

DE,
DANSCO ENGINEERING, LLC

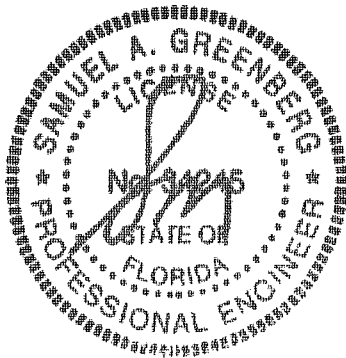
P.O. Box 3400
Apollo Beach, FL 33572
Telephone (813) 645-0166
Facsimile (813) 645-9698

The truss drawing(s) listed below have been prepared by **Simpson Strong-Tie Co.** under my direct supervision based on the parameters provided by the truss designers.

Job: ~~KENTLEM GARAGE~~ DAVID FLYNN

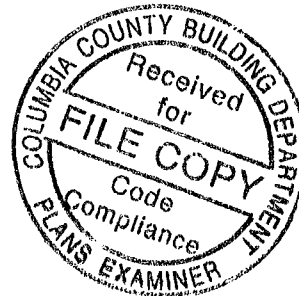
2 truss design(s)

33291-W1



4/14/14

Samuel A. Greenberg, P.E.
FL Reg. #34245
CA 25948



Note: Gable end frames with stud lengths exceeding 4' require permanent bracing. On structural gables, where studs may be made from two or more boards as they cross diagonals, the 4' length is the distance from the top chord to bottom chord.

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1-2007 Chapter 2.

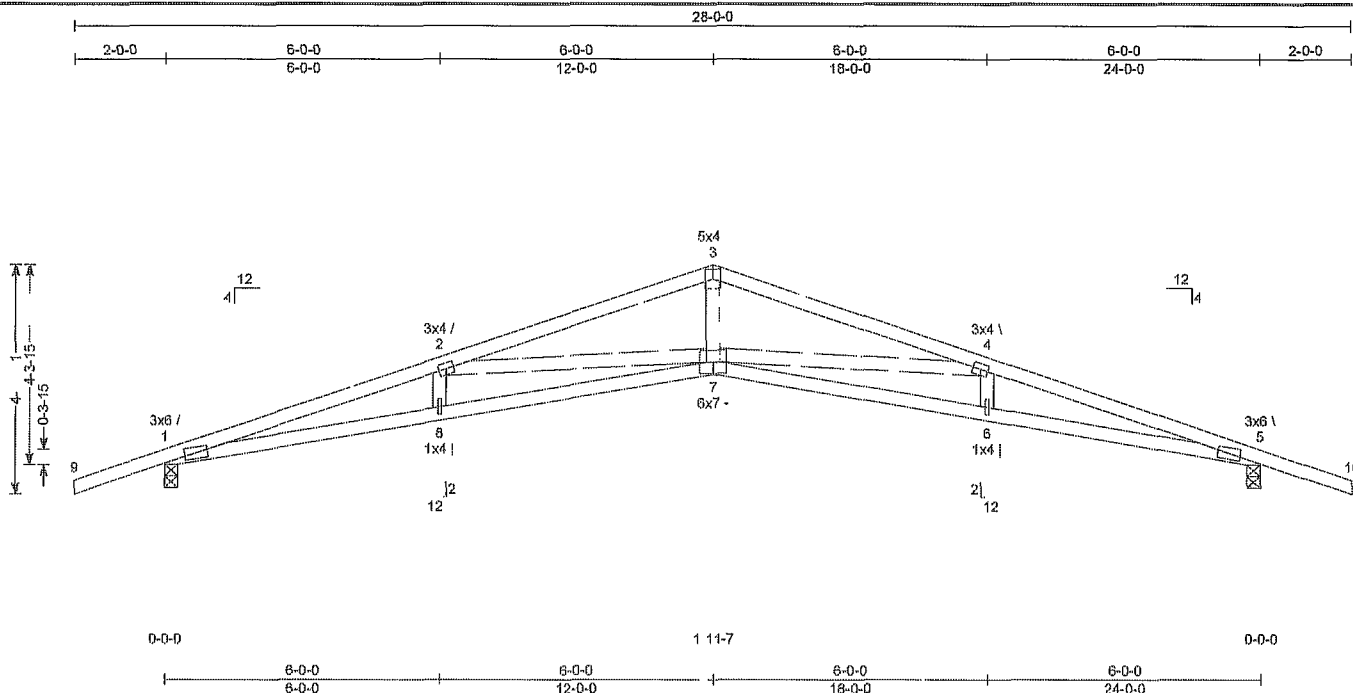
Warning I—Verify design parameters and read notes before use.

These designs are based only upon parameters shown, and are for individual building components to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer — not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 *Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

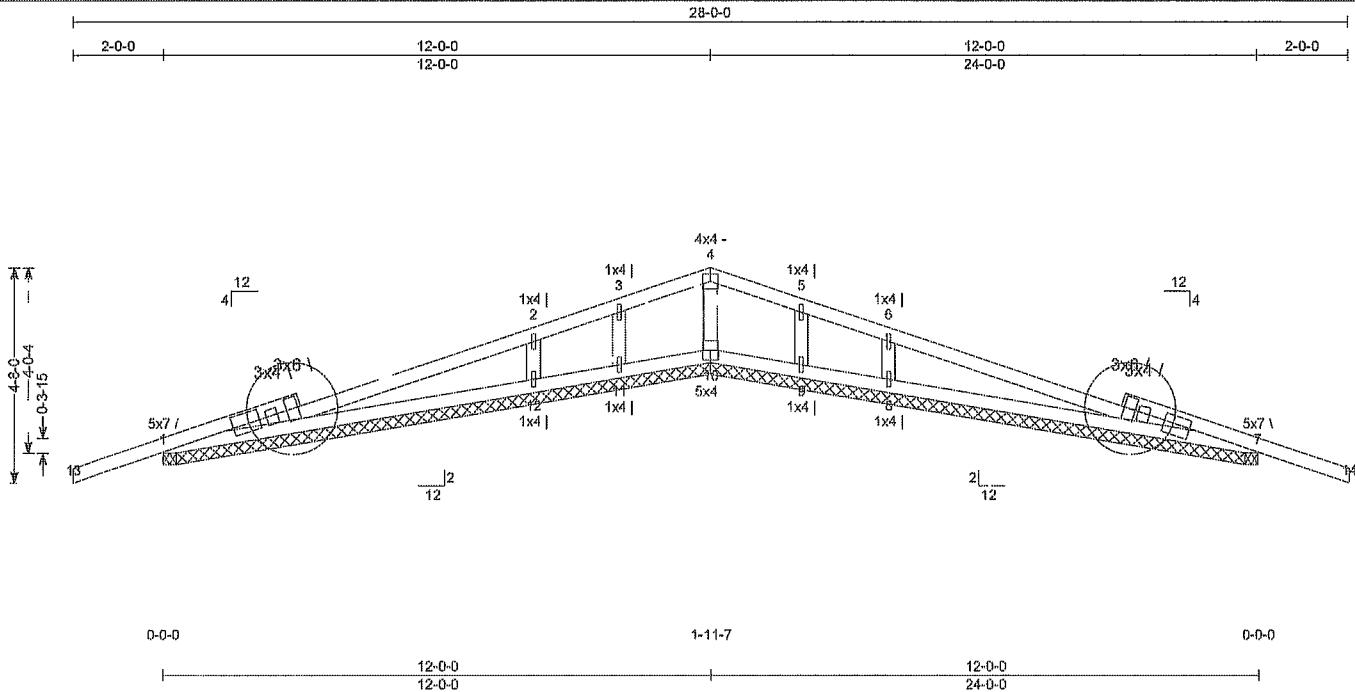
Simpson Strong-Tie

Truss: A1
JobName: ken klem garage
Designer: JASON
Date: 04/11/14 12 04.23
Page: 1 of 1

Span 24-0-0 Pitch 4 / 12 Qty 17 OHL 2-0-0 OHR 2-0-0 CANT L 0-0-0 CANT R 0-0-0 PLYS 1 Spacing 24 in WGT/PLY 97 lbs



Span 24-0-0 Pitch 4 /12 Qty 17 OHL 2-0-0 OHR 2-0-0 CANT L 0-0-0 CANT R 0-0-0 PLYS 1 Spacing 24 in WGT/PLY 90 lbs



Loading	General	CSI Summary	Deflection	L/	(loc)	Allowed
Load (psf)	Bldg Code	TC 0.60 (6-7)	Vert TL 0.18 in	L / 999	(7-7)	L / 180
TCLL 20	FBC 2010/ TPI 1-2007	BC 0.53 (7-8)	Vert LL 0.1 in	L / 999	(7-7)	L / 240
TCDL 10	Rep Mbr Increase Yes	Web 0.02 (7-12)	Horz TL 0.01 in		7	
BCLL 0	D.O.L. 125 %		Creep Factor Kcr = 1.5			
BCDL 7						
Plate Offsets (in X,Y,Ang): (1) 1-9-12, 7-10, 18 x 4 0-0, 3-11 (0) (7) 1-9-10, 6 12 18, x 10 0-0, 3-4-0						

Reaction Summary

JT	Type	Brig Combo	Brig Width	Material	Max React	Ave React	Max Grav Uplift	Max Wind Uplift	Max Uplift	Max Horiz
1	Continuous	1	288 in	Spruce-Pine-Fir	549 lbs	88 plf	-	-83 lbs	-83 lbs	11 lbs

Material Summary

TC SP (ALSC6-2013) #1 2 x 4
BC SP (ALSC6-2013) #1 2 x 4
Web SP (ALSC6-2013) #1 2 x 4

Bracing Summary

TC Bracing: Sheathed or Purlins at 6-10-0. Purlin design by Others.
BC Bracing: Sheathed or Purlins at 6-3-0. Purlin design by Others.

Loads Summary

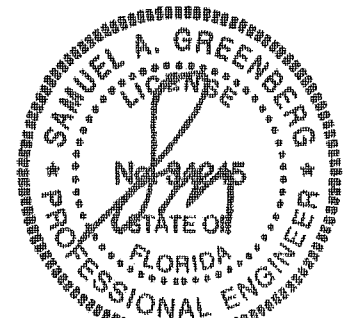
1) This truss has been designed for the effects of wind loads in accordance with ASCE 7-10 with the following user defined input: 130 mph ultimate, Exposure B, Overall Bldg Dims 24 ft x 16 ft, h = 15 ft, End Zone Truss, Both ends and webs considered. DOL = 1.60 CC Zone Width 3 ft.

Member Forces Summary

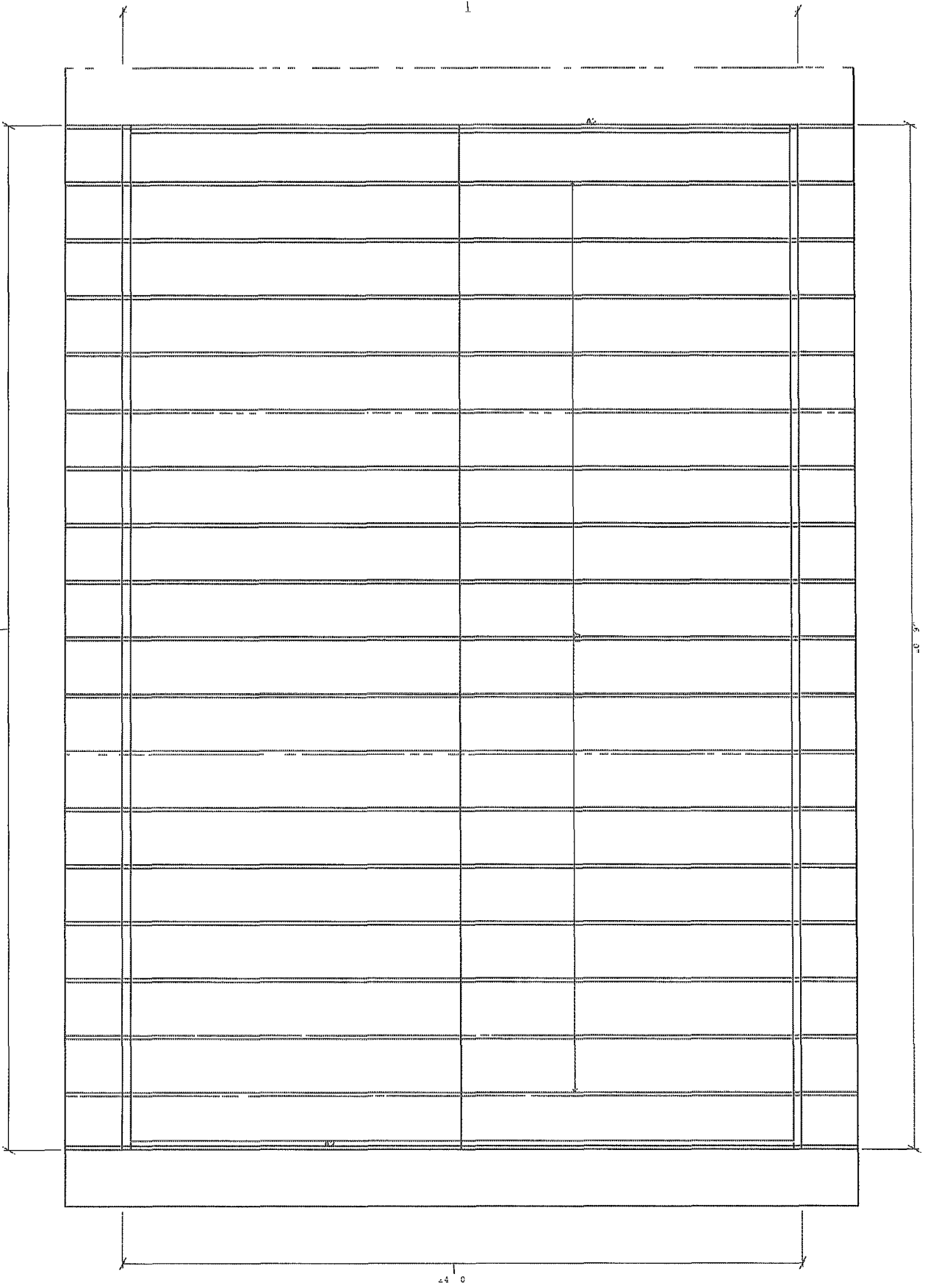
TC	1-2	0.602	-658 lbs	1-4	0.066	158 lbs	(-98 lbs)	5-6	0.185	139 lbs	(-137 lbs)	
	2-3	0.185	-142 lbs	4-5	0.066	150 lbs	(-93 lbs)	6-7	0.602	-658 lbs		
BC	7-8	0.777	677 lbs	(-539 lbs)	9-10	0.039	314 lbs	(133 lbs)	11-12	0.181	301 lbs	(131 lbs)
	8-9	0.180	303 lbs	(131 lbs)	10-11	0.060	314 lbs	(133 lbs)	12-1	0.527	677 lbs	(-539 lbs)
Webs	2-12	0.024	-249 lbs		4-10	0.023	-226 lbs		6-8	0.024	-249 lbs	
	3-11	0.014	116 lbs	(-66 lbs)	5-9	0.014	116 lbs	(-66 lbs)				

Notes:

- 1) This design is based on ASCE 7-10 as referenced by the 2010 Florida Building Code.
- 2) Cable requires continuous bottom chord bearing.
- 3) Cable webs placed at 24" o.c. U.N.O.
- 4) Attach gable webs with 1x4 20ga plates, U.N.O.
- 5) This truss has been designed for in-plane wind loading. For bracing requirements for out-of-plane wind loading, refer to the Construction Documents and/or BCSI-B3.
- 6) When this truss has been chosen for quality assurance inspection, the Plate Placement Method per TPI 1-2002/A3.2 shall be used. C₁ = 1.17
- 7) Listed wind uplift reactions based on MWERS Only loading.
- 8) Bearing material shown in the above table has only been checked for resistance perpendicular to grain, and does not indicate adequacy of material for other design considerations.



Dansco Engineering, LLC
P.O. BOX 3400
Apollo Beach, FL 33572
CA 25948
Date: 04/14/2014
DE Job# 33281-W1



24 0

5 0

5 0

24 0

AMERICAN TRUSS

NOTES:

Not Scaled

SIMPSON
Strong-Tie

JOB: KEN ALLEN GARAGE

DESIGNED BY:
JASON

SHIPPING INFORMATION:

ITEM	QTY	DESCRIPTION
100	100	100
100	100	100
100	100	100
100	100	100

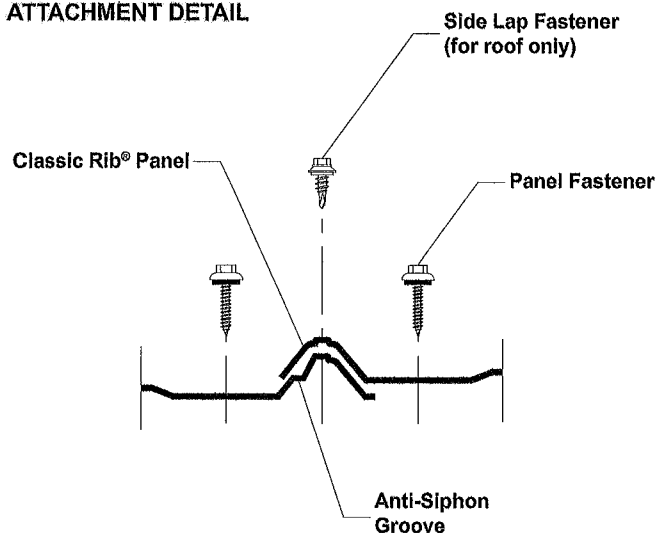
SHEET NUMBER: 1

LAYOUT

CLASSIC RIB®

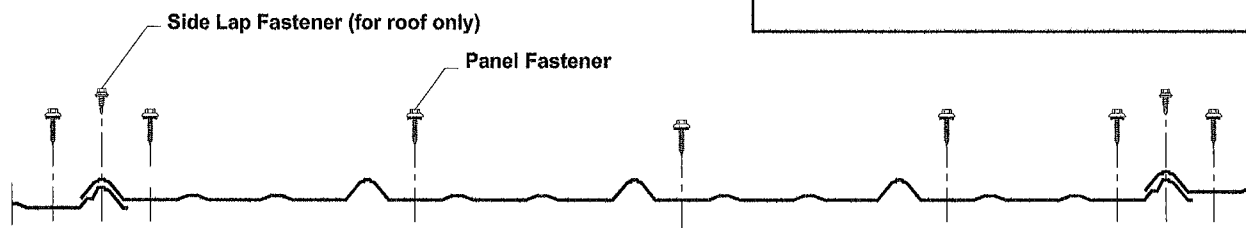
CONDENSED TECHNICAL REFERENCE

ATTACHMENT DETAIL

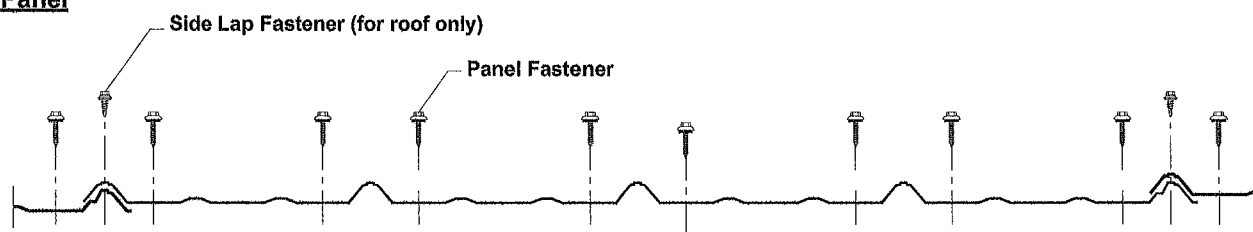


FASTENING PATTERN

Field of Panel



Ends of Panel



FASTENER INFORMATION

Overdriven fasteners will cause panel distortions

Fasteners should extend 1/2" or more past the inside face of the support material.

Thick panels (ex. 18 ga) or supports (ex. 1/2" steel) may require predrilling of holes for screws.

Panel Fasteners

Attaching to Wood:

#10-14 Wood Screw

#10-14 XL Wood Screw

Attaching to Steel:

#12-14 Self Drilling Screw

#12-14 XL Self Drilling Screw

Side Lap Fastener

1/4"-14 x 7/8" Stitch Screw

1/4"-14 x 7/8" XL Stitch Screw

Trim Fastener:

1/4"-14 x 7/8" Stitch Screw

1/4"-14 x 7/8" XL Stitch Screw

SECTION PROPERTIES

Ga	Width in	Yield ksi	Weight psf	Top in Compression		Bottom in Compression	
				Ixx in ⁴ /ft	Sxx in ³ /ft	Ixx in ⁴ /ft	Sxx in ³ /ft
29	36	80	0.62	0.0100	0.0151	0.0053	0.0118
26	36	80	0.79	0.0127	0.0192	0.0070	0.0153
24	36	50	1.03	0.0163	0.0249	0.0103	0.0208

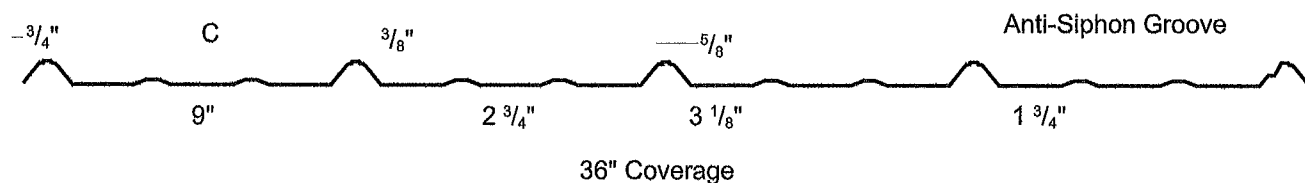
ALLOWABLE UNIFORM LIVE LOADS, psf For various fastener spacings

								Inward Load						Outward Load					
								1.5'	2'	2.5'	3'	3.5'	4'	1.5'	2'	2.5'	3'	3.5'	4'
29	36	80	0.62	0.0100	0.0151	0.0053	0.0118	142	81	52	36	27	21	179	103	66	46	34	26
26	36	80	0.79	0.0127	0.0192	0.0070	0.0153	185	105	68	47	35	27	227	131	84	59	44	33
24	36	50	1.03	0.0163	0.0249	0.0103	0.0208	208	119	77	53	39	30	245	141	91	64	47	36

- Theoretical section properties have been calculated per AISI 2007 'North American Specification for the Design of Cold-Formed Steel Structural Members'. Ixx and Sxx are effective section properties for deflection and bending.
- Allowable load is calculated in accordance with AISI 2007 specifications considering bending, shear, combined bending and shear and deflection. Allowable load considers the 3 or more equal spans condition. Allowable load does not address web crippling, fasteners, support material or load testing. Panel weight is not considered.
- Deflection consideration is limited by a maximum deflection ratio of L/180 of span.
- Allowable loads do not include a 1/3 stress increase for wind.
- Diaphragm Capacity** - 296 plf average Ultimate Shear Strength using the above fastening pattern on 2x supports located 2' on center, per ASTM E 445

CLASSIC RIB®

CONDENSED TECHNICAL REFERENCE



**COMMERCIAL
RESIDENTIAL
PANEL**

**EXPOSED
FASTENED**

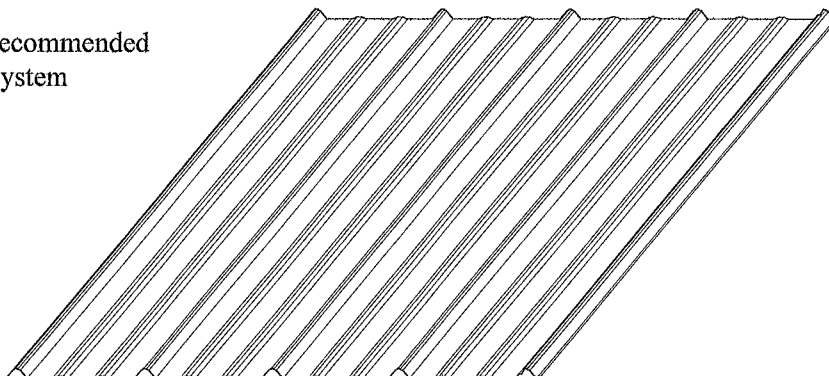
**36"
COVERAGE**

**MINIMUM
SLOPE
3:12**

**OPEN FRAMING OR
SOLID SUBSTRATE**

PANEL OVERVIEW

- ▶ Finishes: MS Colorfast45® and Acrylic Coated Galvalume®
- ▶ Corrosion Protection: AZ55 per ASTM A 792 for unpainted Galvalume®
AZ50 per ASTM A 792 for painted Galvalume®
G60, G90 or G100 per ASTM A 653 for Galvanized
- ▶ Gauges: 29 ga.
- ▶ 36" panel coverage, $\frac{3}{4}"$ rib height
- ▶ Panel Length: Minimum: 5'; Maximum: 45' recommended
- ▶ Exposed fastened, low profile roof and wall system
- ▶ Bell-top trapezoidal rib on 9" centers
- ▶ Minimum roof slope: 3:12



TESTING AND APPROVALS

- ▶ UL 2218 Impact Resistance - Class 4
- ▶ UL 790 Fire Resistance Rating - Class A, per building code
- ▶ UL 263 Fire Resistance Rating - per assembly
- ▶ UL 580 Uplift Resistance - Class 90 Constructions: #560, 584
- ▶ ASTM E 455, Diaphragm Capacity
- ▶ Texas Wind Storm - Evaluation RC-161
- ▶ 2010 FBC Approvals - FL9482.2, FL9482.3, 10999.3, FL 10999.4,
FL14645.7, FL14645.8, FL14645.9 and FL15478.2
- ▶ Miami-Dade County, Florida NOA 11-0617.02
- ▶ Miami-Dade County, Florida NOA 10-0427.04 - Roof