

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: 1TG68228Z0127100753

Truss Fabricator: Anderson Truss Company
Job Identification: 8-092--BRYAN ZECHER Lot #22 The Oaks -- , **
Truss Count: 45
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Versions 7.36, 7.37.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: TCFILLER-BCFILLER-BRCLBSUB-CNBRGBLK-A11015EE-GBLLETIN-VALTRUSS-PIGBACKB-A11030EE-

Seal Date: 03/27/2008

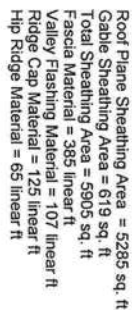
-Truss Design Engineer-
Doug Fleming

Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	31258--A16		08087048	03/27/08
2	31259--A15		08087049	03/27/08
3	31260--A14		08087050	03/27/08
4	31261--A13		08087051	03/27/08
5	31262--A12		08087052	03/27/08
6	31263--A11		08087053	03/27/08
7	31264--A10		08087054	03/27/08
8	31265--A9		08087055	03/27/08
9	31266--A8		08087056	03/27/08
10	31267--AA-GE		08087072	03/27/08
11	31268--A7		08087057	03/27/08
12	31269--A6		08087058	03/27/08
13	31270--A5		08087059	03/27/08
14	31271--A4		08087073	03/27/08
15	31272--A3		08087075	03/27/08
16	31273--A2		08087077	03/27/08
17	31274--A1		08087038	03/27/08
18	31275--A-GE		08087078	03/27/08
19	31276--V5		08087039	03/27/08
20	31277--V4		08087040	03/27/08
21	31278--V3		08087041	03/27/08
22	31279--V2		08087042	03/27/08
23	31280--V1		08087060	03/27/08
24	31281--B1		08087043	03/27/08
25	31282--B-GE		08087080	03/27/08
26	31283--C1		08087061	03/27/08
27	31284--C2		08087044	03/27/08
28	31285--C-GE		08087081	03/27/08
29	31286--HD6		08087062	03/27/08
30	31287--E-GE		08087063	03/27/08
31	31288--E1		08087045	03/27/08
32	31289--J1		08087064	03/27/08
33	31290--HJ6		08087065	03/27/08
34	31291--J3		08087046	03/27/08
35	31292--J5		08087047	03/27/08
36	31293--EJ6		08087066	03/27/08

#	Ref	Description	Drawing#	Date
37	31294--PB9		08087083	03/27/08
38	31295--PB8		08087067	03/27/08
39	31296--PB7		08087068	03/27/08
40	31297--PB6		08087087	03/27/08
41	31298--PB4		08087088	03/27/08
42	31299--PB5		08087069	03/27/08
43	31300--PB3		08087070	03/27/08
44	31301--PB2		08087089	03/27/08
45	31302--PB1		08087071	03/27/08





#8-092
BRYAN ZECHER-
LOT #22
THE OAKS

JOB NO:
8-092

PAGE NO.:

1 OF 1

(B) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

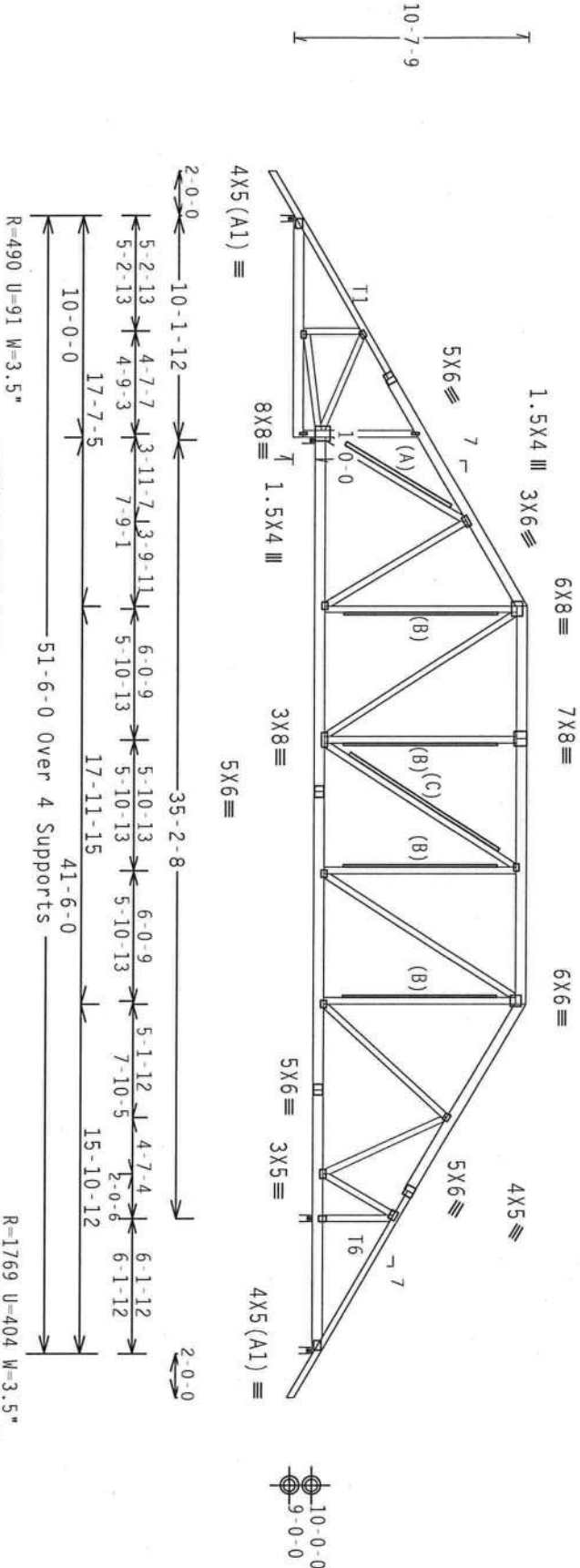
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/0(0)$

7.36.04

QTY:1

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

ITW Building Components Group Inc

Haines City, FL 33844
FL Certificate of Authorization #0078

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN CODE. SEE SERIES AND THE

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H,SS) GALV. STEEL. APPLY

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3, A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING IS THE RESPONSIBILITY OF THE DESIGNER.

SECTION THREE - THE CONTRACTOR AND USE OF THIS SOFTWARE FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31258
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCSR8228 08087048
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON -	82138
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

Webbs	2x4	SP	#3
E1110	2x4	SP	#3

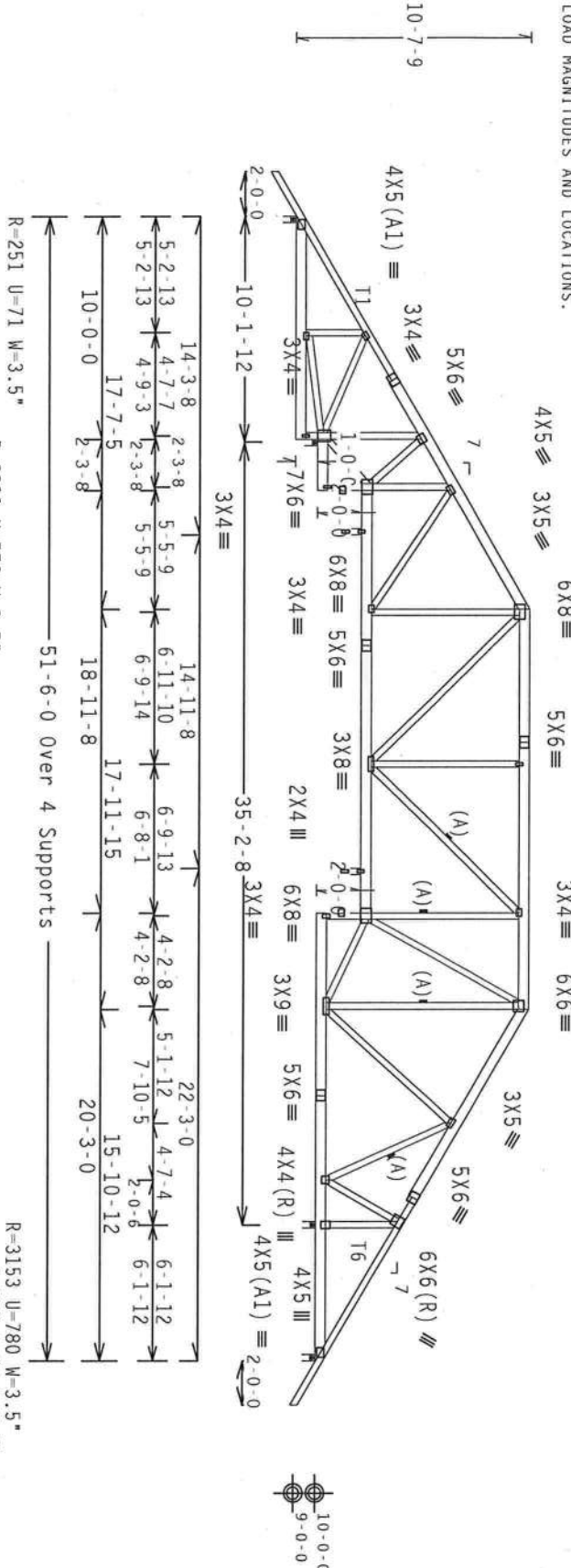
FILED 484 OF #C DENISE

SPECIAL LOADS

TC	From	DUR.FAC.=1.25	PLATE	DUR.FAC.=1.25
TC	From	63 PLF at -2.00 to	63 PLF at 7.25 to	
TC	From	63 PLF at 7.24 to	63 PLF at 17.6 to	
TC	From	63 PLF at 17.61 to	63 PLF at 23.6 to	
TC	From	63 PLF at 23.60 to	63 PLF at 35.6 to	
TC	From	63 PLF at 35.60 to	63 PLF at 39.1 to	
TC	From	192 PLF at 39.17 to	234 PLF at 44.2 to	
TC	From	294 PLF at 44.24 to	261 PLF at 47.5 to	
TC	From	63 PLF at 47.50 to	63 PLF at 53.5 to	
TC	From	5 PLF at -0.00 to	5 PLF at 0.00 to	
BC	From	20 PLF at 0.00 to	20 PLF at 10.00 to	
BC	From	20 PLF at 10.00 to	20 PLF at 12.2 to	
BC	From	20 PLF at 12.29 to	20 PLF at 19.2 to	
BC	From	20 PLF at 19.25 to	20 PLF at 31.2 to	
BC	From	20 PLF at 31.25 to	20 PLF at 39.5 to	
BC	From	20 PLF at 39.50 to	20 PLF at 53.5 to	
BC	From	5 PLF at 51.50 to	5 PLF at 53.5 to	
TC	138 lb Conc.	Load at 47.50		

Wind reactions based on MWKs pressures:

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS"). TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



Note: All Plates Are 1.5X4 Except As Shown

PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

7.36.042

QTY:1

Scale = .125"/Ft.

WARNING: THESE TRILITE EXTREME CASES IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND REACTING REFER TO DC31 (BUILDING COMPONENT SAFETY IN FABRICATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND NICK GOOD TRUSS COMPANY OF AMERICA, 6500 BOWEN ENTERPRISE LANE, MOBILE, AL 36619 FOR SAFETY PRACTICES PRIOR TO PREPARING THESE CONNECTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

CONNECTOR PLATES ARE MADE OF 2011B/16GA (H HESSKY) ASTM A663 GRADE 40/60 CH WITH 663 GALV DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALKPA) AND TPI

PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 1604-
CONNECTION PLATES AND RIBS OF 20/40/100MM (W, h/h, ss/k) MAIN ROSS GRADE 40/50 (W, h/h, ss) GALV. STEEL, APPLT

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THE

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

100

100

110 mph wind; 15.00 ft mean hgt, ASCE 7-02, PART_ENC, bldg, not located within 5.50 ft from roof edge, CAT II, EXP B, wind IC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCPI(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) continuous lateral bracing equally spaced on member.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increases factor for dead load is 1.50.

WARNING: furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



TC LL	20.0 PSF	REF	R8228 - 31259
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087049
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82144
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF -	1TG68228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.55

Wind reactions based on MMFRS pressures.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

Scale = .125"/Ft.

6.042
DOUGLAS FLEMING
LICENSE
No. 66648
QTY

ITW Building Components Group Inc.

PLATES TO EACH FEET OF THUS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2002, SEC. 3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SILENT FOR THE THUS COMPONENT DESIGN SILENT. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

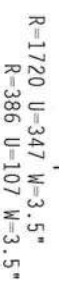
TC LL	20.0 PSF	REF	R8228- 31260
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCSUR8228 08087050
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82128
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART_ENC, bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Tw=1.00 GC01(+/-)=0.55

Wind reactions based on MIFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24"

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

 $Cd/RT=1.00(1.25)/0(0)$

Scale = .125"/Ft.

6.042 QTY

DOUGLAS FLEMING
LICENSE
No. 66648

☆

STATE OF



21

TC LL	20.0 PSF	REF	R8228- 31261
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087051
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82165
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.55

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

Scale = .125"/Ft.

TC LL	20.0 PSF	REF	R8228- 31262
TC DL	10.0 PSF	DATE	03/27/08



at 27 '08

DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

Bearing blocks:	Nail type:	12d Common	(0.148"x3.25", min.)	nails
BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK
3	45.208'	1	12"	4
				Rigid Surface

Refer to drawing CNBRG8LK0207 for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART 1, ENC. bldg. not located within 6:50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI(+/-)=0.55

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Design Crit: $TPI-2002(STD)/FBC$
$$Cq/RT=1.00(1.25)/0(0)$$

7.36.04

QTY:1

FL/-/4/-/-/R/-/

Scale = .125"/Ft.

WARNING: THESE REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI, TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND VICA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MIDDLETON, WI 53119) FOR SAFETY PRACTICES TO PREVENTING THESE FAILURES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANTS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM DCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IT1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES,

DESIGN CONDITIONS APPLICABLE PERMANENTLY FOR JOBS (NATIONAL DESIGN SPECIFICATIONS BY AREA) AND THE BUILDING CODES OF THE JURISDICTION SHALL BE USED TO DETERMINE THE MINIMUM REQUIREMENTS FOR THE CONCRETE CONNECTOR PLATES MADE OF 2010/166A (W-HSS/X), ASTM A663 GRADE 40/60 (W-K/H-SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITIONED PER BRANIFFOS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICA OR AS OF THE YEAR 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SLOTTED FOR THE TRUSS COMPONENT DESIGN SHOWS. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/PTI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31263
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087053
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82204
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1T668228Z01

Bearing	DLOCKS:	NAIL	Type:	120-Common	(U.148-X3.25	-min.)	-nails
BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK	WALL PLATE		
45.208'		1	12"	4	Rigid Surface		

Refer to drawing CNBRG8LK0207 for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART_ENC, bldg, not located within 6.50 ft from roof edge, CAT II, Exp B, wind IC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpf(+/-)=0.55

Wind reactions based on MWRS pressures.

Root overhang supports 2.00 pst soffit load.

(A) continuous lateral bracing equally spaced on member.

in the structural panels use partitions to brace all truss @ 24
OC.

Deflection meets L/240 live and L/180 total load. Creep increases

[illegible]

Special care must be taken during handling, shipping and



BRACING.
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A, 6300
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STATE OF

AL ON THIS
COMPONENT

Year	Percentage of Population 15 and Over with High School Diploma or Higher
1980	65
1990	75
2000	85

TC LL	20.0 PSF	REF	R8228- 31264
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087054
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82219
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TG68228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24 OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

FL/-/4/-/-/R/-/

Scale = .125" / Ft.

6.0421113
QTY

ITW Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778



BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

HC-ENG DF/DF
SEQN - 82227
FROM AH
REF- 1TG68228Z0J

Top chord 2x6 SP #2 :11, T6 2x4 SP #2 Dense:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

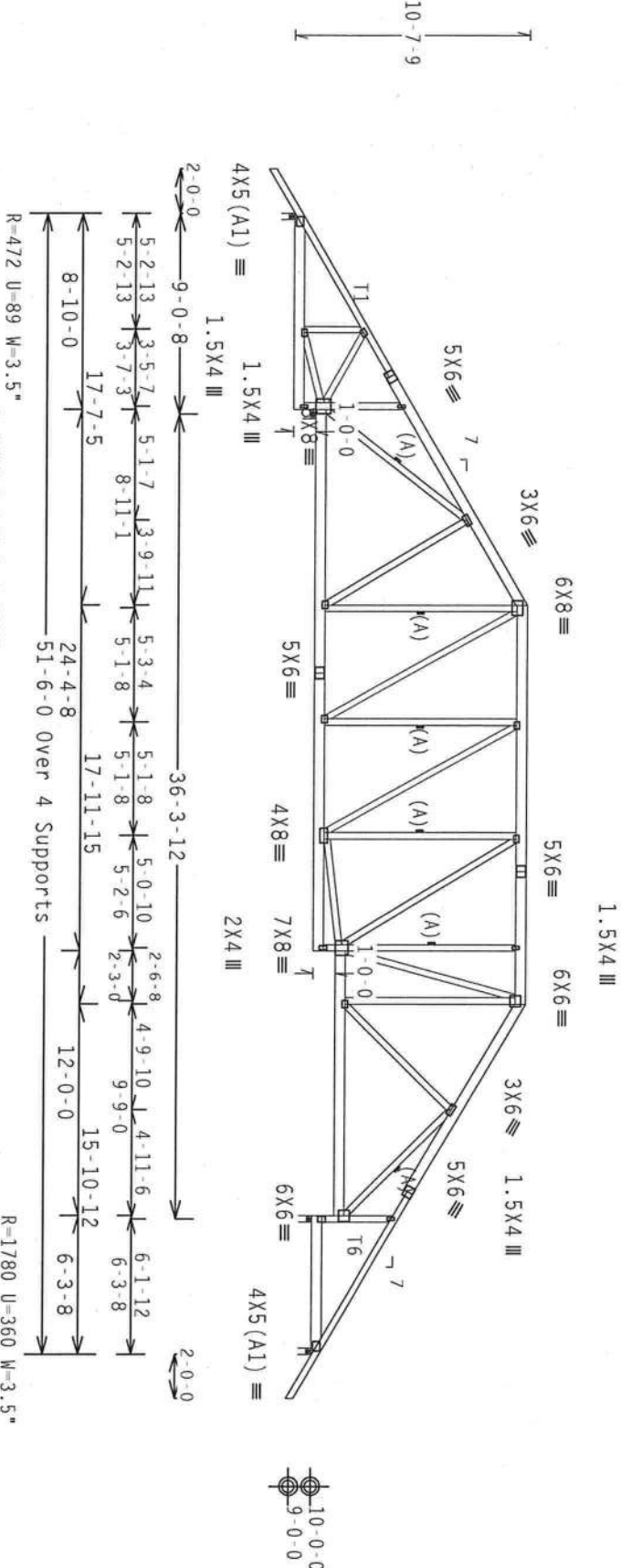
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 gcpl(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.042

OTV:1

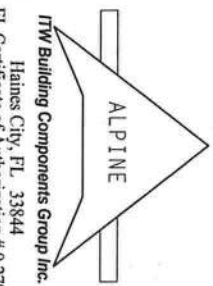
FL/-/4/-/R/-

Scale = .125"/ft.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AISC TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MANASSAS, VA 20108) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

DRAINAGE INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TPI Building Components Group Inc.

Haines City, FL 33844

TPI Certificate of Authorization #0778

TC LL	20.0 PSF	REF	R8228- 31266
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCSR8228 08087056
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	82235
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	UREF-	1TG68228201

Top chord 2x6 SP #2 :T1, T8 2x4 SP #2 Dense:
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W7, W11, W44 2x4 SP #2 Dense:

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 psf. Top chord must not be cut or notched.

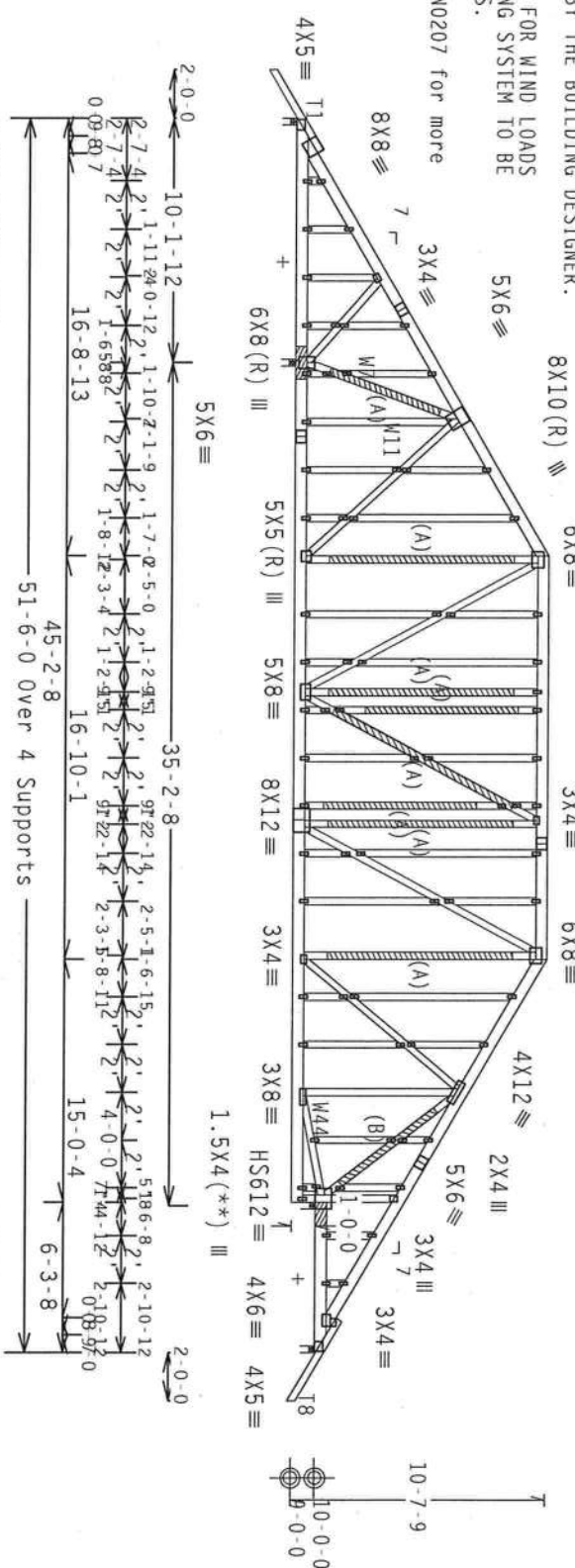
(B) (2) #3 or better scab braces. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3", min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS, DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

See DWG5 A11015EE0207 & GBLLETT0207 for more requirements.



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART 1, ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.55

Wind reactions based on MFRS pressures.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. 20 Gauge HS, Wave

Design Crtt: TPI-2002(STD)/FBC

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization #A-0770

WARNING

TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION. NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304 AND WICA (WOOD TRUSS) COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT

FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF THE (OPTIONAL) DESIGN SPEC. BY AREA) AND TPI. ITW BCG PLATES TO EACH CHORD OF TRUSS. (A) (H/S/S) ASH 4653 GRADE 40/60 (R, R/H/S) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE OWNER AS THE DESIGN, POSITIONING PER DRAWINGS, TOLERANCE, DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING

FLORIDA

PROFESSIONAL ENGINEER

No. 66648

27 '08

TC LL	20.0 PSF	REF	R8228- 31267
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCSR8228 08087072
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	82281
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

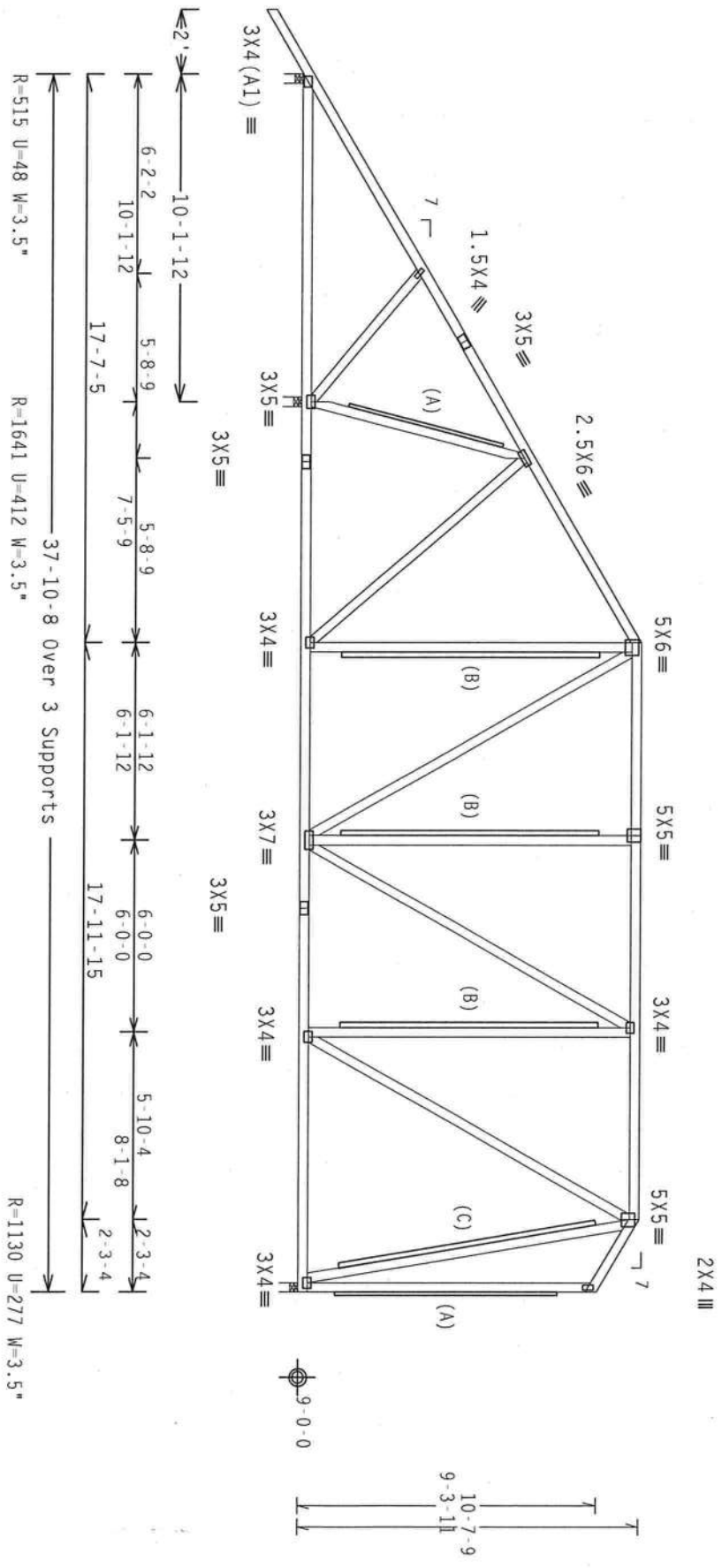
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART 5, bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCp1(+/-)=0.55

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0.00

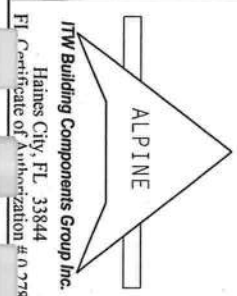
OTY:1 FL/-/4/-/R/-

Scale = .1875"/Ft.

WARNING: TRUSSES REQUIRING EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING OF THE TRUSS. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE.

THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER ATTACHMENT OF THE TRUSS TO THE BUILDING STRUCTURE.



TC LL	20.0 PSF	REF	R8228- 31268
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087057
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	82291
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

See DWGS TCFILLER0207 and BCFILLER0207 for filler details.

(D) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

(B) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Laterally brace BC above filler @ 24" O.C. including a lateral brace at chord ends.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC, bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw=1.00 GCpl(+/-)=0.55

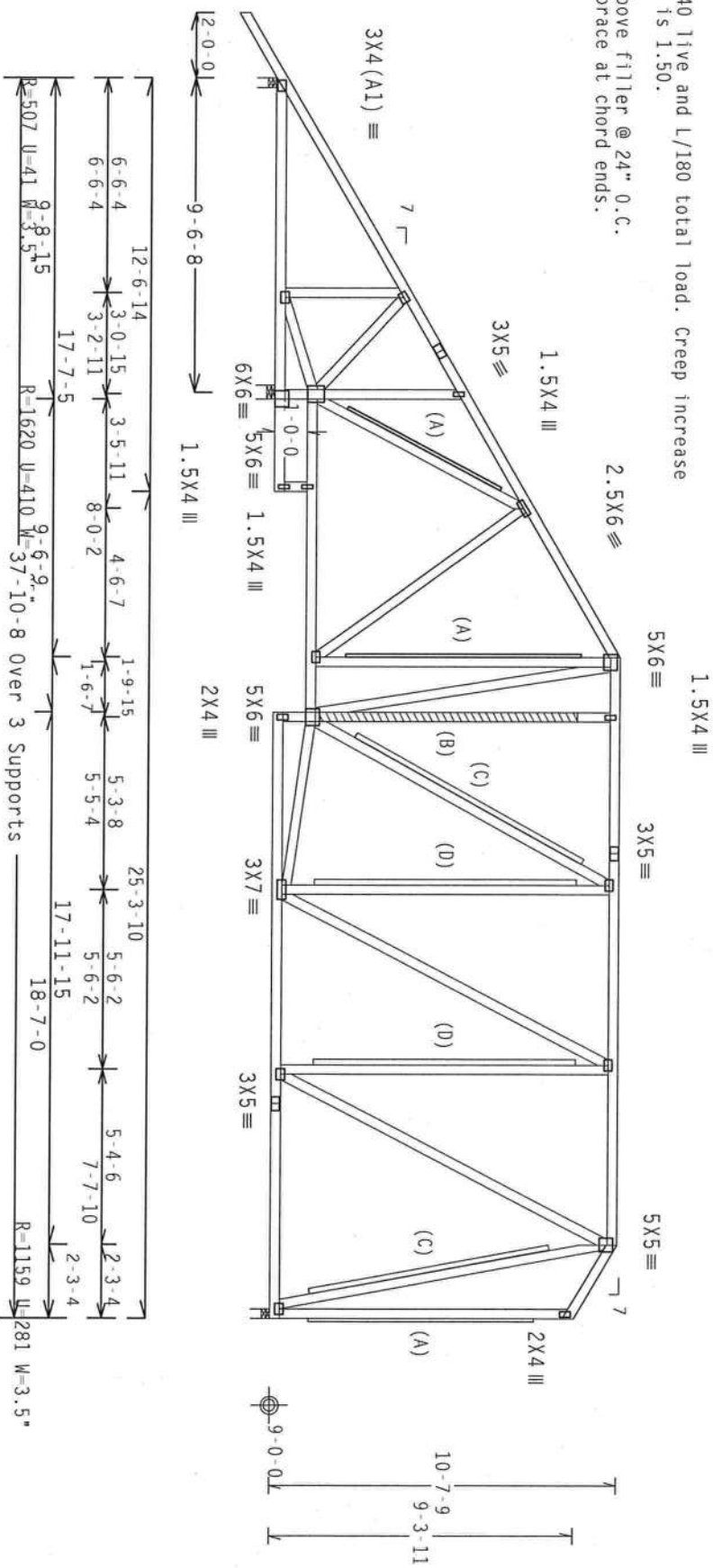
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.



Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq(RT)=1.00(1.25)/0(0)

7.36.042

QTY: 1

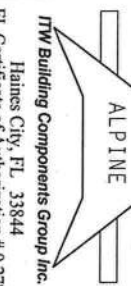
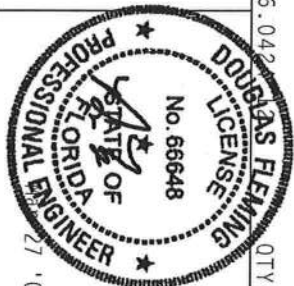
FL/-/4/-/R/-

Scale = .1875"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INSTITUTE) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING. REFER TO BCSTI (BUILDING COMPONENT SAFETY INSTITUTE) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED BY A QUALIFIED PERSONNEL PER DRAWINGS 1604-2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SILENT FOR THE TRUSS COMPONENTS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0-070

TC LL	20.0 PSF	REF	R8228 - 31269
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCSR8228 08087058
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT.LD.	40.0 PSF	SEON-	82306
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1TG68228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Filler 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

See DMGS TCFILLER0207 and BCFILLER0207 for filler details.

(A) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

(B) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART-ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{cpl}(+/-)=0.55$

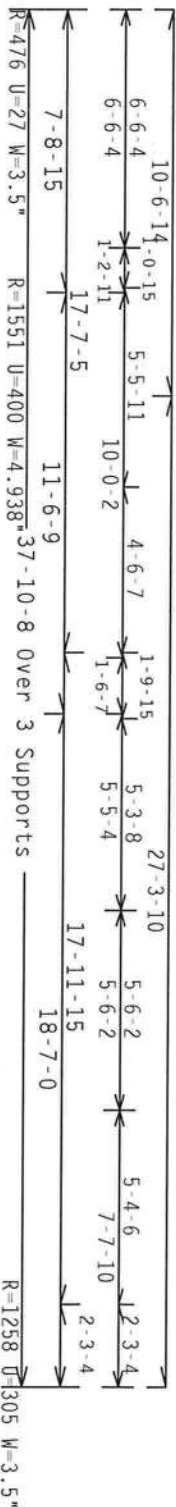
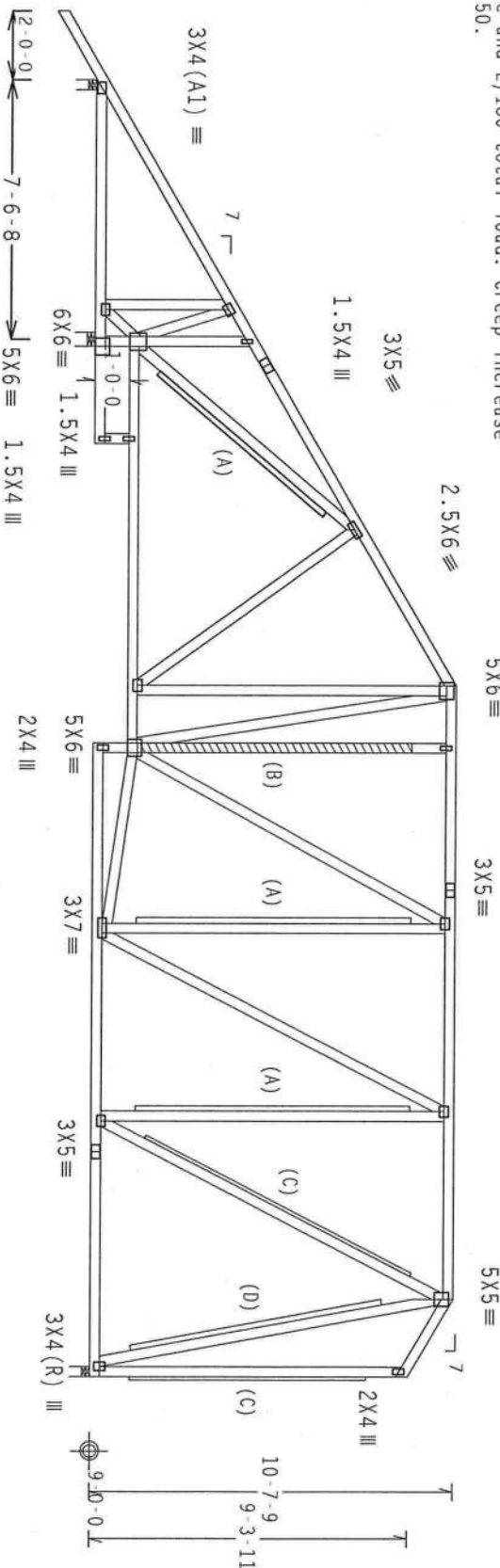
Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

(C) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(D) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

Laterally brace BC at 24" OC in lieu of rigid ceiling. Laterally brace BC above filler at 24" OC.



Note: All Plates Are 3x4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.042

OTY:1 FL/-/4/-/R/-

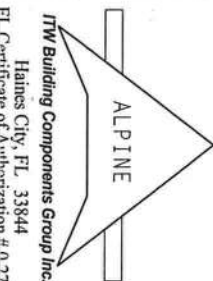
Scale = .1875" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



TC LL	20.0 PSF	REF R8228- 31270
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087059
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 82318
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1T668228201



ITW Building Components Group Inc.
Haines City, FL 33844
PI Certificate of Authorization #0-730

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.55

Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

(B)(1) 2x8x10-1-8 SP SS scab at left end. Attach scab to face of chord with: 12d Common (0.148"x3.25", min.) nails @ 8" OC, plus additional nail clusters at: BRG.: (5), neel: (7), 1st panel point: (2).



Scale = .1875"/Ft.

DOUGLAS
LICENSE
No. 66648



IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE ECG, INC. SMALL NUMBER BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOD NATIONAL DESIGN SPEC. BY AIA/AIA AND TPI.

THE RECORD SHALL BE FOR 20/10/0000 (4) SHALL BE PERMANENT 40/600 (4) SHALL BE GALV. STEEL. APPLY PROTECTIVE COATING TO EACH MEMBER OF TRUSS.

INSPECTION OF PLATE FOLLOWED BY (1) SHALL BE PERMANENT 40/600 (4) SHALL BE GALV. STEEL. APPLY PROTECTIVE COATING TO EACH MEMBER OF TRUSS.

DESIGN SHOWS THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31271
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087073
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN -	82350
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

Top chord 2x4 SP #2 Dense :T1, T2 2x8 SP SS:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

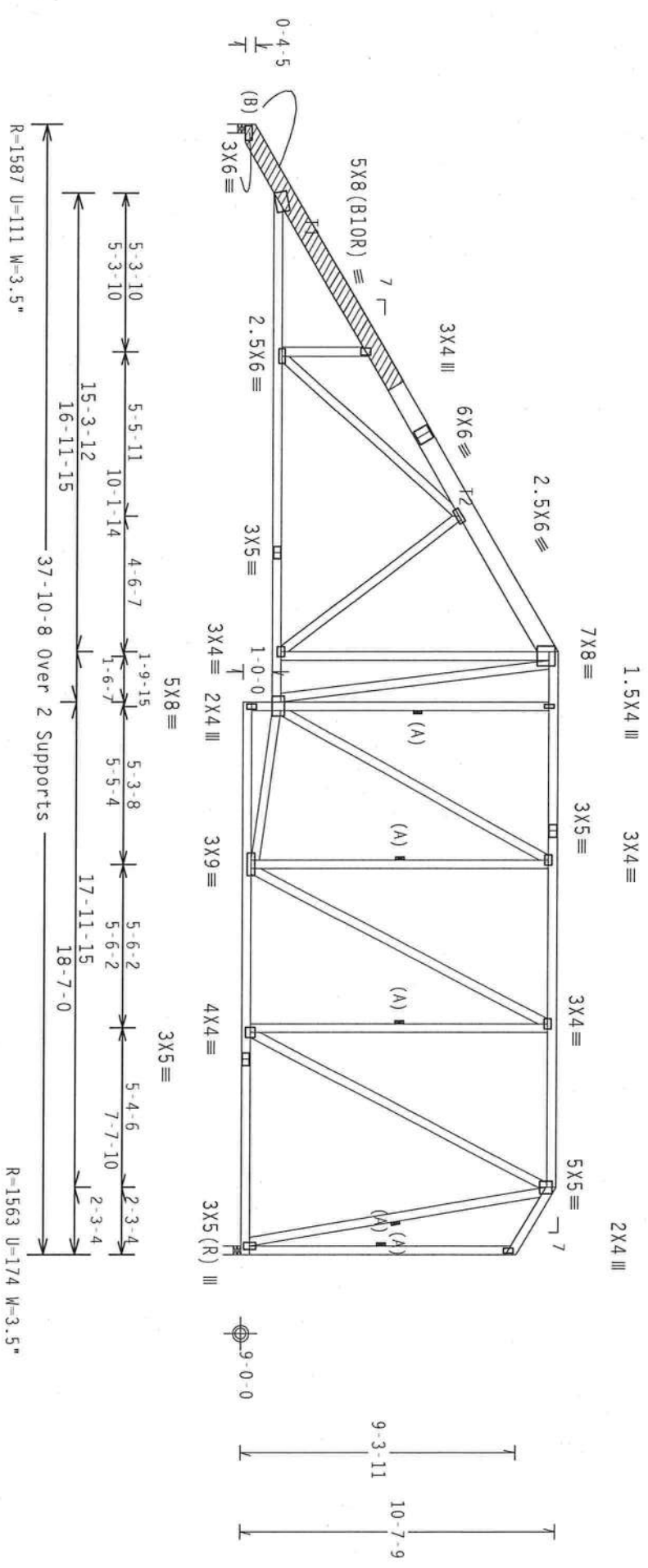
Calculated horizontal deflection is 0.12" due to live load and 0.19" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, Wind BC DL-5.0 psf. $I_w=1.00$ $G_{cpl}(+/-)=0.18$
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

(B) (1) 2x8x10-1-8 SP SS scab at left end. Attach scab to face of chord with: 12d Common (0.148"x3.25", min.) nails @ 8" OC, plus additional nail clusters at: BRG.: (5), heel: (7), 1st panel point: (2).



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.04

QTY: 1 FL/-/4/-/R/-

Scale = .1875"/ft.

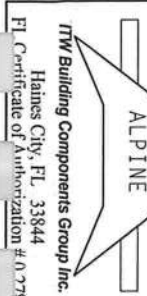
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCST (BUILDING COMPONENT SAFETY INFORMATION) CONSULTED BY TPI, INC. 10000 ROUTE 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND VICA (WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/NA) AND TPI. THE BCG CONNECTION PLATES ARE MADE OF 20/70/100A (40/55/80) ASH 6063 GRADE 40/60 (40/55) GALV. STEEL. APPLY TO THE TRUSS CHORDS AND WEBS. THE TRUSS CHORDS AND WEBS SHALL BE PROTECTED BY A MINIMUM OF 1/2" OF INSULATION. ANY INSPECTION OF PLATES FOLLOWED BY ACCESS THROUGH THE TRUSS SHALL BE THE RESPONSIBILITY OF THE DESIGNER. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 31272
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087075
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 82359
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TG68228201



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-) -0.18

Wind reactions based on MWFRS pressures.
Right end vertical not exposed to wind pressure.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

(C) (1) 2x6x10-1-8 SP SS scab at left end. Attn: scab to face of chord with: 12d Common (0.148"x3.25".min.) nails @ 8" OC, plus additional nail clusters at: BRG.: (5), heel: (7), 1st panel point: (2).



Scale = .1875" / Ft.

DOCKING
LICENSE
No. 66648



TC LL	20.0 PSF	REF	R8228 - 31273
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 0808/077
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN -	82369
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

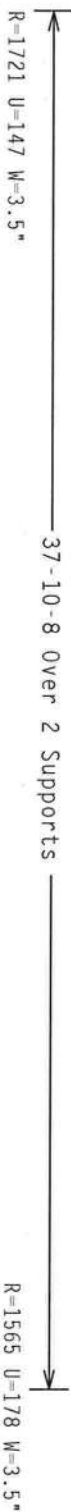
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Scale = .1875" / Ft.

6.042
DOUGLAS FLEMING
LICENSE
No. 66648
QTY

TC LL	20.0 PSF	REF R8228- 31274
TC DL	10.0 PSF	DATE 03/27/08



ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization # 0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

(A) #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

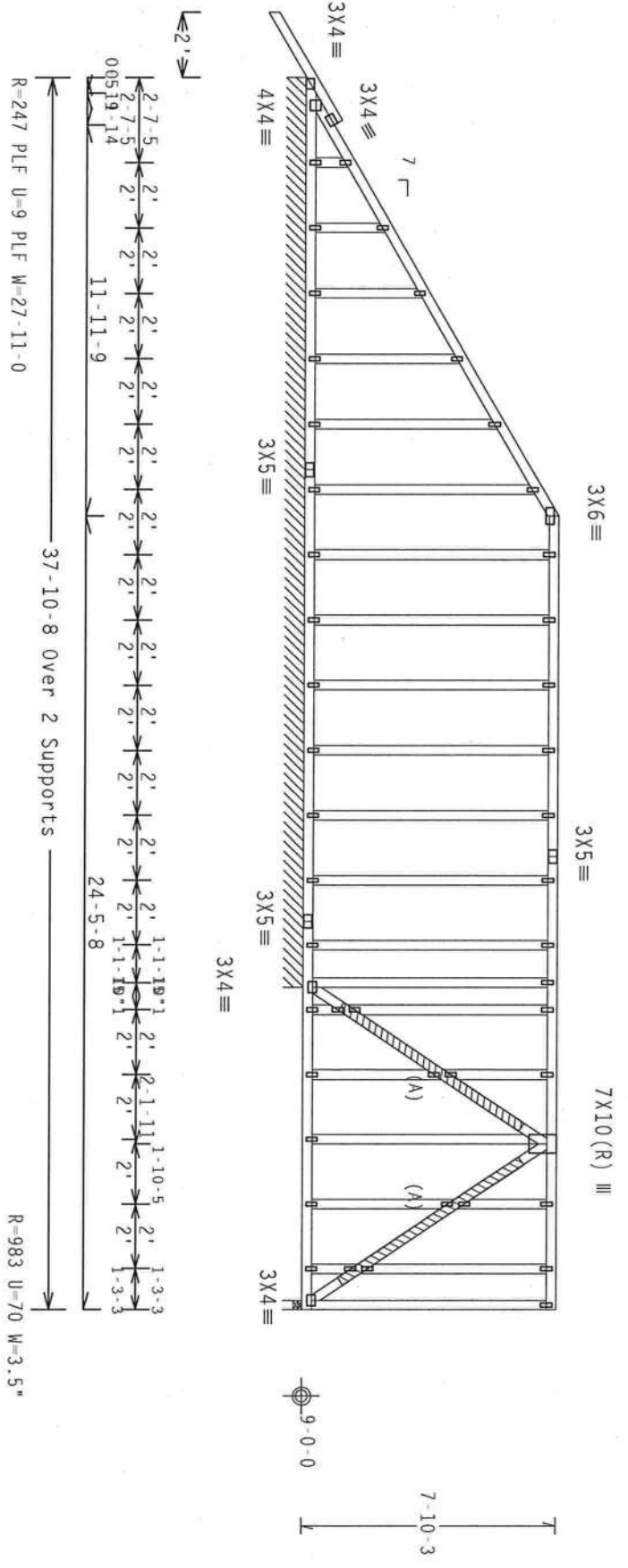
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, closed bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0
psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04

OTV:1 FL/-/4/-/R/- Scale = .1875"/ft.

TC LL 20.0 PSF REF R8228- 31275

TC DL 10.0 PSF DATE 03/27/08

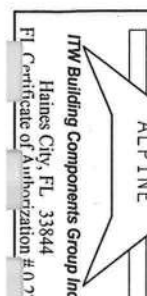
BC DL 10.0 PSF DRW HCUSR8228 08087078

BC LL 0.0 PSF HC-ENG DF/DF

TOT.LD. 40.0 PSF SEON- 82399

DUR.FAC. 1.25 FROM AH

SPACING SEE ABOVE JREF- 1TG68228201



ITW Building Components Group Inc.
Haines City, FL 33844
PL Certificate of Authorization #0778

DOUGLAS FLEMING
LICENSE
No. 66648
FLORIDA
PROFESSIONAL ENGINEER

110 mph wind, 16.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MIFRS pressures.

See DWG VALTRUSS0207 for valley details.

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1

Scale = .5" / Ft.

QTY .0424

DOUGLAS FLEMING
LICENSE

No 66648



ALPINE

ITW Building Components Group Inc

FI Certificate of Authorization # 0 778

[illegible]

042

QTY

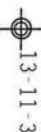
DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF	R8228 - 31276
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087039
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEON-	82009
DUR.FAC.	1.25	FROM AH	
SPACING	24.0"	JREF-	1T568228Z01

REF	R8228 - 31276
DATE	03/27/08
DRW	HCU8R8228 0808/039
HC - ENG	DF/DF *
SEON -	82009
FROM	AH
REF	1TG68228201

110 mph wind, 15.42 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.
See DWG VALTRUSS0207 for valley details.


$$Cq/RT=1.00(1.25)/0(0)$$

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

6.04
DOUGLAS FLEMING
LICENSE
No. 66648
OT

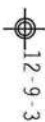


ALPINE

TC LL	20.0 PSF	REF	R8228- 31277
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCU8R8228 08087040
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	82013
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T668228Z01

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi (+/-)=0.18

See DWG VALTRUSS0207 for valley details.



3-10-2

Scale = .5"/Ft.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278



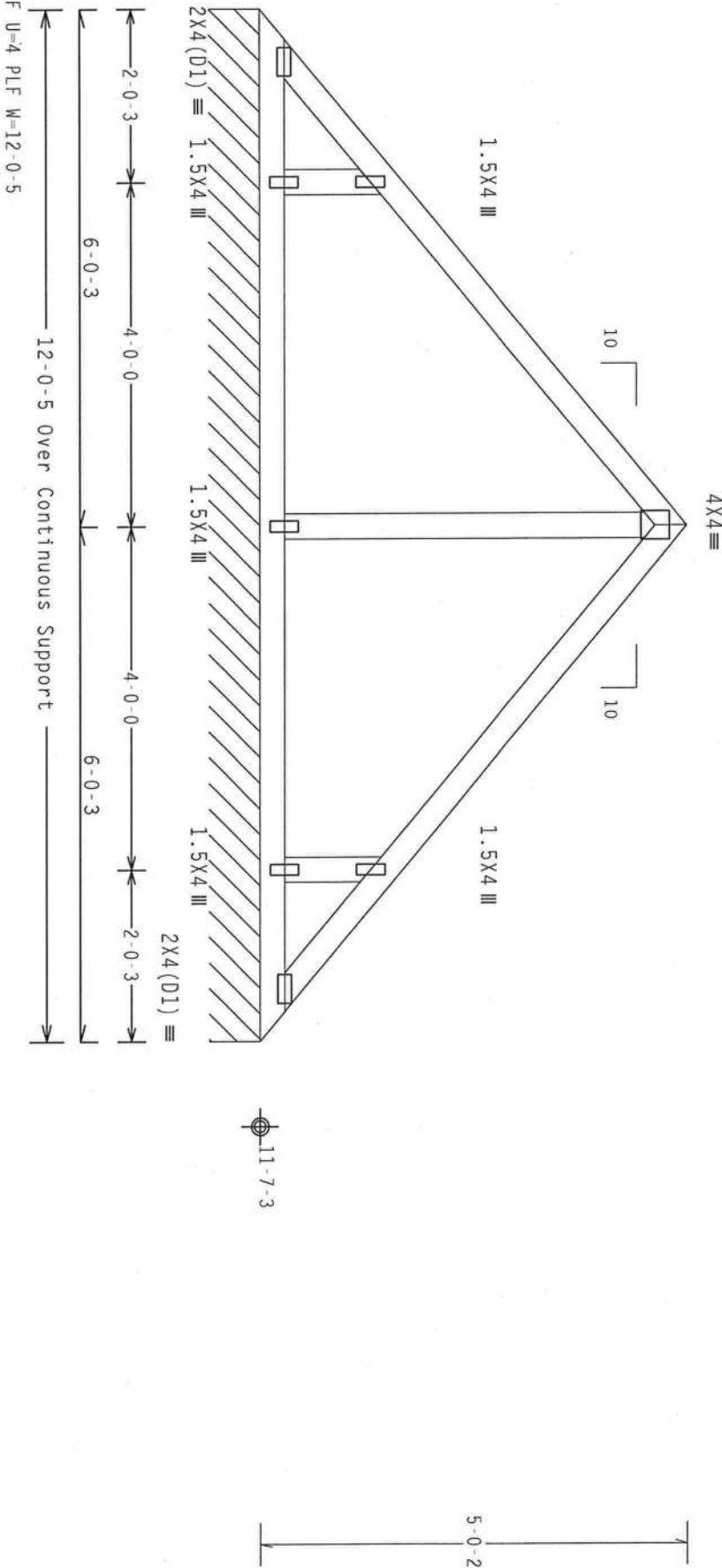
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TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087041
BC LL	0.0 PSF	HC-ENG	DF/DF *
TOT.LD.	40.0 PSF	SEQN-	82017
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webbs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.
See DWG VALTRUSS0207 for valley details.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY:1 FL/-/4/-/-/R/-

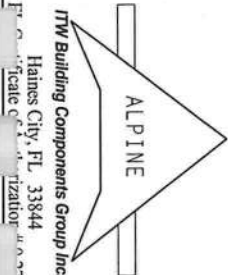
Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/A) AND TPI. CONNECTIONS ARE MADE OF 20/18/16GA (H/H/SS) ASH/ALUMINUM 40/40 (H/H/SS) GALV. STEEL. APPLY 2" PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED, CONNECTIONS PER DRAWINGS OR THIS DESIGN. THE DESIGNER'S ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE DESIGNER'S ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. BUILDING DESIGNED PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 31279
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUSR8228 08087042
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEON- 82021
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TG68228Z01



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Weds 2x4 SP #3

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

See DWG VALTRUSS0207 for valley details.

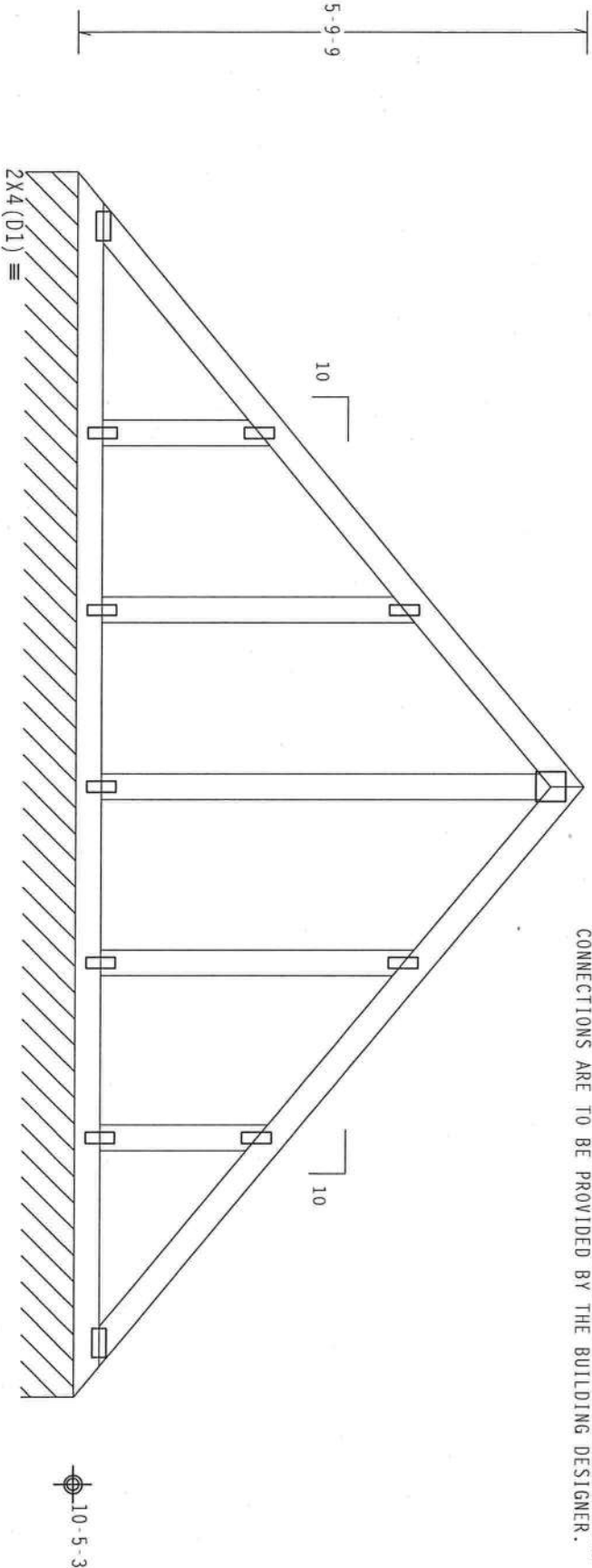
See DWG A11015EE0207 & GBLLETINO207 for more requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf, $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



R=166 PLF U=5 PLF W=13-11-0

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TP1-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.042

QTY:1

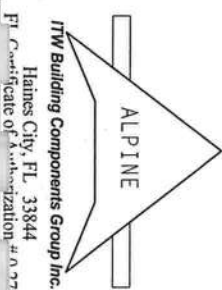
FL/-/4/-/-/R/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN, OR FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TP11-2002 SEC.2. THE BUILDING DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 31280
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087060
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	82028
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	UREF-	1TG68228201

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.36.042

QTY:1

FL/-/141/-/1-/R/-/

Scale = .1875" / Ft.

DOUBLE
LICENSE
No. 66648

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI'S OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BCGD

FI Certificate of Authorization # 00778

ALPINE

FI Certificate of Authorization # 00778

27 '08

DUR. FAC.

1.25

FROM AH

JREF- 1TG68228Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Technical drawing of a roof truss system. The drawing shows a cross-section of a roof with a gabled structure. Key components and labels include:

- Roof Slopes:** Indicated by a triangle with a vertical side of 7 and a horizontal side of 1.
- Members:**
 - 3X4**: Labeled on the top and bottom chords.
 - 3X5**: Labeled on the top and bottom chords.
 - 3X6**: Labeled on the top and bottom chords.
 - 4X4**: Labeled on the top and bottom chords.
 - 1.5X4 (**)**: Labeled on the bottom chord.
- Dimensions:**
 - 8-11-9**: A vertical dimension line on the right side.
 - 9-0-0**: A horizontal dimension line at the bottom.
- Labels:**
 - (A)** and **(B)**: Labels for specific sections of the truss.

[illegible]

PLT TYP. wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.36.043

QTY:1

FL/-/4/-/-/R/-/

Scale = .1875" / Ft.

WARNING—TRUCKS, REQUIRE EXTENSIVE CARE IN FABRICATION, WELDING, SHIPPING, INSTALLING AND OPERATING. SEE THE FOLLOWING REFERENCES FOR SAFETY INFORMATION. PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WILCO GOOD TRUSS COMPANY OF AMERICA, 65000 INTERSTATE LAKE, MADISON, WI 53719. FOR SAFETY PRACTICES PLEASE FOLLOW THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GOOD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS SHALL BE CONSIDERED A PART OF THE TRUSS DESIGN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE TRUSS SHALL BE CONSIDERED A PART OF THE TRUSS DESIGN.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC., BY AIA/AIA AND TYP.
CONNECTOR PLATES ARE MADE OF 20/18/1664 (W./H./SS/K) ASTM A563 GRADE 40/60 (W./H./SS) GALV. STEEL. APPLY
PLATE TO EACH FACE OF THUS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. 1664

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANCH A3 OF TP11-2002 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS
A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 00790



TC LL	20.0 PSF	REF	R8228 - 31282
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087080
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON -	82054
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF -	1TG68228Z01

THE UNIVERSITY OF CHICAGO LIBRARY

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpi (+/-)=0.18

Wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



R=1205 U=94 W=3.5"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.36.042

QTY:1

FL/-/4/-/-/R/-/

Scale = .25"/Ft.

WARNING THESE RESULTS INDICATE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND PROTECTING REFERRED TO ACES1 (THE DESIGN COMPONENT OF SAFETY INFORMATION) PUBLISHED BY THE STRESS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA GOOD THRUSSING CONSULTING, FORT MONROE, VIRGINIA, 22060. ENTERPRISE LANE, MOBILE, AL (561)791 FOR SAFETY PRACTICES PRIOR TO TRUSSING THESE STRUCTURES. UNDESSED IF CONDITIONS INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBD CELLING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AIA/AAS) AND TPI.
CONNECTOR PLATES ARE MADE OF 20/19/1664 (M, H, SS/K) ASTM A653 GRADE 40/60 (M, K/H, SS) GALV. STEEL.
PLATES TO EACH EACH OR TOWERS AND DIRECTLY ATTACHED TO THE TOWER POSITION OR DIVIDING LEGS.
APPLY

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.5 OF TP11-2002 SEC.3.

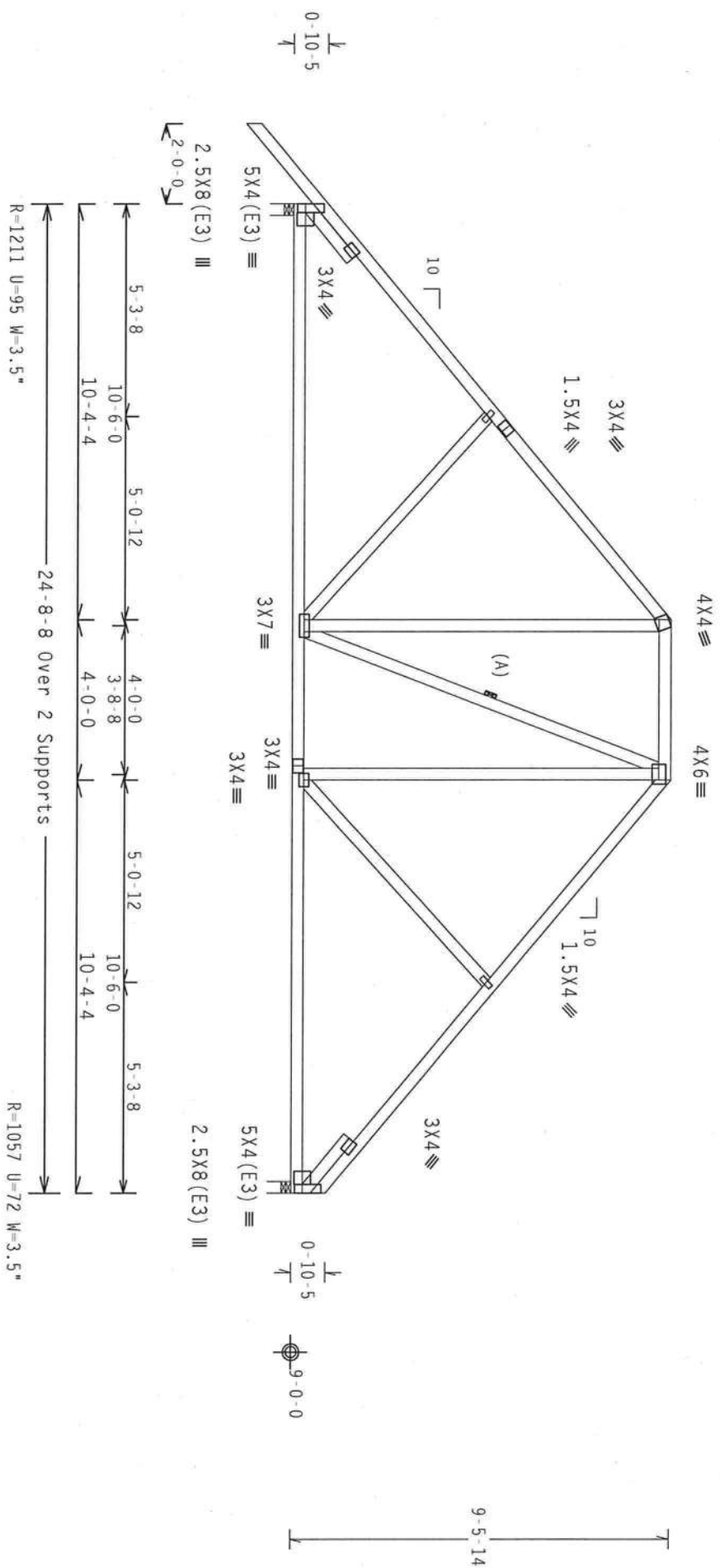
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31283
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087061
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	81976
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.805'
Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.805'
Roof overhang supports 2.00 psf soffit load.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf, $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MWFRS pressures.
(A) Continuous lateral bracing equally spaced on member.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY: 1

Scale = .25"/ft.

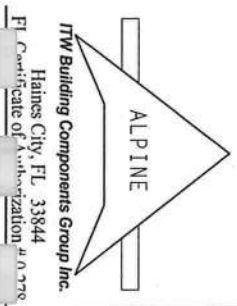
****WARNING**** THUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2718 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 AND WCA (WOOD TRUSS CONFLICT OF AMERICA), 6300 ENTERPRISE LANE, MADISON, MI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR ALUMINUM AND TPI. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR STEEL.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR STEEL. APPLY TO EACH FACE OF TRUSS AND WEBS. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. FOR STEEL. APPLY TO EACH FACE OF TRUSS AND WEBS.

ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PERFORMED AS OF TPI-2002 SEC. 3.1.08 FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 31284
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087044
BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEON-	81970
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1T668228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W14 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

(A) #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3",min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep
increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,
wind BC DL=5.0 psf. $I_w=1.00$ $GCP(+/-)=0.18$

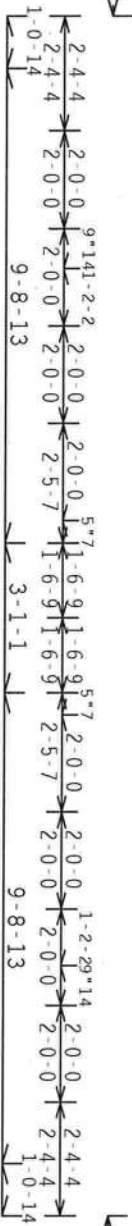
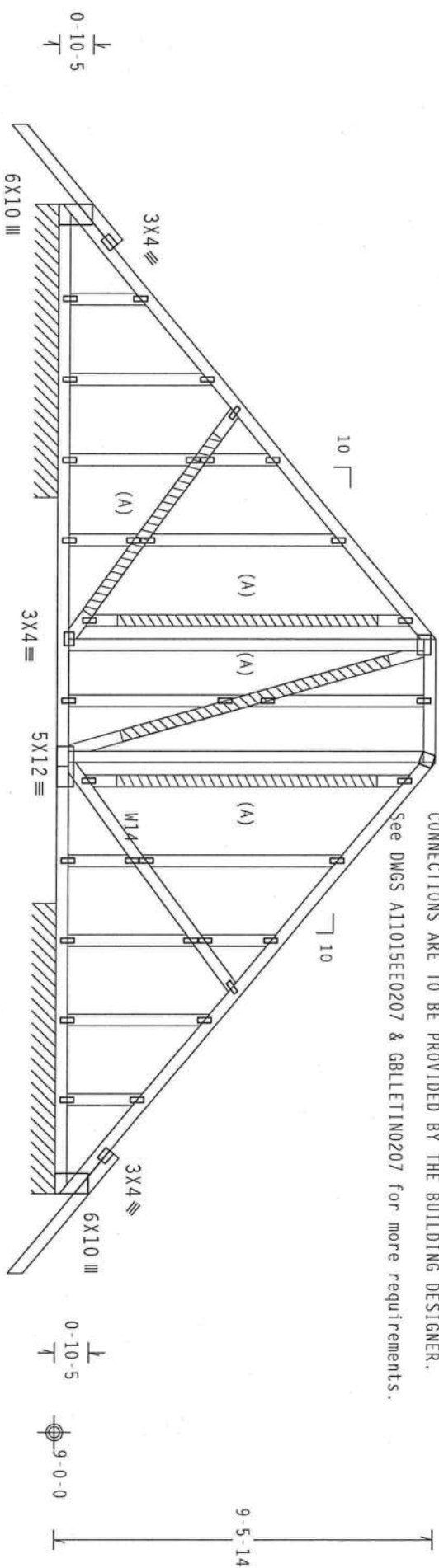
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

See DWGS A11015FE0207 & GBLLETIN0207 for more requirements.



R=371 PLF U=13 PLF W=7-3-8
R=374 PLF U=13 PLF W=7-3-8

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

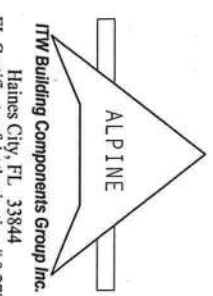
7.36.042

QTY:1 FL/-/4/-/-/R/-

Scale = .25"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS SOCIETY OF AMERICA, 6000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OR NON CONATIONAL DESIGN SPEC. BY AIA/RAI AND TPI. TRUSS TO BE USED FOR 20/10/1600 (24/10/1600) ASH A653 GRADE 40/60 (40, 60/1.35) GALV. STEEL. APPLY TO ALL TRUSSES. TRUSSES SHALL BE DESIGNED, MANUFACTURED, SHIPPED, INSTALLED PER DRAWINGS 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PERFORMED AS OR TPI/RCSE. THE TRUSS COMPANY'S DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 31285
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087081
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 81994
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1T668228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Stubbed Wedge 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 6-9-8 setback with 0-0-0 cant and 2-0-0 overhang. End jacks have 6-9-8 setback with 0-0-0 cant and 2-0-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

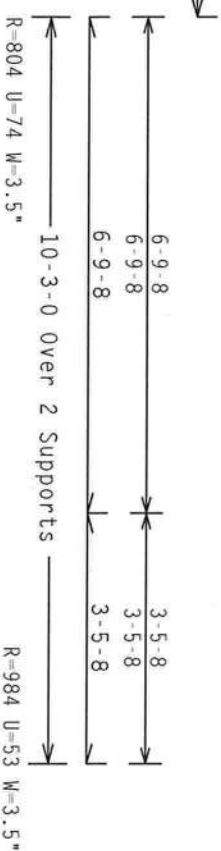
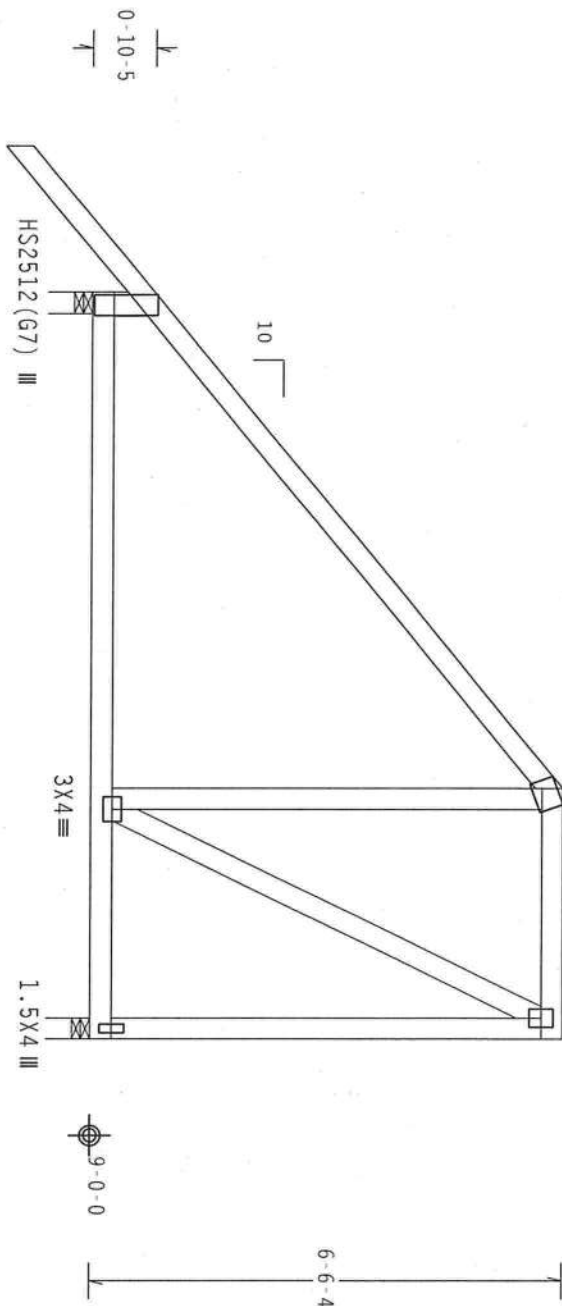
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

#1 hip supports 6-9-8 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase 3X4(R) factor for dead load is 1.50.



PLT TYP. 20 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.36.042

QTY:1

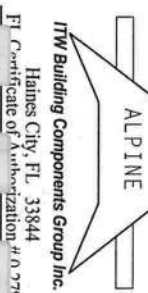
FL/-/4/-/R/-

Scale = .375"/ft.

****WARNING**** TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), UNLESS OTHERWISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF MOST NATIONAL DESIGN SPEC. BY AIA/AIA AND TPI. ITW BCG DESIGNER: JAMES H. HARRIS, JR. (JHH/JSH) ASH AREA: 40/60 (4. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH END OF TRUSS AND TO EACH JOINT. DESIGN, SELECTION PER DRAWINGS. ADDITIONAL DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PL Certificate of Authorization #0-0798

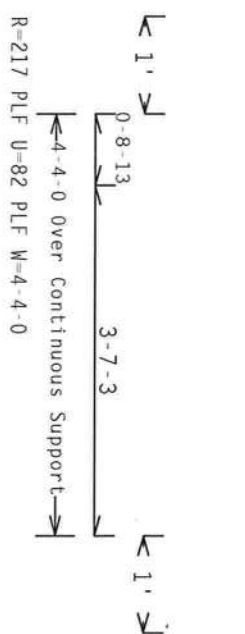
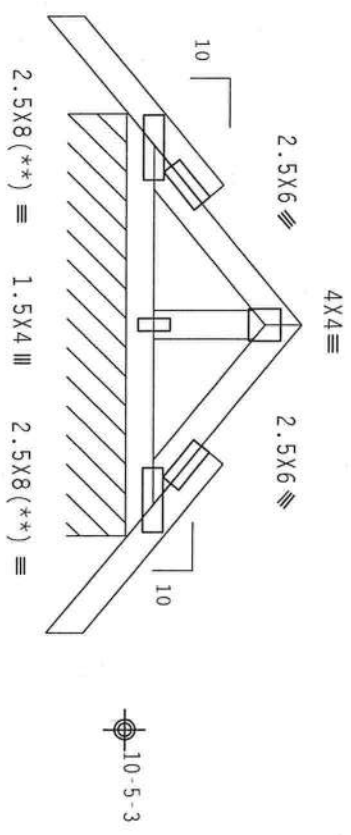
TC LL	20.0 PSF	REF R8228- 31286
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087062
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 81941
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1T668228201

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x4 SP #2 Dense: BLOCK LENGTH = 2.397'
:Lt Slider 2x4 SP #2 Dense: BLOCK LENGTH = 2.397'

Roof overhang supports 2.00 psf soffit load.
Truss spaced at 24.0" OC designed to support 2-0-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

In lieu of structural panels use purlins to brace TC @ 24" OC.
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF
THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS,
AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST
PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL
CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/Rt=1.00(1.25)/0(0)

QTY:1 FL/-/4/-/-/R/-

Scale = .5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS QUALITY DESIGN SPEC. BY ACPA AND TPI. TPI BCG CONNECTION PLATES ARE MADE OF 20/10/10GA (T/10/SS) ASPH A653 GRADE 40/60 (R, R/H, SS) GALV. STEEL. APPLY TO ALL TRUSSES. TRUSSES LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY ACCESS SHALL BE CONDUCTED BY THE TRUSS MANUFACTURER. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. FOR THE TRUSS MANUFACTURER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

(**) 2 plate(s) require special positioning. Refer to scaled plate
plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCpl(+/-)=0.18

Wind reactions based on MMFRS pressures.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

Stacked top chord must NOT be notched or cut in area (NML). Dropped
top chord braced at 24" o.c. intervals. Attach stacked top chord
(SC) to dropped top chord in notched area using 3x4 tie-plates 24"
o.c. Center plate on stacked/dropped chord interface, plate length
perpendicular to chord length. Splice top chord in notched area
using 3x6.



TC LL	20.0 PSF	REF R8228- 31287
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUSR8228 08087063
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 81999
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1TG68228201

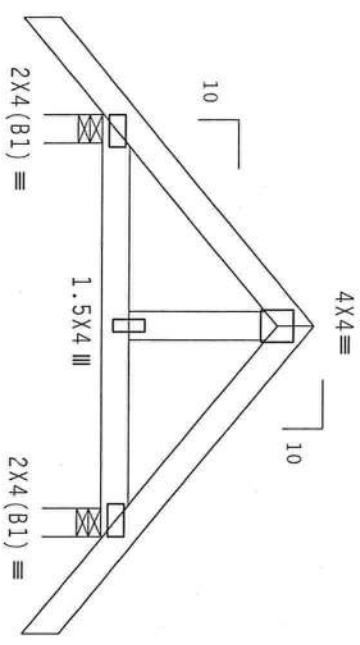
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Meds 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl(+/-)=0.18

Wind reactions based on MMFRS pressures.



10'-5'-3"

2'-2'-8"

1' > 1' >
2'-2'-0" 2'-2'-0"
2'-2'-0" 2'-2'-0"
4'-4'-0" Over 2 Supports
R=258 U=20 W=3.5" R=258 U=20 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

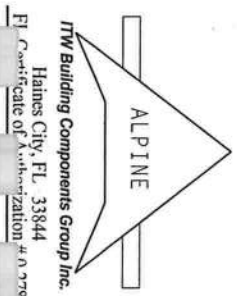
7.36.04 QTY:1 FL/-/4/-/1-/R/-

Scale=.5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WTC (WOOD TRUSS CONNECT OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.



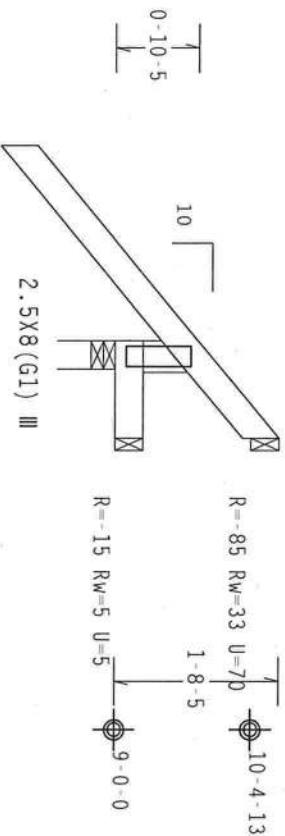
PL Certificate of Authorization #00000000

TC LL	20.0 PSF	REF R8228- 31288
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUSR8228 08087045
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEON- 82005
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TG68228201

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCof (+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.





 2'-0"

 1'-0" Over 3 Supports

 R=328 U=38 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/0(0)$

7.36.042

QTY:1

FL/-14/-1/-1R/-

Scale = 5"/Ft.

WARNING THESE REQUIRE EXTREME CARE IN FABRICATION, HANDLING, UNLOADING, LIFTING, INSTALLING AND DRACING, REFER TO NCSTI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE NATIONAL CONSTRUCTION STEEL INSTITUTE, 220 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC/AISC BRASS COUNCIL OF AMERICA'S RECOMMENDED SAFE WORKING PRACTICES FOR STEEL ERUPTURE LAZE, MAY/JOIN, AT 5/27/1994. FOR SAFETY PRACTICES PLEASE REFER TO REPAIRING OR REPAIRING FUNCTIONS. UNLESS OTHERWISE INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED BUILT CELLING.

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

James City, IL 62044
FI Certificate of Authorization # 0278

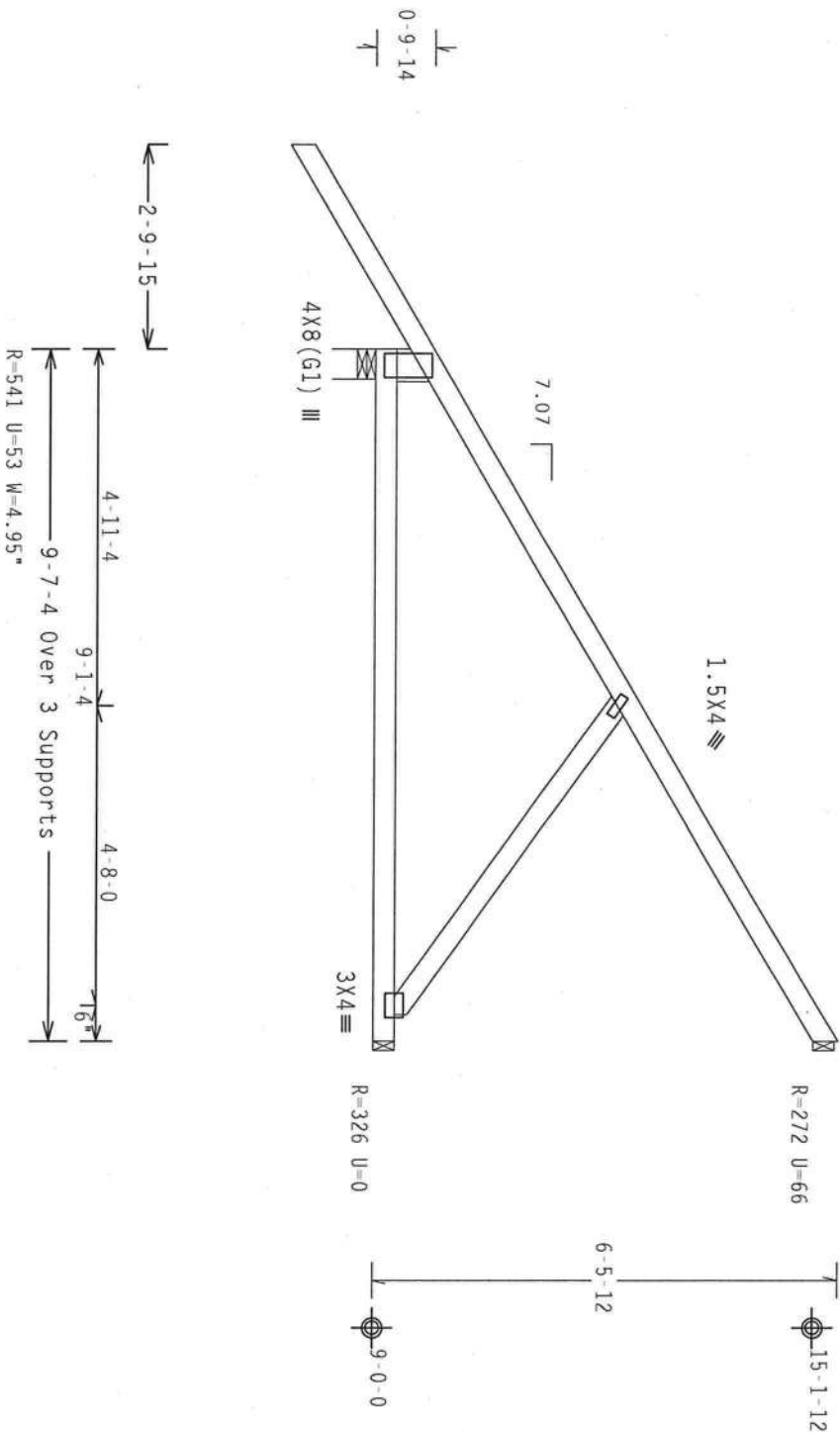


TC LL	20.0 PSF	REF	R8228 - 31289
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087064
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN -	81923
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Lt Studded Wedge 2x6 SP #2:

Hipjack supports 6-9-8 setback jacks with no webs.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18
Wind reactions based on MMFRS pressures.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.04

QTY: 1

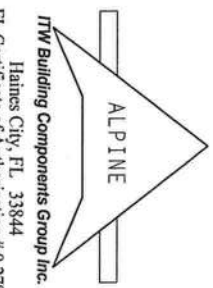
FL/-/4/-/-/R/-

Scale = .375"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPC BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND MATERIALS BEFORE BEGINNING CONSTRUCTION. TPC BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEA A3 OF TPI-2002 SEC. 1.10 PER DR SEAL ON THIS DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31290
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087065
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEON-	81947
DUR.FAC.	1.25	FROM	AH
SPACING	SEE ABOVE	JREF-	1TG68228Z01

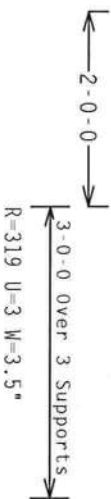
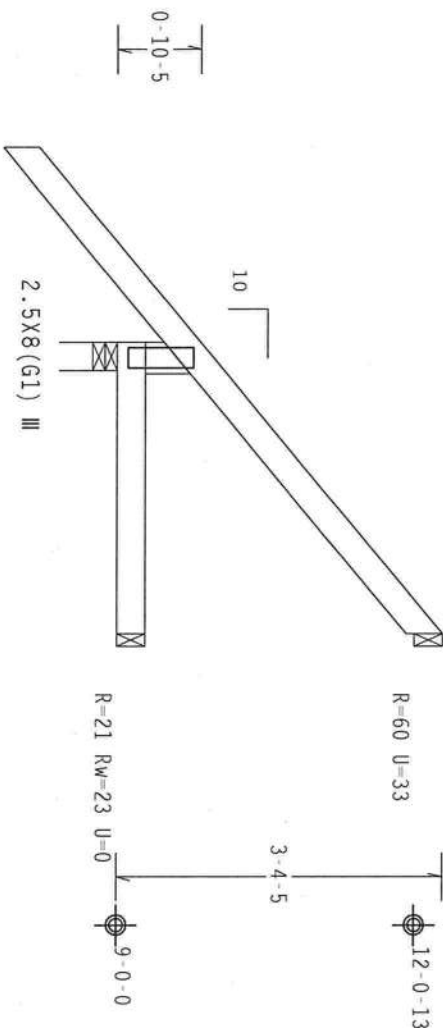
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
:Lt Studded Wedge 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.04

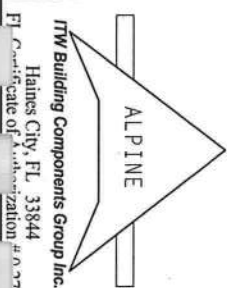
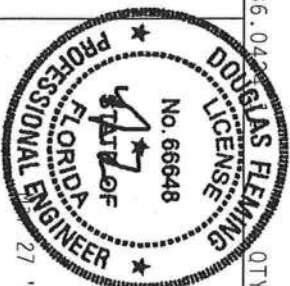
QTY:1

FL/-/4/-/-/R/-

Scale =.5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. THE BCG DESIGN LABLES ARE APPLICABLE TO THIS TRUSS. (SEE TPI/BCG) ASH 2003 GRADE AND 60 (64 K/24.55) GALV. STEEL. APPLY PLATE TO EACH END OF TOP AND BOTTOM CHORDS. (SEE TPI/BCG) SECTION PER DRAWINGS ABOVE. ANY INSPECTION OF PLATES FOLLOWED BY (1) 3.500, SECTION PER DRAWINGS ABOVE. DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31291
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087046
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	81928
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228201

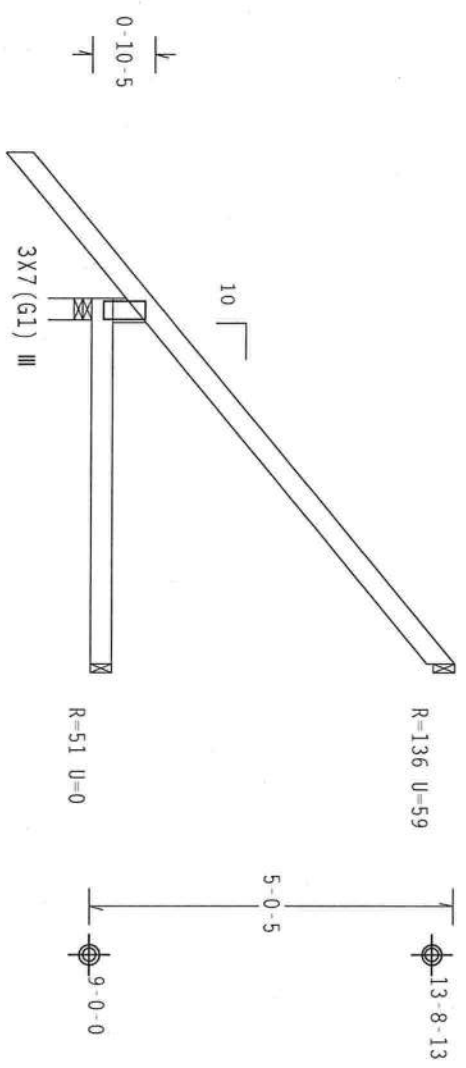
Top Chord 2x4 SP #2 Dense
Bot Chord 2x4 SP #2 Dense
Lt Stubbed Wedge 2x6 SP #2:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



2-0-0
5-0-0 Over 3 Supports
R=386 U=0 W=3.5"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.042

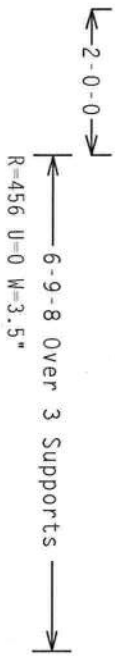
QTY: 1 FL/-/4/-/R/-

Scale = .375"/Ft.

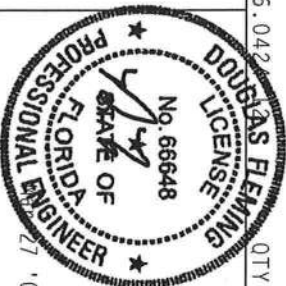
ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL Certificate of Authorization #0-778				DOUGLAS FLEMING FLORIDA PROFESSIONAL ENGINEER No. 66648 DATE OF EXPIRATION 07/27/08			
TC LL	20.0 PSF	REF	R8228-31292	TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087047	BC LL	0.0 PSF	HC-ENG DF/DF	*
TOT.LD.	40.0 PSF	SEQN-	81932	DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01				

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.



Scale = .375"/Ft.



TC LL	20.0 PSF	REF	R8228- 31293
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087066
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	81936
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228201

110 mph wind, 19.32 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Refer to DWG PIGBACKB0207 for piggyback details.

PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.37.052

QTY:1

FL/-/4/-/-/R/-/

Scale = .5" / Ft.

***WARNING:** THESE RIGID, EXTREME CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO ICSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CRIBBS SAFETY INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WOOD GOOD TRUSS COUNCIL OF AMERICA, 65000 INTERSTATE LAKE, MADISON, WI 53719 FOR SAFETY PRACTICES AND WICK TO PASSING THESE STRUCTURES. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

ALPINE

ITW Building Components Group Inc.

FBI Certificate of Authorization # 00778



TC LL	20.0 PSF	REF R8228- 31294
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087083
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEON- 26335 REV
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TG68228Z01

$$\begin{array}{c} \text{---} \\ | \\ \text{---} \end{array} \begin{array}{c} \text{---} \\ | \\ \text{---} \end{array} \begin{array}{c} \text{---} \\ | \\ \text{---} \end{array}$$

JREF - 1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

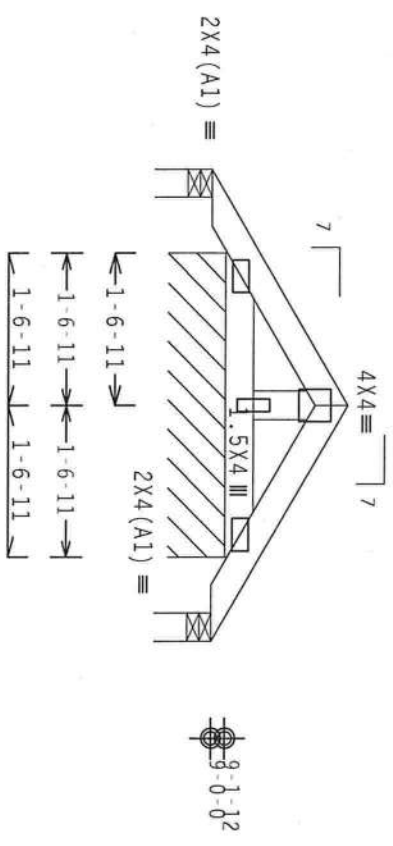
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

See DWGS A11015EE0207 & GBLLETTIN0207 for more requirements.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MMFRS pressures.



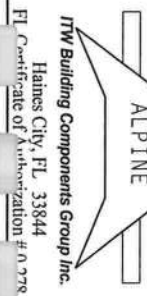
4-10-2 over 3 Supports
R=22 U=8 W=3.5"
R=100 PLF U=7 PLF W=3-1-6

PLT TYP. Wave

Design Crit: TPI-2002 (STD)
Cq/RT=1.00(1.25)/0(0)

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND WCA GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

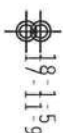


FL / - / 4 / - / - / R / -		Scale = .5" / Ft.	
TC LL	20.0 PSF	REF	R8228 - 31296
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087068
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN -	82060
DUR. FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228201

110 mph wind, 18.84 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ Gcpi (+/-)=0.18

Refer to DWG PIGBACKB0207 for piggyback details.
POSITION OF TRUSS UNDER PIGGYBACK IS TO BE

BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



1-7-4

Scale = .5" / Ft.

WARNING: THESE TRUCKS, INCLUDING EXISTING CAGE IN FABRICATION, MANUFACTURED BY THE TRUSS PRACTICE INSTITUTE, 210 NORTH LEE STREET, SUITE 212, ALEXANDRIA, VA, 22314, AND TRUSS COUNCIL OF AMERICA, 63000 ENTERPRISE LANE, SUITE 501, 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS, UNLESS SPECIFICALLY INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0070



TC LL	20.0 PSF	REF	R8228 - 31297
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087087
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN -	26340 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

110 mph wind, 22.25 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, Wind BC DL=2.0 psf. $I_w=1.00$ Gcpi (+/-)=0.18

Refer to DWG PIGBACKB0207 for piggyback details.
POSITION OF TRUSS UNDER PIGGYBACK IS TO BE

PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

-----	(LUMBER	DUR.FAC.=1.25	/	PLATE	DUR.FAC.=1.25)
TC - From	63 PLF at	0.00 to	63 PLF at	9.00	
TC - From	63 PLF at	9.00 to	63 PLF at	18.00	
BC - From	4 PLF at	0.00 to	4 PLF at	18.00	

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.37.0521

QTY:1

FL/-/4/-/1/-/R/-/

Scale = 375"/Ft.

WARNING THESE RIGIDITY EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO AC308 (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE STEEL PRACTICE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC 308 TRUSS CONNECTOR OF AMERICA, 63000 ENTERPRISE LAKE, NORTON, IA, 52709 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. UNLESS OTHERWISE INDICATED, THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CELLING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI.
CONNECTOR PLATES ARE MADE OF 20/19/1664 (W, H/55/K) ASTM A653 GRADE 40/60 (W, K/H/55) GALV. STEEL, APPLY PLATES TO EACH FACE OF JOISTS AND JOIST ATTACHMENT ON TOP FLANGE.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEA OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS LOCATED ON THIS DESIGN, POSITION PER DRAWTINGS 100A

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228 - 31298
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087088
BC LL	0.0 PSF	HC - ENG	DF /DF
TOT.LD.	40.0 PSF	SEQN -	26345 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 22.09 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=-0.18$

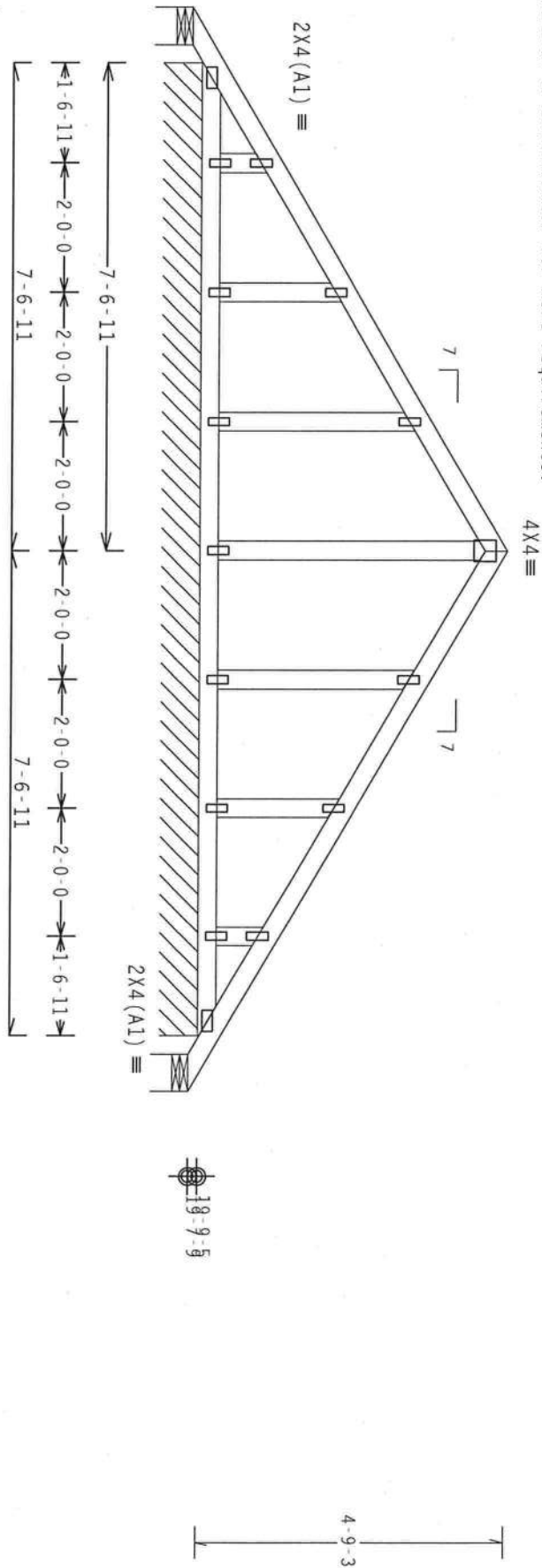
Wind reactions based on MMFRS pressures.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

Refer to DWG PIGBACK0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

See DWG A11030EE0207 & GBLLETT0207 for more requirements.

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 63 PLF at 0.00 to 63 PLF at 8.42
TC - From 63 PLF at 8.42 to 63 PLF at 16.84
BC - From 4 PLF at 0.00 to 4 PLF at 16.84
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=20 $R_w=5.7$ $U=5.4$ $W=6.946"$
R=70 PLF $U=23$ PLF $W=15-1-5$

R=20 $U=7$ $W=6.946"$

Note: A11 Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY: 1 FL/-/4/-/-/R/-

Scale = .375"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATOR'S INSTRUCTIONS, INCLUDING, SHIPPING, INSTALLING AND BRACING. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONNEL DURING THE INSTALLATION OF THE TRUSS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONNEL DURING THE INSTALLATION OF THE TRUSS. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING OR TO THE PERSONNEL DURING THE INSTALLATION OF THE TRUSS.

ALPINE
NTW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #03700



TC LL	20.0 PSF	REF	R8228- 31299
TC DL	10.0 PSF	DATE	03/27/08
BC DL	10.0 PSF	DRW	HCUSR8228 08087069
BC LL	0.0 PSF	HC-ENG DF/DF	
TOT. LD.	40.0 PSF	SEQN-	82153
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TG68228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.10 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

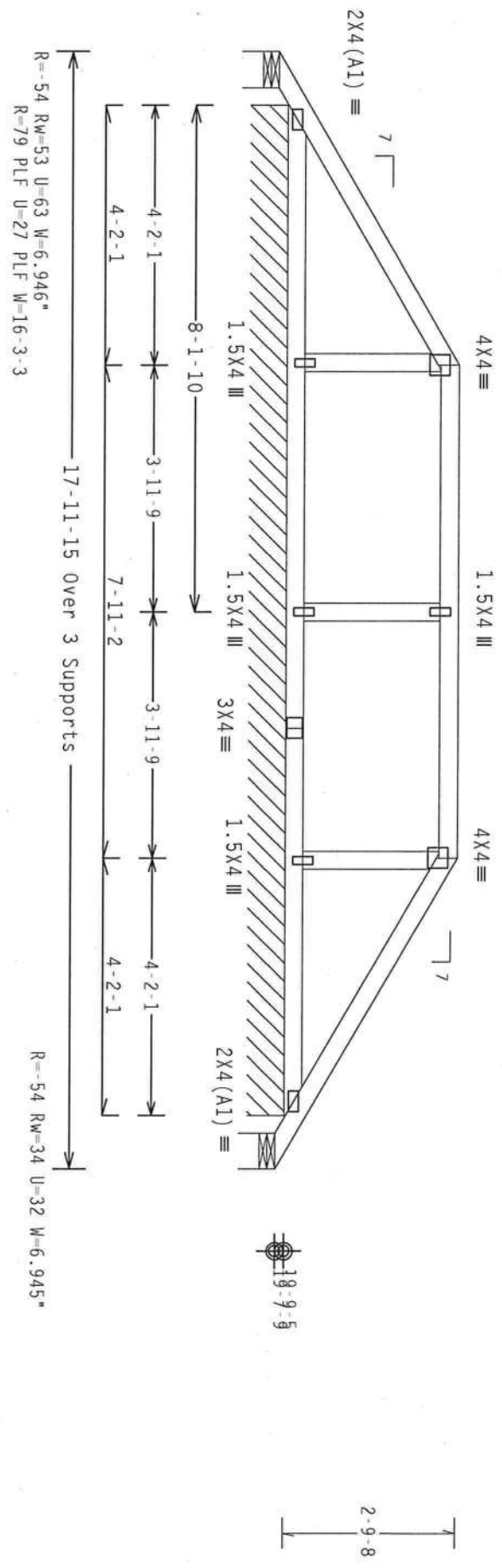
Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)	
TC - From	63 PLF at 0.00 to 63 PLF at 5.03
TC - From	63 PLF at 5.03 to 63 PLF at 12.96
TC - From	63 PLF at 12.96 to 63 PLF at 18.00
BC - From	4 PLF at 0.00 to 4 PLF at 18.00

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

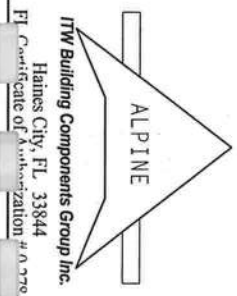


PLT TYP. Wave
Design Crit: TPI-2002 (STD) / FBC
Cq/RT=1.00(1.25)/0(0)

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPARES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AREA) AND TPI. THE BCG DESIGNER HAS REVIEWED THE TRUSS AND ASSURED THAT THE TRUSS MEETS ALL REQUIREMENTS OF THE NDS AND TPI. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNE AS OF TPI 2002 SEC. 2.3. THE DESIGNER'S DESIGN SHALL BE THE SUIVABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



QTY: 1	FL / - / 4 / - / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R8228 - 31301
TC DL	10.0 PSF	DATE 03/27/08
BC DL	10.0 PSF	DRW HCUR8228 08087089
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEQN - 26352 REV
DUR. FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TG68228Z01

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	Web	2x4	SP	#3		

110 mph wind, 20.27 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 Gcpi (+/-)=0.18

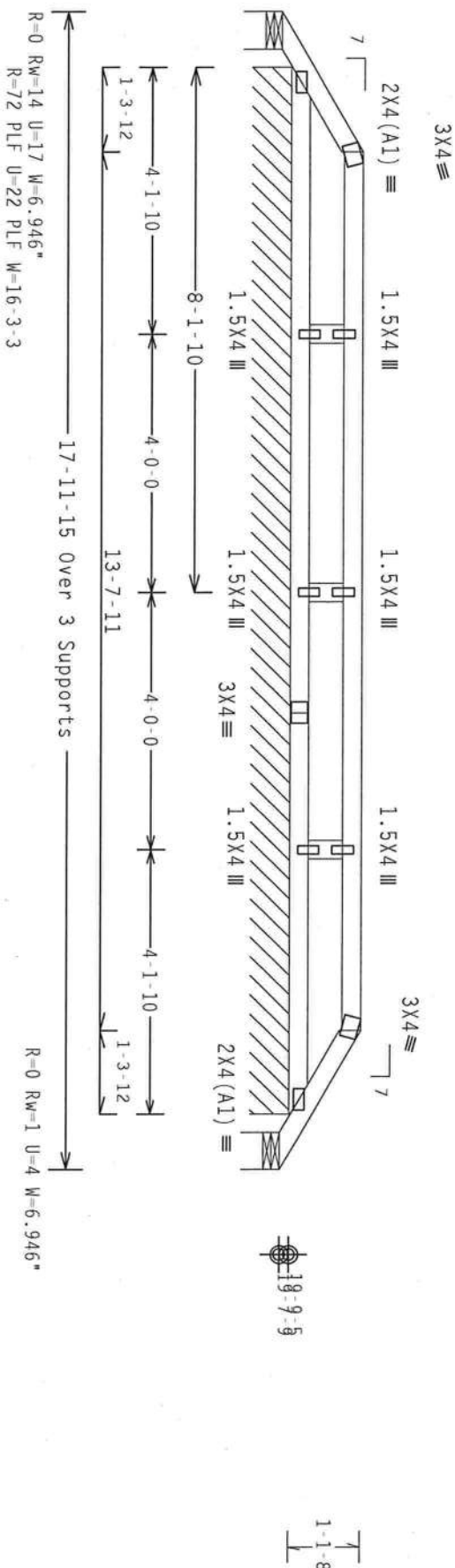
Wind reactions based on MMFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details.
PORTION OF TRUSS UNDER PIGGYBACK IS TO BE
BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 63 PLF at 0.00 to 63 PLF at 2.18
TC - From 63 PLF at 2.18 to 63 PLF at 15.82
TC - From 63 PLF at 15.82 to 63 PLF at 18.00
BC - From 4 PLF at 0.00 to 4 PLF at 18.00

In lieu of structural panels or rigid ceiling use purtins to brace all flat TC @ 24" OC, all BC @ 24" OC.



PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

 $Cq/RT=1.00(1.25)/0(0)$

7.36.043

QTY: 1

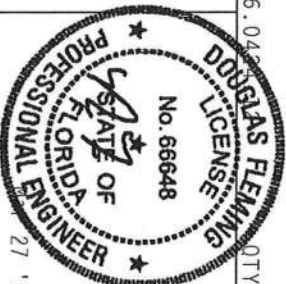
FL/-/4/-/-/R/-/

Scale = .375"/Ft.

WARNING: TRIES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRACING. NEED TO DESIGN (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (GOOD TRUSS COUNCIL OF AMERICA, 65000 ENTERPRISE LAKE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CEILING.

****IMPORTANT*** PURCHASER A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING BRACING OF TRUSSES.

DESIGN COMPONENTS TO BE APPLICABLE REQUIREMENTS FOR MODIFIED DESIGN SPEC. (BY AIAA) AND THE CONDUCTOR PLATES ARE MADE OF 2018-T6 ALUMINUM (AL-7053) ASTM A653 GRADE 40/60 (W/.M/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF THISSAM. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2. PER ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPI-1-2002 SEC. 3. A SEAL ON THIS TSS. DRAINAGE INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLVE FOR THE THISS COMPONENT DESIGN SHOWS THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



27.08

Haines City, FL 33844
FL Certificate of Authorization # 00379

SPACING 24.0"

JREF - 1TG668228Z01

BOTTOM CHORD FILLER DETAIL.

* OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

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+      3X4 WAVE OR 4X8 TRULOX
++     2X4 WAVE OR 3X6 TRULOX

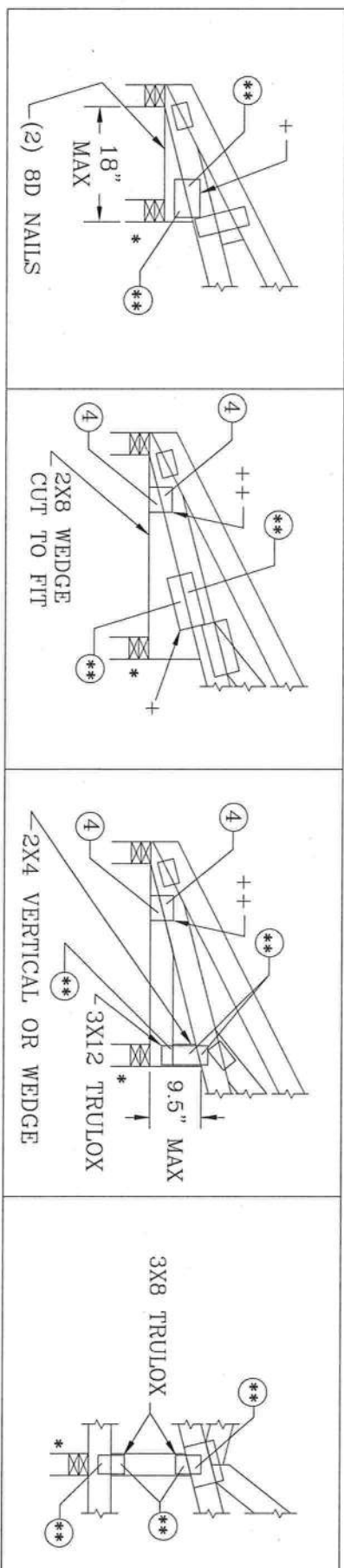
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0.120") 1.375", NAILS, REQUIRED FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED IN CIRCLES MUST BE APPLIED TO EACH FACE OF THE TRUSS. SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS

REFER TO ENGINEERS SEALED DESIGN REFERENCING THIS
DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT
SHOWN.

ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMMODATE REQUIRED NAILS (**)

FILLER BOTTOM CHORD OR WEDGE SPECIES	MAXIMUM REACTION		MINIMUM BEARING AREA	** REQUIRED NAILS PER FACE WITH TRULOX PLATES					
	DOWNWARD	UPLIFT		1.00 D.O.L.	1.15 D.O.L.	1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.	
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	12	11	10	9	8	
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	6	
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6	
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8	
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7	
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	6	

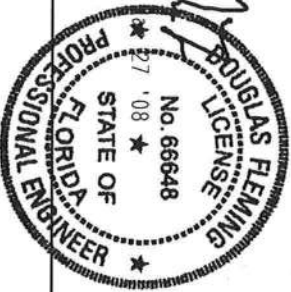


THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884,132

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER AWS1/TF1, SEC. 2.



TC LL	—	PSF	REF	BC FILLER
TC DL	—	PSF	DATE	2/23/07
BC DL	10.0	PSF	DRWG	BCFILLER0207
BC LL	—	PSF	—ENG	DLJ/KAR
TOT. LD.	—	PSF		
DUR. FAC. 1.0/1.15/1.25/1.33				
SPACING 24.0"				

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

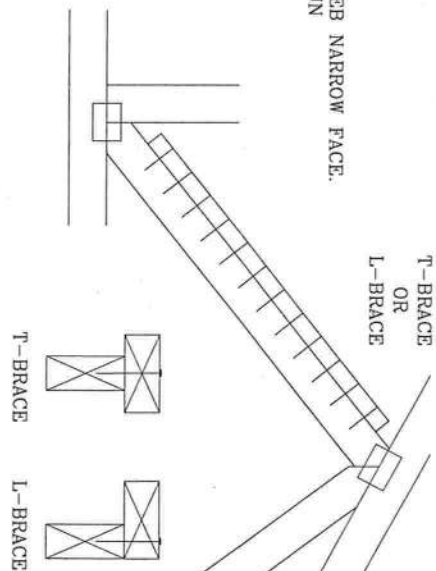
WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

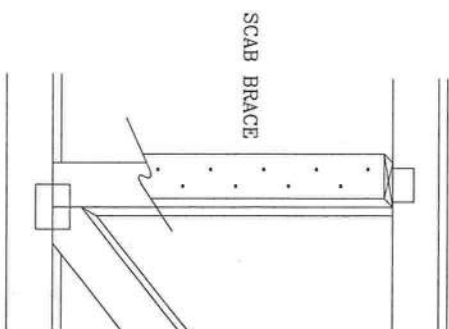
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA 22314 AND VICA (WOOD TRUSS COUNCIL OF AMERICA, 6380 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTORS WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. TPI, BCG, CONNECTOR PLATES ARE MADE OF 20/90/60GA (V/A/SS/30) ASTM A653 GRADE 40/60 (V/A/SS/30) GALVALUME. ALL TRUSSES SHALL BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH THE DESIGN POSITION PER DRAWINGS 1600-2. ANY INSPECTION OF TRUSSES SHALL BE CONDUCTED ON THIS PER ANNEA A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCB SUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

MAXIMUM NUMBER OF NAIL TIMES DURING EQ TRAINING

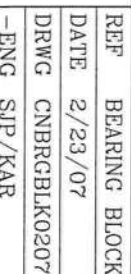
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (fc-perp) IS AT LEAST THAT OF THE CHORD.

MINIMUM NAIL SPACING DISTANCES

THIS DRAWING REPLACES DRAWING B139 AND CNBRG1K0699

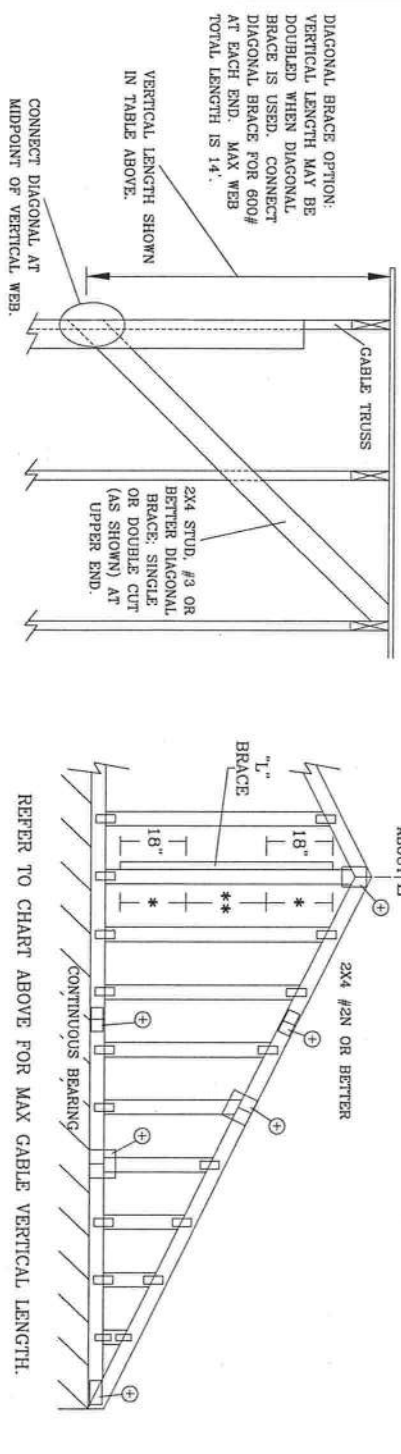
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

[illegible]

MAX GABLE VERTICAL LENGTH																
2X4 GABLE VERTICAL SPACING	BRACE SPECIES	GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
			STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STANDARD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SP	#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			STUD	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:		GROUP A:		GROUP B:	
SPRUCE-PINE-FIR	#1 / #2	STUD	STUD	HEM-FIR	#1 & BTR
DOUGLAS FIR-LARCH	#3	STUD	STUD	DOUGLAS FIR-LARCH	#1
SOUTHERN PINE	#3	STUD	STUD	SOUTHERN PINE	#2

GABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
 PROVIDE UPLIFT CONNECTIONS FOR 80 PSF OVER CONTINUOUS BEARING (5 PSF TO DEAD LOAD).
 GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH "L" BRACE WITH 10d NAILS.
 * FOR (1) "L" BRACE: SPACE NAILS AT 2' 0".
 ** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0".
 IN 18' END ZONES AND 6" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 90% OF WEB MEMBER LENGTH.



ATTACH EACH 1" BRACE WITH 10d NAILS.

* FOR (1) 1" BRACE: SPACE NAILS AT 2' O.C.

** IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) 1" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

1" BRACING MUST BE A MINIMUM OF 80% OF WEB LENGTH.

GABLE VERTICAL PLATE SIZES			
VERTICAL LENGTH	NO SPICE		
LESS THAN 4' 0"	1X4 OR 2X3		
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4		
GREATER THAN 11' 6"	2.5X4		

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND HEEL PLATES.

REF ASCE7-02-CAB11015
 DATE 2/23/07
 DRWG A11015EE0207
 -ENG



ITV BUILDING COMPONENTS GROUP, INC.
 POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY ITV TRUSS COUNCIL OF AMERICA, 218 NORTHEAST ST. STE. 312, ALEXANDRIA, VA 22314 AND VICA C/O/D TRUSS COUNCIL OF AMERICA, 218 NORTHEAST ST. STE. 312, ALEXANDRIA, VA 22314. SAFETY FACTORS PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL DIMENSIONS AND ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
 IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES IN ACCORDANCE WITH ITV OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITV BCG CONNECTOR PLATES ARE MADE OF 2017/2016 ALUMINUM. DESIGN SPECIFIED BY AISC AND ITV. GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MAX. TOT. LD. 60 PSF
 MAX. SPACING 24' 0"

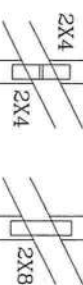
The diagram illustrates a cable-stayed bridge structure. A central vertical line is labeled "SYM. ABOUT C.L." (Symmetrical about Centerline). A cable is shown extending from the centerline down to the deck. The vertical distance from the centerline to the deck is labeled "CABLE VERTICAL LENGTH 11'". The deck is supported by a series of vertical hangers. The diagram also shows the bridge piers and the approach spans. The overall structure is symmetrical about the centerline.

GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

④ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "I" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

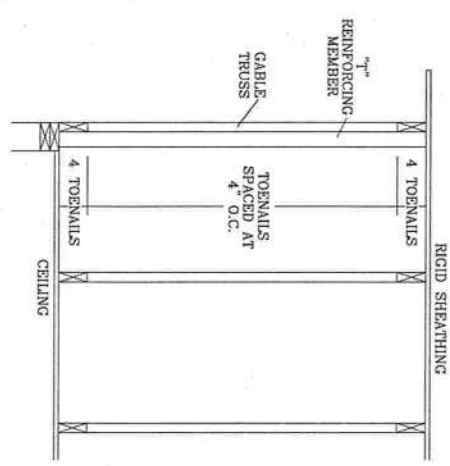
10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE CABLE DETAIL FOR ASCE
OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS
 A11035EN0207, A110155EN0207, A09015EN0207, A08015EN0207, A07015EN0207
 A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
 ASCE 7-98 GABLE DETAIL DRAWINGS
 A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A09015EC0207,
 A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A09030EC0207
 ASCE 7-02 GABLE DETAIL DRAWINGS
 A13015E0207, A12015E0207, A11015E0207, A10015E0207, A09015E0207,
 A13030E0207, A12030E0207, A11030E0207, A10030E0207, A09030E0207
 ASCE 7-05 GABLE DETAIL DRAWINGS^a
 A13015E0207, A12015E0207, A11015E0207, A10015E0207, A09015E0207,
 A13030E0207, A12030E0207, A11030E0207, A10030E0207, A09030E0207
 SEE APPROPRIATE ALPINE GABLE DETAIL. (ASCE OR SBCCI
 WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE
 VERTICAL LENGTH.



THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035

TO CONVERT FROM l_v TO l_r REINFORCING MEMBERS, MULTIPLY l_v FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 l_v BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCS WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

SBCI WIND LOAD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	" ⁴ " REIN, MR. SIZE	SBCI	ASCE
110 MPH 15 FT	2x4	10 %	10 %
	2x6	40 %	50 %
110 MPH 30 FT	2x4	10 %	10 %
	2x6	50 %	50 %
100 MPH 15 FT	2x4	10 %	10 %
	2x6	30 %	50 %
100 MPH 30 FT	2x4	10 %	10 %
	2x6	40 %	40 %
90 MPH 15 FT	2x4	20 %	10 %
	2x6	20 %	40 %
90 MPH 30 FT	2x4	10 %	10 %
	2x6	30 %	50 %
80 MPH 15 FT	2x4	10 %	20 %
	2x6	10 %	30 %
80 MPH 30 FT	2x4	20 %	10 %
	2x6	20 %	40 %
70 MPH 15 FT	2x4	0 %	20 %
	2x6	0 %	20 %
70 MPH 30 FT	2x4	10 %	20 %
	2x6	10 %	30 %

EXAMPLE

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT

CABLE VERTICAL = 24" O.C. SP #3

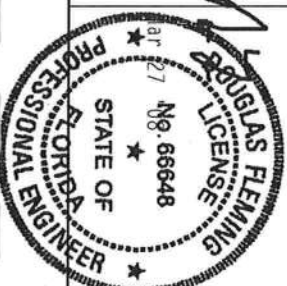
"I" REINFORCING MEMBER SIZE = 2X4
"J" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM 1 REINFORCED CABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA



REF	LET-IN VERT
DATE	2/23/07
DRWG	GBILETTIN0207
-ENG	DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

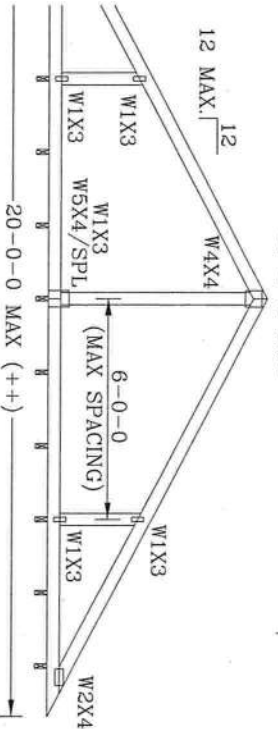
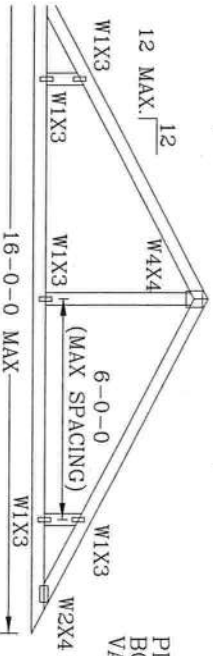
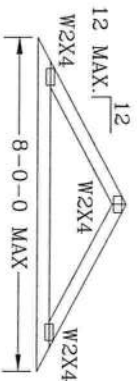
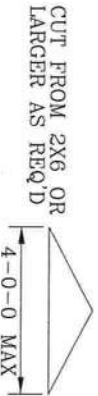
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

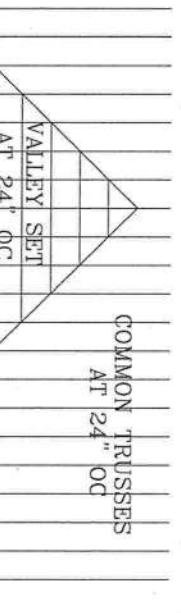
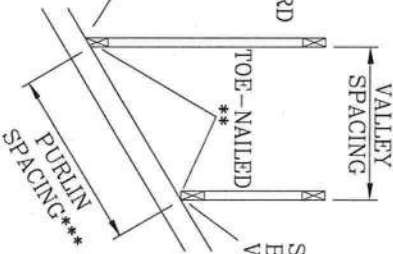
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98,
ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN
HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL,
WIND TC DL=5 PSF



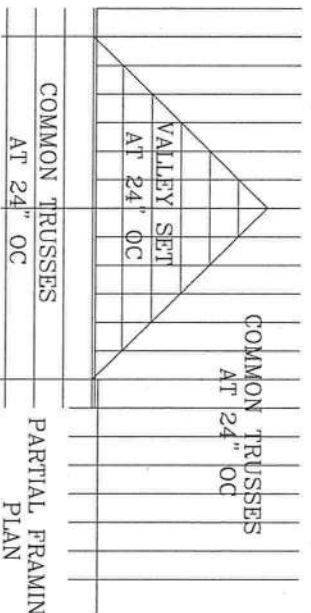
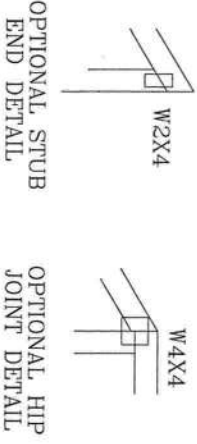
SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.



*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%
LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED
WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,
EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.



THIS DRAWING REPLACES DRAWING A105



ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE
INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22304 AND WTCA (WOOD TRUSS COUNCIL OF
AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE
FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL
PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL
NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN
CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PAI AND TPI
N.A.). BRACING DETAILS ARE MADE UP OF 2018/16/6 C/A/H/SS/2 STR A653 GRADE 40/60 (AIA/H/SS)
CAN. STEEL. BRACING PLATES ARE MADE UP OF 2018/16/6 C/A/H/SS/2 STR A653 GRADE 40/60 (AIA/H/SS)
CAN. STEEL. BRACING PLATES ARE MADE UP OF 2018/16/6 C/A/H/SS/2 STR A653 GRADE 40/60 (AIA/H/SS)
DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FURNISHED UNDER THIS PER
ANNEX A3 OF TPI 1-2008 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL
ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND
USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER
ANSI/TPI 1 SEC. 2.



TC LL	30	30	40 PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	2/23/07
BC DL	10	10	10 PSF	DRWG	VALTRUSS0207
BC LL	0	0	0 PSF	-ENG	MLH/KAR
TOT. LD.	60	55	57 PSF		
DUR.FAC.	1.25/1.33	1.15/1.15			
SPACING	24"				

TOP CHORD	2X4	#2 OR BETTER
BOT CHORD	2X4	#2 OR BETTER
WEBS	2X4	#3 OR BETTER

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

PIGGBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

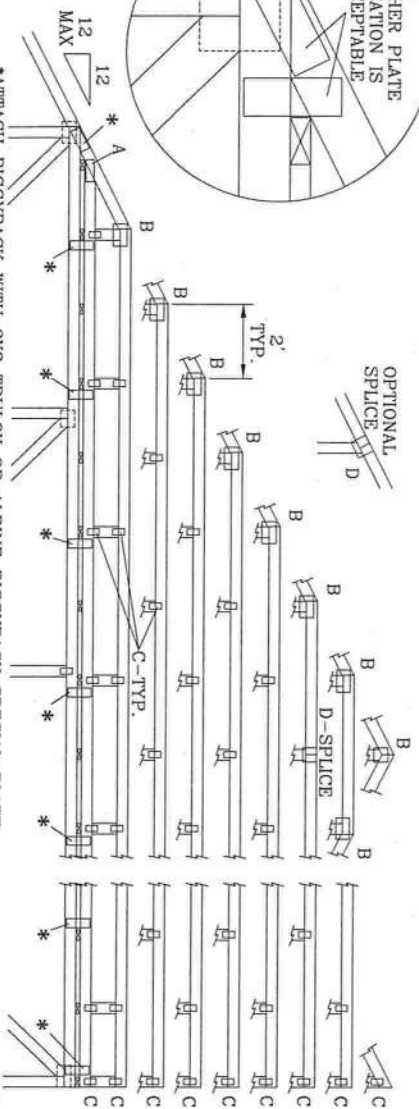
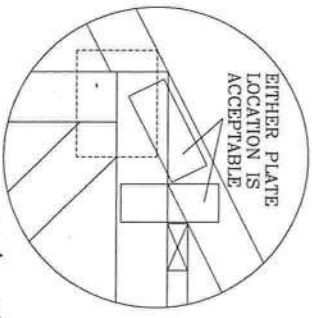
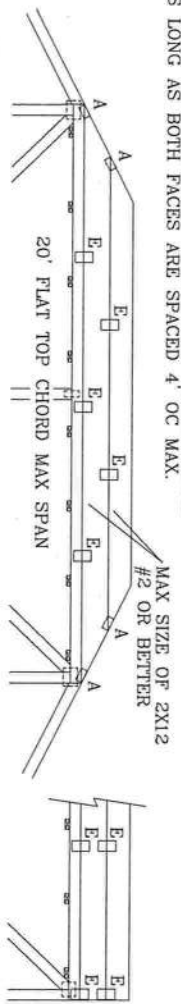
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLD LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

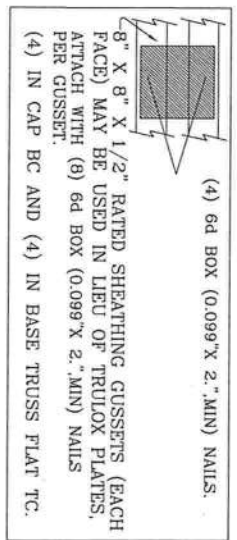
150 MPH WIND, 30 MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E.*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4" OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

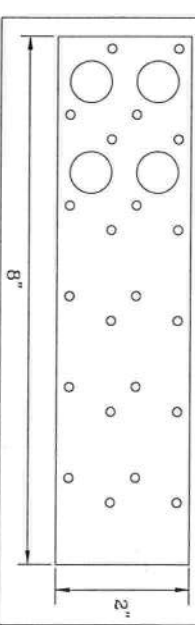


JOINT TYPE	SPANS UP TO			
	30'	34'	36'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS OR EQUIV. PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7' 9"	NO BRACING
7' 9" TO 10'	1x4 "L" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113 X 2.5" MIN) NAILS AT 4' OC.
10' TO 14'	2x4 "L" BRACE. SAME GRADE, SPECIES AS WEB MEMBER. OR BETTER. AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135 X 3.5" MIN) NAILS AT 4' OC

ATTACH TEETH TO THE PIGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045



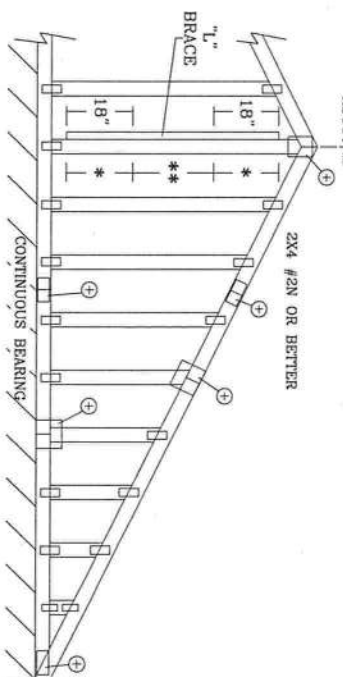
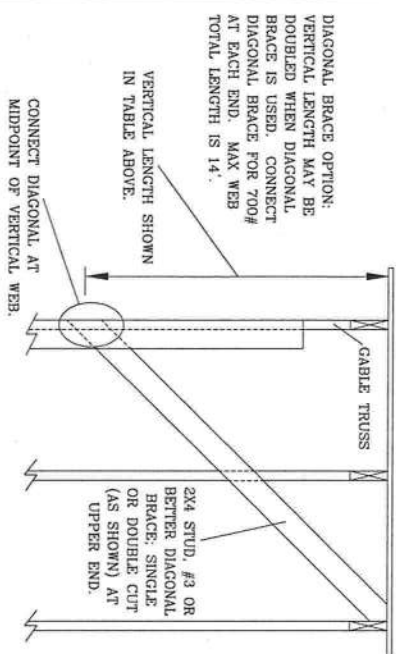
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING ISSUES REQUIRE EXTREME CARE IN FABRICATING, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA 22314 AND VITA CLOUD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LN, MADISON, WI 53793 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TASKS. IF ANY UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR, ITL BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AN FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE PER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONSIDERS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AREA AND TPI. DESIGN CONSIDERS THAT PLATES ARE MADE OF 20/18/66GA W/H/S55/50 ASYR A653 GRADE 40/60 W/H/A550 GALV. ZN COATED STEEL. ALL DIMENSIONS ARE IN FEET AND DECIMALS THEREOF. ALL DIMENSIONS ON THIS PER DESIGN POSITION PER DRAWINGS 1604-2 AND INSPECTION OF LESS THAN THOSE LOCATED ON THIS PER ANNEX A OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROTECTIVE ENGINEERING RESPONSIBILITY SCALE FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MAX LOADING	REF	PIGGBACK
55 PSF AT	DATE	2/23/07
1.33 DUR. FAC.	DRWG	PIGBACKB0207
50 PSF AT	-ENG	DLJ/KAR
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING		24.0"



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPLICE, AND HEEL PLATES.

* ATTACH EACH T_e BRACE WITH 10d NAILS.
* FOR (1) T_e BRACE: SPACE NAILS AT 2" O.C.
IN 16" END ZONES AND 4" O.C. BETWEEN ZONES
** FOR (2) T_e BRACES: SPACE NAILS AT 3" O.C.
IN 16" END ZONES AND 6" O.C. BETWEEN ZONES
 T_e BRACING MUST BE A MINIMUM OF 80% OF WEB
MEMBER LENGTH.

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
GABLE END SUPPORTS LOAD FROM 4' 0"

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$

CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

PLYWOOD OVERHANG

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
ALONG EACH L BRACE WITH 10D NAILS.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

MEMBER LENGTH.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING THESE REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSSING PLATE INSTITUTE, 2108 NINE E. ST., SUITE 302, ALEXANDRIA, VA 22304 AND VITA (VOID) TRUSS CONDUCT OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 535 NORTH ZEEB RD., CHICAGO, IL 60610. THESE FUNCTIONS ARE THE RESPONSIBILITY OF THE DESIGNER. THE TRUSSING PLATE ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCO, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND THE DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF END PLATES (NDS/ASCE 40.60 AND 40.60.1.3.3) AND THE DESIGN OF THE TRUSSING PLATE. THE END PLATE SHALL BE DESIGNED TO RESIST A MINIMUM OF 150 KIPS PER ANGLE, 43 OF TPI-1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER AISC/TPI-1 SEC. 2.

DOUGLAS FLEMING
LICENSE
No. 66648
JUN 27 '08

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF: ASCE7-02-GABI1030

DATE 2/23/07

DRWG A11030FE0207

-ENG

COLUMBIA COUNTY OFFICE OF BRYAN ZECHER

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 18-5S-17-09280-122

Building permit No. 000026923

Use Classification SFD, UTILITY

Fire: 77.00

Permit Holder BRYAN ZECHER

Waste: 201.00

Owner of Building BRYAN ZECHER

Total: 278.00

Location: 699 SW MANDIBA DRIVE

Date: 11/03/2008

John D. Howe

Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)

COLUMBIA COUNTY OFFICE OF CIVIL ENGINEERING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

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Date: 11/03/2008

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



John D. Hove

0804-15

FEEs:

	CODE	UNIT
ROAD IMPACT FEE 10100003632400		
	210	1
EMS IMPACT FEE 10300003632210		
FIRE PROTECTION IMPACT FEE 10200003632220		
CORRECTIONS IMPACT FEE 00100003632200		
SCHOOL IMPACT FEE 00100003632900		
TOTAL FEES CHARGED	CHECK NUMBER	

\$1,046.00

\$29.88

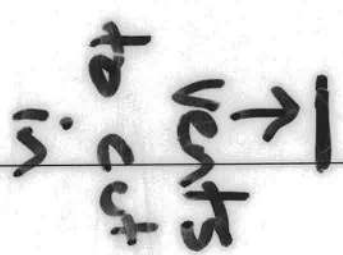
\$78.63

\$409.16

\$1,500.00

\$3,063.67

26325



PAGE NO:
1 OF 1