

DATE 10/16/2006

Columbia County Building Permit

PERMIT

This Permit Expires One Year From the Date of Issue

000025126

APPLICANT DENNIS O'NEIL

PHONE 454-2476

ADDRESS P.O. BOX 1633

HIGH SPRINGS

FL 32643

OWNER DAVID WATSON

PHONE _____

ADDRESS 103 SW BUCK COURT

FT. WHITE

FL 32038

CONTRACTOR O'NEIL CONSTRUCTION

PHONE 454-2476

LOCATION OF PROPERTY

47S, TL ON 27, TL ON BENJAIN ROAD, TL ON BUCK COURT,

5TH LOT ON RIGHT

TYPE DEVELOPMENT

SFD, UTILITY

ESTIMATED COST OF CONSTRUCTION

114500.00

HEATED FLOOR AREA

2290.00

TOTAL AREA

4167.00

HEIGHT

1 STORIES

FOUNDATION CONC

WALLS

FRAMED

ROOF PITCH

8/12

FLOOR

SLAB

LAND USE & ZONING

A-3

MAX. HEIGHT

25

Minimum Set Back Requirements:

STREET-FRONT

30.00

REAR

25.00

SIDE

25.00

NO. EX.D.U.

FLOOD ZONE

X

DEVELOPMENT PERMIT NO. _____

PARCEL ID 20-7S-17-10027-127

SUBDIVISION _____

LOT _____

BLOCK

PHASE

UNIT

TOTAL ACRES _____

Culvert Permit No. _____

Culvert Waiver

Contractor's License Number

CGC061581

Applicant/Owner/Contractor

EXISTING _____

06-0870-N

BK

JH

N

Driveway Connection _____

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

MH TO BE REMOVED TIN 45 DAYSOFCO ISSUANCE

Check # or Cash 18465

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____

date/app. by _____

Foundation

date/app. by _____

Monolithic

date/app. by _____

Under slab rough-in plumbing _____

date/app. by _____

Slab

date/app. by _____

Sheathing/Nailing

date/app. by _____

Framing _____

date/app. by _____

Rough-in plumbing above slab and below wood floor

date/app. by _____

8

7

6

5

4

400650-1112 MS-2 STRUT

37 407805-0004 HINGE ASSY
 405742-0001 BRKT. 1/2 CHAPT
 391156-0002 ROLLER ASSY
 37 400624-0001 PUSH NUT

409808-0001 TOP FITZ
 341156-0002 ROLLER ASSY
 400624-0001 PUSH NUT

37 407805-0004 HINGE ASSY
 405742-0001 BRKT. 1/2 CHAPT
 391156-0002 ROLLER ASSY
 37 400624-0001 PUSH NUT

DETAIL B
 SCALE 1/2" ON C.D.
 TYPICAL BOTH SIDES

50 TYP
 25 TYP

37 407805-0004 HINGE ASSY
 405742-0001 BRKT. 1/2 CHAPT
 391156-0002 ROLLER ASSY
 37 400624-0001 PUSH NUT

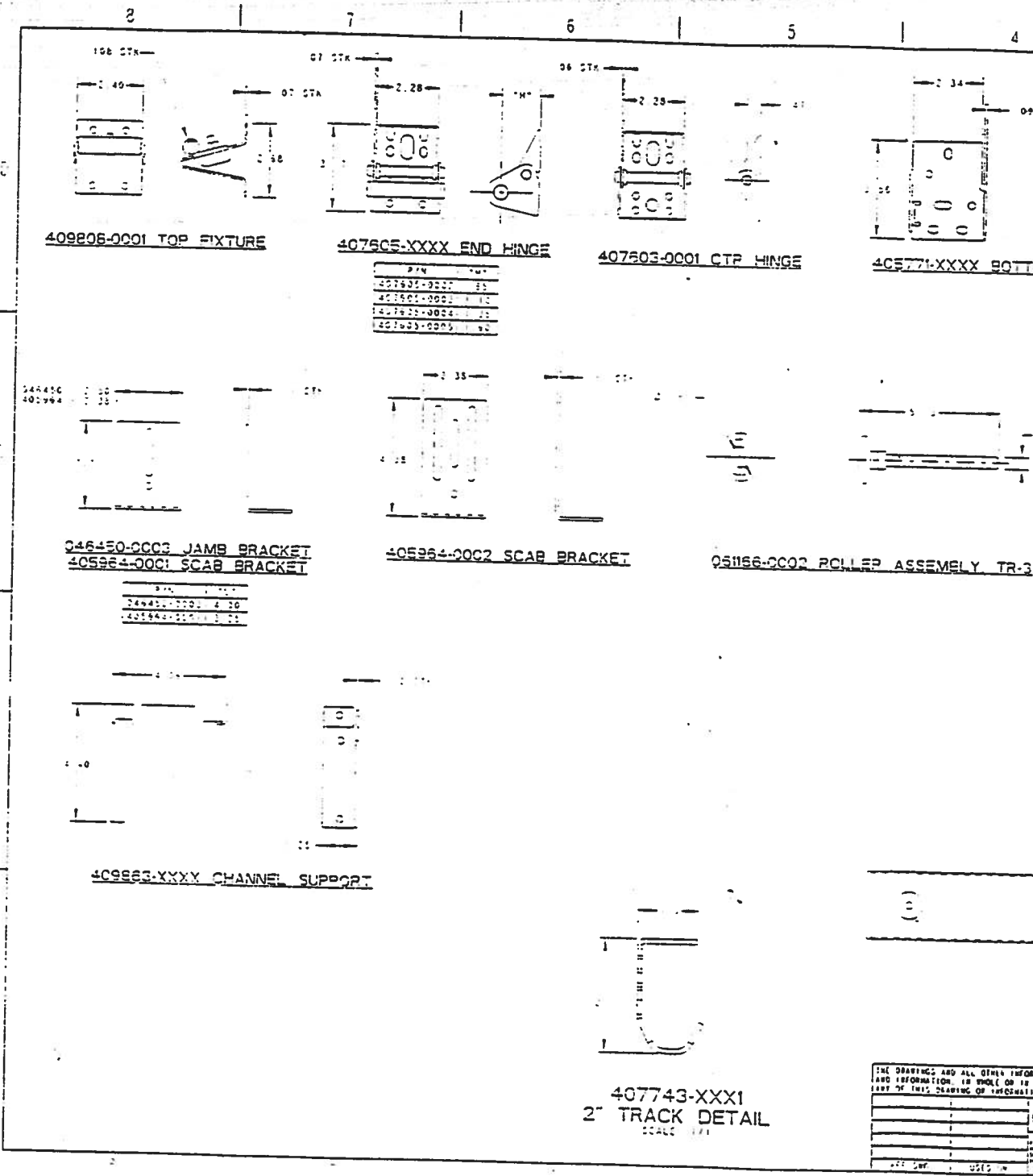
400650-1112 MS-2 STRUT

400624-0001
PUSH NUT

409863-0002 BRACKET (SHOWN)
 409863-0001 BRACKET (GPP)

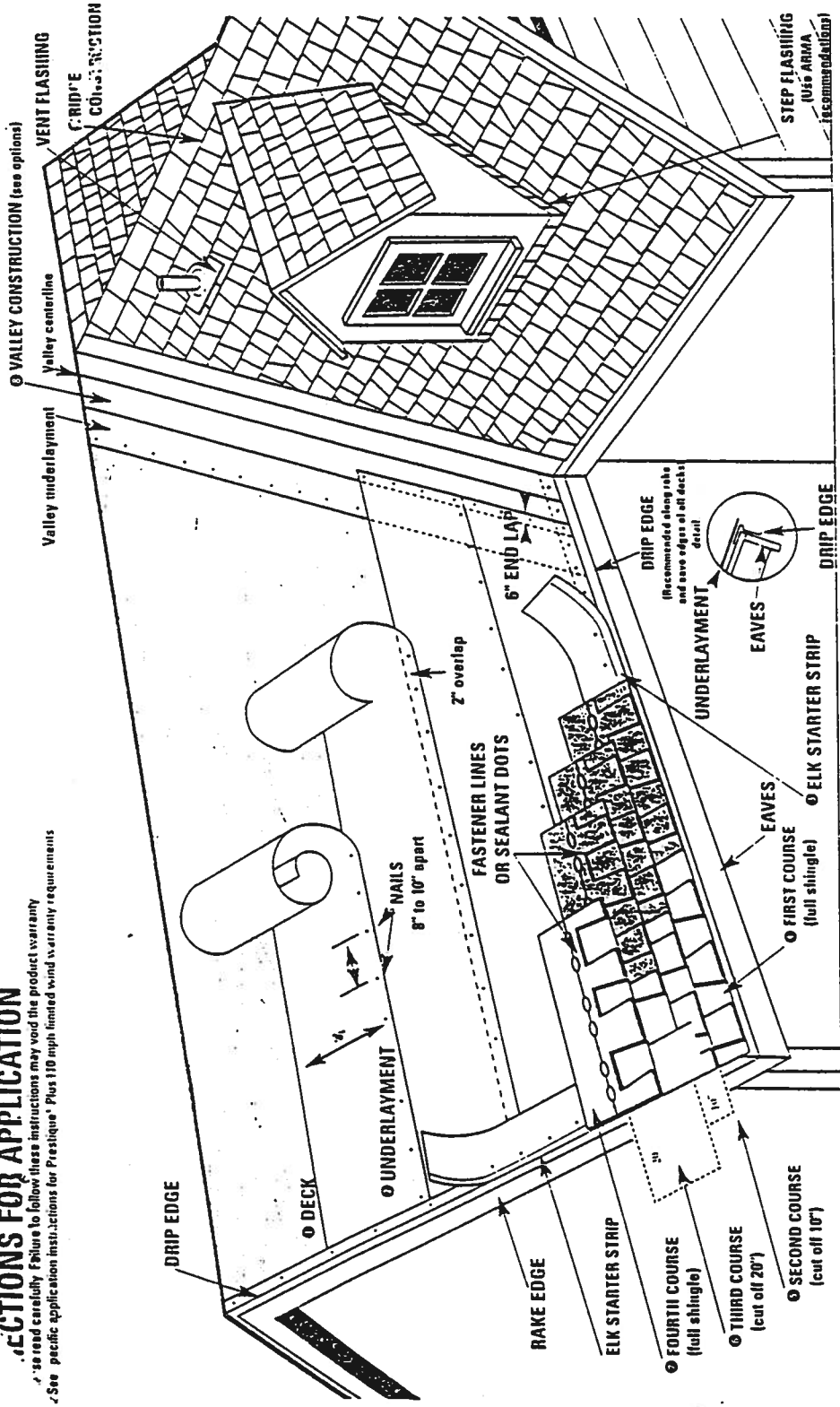
VIEW A-A
 (TRACK ASSY REMOVED FOR CLARITY)
 SCALE 1/2" ON A.D.

THE DRAWINGS AND ALL DIMENSIONS
 AND INFORMATION IS MADE UP OF
 SET OF THIS DRAWING TO INFORMATION
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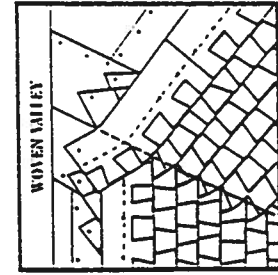


DIRECTIONS FOR APPLICATION

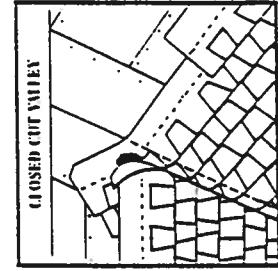
See specific application instructions for Prestique® Plus 110 mph limited wind warranty requirements



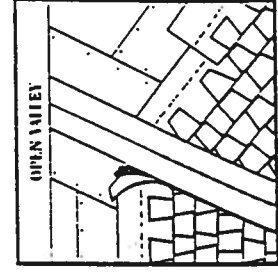
VALLEY CONSTRUCTION OPTION (California Open and California Closed are also acceptable) NOTE: For complete ARMA valley installation details, see ARMA Residential Asphalt Roofing Manual.



WOVEN VALLEY



CLOSED CUT VALLEY



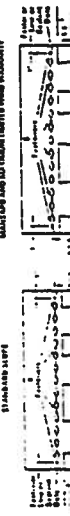
OPEN VALLEY

DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these

FOURTH COURSE

Start at the rake and continue with full shingles across roof FIFTI AND SUCCEEDING COURSES.



Columbia County Building Permit Application

Revised 9-23-04

10/31/06 Lett message

For Office Use Only Application # 0609-91 Date Received 9/29 By JW Permit # 25126

Application Approved by - Zoning Official BZK Date 06-10-06 Plans Examiner 10-13-06 Date 5/14

Flood Zone R Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments *HAS EXISTING WELL 386-454-4244 of CO being Issued.

Applicants Name DAVID WATSON Dennis O'Neil O'Neil Cousin P.O. Box 1633 High Springs FL 32643 Phone 454-2476

Address 103 SW Buck Court Ft. White FL

Owners Name DAVID WATSON Phone _____

911 Address 103 SW Buck Court Ft. White

Contractors Name ONEIL CONSTRUCTION Phone 386 454 2476

Address P.O. Box 1633 High Springs, FL

Fee Simple Owner Name & Address DAVID WATSON 103 SW Buck Court, Ft. White

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Steve Ryall 16194 212th terr. High Springs

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 20-75-17-10027-127 Estimated Cost of Construction 250,000

Subdivision Name _____ Lot 27 Block _____ Unit _____ Phase _____

Driving Directions From High Springs go N on Hwy 27 pass caution light
(138) T-L on Benjamin Rd to END, T-L on Buck Court
to END of Road - Dead End IN Property. 5th lot on right

Type of Construction Single Family RES. FRAME-Brick Number of Existing Dwellings on Property 1

Total Acreage 10 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 300? Side ? Side 1140? Rear 35?

Total Building Height 25' 8" Number of Stories 2 Heated Floor Area 2290 Roof Pitch 8/12
TOTAL 4167

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

ONEIL CONSTRUCTION

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

29 day of Sept. 2006.

Personally known ☒ or Produced Identification _____

Contractor Signature Don O'Neil
Contractors License Number CGC061581
Competency Card Number _____

NOTARY STAMP/SEAL

BONNIE P. PRESNELL
Notary Public, State of Florida
My comm. exp. Mar. 1, 2008
Comm. No. 0527222
Notary Signature B. Presnell



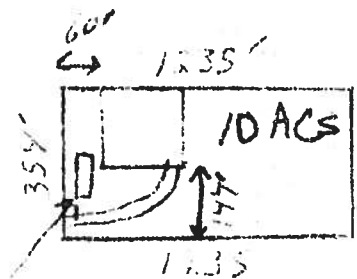
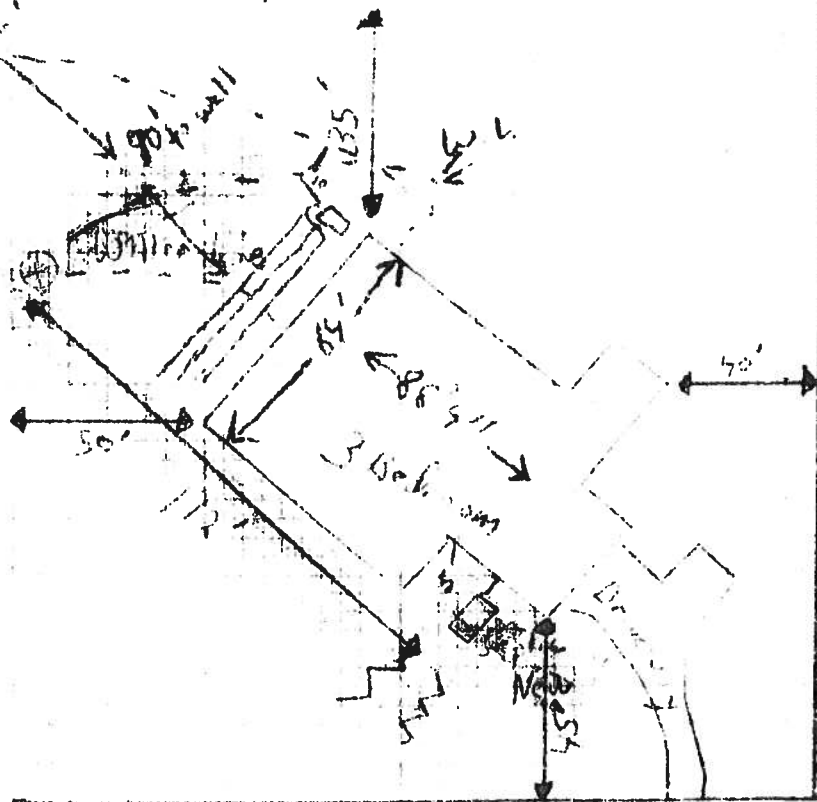
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0870N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: 1 of 10

Site Plan submitted by: [Signature]

Signature

[Signature]

Title

Plan Approved ☒

Not Approved ☐

Date 10/2/06

By [Signature]

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
CONSTRUCTION PERMIT

FEE PAID : 7332 -
RECEIPT : 1060827004
OSTDSNBR : 06-0870- -N

CONSTRUCTION PERMIT FOR:

[X] New System [] Existing System [] Holding Tank [] Innovative Other
[] Repair [] Abandonment [] Temporary [NA]

APPLICANT: WATSON, DAVID/L-27/COX SURVE AGENT: 96-0443, Beltz Sr Billy

PROPERTY STREET ADDRESS: 103 SW BUCK CRT High Springs FL 32038

LOT: 27 BLOCK: _____ SUBDIVISION: Cox Survey

[Section/Township/Range/Parcel No.]

PROPERTY ID #: 20-7S-17-10027-127

[OR TAX ID NUMBER]

SYSTEM MUST BE CONSTRUCTED IN ACCORDANCE WITH SPECIFICATIONS AND STANDARDS OF CHAPTER 64E-6, FAC DEPARTMENT APPROVAL OF SYSTEM DOES NOT GUARANTEE SATISFACTORY PERFORMANCE FOR ANY SPECIFIC TIME PERIOD. ANY CHANGE IN MATERIAL FACTS WHICH SERVED AS A BASIS FOR ISSUANCE OF THIS PERMIT, REQUIRE THE APPLICANT TO MODIFY THE PERMIT APPLICATION. SUCH MODIFICATIONS MAY RESULT IN THIS PERMIT BEING MADE NULL AND VOID. ISSUANCE OF THIS PERMIT DOES NOT EXEMPT THE APPLICANT FROM COMPLIANCE WITH OTHER FEDERAL, STATE OR LOCAL PERMITTING REQUIRED FOR PROPERTY DEVELOPMENT.

SYSTEM DESIGN AND SPECIFICATIONS

T [1050] Gallons SEPTIC TANK

A [0] Gallons

N [0] GALLONS GREASE INTERCEPTOR CAPACITY

K [0] GALLONS DOSING TANK CAPACITY [0] GALLONS @ [0] DOSES PER 24 HRS # PUMPS [0]

MULTI-CHAMBERED/IN SERIES: [Y]

MULTI-CHAMBERED/IN SERIES: [Y]

D [444] SQUARE FEET PRIMARY DRAINFIELD SYSTEM

R [0] SQUARE FEET

A TYPE SYSTEM: [Y] STANDARD [N] FILLED [N] MOUND [N]

CONFIGURATION: [Y] TRENCH [N] BED [N]

F LOCATION TO BENCHMARK: Nail In Pumphouse W of Site

I ELEVATION OF PROPOSED SYSTEM SITE [12.0] [INCHES] [BELOW] BENCHMARK/REFERENCE POINT

E BOTTOM OF DRAINFIELD TO BE [42.0] [INCHES] [BELOW] BENCHMARK/REFERENCE POINT

L D FILL REQUIRED: [0.0] INCHES EXCAVATION REQUIRED: [0.0] INCHES

OTHER REMARKS:

The licensed contractor installing the system is responsible for installing the minimum category of tank in accordance with s. 64E-6.013(3)(f), FAC.

Run system in 3 to 4 lines parallel to the side of the house. Tank should be at back corner of home with lines running to the front.

SPECIFICATIONS BY: Lander, Mark

APPROVED BY: Lander, Mark

DATE ISSUED: 10/2/06

TITLE: ES Specialist III

TITLE: EH Specialist III

Columbia

CHD

EXPIRATION DATE: 4/2/08

7H 4016, 03/97 (Obsoletes previous editions which may not be used)
Stock Number: 5744-001-4016-0

Columbia County Property Appraiser

DB Last Updated: 8/1/2006

2006 Proposed Values

Parcel: 20-7S-17-10027-127

Tax Record

Property Card

Interactive GIS Map

Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	WATSON DAVID F JR & MELISSA H
Site Address	BUCK
Mailing Address	24710 NW 94TH AVE ALACHUA, FL 32615
Description	COMM NW COR OF SW1/4 OF NW1/4, RUN E 87.45 FT, S 370.28 FT TO POB, CONT S 354.70 FT, E 1235.48 FT, N 354.70 FT, W 1235.48 FT TO POB. AKA LOT 27 COX SURVEY OFF US-27. ORB 647-323, 670-59, 669-159, 851-352, QCD 1019-51, WD 1019-1012.

Use Desc. (code)	IMPROVED A (005000)
Neighborhood	20717.01
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	10.060 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (2)	\$8,400.00
Ag Land Value	cnt: (1)	\$1,630.00
Building Value	cnt: (1)	\$25,023.00
XFOB Value	cnt: (7)	\$2,200.00
Total Appraised Value		\$37,253.00

Just Value	\$93,607.00
Class Value	\$37,253.00
Assessed Value	\$37,253.00
Exempt Value	\$0.00
Total Taxable Value	\$37,253.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
6/25/2004	1019/1012	WD	I	Q		\$75,000.00
10/17/1988	669/159	AD	V	U		\$28,000.00
3/12/1988	647/323	AD	V	U		\$28,000.00

Building Characteristics

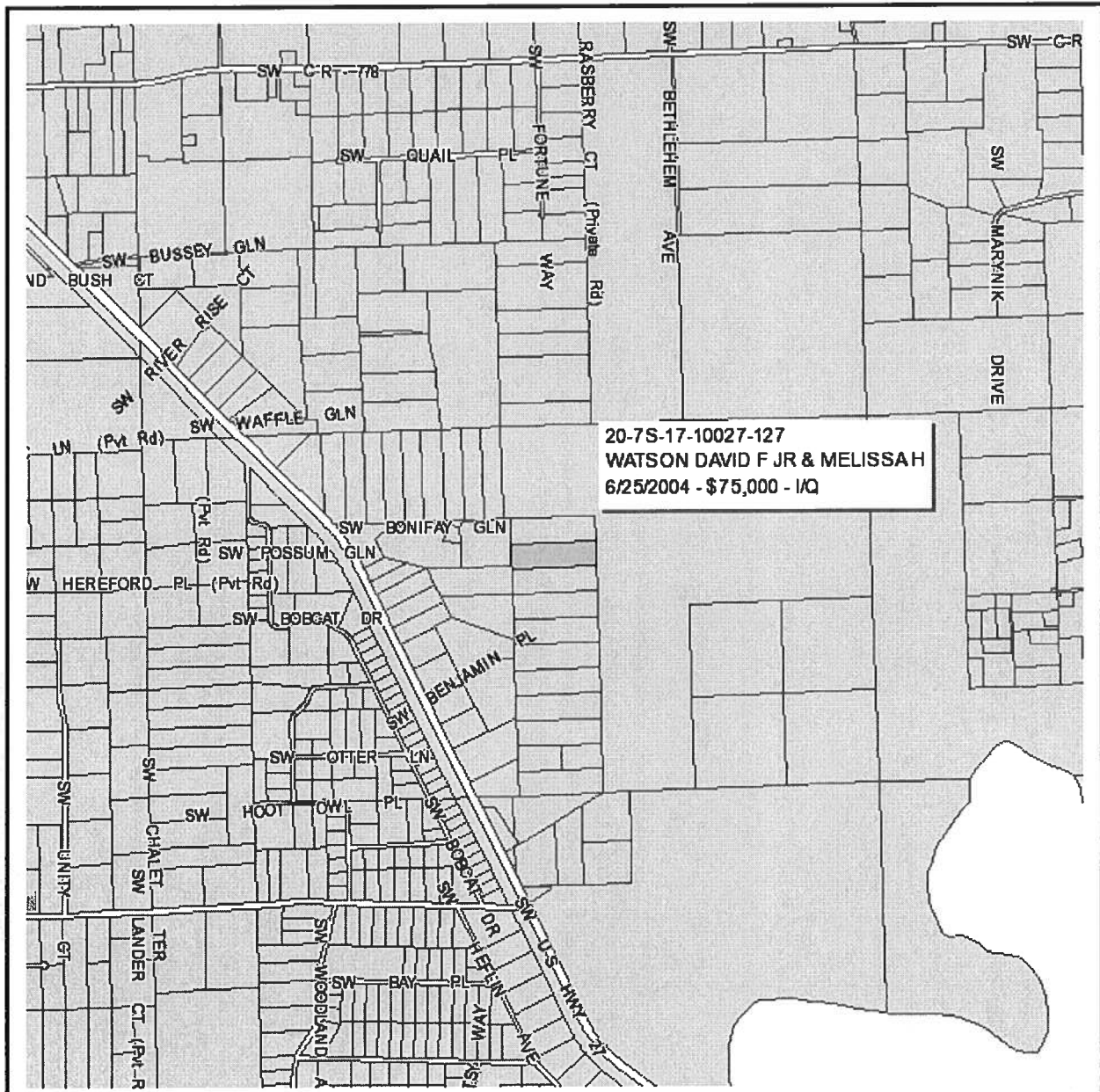
Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	MOBILE HME (000800)	1989	Average (05)	1296	1400	\$25,023.00
Note: All S.F. calculations are based on exterior building dimensions.						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0166	CONC,PAVMT	1993	\$500.00	1.000	0 x 0 x 0	(.00)
0294	SHED WOOD/	2003	\$600.00	80.000	8 x 10 x 0	(.00)
0252	LEAN-TO W/	2003	\$100.00	50.000	5 x 10 x 0	(.00)
0252	LEAN-TO W/	2003	\$100.00	50.000	5 x 10 x 0	(.00)
0252	LEAN-TO W/	2003	\$100.00	50.000	5 x 10 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000102	SFR/MH (MKT)	1.000 AC	1.00/1.00/1.00/1.00	\$6,400.00	\$6,400.00
009910	MKT.VAL.AG (MKT)	9.060 AC	1.00/1.00/1.00/1.00	\$0.00	\$57,984.00
006200	PASTURE 3 (AG)	9.060 AC	1.00/1.00/1.00/1.00	\$180.00	\$1,630.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	\$2,000.00	\$2,000.00



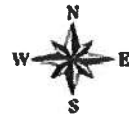
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 20-7S-17-10027-127 - IMPROVED A (005000)

Name:	WATSON DAVID F JR & MELISSA H	LandVal	\$8,400.00
Site:	BUCK	BldgVal	\$25,023.00
Mail:	24710 NW 94TH AVE	ApprVal	\$37,253.00
	ALACHUA, FL 32615	JustVal	\$93,607.00
Sales	6/25/2004 \$75,000.00 I / Q	Assd	\$37,253.00
Info	10/17/1988 \$28,000.00 V / U	Exmpt	\$0.00
	3/12/1988 \$28,000.00 V / U	Taxable	\$37,253.00

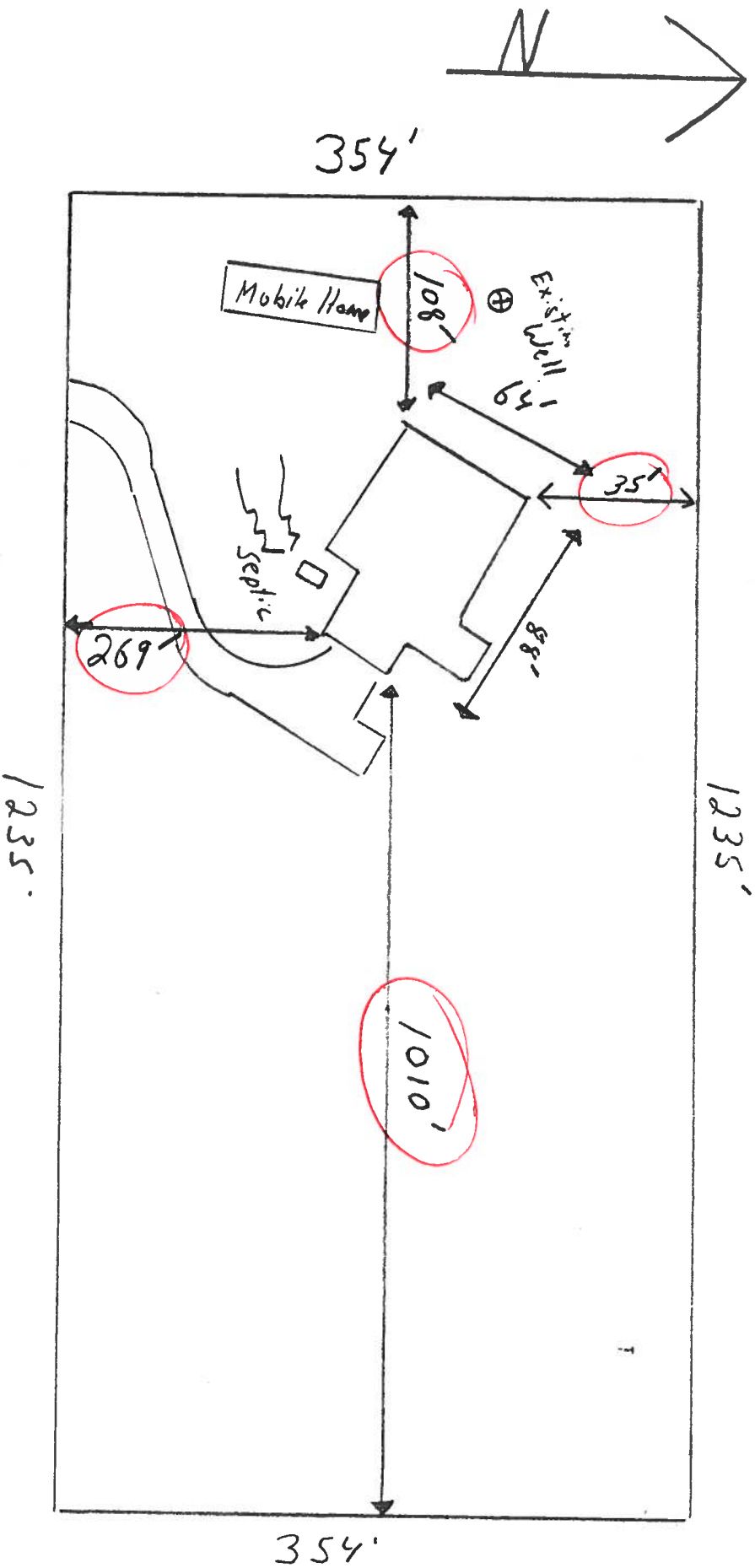
0 0.1 0.2 0.3 mi



This information, GIS Map Updated: 9/1/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, its use, or its interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Parcel: 20-95-17-10027-127
Owner DAVID & MELISSA WATSON
911 address 103 SW Buck Court
Ft. White

Site Plan



Note: Mobile Home to be removed
After House is complete



20

ZONE X

20

27

0609-91

29

ZONE X

ZONE AE

SANTA FE RIVER

CSX

ZONE AE

RM17

ZONE X

COUNTY BOUNDARY

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FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name:	WATSON RESIDENCE	Builder:	O'NEIL CONSTRUCTION
Address:		Permitting Office:	COLUMBIA COUNTY
City, State:	,	Permit Number:	25126
Owner:	DAVID AND MELISSA WATSON	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 60.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No	c. N/A	
6. Conditioned floor area (ft²)	2401 ft²	13. Heating systems	
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		a. PTHP	Cap: 60.0 kBtu/hr
a. U-factor:	Description Area		COP: 3.50
(or Single or Double DEFAULT)	7a. (Dble Default) 188.0 ft²	b. N/A	
b. SHGC:		c. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 281.5 ft²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 50.0 gallons
a. Slab-On-Grade Edge Insulation	R=0.0, 213.5(p) ft		EF: 0.93
b. Raised Wood, Adjacent	R=0.0, 24.0ft²	b. N/A	
c. N/A		c. Conservation credits	
9. Wall types		(HR-Heat recovery, Solar	
a. Frame, Wood, Exterior	R=13.2, 1832.3 ft²	DHP-Dedicated heat pump)	
b. Frame, Wood, Adjacent	R=12.9, 433.5 ft²	15. HVAC credits	CF,
c. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
d. N/A		HF-Whole house fan,	
e. N/A		PT-Programmable Thermostat,	
10. Ceiling types		MZ-C-Multizone cooling,	
a. Under Attic	R=30.0, 2400.9 ft²	MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 70.0 ft		
b. N/A			

Glass/Floor Area: 0.12

Total as-built points: 24145

Total base points: 34143

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Larry Resmondo a/c

DATE: Sept 21, 2006

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: O'Neil Construction

DATE: 9/29/06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	2401.0	20.04	8660.9	Double, Clear	E	8.5	5.5	36.0	42.06	0.43	646.4
				Double, Clear	W	9.0	5.5	18.0	38.52	0.43	299.8
				Double, Clear	S	1.5	5.5	22.5	35.87	0.83	673.6
				Single, Clear	W	9.0	8.0	45.5	43.84	0.50	995.8
				Single, Clear	E	8.5	4.0	6.0	47.92	0.39	111.0
				Double, Clear	E	14.0	3.5	6.3	42.06	0.36	94.6
				Double, Clear	W	1.5	5.5	47.3	38.52	0.90	1638.2
				Double, Clear	S	1.5	3.5	5.0	35.87	0.70	126.0
				Single, Clear	W	1.5	8.0	21.0	43.84	0.96	880.4
				Double, Clear	E	8.5	8.0	7.0	42.06	0.50	148.1
				Double, Clear	N	1.5	3.0	3.8	19.20	0.83	60.6
				Single, Clear	S	1.5	8.0	21.0	40.81	0.92	788.9
				Double, Clear	E	1.5	5.5	9.0	42.06	0.90	340.1
				Double, Clear	S	9.0	5.5	18.0	35.87	0.46	299.6
				Double, Clear	W	1.5	3.5	3.8	38.52	0.78	113.8
				Double, Clear	E	1.5	5.5	11.3	42.06	0.90	427.0
				As-Built Total:				281.5		7644.0	
WALL TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	433.5	0.70	303.4	Frame, Wood, Exterior	13.2		1832.3	1.48		2711.8	
Exterior	1832.3	1.70	3114.9	Frame, Wood, Adjacent	12.9		433.5	0.61		262.3	
Base Total:		2265.8	3418.4	As-Built Total:		2265.8		2974.1			
DOOR TYPES Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	21.0	2.40	50.4	Adjacent Wood			21.0	2.40		50.4	
Exterior	0.0	0.00	0.0								
Base Total:		21.0	50.4	As-Built Total:		21.0		50.4			
CEILING TYPES Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	2400.9	1.73	4153.6	Under Attic	30.0		2400.9	1.73 X 1.00		4153.6	
Base Total:		2400.9	4153.6	As-Built Total:		2400.9		4153.6			
FLOOR TYPES Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	213.5(p)	-37.0	-7899.5	Slab-On-Grade Edge Insulation	0.0		213.5(p)	-41.20		-8796.2	
Raised	24.0	-3.99	-95.8	Raised Wood, Adjacent	0.0		24.0	2.20		52.8	
Base Total:		-7995.3		As-Built Total:		237.5		-8743.4			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT			
INFILTRATION Area X BSPM = Points				Area X SPM = Points			
2401.0 10.21 24514.2				2401.0 10.21 24514.2			
Summer Base Points: 32802.2				Summer As-Built Points: 30592.8			
Total Summer X System = Cooling Points Multiplier Points				Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)			
32802.2 0.4266 13993.4				(sys 1: Central Unit 60000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS) 30593 1.00 (1.09 x 1.147 x 0.91) 0.263 0.950 8681.0 30592.8 1.00 1.138 0.263 0.950 8681.0			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2401.0	12.74	5506.0	Double, Clear	E	8.5	5.5	36.0	18.79	1.40	946.1
				Double, Clear	W	9.0	5.5	18.0	20.73	1.21	452.1
				Double, Clear	S	1.5	5.5	22.5	13.30	1.14	341.7
				Single, Clear	W	9.0	8.0	45.5	28.84	1.18	1549.6
				Single, Clear	E	8.5	4.0	6.0	26.41	1.46	231.3
				Double, Clear	E	14.0	3.5	6.3	18.79	1.51	178.4
				Double, Clear	W	1.5	5.5	47.3	20.73	1.03	1007.3
				Double, Clear	S	1.5	3.5	5.0	13.30	1.47	97.7
				Single, Clear	W	1.5	8.0	21.0	28.84	1.01	612.7
				Double, Clear	E	8.5	8.0	7.0	18.79	1.30	171.3
				Double, Clear	N	1.5	3.0	3.8	24.58	1.01	94.3
				Single, Clear	S	1.5	8.0	21.0	20.24	1.04	443.8
				Double, Clear	E	1.5	5.5	9.0	18.79	1.04	176.0
				Double, Clear	S	9.0	5.5	18.0	13.30	3.38	809.2
				Double, Clear	W	1.5	3.5	3.8	20.73	1.07	84.0
				Double, Clear	E	1.5	5.5	11.3	18.79	1.04	221.0
				As-Built Total:				281.5			7416.5
WALL TYPES				Area X BWPM = Points		Type		R-Value	Area X WPM = Points		
Adjacent		433.5	3.60	1560.6	Frame, Wood, Exterior		13.2	1832.3	3.36	6156.5	
Exterior		1832.3	3.70	6779.5	Frame, Wood, Adjacent		12.9	433.5	3.32	1437.1	
Base Total:		2265.8	8340.1		As-Built Total:		2265.8		7593.6		
DOOR TYPES				Area X BWPM = Points		Type		Area X WPM = Points			
Adjacent		21.0	11.50	241.5	Adjacent Wood		21.0		11.50	241.5	
Exterior		0.0	0.00	0.0							
Base Total:		21.0	241.5		As-Built Total:		21.0		241.5		
CEILING TYPES				Area X BWPM = Points		Type		R-Value	Area X WPM X WCM = Points		
Under Attic		2400.9	2.05	4921.8	Under Attic		30.0	2400.9	2.05 X 1.00	4921.8	
Base Total:		2400.9	4921.8		As-Built Total:		2400.9		4921.8		
FLOOR TYPES				Area X BWPM = Points		Type		R-Value	Area X WPM = Points		
Slab		213.5(p)	8.9	1900.1	Slab-On-Grade Edge Insulation		0.0	213.5(p)	18.80	4013.8	
Raised		24.0	0.96	23.0	Raised Wood, Adjacent		0.0	24.0	10.40	249.6	
Base Total:		1923.2		As-Built Total:		237.5		4263.4			

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:
----------------	-----------

BASE				AS-BUILT			
INFILTRATION Area X BWPM = Points				Area X WPM = Points			
2401.0 -0.59 -1416.6				2401.0 -0.59 -1416.6			
Winter Base Points: 19516.0				Winter As-Built Points: 23020.2			
Total Winter X System = Heating Points Multiplier Points				Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)			
19516.0 0.6274 12244.4				(sys 1: PTHP 60000 btuh ,EFF(3.5) Ducts:Unc(S),Unc(R),Int(AH),R6.0 23020.2 1.000 (1.069 x 1.169 x 0.93) 0.286 1.000 7643.9 23020.2 1.00 1.162 0.286 1.000 7643.9			

WATER HEATING & CODE COMPLIANCE STATUS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING									
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit = Total Multiplier
3		2635.00	7905.0	50.0	0.93	3		1.00	2606.67
				As-Built Total:					7820.0

CODE COMPLIANCE STATUS									
BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	= Total Points
13993		12244		7905	34143	8681		7644	24145

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.4

The higher the score, the more efficient the home.

DAVID AND MELISSA WATSON, , , ,

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 60.0 kBtu/hr
3. Number of units, if multi-family	1	___		SEER: 13.00
4. Number of Bedrooms	3	___	b. N/A	___
5. Is this a worst case?	No	___	c. N/A	___
6. Conditioned floor area (ft ²)	2401 ft ²	___		___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems	
a. U-factor:	Description Area	___	a. PTHP	Cap: 60.0 kBtu/hr
(or Single or Double DEFAULT) 7a. (Dble Default)	188.0 ft ²	___		COP: 3.50
b. SHGC:		___	b. N/A	___
(or Clear or Tint DEFAULT) 7b. (Clear)	281.5 ft ²	___	c. N/A	___
8. Floor types		___	14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 213.5(p) ft	___	a. Electric Resistance	Cap: 50.0 gallons
b. Raised Wood, Adjacent	R=0.0, 24.0ft ²	___		EF: 0.93
c. N/A	___	___	b. N/A	___
9. Wall types		___	c. Conservation credits	___
a. Frame, Wood, Exterior	R=13.2, 1832.3 ft ²	___	(HR-Heat recovery, Solar	___
b. Frame, Wood, Adjacent	R=12.9, 433.5 ft ²	___	DHP-Dedicated heat pump)	___
c. N/A	___	___	15. HVAC credits	CF, ___
d. N/A	___	___	(CF-Ceiling fan, CV-Cross ventilation,	___
e. N/A	___	___	HF-Whole house fan,	___
10. Ceiling types		___	PT-Programmable Thermostat,	___
a. Under Attic	R=30.0, 2400.9 ft ²	___	MZ-C-Multizone cooling,	___
b. N/A	___	___	MZ-H-Multizone heating)	___
c. N/A	___	___		___
11. Ducts		___		___
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 70.0 ft	___		___
b. N/A	___	___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



***NOTE:** The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.0)

WINDOWS

ATTACHMENTS

MASTER COPY

TYPE	MANUFACTURER	SERIES	SIZE	JAMB TYPE/SPACING	HEADER TYPE/SPACING	SILLS TYPE/SPACING	MULLIONS
SH	Capitol	650	4670	8 BY 1 1/2 3" FROM EVID NEEDED FROM 24"	"	"	
PW	"	"	6020	3 - SCREWS	4 - SCREWS	4 - SCREWS	
SH Twin	"	"	3060 TWIN	4 - SCREWS	7/8" 3060 3 - SCREWS	7/8" 3060 3 - SCREWS	STRUCTURAL MULLION 4 - 10P 1 1/2 4 - 8P 1 1/2 4 - 8P 1 1/2
PW	"	"	6020	3 - SCREWS	4 - SCREWS	4 - SCREWS	
PW	"	"	4660	4 - SCREWS	3 - SCREWS	3 - SCREWS	
SH Twin	"	"	3050 Twin	4 - SCREWS	4 - SCREWS	4 - SCREWS	INTERIOR MULLION

NOTE # Caulk Behind the Nail Fin
DOORS around the whole Perimeter

ATTACHMENT OF FRAME

TYPE	MANUFACTURER	SERIES	SIZE	JAMB	HEADER	SILL
SH	Capitol	420	UP TO 6068	410 x 1.75 INCHES 8 - SCREWS	" SAME AS JAMB	8 - #10 P 2 3/16 TAPCON USED. MIN 1 1/4" PANELS SILL SHOULD BE SILICONE IN.

WINDOWS

ATTACHMENTS

TYPE	MANUFACTURER	SERIES	SIZE	JAMB TYPESPACING	HEADER TYPESPACING	SILLS TYPESPACING	MULLIONS
SH	CAPIPOZ	650	3/0 5/0	4	3	3	N/A
SH	"	"	4/0 5/0	4	4	4	N/A
SH	"	"	3/0 2/0	3	3	3	N/A
SH	"	"	1/6 5/0	4	3	3	N/A
SH	"	"	4/0 3/0	3	4	4	N/A
SH/CT	"	"	4/0 5/0	4	5	4	N/A

DOORS

ATTACHMENT OF FRAME

TYPE	MANUFACTURER	SERIES	SIZE	JAMB	HEADER	SILL
OUTSWING	MASONITE	Tortenson	6068	5	6	6
OUTSWING	"	"	3068	5	3	3
OUTSWING	"	"	2868	5	3	3

ALACHUA COUNTY OFFICE OF CODES ENFORCEMENT

ROOFING, WINDOW & DOOR REQUIREMENTS

ROOF SYSTEM DESCRIPTION

The diagram illustrates a cross-section of a roof system. It shows a sloped roof with a horizontal line indicating the ridge. A vertical line on the left indicates the roof slope, with a horizontal dimension of 12" shown. A vertical line on the right indicates the mean height. Various components are labeled with arrows pointing to them:

- DECK TYPE: 1/2" 4 Ply Plywood
(OSB etc, Nail pattern & fastener size)
- UNDERLAYMENT: 30 LB Felt
(Felt weight)
- FLASHING: 26 GA Valley & L Flashings
- CAP SHEET: NONE
(Flat roof only)
- ROOF COVERING: EIK RP 30
(Mfg & Nailing) 4 Nails per shingle
- DRIP EDGE MATERIAL: Aluminum
- EAVE VENTILATION: Aluminum Soffit
- RIDGE VENTILATION: Off Ridge
- 12"
- ROOF SLOPE
- MEANHEIGHT

WINDLOAD INFO

1. Basic wind speed 100 mph X 110 mph
2. Wind importance factor _____
3. Wind exposure _____
4. Internal pressure coefficient _____
5. Components & cladding (design pressure) _____

LBC

Warmth. Style. And efficiency. These are just a few of the attributes that Superior's LBC heat-circulating fireplace delivers in a big way.

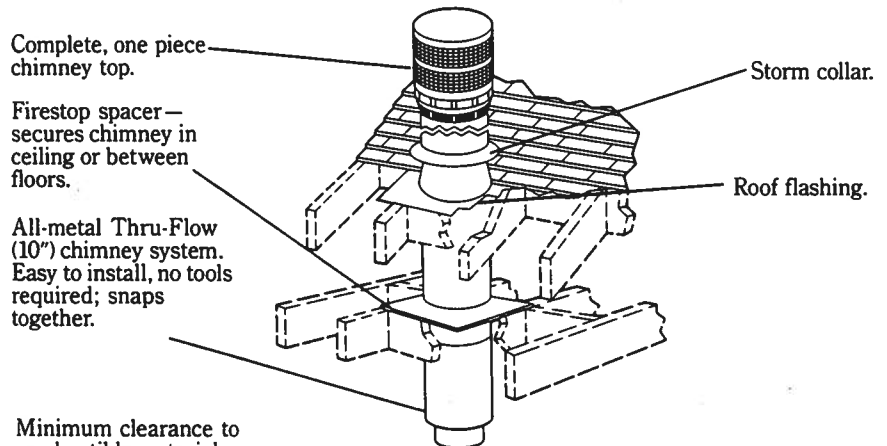
Available in two sizes — 38" and 43" screen opening widths — the LBC offers an extremely generous firebox that's 24" high and 20" deep. Of course, due to the LBC's recessed screen pockets, the appearance of the firebox opening is even larger.

Aesthetically, the LBC offers typically Superior flair. It features a realistic masonry-look refractory, an integrated ash lip and decorative flexibility with your choice of either standard black or optional polished brass grilles.

The LBC's heat circulating effectiveness can be increased with the following options: an outside combustion air kit, fan kit and stylish ALL-GLASS™ brass trim bi-fold doors.

Plus, with Superior's zero clearance design and proven 10" chimney system, installation is quick and easy.

The Superior LBC fireplace is definitely a lot of circulation for the money.



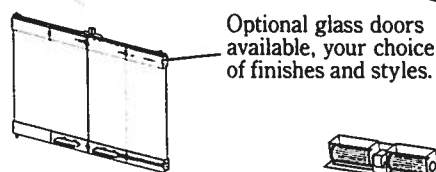
Minimum clearance to combustible materials. Frame with wood right up to fireplace.

Optional outside air kit.

Realistic refractory interior with hidden screen pockets.

Easy gas line access with accessible knockout.

Standard metal safety strip.



Decorator room air inlet grilles.

Decorator warm air outlet grilles.

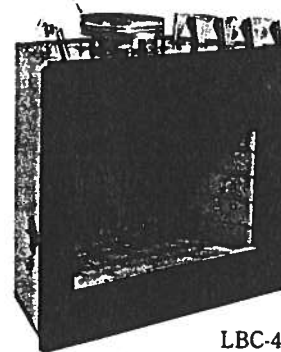
Energy saving damper, with positive seal.

Construction anchors secure fireplace to floor.

Standard fuel grate.

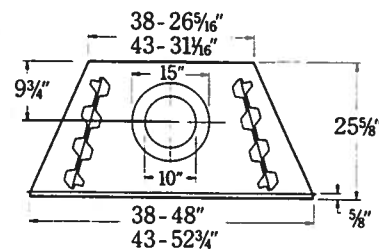
Hearth extension — secure to platform or floor. No special foundation required.

Optional fan kit.

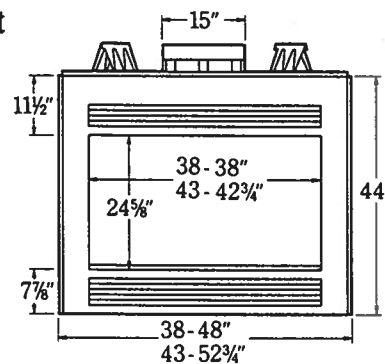


LBC-4324

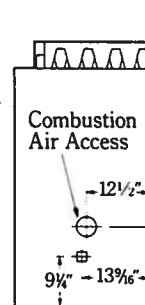
Top View



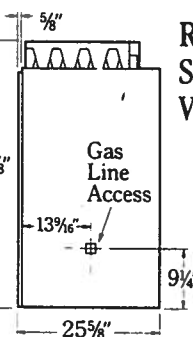
Front View



Left Side View



Right Side View



Framing Dimensions

Model	Width	Height	Depth
LBC-3824	48"	49 $\frac{5}{8}$ "	25 $\frac{5}{8}$ "
LBC-4324	52 $\frac{3}{4}$ "	49 $\frac{5}{8}$ "	25 $\frac{5}{8}$ "

NOTE: Diagrams and illustrations are not to scale and should not be used for framing purposes — consult installation instructions. Product designs, materials, dimensions, specifications, colors and prices subject to change or discontinuance without notice.

Consult your distributor for local fireplace code information.

Distributed by:

SUPERIOR®
The Fireplace Company

4325 Artesia Ave., Fullerton, CA 92633, (714) 521-7302

NOTES

1. ALL THE LOAD FROM THE DOOR IS TRANSMITTED TO THE TRACK AND THRU FROM THE TRACK TO THE VERTICAL JAMBS (SPR STUD GRADE OR BETTER). NO LOAD FROM THE DOOR IS TRANSMITTED TO THE HORIZONTAL (TOP) JAMB WITH STANDARD WINDOW STYLE WITH VERTICAL WINDOW POST. PART OF THE FORCE IS TRANSMITTED TO THE HORIZONTAL GRADE DOOR JAMB.
2. EACH VERTICAL JAMB SETS A MAXIMUM DESIGN LOAD OF +2989 LB & -2989 LB AND A MAXIMUM TEST LOAD OF +4480 LB & -4480 LB THE HORIZONTAL GABLE DOOR JAMB SETS A MAX TEST LOAD OF 1200Y FOR SINGLE POST, AND MAX COMBINED LOAD OF 2400Y FOR DOUBLE POST.
3. ALL JAMB FASTENERS MAY BE (BUT NOT REQUIRED) CONSIDERED TO PROVIDE A FLUSH FINISHING SURFACE.

WOOD FRAM BUILDINGS

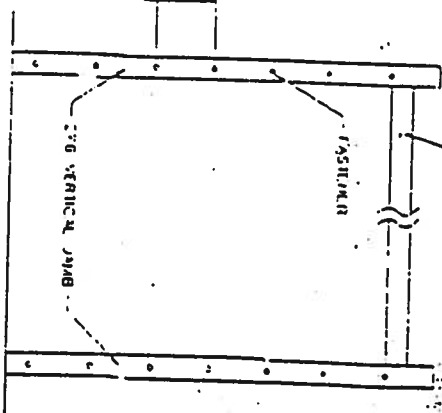
STUD WALLS OF DOOR OPENING SHALL BE FRAMED SOON BY NOT LESS THAN 2 FULL LENGTH STUDS AND 2 HORIZONTALS USING SPR STUD GRADE OR BETTER WOOD. STUD WALLS TO BE CONFINED FROM FOOTING TO THE JAMBS AND IN ACCORDANCE WITH SBC SECTION 2903.1 INSTALLATION IN ACCORDANCE WITH DMC 409783 IS AN ACCEPTABLE ALTERNATIVE.

BLOCK WALL OR CONCRETE

2x4 WEL WOOD JAMB SHALL BE ADDED TO GROUT REINFORCED BLOCK WALL OR CONCRETE CONFINED BLOCK WALL SHALL BE FILLED WITH CONCRETE AND REINFORCED WITH #5 BAR EXTENDING INTO THE FOOTING AND INTO THE CEILING. (STRENGTH IS ASSUMED TO BE 3500 PSI). ALL JAMBS SHALL BE CONTINUOUS FROM THE BEAMS TO FOOTING PER BLOCK WALL OR CONCRETE CONFINED BLOCK WALLS AND CONCRETE CONTAINS TO BE REINFORCED BY REINFORCING PROFESSIONAL OF RECORD AND IN ACCORDANCE WITH SBC SECTION 2701.2.

2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

(NOT TO BE USED FOR ATTACHMENT OF TRACK FASTENERS TO 2x6 VERTICAL JAMBS OR SUPPORTING STRUCTURE)



HORIZONTAL JAMB, 2x6 HEADRESTS (REQUIRED ONLY WITH POST SYSTEM DUE TO PARALLEL LOADS)

INSTALLING TYPE	FASTENING TYPE	MIN. NO. OF FASTENERS PER VERTICAL JAMB	MAXIMUM DISTANCE BETWEEN FASTENERS	STEEL WASHERS REQUIRED*
WOOD TRAIL (SFR)	5/16" x 3" LAG SCREW (ASTM A307) USED AT 1-3/8" ON CENTER	7	16"	YES
U-90 BLOCK (2500 LB DESIGN)	1/4" x 3" HW TAPPED CONCRETE ANCHOR 1-3/4" MIN. LENGTH	7	16"	YES
C-90 BLOCK (2500 PSI GROUP)	3/8" x 1" BOLT FOR/POST ANCHOR DOWEL 1-3/2" MIN. LENGTH	6	16"	YES
CONCRETE COLUMN (2500 PSI)	3/8" x 1" RAIL TUB/BOLT ANCHOR DOWEL 1-3/8" MIN. LENGTH	6	16"	YES

* FASTENERS/ANCHORS MUST BE AT MINIMUM 16" ON CENTER FROM EACH OTHER AND 5/16" x 1-3/8" LAG SCREWS PER U-90/BLOCK SHALL BE TORQUED TO 50 FT LBS (TORQUE) AND ANCHORS MUST BE SET IN 50 PSI MINIMUM.

APPROVED

DATE OF THIS REPORT: 8/12/98
 PROJECT: 2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT
 DRAWN BY: DAVID LAY
 CHECKED BY: G. FRIEDMAN
 DATE: 8/19/98
 SCALE: 1/4" = 1'-0"
 SHEET: 1 OF 1
 TITLE: 2x6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

NO.	REV.	DESCRIPTION	DATE
1	0	ISSUED FOR CONSTRUCTION	8/12/98
2	1	REVISED FOR 10/12/98	10/12/98



DUCT SYSTEM SUMMARY

Entire House

LARRY RESMONDO A/C

Job: DAVE AND MELISSA
WATSON 9/21/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

	HEATING	COOLING
External Static Pressure:	0.10 in H2O	0.10 in H2O
Pressure Losses:	0.25 in H2O	0.25 in H2O
Available Static Pressure:	-0.2 in H2O	-0.2 in H2O
Friction Rate:	0.100 in/100ft	0.100 in/100ft
Actual AVF:	2000 cfm	2000 cfm

Total Effective Length (TEL): 230 ft

Supply Branch Detail Table

Name	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trnk
FIRST FLOOR	4734	3765	269	266	0.100	608	9	0x 0	ShMt	st1A
FIRST FLOOR-A	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
FIRST FLOOR-B	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
FIRST FLOOR-C	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
FIRST FLOOR-D	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
FIRST FLOOR-E	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
FIRST FLOOR-F	4729	3760	268	265	0.100	607	9	0x 0	ShMt	st1
BONUS ROOM	2141	2019	121	143	0.100	533	7	0x 0	ShMt	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	2000	2000	693	23	0 x 0	ShtMetl	
st1A	Peak AVF	269	266	492	10	0 x 0	ShtMetl	st1

Return Branch Detail Table

Name	Diffus Sz (in)	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trunk
rb1	0 x 0	35248	28344	2000	2000	0.100	587	25	0x 0	ShMt	

Bold/italic values have been manually overridden



RIGHT-J BUILDING ANALYSIS REPORT

Entire House

LARRY RESMONDO A/C

Job: DAVE AND MELISSA
WATSON 9/21/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

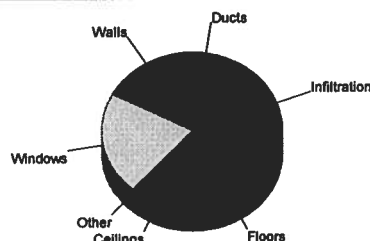
For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	92		Average
Inside db (°F)	70	75		0
Design TD (°F)	37	17		
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		
			Method	
			Construction quality	
			Fireplaces	

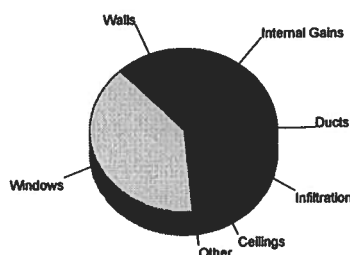
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	2.7	6117	17.4
Windows	26.5	7445	21.1
Doors	17.0	357	1.0
Ceilings	1.2	2932	8.3
Floors	27.5	6537	18.5
Infiltration	33.7	10178	28.9
Ducts		1678	4.8
Total		35244	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	1.5	3436	12.1
Windows	39.6	11141	39.3
Doors	9.5	199	0.7
Ceilings	1.1	2615	9.2
Floors	0.0	0	0.0
Infiltration	8.8	2672	9.4
Ducts		2576	9.1
Internal gains		5700	20.1
Total		28339	100.0



Cooling at 83 % SHR = 2.8 ton
Cooling at 70 % SHR = 3.3 ton

Cooling air flow = 723 cfm/ton
Cooling at 400 cfm/ton = 5.0 ton

Overall U-Value = 0.125 Btuh/ft²-°F

Data entries checked.



RIGHT-J LOAD AND EQUIPMENT SUMMARY

Entire House

LARRY RESMONDO A/C

Job: DAVE AND MELISSA
WATSON 9/21/06

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Project Information

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P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Building heat loss	35244 Btuh
Ventilation air	0 cfm
Ventilation air loss	0 Btuh
Design heat load	35244 Btuh

Sensible Cooling Equipment Load Sizing

Structure	28339 Btuh
Ventilation	0 Btuh
Design temperature swing	3.0 °F
Use mfg. data	n
Rate/swing multiplier	0.97
Total sens. equip. load	27489 Btuh

Infiltration

Method	Simplified	
Construction quality	Average	
Fireplaces	0	
	Heating	Cooling
Area (ft²)	2401	2401
Volume (ft³)	21391	21391
Air changes/hour	0.7	0.4
Equiv. AVF (cfm)	250	143

Latent Cooling Equipment Load Sizing

Internal gains	690 Btuh
Ventilation	0 Btuh
Infiltration	5009 Btuh
Total latent equip. load	5699 Btuh
Total equipment load	33188 Btuh

Heating Equipment Summary

Make	RUUD AIR COND
Trade	Ruud UPMB Series
	UPMB-060JA
Efficiency	8.0 HSPF
Heating input	0 Btuh
Heating output	0 Btuh
Heating temp rise	0 °F
Actual heating fan	2000 cfm
Heating air flow factor	0.057 cfm/Btuh
Space thermostat	

Cooling Equipment Summary

Make	RUUD AIR COND
Trade	Ruud UPMB Series
	UPMB-060JA
	UBHJ-25+RCHJ-61A1
Efficiency	13.0 SEER
Sensible cooling	37450 Btuh
Latent cooling	16050 Btuh
Total cooling	53500 Btuh
Actual cooling fan	2000 cfm
Cooling air flow factor	0.071 cfm/Btuh
Load sensible heat ratio	83 %

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RIGHT-J SHORT FORM Entire House

LARRY RESMONDO A/C

Job: DAVE AND MELISSA
WATSON 9/21/06

715 NW 1ST AVE, HIGH SPRINGS, FL 32643 Phone: 386-454-4433 Fax: 386-454-8843 Email: resmondoac@netcommander.com

Project Information

For: O'NEIL CONSTRUCTION
P.O. BOX 1633, HIGH SPRINGS, FL 32655
Phone: 386-454-2476 Fax: 386-454-4244

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	92	Method	Average
Inside db (°F)	70	75	Construction quality	0
Design TD (°F)	37	17	Fireplaces	
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-060JA

Efficiency 8.0 HSPF

Heating input 0 Btuh
Heating output 0 Btuh
Heating temperature rise 0 °F
Actual heating fan 2000 cfm
Heating air flow factor 0.057 cfm/Btuh

Space thermostat

COOLING EQUIPMENT

Make RUUD AIR COND
Trade Ruud UPMB Series
UPMB-060JA
UBHJ-25+RCHJ-61A1

Efficiency 13.0 SEER

Sensible cooling 37450 Btuh
Latent cooling 16050 Btuh
Total cooling 53500 Btuh
Actual cooling fan 2000 cfm
Cooling air flow factor 0.071 cfm/Btuh

Load sensible heat ratio 83 %

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
FIRST FLOOR	2184	33103	26320	1878	1857
BONUS ROOM	217	2141	2019	122	143
Entire House	2401	35244	28339	2000	2000
Ventilation air		0	0		
Equip. @ 0.97 RSM			27489		
Latent cooling			5699		
TOTALS	2401	35244	33188	2000	2000

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.



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RESIDENTIAL WIND DESIGN & ANALYSIS

FBC SECTION 1609 \ ***NO COPIES ARE TO BE PERMITTED***

PREPARED FOR:

ONEIL CONSTRUCTION \ THE WATSON RESIDENCE

PREPARED BY:

**MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329**

SUMMARY

OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/131 GA.
Interior zone spacing: Interior 8 in. Periphery 4 in.
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.
89' — Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.
92' — Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 322 pounds per linear foot

Wall tension transferred by: Siding nails 8d/131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 7 in.
Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. #5 at 96 in.
Monolithic footing: Depth 20 in. Bottom width 12 in.

Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

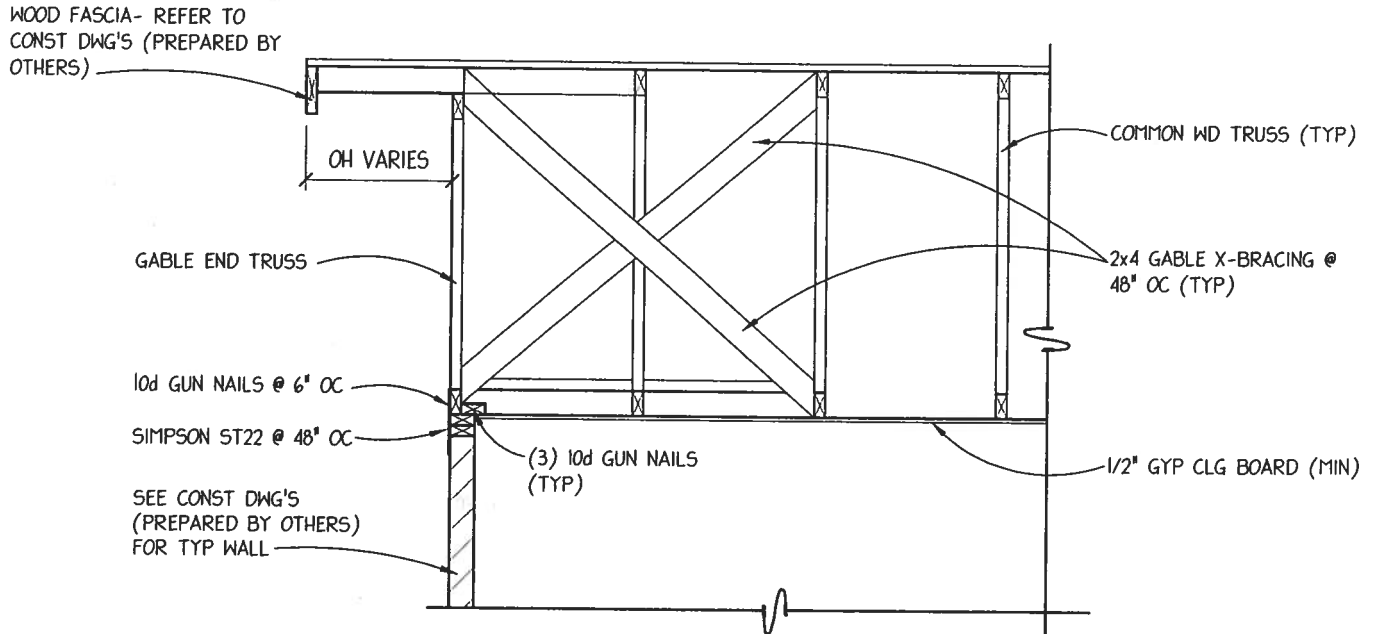
Porch Columns: 4x4x9'5" #2 PT @ 12" O.C.

Porch Column Fasteners: Single C1344/CC44 OR EQUAL

NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

W. J. Watson
AMU 139857
9/27/16



THIS WIND ANALYSIS ASSUMES IDEAL SOIL
CONDITIONS W/MIN 2500 PSI SOIL BRG
CAPACITY & 95% DRY PROCTOR DENSITIES

GABLE END DETAIL

SCALE: 1/2" = 1'-0"

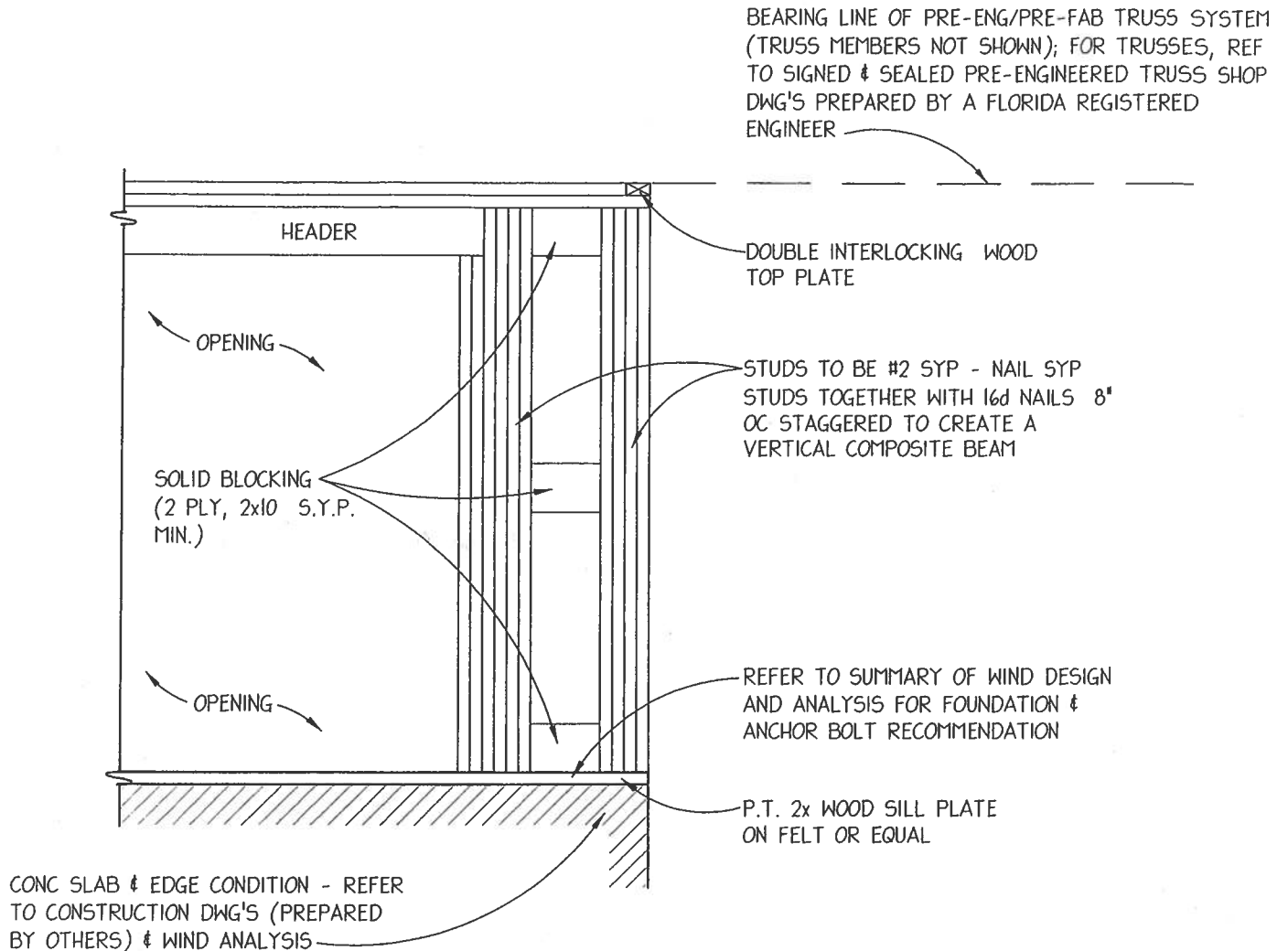
NOTE:

INSTALL AND ERECT ALL TRUSS MEMBERS IN STRICT CONFORMANCE WITH
THE PRE-ENGINEERED ROOF TRUSS MANUFACTURERS ERECTION SHEET
ACCOMPANYING THE TRUSS PACKAGE. IF NOT AVAILABLE, IT IS THE
CONTRACTOR'S RESPONSIBILITY TO CONTACT THE SUPPLIER TO OBTAIN
ERECTION & BRACING SHEET.

Water
9/27/06

O:\SUPPORT\DETAILS\WINDLOADS\02-GABLE END.DWG 06/22/06 15:27

TYPICAL GABLE END DETAILS	PAUL STRESING ASSOCIATES, INC. 14517 MAIN STREET ALACHUA, FLORIDA 32816 E-MAIL: paul@psa-usa.com TELEPHONE: (386) 462-8407 FAX: (386) 462-8408 REGISTRATION NO. JPD00088 - CA NO. A40003371	DRAWN BY: C.A.D. DATE: 2006 PROJECT FILE WINDLOADS	SHEET NO. OF



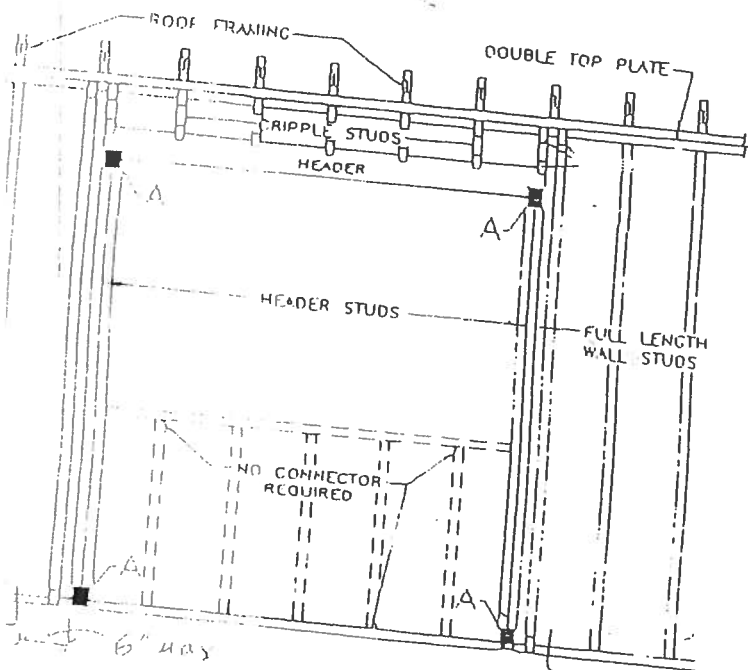
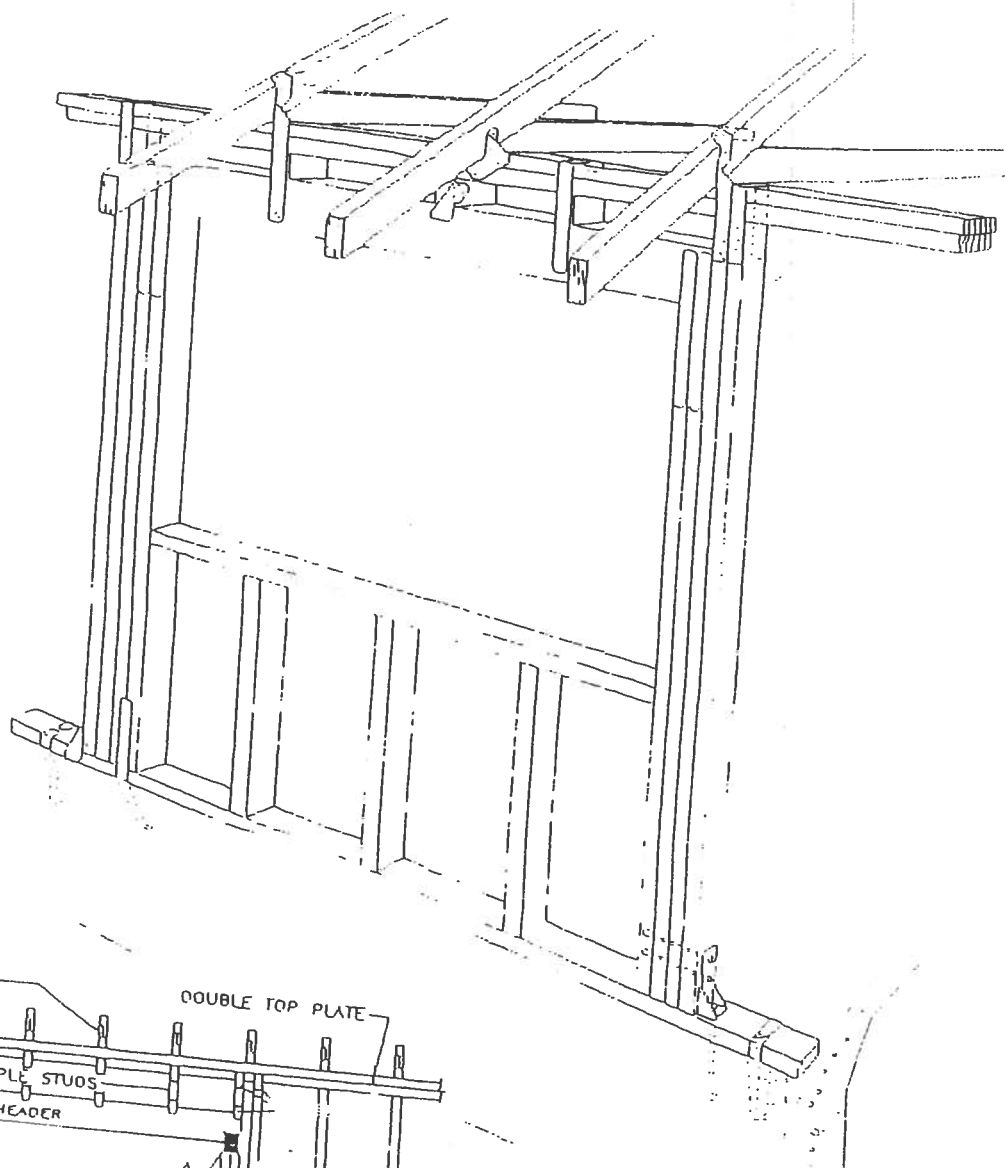
MINIMUM 2'-0" SHEAR WALL SEGMENT

NOTE: ALLOWABLE EQUIVALENT SHEAR WALL EQUAL TO 1.5 TIMES THE ACTUAL WALL SEGMENT LENGTH. MINIMUM WALL SEGMENT LENGTH OF 2'-0" WHICH EQUATES TO A 3'-0" SHEAR WALL SEGMENT. SEE WINDLOAD ANALYSIS FOR NAIL SIZES AND SPACING ON SHEATHING.

Handwritten signature and date:
 9/27/06
 3085

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TYPICAL SHEAR WALL DETAIL	PAUL STRESING ASSOCIATES, INC. 14817 MAIN STREET ALACHUA, FLORIDA 32010 E-MAIL: paul@psa.net TELEPHONE: (904) 482-6407 FAX: (904) 482-6408 REGISTRATION NO. AP070866 - CA NO. AA0003377	DRAWN BY: CAD DATE: 7/06 PROJECT FILE: WINDLOAD SHEET 110 OF



Total each truss uplift on the header divide by 2 for header anchorage

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700
Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.				

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A
Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors. Strap rafters to truss or at each end with minimum uplift resistance of 450# each end. Strap ridge beam at each end with minimum uplift resistance of 1000#. It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.		

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300
*or per truss engineering Use proper bolt anchors All beams to be sheathed or strapped to Double Top Plate when applicable.				

CRIPPLES | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS

Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
 Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
 Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
 Interior anchor bolts to be ½" x 8" A307 or ½" x 6" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE: For nailing into SPF members, multiply table values by .86

ASCE 7-02

9/25/06

Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	40	Deg
Type of Roof	Gabled	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	27.67	ft
Mean Roof Height (Ht)	19.36	ft
Width Perp. to Wind (B)	64.00	ft
Width Parallel to Wind (L)	88.33	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.30
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	I*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8855	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8574	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	1.26	
Nb	4.6*NatFreq*B/Vzm	4.15	
Nd	15.4*NatFreq*Depth/Vzm	19.19	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh))))	0.5049	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb))))	0.2118	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd))))	0.0508	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.5717	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.00	

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

ASCE 7-02

9/25/06

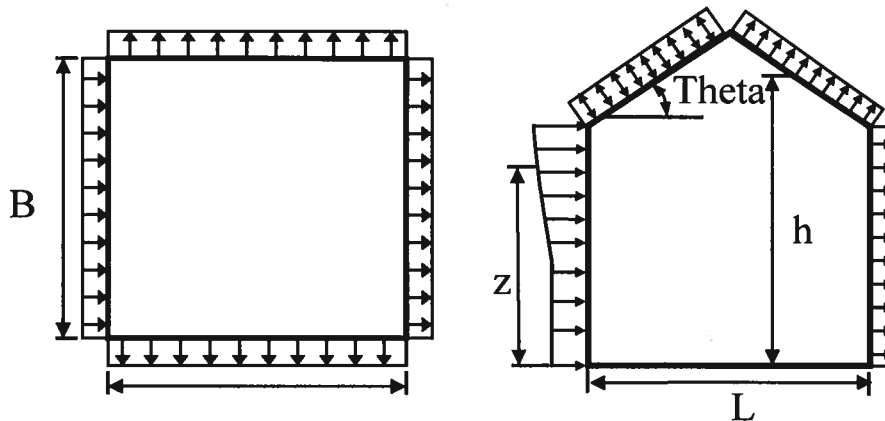
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft^2	Pressure (lb/ft^2)	
					Windward Wall*	
					+GCpi	-GCpi
27.67	0.70	1.00	1.00	21.70	11.44	18.33
20	0.70	1.00	1.00	21.70	11.44	18.33
19.36	0.70	1.00	1.00	21.70	11.44	18.33
15	0.70	1.00	1.00	21.70	11.44	18.33

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (H/z_g)^{2/\alpha}$	0.62	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d$	19.15	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 64 ft wall)	-0.42	-10.41	-3.51
Leeward Walls (Wind Dir Parallel to 88.33 ft wall)	-0.50	-11.66	-4.76
Side Walls	-0.70	-14.94	-8.05
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	0.00	0.00	0.00
Windward - Max Positive	0.00	0.00	0.00
Leeward Normal to Ridge	-0.60	-13.30	-6.40
Overhang Top	0.00	0.00	0.00
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 9.68 ft	-0.90	-18.22	-11.33

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Wind Load Design per ASCE 7-02

Dist from Windward Edge: 9.68 ft to 19.36 ft	-0.90	-18.22	-11.33
Dist from Windward Edge: 19.36 ft to 38.72 ft	-0.50	-11.66	-4.76
Dist from Windward Edge: > 38.72 ft	-0.30	-8.37	-1.48

* Horizontal distance from windward edge

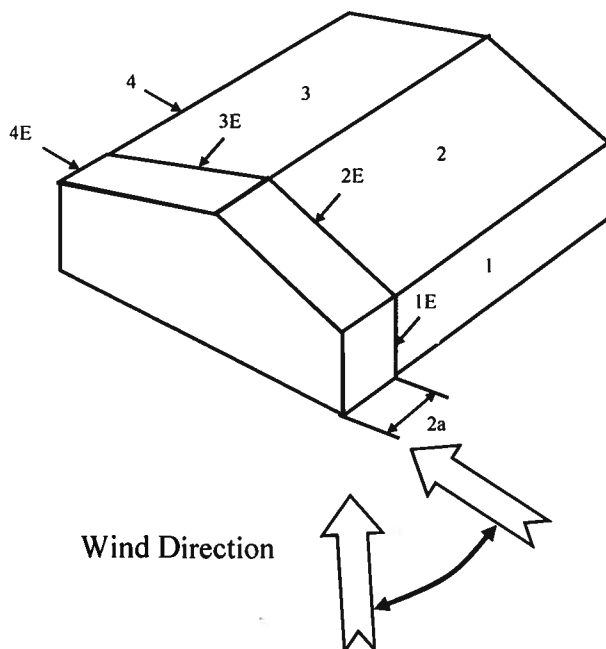
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

Kh =	$2.01 \cdot (Ht/zg)^{(2/\alpha)}$	=	0.62
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	$0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	=	19.15

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* $p = qh \cdot (GCpf - GCpi)$



ASCE 7-02

9/25/06

Wind Load Design per ASCE 7-02

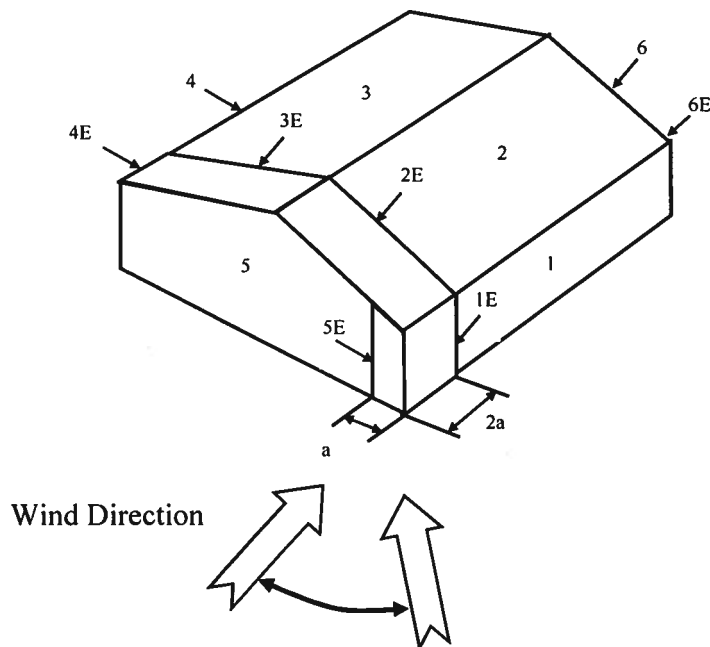
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (H_t/z_g)^{2/\alpha} &= & 0.62 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 19.15 \end{aligned}$$

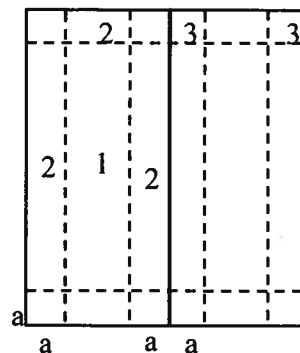
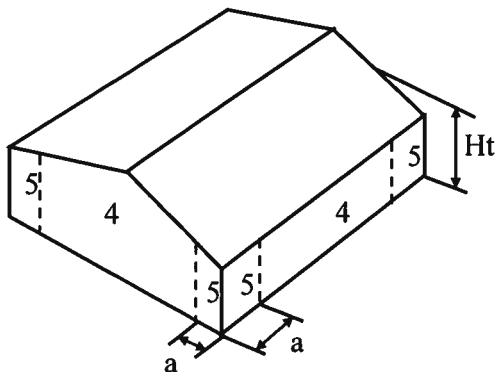
Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

$$* p = q_h \cdot (GC_{pf} - GC_{pi})$$



9/25/06

Figure 6-5 - External Pressure Coefficients, GCp
Loads on Components and Cladding for Buildings w/ Ht <= 60 ft



Gabled Roof
 $10 < \text{Theta} \leq 45$

$$a = 6.4 \implies \boxed{6.40 \text{ ft}}$$
[illegible]

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, C_{gpi}

ASCE 7-02

9/25/06

Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.39	-19.87	-11.66
	P (-GCpi) -psf	5.50	-12.97	-4.76
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.45	-19.87	-11.66
	P (-GCpi) -psf	3.45	-19.87	-11.66

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	88.33	ft
B	Roof dimension parallel to wind direction	64.00	ft
L/B	Ratio of L to B	1.380	
Theta	Slope of Roof	40	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft

Kelly O'Neil

From: <GDDampier@aol.com>
To: <oneilconstruction@atttel.net>
Sent: Friday, October 13, 2006 11:37 AM
Attach: anthony garage headers.htm
Subject: anthony

Let me know if you need anything else.

Glenda

☒																																																																																											
☒	Garage Header PLF Capacity																																																																																										
3-1/2" Width Garage Header PLF Capacity																																																																																											
GARAGE HEADER SUPPORTING ROOF LOADS ONLY 115% SNOW LOAD AREA																																																																																											
	<table border="1"> <thead> <tr> <th></th> <th colspan="2">8-3/8"</th> <th colspan="2">9-3/4"</th> <th colspan="2">11-1/4"</th> <th colspan="2">12-5/8"</th> <th colspan="2">14"</th> <th colspan="2">15-3/8"</th> <th colspan="2">16-3/4"</th> </tr> <tr> <th></th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> <th>Live</th> <th>Total</th> </tr> </thead> <tbody> <tr> <td>Rough Opening</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> <td>L/240</td> <td>L/180</td> </tr> <tr> <td>9'3"</td> <td>884</td> <td>896</td> <td>254</td> <td>1216</td> <td>390</td> <td>1573</td> <td>552</td> <td>1790</td> <td>752</td> <td>2048</td> <td>974</td> <td>2322</td> <td>896</td> <td>2568</td> </tr> <tr> <td>16'3"</td> <td>161</td> <td>207</td> <td>180</td> <td>330</td> <td>277</td> <td>510</td> <td>669</td> <td>510</td> <td>534</td> <td>824</td> <td>974</td> <td>980</td> <td>1165</td> <td>1165</td> </tr> <tr> <td>18'3"</td> <td>114</td> <td>145</td> <td>180</td> <td>231</td> <td>359</td> <td>391</td> <td>510</td> <td>534</td> <td>653</td> <td>707</td> <td>789</td> <td>896</td> <td>925</td> <td>925</td> </tr> </tbody> </table>		8-3/8"		9-3/4"		11-1/4"		12-5/8"		14"		15-3/8"		16-3/4"			Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Live	Total	Rough Opening	L/240	L/180	L/240	L/180	L/240	L/180	L/240	L/180	L/240	L/180	L/240	L/180	L/240	L/180	9'3"	884	896	254	1216	390	1573	552	1790	752	2048	974	2322	896	2568	16'3"	161	207	180	330	277	510	669	510	534	824	974	980	1165	1165	18'3"	114	145	180	231	359	391	510	534	653	707	789	896	925	925
	8-3/8"		9-3/4"		11-1/4"		12-5/8"		14"		15-3/8"		16-3/4"																																																																														
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☒	Power Beam																																																																																										
☒	Power Column																																																																																										
☒	Power Log																																																																																										
☒	Power Joist																																																																																										
☒	Power Header																																																																																										
☒	Power Plank																																																																																										
☒	24F Stock Glulam																																																																																										
☒	AEP 1.6 Short																																																																																										
☒	Span Header																																																																																										
☒	Southern Pine																																																																																										
☒	Lumber																																																																																										
☒	Custom Laminated																																																																																										
☒	Products																																																																																										

Load Capacity on Beam 16' x 669 lbs = 10,704 lbs
 Total allowable load on Beam

- ☐ Manufactured Wood Chips
- ☐ Glulam MSDS Sheets
- ☐ I-Joist MSDS Sheets
- ☐ Code Reports

☒

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☒ info@AnthonyForest.com

GARAGE HEADER SUPPORTING ROOF LOADS ONLY 125% NON-SNOW LOAD AREA

Rough Opening	8-3/8"		9-3/4"		11-1/4"		12-5/8"		14"		15-3/8"		16-3/4"	
	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180	Live Load L/240	Total Load L/180
9'3"	884	975	1322	1683	390	510	552	724	752	897	974	1067	1259	1267
16'3"	161	207	254	330	277	359	391	510	534	699	693	847	896	1007
18'3"	114	145	180	231										

GARAGE HEADER SUPPORTING ROOF, WALL, AND FLOOR LOADS 100% LOAD DURATION

Rough Opening	8-3/8"		9-3/4"		11-1/4"		12-5/8"		14"		15-3/8"		16-3/4"	
	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240	Live Load L/360	Total Load L/240
9'3"	562	778	888	1056	1363	1367	368	540	501	715	664	864	840	1011
16'3"	107	153	169	245	260	380	261	380	356	521	471	684	609	813
18'3"	76	107	120	171	185	267								

Notes:

- Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
- Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
- Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
- When no live load is listed, total load controls.

5. Deflection limits are listed within the PLF table heading.

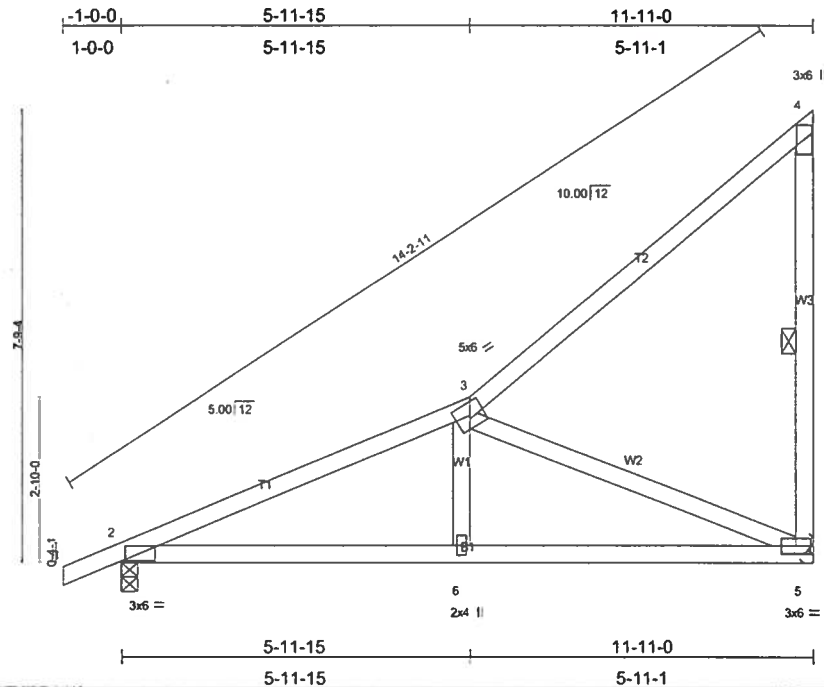
Garage header sizing using PLF tables:

To size a garage header supporting roof only, determine the total load in pounds per lineal foot (PLF). Check the appropriate PLF table for a deader supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load *and* live load capacity equal to or greater than the design load for the appropriate rough opening.

☐ [Back to Power Header](#)

☒ Copyright. All rights reserved.

Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
T13	PORCH TRUSS	10	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055		6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Sep 18 17:15:44 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.32	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.31	Vert(LL) -0.05 2'-6 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.50	Vert(TL) -0.07 2'-6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 5 n/a n/a		
	Code FBC2004/TPI2002				
					Weight: 64 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
 WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 5=485/Mechanical, 2=553/0-3-8
 Max Horz 2=362(load case 5)
 Max Uplift 5=-287(load case 5), 2=-146(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/20, 2-3=-791/34, 3-4=-135/64, 4-5=-122/169
 BOT CHORD 2-6=-305/677, 5-6=-308/666
 WEBS 3-6=0/204, 3-5=-706/318

JOINT STRESS INDEX
 2 = 0.37, 3 = 0.66, 4 = 0.34, 5 = 0.29 and 6 = 0.15

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 5 and 146 lb uplift at joint 2.

LOAD CASE(S) Standard

7 trusses on garage door Header
 7 x 553 Lbs = 3,871 Lbs
 total load on beam

TRIM-IT™ SPAN TABLES

Loading 40 TCLL-10 TCDL-0 BCLL-5 BCDL

TCLL = Top Chord Live Load • TCDL = Top Chord Dead Load

BCLL = Bottom Chord Live Load • BCDL = Bottom Chord Dead Load

3 x 2 SPF 2100F 1.8E • 4 x 2 SYP SS

Deflection L/360

Depth	Product Number	Lumber Size	On-Center Spacing			
			12"	16"	19.2"	24"
11-1/4"	TI 1223S	3 x 2	20-0-0	19-4-0	18-3-0	16-11-0
	TI 1224P	4 x 2	20-0-0	20-0-0	20-0-0	18-0-0
14"	TI 1423S	3 x 2	24-0-0	23-0-0	21-2-0	18-0-0
	TI 1424P	4 x 2	24-0-0	24-0-0	24-0-0	22-0-0
16"	TI 1623S	3 x 2	26-0-0	25-1-0	23-0-0	19-4-0
	TI 1624P	4 x 2	28-0-0	28-0-0	26-0-0	22-0-0

Deflection L/480

Depth	Product Number	Lumber Size	On-Center Spacing			
			12"	16"	19.2"	24"
11-1/4"	TI 1223S	3 x 2	19-4-0	17-7-0	16-7-0	15-0-0
	TI 1224P	4 x 2	20-0-0	19-5-0	18-0-0	16-11-0
14"	TI 1423S	3 x 2	22-0-0	20-8-0	19-6-0	18-0-0
	TI 1424P	4 x 2	24-0-0	23-2-0	21-5-0	20-0-0
16"	TI 1623S	3 x 2	25-1-0	22-6-0	21-0-0	19-4-0
	TI 1624P	4 x 2	28-0-0	25-4-0	23-5-0	21-5-0

Notes

- No composite action is assumed when sheathing is fastened to the top chord.
- For web configurations see specific component designs
- Spans can be applied to non-pitched roofs.
- Span tables indicate the maximum design spans (including a 1-3/4" bottom chord minimum bearing at each end).

Handwritten signature
6/17/05

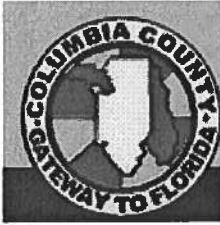
WARNING - Verify design parameters and READ NOTES

Design valid for use only with MiTek connector plates. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult QST-88 Quality Standard, DSB-89 Bracing Specification, and HIB-91 Handling Installing and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

MiTek Industries Inc
314-434-1200

14515 North Outer Forty Drive
Chesterfield, MO 63017-5746





From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: **0609-91**

O'Neal Construction Owner David Watson Property ID 20-7s-17-10027-127
On the date of October 2, 2006 application 0609-91 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0609-91 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.

2. Please submit a recorded (with the Columbia County Clerk Office) notice of commencement before any inspections can be performed by the Columbia County Building Department.
3. A second floor bonus room is shown to be located above the garage which will require the ceiling beneath the habitable rooms to comply with section R309.2 of the Florida Residential Building Code. Separation required: The garage shall be separated from the residence and its attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent.
4. The entry door into the dwelling from the garage shall comply with sections R309.1 of the Florida Residential Building Code. Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute fire-rated doors.
5. The engineered truss plan by First Source Builders show Trim-It floor joist to be installed to support the floor system for the second floor bonus room. Please submit the engineered plan for these trim joists and also show the material type, thickness and nailing pattern for the sub-flooring material to be used on the bonus room floor.

- 6.** Please show the engineered designed method to be used to support the trusses which will be placed over the 16' garage door opening also show the method of attachment of the header beam to the foundation and the required number of jack and king studs to support this header beam.
- 7.** The elevation plan and the truss plan both show inactive dormers, please show the design, material to be used and attachment method of the dormers to the roof decking and truss system.

Thank You:



Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

PERMIT # 000025126

PARCEL # 10027-127

NOTICE OF COMMENCEMENT

STATE OF: FLORIDA

COUNTY OF: Columbia

CITY OF: _____

THE UNDERSIGNED hereby gives notice that improvement(s) will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

DESCRIPTION OF PROPERTY:

LOT: 29 BLOCK: _____ SECTION: 20 TOWNSHIP: 7S RANGE: 17

SUBDIVISION: _____ PLATBOOK: _____ MAP PAGE #: _____

STREET ADDRESS: 103 SW Buck Court Ft. White FL 32038

GENERAL DESCRIPTION OF IMPROVEMENT:

TO CONSTRUCT: SINGLE FAMILY RESIDENCE

OWNER(S) NAME: DAVID Watson OWNER INFORMATION:

ADDRESS: 103 SW Buck Court

CITY: Ft. White

STATE: FL

PHONE NO.: 386 454 2476

ZIP CODE: 32038

INTEREST IN THE PROPERTY: OWNER

FEE SIMPLE TITLEHOLDER NAME: SAME

FEE SIMPLE TITLEHOLDER ADDRESS: SAME

(if other than owner)

CONTRACTOR NAME: ONEIL CONSTRUCTION

ADDRESS: P.O. Box 1633

CITY: HIGH Springs

STATE: FL

PHONE NO.: 386 454 2476

ZIP CODE: 32643

BONDING COMPANY: N/A

ADDRESS: _____

CITY: _____

LENDER NAME: N/A

ADDRESS: _____

CITY: _____

Inst: 2006024531 Date: 10/16/2006 Time: 14:15

DC, P. DeWitt Cason, Columbia County B: 1099 P: 145

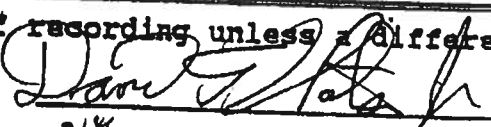
Persons within the State of Florida designated by owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:

NAME: DAVID Watson

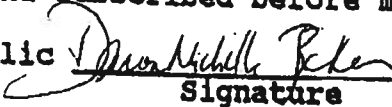
ADDRESS: 103 SW Buck Court Ft. White FL

In addition to himself, owner designates DENNIS ONEIL of ONEIL CONSTRUCTION to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

Expiration date is 1 year from date of recording unless a different date is specified.

SIGNATURE OF OWNER: 

Sworn to and subscribed before me this 14th day of September A.D. 2006

Notary Public  Signature

My Commission Expires: 4/14/08
Dawn Michelle Baker
My Commission DD300046
Expires April 14, 2008

41168

Notice of Treatment**Applicator:** Florida Pest Control & Chemical Co. (www.flapest.com)Address: 116 NW 16 Ave
City Gulf Breeze Phone 326-26601**Site Location:** Subdivision _____Lot # _____ Block# _____ Permit # 25126Address 103 SW Buck CRT Ft White**Product used****Active Ingredient****% Concentration**

- | | | |
|---|----------------------------------|-------|
| ☒ Premise | Imidacloprid | 0.1% |
| ☐ Termidor | Fipronil | 0.12% |
| ☐ Bora-Care | Disodium Octaborate Tetrahydrate | 23.0% |

Type treatment:☒ Soil☐ Wood**Area Treated****Square feet****Linear feet****Gallons Applied**

<u>MB</u>	<u>3950</u>	_____	<u>360</u>
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

<u>11-17-06</u>	<u>11:35</u>	<u>Guy</u>
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05



Notice of Treatment 41168

Applicator: **Florida Pest Control & Chemical Co. (www.flapest.com)**

Address: 116 NW 16 AVE

City FT. WILLY Phone 376-2661

Site Location: Subdivision _____

Lot # _____ Block# _____ Permit # 25126

Address 103 SW BUCK CT FT. WILLY

<u>Product used</u>	<u>Active Ingredient</u>	<u>% Concentration</u>
---------------------	--------------------------	------------------------

☒ Premise	Imidacloprid	0.1%
---	--------------	------

☐ Termidor	Fipronil	0.12%
-----------------------------------	----------	-------

☐ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%
------------------------------------	----------------------------------	-------

Type treatment: ☒ Soil ☐ Wood

<u>Area Treated</u>	<u>Square feet</u>	<u>Linear feet</u>	<u>Gallons Applied</u>
---------------------	--------------------	--------------------	------------------------

<u>PERIMETER</u>	<u> </u>	<u>212.8</u>	<u>43</u>
------------------	---------------	--------------	-----------

_____	_____	_____	_____
-------	-------	-------	-------

_____	_____	_____	_____
-------	-------	-------	-------

_____	_____	_____	_____
-------	-------	-------	-------

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line .

<u>6/7/07</u>	<u>1020</u>	<u>BILL E</u>
Date	Time	Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05



COLUMBIA AVENUE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-7S-17-10027-127

Building permit No. 000025126

Use Classification SFD, UTILITY

Fire: 0.00

Permit Holder O'NEIL CONSTRUCTION

Waste:

Owner of Building DAVID WATSON

Total: 0.00

Location: 103 SW BUCK COURT, FT. WHITE, FL

Date: 06/11/2007



[Signature]

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

COLUMBIA COUNTY OFFICE OF CIVIL ENGINEERING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-7S-17-10027-127

Building permit No. 000025126

Use Classification SFD, UTILITY

Fire: 0.00

Permit Holder O'NEIL CONSTRUCTION

Waste:

Owner of Building DAVID WATSON

Total: 0.00

Location: 103 SW BUCK COURT, FT. WHITE, FL

Date: 06/11/2007



A handwritten signature in black ink, likely belonging to the Building Inspector.

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:

Builder: L210395
 Lot: O'NEIL CONST.
 Subdivision: N/A
 County or City: 103 BUCK COURT
 Truss Page Count: COLUMBIA COUNTY
 39

Date: 9/19/2006
 Start Number: 1074
 SEI Ref: L210395

Truss Design Load Information (UNO)

Design Program: MiTek

Gravity**Wind**

Building Code:

FBC2004

Roof (psf): 42 Wind Standard: ASCE 7-02
 Floor (psf): 55 Wind Speed (mph): 110

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

O'NEIL CONSTRUCTION OF HIGH SPRING INC. QB 0010656
 Address: 235 NE 2ND STREET
 HIGH SPRINGS, FL. 32643

Designer: 115

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987

Company: Structural Engineering and Inspections, Inc. EB 9196
 Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitability and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0919061074	9/19/2006				
2	CJ3	0919061075	9/19/2006				
3	CJ5	0919061076	9/19/2006				
4	CJ7	0919061077	9/19/2006				
5	EJ8	0919061078	9/19/2006				
6	EJ9	0919061079	9/19/2006				
7	FG1	0919061080	9/19/2006				
8	FG2	0919061081	9/19/2006				
9	FG2	0919061082	9/19/2006				
10	HJ2	0919061083	9/19/2006				
11	HJ12	0919061084	9/19/2006				
12	MG1	0919061085	9/19/2006				
13	PB15	0919061086	9/19/2006				
14	PB15G	0919061087	9/19/2006				
15	T01	0919061088	9/19/2006				
16	T01	0919061089	9/19/2006				
17	T01G	0919061090	9/19/2006				
18	T02	0919061091	9/19/2006				
19	T02	0919061092	9/19/2006				
20	T03	0919061093	9/19/2006				
21	T03	0919061094	9/19/2006				
22	T03G	0919061095	9/19/2006				
23	T04	0919061096	9/19/2006				
24	T05	0919061097	9/19/2006				
25	T06	0919061098	9/19/2006				
26	T07	0919061099	9/19/2006				
27	T08	0919061100	9/19/2006				
28	T09	0919061101	9/19/2006				
29	T09	0919061102	9/19/2006				
30	T10	0919061103	9/19/2006				
31	T10A	0919061104	9/19/2006				
32	T10G	0919061105	9/19/2006				
33	T11	0919061106	9/19/2006				
34	T12	0919061107	9/19/2006				
35	T13	0919061108	9/19/2006				
36	T13G	0919061109	9/19/2006				
37	T14	0919061110	9/19/2006				
38	T14G	0919061111	9/19/2006				
39	T15	0919061112	9/19/2006				

SEP 19 2006



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07:09:44 AM

Name: **O'NEIL CONSTRUCTION OF HIGH SPRING
INC (Primary Name)**

Main Address: **P O BOX 1633
HIGH SPRINGS, Florida 32655**

Lic. Location: **110 NE 1ST AVENUE
HIGH SPRINGS, FL 32643
Alachua**

License Type:	Construction Qualified Business
Rank:	Qualified Org
License Number:	QB0010656
Status:	Current, Active
Licensure Date:	12/30/1998
Expires:	08/31/2005

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III

SEPTEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. FEB 9 1998
18105 N. FLORIDA AVE. STE B, LOTZ, FL 33549

Job L210395	Truss CJ1	Truss Type JACK	Qty 8	Ply 1	ONEIL / WATSON RES. Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:16:35 2006 Page 1

WndRoofZone~2

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.06	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(LL) -0.00 2 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.00 2 >999 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 5 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=126/0-3-8, 4=14/Mechanical, 3=-4/Mechanical
 Max Horz 2=45(load case 5)
 Max Uplift 2=-116(load case 5), 4=-9(load case 3), 3=-4(load case 1)
 Max Grav 2=126(load case 1), 4=14(load case 1), 3=13(load case 5)

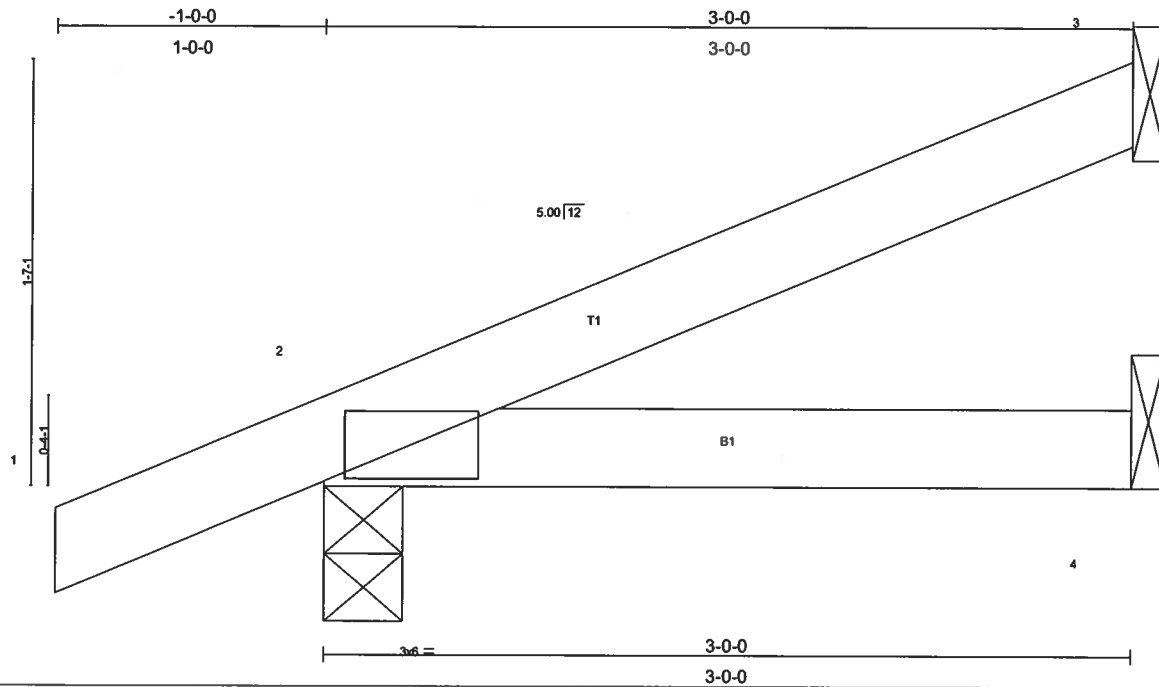
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/19, 2-3=-24/5
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.06

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2, 9 lb uplift at joint 4 and 4 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L210395	Truss CJ3	Truss Type JACK	Qty 6	Ply 1	ONEIL / WATSON RES.
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MITek Industries, Inc. Mon Sep 18 17:15:16 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.08	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=63/Mechanical, 2=192/0-3-8, 4=42/Mechanical
Max Horz 2=83(load case 5)
Max Uplift 3=51(load case 5), 2=146(load case 5), 4=27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-3=-45/20
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.07

NOTES

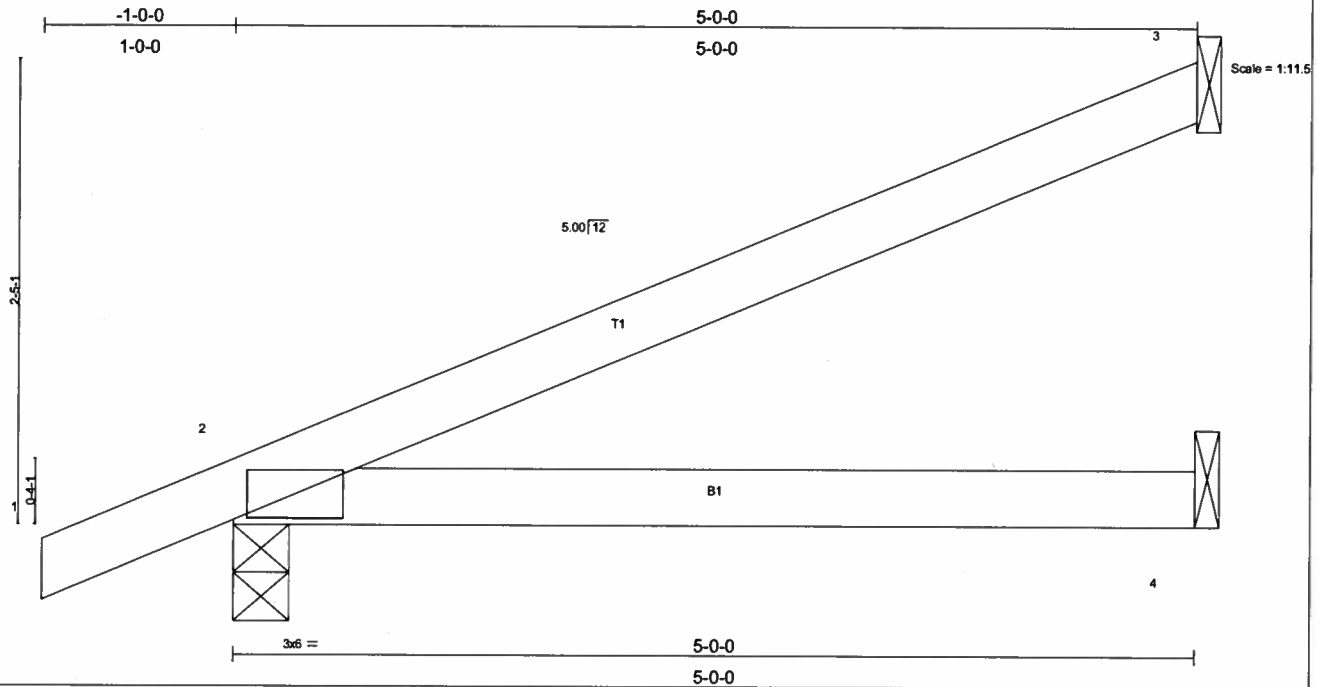
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 3, 146 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	CJ5	JACK	6	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) 0.09	2-4	>663	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.24	Vert(TL) 0.07	2-4	>774	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=122/Mechanical, 2=271/0-3-8, 4=72/Mechanical
Max Horz 2=121(load case 5)
Max Uplift 3=-103(load case 5), 2=-188(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-3=-83/38
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.10

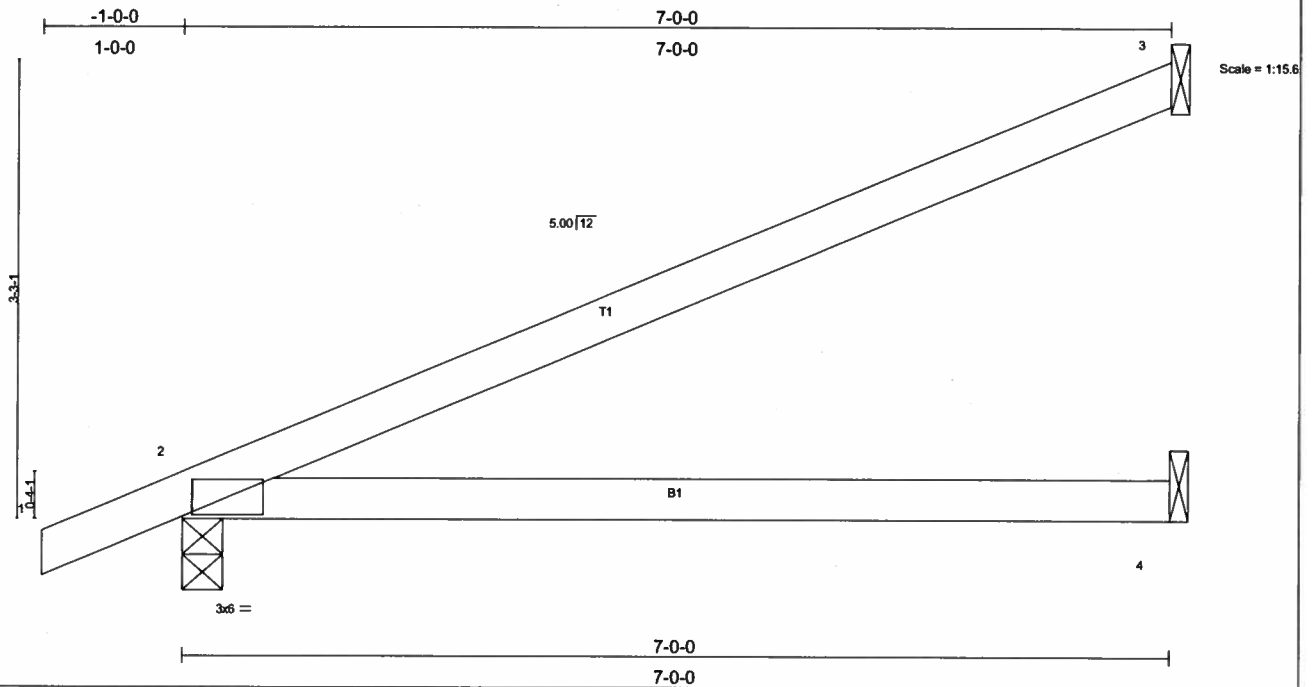
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf, BCDF=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 3, 188 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

**SEPTEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Job L210395	Truss CJ7	Truss Type JACK	Qty 6	Ply 1	ONEIL / WATSON RES.
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Sep 18 17:15:17 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.48	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.43	Vert(LL) 0.31 2-4 >263 240		
BCCL 10.0	Lumber Increase 1.25	WB 0.00	Vert(TL) 0.25 2-4 >321 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2004/TPI2002			Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=169/Mechanical, 2=352/0-3-8, 4=111/Mechanical
Max Horz 2=160(load case 5)
Max Uplift 3=141(load case 5), 2=234(load case 5), 4=74(load case 5)

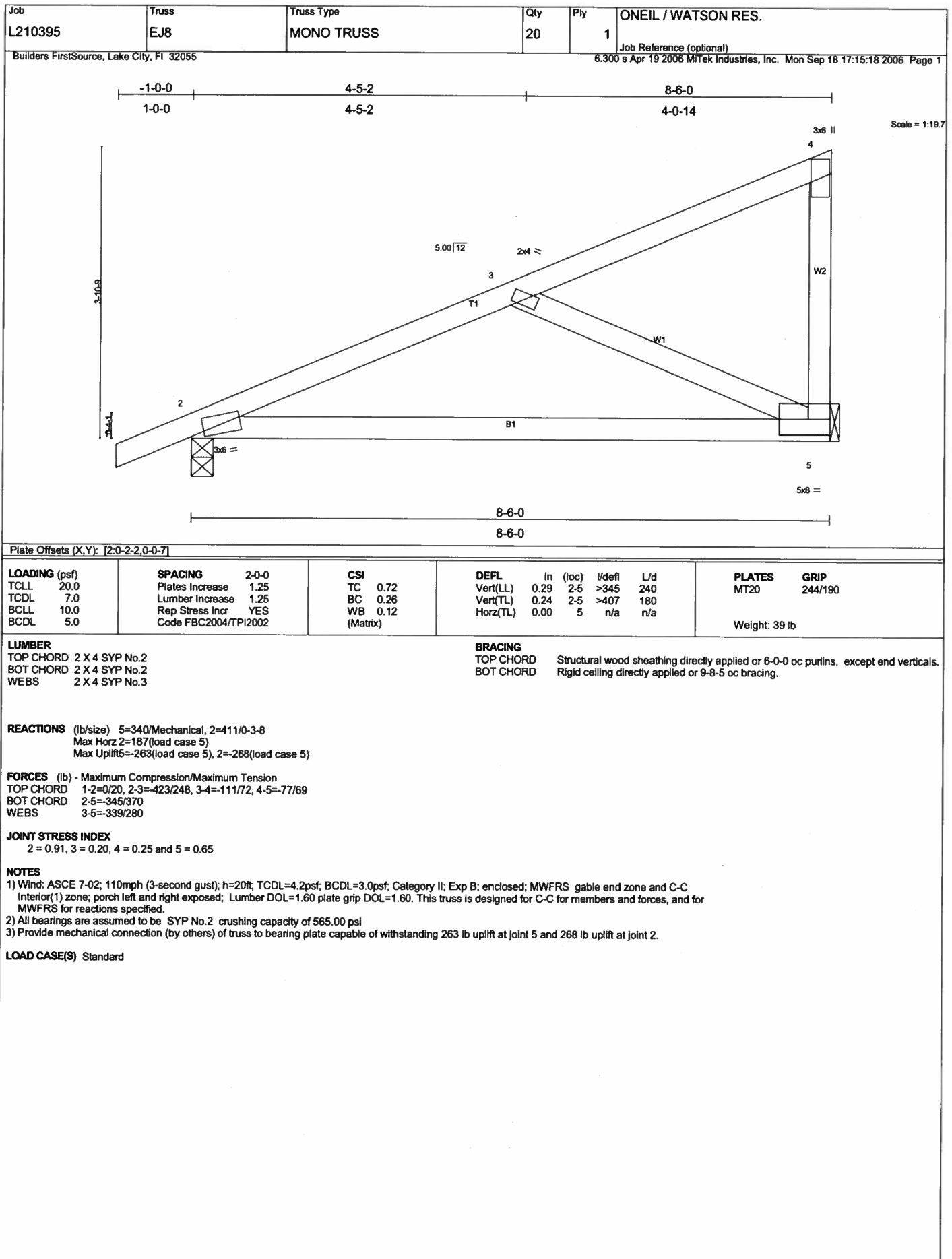
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-3=-78/52
BOT CHORD 2-4=0/0

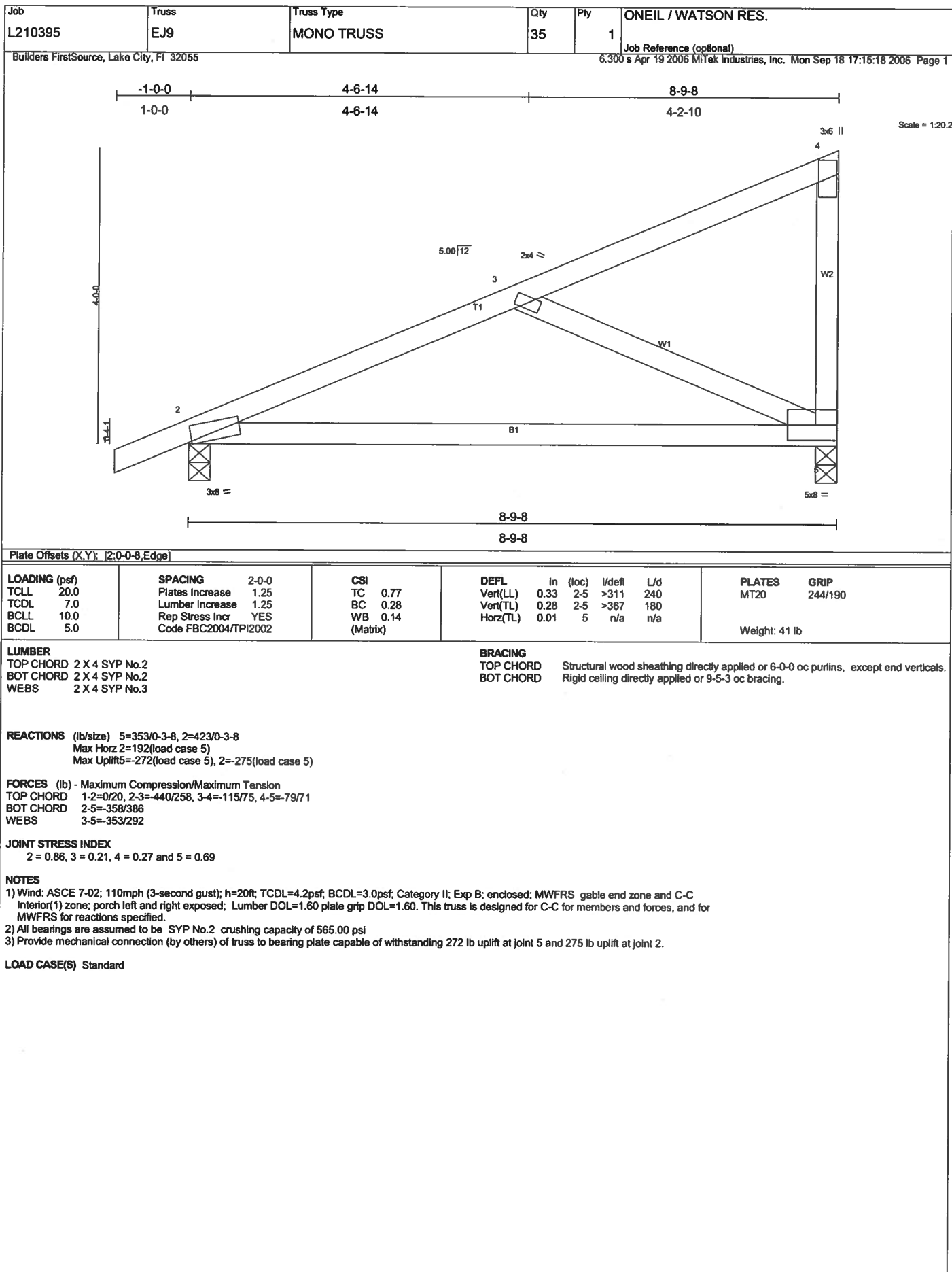
JOINT STRESS INDEX
2 = 0.45

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 3, 234 lb uplift at joint 2 and 74 lb uplift at joint 4.

LOAD CASE(S) Standard





Job L210395	Truss FG1	Truss Type SPECIAL	Qty 1	Ply 2	ONEIL / WATSON RES. Job Reference (optional)
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Builders FirstSource, Lake City, Fl 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:19 2006 Page 1

Scale = 1.27.7

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.03 BC 0.20 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 3-4 >999 240 Vert(TL) -0.02 3-4 >999 180 Horz(TL) -0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 107 lb
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LUMBER
 TOP CHORD 2 X 8 SYP No.1D
 BOT CHORD 2 X 8 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=729/Mechanical, 3=729/Mechanical
 Max Uplift 4=-275(load case 2), 3=-275(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-4=-114/79, 1-2=0/0, 2-3=-114/79
 BOT CHORD 4-5=0/0, 5-6=0/0, 3-6=0/0
 WEBS 1-3=0/0

JOINT STRESS INDEX
 1 = 0.01, 2 = 0.02, 3 = 0.01 and 4 = 0.01

NOTES
 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 3) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 4) Provide adequate drainage to prevent water ponding.
 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 4 and 275 lb uplift at joint 3.
 7) Girder carries tie-in span(s): 23-4-0 from 1-0-0 to 3-6-0

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54, 4-5=-30, 5-6=-472(B=-442), 3-6=-30

Job L210395	Truss FG2	Truss Type SPECIAL	Qty 1	Ply 3	ONEIL / WATSON RES. Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:20 2006 Page 1		

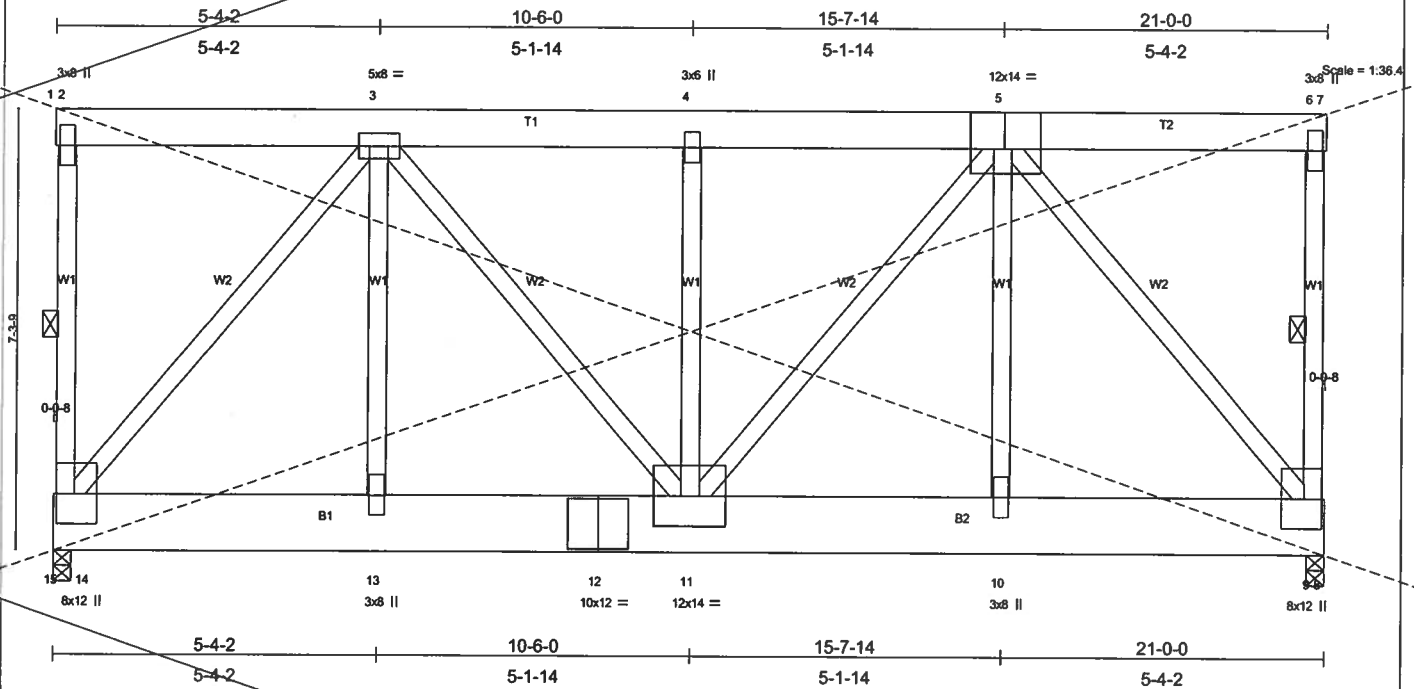


Plate Offsets (X,Y): [5:0-6-12,0-7-2], [9:0-6-0,0-3-8], [14:0-6-0,0-3-8]

LOADING (psf)	SPACING	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.18	Vert(LL)	-0.05	11	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	TC 0.25	Vert(TL)	-0.08	11	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.58	Horz(TL)	0.01	9	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002								
								Weight: 758 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D
BOT CHORD 2 X 12 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
WEBS 1 Row at midpt 2'-14, 6'-9

REACTIONS (lb/size) 14=7113/0-3-8, 9=7113/0-3-8
Max Uplift 14=1166(load case 2), 9=1166(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-14=527/219, 1-2=0/0, 2-3=47/9, 3-4=5784/932, 4-5=5784/932, 5-6=47/8, 6-7=0/0, 6-9=524/218
BOT CHORD 14-15=0/0, 13-14=717/4285, 12-13=717/4285, 11-12=717/4285, 10-11=715/4278, 9-10=715/4271, 8-9=0/0
WEBS 3-14=6836/1142, 3-13=20/2159, 3-11=347/2418, 4-11=1056/479, 5-11=349/2423, 5-10=21/2161, 5-9=6831/1142

JOINT STRESS INDEX
2 = 0.12, 3 = 0.55, 4 = 0.16, 5 = 0.25, 6 = 0.12, 9 = 0.32, 10 = 0.23, 11 = 0.11, 12 = 0.18, 13 = 0.23 and 14 = 0.32

NOTES

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 12 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- All bearings are assumed to be SYP No.1D crushing capacity of 660.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1166 lb uplift at joint 14 and 1166 lb uplift at joint 9.
- Load case(s) 1, 2, 3, 4, 5, 6, 7 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- Girder carries tie-in span(s): 10-7-8 from 0-0-0 to 21-0-0; 12-0-8 from 0-0-0 to 21-0-0

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 2-6=229(F=175), 9-14=461(F=226, B=205)
- MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 2-6=104(F=66), 9-14=9(F=62, B=77)
- MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 2-6=104(F=66), 9-14=9(F=62, B=77)
- MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 2-6=104(F=66), 9-14=9(F=62, B=77)
- MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 2-6=90(F=66), 9-14=9(F=62, B=77)

Continued on page 2

Job L210395	Truss FG2	Truss Type SPECIAL	Qty 1	Ply 3	ONEIL / WATSON RES. Job Reference (optional)
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Builders FirstSource, Lake City, Fl 32055

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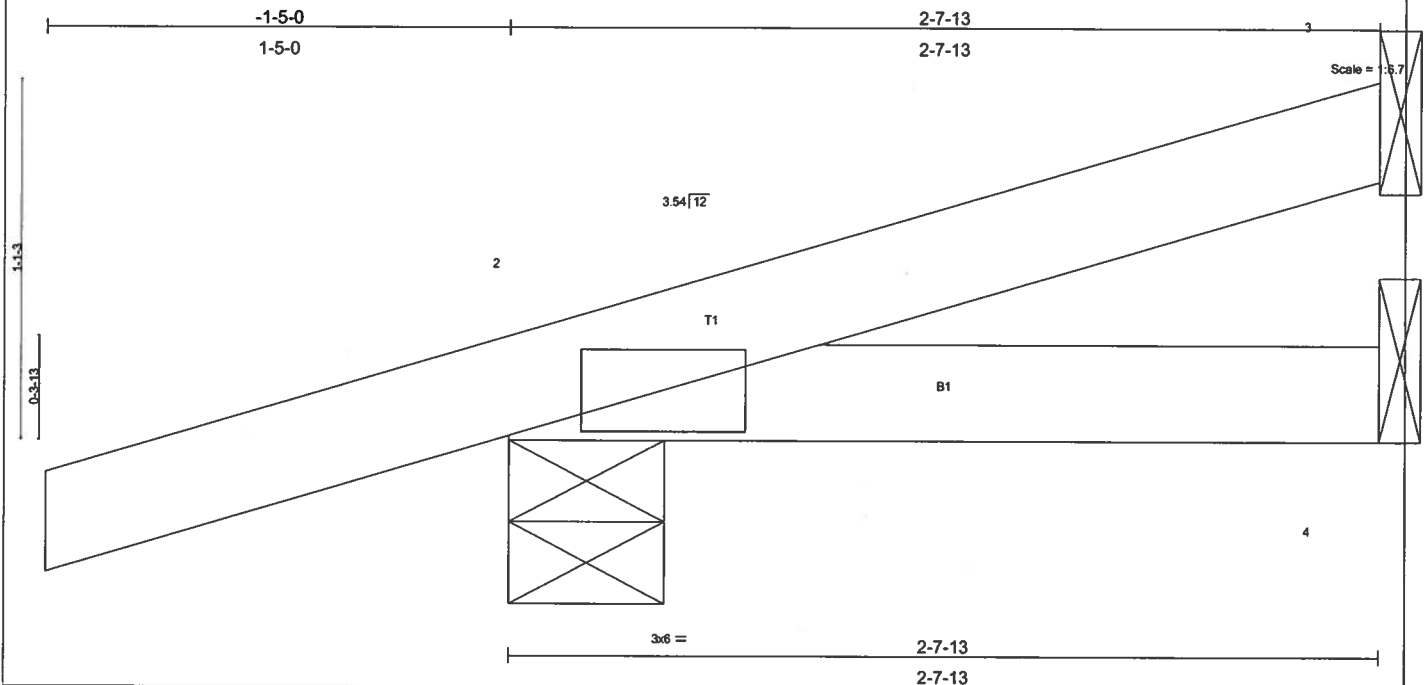
LOAD CASE(S) Standard6) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)

Vert: 2-6=84(F=66), 9-14=9(F=-62, B=77)

7) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)

Vert: 2-6=78(F=66), 9-14=9(F=-62, B=77)

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	HJ2	JACK	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:22 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.04	Vert(TL)	-0.00	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 10 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=32/Mechanical, 2=219/0-5-11, 4=35/Mechanical
 Max Horz 2=62(load case 2)
 Max Uplift 3=21(load case 5), 2=159(load case 2)

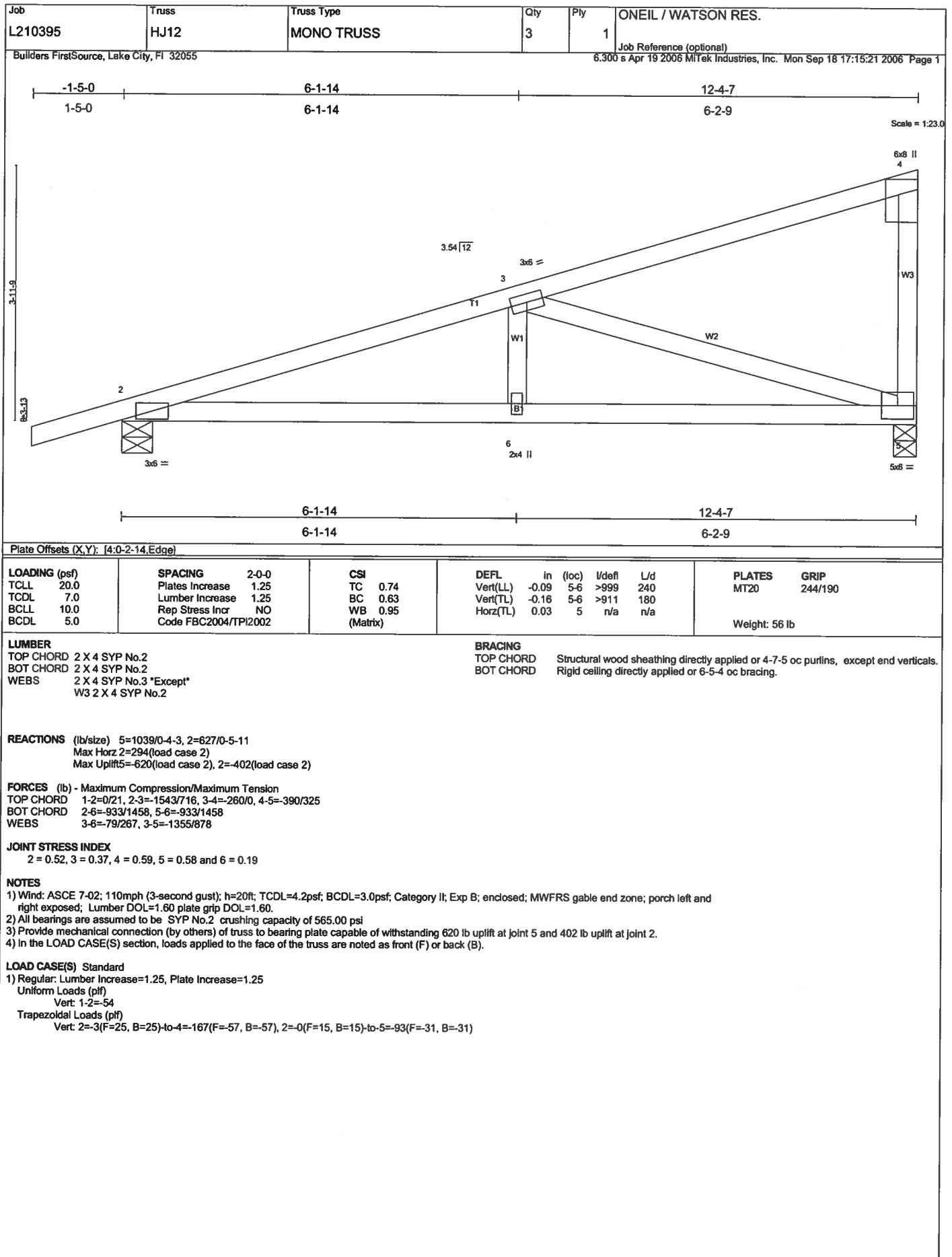
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/21, 2-3=30/6
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.07

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 3 and 159 lb uplift at joint 2.

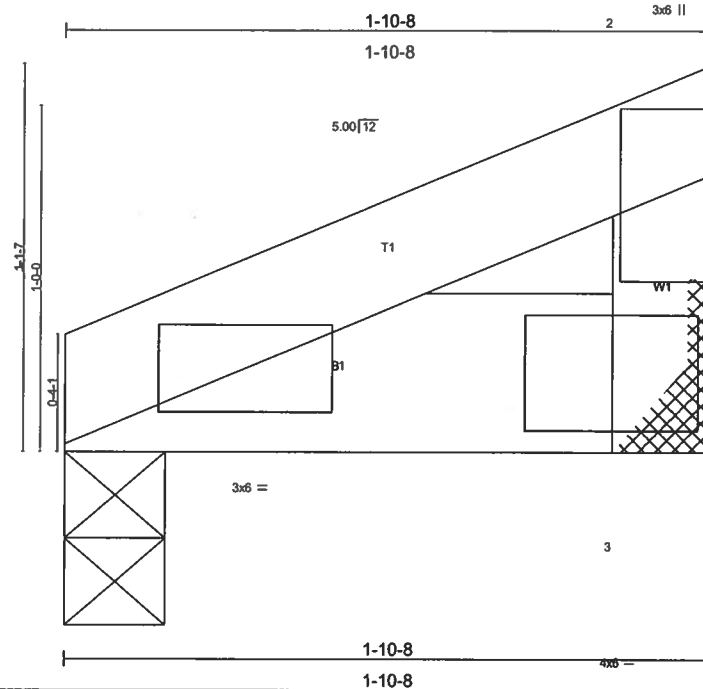
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	MG1	JACK	1	1	Job Reference (optional)

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Scale = 1:6.4

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.04	Vert(LL) -0.00 1-3 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.00 1-3 >999 180		
BCLL 10.0	Rep Stress Incr NO	WB 0.00	Horz(TL) 0.00 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			Weight: 8 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 1-10-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=557/0-3-8, 3=557/Mechanical
Max Horz 1=31(load case 4)
Max Uplift1=-204(load case 4), 3=-216(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-19/13, 2-3=-46/38
BOT CHORD 1-3=0/0

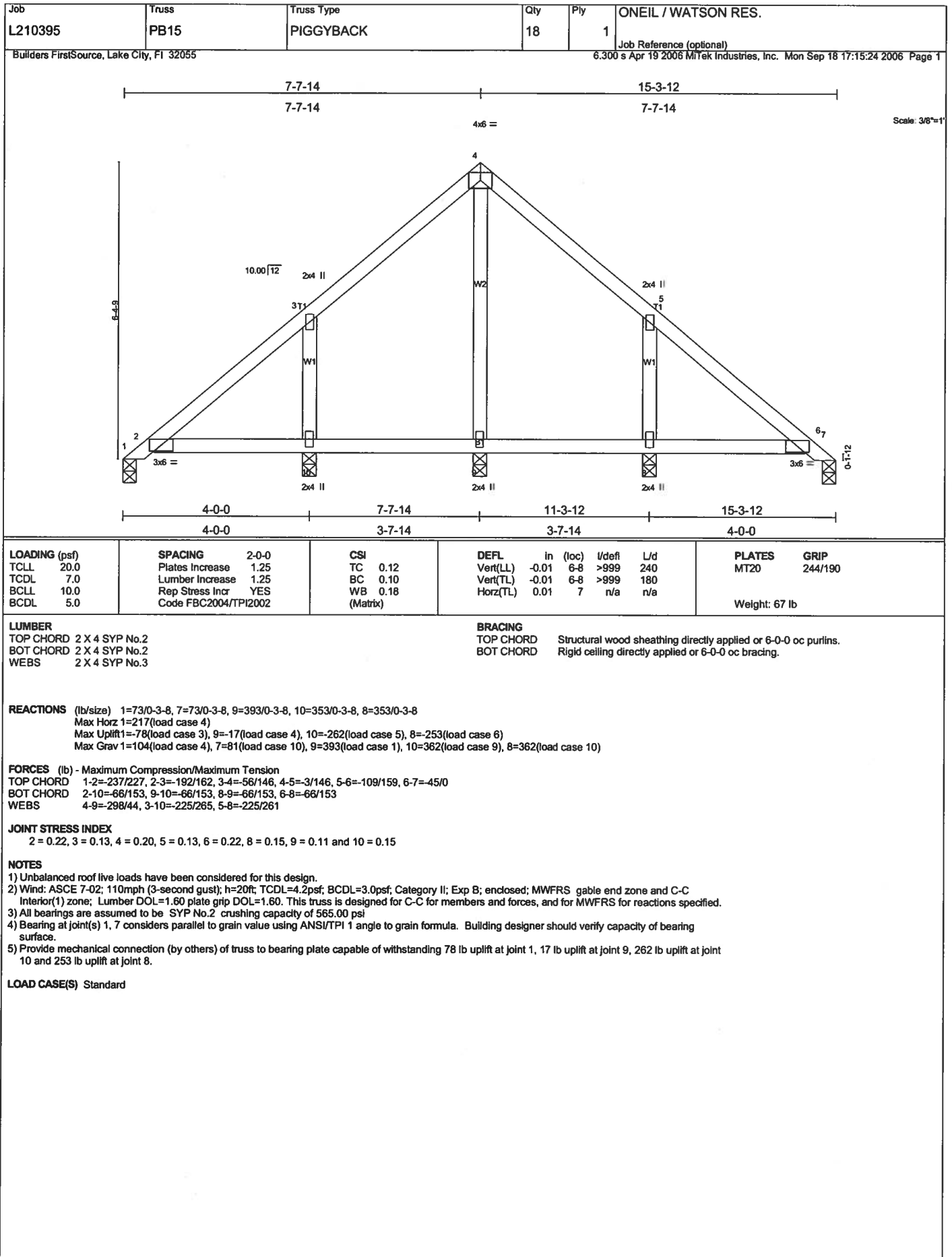
JOINT STRESS INDEX
1 = 0.02, 2 = 0.01 and 3 = 0.21

NOTES

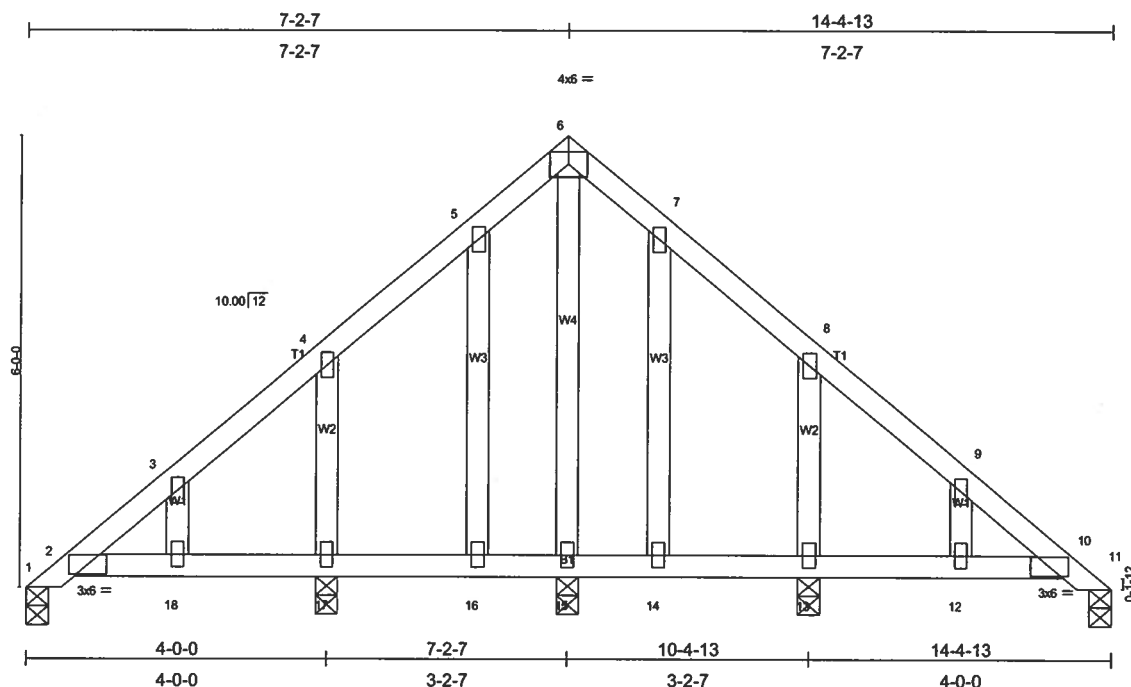
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 1 and 216 lb uplift at joint 3.
- 4) Girder carries tie-in span(s): 29-6-0 from 0-0-0 to 1-10-8
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular. Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 1-3=-601(8=-571)



Job L210395	Truss PB15G	Truss Type PIGGYBACK	Qty 2	Ply 1	ONEIL / WATSON RES. Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MITek Industries, Inc. Mon Sep 18 17:15:25 2006 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.12	In (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.12	Vert(LL) -0.01 12 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.12	Vert(TL) -0.01 12 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 79 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=63/0-3-8, 11=63/0-3-8, 15=365/0-3-8, 17=338/0-3-8, 13=338/0-3-8
 Max Horz 1=204(load case 4)
 Max Uplift 1=69(load case 3), 15=22(load case 5), 17=249(load case 5), 13=240(load case 6)
 Max Grav 1=97(load case 4), 11=76(load case 10), 15=365(load case 1), 17=346(load case 9), 13=346(load case 10)

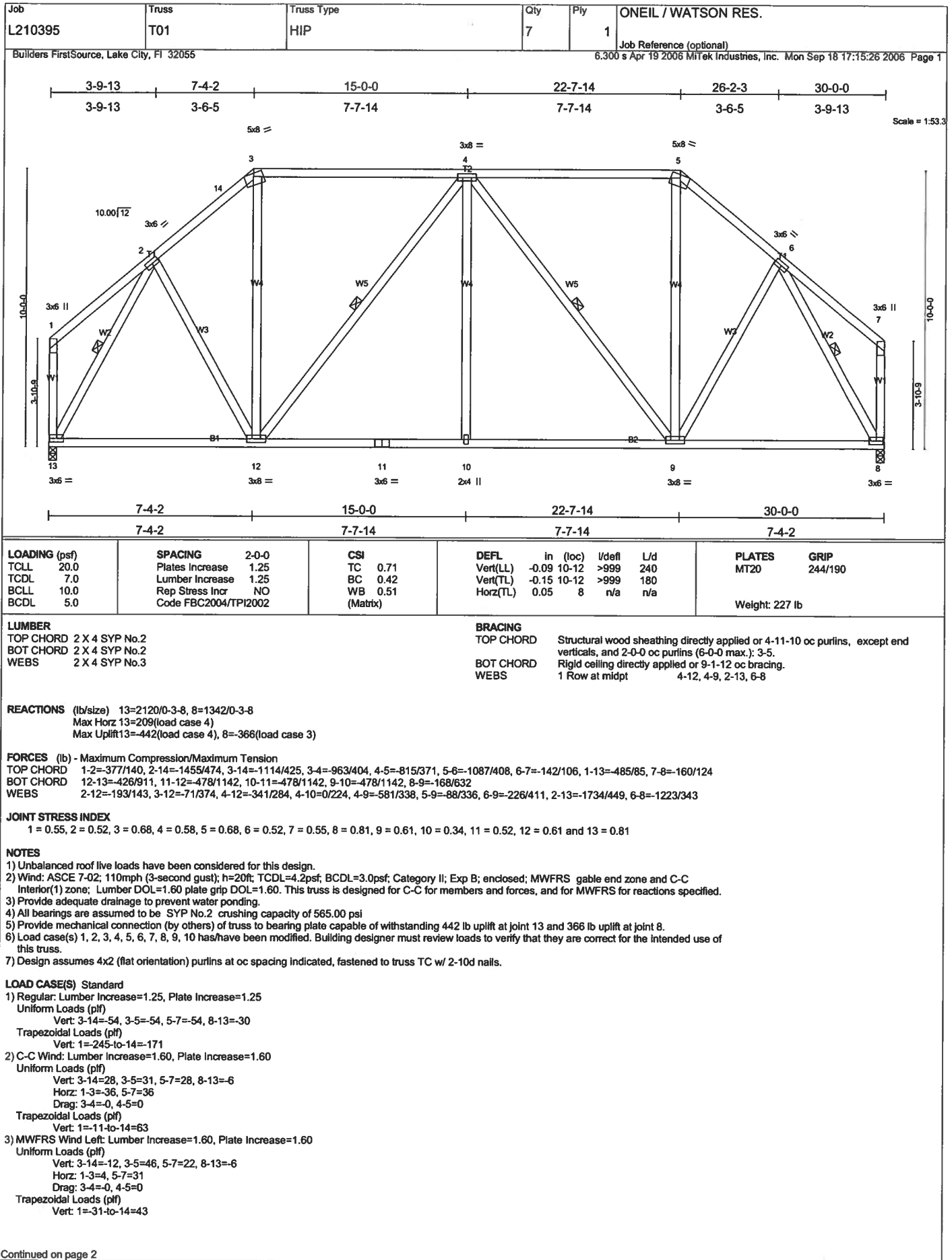
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-222/211, 2-3=-202/141, 3-4=-161/169, 4-5=-68/118, 5-6=-25/134, 6-7=0/134, 7-8=-3/118, 8-9=-90/169, 9-10=-148/118, 10-11=-42/0
 BOT CHORD 2-18=-82/160, 17-18=-82/160, 16-17=-82/160, 15-16=-82/160, 14-15=-82/160, 13-14=-82/160, 12-13=-82/160, 10-12=-82/160
 WEBS 6-15=-222/18, 4-17=-181/196, 8-13=-181/193, 5-16=-49/68, 7-14=-49/64, 3-18=-36/78, 9-12=-36/77

JOINT STRESS INDEX
 2 = 0.13, 3 = 0.04, 4 = 0.10, 5 = 0.03, 6 = 0.09, 7 = 0.03, 8 = 0.10, 9 = 0.04, 10 = 0.13, 12 = 0.04, 13 = 0.11, 14 = 0.04, 15 = 0.08, 16 = 0.04, 17 = 0.11 and 18 = 0.04

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All plates are 2x4 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 1, 22 lb uplift at joint 15, 249 lb uplift at joint 17 and 240 lb uplift at joint 13.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T01	HIP	7	1	Job Reference (optional)

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LOAD CASE(S) Standard**4) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60**

Uniform Loads (plf)

Vert: 3-14=22, 3-5=46, 5-7=-12, 8-13=-6

Horz: 1-3=-31, 5-7=-4

Drag: 3-4=-0, 4-5=0

Trapezoidal Loads (plf)

Vert: 1=4-to-14=78

5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 3-14=38, 3-5=24, 5-7=24, 8-13=-6

Horz: 1-3=-46, 5-7=32

Drag: 3-4=-0, 4-5=0

Trapezoidal Loads (plf)

Vert: 1=8-to-14=82

6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 3-14=24, 3-5=24, 5-7=38, 8-13=-6

Horz: 1-3=-32, 5-7=46

Drag: 3-4=-0, 4-5=0

Trapezoidal Loads (plf)

Vert: 1=-6-to-14=68

7) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 3-14=18, 3-5=12, 5-7=12, 8-13=-6

Horz: 1-3=-26, 5-7=20

Drag: 3-4=-0, 4-5=0

Trapezoidal Loads (plf)

Vert: 1=-40-to-14=34

8) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 3-14=12, 3-5=12, 5-7=18, 8-13=-6

Horz: 1-3=-20, 5-7=26

Drag: 3-4=-0, 4-5=0

Trapezoidal Loads (plf)

Vert: 1=-46-to-14=28

9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 3-14=-54, 3-5=-54, 5-7=-14, 8-13=-30

Trapezoidal Loads (plf)

Vert: 1=-245-to-14=-171

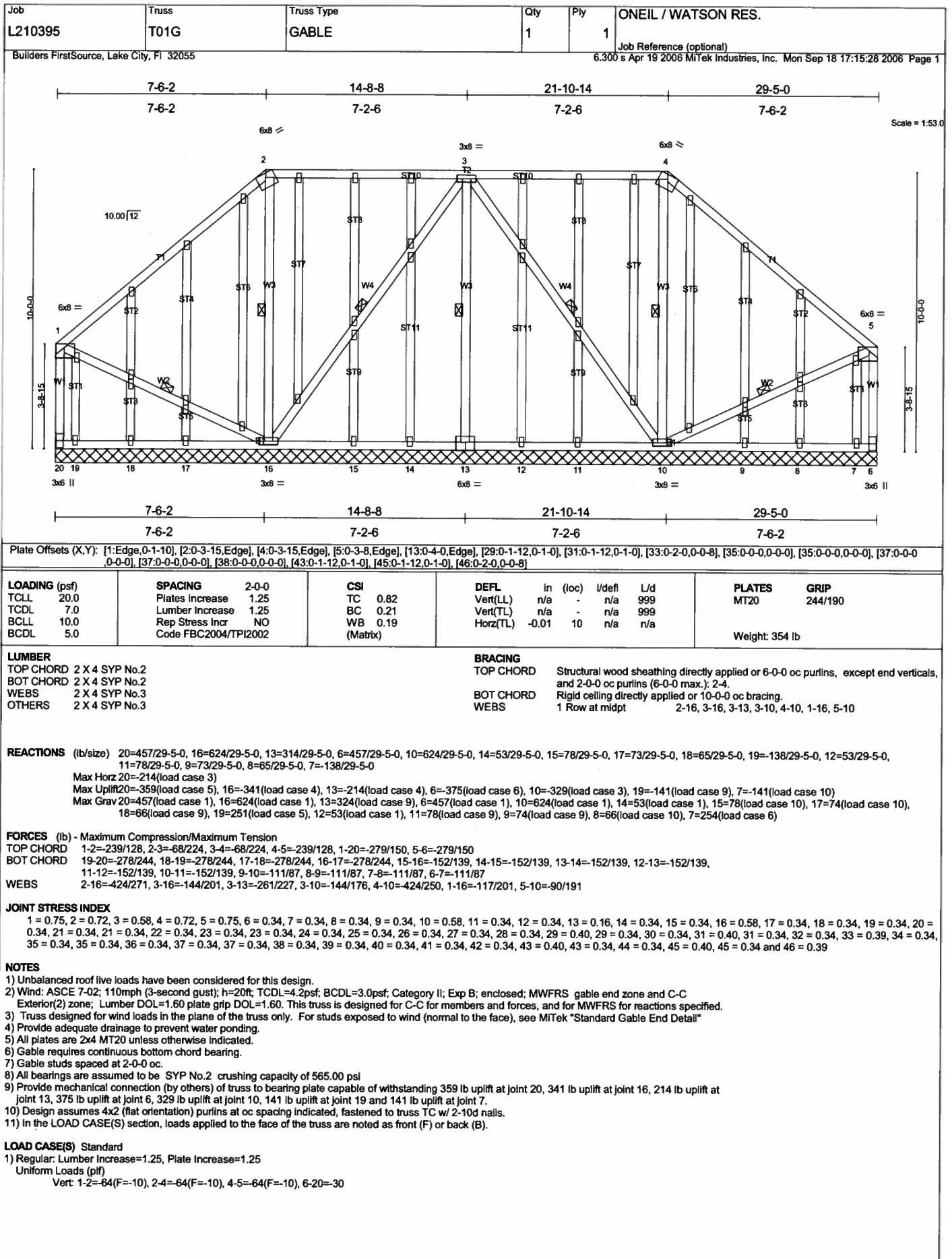
10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

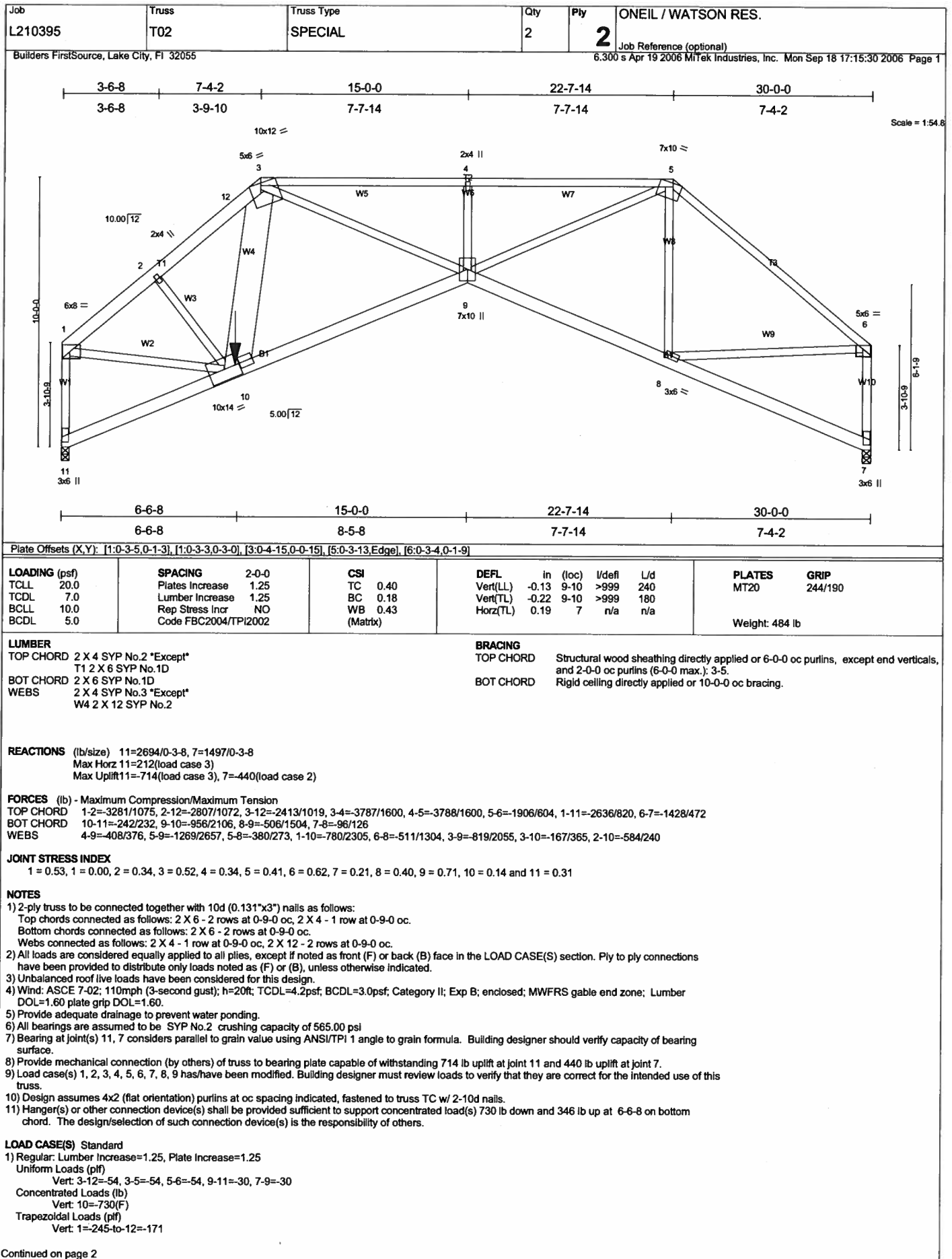
Uniform Loads (plf)

Vert: 3-14=-14, 3-5=-54, 5-7=-54, 8-13=-30

Trapezoidal Loads (plf)

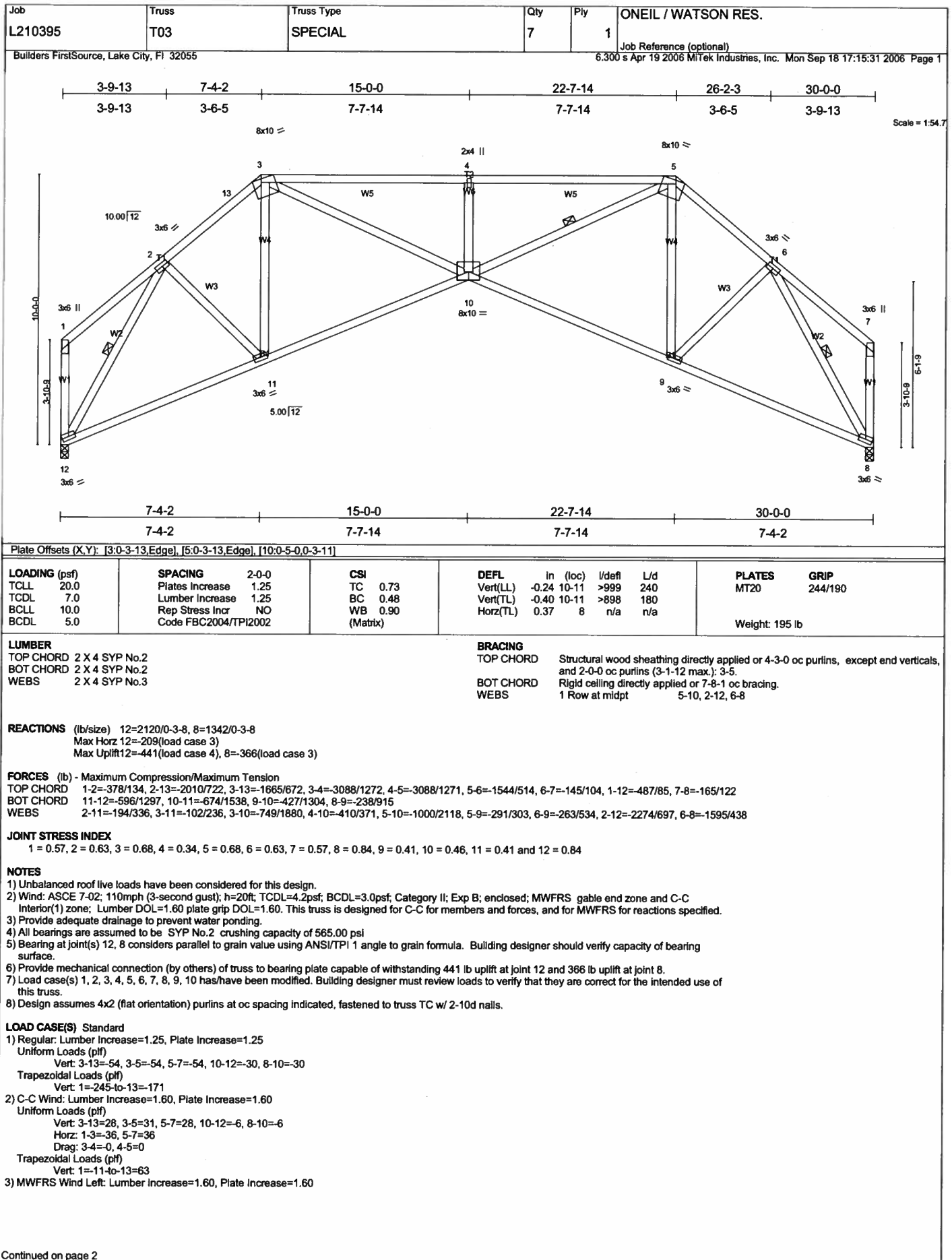
Vert: 1=-205-to-14=-131





Continued on page 2

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T02	SPECIAL	2	2	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:30 2006 Page 2		
LOAD CASE(S) Standard					
2) MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=-12, 3-5=46, 5-6=22, 9-11=-6, 7-9=-6					
Horz: 1-3=4, 5-6=31					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=346(F)					
Trapezoidal Loads (plf)					
Vert: 1=-31-to-12=43					
3) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=22, 3-5=46, 5-6=-12, 9-11=-6, 7-9=-6					
Horz: 1-3=-31, 5-6=-4					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=346(F)					
Trapezoidal Loads (plf)					
Vert: 1=4-to-12=78					
4) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=38, 3-5=24, 5-6=24, 9-11=-6, 7-9=-6					
Horz: 1-3=-46, 5-6=32					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=276(F)					
Trapezoidal Loads (plf)					
Vert: 1=8-to-12=82					
5) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=24, 3-5=24, 5-6=38, 9-11=-6, 7-9=-6					
Horz: 1-3=-32, 5-6=46					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=276(F)					
Trapezoidal Loads (plf)					
Vert: 1=-6-to-12=68					
6) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=18, 3-5=12, 5-6=12, 9-11=-6, 7-9=-6					
Horz: 1-3=-26, 5-6=20					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=102(F)					
Trapezoidal Loads (plf)					
Vert: 1=-40-to-12=34					
7) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60					
Uniform Loads (plf)					
Vert: 3-12=12, 3-5=12, 5-6=18, 9-11=-6, 7-9=-6					
Horz: 1-3=-20, 5-6=26					
Drag: 4-5=0					
Concentrated Loads (lb)					
Vert: 10=102(F)					
Trapezoidal Loads (plf)					
Vert: 1=-46-to-12=28					
8) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25					
Uniform Loads (plf)					
Vert: 3-12=-54, 3-5=-54, 5-6=-14, 9-11=-30, 7-9=-30					
Concentrated Loads (lb)					
Vert: 10=-730(F)					
Trapezoidal Loads (plf)					
Vert: 1=-245-to-12=-171					
9) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25					
Uniform Loads (plf)					
Vert: 3-12=-14, 3-5=-54, 5-6=-54, 9-11=-30, 7-9=-30					
Concentrated Loads (lb)					
Vert: 10=-730(F)					
Trapezoidal Loads (plf)					
Vert: 1=-205-to-12=-131					



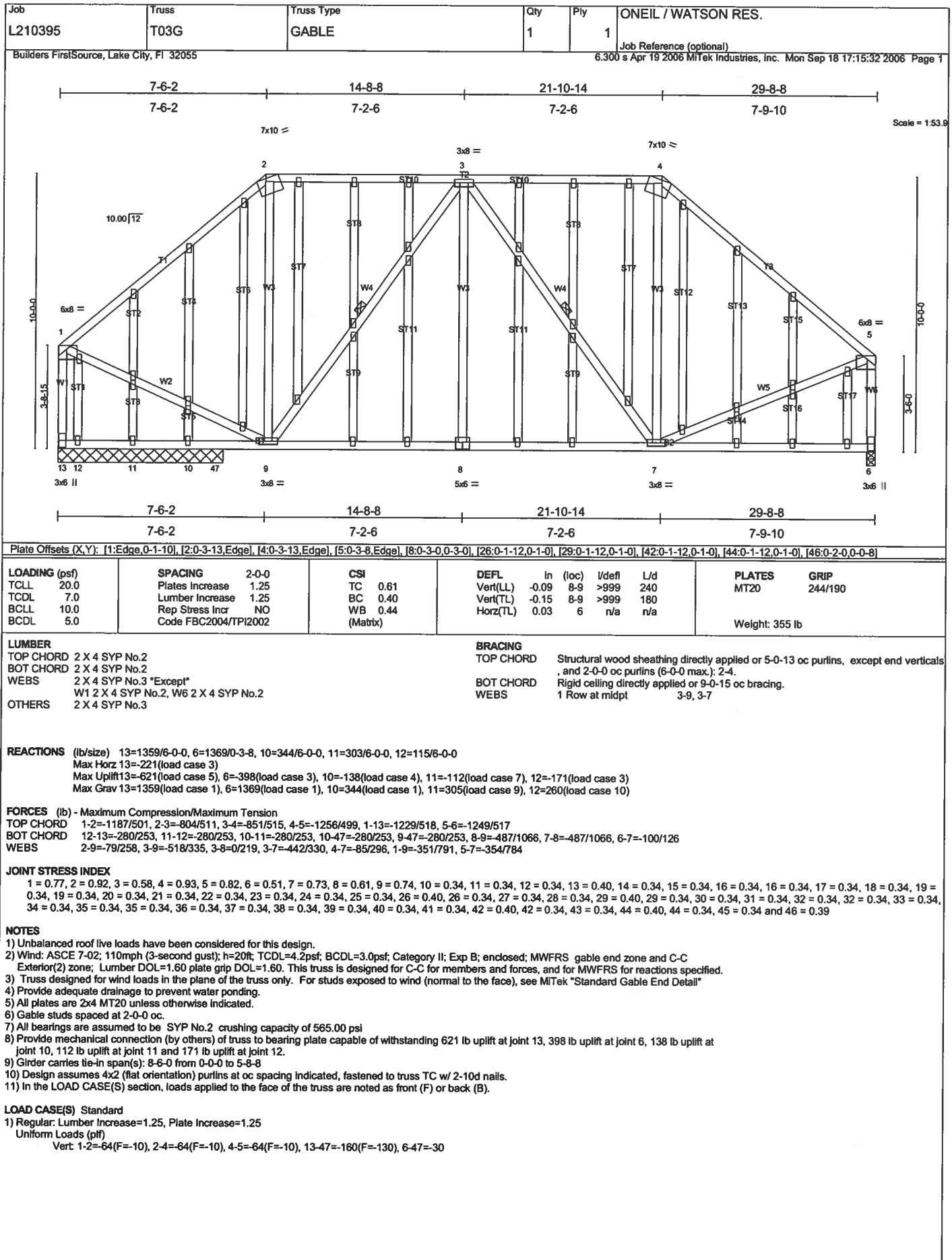
Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T03	SPECIAL	7	1	Job Reference (optional)

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LOAD CASE(S) Standard

- Uniform Loads (plf)
 Vert: 3-13=-12, 3-5=46, 5-7=22, 10-12=-6, 8-10=-6
 Horz: 1-3=4, 5-7=31
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-31-to-13=43
- 4) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-13=22, 3-5=46, 5-7=-12, 10-12=-6, 8-10=-6
 Horz: 1-3=-31, 5-7=-4
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=4-to-13=78
- 5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-13=38, 3-5=24, 5-7=24, 10-12=-6, 8-10=-6
 Horz: 1-3=-46, 5-7=32
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=8-to-13=82
- 6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-13=24, 3-5=24, 5-7=38, 10-12=-6, 8-10=-6
 Horz: 1-3=-32, 5-7=46
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-6-to-13=68
- 7) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-13=18, 3-5=12, 5-7=12, 10-12=-6, 8-10=-6
 Horz: 1-3=-26, 5-7=20
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-40-to-13=34
- 8) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 3-13=12, 3-5=12, 5-7=18, 10-12=-6, 8-10=-6
 Horz: 1-3=-20, 5-7=26
 Drag: 3-4=0, 4-5=0
 Trapezoidal Loads (plf)
 Vert: 1=-46-to-13=28
- 9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 3-13=-54, 3-5=-54, 5-7=-14, 10-12=-30, 8-10=-30
 Trapezoidal Loads (plf)
 Vert: 1=-245-to-13=-171
- 10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 3-13=-14, 3-5=-54, 5-7=-54, 10-12=-30, 8-10=-30
 Trapezoidal Loads (plf)
 Vert: 1=-205-to-13=-131



Job L210395	Truss T04	Truss Type SPECIAL	Qty 2	Ply 1	ONEIL / WATSON RES. Job Reference (optional)
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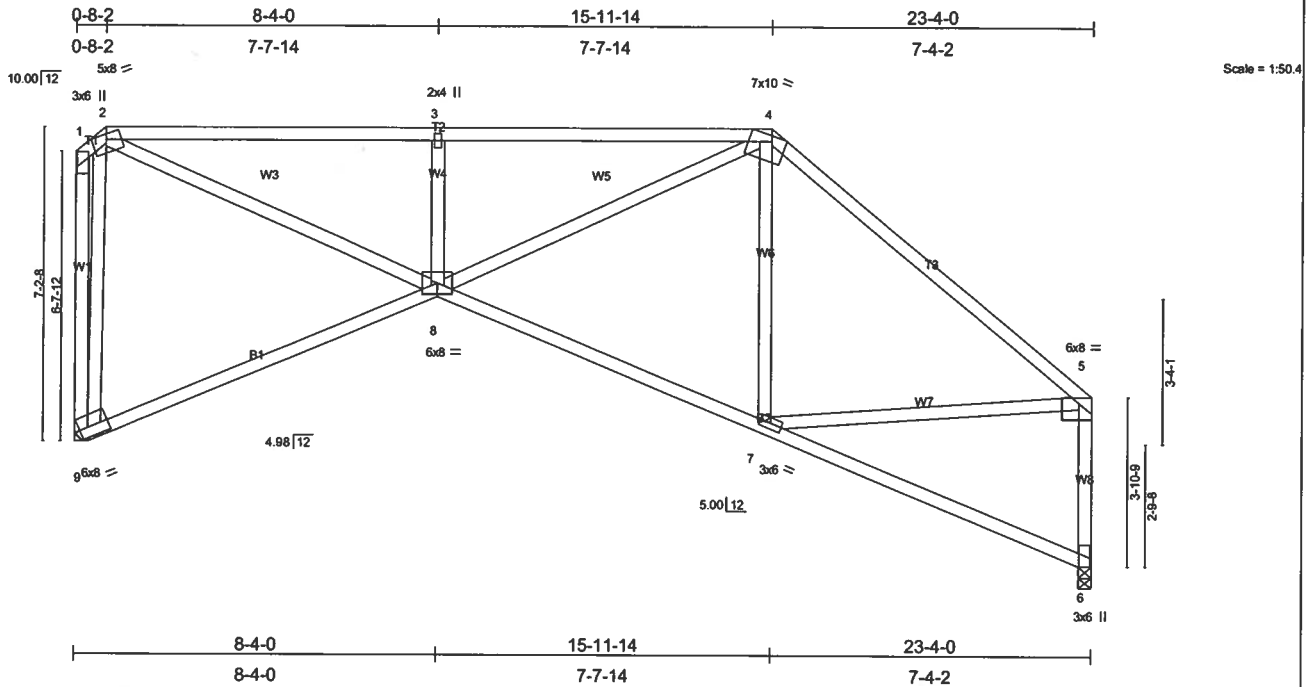


Plate Offsets (X,Y): [4:0-3-13,Edge], [5:0-3-8,Edge], [9:0-2-5,0-2-12]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.74	Vert(LL)	-0.14	8-9	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.35	Vert(TL)	-0.23	8-9	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.81	Horz(TL)	0.13	6	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 150 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 5-6-15 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-1 max.): 2-4.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=968/Mechanical, 6=968/0-3-8
 Max Horz 9=-263(load case 6)
 Max Uplift 9=434(load case 3), 6=-284(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-174/81, 2-3=-1545/622, 3-4=-1546/622, 4-5=-1128/377, 1-9=-262/155, 5-6=-892/315
 BOT CHORD 8-9=-103/266, 7-8=-233/881, 6-7=-79/127
 WEBS 2-8=-611/1579, 3-8=-440/393, 4-8=-481/880, 4-7=-128/162, 5-7=-247/701, 2-9=-1042/664

JOINT STRESS INDEX
 1 = 0.44, 2 = 0.59, 3 = 0.34, 4 = 0.77, 5 = 0.63, 6 = 0.41, 7 = 0.40, 8 = 0.69 and 9 = 0.49

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 9 and 284 lb uplift at joint 6.
- 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T05	COMMON	5	1	Job Reference (optional)

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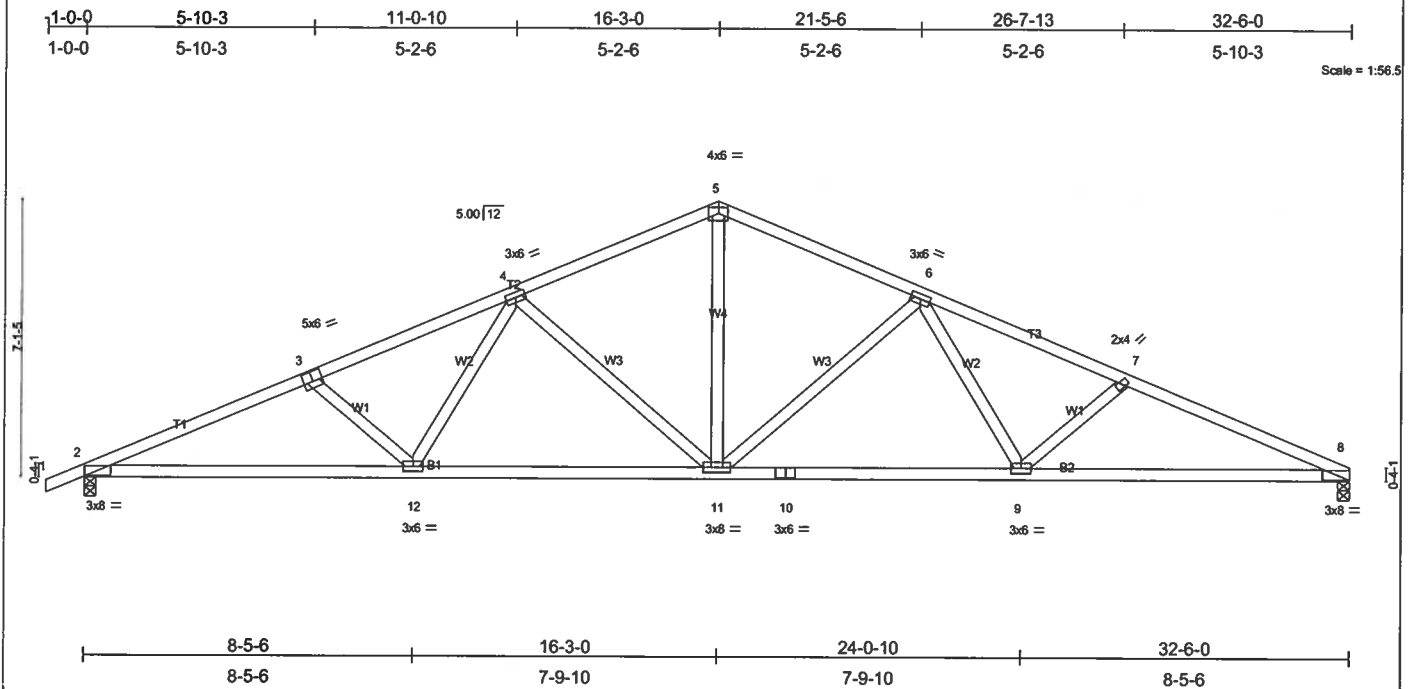


Plate Offsets (X,Y): [2-0-4-2,0-1-8], [3-0-3-0,0-3-0], [8-0-4-2,0-1-8]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES	
TCLL	20.0	Plates Increase	1.25	TC	0.43	in (loc)	l/def	L/d	GRIP
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(LL)	-0.24 11-12	>999	240
BCLL	10.0	Rep Stress Incr	YES	VB	0.56	Vert(TL)	-0.38 11-12	>999	180
BCDL	5.0	Code FBC2004/TPI2002		WB	0.56	Horz(TL)	0.12 8	n/a	n/a
				(Matrix)					
						Weight: 157 lb			

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-3-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-7-13 oc bracing.

REACTIONS (lb/size) 8=1352/0-3-8, 2=1416/0-3-8
Max Horz 2=120(load case 5)
Max Uplift 8=-444(load case 6), 2=-510(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

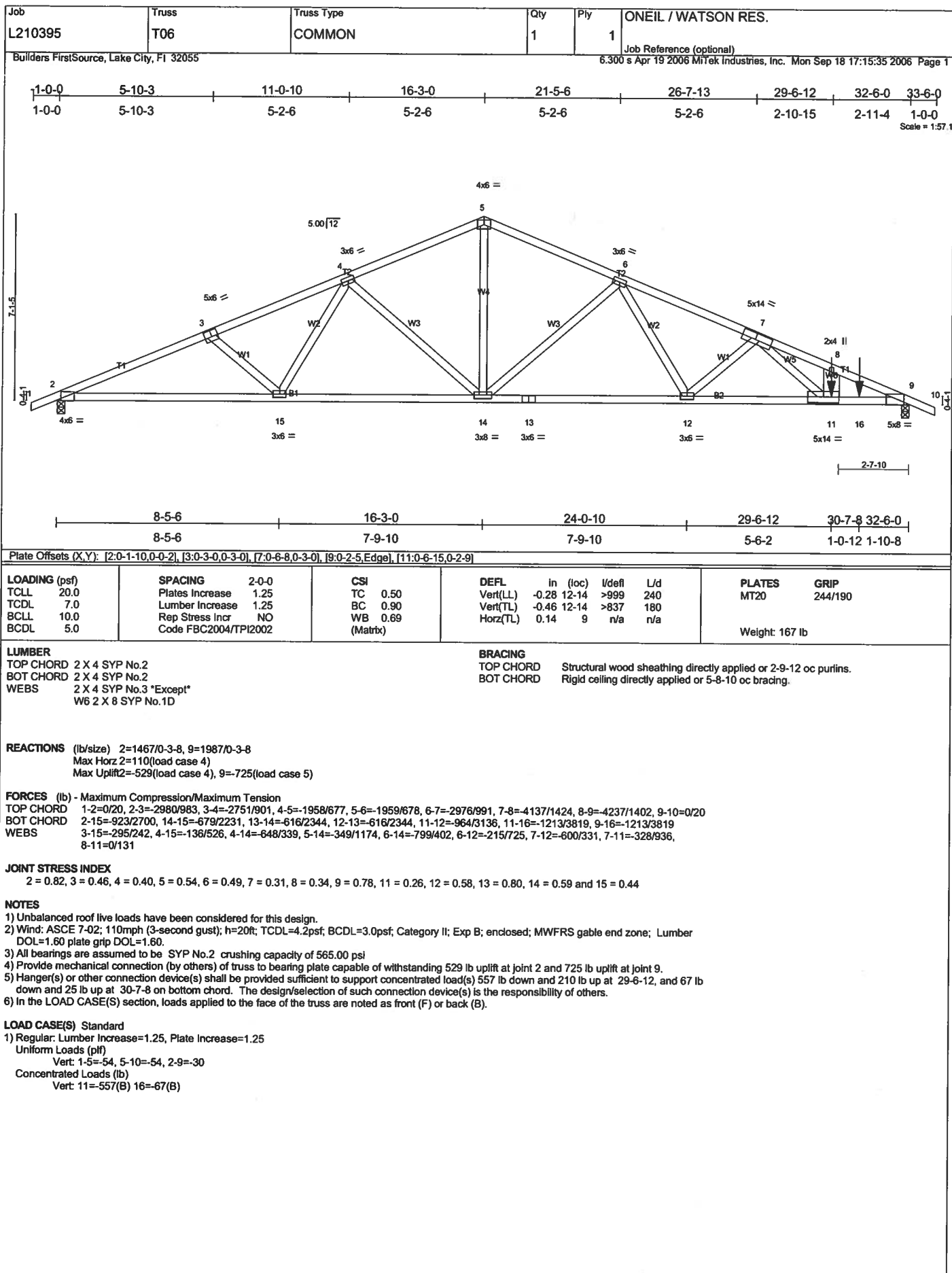
TOP CHORD	1-2=0/20, 2-3=2854/935, 3-4=2624/852, 4-5=1826/631, 5-6=1826/629, 6-7=2631/862, 7-8=2867/952
BOT CHORD	3-12=889/2584, 11-12=644/2111, 10-11=554/2114, 9-10=554/2114, 8-9=812/2599
WEBS	1-2=297/724, 4-12=136/531, 4-11=651/339, 5-11=311/1070, 6-11=655/344, 6-9=142/535, 7-9=305/251

JOINT STRESS INDEX
2 = 0.80, 3 = 0.46, 4 = 0.40, 5 = 0.49, 6 = 0.40, 7 = 0.34, 8 = 0.79, 9 = 0.44, 10 = 0.72, 11 = 0.57 and 12 = 0.44

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 8 and 510 lb uplift at joint 2.

LOAD CASE(S) Standard



Job L210395	Truss T07	Truss Type COMMON	Qty 1	Ply 1	ONEIL / WATSON RES.
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Builders FirstSource, Lake City, FL 32055

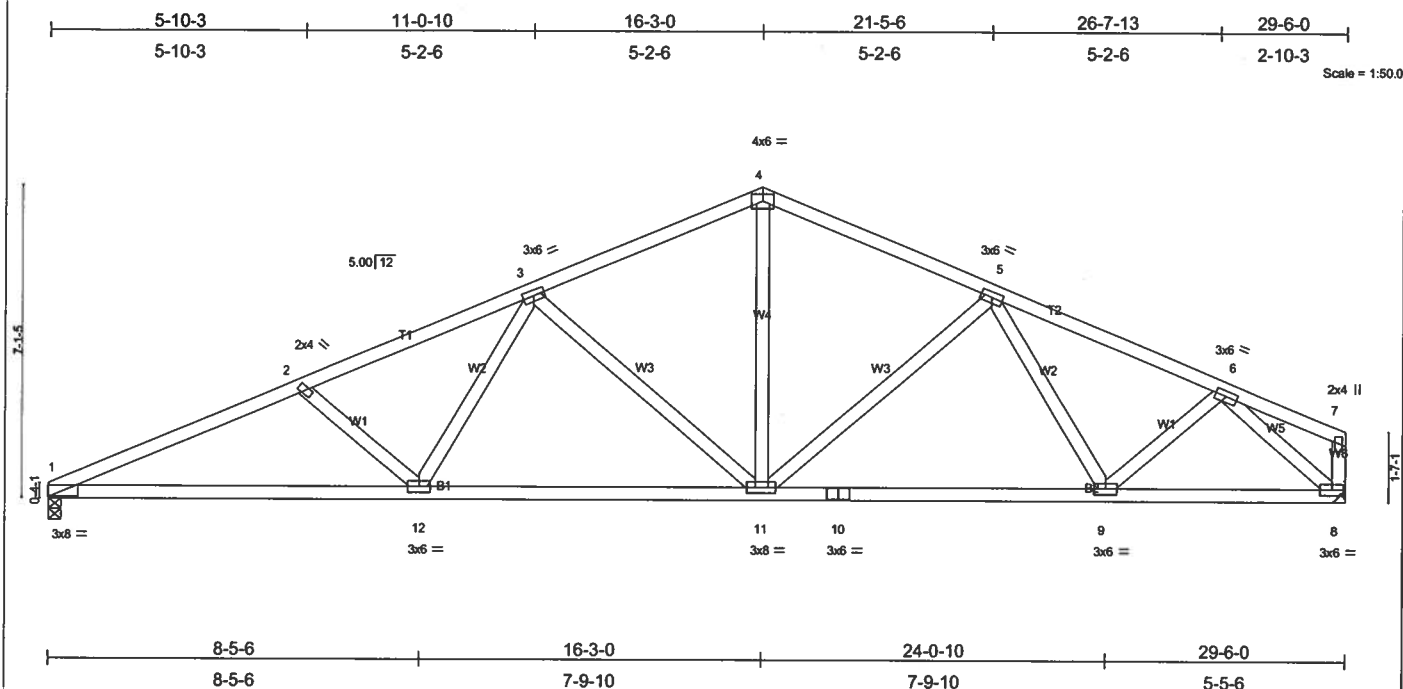
Job Reference (optional)
6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:36 2006 Page 1

Plate Offsets (X,Y): [1:0-4-2,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	In	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	Vert(LL)	-0.20	1-12	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.62	Vert(TL)	-0.33	1-12	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.56	Horz(TL)	0.08	8	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 153 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-7-14 oc bracing.

REACTIONS (lb/size) 1=1227/0-3-8, 8=1227/Mechanical
 Max Horz 1=139(load case 5)
 Max Uplift 1=410(load case 5), 8=391(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2554/867, 2-3=-2315/776, 3-4=-1508/530, 4-5=-1510/544, 5-6=-1681/547, 6-7=-103/30, 7-8=-88/37
 BOT CHORD 1-12=-872/2312, 11-12=-613/1822, 10-11=-426/1535, 9-10=-426/1535, 8-9=-396/1214
 WEBS 2-12=-309/252, 3-12=-143/535, 3-11=-656/346, 4-11=-246/834, 5-11=-322/222, 5-9=-64/89, 6-9=-50/387, 6-8=-1562/532

JOINT STRESS INDEX

1 = 0.70, 2 = 0.34, 3 = 0.40, 4 = 0.40, 5 = 0.40, 6 = 0.47, 7 = 0.47, 8 = 0.49, 9 = 0.44, 10 = 0.54, 11 = 0.57 and 12 = 0.44

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 1 and 391 lb uplift at joint 8.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T08	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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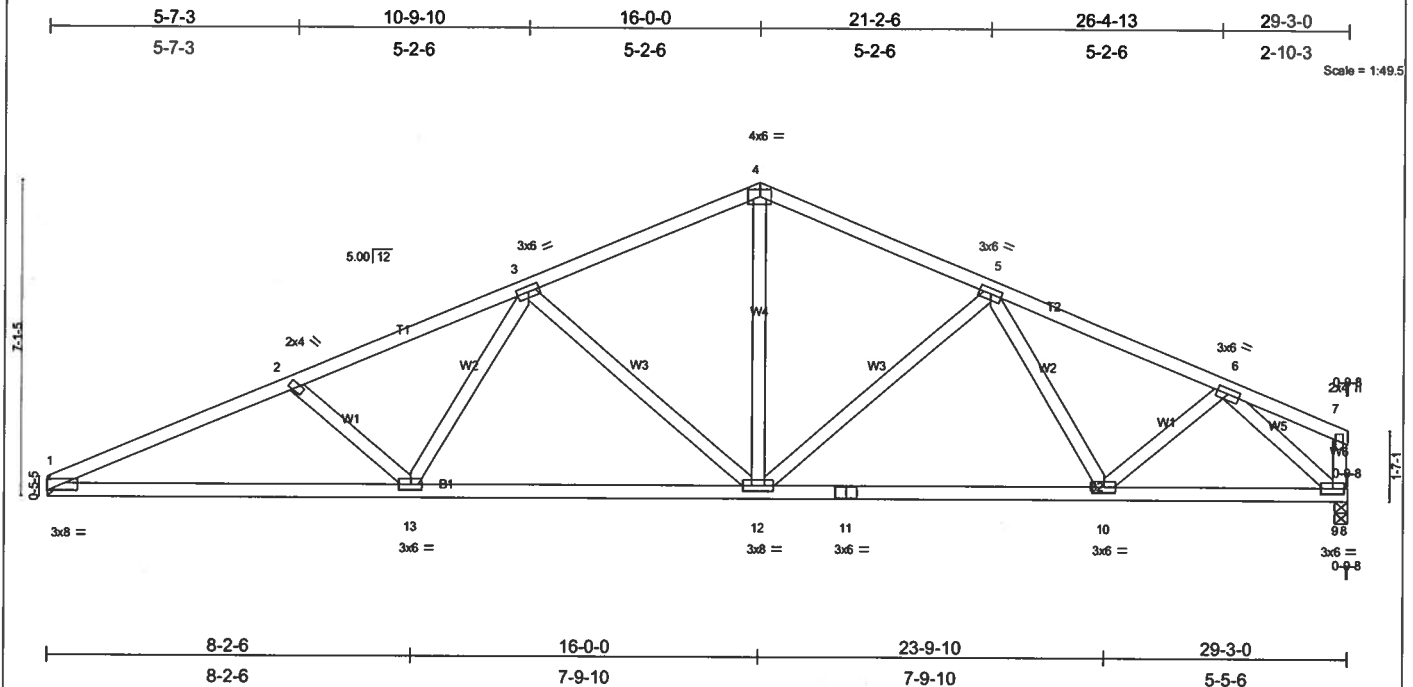


Plate Offsets (X,Y): [1:0-0-2,0-0-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.37	Vert(LL)	-0.18	1-13	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.60	Vert(TL)	-0.30	1-13	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.54	Horz(TL)	0.08	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-7-4 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-11-6 oc bracing.

REACTIONS (lb/size) 1=1214/Mechanical, 9=1220/0-3-8
Max Horiz 1=107(load case 5)
Max Uplift1=-406(load case 5), 9=-386(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=2453/834, 2-3=2240/753, 3-4=1482/524, 4-5=1484/537, 5-6=1651/544, 6-7=97/50, 7-9=84/42
BOT CHORD 1-13=805/2203, 12-13=569/1777, 11-12=392/1509, 10-11=392/1509, 9-10=364/1184, 8-9=0/0
WEBS 2-13=280/232, 3-13=128/492, 3-12=630/337, 4-12=241/814, 5-12=319/224, 5-10=67/86, 6-10=47/391, 6-9=1538/515

JOINT STRESS INDEX
1 = 0.74, 2 = 0.34, 3 = 0.40, 4 = 0.40, 5 = 0.40, 6 = 0.47, 7 = 0.44, 9 = 0.48, 10 = 0.44, 11 = 0.56, 12 = 0.57 and 13 = 0.44

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 406 lb uplift at joint 1 and 386 lb uplift at joint 9.

LOAD CASE(S) Standard

Job L210395	Truss T09	Truss Type COMMON	Qty 1	Ply 2	ONEIL / WATSON RES. Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:38 2006 Page 1		

5-7-3 10-9-10 16-0-0 21-2-6 26-4-13 29-3-0

5-7-3 5-2-6 5-2-6 5-2-6 5-2-6 2-10-3

Scale = 1:49.5

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.61	Vert(LL) -0.20 1-13 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.32	Vert(TL) -0.33 1-13 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.04 9 n/a n/a		
	Code FBC2004/TP12002			Weight: 351 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-7 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2872/Mechanical, 9=1962/0-3-8
 Max Horz 1=106(load case 4)
 Max Uplift 1=800(load case 4), 9=254(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-5091/1256, 2-14=-4844/1141, 3-14=-4777/1173, 3-4=-2570/480, 4-5=-2574/493, 5-6=-2760/441, 6-7=-179/55, 7-9=-146/30
 BOT CHORD 1-15=-1206/4656, 13-15=-1206/4656, 12-13=-699/3533, 11-12=-275/2568, 10-11=-275/2568, 9-10=-249/2009, 8-9=0/0
 WEBS 2-13=-327/263, 3-13=-617/1773, 3-12=-1667/579, 4-12=-282/1423, 5-12=-432/139, 5-10=-204/72, 6-10=-54/602, 6-9=-2552/351

JOINT STRESS INDEX
 1 = 0.82, 2 = 0.34, 3 = 0.59, 4 = 0.35, 5 = 0.40, 6 = 0.42, 7 = 0.38, 9 = 0.30, 10 = 0.44, 11 = 0.40, 12 = 0.57 and 13 = 0.69

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 800 lb uplift at joint 1 and 254 lb uplift at joint 9.
- Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- Girder carries tie-in span(s): 12-10-8 from 0-10-8 to 8-3-0

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-14=-54, 4-14=-91, 4-7=-91, 1-15=-30, 13-15=-252(B=-222), 8-13=-30
- MWFRS Wind Left: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-14=25, 4-14=8, 4-7=5, 1-15=-6, 13-15=78(B=84), 8-13=-6
 Horz: 1-4=-33, 4-7=31, 7-9=24
- MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60
 Uniform Loads (plf)
 Vert: 1-14=22, 4-14=5, 4-7=8, 1-15=-6, 13-15=78(B=84), 8-13=-6
 Horz: 1-4=-31, 4-7=33, 7-9=19
- MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T09	COMMON	1	2	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Sep 18 17:15:38 2006 Page 2		

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-14=38, 4-14=21, 4-7=7, 1-15=-6, 13-15=78(B=84), 8-13=-6

Horz: 1-4=-46, 4-7=32, 7-9=23

5) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-14=24, 4-14=7, 4-7=21, 1-15=-6, 13-15=78(B=84), 8-13=-6

Horz: 1-4=-32, 4-7=46, 7-9=23

6) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-14=18, 4-14=1, 4-7=-5, 1-15=-6, 13-15=78(B=84), 8-13=-6

Horz: 1-4=-26, 4-7=20, 7-9=23

7) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-14=12, 4-14=-5, 4-7=1, 1-15=-6, 13-15=78(B=84), 8-13=-6

Horz: 1-4=-20, 4-7=26, 7-9=23

8) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-14=-54, 4-14=-91, 4-7=-51, 1-15=-30, 13-15=-252(B=-222), 8-13=-30

9) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-14=-14, 4-14=-51, 4-7=-91, 1-15=-30, 13-15=-252(B=-222), 8-13=-30

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T10	SCISSOR	6	1	

Builders FirstSource, Lake City, FL 32055

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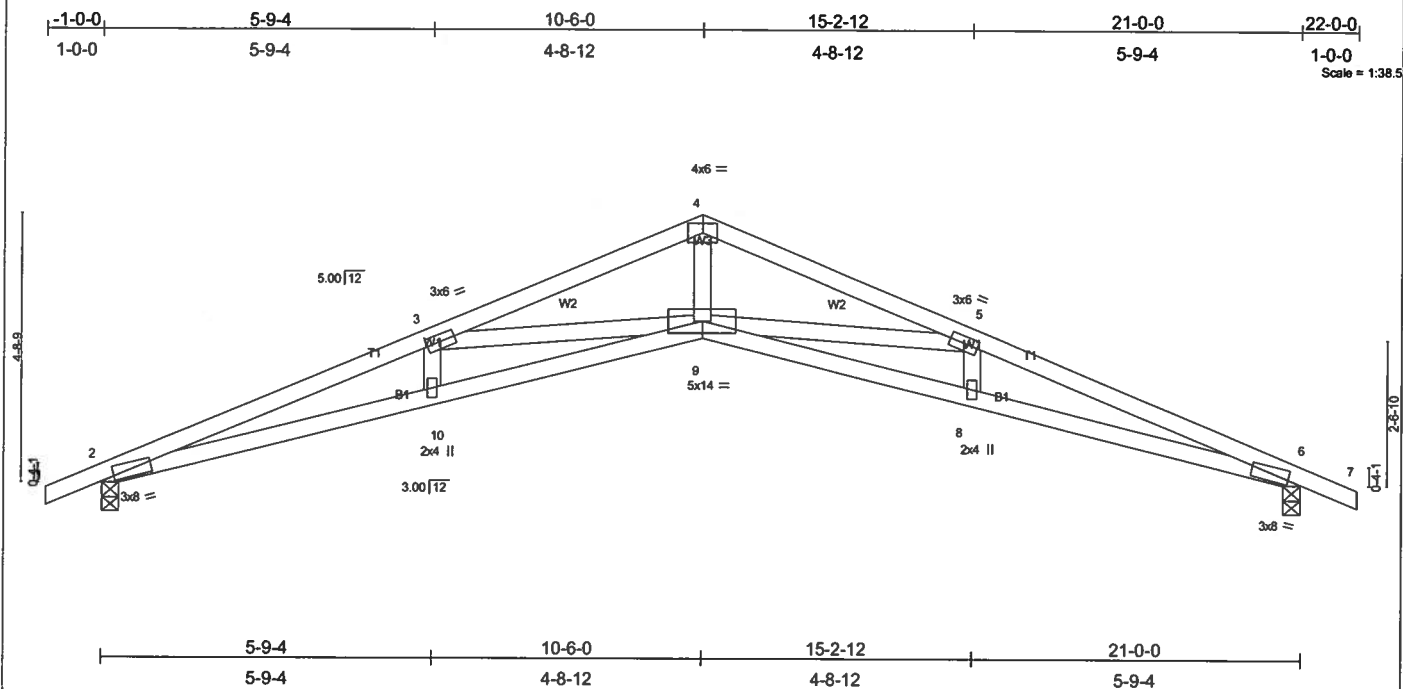


Plate Offsets (X,Y): [2:0-2-12,0-0-9], [6:0-2-12,0-0-9]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.50	In (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.84	Vert(LL) -0.43 9-10 >573 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.58	Vert(TL) -0.69 9-10 >358 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.46 6 n/a n/a		
	Code FBC2004/TPI2002			Weight: 88 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-9-4 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-14 oc bracing.

REACTIONS (lb/size) 2=932/0-3-8, 6=932/0-3-8
 Max Horz 2=76(load case 5)
 Max Uplift 2=351(load case 5), 6=351(load case 6)

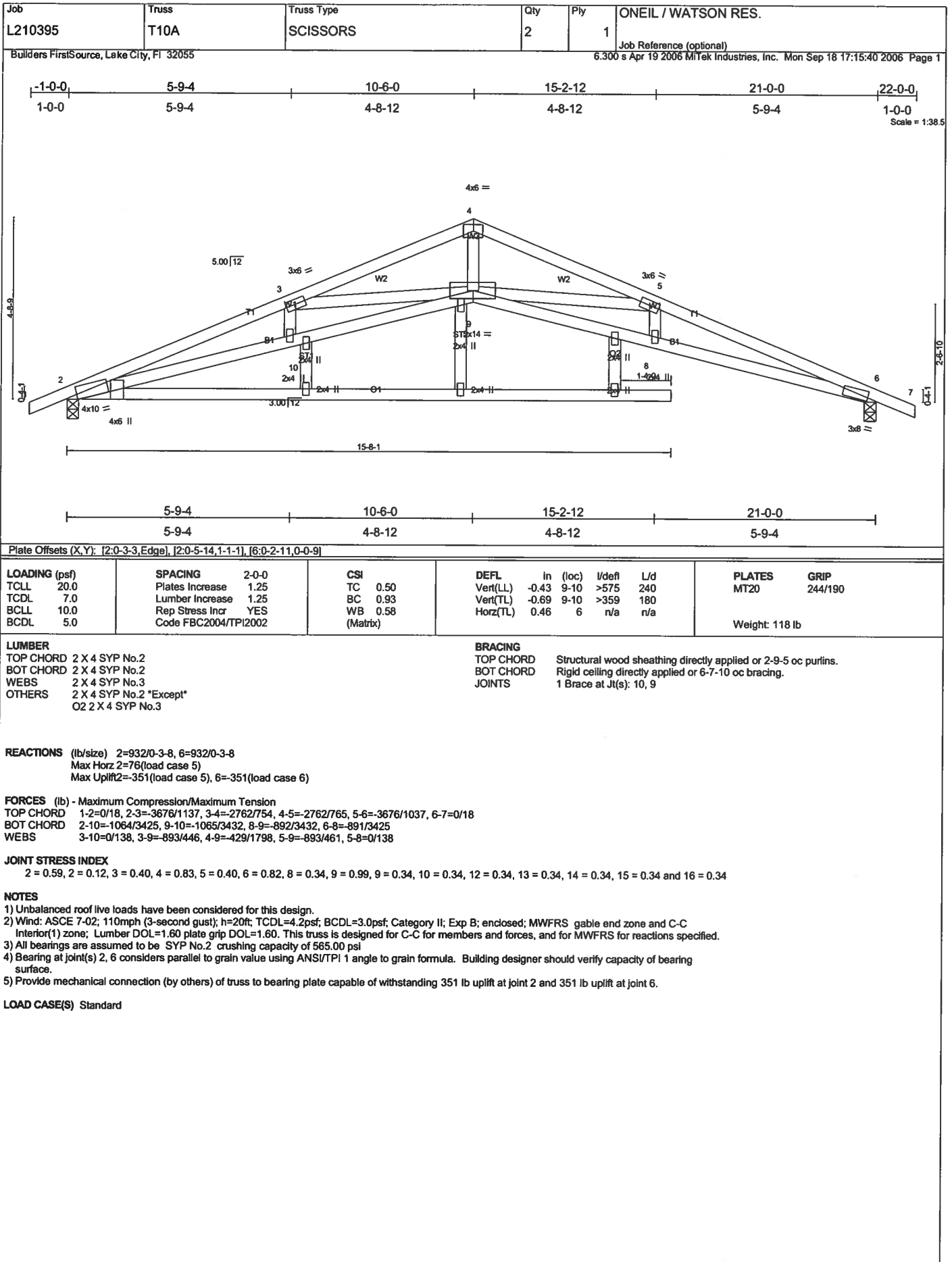
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/18, 2-3=-3680/1138, 3-4=-2765/754, 4-5=-2765/766, 5-6=-3680/1038, 6-7=0/18
 BOT CHORD 2-10=-1065/3430, 9-10=-1066/3437, 8-9=-893/3437, 6-8=-892/3430
 WEBS 3-10=0/138, 3-9=-895/447, 4-9=-429/1800, 5-9=-895/462, 5-8=0/138

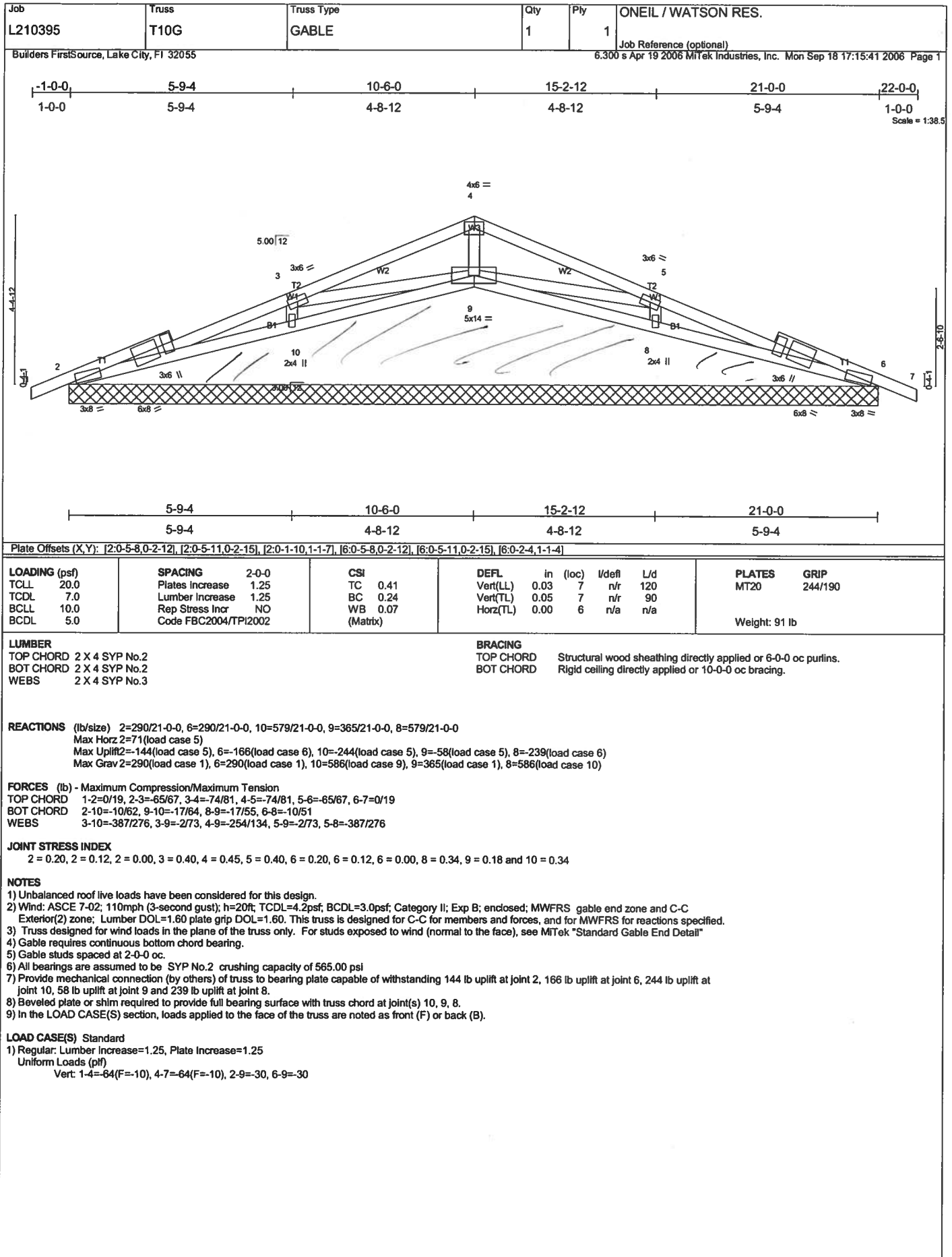
JOINT STRESS INDEX
 2 = 0.82, 3 = 0.40, 4 = 0.83, 5 = 0.40, 6 = 0.82, 8 = 0.34, 9 = 0.99 and 10 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 2 and 351 lb uplift at joint 6.

LOAD CASE(S) Standard



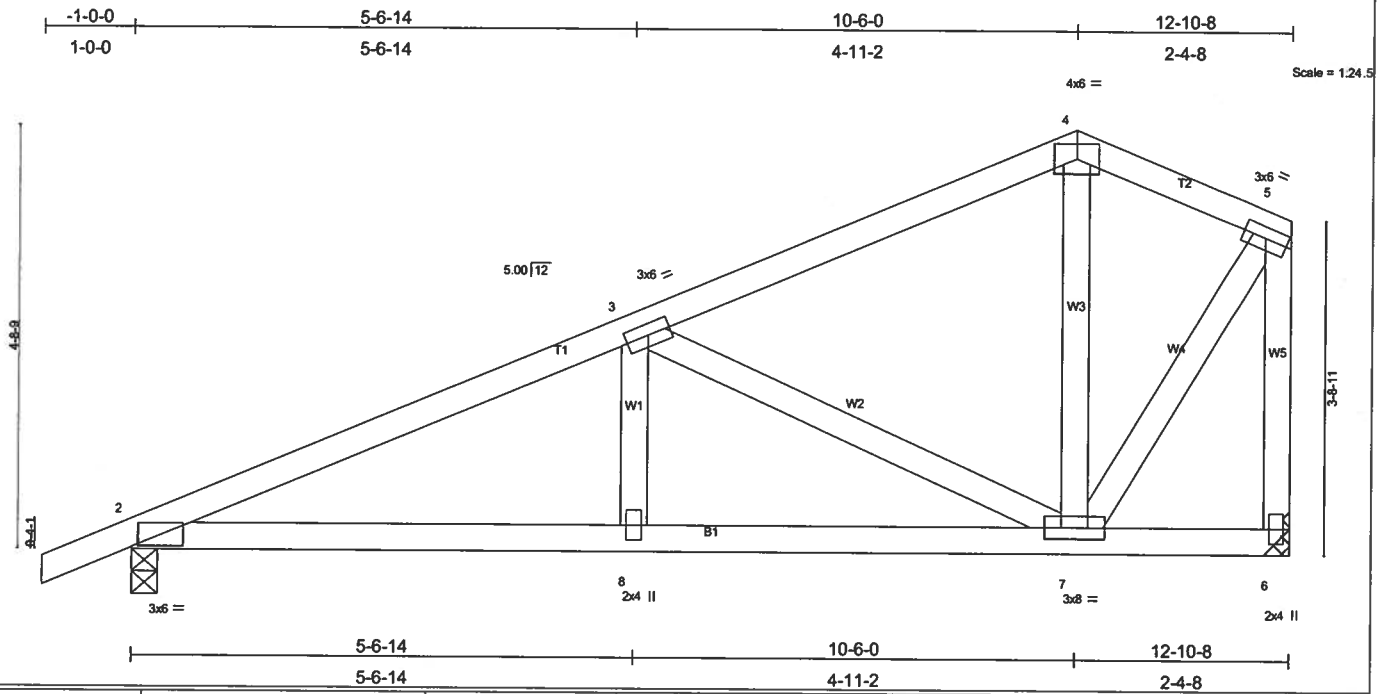


Job: L210395 Truss: T11 Truss Type: COMMON Qty: 1 Ply: 2 ONEIL / WATSON RES. Job Reference (optional): 6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Sep 18 17:15:42 2006 Page 1

Builders FirstSource, Lake City, FL 32055

Scale = 1:36.4

Job	Truss	Truss Type	Qty	Ply	ONEIL / WATSON RES.
L210395	T12	COMMON	4	1	
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional)
					6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:43 2006 Page 1



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.19	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.29	Vert(LL) -0.04 2-8 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.31	Vert(TL) -0.07 2-8 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 6 n/a n/a		
	Code FBC2004/TPI2002			Weight: 69 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=593/0-3-8, 6=526/Mechanical
 Max Horz 2=198(load case 5)
 Max Uplift 2=238(load case 5), 6=194(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/20, 2-3=899/255, 3-4=320/98, 4-5=-275/113, 5-6=-504/194
 BOT CHORD 2-8=-346/777, 7-8=-346/777, 6-7=-3/6
 WEBS 3-8=0/182, 3-7=-594/277, 4-7=-9/59, 5-7=-179/444

JOINT STRESS INDEX
 2 = 0.37, 3 = 0.17, 4 = 0.27, 5 = 0.38, 6 = 0.18, 7 = 0.48 and 8 = 0.13

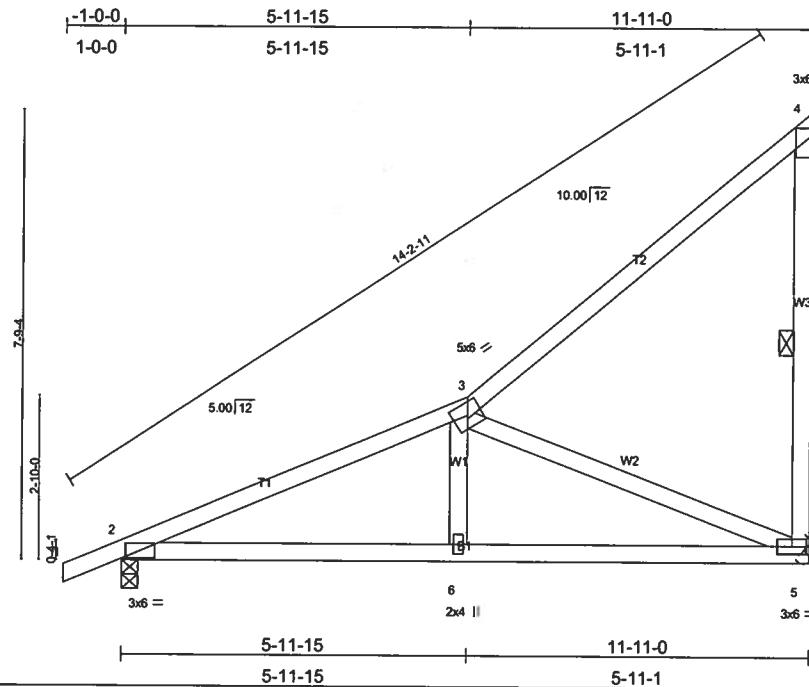
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 194 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L210395	Truss T13	Truss Type PORCH TRUSS	Qty 10	Ply 1	ONEIL / WATSON RES.
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Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)
6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:44 2006 Page 1

Scale = 1/32

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.32	Vert(LL) -0.05 2-6 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.31	Vert(TL) -0.07 2-6 >999 180		
BCCL 10.0	Rep Stress Incr YES	WB 0.50	Horz(TL) 0.01 5 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 64 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 4-5

REACTIONS (lb/size) 5=485/Mechanical, 2=553/0-3-8
 Max Horz 2=362/load case 5)
 Max Uplift 5=-287/load case 5), 2=-146/load case 5)

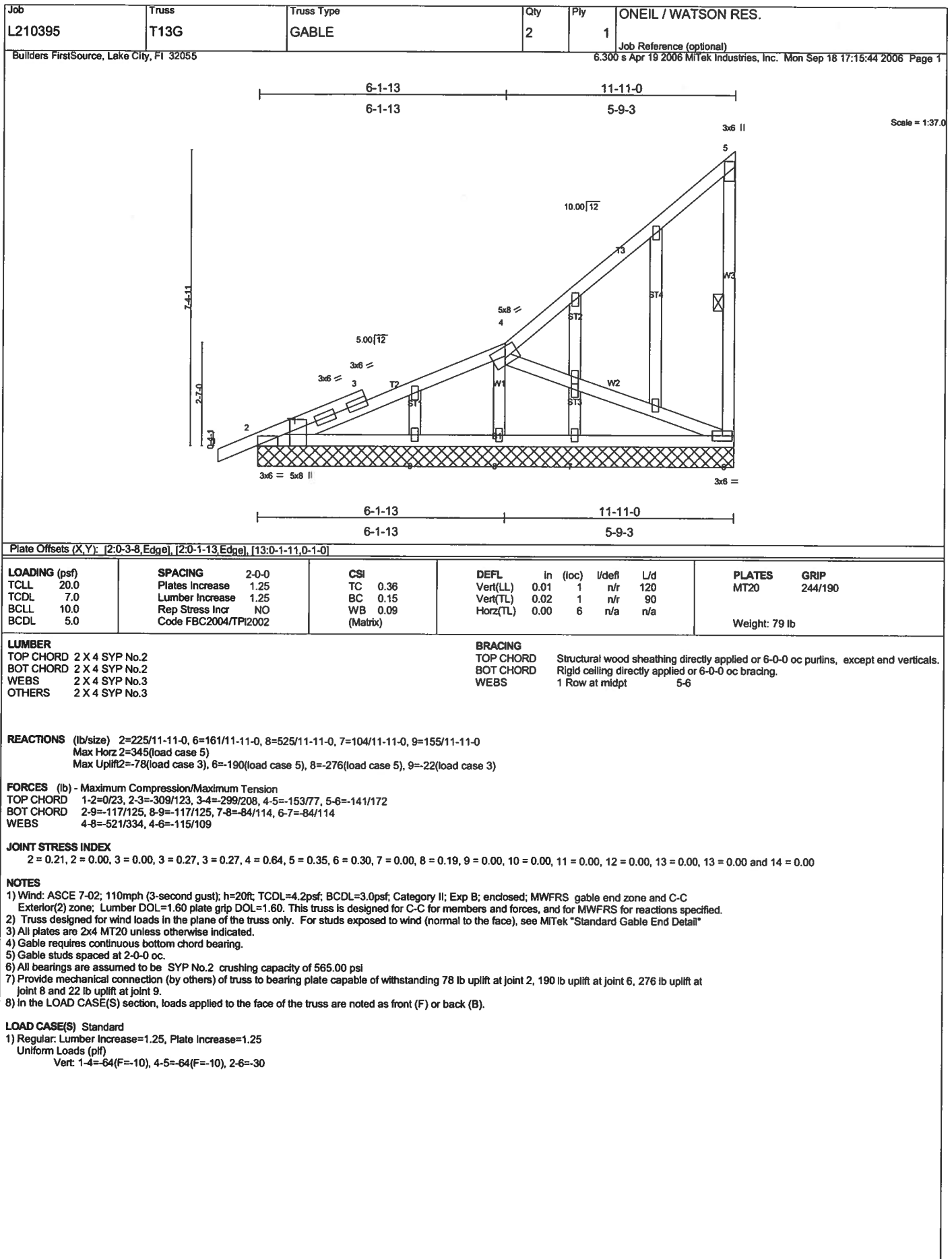
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/20, 2-3=791/34, 3-4=-135/64, 4-5=-122/169
 BOT CHORD 2-6=-305/677, 5-6=-308/666
 WEBS 3-6=0/204, 3-5=-706/318

JOINT STRESS INDEX
 2 = 0.37, 3 = 0.66, 4 = 0.34, 5 = 0.29 and 6 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 5 and 146 lb uplift at joint 2.

LOAD CASE(S) Standard



Job L210395	Truss T14	Truss Type SPECIAL	Qty 10	Ply 1	ONEIL / WATSON RES. Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Sep 18 17:15:45 2006 Page 1

Scale = 1:30.9

Plate Offsets (X,Y): [2:0-3-0,0-1-10], [4:5-1-0,6-8-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.29	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.20	Vert(LL) -0.04 2-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.14	Vert(TL) -0.07 2-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.05 6 n/a n/a		
	Code FBC2004/TPI2002			Weight: 74 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-4-6 oc bracing.

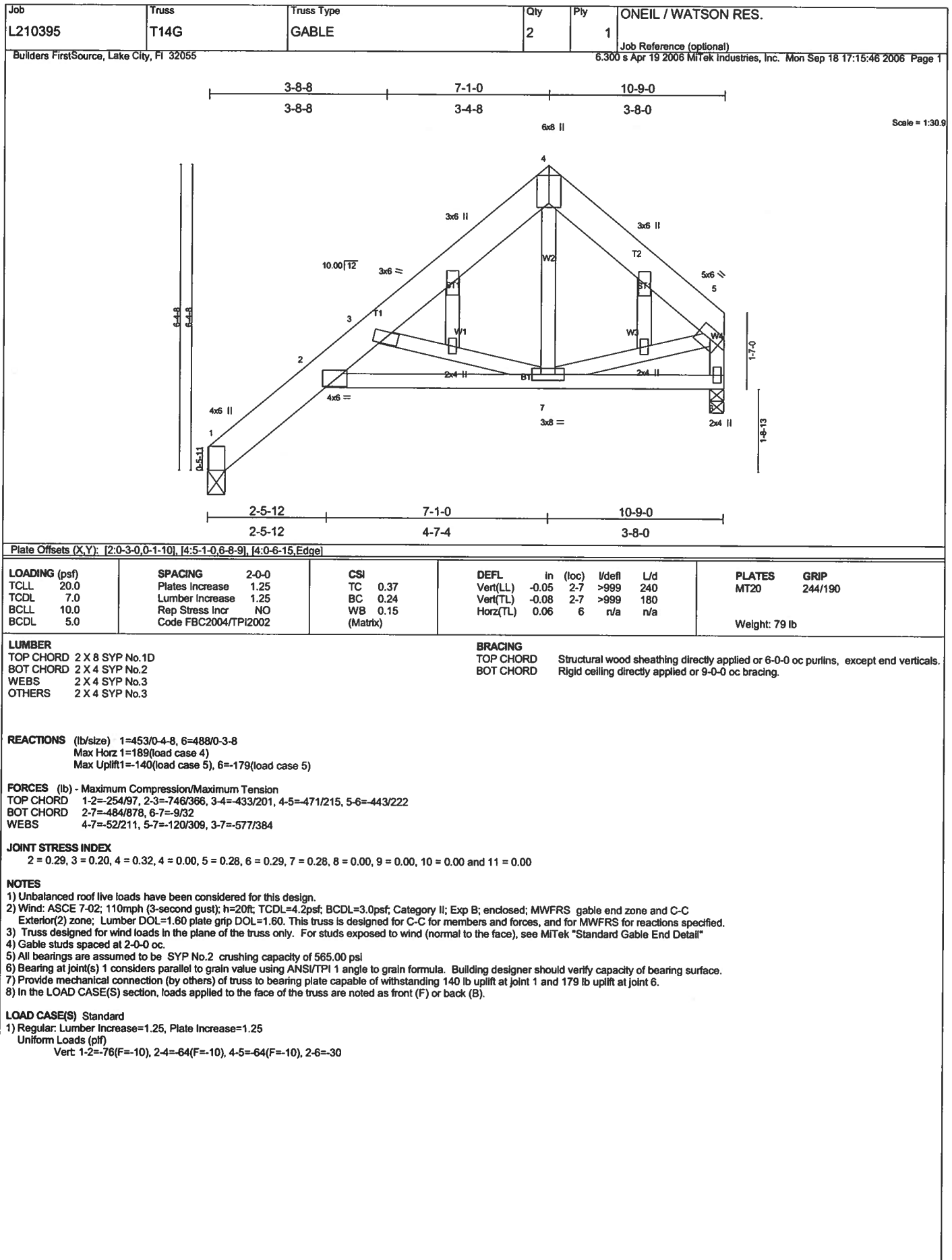
REACTIONS (lb/size) 1=401/0-4-8, 6=436/0-3-8
Max Horz 1=189(load case 4)
Max Uplift 1=120(load case 5), 6=159(load case 5)

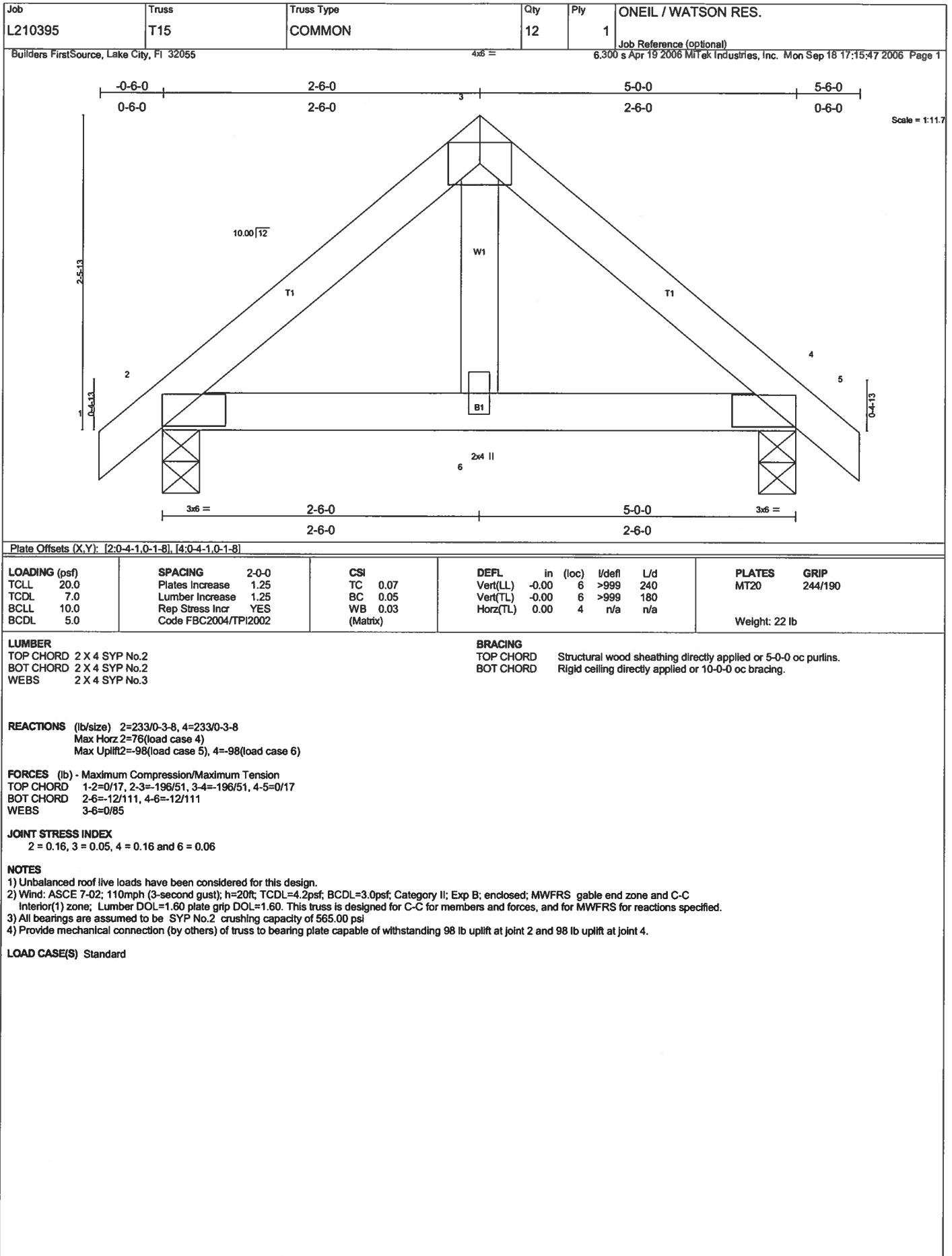
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-225/94, 2-3=-659/293, 3-4=-384/156, 4-5=-418/184, 5-6=-390/169
BOT CHORD 2-7=-448/780, 6-7=-9/30
WEBS 4-7=-46/202, 5-7=-107/274, 3-7=-512/360

JOINT STRESS INDEX
2 = 0.26, 3 = 0.18, 4 = 0.17, 4 = 0.00, 5 = 0.25, 6 = 0.27 and 7 = 0.26

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 1 and 159 lb uplift at joint 6.

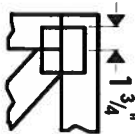
LOAD CASE(S) Standard



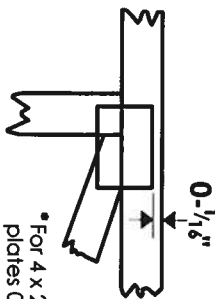


Symbols

PLATE LOCATION AND ORIENTATION



- Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



- For 4 x 2 orientation, locate plates 0-1/16\" from outside edge of truss.



- This symbol indicates the required direction of slots in connector plates.

- Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

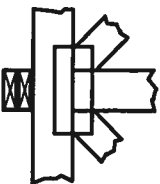
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

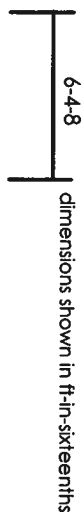


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

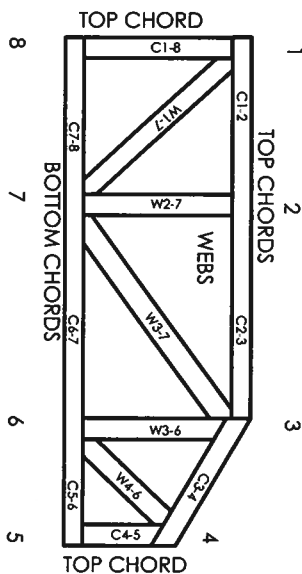
Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 96048, 9511, 9432A



Mitek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
- Never exceed the design loading shown and never stock materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP11.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Install and load vertically unless indicated otherwise.

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(3) HGUS26-2
(21) HTU26
(11) IUT412

