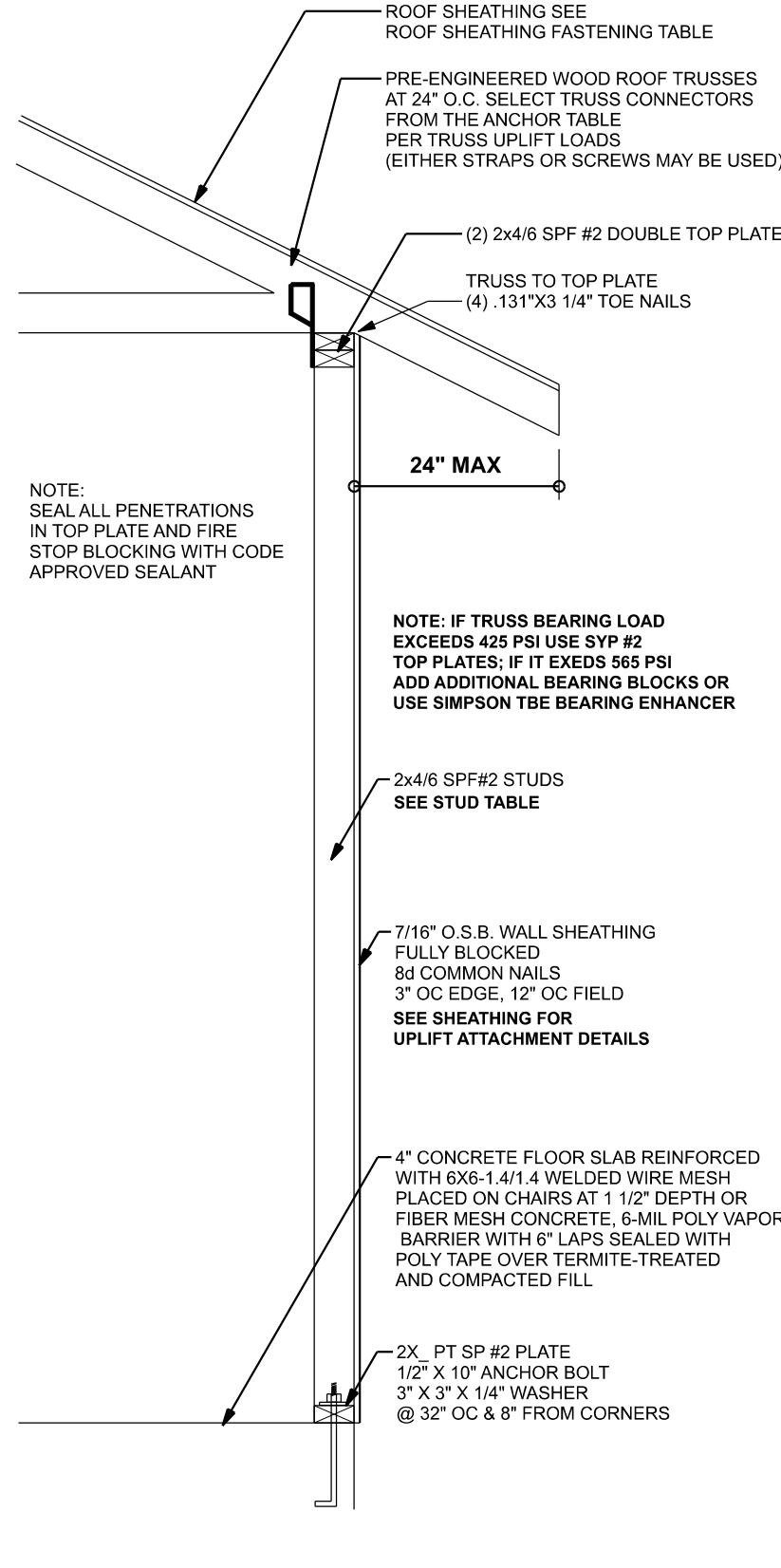
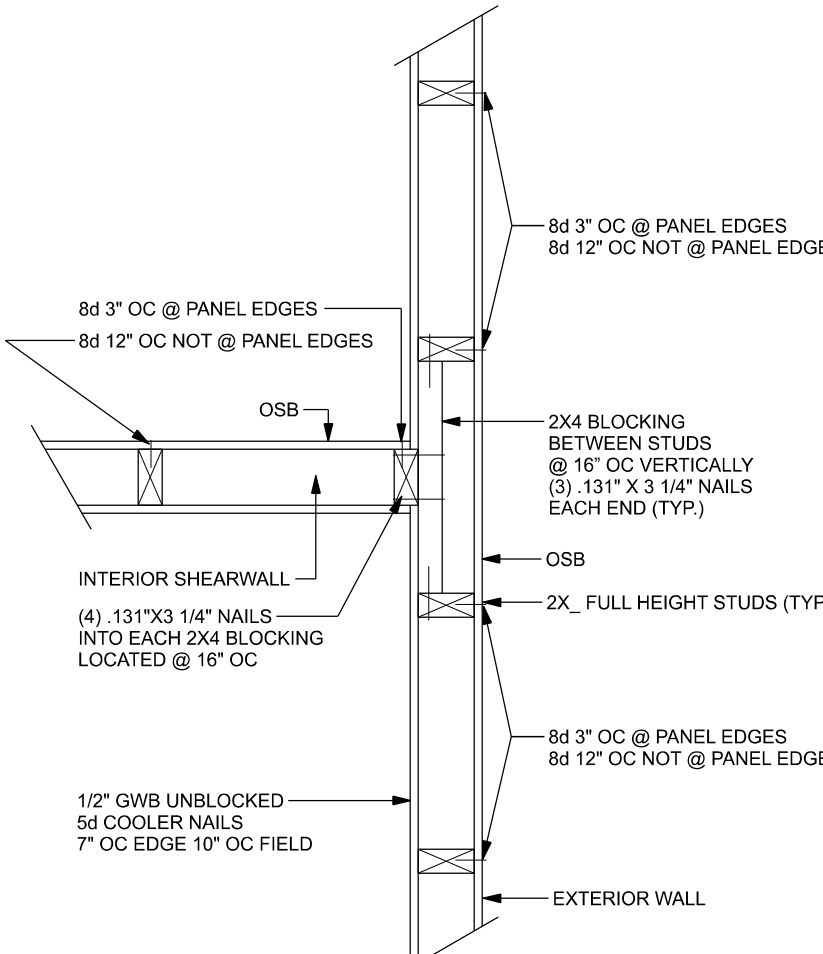


APPROVED

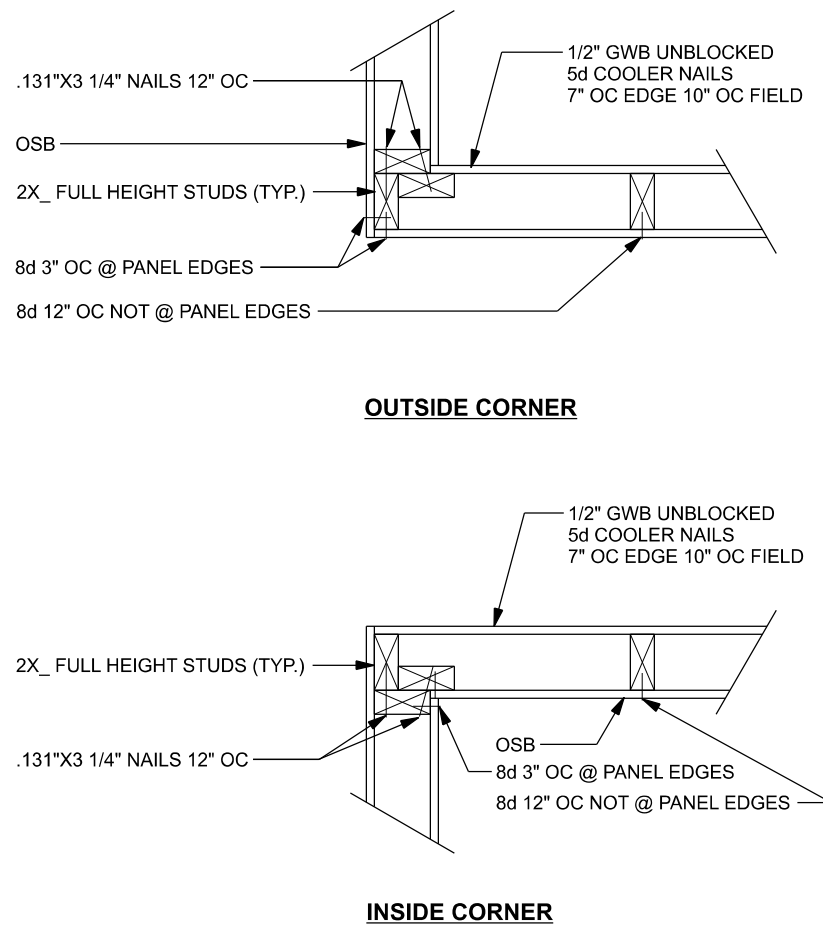
By troy crews at 7:37 am, May 01, 2025



ONE STORY WALL SECTION  
SCALE: 3/4\"/>



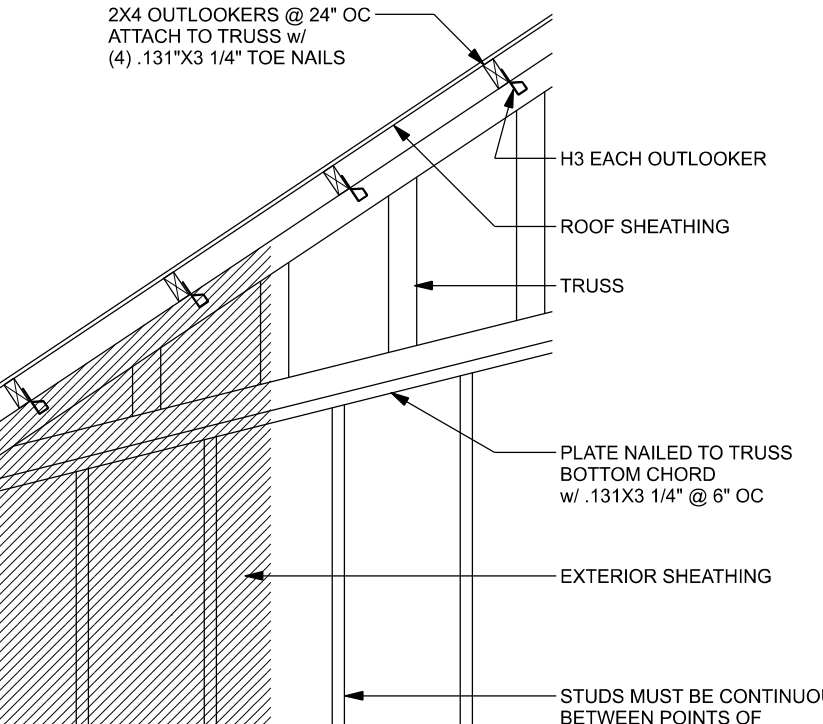
(TYP.) INTERSECTING WALL FRAMING  
WOOD FRAME



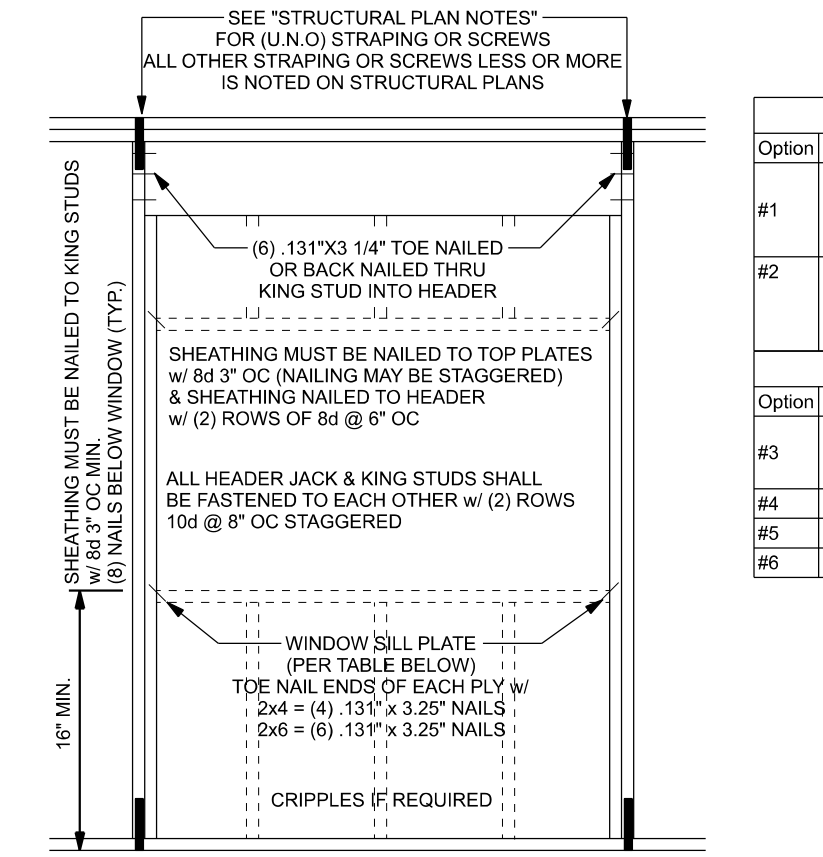
(TYP.) CORNER FRAMING  
WOOD FRAME

| ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49) |                                    |  |                                |   |  |
|---|------------------------------------|--|--------------------------------|---|--|
| Wind Speed  | Sheathing Thickness Plywood Or OSB | Required Nail  | Nail spacing along panel edges | Nail spacing along intermediate supports in the panel field |  |
| 120 mph Exp. B  | 7/16"                              | ASTM F1667 RRSR-01 (2 3/8" x 0.131")                                     | 6" oc                          | 12" oc  |  |
| 120 mph Exp. C  | 7/16"                              | ASTM F1667 RRSR-01 (2 3/8" x 0.131")                                     | 6" oc                          | 6" oc   |  |
| 120 mph Exp. D  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 130 mph Exp. B  | 7/16"                              | ASTM F1667 RRSR-01 (2 3/8" x 0.131")                                     | 6" oc                          | 6" oc   |  |
| 130 mph Exp. C  | 19/32"                             | ASTM F1667 RRSR-01 (2 3/8" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 130 mph Exp. D  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 140 mph Exp. B  | 7/16"                              | ASTM F1667 RRSR-01 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 140 mph Exp. C  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 140 mph Exp. D  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 150 mph Exp. B  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 150 mph Exp. C  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 6" oc                          | 6" oc   |  |
| 150 mph Exp. D  | 19/32"                             | ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120") | 4" oc                          | 4" oc   |  |

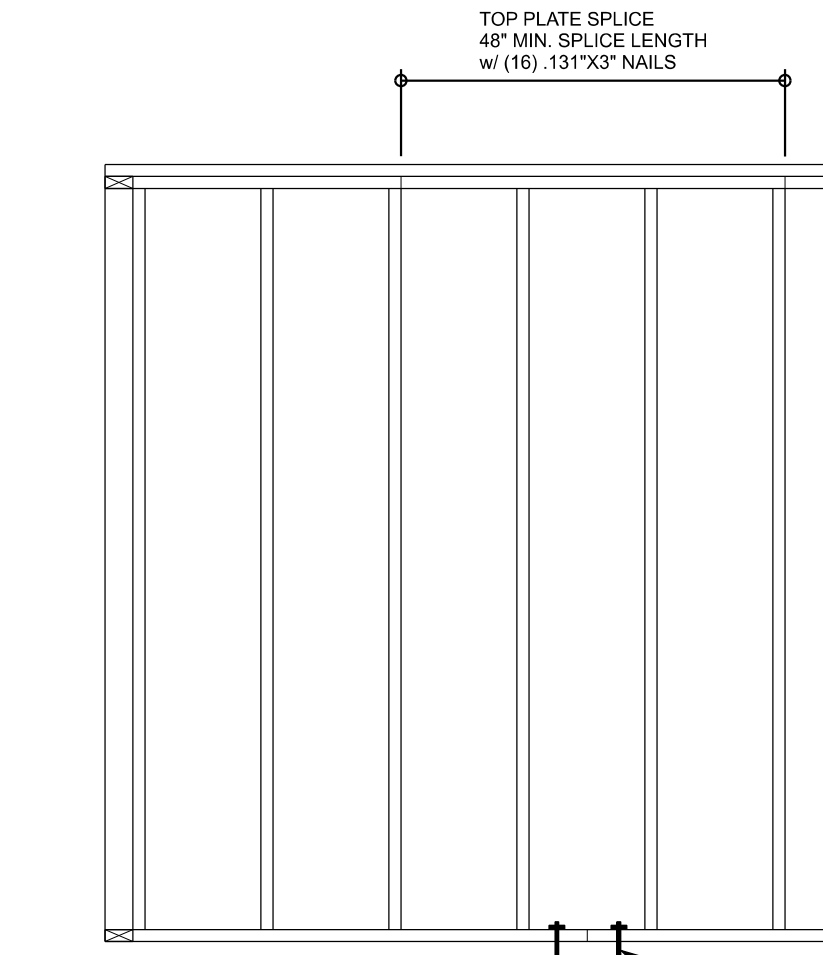
Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



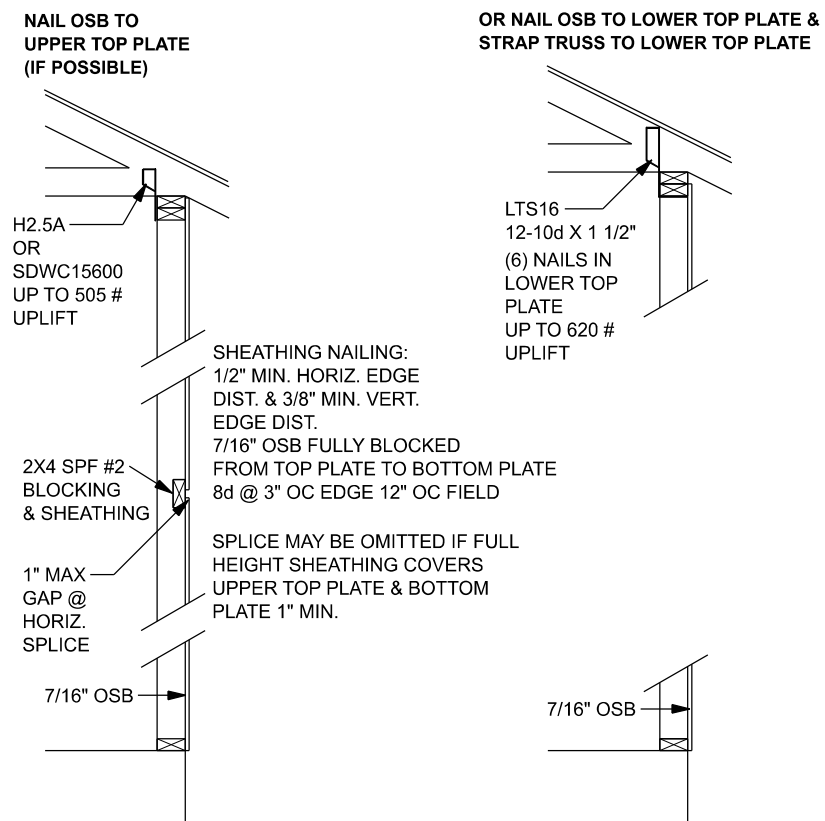
(TYP.) GABLE WALL w/ VAULTED CEILING  
WOOD FRAME



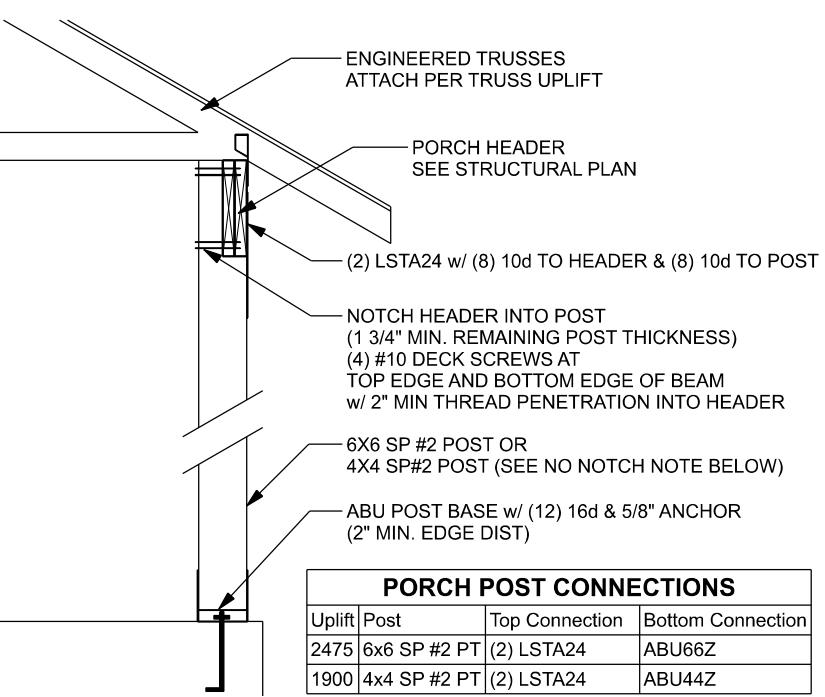
TYPICAL HEADER STRAPPING OR SCREWS DETAIL  
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS  
ONE STORY WOOD FRAME



SHEATHING FOR UPLIFT ATTACHMENT DETAILS  
ONE STORY WOOD FRAME



POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:  
EAVE / HIP ROOF:  
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)  
- USE LONGER STRAPS NEEDED IF TOP PLATES ARE SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

(TYP.) PORCH POST  
ONE STORY WOOD

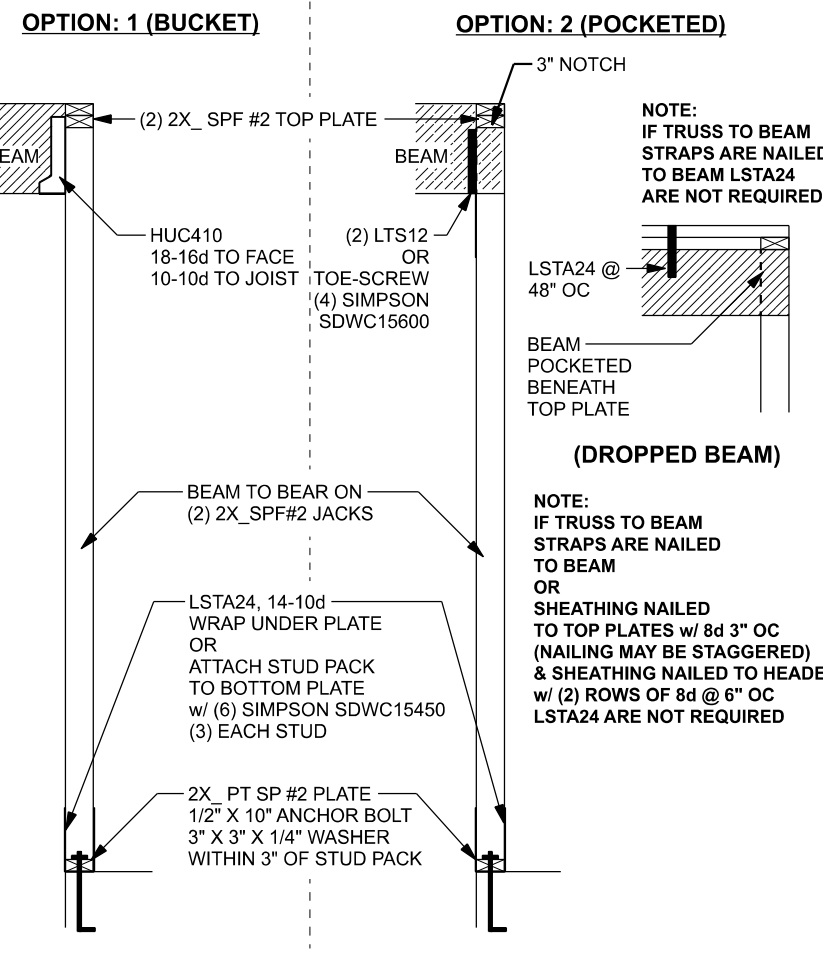
| CONNECTOR TABLE |            |                        |                        |                          |  |
|-----------------|------------|------------------------|------------------------|--------------------------|--|
| Uplift SP       | Uplift SPF | Truss Connector        | To Plate               | To Truss/Rafter          |  |
| 805             | 505        | SDWC15600              | -                      | -                        |  |
| 415             | 290        | H3                     | 4-8d x 1 1/2"          | 4-8d x 1 1/2"            |  |
| 615             | 540        | H2.5A                  | 5-8d x 1 1/2"          | 5-8d x 1 1/2"            |  |
| 1340            | 1015       | H10A                   | 9-10d1 1/2"            | 9-10d1 1/2"              |  |
| 720             | 620        | LTS12-20               | 6-10d1 1/2"            | 6-10d1 1/2"              |  |
| 1000            | 860        | MTS12-30               | 7-10d1 1/2"            | 7-10d1 1/2"              |  |
| 1450            | 1245       | HTS20-30               | 12-10d1 1/2"           | 12-10d1 1/2"             |  |
| Uplift SP       | Uplift SPF | Strap Ties             | To One Member          | To Other Member          |  |
| 1235            | 1235       | LSTA21                 | 8-10d                  | 8-10d                    |  |
| 1640            | 1455       | MSTA24                 | 9-10d                  | 9-10d                    |  |
| 1030            | 1030       | CS20                   | 7-10d                  | 7-10d                    |  |
| Uplift SP       | Uplift SPF | Stud Plate Ties        | To Stud                | To Plate                 |  |
| 585             | 535        | SP1                    | 6-10d                  | 4-10d                    |  |
| 1065            | 905        | SP2                    | 6-10d                  | 6-10d                    |  |
| 771             | 771        | LSTA24                 | 10-10d                 | wrap under or over plate |  |
| 1235            | 1235       | LSTA24                 | 14-10d                 | wrap under or over plate |  |
| Uplift SP       | Uplift SPF | Holdowns @ Stemmwall   | To Stud / Post         | Anchor                   |  |
| 1825            | 1800       | DTT22                  | 8-SDS 1/4"x1 1/2"      | 1/2"x12" Titen HD        |  |
| 4235            | 3640       | HTT4                   | 18-16d x 12"           | 1/2"x12" Titen HD        |  |
| Uplift SP       | Uplift SPF | Holdowns @ Mono        | To Stud / Post         | Anchor                   |  |
| 1825            | 1800       | DTT22                  | 8-SDS 1/4"x1 1/2"      | 1/2"x12" Titen HD        |  |
| 4235            | 3640       | HTT4                   | 18-16d x 12"           | 1/2"x12" Titen HD        |  |
| Uplift SP       | Uplift SPF | Post Bases @ Stemmwall | To Post                | Anchor                   |  |
| 1900            | ABU44Z     | 12-16d                 | 5/8"x12" Drill & Epoxy |                          |  |
| 2475            | ABU66Z     | 12-16d                 | 5/8"x12" Drill & Epoxy |                          |  |
| Uplift SP       | Uplift SPF | Post Bases @ Mono      | To Post                | Anchor                   |  |
| 1900            | ABU44Z     | 12-16d                 | 5/8"x7" Drill & Epoxy  |                          |  |
| 2475            | ABU66Z     | 12-16d                 | 5/8"x7" Drill & Epoxy  |                          |  |

| 1 PLY GIRDER TRUSS HOLD DOWN TABLE |                |  |
|------------------------------------|----------------|--|
| Uplift                             | Top Connection | Bottom Connection  |
| up to 850 lb                       | (3) SDWC15600  | (3) SDWC15600 w/ anchor bolt within 6"                   |
| up to 1125 lb                      | HTS16          | LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6" |
| up to 2250 lb                      | (2) HTS16      | HTT4   |

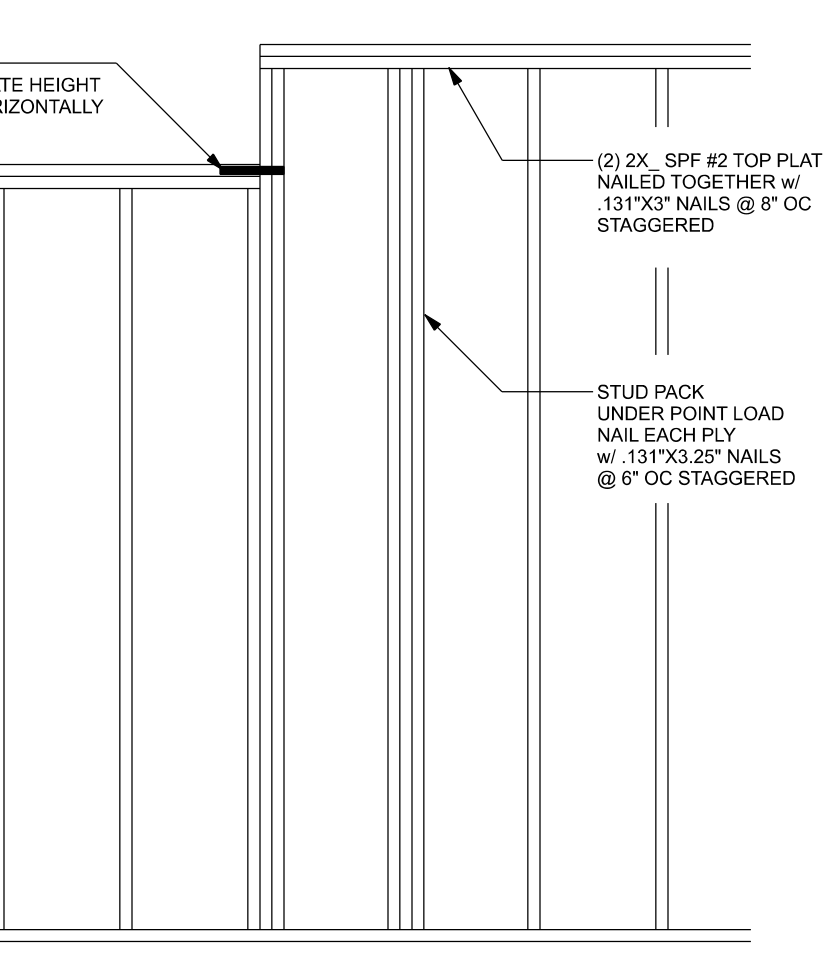
| MULTI PLY GIRDER TRUSS HOLD DOWN TABLE |                |  |
|--|----------------|--|
| Uplift                                 | Top Connection | Bottom Connection  |
| up to 1125 lb                          | HTS16          | LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6" |
| up to 1700 lb                          | (4) SDWC15600  | HTT4   |
| up to 2250 lb                          | (2) Each Ply   | HTT4   |
| up to 3375 lb                          | (3) HTS16      | HTT4   |

STUD PACK UNDER GIRDER TRUSS  
USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB OF LOAD NAIL EACH PLY w/ 131\"/>

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL  
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL  
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS  
WOOD FRAME

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6S RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 2500 PSI.

WELED WIRE REINFORCED SLAB: 6" x 6" W14 x 4, F<sub>y</sub> = 80ksi, WELED WIRE REINFORCEMENT FABRIC W/WM1 CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 10 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.15 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. REINFORCEMENT C-116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND DESIGNER APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP SPLICES 40" DB (25' FOR 45 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-16, UNO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO DRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS' INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROTTED CMU.

## BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE REQUIREMENTS FOR THE SITED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN ON ONE A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCE, IS BASED ON REACTING UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## EXTERIOR WALL STUD TABLE FOR SPSF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20BS, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (ENZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

| (1) 2x4 @ 16" OC | TO 10'-1" STUD HEIGHT |
|------------------|-----------------------|
| (1) 2x4 @ 12" OC | TO 11'-2" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-7" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 17'-3" STUD HEIGHT |

## GRADE & SPECIES TABLE

|      |              | Fb   | E   |
|------|--------------|------|-----|
| 2x8  | SP #2        | 925  | 1.4 |
| 2x10 | SP #2        | 800  | 1.4 |
| 2x12 | SP #2        | 750  | 1.4 |
| GLB  | 24F-V3 SP    | 2600 | 1.9 |
| LVL  | TIMBERSTRAND | 1700 | 1.7 |
| LVL  | MICROLAM     | 2350 | 2.0 |
| PSL  | PARALAM      | 2900 | 2.0 |

## DESIGN CRITERIA & LOADS:

|  |  |
|--|--|
| BUILDING CODE                                  | 8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023) |
| CODE FOR DESIGN LOADS                          | ASCE 7-22  |
| WINDLOADS                                      |  |
| BASIC WIND SPEED (ASCE 7-22, 3S GUST)          | 130 MPH  |
| WIND EXPOSURE (BUILDER MUST FIELD VERIFY)      | C  |
| TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY) | I  |
| RISK CATEGORY                                  | II   |
| ENCLOSURE CLASSIFICATION                       | ENCLOSED   |
| INTERNAL PRESSURE COEFFICIENT                  | 0.18   |
| ROOF ANGLE                                     | 7-45 DEGREES   |
| MEAN ROOF HEIGHT                               | 30 FT  |
| C&C DESIGN PRESSURES                           | SEE TABLE  |
| FLOOR LOADING                                  |  |
| ROOMS OTHER THAN SLEEPING ROOM                 | 40 PSF LIVE LOAD                                     |
| SLEEPING ROOMS                                 | 30 PSF LIVE LOAD                                     |
| ROOF LOADING                                   |  |
| FLAT OR < 4:12                                 | 20 PSF LIVE LOAD                                     |
| 4:12 TO < 12:12                                | 16 PSF LIVE LOAD                                     |
| 12:12 & GREATER                                | 12 PSF LIVE LOAD                                     |
| SOIL BEARING CAPACITY                          | 1500 PSF   |
| FLOOD ZONE                                     | THIS BUILDING IS NOT IN THE FLOOD ZONE               |

| COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) |                 |                                      |              |
|---|-----------------|--------------------------------------|--------------|
| EFFECTIVE WIND AREA (F <sub>T2</sub> )                | ZONE 4 INTERIOR | ZONE 5 END 4 FROM ALL OUTSIDE CORNER |              |
| 0 - 20  | +25.6(Vasid)    | -27.8(Vasid)                         | +25.6(Vasid) |
| 0 - 20  | +42.6(VuId)     | -46.2(VuId)                          | +42.6(VuId)  |

Bryan Zecher Construction

Mullins, Tyrone & Thayla

PROJECT ADDRESS:  
Parcel: 27-25-16-01770-241  
430 NW Mule Way, Lake City  
Columbia County, Florida

FL PE 53915  
This item has been digitally signed and sealed by Mark Disoway PE on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C000017E97DE07CA000746F0, CN=Mark d Disoway  
2025-01-17 11:53:04

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

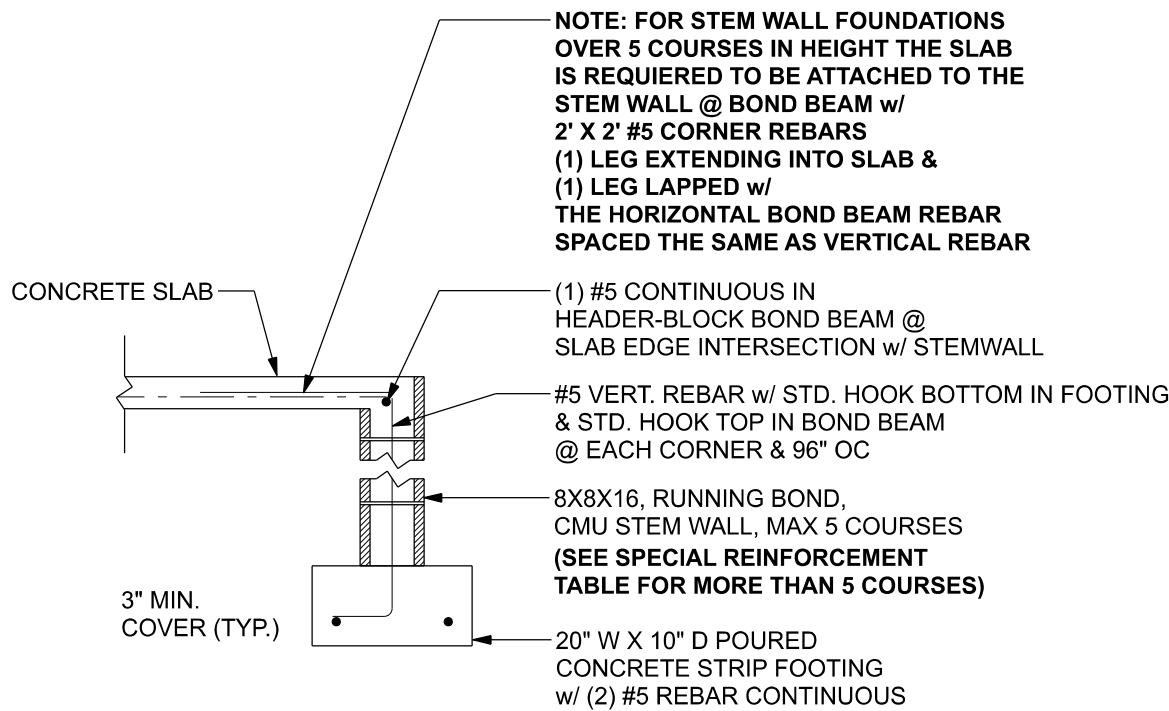
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163 SW Midtown Place  
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386.754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
250013

S-1

OF 3 SHEETS



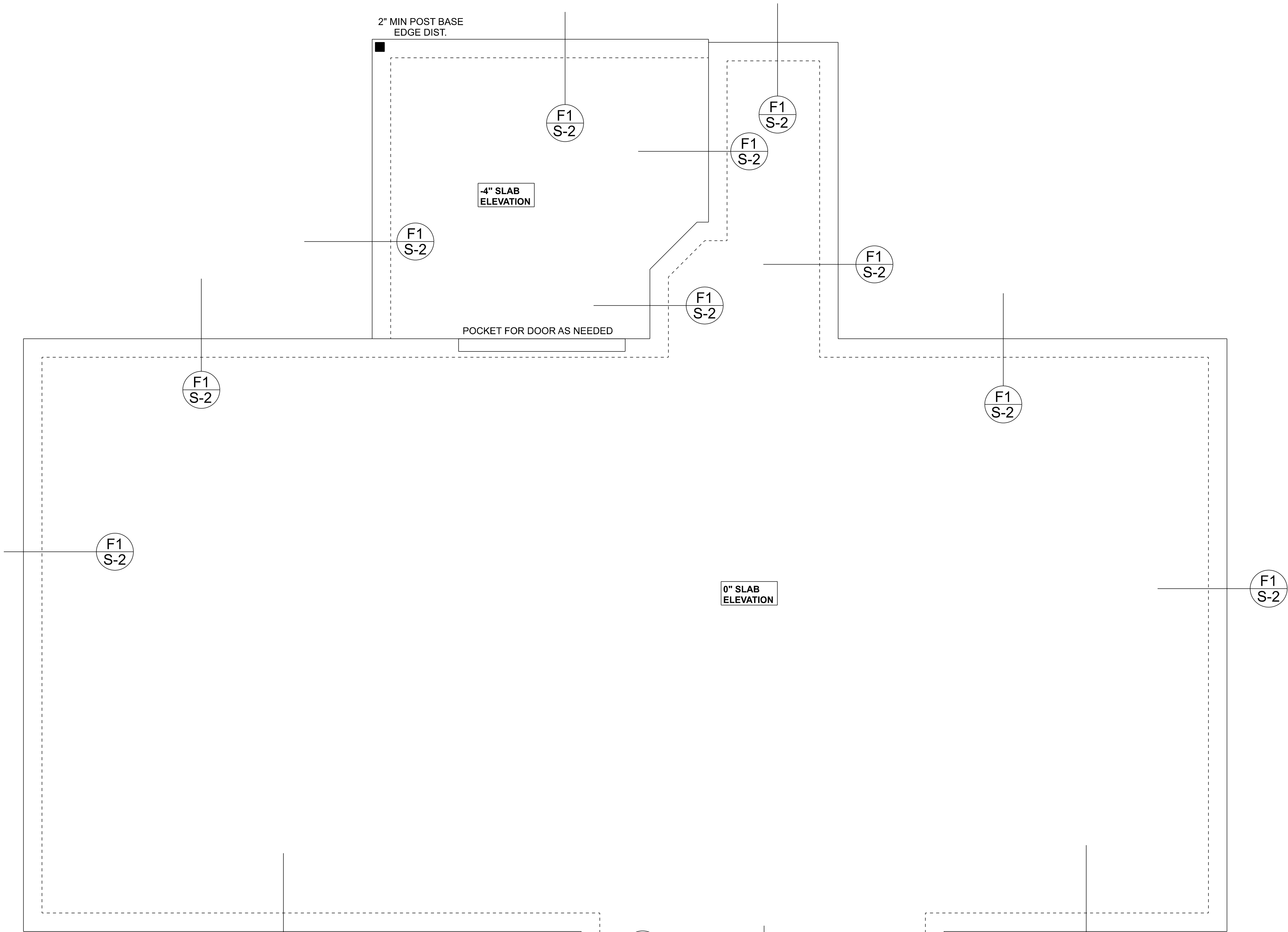


**F1 S-2** OPTIONAL STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"

| TALL STEM WALL TABLE:  |                            |  |    |    |   |    |    |    |
|--|----------------------------|--|----|----|---|----|----|----|
| The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below. |                            |  |    |    |   |    |    |    |
| STEMWALL HEIGHT (FEET)   | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.) |    |    | VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.) |    |    |    |
|  |                            | #5   | #7 | #8 | #5  | #7 | #8 |    |
| 3.3  | 3.0                        | 96   | 96 | 96 | 96  | 96 | 96 | 96 |
| 4.0  | 3.7                        | 96   | 96 | 96 | 96  | 96 | 96 | 96 |
| 4.7  | 4.3                        | 88   | 96 | 96 | 96  | 96 | 96 | 96 |
| 5.3  | 5.0                        | 56   | 96 | 96 | 96  | 96 | 96 | 96 |
| 6.0  | 5.7                        | 40   | 80 | 96 | 80  | 96 | 96 | 96 |
| 6.7  | 6.3                        | 32   | 56 | 80 | 56  | 96 | 96 | 96 |
| 7.3  | 7.0                        | 24   | 40 | 56 | 40  | 80 | 96 | 96 |
| 8.0  | 7.7                        | 16   | 32 | 48 | 32  | 64 | 80 | 80 |
| 8.7  | 8.3                        | 8  | 24 | 32 | 24  | 48 | 64 | 64 |
| 9.3  | 9.0                        | 8  | 16 | 24 | 16  | 40 | 48 | 48 |

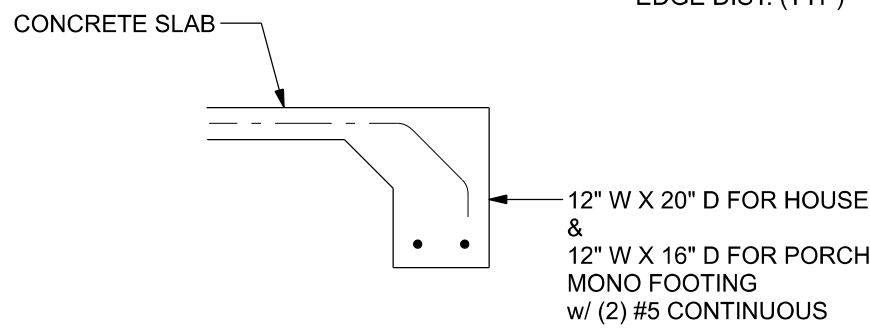
| MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING. |  |
|--|--|
| ACI 530.1-02 Section   | Specific Requirements  |
| 1.4A Compressive strength  | 8" block bearing walls Fm = 1500 psi   |
| 2.1 Mortar   | ASTM C 270, Type N, UNO  |
| 2.2 Grout  | ASTM C 476, admixtures require approval  |
| 2.3 CMU standard   | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block   |
| 2.3 Clay brick standard  | ASTM C 216-02, Grade SW, Type FBS, 8"x2 1/2"x11 1/2"   |
| 2.4 Reinforcing bars, #3 - #11   | ASTM A615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)   |
| 2.4F Coating for corrosion protection  | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS   |
| 2.4F Coating for corrosion protection  | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS |
| 3.3.E.2 Pipes, conduits, and accessories   | Any not shown on the project drawings require engineering approval.  |
| 3.3.E.7 Movement joints  | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.  |

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"

| FOUNDATION NOTES |   |
|------------------|---|
| FN - 1           | DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.   |
| FN - 2           | CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.  |
| FN - 3           | THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 8X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD). |



**F1 S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"

Bryan Zecher Construction

Mullins, Tyrone & Thayla

PROJECT ADDRESS:  
Parcel: 27-2S-16-01770-241  
430 NW Mule Way, Lake City  
Columbia County, Florida

FL PE 53915  
This item has been digitally signed and sealed by Mark Disosway P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

C=US, O=Florida, dnQualifier=A01410C0000017E97DE07CA000746F0, CN=Mark d Disosway  
2025-01-17 11:53:26

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.  
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JOB NUMBER:  
250013

**S-2**  
OF 3 SHEETS

