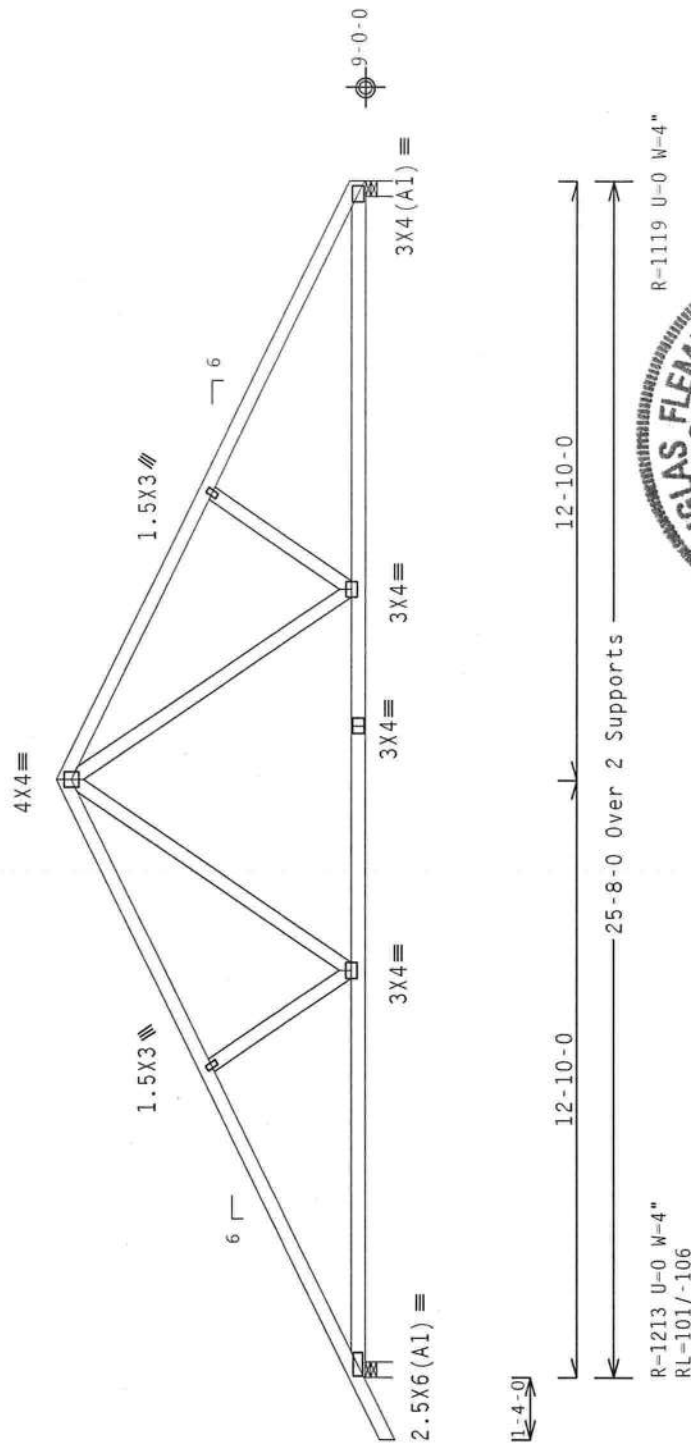
FL/-/4/-/-/R/-		Scale = .25" /Ft.
TC LL	20.0 PSF	REF R487-- 3103
TC DL	10.0 PSF	DATE 04/10/12
BC DL	10.0 PSF	DRW HCUSR487 12101019
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 62150
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UL5487 Z01

1120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpf (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1213 U=0 W=4"
RL=101/-106

Design Crit: FBC2010Res/TPI - 2007 (STD)
FT/RT=10% (0%) / 0 (0)

PLT TYP. Wave

$$FT/RT = 10\% (0\%) / 0 (0)$$

Scale = .25"/Ft.

***WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

OFF R487-- 3104

[illegible]

DATE	04/10/12
ORW	HCUSR487 1210102

[illegible]

IC-ENG DF/DF	
EQN-	62157
REF-	1UL5487 20

TTW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

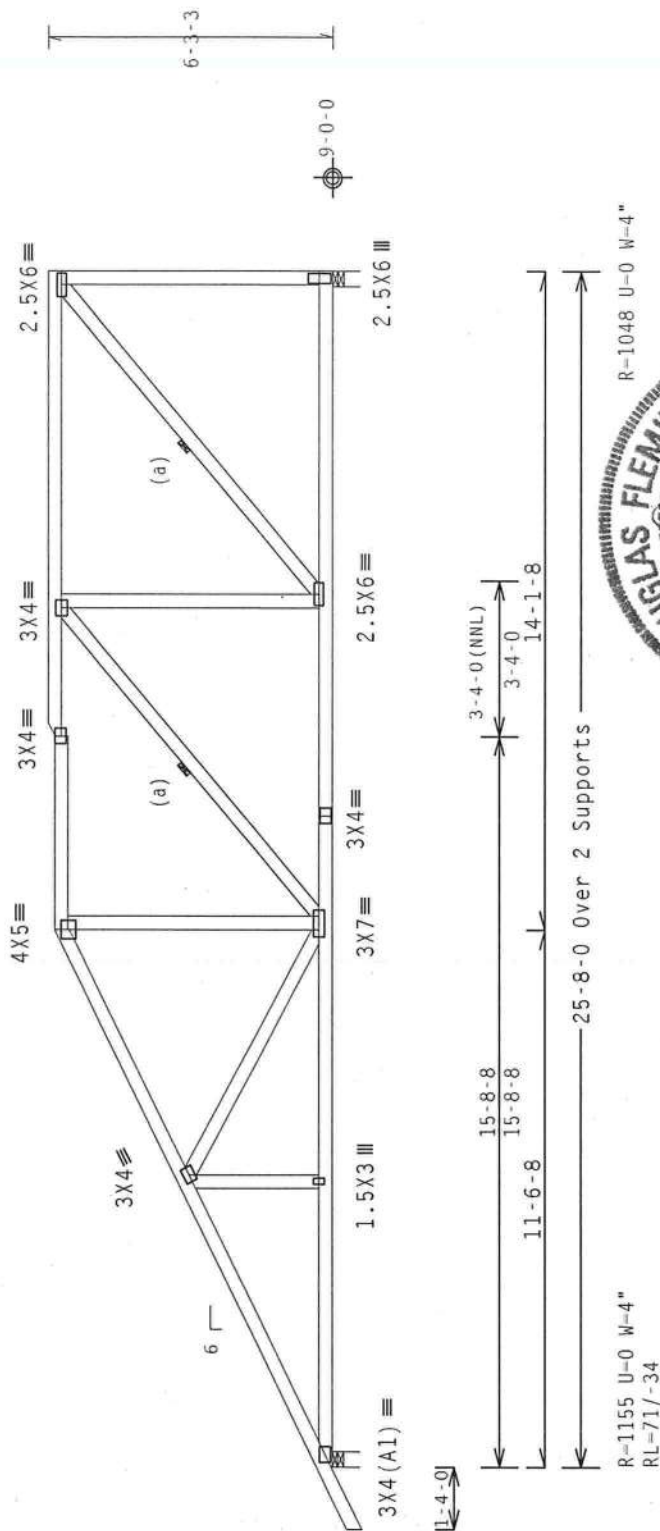
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpt (+/-)=-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



R-1155 U-0 W-4"
RL-71/-34

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

Scale = .25"/Ft.

FL/-/4/-/-/R/-

10.05.11.0209.20

FT/RT=10%(0%) / 0(0)

—
•
5
—
5
= 79
—
7
2
5

PLT TYP. Wave

ALPINE

nTW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

STATE OF FLORIDA

PROFESSIONAL ENGINEER

No. 06648

DATE: 04/10/2012

REF: R487 -- 3105

DATE: 04/10/12

DRW: HCUSR487 12101021

HC-ENG DF/DF

SEQN- 62167

JREF- 1UL5487_Z01

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
 FURNISH THIS DESIGN TO ALL COMPANIES INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following the latest edition of BCSP (Building Component Safety Information, by TPI and NCCA) for best practices prior to performing these functions. Installers shall provide temporary bracing per NCCA. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of truss shall have bracing installed per BCSP sections 83, 87 or 810, as applicable.

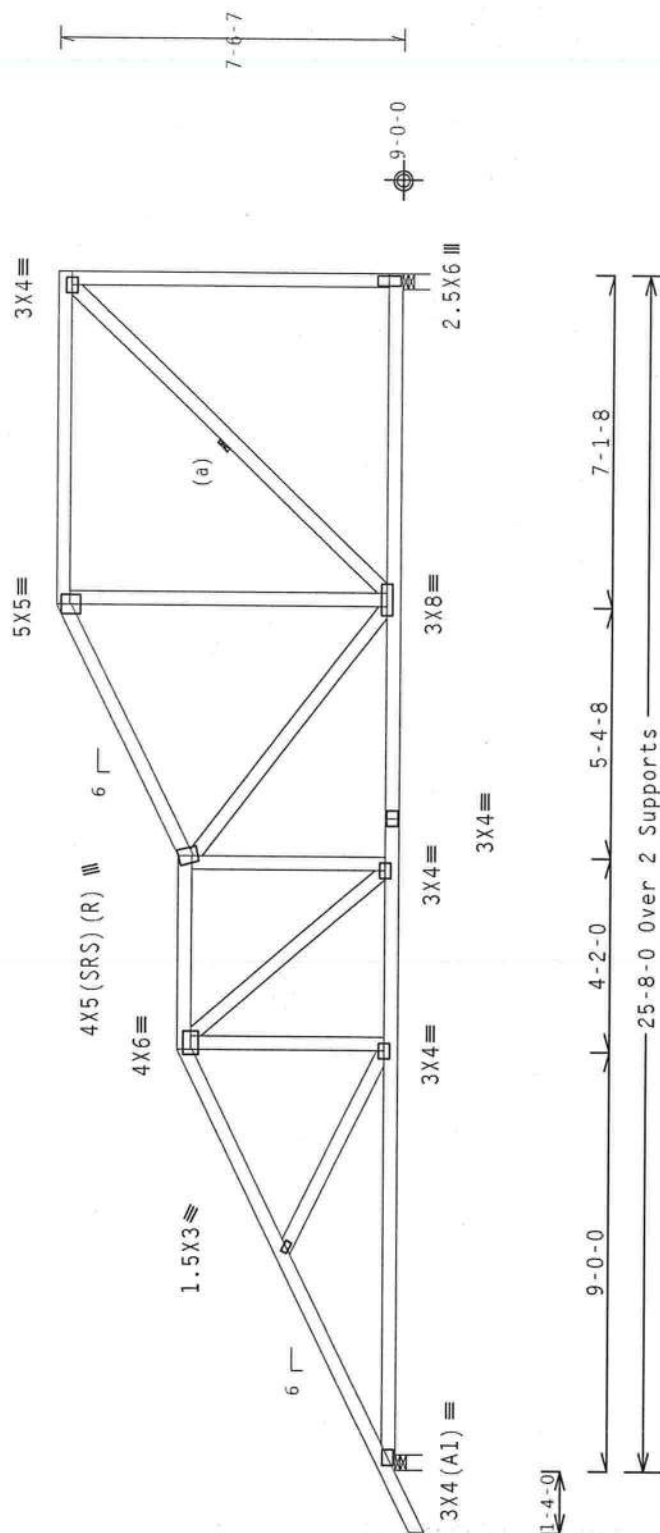
nTW Building Components Group Inc. (nTWBCG) shall not be responsible for any deviation from this drawing or cover page listing this drawing. Indicates acceptance of professional engineering structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This job's general notes page: 11N-BCG; www.11nbcg.com; TPI: www.tpiinst.org; NCCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	
SPACING	24.0"	

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1155 U=0 W=4"
RL-86/-39

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

11. $25 \times 67 = 1675$

SCALE = .25 / FL.	
REF R487 -- 3107	
DATE 04/10/12	
DRW HCUR487 12101023	
HC-ENG DF/DF	
SEQN- 62184	
JREF- 1UL5487 Z01	

[illegible]

ALPTNF

TW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

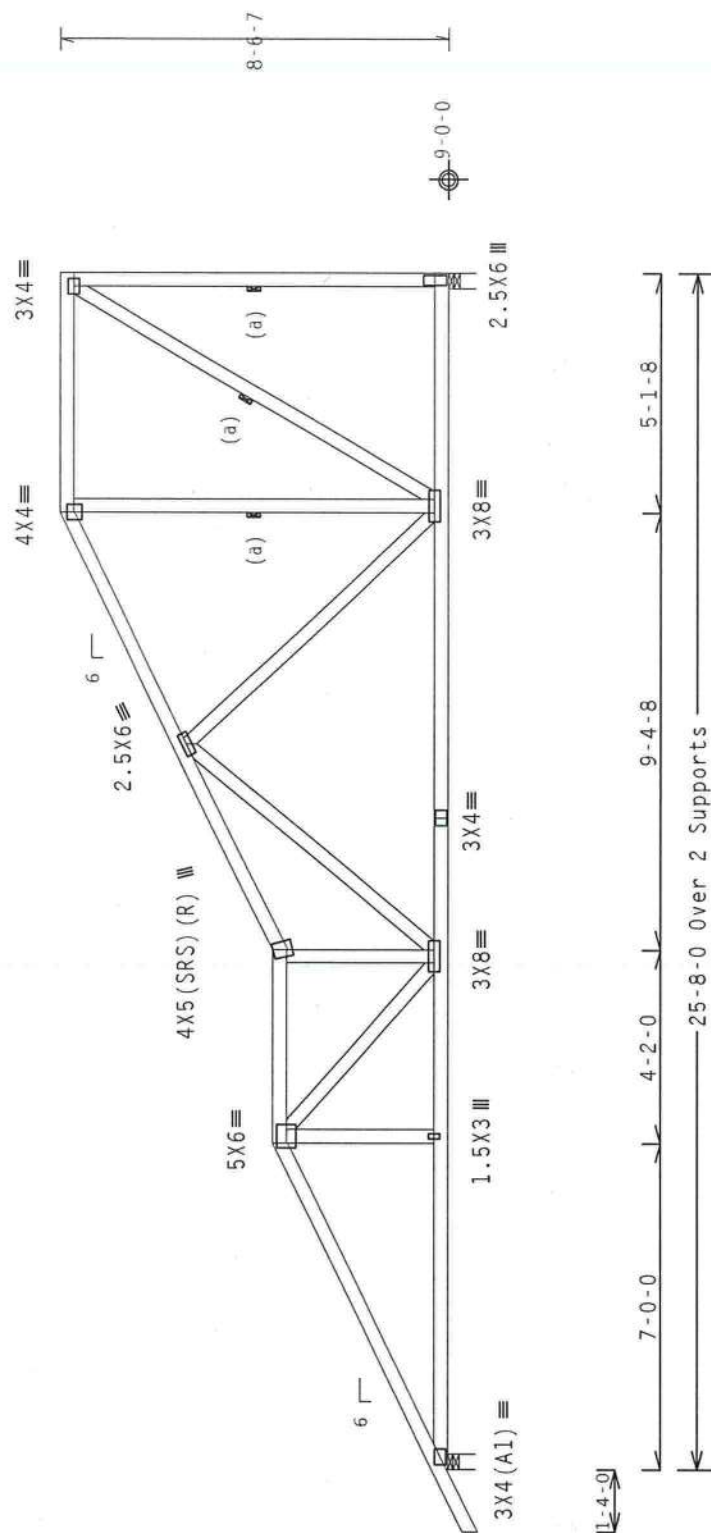
1120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpt (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



R-1155 U-0 W-4"
RL-97/-42

R-1048 U-0 W-4"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .25"/Ft.

FL/-/4/-/-/R/-

REF R487-- 3108

TC LL	20.0 P
-------	--------

DATE 04/10/12

TC DL 10.0 P

MR. H. C. H. 1210102

BC	DI	10	0	P

2010121 HCU3K487 MNA

[illegible]

IC-ENG UF/UF

$$\frac{0.0}{77.29}$$

62192

40.0 P

DUR.FAC. 1.25

REF- 1UL5487_Z0:

SPACING 24.0"

Top	chord	2x4	SP	M-30
Bot	chord	2x4	SP	M-30
	Webs	2x4	SP	#3

1120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpt (+/-)=0.18

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

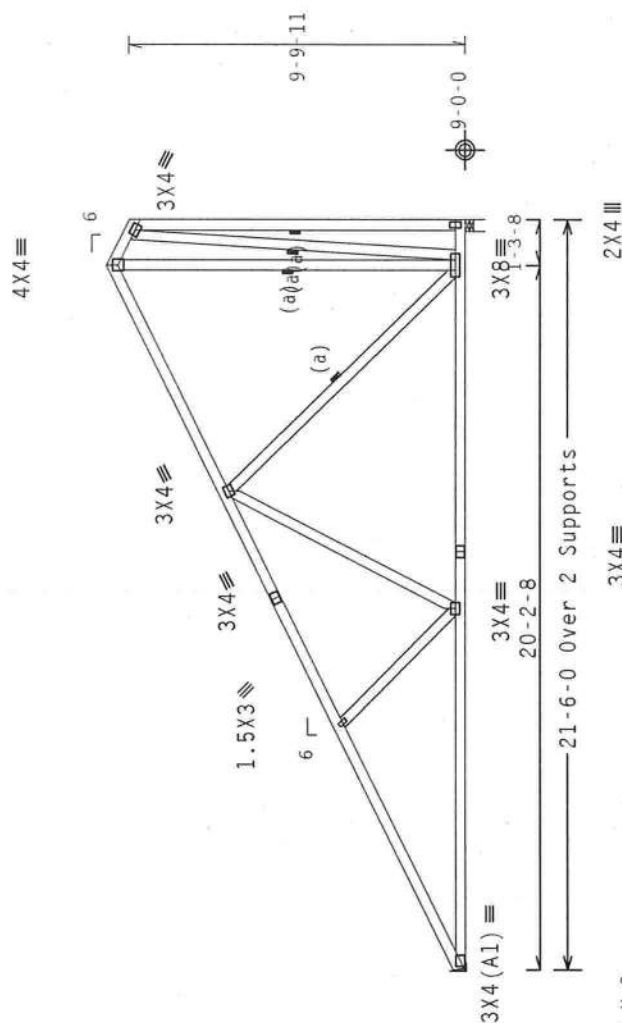
wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(H1) = (J) Hanger not calculated (1)2x6 SP #2 supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R-891 U=0 H-Simpson LUS24
 RRL-113/-44 Supported Member Face: (2)
 Supporting Member Face: (4)
 Supporting Member: (1) 2x4 SP #2 Dense

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

$$\text{Scale} = .1875"/\text{Ft.}$$

FL/-/4/-/-/R/-

0209.20

0.05%.

10

FT/RT=10%(0%)/0(0)

designer client:

PLT TYP. Wave

* *WARNING* * READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
* *IMPORTANT* *

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP Building Component Safety Information, by IP1 and WICA for more information.

practices prior to performing these functions. Installers shall provide temporary bracing per B3.1. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord

shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this specification, or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation.

metals, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the bottom or cover may be required to protect the interior of the truss. Indicate acceptance of professional engineering

responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

general notes page; ITH-SCG: www.ithscg.com; IPI: www.ipihst.org; KICA: www.sbcindustry.com; CC: www.1ccsafe.org

(12-071--Fill in later BRYAN ZECHER/VICTORES --, ** - H3C)

Top chord 2x4 SP M-30 :T5 2x4 SP #1:

Bot chord 2x4 SP M-30

Webs 2x4 SP #3 :W6 2x4 SP M-30:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

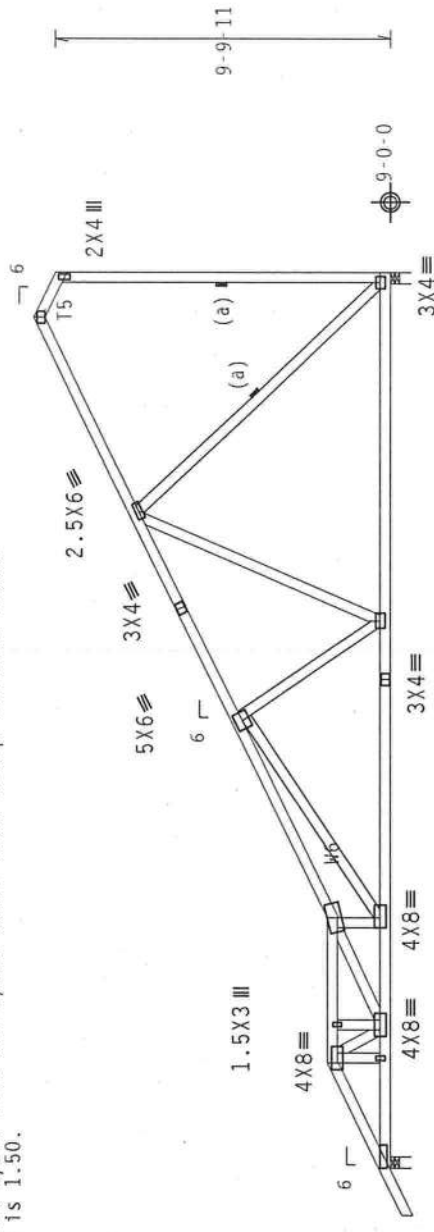
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads

----- (Lumber)
Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25
TC- From 62 plf at -1.33 to 62 plf at 3.00
TC- From 62 plf at 3.00 to 62 plf at 7.17
TC- From 62 plf at 7.17 to 62 plf at 24.38
TC- From 62 plf at 24.38 to 62 plf at 25.67
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 10 plf at 0.00 to 20 plf at 3.03
BC- From 20 plf at 3.03 to 10 plf at 4.10
BC- From 20 plf at 4.10 to 20 plf at 25.67
TC- 107.36 lb Conc. Load at 3.03
BC- 120.99 lb Conc. Load at 3.03
BC- 403.24 lb Conc. Load at 4.10

Right end vertical not exposed to wind pressure.

3X4=



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10% (0%) / 0 (0)

Scale = .1875" / Ft.

REF	R487--	3111
DATE	04/10/12	
DRW	HCUSR487	12101027
HC-ENG	DF/DF	
SEQN-	62255	
DUR.FAC.	1.25	
SPACING	24.0"	



****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following latest edition of the Building Components Group Inc. (BCGI) Truss Installation Manual for details and instructions. The truss manufacturer shall be responsible for the design and construction of the truss. The truss manufacturer shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCGI sections B3, B7 or B10, as applicable.
The Building Components Group Inc. (BCGI) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. For more information, contact BCGI at 1-800-368-7663 or visit our website at www.bcgigroup.com. ICC: www.iccinc.org



ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(12-071--Fill) in later BRYAN ZECHER/VICTORES --, ** - D)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Left cantilever is exposed to wind

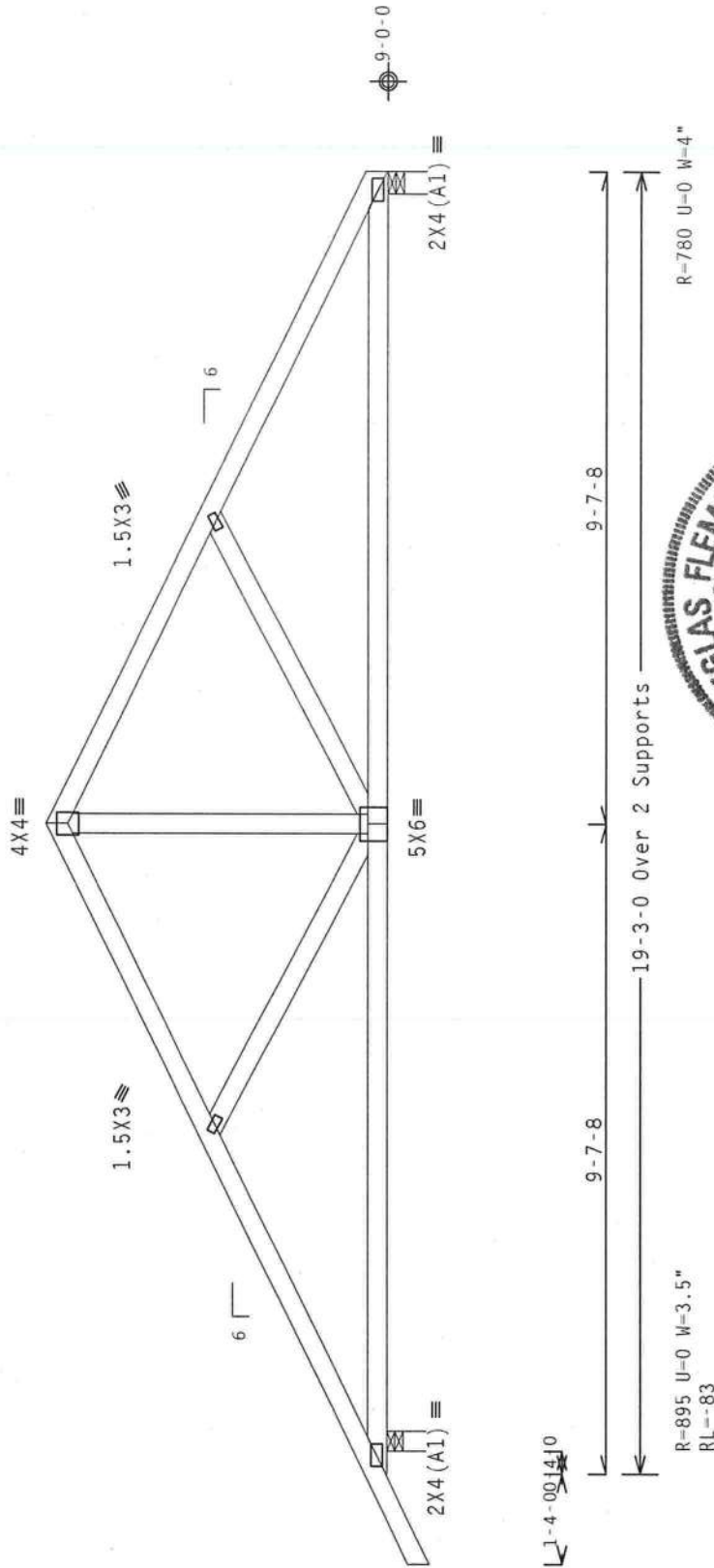
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpl(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



PLT TYP. Wave
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10% (0%) / 0 (0)

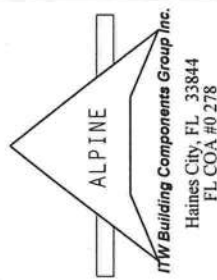
Scale = .375" / Ft.

TC LL	20.0 PSF	FL / - / 4 / - / - / R / -
TC DL	10.0 PSF	
BC DL	10.0 PSF	
BC LL	0.0 PSF	
TOT.LD.	40.0 PSF	
DUR.FAC.	1.25	
SPACING	24.0"	

REF	R487--	3112
DATE	04/10/12	
DRW	HCSR487	12101028
HC-ENG	DF/DF	
SEQN-	62115	
JREF-	1UL5487_Z01	



****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following notes for details and practices prior to performing these functions. Installers shall provide temporary bracing per the notes shown. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCS sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joining Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this design, indicates acceptance of professional engineering responsibility for this design. This design shall not be used for any structure unless it is specifically noted on the drawing. For more information, contact ITWBCG at 1-800-368-7777 or www.itwbcg.com. This job's general notes page: ITN-BGS: www.160deg.com; TPI: www.tpinet.org; NCA: www.sbcindustry.com; ICC: www.iccsafe.org



(12-071--Fill in later BRYAN ZECHER/VICTORES --, ** - DGE)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Hebs 2x4 SP #3

:Stack Chord SC1 2x4 SP M-30::Stack Chord SC2 2x4 SP M-30:

Roof overhang supports 2.00 psf soffit load.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

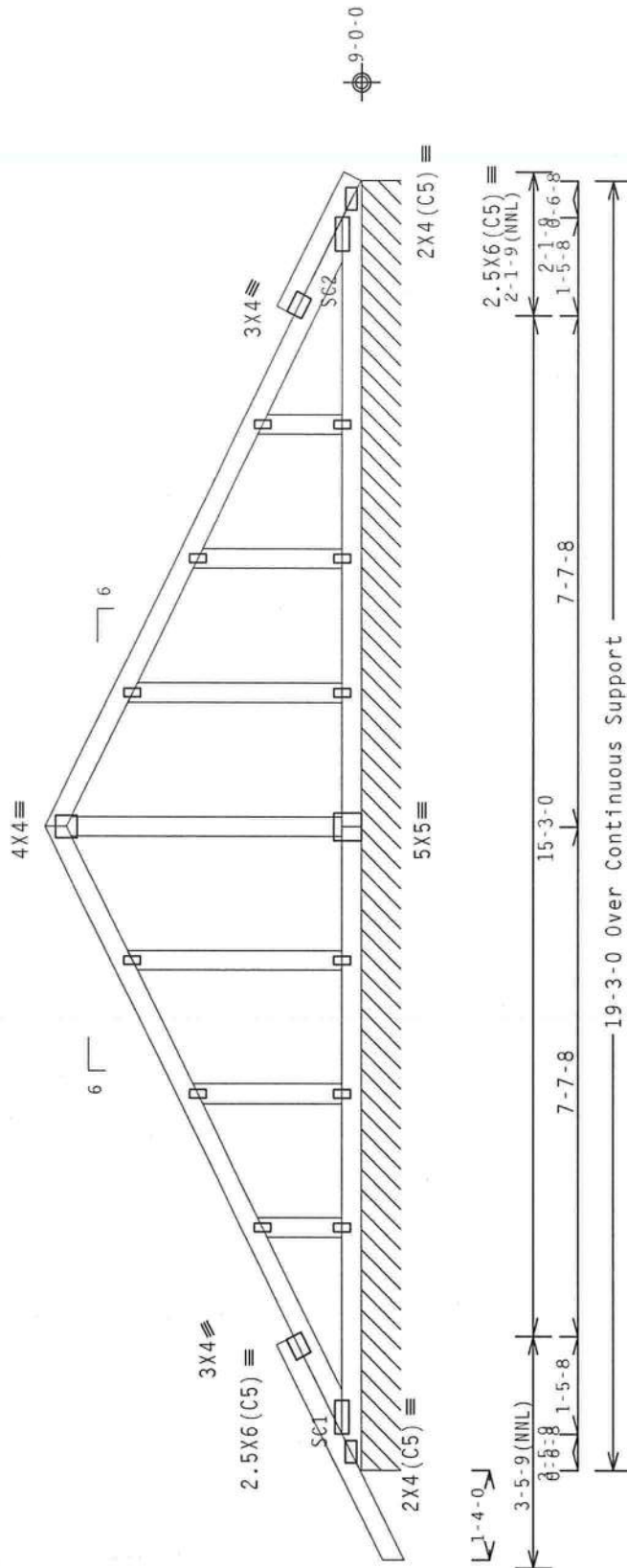
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpf(+/-)=0.18

Wind loads and reactions based on MNFRS with additional C&C member design.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.



R-112 PLF U-0 PLF W-19-3-0
RL-4/-4 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10% (0%) / 0 (0)

PLT TYP. Wave

FL/-4/-1-R/-

Scale = .375" / Ft.

REF	R487--	3113
DATE	04/10/12	
DRW	HGUSR487	12101029
HC-ENG	DF/DF	
SEQN-	62123	
JREF-	1UL5487_Z01	



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by IPI and NICA) for details and practices prior to performing these functions. Installers shall provide temporary bracing per BCSI requirements. Trusses shall be installed in accordance with the manufacturer's instructions. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
The Building Components Group Inc. (BTBCG) shall not be responsible for any deviation from this design or failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. The user shall obtain the necessary permits and approvals from the appropriate authorities. IPI-BCSI: www.iipibcsi.com; TPI: www.tpinet.org; NICA: www.nicaindustry.com; ICC: www.iccsafe.org

ALPINE

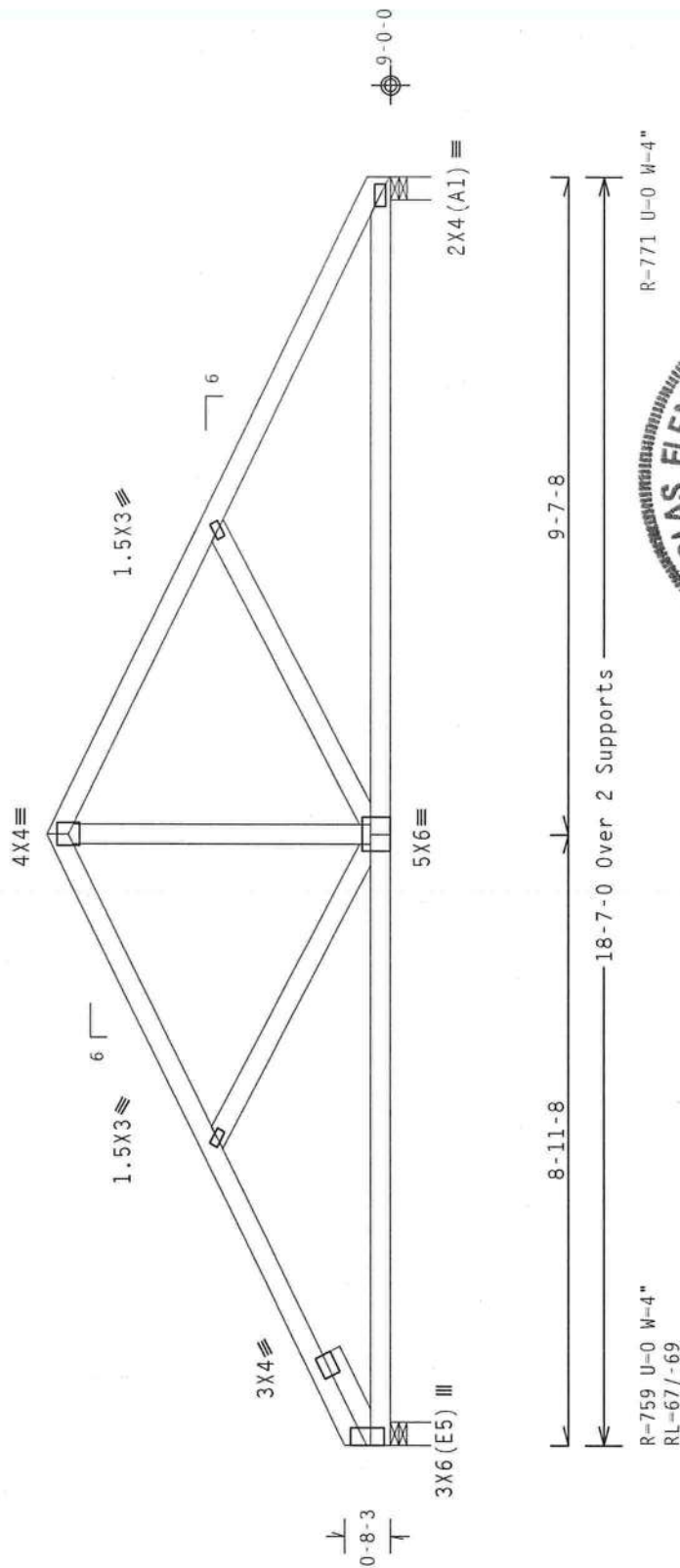
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

1120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-759 U-0 W-4"
RL-67/-69

18-7-0 Over 2 Supports

R-771 U-0 W-4"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .375"/Ft.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

REF R487-- 3114

DATE 04/10/12

DRW HCUSR487 12101030

HC-ENG DF/DF

SEQN- 62133

JREF- 1UL5487_Z01

TC LL	20.0	PSF	★	No. 66648	★
TC DL	10.0	PSF			
BC DL	10.0	PSF			
BC LL	0.0	PSF			
TOT.LD.	40.0	PSF			
DUR.FAC.	1.25				
SPACING	24.0"				

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.**

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IPI and WICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, Top Chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint for web shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/IPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the 3rd details, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal of the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility. Review drawings showing use of this design for use of this design for any structure. Responsibility of the building owner per ANSI/IPI 1 Sec. 2. For more information, see: This job's general notes page; ITH-BCG: www.ithbcg.com; IPI: www.ipiinst.org; WICA: www.sbcindustry.com; www.icesafe.org

This Job's Date: 04/10/2012

(12-071--Fill in later BRYAN ZECHER/VICTORES -- , ** - D2)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

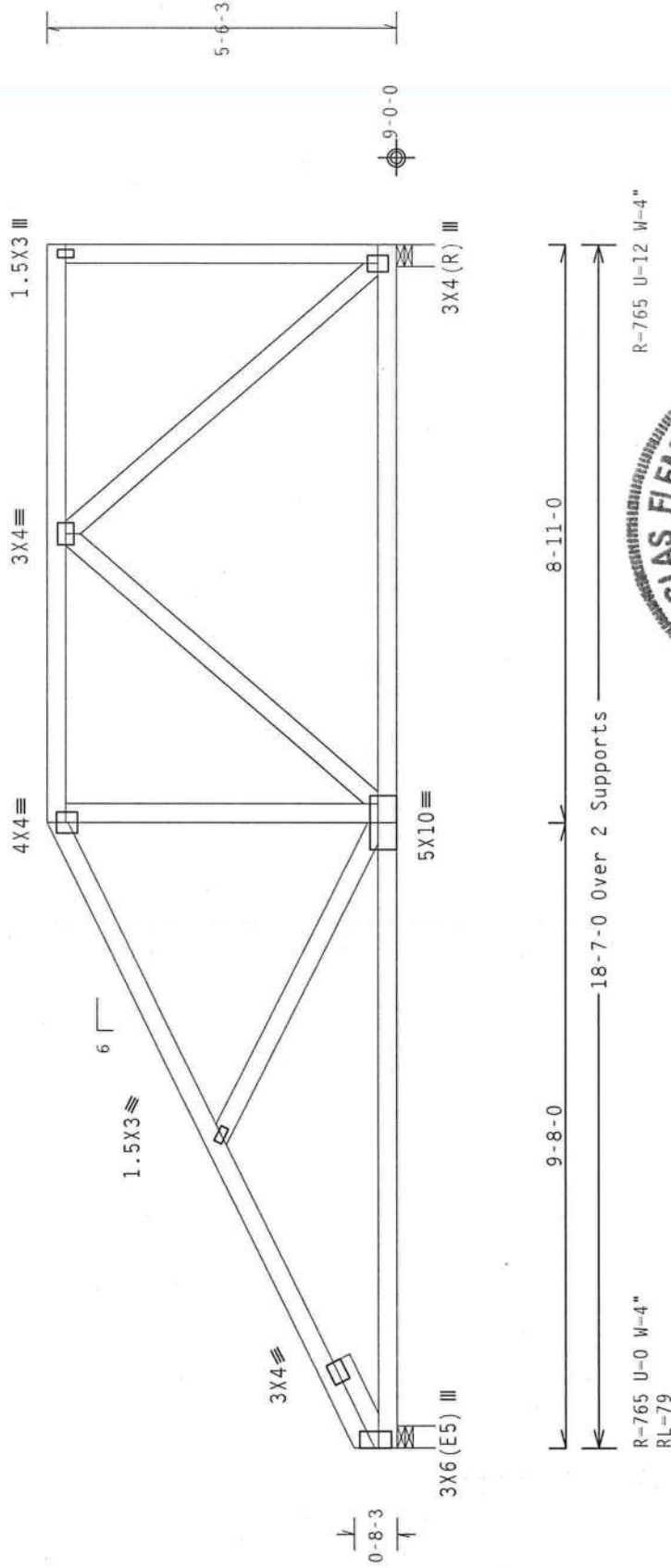
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCp1 (+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

Scale = .375"/Ft.

REF R487-- 3115

DATE 04/10/12

DRW HCUSR487 12101031

HC-ENG DF/DF

SEQN- 62144

JREF- 1UL5487_Z01

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
10-08-11/0208-20
10/10/2012

****IMPORTANT**** FURNISH THIS DESIGN TO ALL COMPANIES INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information, by IPI and WCAI for details and practices prior to performing these tasks. Installers shall provide temporary bracing and purlins to support the trusses until they are permanently braced. The trusses shall be installed in accordance with the details shown for permanent lateral restraint. A seal on the web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the details shown for any reason. The truss shall be installed in accordance with the details shown above and on the job site. Details, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal on the web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
The responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This job's general notes page: IPI-BG: www.tubeg.com; TPI: www.tpinet.org; WCAI: www.sbcindustry.com; IBC: www.iccsafe.org

Top chord 2x4 SP M-30
Bot chord 2x6 SP #2 :B2 2x6 SP SS:
Webs 2x4 SP #3 :W4, W8 2x4 SP M-30:

Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

Special loads

Tree	Species	DBH	Height	Volume	Weight	Stem	Branch	Leaf	Seed	Other	Notes
1	TC	From	62 pif	at -1.33	to 62 pif	at 7.00					
2	TC	From	62 pif	at 7.00	to 62 pif	at 30.92					
3	BC	From	4 pif	at -1.33	to 4 pif	at 0.00					
4	BC	From	20 pif	at -0.00	to 20 pif	at 30.92					
5	TC	433.75	1b Conc.	Load at 7.06							
6	TC	188.65	1b Conc.	Load at 9.06	11.06						
7	BC	509.24	1b Conc.	Load at 7.03							
8	BC	129.14	1b Conc.	Load at 9.06	11.06						
9	BC	1044.98	1b Conc.	Load at 18.23							
10	BC	433.07	1b Conc.	Load at 20.23	22.23	24.23	26.23				
11	BC	23.30	23								

Brq blocks:0.131"x3", min. nails
brg x-loc #blocks length/blk #nails/blk wall plate
2 11.333' 1 12" 4 Rigid Surface

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpf (+/-) = 0.18

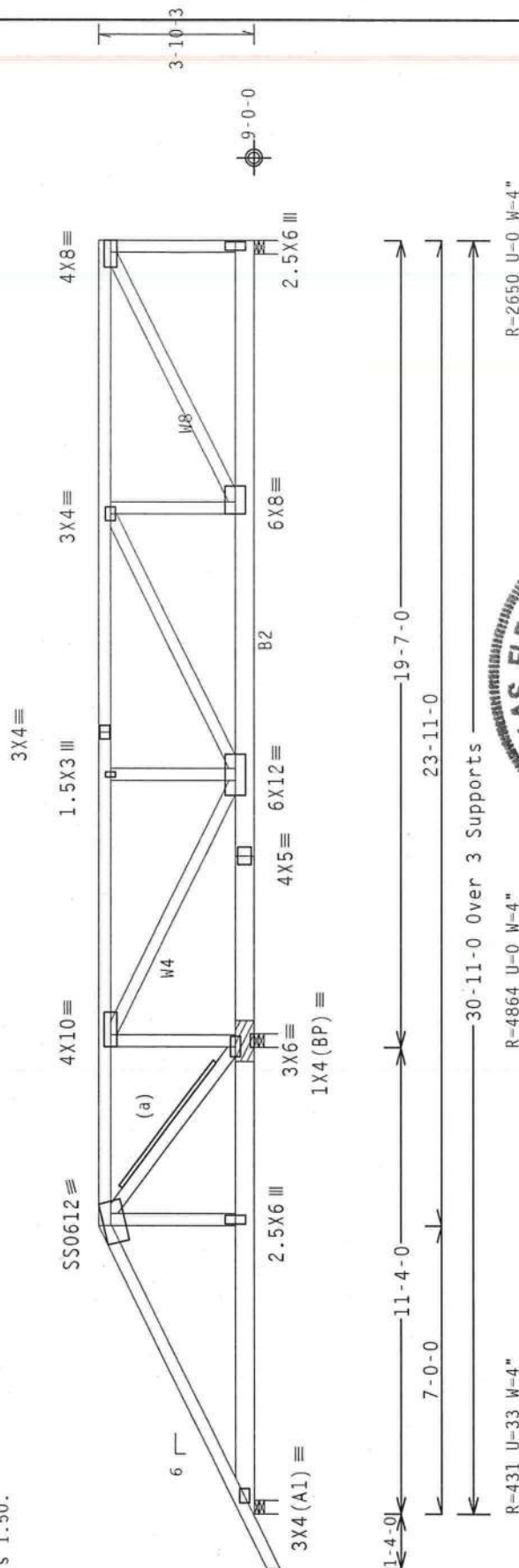
Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Wind Loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



$P=437$ $II=33$ $W=4''$

30-11-0 Over 3 Supports

2=4864 11=0 W=4"

P=2650 H=0 W=4"

PLT TYP. 18 Gauge HS. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
 $FT/RT=10\%(0\%)/0(0)$

Scale = .25"/Ft.

FL/-/4/-/-/R/-

9/20/82 TY:

0.05.11

0.0% (0%) / 0.0% (0%)

FT/F

222

Gauge HS. Wa

PLT TYP.

* **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET

•IMPORTANT•
WARNING: READ AND FOLLOW ALL RULES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of HCSS (Building Component Safety Information, by IPI and WCA) for details.

practices prior to performing these functions. Installers shall provide temporary bracing per BSI.

hall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of columns shall have properly attached structural sheathing and bottom chords otherwise. Top chord shall have properly attached structural sheathing and bottom chords unless noted otherwise.

shall have bracing installed per BCS1 sections B3, B7 or B10, as applicable.

any failure to build the truss in conformance with ANSI/IPI 1, or for handling, shipping, installation or use of the truss. (IPIBCA) shall not be responsible for any deviation from this specification.

ing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this

drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is

the responsibility of the Building Designer per ANSI/TPI 1 Sec-2. For more information see: This Job's

general notes page; in-also: www.library.com; ip: www.ipinst.org; nlc: www.socindustry.com; CC: www.fccsafe.org

Top chord 2x4 SP M-30
Bot chord 2x6 SP #2 :B2 2x6 SP S5:
Webs 2x4 SP #3 :W5 2x6 SP #2:
:W6, W8, W12 2x4 SP M-30:

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger
in each row to avoid splitting.

Brg blocks: 0.131"x3", min. nails
 Brg x-loc #blocks length/blk wall plate
 2 11.333' 1 12" 15 Rigid Surface
 Brg block to be same size and species as chord.
 Refer to drawing CWNAILSP0109 for more information.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpt (+/-) = -0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

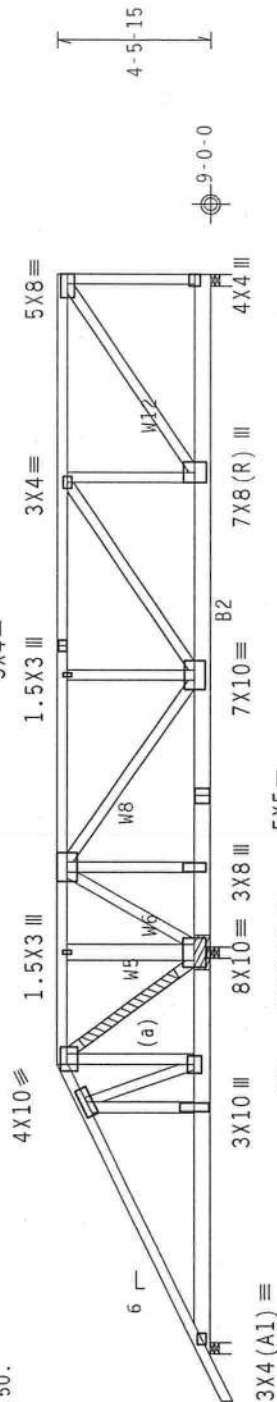
Special loads

	(Lumber	Dur.Fac.=1.25	/	Plate	Dur.Fac.=1.25)
TC- From	62 plf at	-1.33	to	62 plf at	8.29
TC- From	62 plf at	8.29	to	62 plf at	30.92
BC- From	4 plf at	-1.33	to	4 plf at	0.00
BC- From	20 plf at	0.00	to	20 plf at	7.06
BC- From	10 plf at	7.06	to	10 plf at	30.92
BC- 3652.08	1b Conc.	Load at	7.06		
BC- 1712.26	1b Conc.	Load at	9.06		
BC- 1713.37	1b Conc.	Load at	11.06		
BC- 1508.17	1b Conc.	Load at	13.06, 15.06, 17.06, 19.06		
BC- 1709.71	1b Conc.	Load at	21.06, 23.06		
BC- 1636.69	1b Conc.	Load at	25.06, 27.02		
BC- 1454.78	1b Conc.	Load at	29.02		

Roof overhang supports 2.00 psf soffit load.

(a) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
 $FT/RT=10\%(0\%)/0(0)$

FL / - / 4 / - / - / R / - /
Scale = 1875" / Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. After truss erection, the contractor must ensure that all members are properly braced against buckling. The contractor shall provide temporary bracing for all trusses until permanent bracing is installed. The installer shall provide temporary bracing for all trusses unless otherwise noted. Otherwise, top chord shall have properly attached crossmember sheathing and bottom chord shall have a properly attached rafter ceiling. Locations for permanent lateral restraining web members shall have bracing installed per BC93 sections B3, B7 or B10, as applicable.

LTD Building Components Group Inc. (LTD863) shall not be responsible for any deviation from the design shown. If the contractor fails to follow the design shown, it will be its responsibility if any failure to build the truss in accordance with ANSI/TPI 1 or for handling, shipping, installing or erecting of the trusses occurs. The contractor shall be responsible for the design, fabrication, handling, shipping, installation and erection of the trusses. Apply plates to each face of truss and position as shown above and on the seal on the end of the truss. Refer to drawings LBD-2 for standard plate positions. A seal on the end of the truss indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 sec.2. For more information see: This Job's General notes page; LTD-Bldg.; www.lbdcg.com; www.ltdinc.org; NICA; www.nicaindustries.com.

TC DL	10.0	PSF	DATE	04/10/12
BC DL	10.0	PSF	DRW	HCUSR487 12101033
BC LL	0.0	PSF	HC-ENG	DF/DF
TOT.LD.	40.0	PSF	SEQN-	62537
DUR.FAC.	1.25			
SPACING	24.0"		JREF-	11115487 701

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(12-071--Fill) in later BRYAN ZECHER/VICTORES -- , ** - FG)

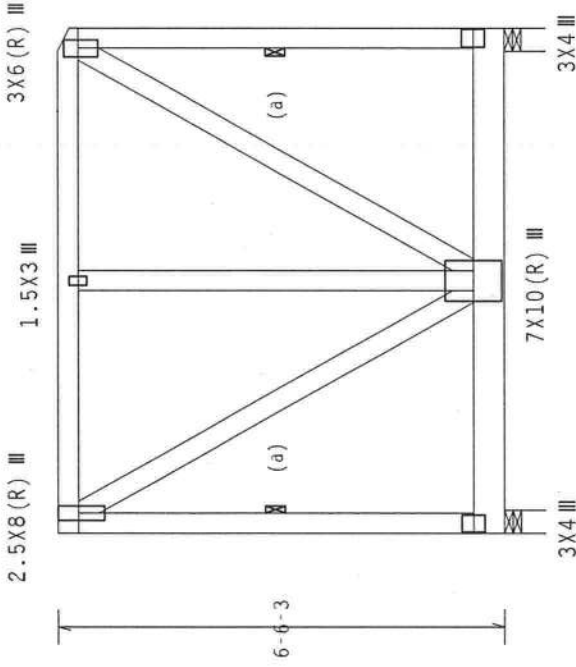
Top chord 2x4 SP M-30
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

120 mph wind, 15.51 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. GCpl(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor
for dead load is 1.50.



7'-3'-0" Over 2 Supports
R-2390 U-0 W-4" R-2814 U-0 W-4"

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at 0.00 to 60 plf at 6.92
TC- From 60 plf at 6.92 to 60 plf at 7.25
BC- From 10 plf at 0.00 to 10 plf at 7.25
BC- 1454.69 lb Conc. Load at 1.90
BC- 1621.10 lb Conc. Load at 3.90, 5.85

End verticals not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in
lieu of structural sheathing.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

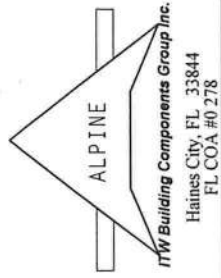
Scale = .375"/Ft.

TC LL	20.0 PSF	REF	R487--	3118
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101034
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	62415	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information, by TPI and WCAI for details and practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Trusses shall be installed in accordance with the manufacturer's instructions. Trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (114860) shall not be responsible for any deviation from the design shown. Trusses shall be installed in accordance with ANSI/TPI-1 for handling, shipping, installing and bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1004-2 for standard plate positions. A seal on drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: 114-860: www.tpiinc.com; TPI: www.tpinet.org; WCAI: www.bcsiindustry.com; ICC: www.iccsafe.org



Top chord	2x4	SP	M-30
Bot chord	2x4	SP	M-30

Roof overhang supports 2.00 psf soffit load.

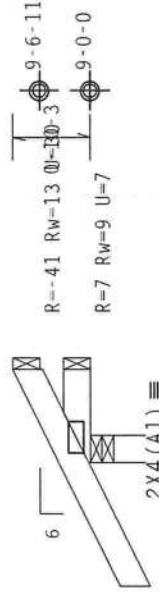
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCP(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.


$$\hookrightarrow 1-4-0 \rightarrow$$

R-222 U-17 W-3.5"

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP.: Wave

0 FL/-/4/-/-/R/-

Scale = .5"/Ft.

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI detailing components for safety information by attaching a copy of this document to the erection drawings. The following are the minimum bracing requirements for the trusses noted above. Top chords shall have properly attached structural sheathing and bottom chords shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B for B10, as applicable.

LTV Building Components Group Inc. (LTHBCG) shall not be responsible for any deviation from or failure to follow instructions regarding design, fabrication, erection, bracing of trusses, apply plates to each face of truss and position as shown above and on the LTR drawings, unless noted otherwise. Refer to Drawings 160A-Z for standard plate positions. A seal of the drawing or cover page listing this drafting, indicates acceptance of professional engineering responsibility solely for the design shown. The availability and use of this design or any structural components must conform to applicable codes and standards.

This job was prepared by:
Designer: JAMES HARRIS
Checker: BOB WILSON

Contact Information:
Company Name: LTHBCG; web: ltvbcg.com; fax: 781-937-3939; email: sales@lvtcusa.org;
Phone Number: 1-800-351-5538; Fax: 1-800-351-5538; Email: info@lvtcusa.org;

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Haines City FL 33844

FL COA #0 278

FINAL 04/10/2012

SPACING 24.0"

JREF- 1UL5487 Z01

(12-071--Fill in later BRYAN ZECHER/VICTORES -- ** - CQ3)

Top chord 2x4 Sp M-30
Bot chord 2x4 Sp M-30

Roof overhang supports 2.00 psf soffit load.

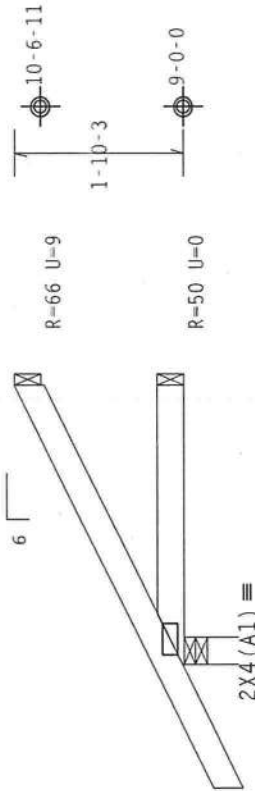
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

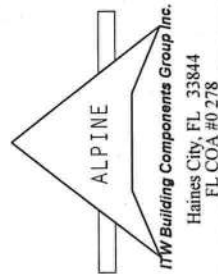


6' 3'-0-0 Over 3 Supports

R-244 U=0 W=3.5"
RL=35

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave



Scale = .5"/Ft.

TC LL	20.0 PSF	FL/-/4/-/-R/-	REF	R487--	3120
TC DL	10.0 PSF		DATE	04/10/12	
BC DL	10.0 PSF		DRW	HCUSR487	12101036
BC LL	0.0 PSF		HC-ENG	DF/DF	
TOT.LD.	40.0 PSF		SEQN-	62021	
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	1UL5487_Z01	

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for details and practices prior to performing these functions. Installers shall provide temporary bracing per the notes on this drawing. The truss shall be braced in accordance with the notes on this drawing. The truss shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPV Building Components Group Inc. (TPVBCG) shall not be responsible for any deviation from this design. The truss shall be installed in accordance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: 11W-BG: www.tlbcg.com; TPI: www.tpinat.org; WCA: www.bcsiindustry.com; ICC: www.iccsafe.org

(12-071--Fill in later BRYAN ZECHE/VICTORES -- . ** - CJ5)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30

Roof overhang supports 2.00 psf soffit load.

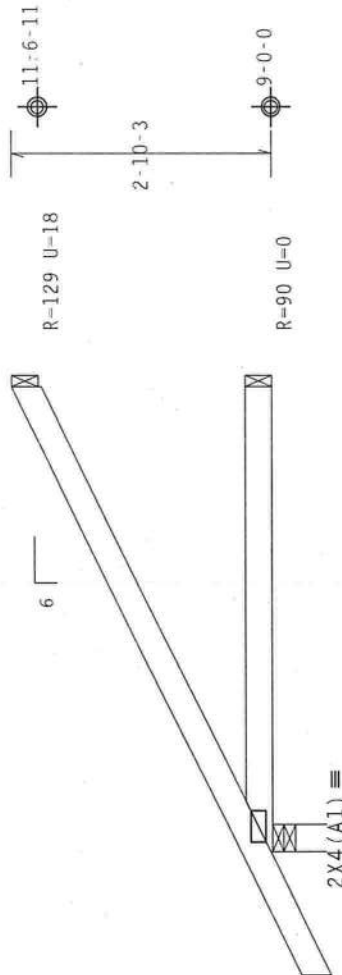
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



←1-4-0→
←5-0-0 Over 3 Supports→
R-316 U-0 W-3.5"
RL=52/-21

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5"/Ft.

FL/-4/-/-R/-

10.00.11.0209.20

TC LL	20.0 PSF	REF	R487--	3121
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101037
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	62024	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	

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****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information, by IPI and WICA for details on proper practices. The top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

IPI Building Components Group Inc. (IPI/BCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; IPI-BCG; www.ipeco.com; IPI: www.tpinet.org; WICA: www.bcsiindustry.com; IEC: www.ecosafe.org

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
04/10/2012

ALPINE

ALPINE Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

(12-071--Fill in later BRYAN ZECHER/VICTORIES -- , ** - CUSJ)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30

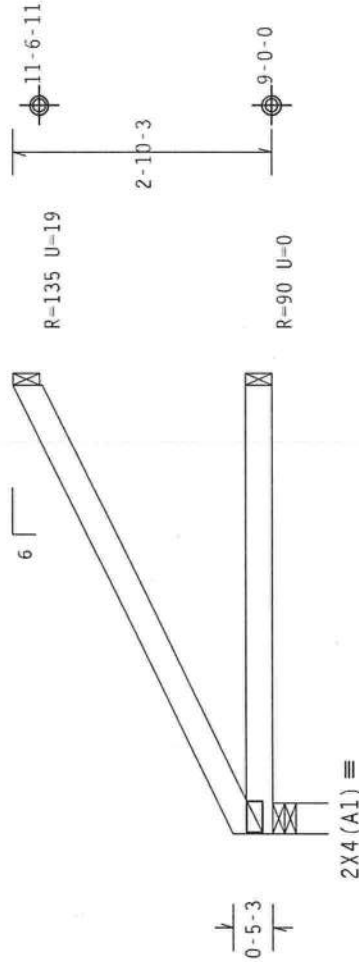
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl (+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



4-10-0 Over 3 Supports

R-205 U=0 W=4"
RL=39

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

FL/-4/-/-R/-

Scale =.5"/Ft.

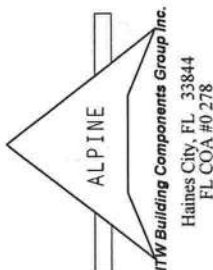
TC LL	20.0 PSF	REF	R487--	3122
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101038
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	62027	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	



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FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by IPI and NICA, for details on practices and other information. The truss manufacturer shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. ITWBCG is not responsible for the use of this design. ITWBCG: www.itwbcg.com; TPI: www.tpinat.org; NICA: www.nicaindustry.com; ICC: www.iccsafe.org.



(12-071--Fill in later BRYAN ZECHER/VICTORIES -- , ** - EJS)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30

Bottom chord checked for 10.00 psf non-concurrent live load.

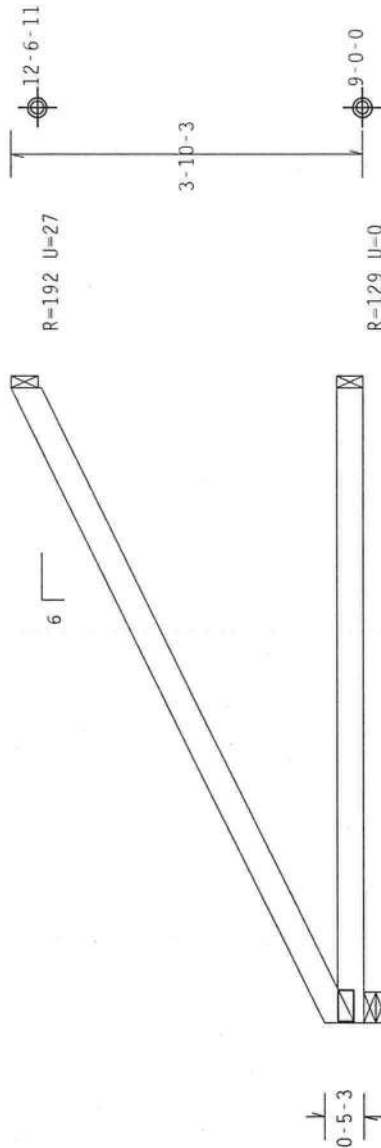
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCp1 (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



6'-10-0 Over 3 Supports

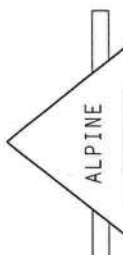
R-288 U=0 W=4"
RL=56

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

FL / - / 4 / - / - / R / -

Scale = .5" / Ft.

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.		No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/20/2012		TC LL	20.0 PSF	REF R487--	3123
	Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Components Safety Information, by TPI and WCA, for details on proper handling, shipping, installing and bracing of trusses. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.		Refer to Job Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure other than that for which it was specifically designed is not intended. This job's general notes pages: ITM-BCSI: www.itm-bcsi.com; TPI: www.tpinet.org; WCA: www.wcaindustry.com; ICC: www.iccsafe.org		TC DL	10.0 PSF	DATE	04/10/12
	ITM Building Components Group Inc. (ITMBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure other than that for which it was specifically designed is not intended. This job's general notes pages: ITM-BCSI: www.itm-bcsi.com; TPI: www.tpinet.org; WCA: www.wcaindustry.com; ICC: www.iccsafe.org				BC DL	10.0 PSF	DRW	HCUSR487 12101039
					BC LL	0.0 PSF	HC-ENG	DF/DF
					TOT.LD.	40.0 PSF	SEQN-	62030
					DUR.FAC.	1.25		
					SPACING	24.0"	JREF-	1UL5487_Z01

(12-071--Fill) in later BRYAN ZECHER/VICTORES --, ** - EJT)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

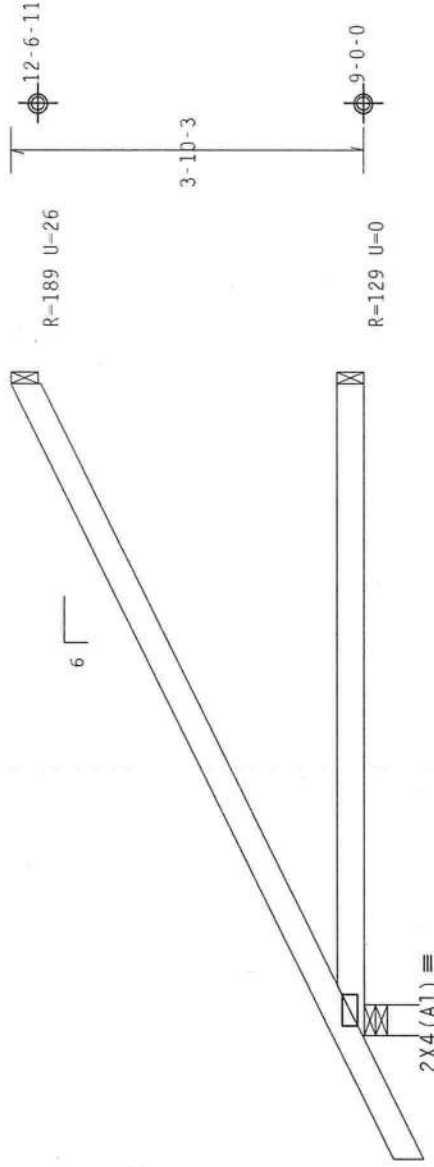
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II OR III, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCp1(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



7-0-0 Over 3 Supports

R-394 U=0 W=4"
RL-68/-25

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5"/Ft.

 ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	**IMPORTANT** READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.				REF R487 -- 3124
	Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to a qualified truss manufacturer for detailed information. Do not use any bracing practices or methods not specifically approved by the manufacturer. Do not use any bracing practices prior to performing these functions. Installers shall provide temporary bracing per BCSG unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSG sections B3, B7 or B10, as applicable.		TC LL 20.0 PSF		DATE 04/10/12
	ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the Truss in conformance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. Apply gusset plates to each face of truss and position as shown above and on the joint between trusses. 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Top	chord	2x4	SP	M-30
Bot	chord	2x4	SP	M-30

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

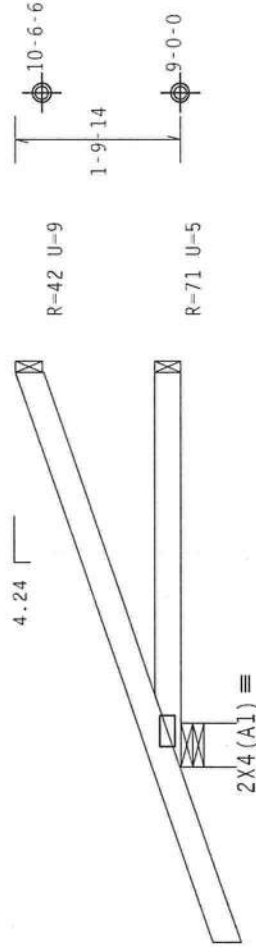
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Special loads

Special loads	(Lumber	Dur.Fac.-1.25 /	Plate	Dur.Fac.-1.25)
TC- From	0 pif at -1.89 to	61 pif at 0.00		
TC- From	2 pif at 0.00 to	2 pif at 4.24		
BC- From	0 pif at -1.89 to	4 pif at 0.00		
BC- From	2 pif at 0.00 to	2 pif at 4.24		
TC- 30.42 lb Conc.	Load at	1.48		
TC- 14.10 lb Conc.	Load at	1.48		

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



$\leftarrow 1-10-10 \rightarrow$
 $\leftarrow 4-2-15 \text{ Over } 3 \text{ Supports } \rightarrow$
 $R=105 \quad I=53 \quad W=5.657''$

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale = .5"/Ft.

* *WARNING* * READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

* *IMPORTANT* *

Installers shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint webs shall have bracing installed per GCSI sections B3, B or B10, as applicable.

ITW Building Components Group Inc. (IIBC65) shall not be responsible for any deviation from the design of the trusses, including but not limited to, any failure to build the truss in conformance with ANSI/IPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Job's Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility for the design of the truss. The design of the truss is the responsibility of the truss manufacturer. For more information see: This Job's general notes page. IIBC65: www.iibc65.com; IPI: www.ipi.net; IBCA: www.ibcaindustry.com; IBCA: www.1ccsafe.org

ALPINE
ITW Building Components
Haines City, FL
FL COA #02

04/10/2012

(12-071--Fill) in later BRYAN ZECHER/VICTORIES --, ** - HJ7)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

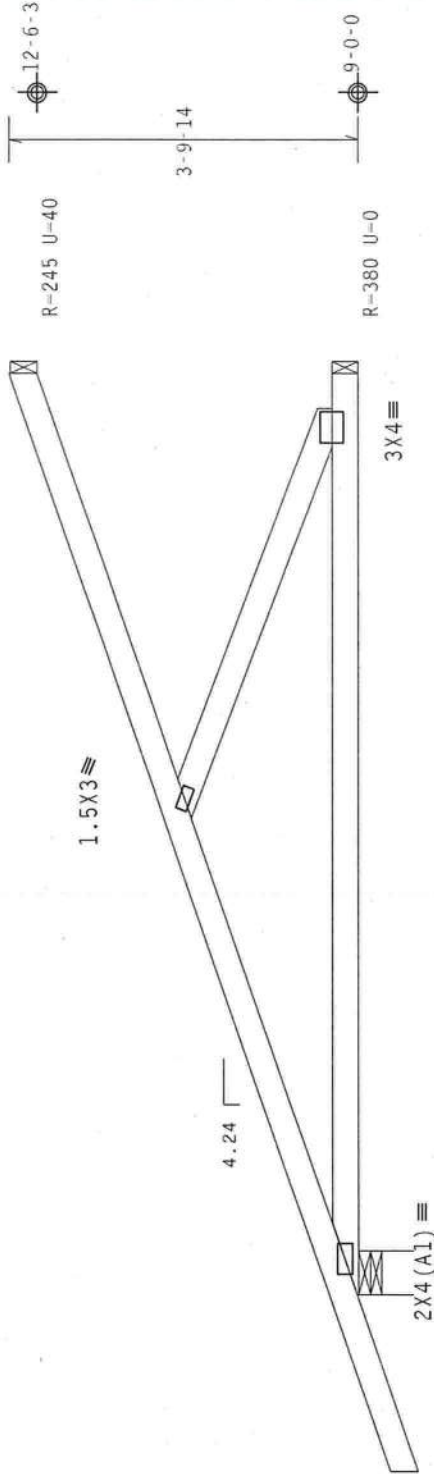
Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



← 1-10-10 →
← 9-10-13 Over 3 Supports →
R=439 U-5 W-5.657"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.05.11/0208.20
No. 66648

FL/-4/-/-R/-

Scale = .5" / Ft.

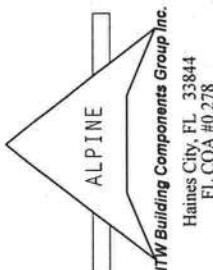
TC LL	20.0 PSF	REF	R487--	3126
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101042
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	62060	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of the Building Components Group Inc. (BCGI) Truss Installation Manual for details. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's General Notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinat.org; WCA: www.safetysure.org.



Top	chord	2x4	SP	M-30
Bot	chord	2x4	SP	M-30
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) - (J) Hanger not calculated (1)2x6 SP SS supporting member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

1120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpl (+/-)=-0.18

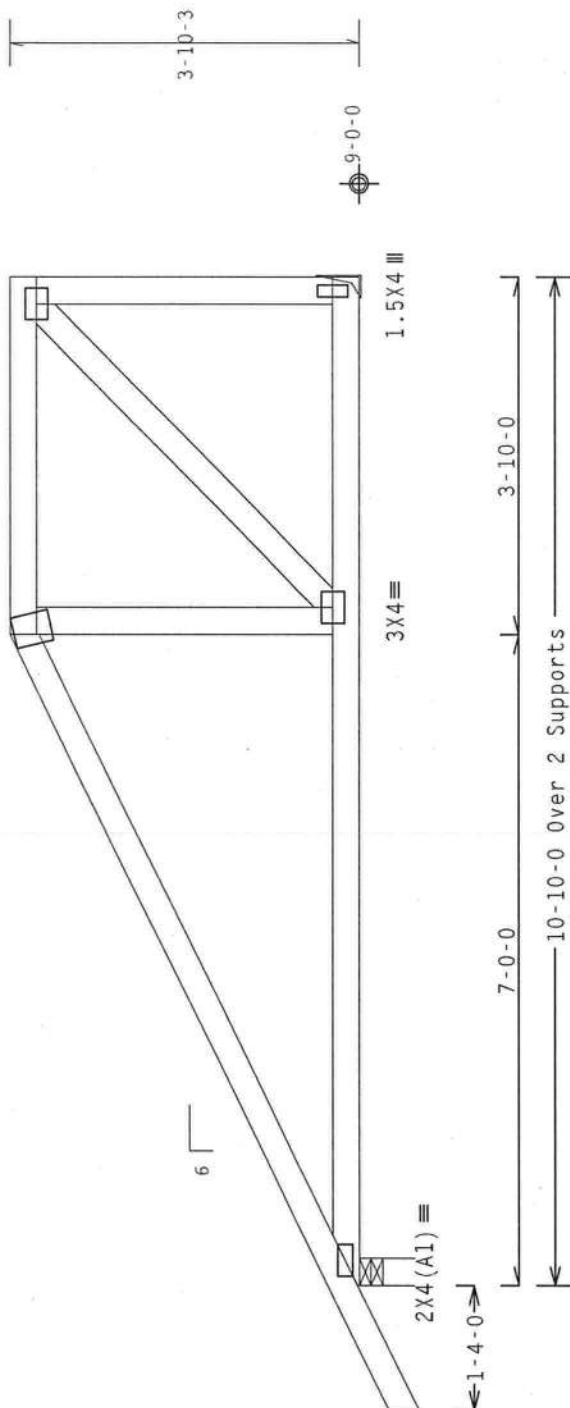
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase $3X4 =$ factor for dead load is 1.50.



2-791 11-6 W-3 5"

R-1045 U-0 H-Simpson LUS26
Supporting Member Face: (4) 0.148"x3" nails
Flange Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

PLOT TYP. Wave

FL/-/4/-/-/R/-

Scale = .5"/Ft.

*****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
IMPORTANT*

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings. The installer shall provide temporary bracing for all trusses unless otherwise noted. The chord shall have a properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations for permanent lateral restraint shall have bracing installed per AISC sections B3, B7 or B10, as applicable.

The Building Components Group Inc. (LBGBC) shall not be responsible for any deviation from the design shown on drawings or specifications. LBGBC's responsibility is limited to the manufacturing of the trussing system. Failure to build the truss in conformance with ANSI/TPI-1 or for handling, shipping, installation, bracing of cranes, applying plates to each face of truss and position as shown above and on the job site drawings, may result in structural failure. The manufacturer will not be responsible for details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI-1 Sec.2. For more information see: This Job's general notes page; LBN-BDCB; www.lbn-bdc.com; http://www.tpi-inc.org; NPGA; www.sbcindustries.com;

ALPINE

MTW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Haines City, FL 33844
FL COA #0 278

REF- 1UL5487 Z01

SPACING 24.0"

2107/01/40 minutes

100

org: WTCA: www.sbcIndustry.org.

www.twbcg.com; TPI: www.tpin

General notes page: ITW-BCG:

City, FL 33844

Haine

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpt (+/-)=0.18

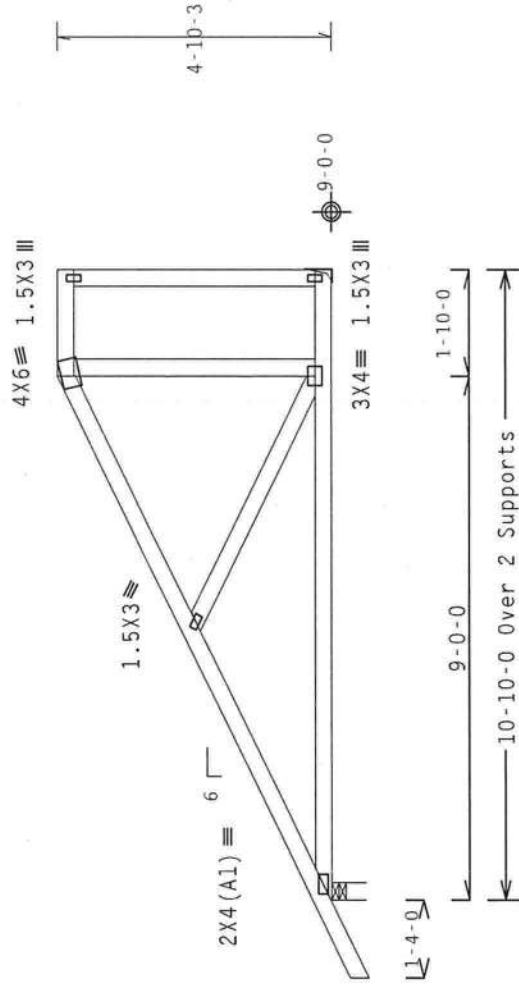
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=433 U=0 H=Simpson LU24

Supported Member Face: (2) 0.18" x 1" nails

Supporting Member Face: \$148.00

Supporting Member : (X)

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%)/0(0)

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following information for more details. For more information, refer to the following references:

- Follow the latest edition of BCSI (Building Component Safety Information), by IPI and IBCA for best practices prior to erection of these trusses. This manual also provides information on bracing and erection of trusses.
- Refer to the BCSI (Building Component Safety Information) manual, Shearwalls and Bracing, for more information on bracing.
- All trusses shall have properly attached rigid diaphragms. Locations shown for permanent lateral restraint will be based on properly attached rigid diaphragms.
- All trusses shall have bracing installed per BCSI, sections 83, 87 or 80, as applicable.

[illegible]

ICC: www.iccsafe.org
general notes page; IIN-BLG: www.iccbg.com; IPI: www.ipinst.org; WICA: www.sbcindustry.com;

PLT TYP. Wave

FL/-/4/-/-/R/-
Scale = .3125"/Ft.

TC LL	20.0 PSF	REF	R487--	3128
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10 0 PSE	DATE	04/10/12
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DATE	04/10/21
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ABC DL	10.0 PSF	DRW HCUSR487	12101044
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BC LL	0.0 PSF	HC-ENG DF/DF
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TOT.LD.	40.0 PSF	SEON-	62051
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DIIP FAC	1 25
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[illegible]

(12-071--Fill) in later BRYAN ZECHER/VICTORIES --, ** - M)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (1)2x6 SP SS supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

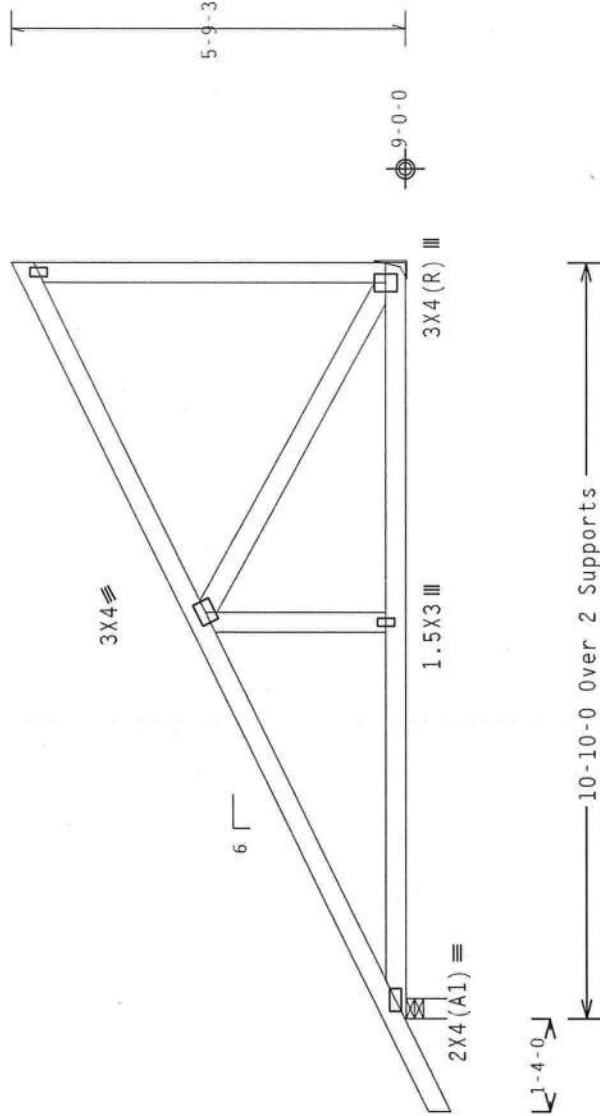
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load 1.50.



R-433 U-0 H-Simpson LU24

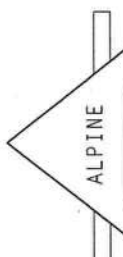
Supplied Member Face: (2) 0.148"x1.5" nails
Supplier Member Face: (4) 0.148"x3" nails
Supplier Member Face: (1) 2x6 SP #2

R-548 U-0 W-3.5"
RL=67/-32

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .375"/Ft.

 ALPINE iTW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER 10/10/2012		TC LL	20.0 PSF	REF	R487--	3129
			TC DL	10.0 PSF	DATE	04/10/12	
			BC DL	10.0 PSF	DRW	HCUSR487	12101045
			BC LL	0.0 PSF	HC-ENG	DF/DF	
			TOT.LD.	40.0 PSF	SEQN-	62057	
			DUR.FAC.	1.25	JREF-	1UL5487_Z01	
PLT TYP. Wave			SPACING	24.0"			

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by IPI and NICA) for safety practices and to permit the manufacturer to perform the design. The manufacturer shall be responsible for the design and the manufacturer shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

iTW Building Components Group Inc. (iTWBCG) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the job site details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal of the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer. For more information see: iTWBCG website: www.itwbcg.com; IPI: www.tpinet.org; NICA: www.nicainc.org

(12-071--Fill in later BRYAN ZECHER/VICTORIES -- ** - M36)

Top chord 2x4 SP M-30
Bot chord 2x6 SP #2
Webs 2x4 SP #3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

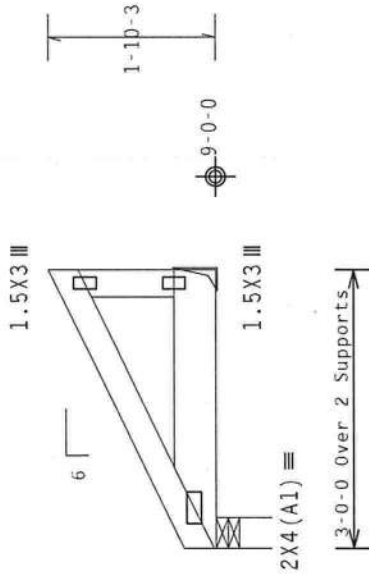
Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
-----Lumber
TC- From Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)
62 plf at 0.00 to 62 plf at 3.00
BC- From 20 plf at 0.00 to 20 plf at 3.00
BC- 890.54 lb Conc. Load at 1.06

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (1)2x4 SP M-30 supporting member.

Deflection meets L/240 live and L/180 total load. Creep Increase factor for dead load is 1.50.



R-734 U=0 W=4 R-403 U=0 H-Simpson LU24
Supported Member Face: (2) 0.148"x1.5" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member: (1) 2x6 SP #2
Design Crif: FBC2010Res/TP1-2007 (STD)
FT/RT=10% (0%) / 0 (0)

PLT TYP. Wave

REF	R487--	3130
DATE	04/10/12	
DRW	HCUSR487	12101046
HC-ENG	DF/DF	
SEQN-	62235	
JREF-	1UL5487_Z01	

Scale = .5"/Ft.

FL/-4/-/R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

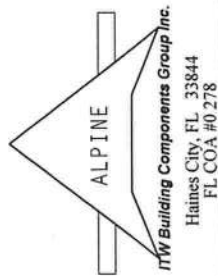
TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCS (Building Component Safety Information, by TPI and WCA) for details and practices prior to performing these functions. Installers shall provide temporary bracing per BCS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached lateral bracing system. Lateral restraint of top chord shall be provided by installing per BCS sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build in accordance with this design. ITWBCG shall not be responsible for any deviation from the details, unless noted otherwise. Refer to drawings 1004-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; WCA: www.bcsindustry.com; ICC: www.iccsafe.org



(12-071--Fill) in later BRYAN ZECHER/VICTORIES -- . ** - H23AP)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 20.35 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf.
GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 20.35 ft. from roof edge.

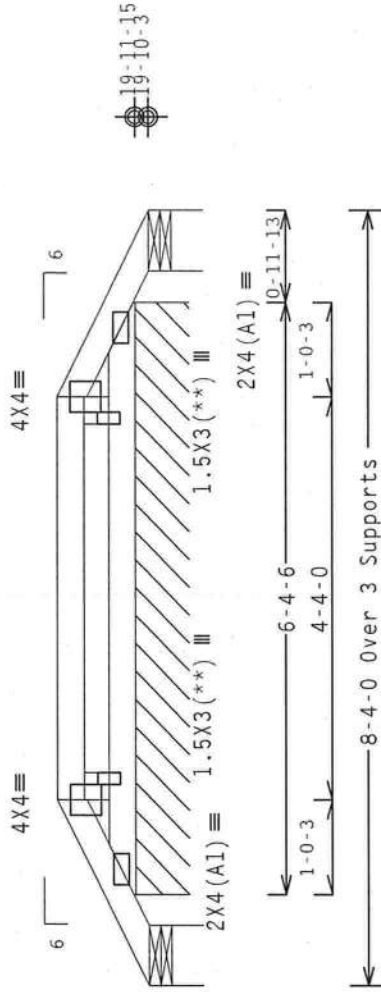
Refer to DWG PB1200310 for piggyback details.

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 2.00
TC- From 62 plf at 2.00 to 62 plf at 6.33
TC- From 62 plf at 6.33 to 62 plf at 8.33
BC- From 4 plf at 0.00 to 4 plf at 8.33

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



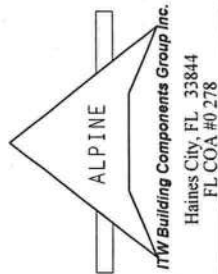
R-23 U-2 W-7.826"

R-73 PLF U=7 PLF W=6-4-6

R-23 U-3 V-3.000"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and BCSA) recommended practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, all chords shall have properly attached bracing (top and bottom) and webs shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

THE Building Components Group Inc. (11/06/03) shall not be responsible for any deviation from any fabricating, handling, shipping, installing and bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal of the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: this job's general notes page: (TW-BCSI: www.tlwbcs.com; TPI: www.tpinat.org; BCSA: www.bcsaindustry.com; ICC: www.iccsafe.org)



Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487--	3131
TC DL	10.0 PSF	DATE	04/10/12	
BC DL	10.0 PSF	DRW	HCUSR487	12101047
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	62287	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UL5487_Z01	

(12-071--Fill) in later BRYAN ZECHER/VICTORES --, ** - AP)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3

120 mph wind, 20.85 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf.
GCPI(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 20.85 ft. from roof edge.

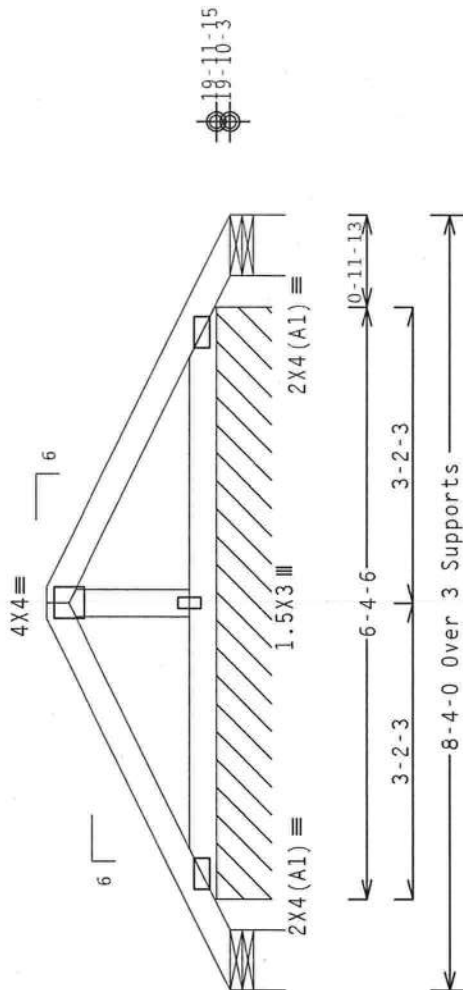
Refer to DWG PB1200310 for piggyback details.

Special loads

-----Lumber Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)
TC- From 62 plf at 0.00 to 62 plf at 4.17
TC- From 62 plf at 4.17 to 62 plf at 8.33
BC- From 4 plf at 0.00 to 4 plf at 8.33

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-3 Rw-16 U-24 W-7.826"
RL-28/-28-82 PLF U-2 PLF W-6-4-6

R-3 Rw-3 U-24 W-7.826"

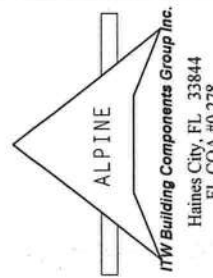
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of ECSI (Building Component Safety Information, by TPI and AISC) for details and practices prior to erection. Trusses shall be erected in accordance with the manufacturer's instructions. The manufacturer shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per ECSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the job. Details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. For more information see: This job's website: www.itwbcg.com; TPI: www.tpinet.org; AISC: www.aisc.org



Scale = .5" / Ft.

TC LL	20.0 PSF	FL / - / 4 / - / - / R / -	REF	R487 --	3132
TC DL	10.0 PSF		DATE	04/10/12	
BC DL	10.0 PSF		DRW	HCUSR487	12101048
BC LL	0.0 PSF		HC-ENG	DF/DF	
TOT.LD.	40.0 PSF		SEQN-	62290	
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	1UL5487_Z01	

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.
ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

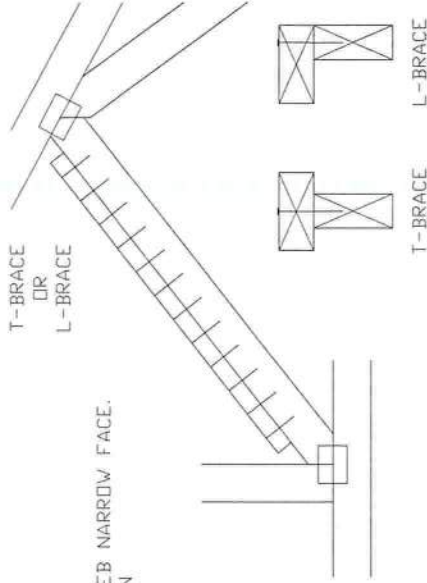
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

⌘ CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

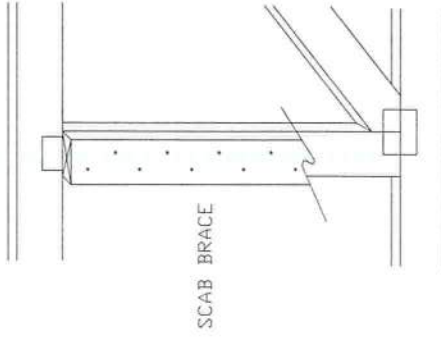
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3" MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3" MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



*****WARNING***** READ AND FOLLOW ALL NOTES ON THIS SHEET
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the manufacturer's instructions for proper handling, storage and installation. Trusses are not to be used for any purpose other than that for which they were designed. Trusses are not to be used for any purpose other than that for which they were designed. Trusses are not to be used for any purpose other than that for which they were designed.

*****IMPORTANT***** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/16GA (V.H/S/K) ASTM A572 Grade 50 (K/M/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details.
The manufacturer shall not be responsible for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com, TPI: www.tpinet.com, VITCA: www.vitcaindustry.com, ICD: www.icdusa.org



Earth City, MO 63045



REF	CLB SUBST.
DATE	1/1/09
DRWG	BRCLBSUB0109
PSF	PSF
PSF	PSF
PSF	PSF
PSF	PSF
TOT. L.D.	
DUR. FAC.	
SPACING	

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Species		Brace		(1) 1x4 'L' Brace		(2) 2x4 'L' Brace		(1) 2x6 'L' Brace		(2) 2x6 'L' Brace	
Spacing	Grade	#1 / #2	No Braces	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
12" O.C.	SPF	#1	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"
	HF	#2	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"
	Standard	#3	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"
24" O.C.	SP	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"
	DFL	#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"
	Standard	#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	4' 7"	6' 0"	6' 4"	8' 0"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"
	HF	#3	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"
	Standard	#3	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"
12" O.C.	SPF	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"
	HF	#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"
	Standard	#3	5' 3"	9' 0"	9' 4"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"
	HF	#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:		Group A:		Group B:	
Spruce-Pine-Fir #1 / #2 Standard	Standard	Standard	Standard	Standard	Standard
	Stud	Stud	Stud	Stud	Stud
	Standard	Standard	Standard	Standard	Standard
Douglas Fir-Larch #3 Standard	Standard	Standard	Standard	Standard	Standard
	Stud	Stud	Stud	Stud	Stud
	Standard	Standard	Standard	Standard	Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 35 psf over continuous bearing (3 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rule

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.

* For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	25X4

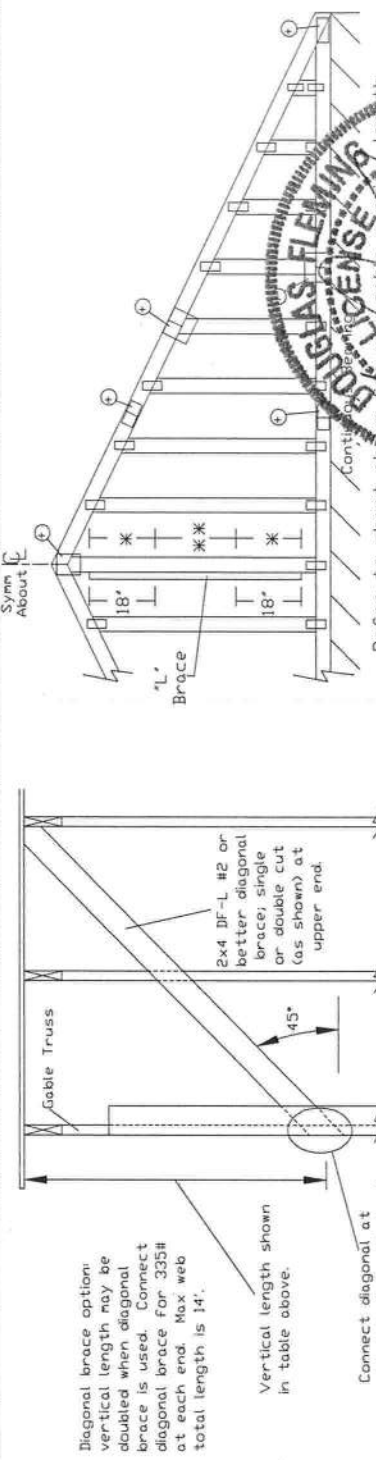
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

REF ASCE 7-10-GABI2015

DATE 2/14/12

DRWG A12015ENC100212



Refer to chart above for max. gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer and follow the latest edition of BCSI Building Component Safety Information, by TPI and VTCa for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral resistant of eaves shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates and bolts as indicated in the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing. Failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing states acceptance of the building component manufacturer. The seal shall be placed on the drawing. The seal of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2. For more information see this job's general notes page and these web sites: ITVBCG www.itvbcg.com, TPI www.tpinet.org, VTCa www.vtcacsa.org, ICC www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

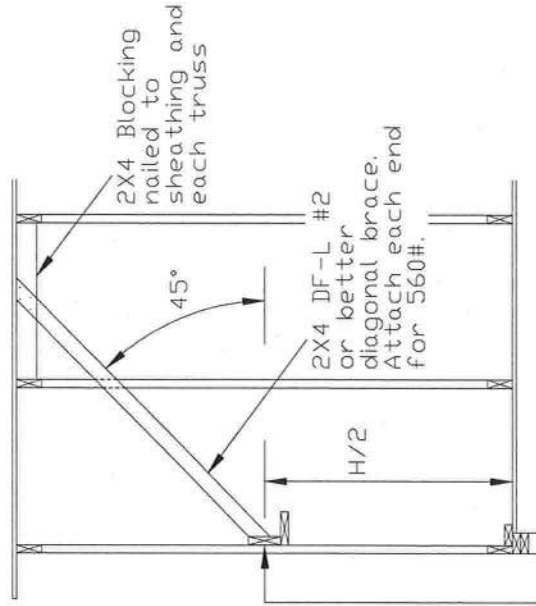
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

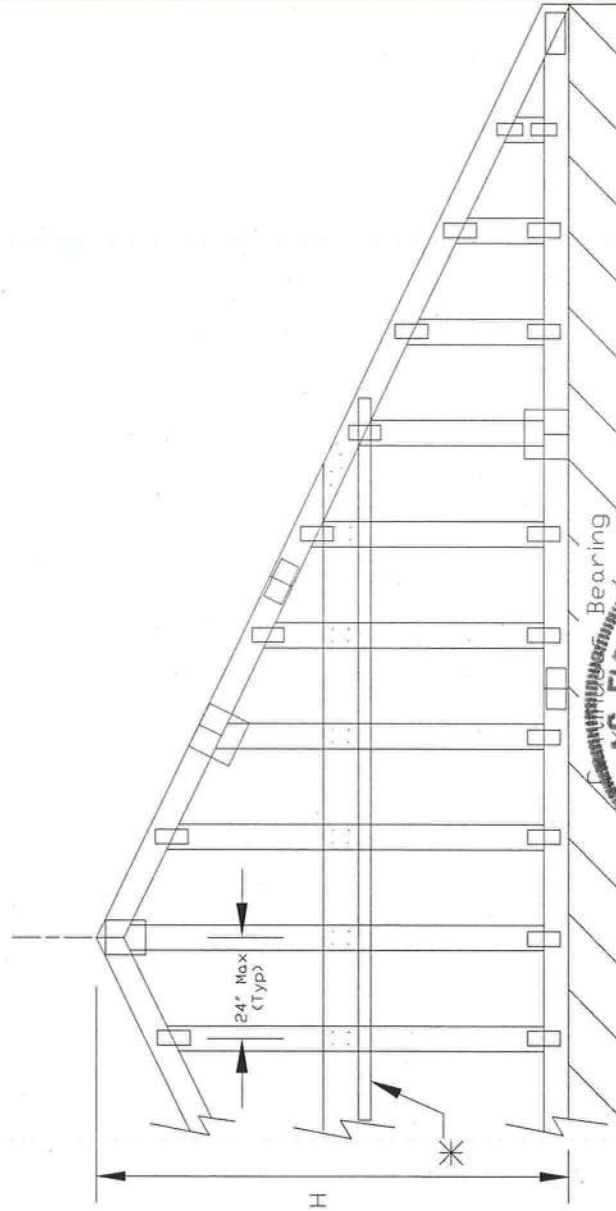
H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC10).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



2x6 #2 Stiffback
attached to each
Stud w/ (4) 10d box or gun (0.123" x 3", min.) nails.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information for TPI and VTCa for best practices prior to performing these functions. Installers shall provide temporary bracing per the instructions provided in the BCSI Building Component Safety Information. The design of this detail is based on the assumption that the truss chord will have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B1, B7 or B10, as applicable. Apply plates to the webs of the truss chord and install them in accordance with the details shown on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, or failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of this drawing by a professional engineer. The design of this detail is based on the assumption that the truss chord will have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B1, B7 or B10, as applicable. Apply plates to the webs of the truss chord and install them in accordance with the details shown on the Joint Details, unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.

Earth City, MO 63045



Building Components Group Inc.

REF GE WHALER

DATE 2/14/12

DRWG GABRST100212

MAX. TOT. LD. 60 PSF

MAX. SPACING

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS

OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

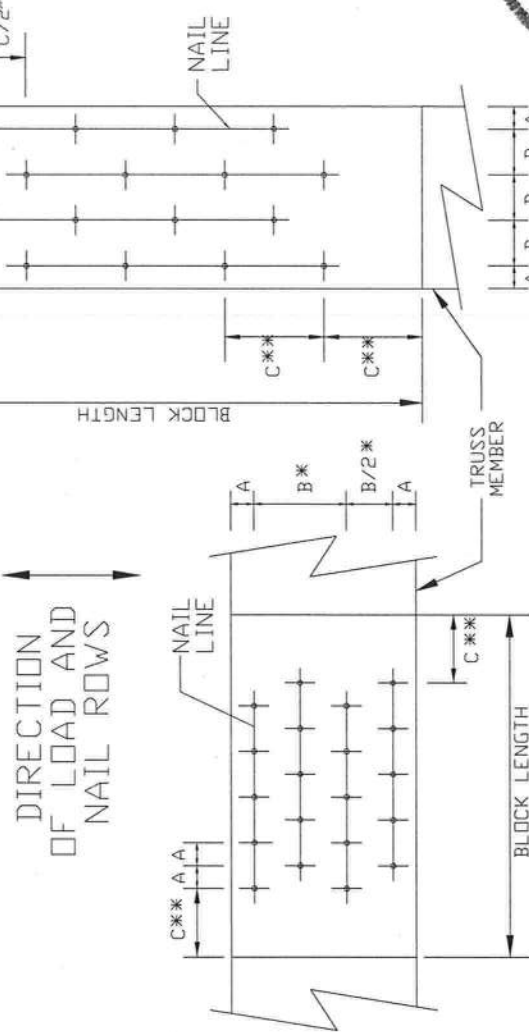
IF NATI HOLES ARE PROPOSED SOME SPACING

IF NAIL HOLES ARE PREBURED, SOME SPACE MAY BE REDUCED BY THE AMOUNTS GIVEN

MAY BE
REFILED:

BELOW: * SPACING MAY BE REDUCED BY 50%.

*** SPACING MAY BE REDUCED BY 33%.



LOAD APPLIED PERPENDICULAR TO GRAIN

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following BCSI (Building Component Safety) Information, by TPI and VITAC for safety practices prior to performing these functions. Installer shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shown for bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT: FURNISH COPY OF THIS DRAWING TO INSTALLATION CONTRACTOR.
ITW Building Components Group, Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2018/1604 CH/250 AS per TPI drawing 100-10353, G35 galv. steel. Apply plates to each face of truss, positioned as shown and on Joint B drawing.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any other application is the responsibility of the user.
ITW-BDG, www.itwbdg.com, TPI, www.tpi.net, ITW-SPIN, www.spin.net, ITW-2, www.itw2.com, ITW-ICC, www.itwicc.com, ITW-BDG, www.itwbdg.com, ITW-SPIN, www.spin.net, ITW-2, www.itw2.com, ITW-ICC, www.itwicc.com

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX <0.113"X 2.5",MIN>	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX <0.128"X 3",MIN>	7/8"	1 5/8"	2"	1"
12d BOX <0.128"X 3.25",MIN>	7/8"	1 5/8"	2"	1"
16d BOX <0.135"X 3.5",MIN>	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX <0.148"X 4",MIN>	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON <0.131"X 2.5",MIN>	7/8"	1 5/8"	2"	1"
10d COMMON <0.148"X 3",MIN>	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON <0.148"X 3.25",MIN>	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON <0.162"X 3.5",MIN>	1"	2"	2 1/2"	1 1/4"
GUN <0.120"X 2.5",MIN>	3/4"	1 1/2"	1 7/8"	1"
GUN <0.131"X 2.5",MIN>	7/8"	1 5/8"	2"	1"
GUN <0.120"X 3",MIN>	3/4"	1 1/2"	1 7/8"	1"
GUN <0.131"X 3",MIN>	7/8"	1 5/8"	2"	1"

Professional Engineer Seal for Douglas Fleming, State of Florida, License No. 66648, expires 10/10/2012.



Building Components Group Inc.

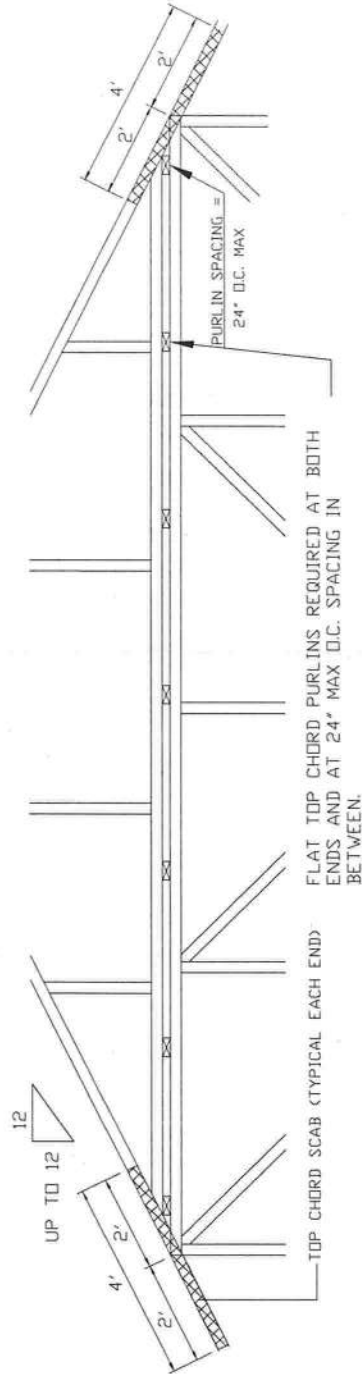
Earth City, MO 63045

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF (MIN), Kzt=1.0.

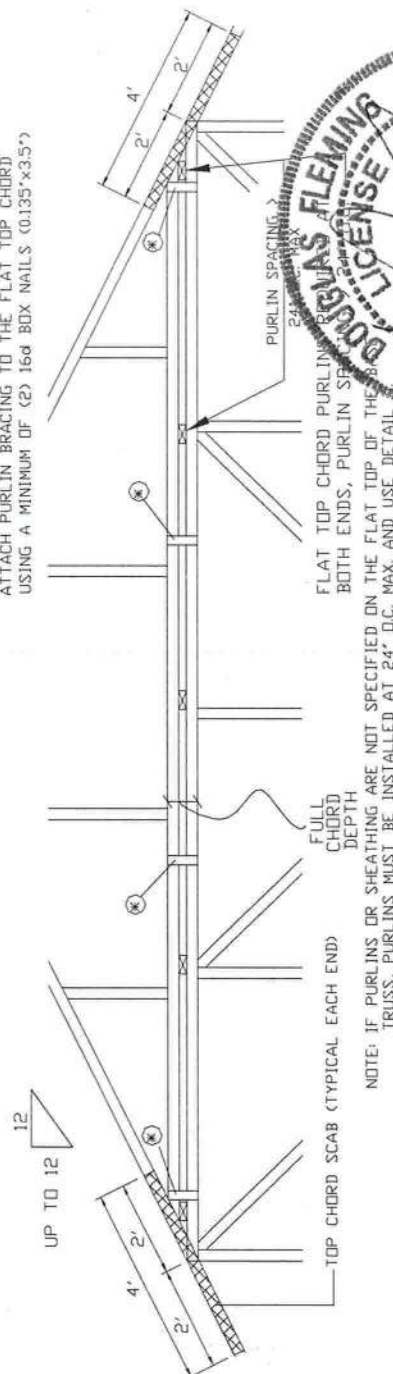
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING OR ANY OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS, AND LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS.

★ REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



DETAIL B : PURLIN SPACING > 24" O.C.



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following information for details.

Follow BCST Building Component Safety Information, by TPI and VITAC for safety practices pertaining to the installation of BCST products.

If you are having difficulty performing these functions, installers shall provide temporary bracing per BCST. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall be properly attached per BCST sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Corp. Inc. ITWBCCO shall not be responsible for any deviation from the design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, or bracing of trusses. ITWBCCO connector plates are made of 20/18/76GR (A3/JIS-K563 ASTM A992) steel, galvanized, 1/4" thick, G40, yield strength, 50ksi, max. Apply plates to each face of truss, positioned as shown above and on Joint Details.

A seal on this drawing or cover page indicates acceptance and professional engineering responsibility for the truss component design shown. The suitability and use of this component for any other application than that intended by ITW Building Components Corp. Inc. is the responsibility of the user.

ITW-Building Components Corp. Inc. 2
11W-Bldg., Westlakes Corporate Center, #200, Redwood City, CA 94063-1000
www.itwbuildingcomponents.com, www.tpi.com, www.bspn.com, www.sccsf.org



TW
Building Components Group Inc.

Earth City, MO 63045

✱ IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRULOX

USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4" O.C.FRONT TO BACK FACES.

APA RATED GUSSET

8"x8"x7/16" (MIN) APA RATED SHEATHING GUSSETS (EACH FACE), ATTACH @ 8" O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET, (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS, TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS

2x4 SPF#2, FULL CHORD DEPTH SCABS (EACH FACE):
ATTACH @ 8' O.C. WITH (6) 10d BOX NAILS (0.128"x3")
PER SCAB, (3) IN CAP BOTTOM CHORD AND (3) IN
BASE TRUSS TOP CHORD. SCABS MAY BE STAGGERED
1' O.C. FRONT TO BACK FACE.

28PB WAVE PIGGYBACK PLATE

ONE 28"PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8' D.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4' D.C. FRONT TO BACK FACES.

REF PIGGYBACK

DATE 03/15/10

DRWG PBI200310

SPACING	24.0"
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