

GENERAL NOTES

140 MPH, EXPOSURE B

THE FLORIDA BUILDING CODE 6TH EDITION 2017, BUILDING (FBCB) SECTION 2002.1 ALLOY SPECIFICATIONS, ALUMINUM STANDARDS AND DAQTA, AND ASCE 7-10 USING THE FOLLOWING CRITERIA AA ASM 35, ASTM ALUMINUM AND ALUMINUM THE DESIGN OF THIS STRUCTURE HAS BEEN REVIEWED FOR COMPLIANCE WITH

NOMINAL DESIGN WIND SPEED, Vasd = 108 M.P.H. ULTIMATE DESIGN WIND SPEED, Vult = 140 M.PH.(3 SECOND GUST)

EXPOSURE CATEGORY = B (WORSE CASE, ALL DIRECTIONS) **BUILDING RISK CATEGORY = I**

- +- 0.18 FOR ENCLOSURE STRUCTURES
- +- 0.55 FOR PARTIALLY ENCLOSED STRUCTURES +- 0.00 FOR OPEN STRUCTURES

DESIGN PRESSURE TAKEN FROM TABLE 2002.4 FBC 2017 6TH EDITION

4.

- BOLT CENTERS SHALL BE 2.5 TIMES BOLT DIAMETER. UNLESS NOTED SHALL BE LESS THAN ROCKWELL C35. MINIMUM SPACING BETWEEN HOT DIPPED TO A490 BOLTS SHALL NOT BE USED. BOLT HARDNESS GALVANIZED TO ASTM A153 OR ELECTRO-GALVANIZED TO ASTM B633. ALL BOLTS, NUTS, AND WASHER SHALL BE CARBON STEEL HOT DIPPED
- Ņ A123, A641, OR B633 OR NICKEL/CHROMIUM PLATED PER ASTM B456, HARDNESS LESS THAN C35. SCREWS SHALL BE ZINC COATED PER ASTM, SMS SHALL BE COATED OR PLATED CARBON STEEL WITH A ROCKWELL
- 7 6 5 4 9 ALL FASTENERS TO BE CARBON STEEL WITH A CORROSION RESISTANT COATING.
 - ALL CONCRETE ANCHORS SHALL BE A MINIMUM OF 2" FROM EDGE OF CONCRETE.
 - NO ALUMINUM SHALL BE WELDED OR EXPOSED TO EXTREME TEMPERATURE.
- ALL TAPCONS SHALL HAVE A MINIMUM OF 1-1/4" EMBEDMENT INTO CONCRETE. CHAIR RAIL CONNECTION AND NO MORE THAN 18" O.C. WITH #12X3/4 STS ON BOTH SELF MATING BEAMS (SMB) SHALL BE STITCHED AT EACH END, AT EACH PURLIN OR
- #12X2" SMS AT 24" O.C. MAX. AND WITHIN 2" OF EACH END UNLESS NOTED 1X2 EXTRUSION (LACING) SHALL BE FASTENED WITH EITHER TAPCONS OR

TYP CARRIER BEAM CONNECTION

- œ OTHERWISE. THIS INCLUDES ATTACHMENT OF FRONT WALL TO SIDE WALLS
- 9 2X3'S. USE 2X4'S BETWEEN 2X10 BEAMS OR POSTS. GIRTS 88" AND UNDER ARE TO BE 2X2'S AND GIRTS OVER 88" ARE TO BE
- 50 CHAPTER 18, FBCB. SUBSURFACE GEO-TECHNICAL INFORMATION HAS NOT 2000 PSF PER FBCB TABLE 1806.2. COMPACT FILL TO 95% MODIFIED ORGANIC MATERIAL WITH A MINIMUM ALLOWABLE BEARING PRESSURE OF CONDITIONS: LOOSE GRANULAR MATERIAL WITH NO APPRECIABLE CLAY OR FOOTINGS ARE DESIGNED FOR THE FOLLOWING ASSUMED SOIL BEARING BEEN PROVIDED TO THE ENGINEER. THEREFORE FOUNDATIONS AND CONCRETE FOUNDATIONS SHALL COMPLY WITH THE REQUIREMENTS OF

W/(2) ‡ DIA. X 1-1/2" LG. Q-SET ANCHORS INTO FOOTER OR EDGE

CABLE BRACING

OF SLAB

1-4" X 6-4" X 8" STRAP ATTACHED

2X POST

10 SMS (MIN)

CABLE PLATE ATTACHED W/(7)

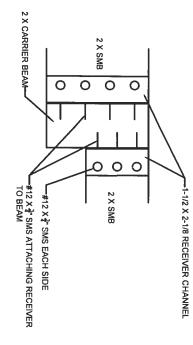
2X TOP

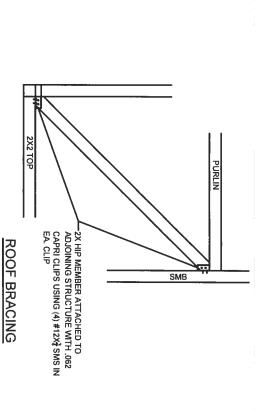
BTTM_PLATE SLAB

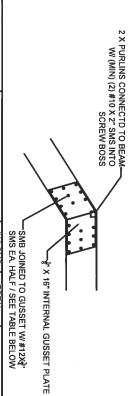
- MASONRY CONSTRUCTION SHALL CONFORM TO REQUIREMENTS OF CHAPTER 21, 2110, FBCB WITH HEAD JOINTS IN SUCCESSIVE COURSES OFFSET BY NOT LESS THAN GLASS UNIT MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION ONE-FOURTH THE UNIT LENGTH. THICKNESS OF BED JOINTS SHALL NOT EXCEED 5/8" MORTAR SHALL BE USED. ALL MASONRY SHALL BE LAID IN RUNNING BOND PATTERN FBCB. NET AREA COMPRESSIVE STRENGTH OF MASONRY IS 1500 PSI. TYPE M, OR S
- COMPRESSIVE STRENGTH IS 2000 PSI AT 28 DAYS UNLESS OTHERWISE NOTED REQUIREMENTS OF ASTM C476 AND CHAPTER 21, FBCB. REQUIRED MINIMUM GROUT USED TO FILL CELLS, LINTELS AND BOND BEAMS SHALL CONFORM TO

12

- 13. CONCRETE SHALL CONFORM TO REQUIREMENTS OF CHAPTER 19, FBCB, AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS UNLESS OTHERWISE
- CONFORM TO REQUIREMENTS OF SECTION 1910, FBCB. CONFORM TO ASTM A 185 REQUIREMENTS. WIRE FABRIC SHALL BE SUPPORTED AS REQUIRED IN SECTION 1910, FBCB. SYNTHETIC FIBER REINFORCEMENT SHALL CONFORMING TO ASTM A82 REQUIREMENTS. WELDED WIRE FABRIC SHALL REQUIREMENTS. JOINT REINFORCING IF USED, SHALL BE 9 GAGE, GALVANIZED STEEL BARS SHALL BE DEFORMED BILLET STEEL BARS AND COMPLY WITH ASTM A 615 FOUNDATION WALLS, AND CMU WALLS UNLESS OTHERWISE NOTED. REINFORCING REINFORCING BARS SHALL BE GRADE 40 OR 60 MINIMUM IN FOUNDATIONS, MASONRY

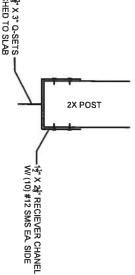




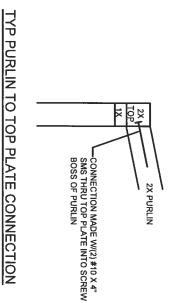


Т	_					_		
	2X9X0.082X0.306 SMB	2X8X0.072X0.224 SMB	2X7X0.050X0.120 SMB	2X6X0.050X0.120 SMB	2X4X0.044X0.100 SMB	BEAM	TABLE	BEAM SPLICE DETAILS
	3/16"	3/16"	1/8"	1/8"	1/8"	THICKNESS LENGTH	SPLICE PLATE DIMENSIONS	
	8°	18"	16"	16"	16"	LENGTH	ATE DIMEN	
	8.25*	7.25"	6.25"	5.25"	4.25*	HEIGHT	SNOIS	
3	18	16	14	10	7		PER END	#12 SMS
	72	64	56	40	24		PER SPLICE	TOTAL
	4	4	3	3	3	HORIZONTAL	NUMBER OF SPACES	FASTNER SPACES
	4	3	3	3	3	VERTICAL	SPACES	PACES

IANSERED BEAM CONNECTION



(4) g X 3" Q-SETS —— ATTACHED TO SLAB	
\ [
Δ	2X POST
L	
1½" X 2½" RECIEVER (W/ (10) #12 SMS EA.	



TYP 2X POST TO CONCRETE DETAIL

Contract STRUCTURAL SYSTEM FOR THIS BUILDING HAVE BEEN REVIEWED COMPLIANOF WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE EQITION 2013 VSING ASCE/SEI 7-10 140 M.P.H. 3 SECOND GUST.

Prelim: Final: Date: Project: SHEET O T BW-8309-4 SIPU Michael A. Robinson

STRUCTURAL ENGINEERING, INC. MICHAEL A ROBINSON, P.E.

INDUSTRIAL, COMMERCIAL, and RESIDENTIAL 921 SHADOW DRIVE SUITE No. 3 LAKELAND, FLORIDA 33809

DRAF' **BLUEWA1** TING, INC.

EMAIL: plans@bluewaterdrafting.com 1075 W. Gulf to Lake bury, Lecanto, Fl 34461 Citrus PH: 352436-4225 Marion PH: 352-522-0650

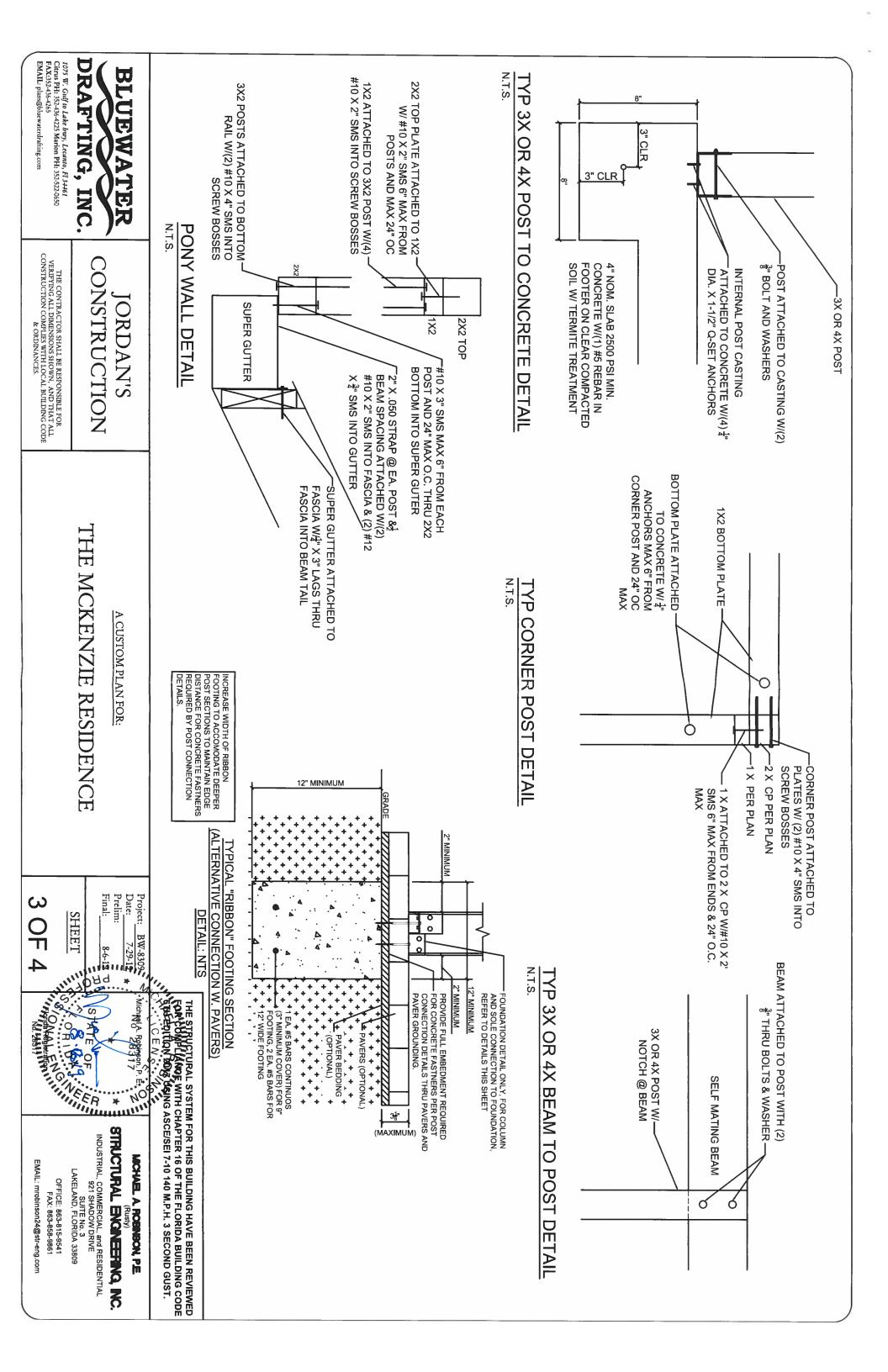
CONSTRUCTION JORDAN'S

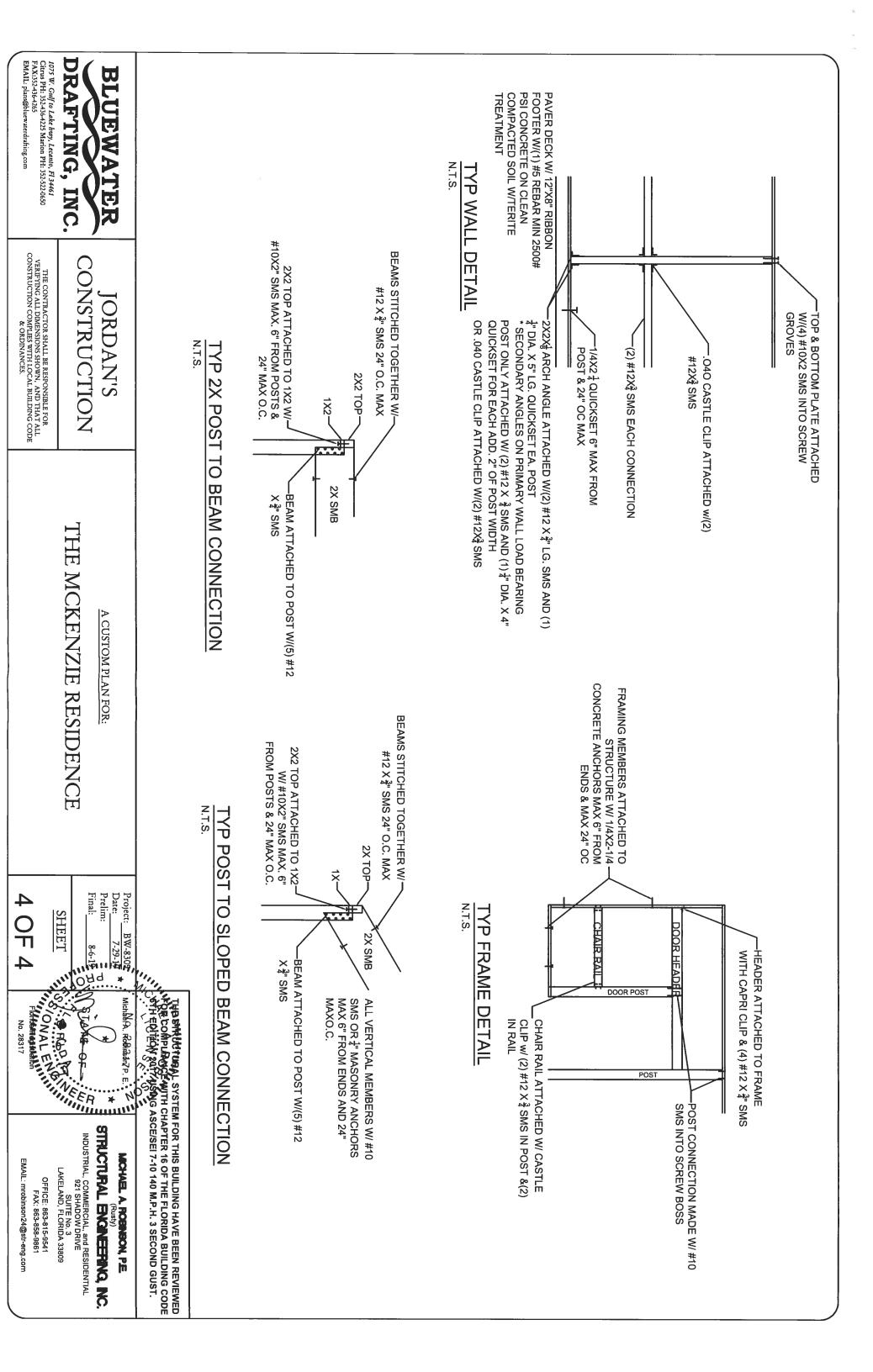
THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS SHOWN, AND THAT ALL CONSTRUCTION COMPLIES WITH LOCAL BUILDING CODE

THE MCKENZIE RESIDENCE

A CUSTOM PLAN FOR:

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