DATE 10/20/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023741

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER DAVID & JUDITH ONORATI PHONE 752-4072

ADDRESS 198 SW STILLVIEW GLEN FT. WHITE FL 32038

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY 47S, TL ON HERLONG, TR ON SKYLINE LOOP, TL ON STILLVIEW, 2ND LOT ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 111300.00

HEATED FLOOR AREA 2226.00 TOTAL AREA 3865.00 HEIGHT .00 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT 23

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 15-4S-16-03815-120 SUBDIVISION CARDINAL FARMS

LOT 20 BLOCK PHASE UNIT TOTAL ACRES 10.00

000000853 (Culvert Permit No.) | (Culvert Waiver) | CGC036224 (Contractor's License Number) | Katie Reed (Applicant/Owner/Contractor)

WAIVER (Driveway Connection) | 05-0963-N (Septic Tank Number) | BK (LU & Zoning checked by) | JH (Approved for Issuance) | Y (New Resident)

COMMENTS: ONE FOOT ABOVE THE ROAD,NOC ON FILE

Check # or Cash 4411

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 560.00 CERTIFICATION FEE $ 19.32 SURCHARGE FEE $ 19.32

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 673.64

INSPECTORS OFFICE CLERKS OFFICE CN

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Project Information for: L128901
Builder: Don Reed Const. Date: 8/31/2005
Lot: Lot 20 Start Number: 492
Subdivision: Cardinal Farms
County or City: Columbia County
Truss Page Count: 28

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2001
Roof (psf):	42	Wind Standard:	ASCE 7-98		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)
REED, LARRY DON CGC 036224
Address: 2230 E BAYA AVE. STE 101
GLEN ST MARY FL 32040 Designer: 83

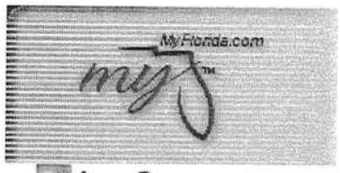
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	083105492	8/31/2005				
2	CJ3	083105493	8/31/2005				
3	CJ5	083105494	8/31/2005				
4	D01	083105495	8/31/2005				
5	D02	083105496	8/31/2005				
6	D03	083105497	8/31/2005				
7	D04	083105498	8/31/2005				
8	EJ7	083105499	8/31/2005				
9	HGBL01	083105500	8/31/2005				
10	HGBL02	083105501	8/31/2005				
11	HJ7	083105502	8/31/2005				
12	T01	083105503	8/31/2005				
13	T01G	083105504	8/31/2005				
14	T01G	083105505	8/31/2005				
15	T02	083105506	8/31/2005				
16	T02G	083105507	8/31/2005				
17	T03	083105508	8/31/2005				
18	T04	083105509	8/31/2005				
19	T05	083105510	8/31/2005				
20	T06	083105511	8/31/2005				
21	T07	083105512	8/31/2005				
22	T07A	083105513	8/31/2005				
23	T08	083105514	8/31/2005				
24	T09	083105515	8/31/2005				
25	T09G	083105516	8/31/2005				
26	T10	083105517	8/31/2005				
27	T10G	083105518	8/31/2005				
28	T11	083105519	8/31/2005				

AUG 31 2005

Dwg.#083105491



Licensee Details

Licensee Information

Name:	**REED, LARRY DON (Primary Name)**
	DON REED CONSTRUCTION INC (DBA Name)
Main Address:	**2230 E BAYA AVE STE 101**
	LAKE CITY Florida 32025
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2230 E BAYA AVE STE 101**
	LAKE CITY FL 32025
County:	**COLUMBIA**

License Information

License Type:	**Certified General Contractor**
Rank:	**Cert General**
License Number:	**CGC036224**
Status:	**Current,Active**
Licensure Date:	**03/08/1986**
Expires:	**08/31/2006**
Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**08/13/2004**

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View License Complaint

| Terms of Use | | Privacy Statement |

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105492
L128901	CJ1	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:28 2005 Page 1

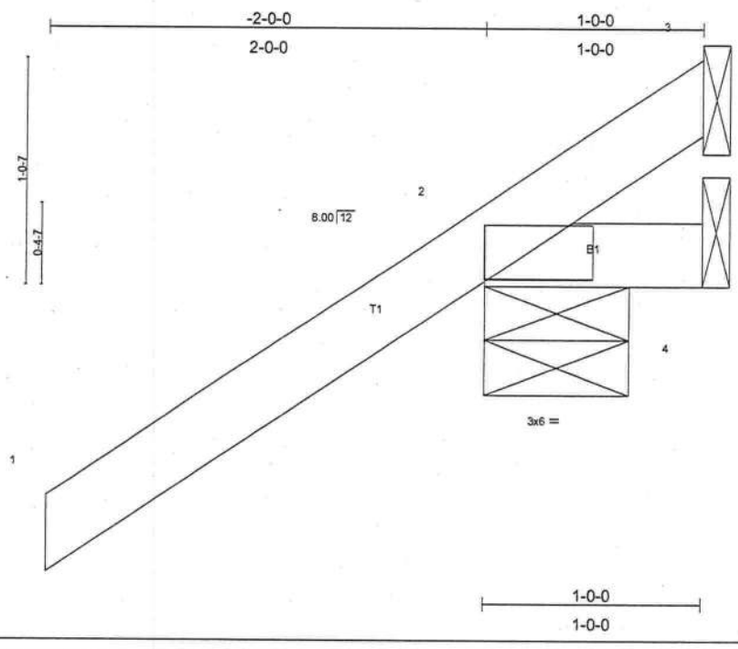


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=276/0-8-0, 4=14/Mechanical, 3=-100/Mechanical
Max Horz 2=137(load case 5)
Max Uplift2=-372(load case 5), 3=-100(load case 1)
Max Grav 2=276(load case 1), 4=14(load case 1), 3=191(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-92/135
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.22

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 2 and 100 lb uplift at joint 3.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105493
L128901	CJ3	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:29 2005 Page 1

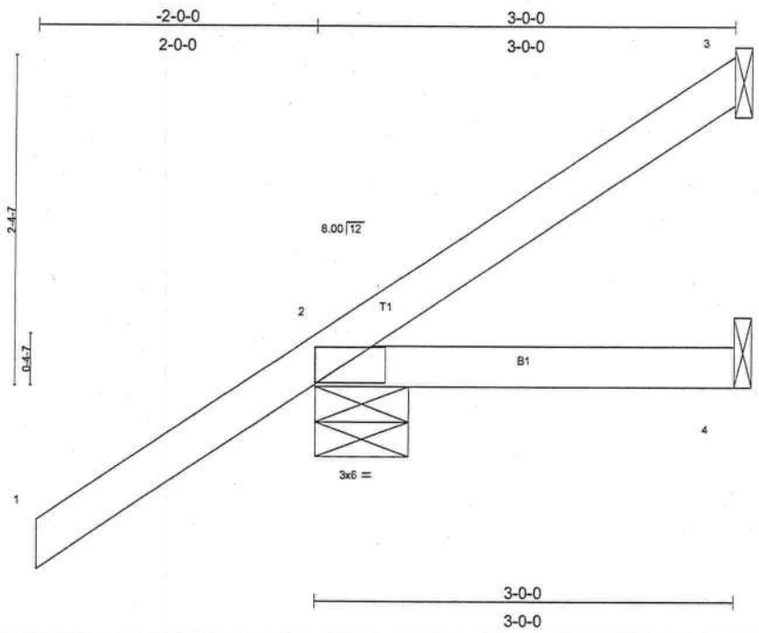


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 14 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=14/Mechanical, 2=292/0-8-0, 4=39/Mechanical
Max Horz 2=209(load case 5)
Max Uplift3=-31(load case 4), 2=-266(load case 5)
Max Grav 3=41(load case 3), 2=292(load case 1), 4=39(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-77/23
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.19

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 3 and 266 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105494
L128901	CJ5	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:29 2005 Page 1

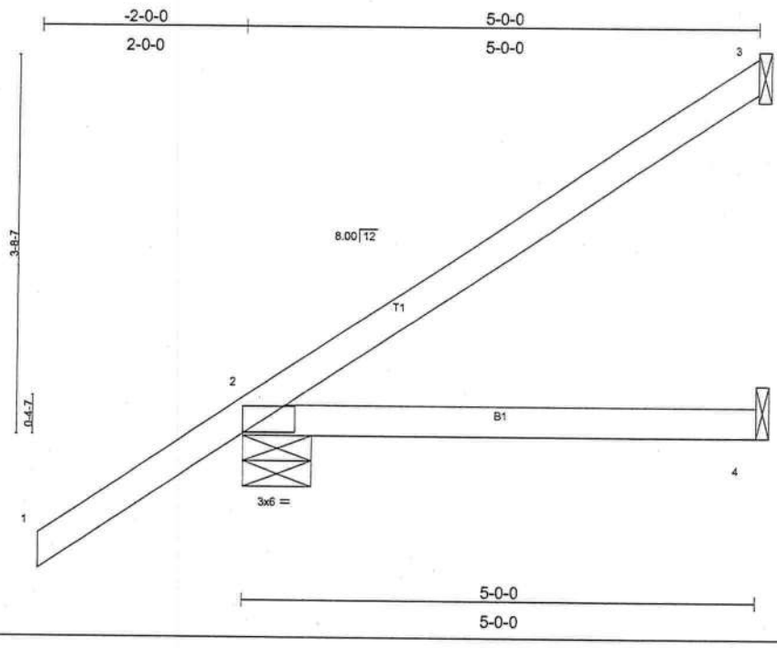


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=92/Mechanical, 2=351/0-8-0, 4=69/Mechanical
Max Horz 2=282(load case 5)
Max Uplift3=-119(load case 5), 2=-241(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-100/40
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.21

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 119 lb uplift at joint 3 and 241 lb uplift at joint 2.

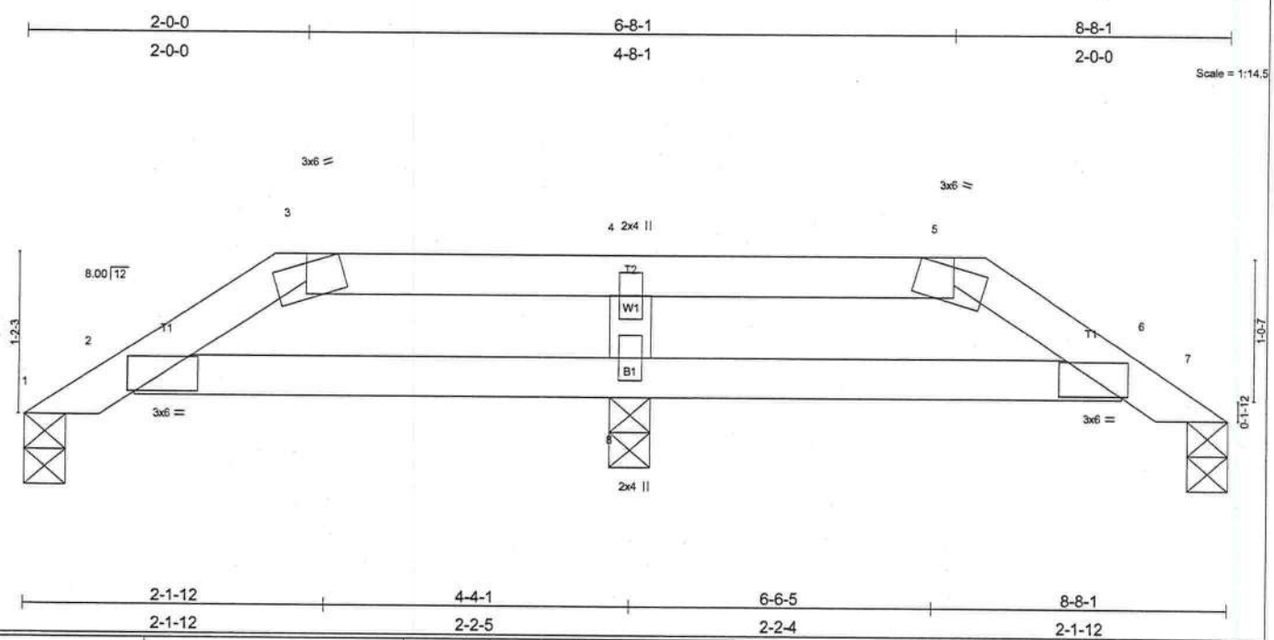
LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105495
L128901	D01	HIP PIGGYBACK	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:30 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.02	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=158/0-3-8, 7=158/0-3-8, 8=364/0-3-8
Max Horz 1=-48(load case 3)
Max Uplift1=-70(load case 5), 7=-74(load case 6), 8=-168(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-78/62, 2-3=-178/172, 3-4=-153/171, 4-5=-153/171, 5-6=-178/172, 6-7=-78/62
BOT CHORD 2-8=-104/151, 6-8=-104/151
WEBS 4-8=-197/230

JOINT STRESS INDEX
2 = 0.27, 3 = 0.15, 4 = 0.13, 5 = 0.12, 6 = 0.27 and 8 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 1, 74 lb uplift at joint 7 and 168 lb uplift at joint 8.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
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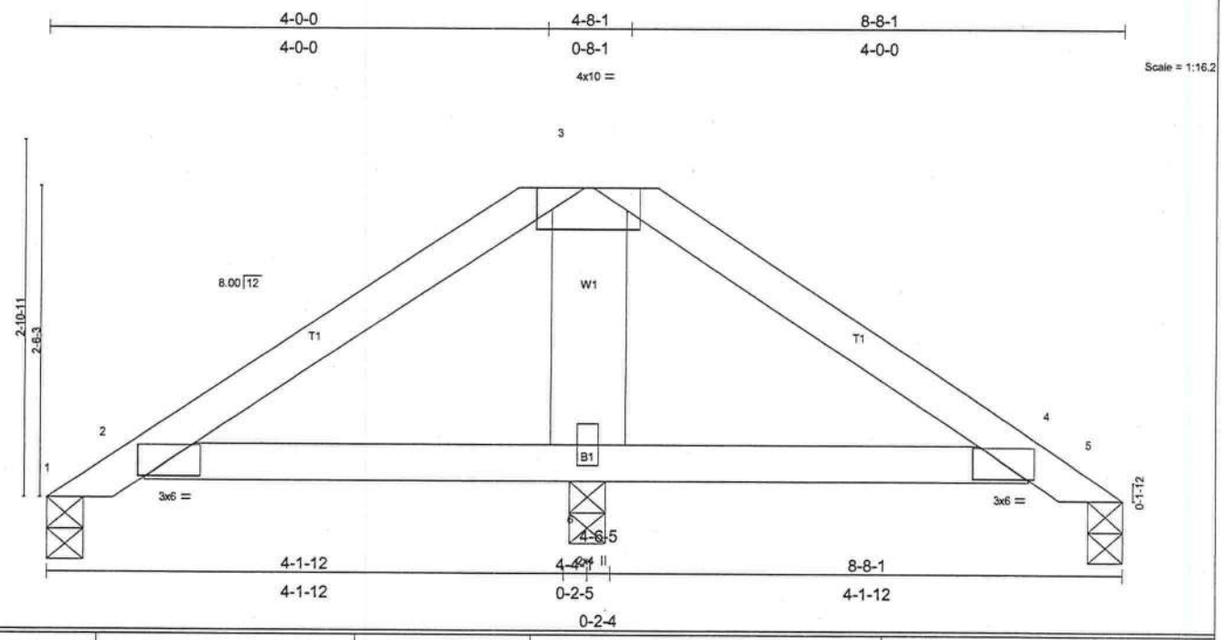
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105496
L128901	D02	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:30 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=26/0-3-8, 5=25/0-3-8, 6=630/0-3-8
Max Horz 1=-113(load case 3)
Max Uplift1=-25(load case 8), 5=-48(load case 3), 6=-252(load case 5)
Max Grav 1=60(load case 7), 5=58(load case 8), 6=630(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-97/101, 2-3=-126/279, 3-4=-139/297, 4-5=-27/29
BOT CHORD 2-6=-178/182, 4-6=-197/195
WEBS 3-6=-477/335

JOINT STRESS INDEX
2 = 0.31, 3 = 0.55, 4 = 0.31 and 6 = 0.19

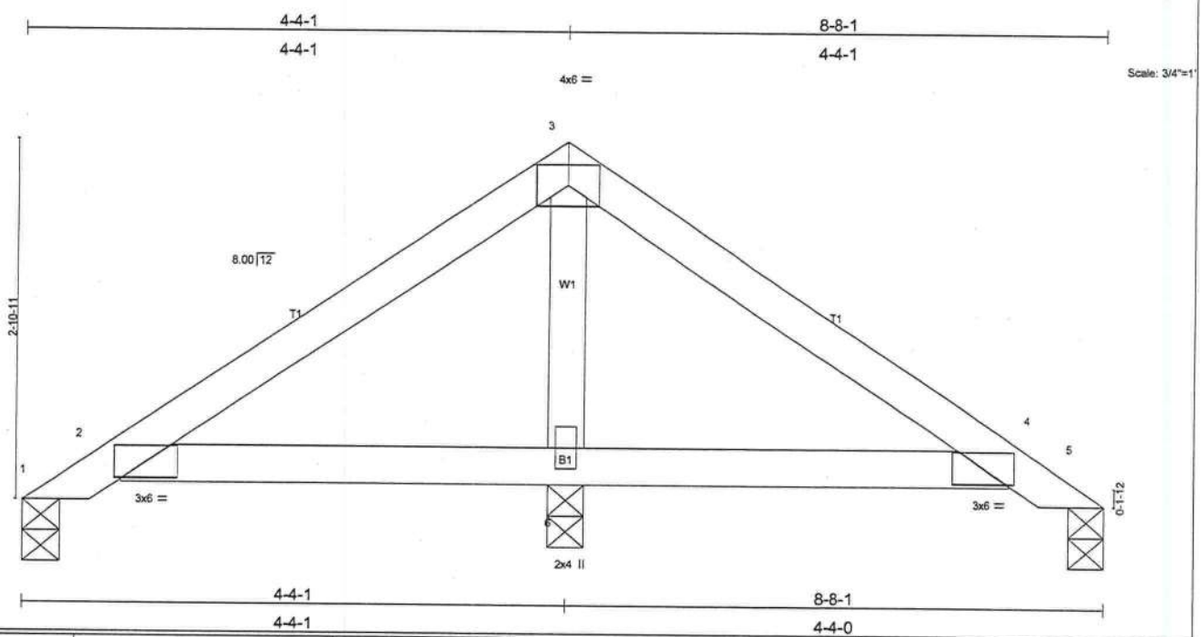
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 1, 48 lb uplift at joint 5 and 252 lb uplift at joint 6.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105497
L128901	D03	PIGGYBACK	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:31 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.02	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=36/0-3-8, 5=36/0-3-8, 6=609/0-3-8
Max Horz 1=-115(load case 3)
Max Uplift1=-17(load case 8), 5=-48(load case 3), 6=-246(load case 5)
Max Grav 1=66(load case 7), 5=66(load case 8), 6=609(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-100/104, 2-3=-116/261, 3-4=-115/261, 4-5=-32/29
BOT CHORD 2-6=-163/174, 4-6=-163/174
WEBS 3-6=-451/318

JOINT STRESS INDEX
2 = 0.34, 3 = 0.36, 4 = 0.34 and 6 = 0.18

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1, 48 lb uplift at joint 5 and 246 lb uplift at joint 6.

LOAD CASE(S) Standard

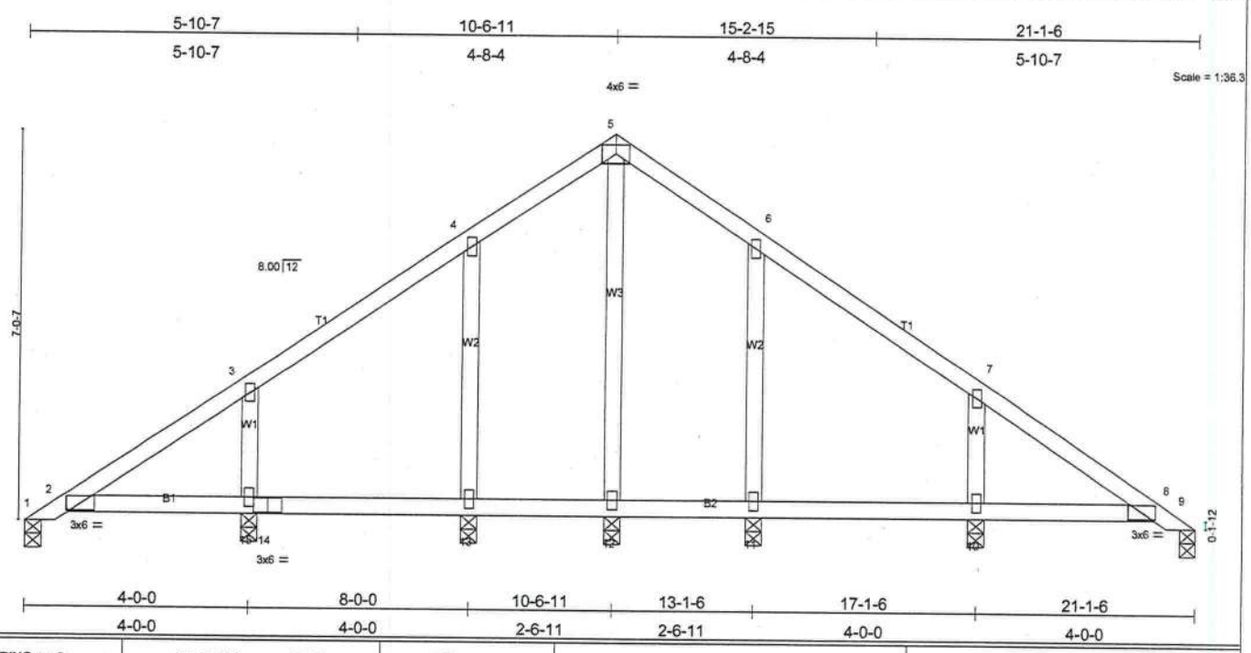
AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105498
L128901	D04	PIGGYBACK	25	1	

Builders FirstSource, Lake City, Fl 32055

Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:32 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.15	Vert(LL)	0.01	2-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	2-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 96 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=78/0-3-8, 9=78/0-3-8, 12=259/0-3-8, 15=370/0-3-8, 13=286/0-3-8, 11=286/0-3-8, 10=370/0-3-8
Max Horz 1=-267(load case 3)
Max Uplift1=-91(load case 3), 12=-14(load case 4), 15=-280(load case 5), 13=-221(load case 5), 11=-222(load case 6), 10=-267(load case 6)
Max Grav 1=114(load case 4), 9=86(load case 8), 12=259(load case 1), 15=372(load case 7), 13=288(load case 7), 11=288(load case 8), 10=372(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-304/298, 2-3=-253/204, 3-4=-136/182, 4-5=-29/213, 5-6=-1/197, 6-7=-0/138, 7-8=-142/145, 8-9=-42/4
BOT CHORD 2-15=-67/184, 14-15=-67/184, 13-14=-67/184, 12-13=-67/184, 11-12=-67/184, 10-11=-67/184, 8-10=-67/184
WEBS 5-12=-196/24, 3-15=-229/287, 4-13=-190/247, 6-11=-190/247, 7-10=-229/281

JOINT STRESS INDEX
2 = 0.24, 3 = 0.34, 4 = 0.34, 5 = 0.28, 6 = 0.34, 7 = 0.34, 8 = 0.24, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.15 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 1, 14 lb uplift at joint 12, 280 lb uplift at joint 15, 221 lb uplift at joint 13, 222 lb uplift at joint 11 and 267 lb uplift at joint 10.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105499
L128901	EJ7	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:32 2005 Page 1

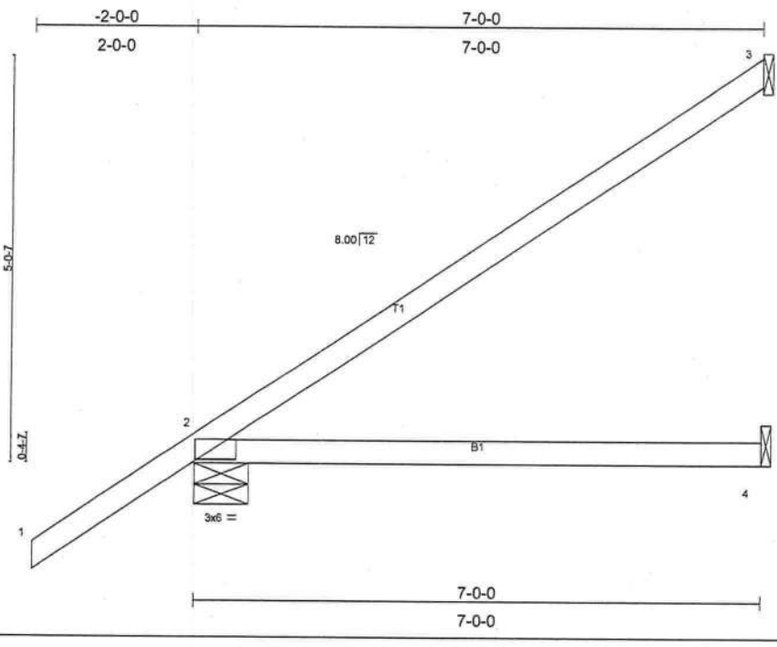


Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.10	2-4	>795	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.17	2-4	>473	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=155/Mechanical, 2=426/0-8-0, 4=100/Mechanical
Max Horz 2=355(load case 5)
Max Uplift3=-191(load case 5), 2=-240(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-151/69
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.61

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 3 and 240 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105500
L128901	HGBL01	MONO PITCH	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:33 2005 Page 1

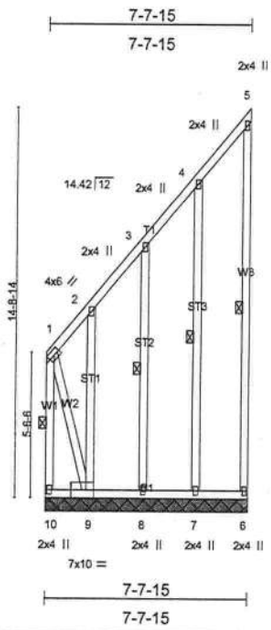


Plate Offsets (X,Y): [9:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 108 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except* W1 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 8-9-15 oc bracing: 9-10.
WEBS 1 Row at midpt 5-6, 4-7, 3-8, 1-10

REACTIONS (lb/size) 6=65/7-7-15, 10=50/7-7-15, 7=173/7-7-15, 8=171/7-7-15, 9=159/7-7-15
Max Horz 10=486(load case 5)
Max Uplift6=-113(load case 5), 10=-278(load case 3), 7=-218(load case 5), 8=-258(load case 5), 9=-1911(load case 5)
Max Grav 6=65(load case 1), 10=2202(load case 5), 7=173(load case 1), 8=171(load case 1), 9=281(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-827/97, 2-3=-611/73, 3-4=-343/43, 4-5=-106/32, 5-6=-42/126, 1-10=-2538/275
BOT CHORD 9-10=-554/62, 8-9=-3/0, 7-8=-3/0, 6-7=-3/0
WEBS 4-7=-110/291, 3-8=-111/332, 2-9=-99/270, 1-9=-223/2013

JOINT STRESS INDEX
1 = 0.74, 2 = 0.13, 3 = 0.16, 4 = 0.14, 5 = 0.31, 6 = 0.24, 7 = 0.17, 8 = 0.19, 9 = 0.51 and 10 = 0.85

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Gable requires continuous bottom chord bearing.
4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 6, 278 lb uplift at joint 10, 218 lb uplift at joint 7, 258 lb uplift at joint 8 and 1911 lb uplift at joint 9.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#08310550l
L128901	HGBL02	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:34 2005 Page 1

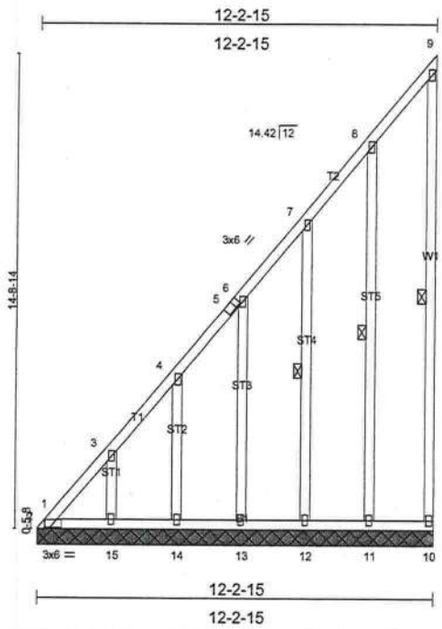


Plate Offsets (X,Y): [5:0-2-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	-0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 119 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 9-10, 8-11, 7-12

REACTIONS (lb/size) 10=65/12-2-15, 2=132/12-2-15, 11=174/12-2-15, 12=168/12-2-15, 13=168/12-2-15, 14=168/12-2-15, 15=170/12-2-15, 1=-34/12-2-15
Max Horz 1=798(load case 5)
Max Uplift10=-99(load case 5), 11=-239(load case 5), 12=-243(load case 5), 13=-239(load case 5), 14=-238(load case 5), 15=-280(load case 5), 1=-112(load case 3)
Max Grav 10=65(load case 1), 2=132(load case 1), 11=174(load case 1), 12=168(load case 1), 13=168(load case 1), 14=168(load case 1), 15=170(load case 1), 1=816(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1225/157, 2-3=-1228/155, 3-4=-986/126, 4-5=-763/92, 5-6=-751/98, 6-7=-542/72, 7-8=-319/44, 8-9=-94/32, 9-10=-42/107
BOT CHORD 2-15=-1/0, 14-15=-1/0, 13-14=-1/0, 12-13=-1/0, 11-12=-1/0, 10-11=-1/0
WEBS 8-11=-111/277, 7-12=-109/273, 6-13=-108/270, 4-14=-108/274, 3-15=-107/296

JOINT STRESS INDEX
2 = 0.13, 3 = 0.14, 4 = 0.13, 5 = 0.11, 6 = 0.13, 7 = 0.13, 8 = 0.13, 9 = 0.14, 10 = 0.10, 11 = 0.16, 12 = 0.16, 13 = 0.15, 14 = 0.16 and 15 = 0.17

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) All plates are 2x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 10, 239 lb uplift at joint 11, 243 lb uplift at joint 12, 239 lb uplift at joint 13, 238 lb uplift at joint 14, 280 lb uplift at joint 15 and 112 lb uplift at joint 1.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105502
L128901	HJ7	MONO TRUSS	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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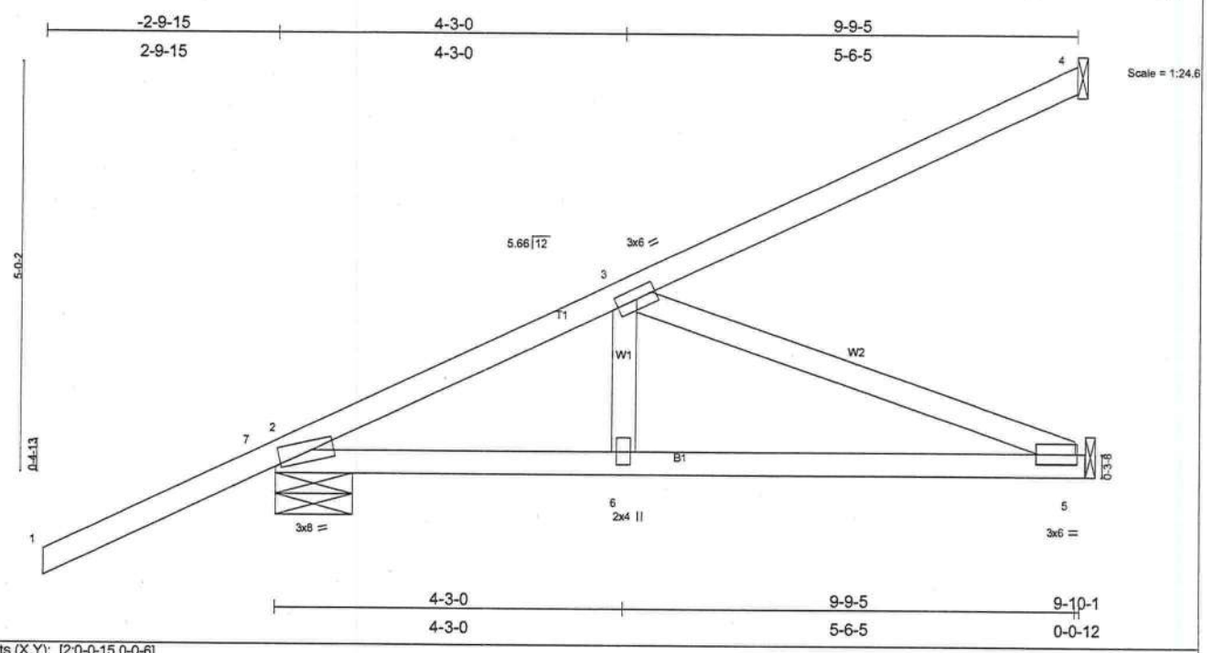


Plate Offsets (X,Y): [2:0-0-15,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.07	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.12	5-6	>961	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-1 oc bracing.

REACTIONS (lb/size) 4=258/Mechanical, 2=526/0-11-5, 5=367/Mechanical
Max Horz 2=414(load case 4)
Max Uplift4=-316(load case 4), 2=-286(load case 4), 5=-105(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-7=0/56, 2-7=0/56, 2-3=-660/10, 3-4=-169/80
BOT CHORD 2-6=-321/572, 5-6=-321/572
WEBS 3-6=0/164, 3-5=-612/344

JOINT STRESS INDEX
2 = 0.99, 3 = 0.17, 5 = 0.17 and 6 = 0.12

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 316 lb uplift at joint 4, 286 lb uplift at joint 2 and 105 lb uplift at joint 5.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-54
Trapezoidal Loads (plf)
Vert: 7=-0(F=27, B=27)-to-4=-132(F=-39, B=-39), 2=-2(F=14, B=14)-to-5=-73(F=-22, B=-22)

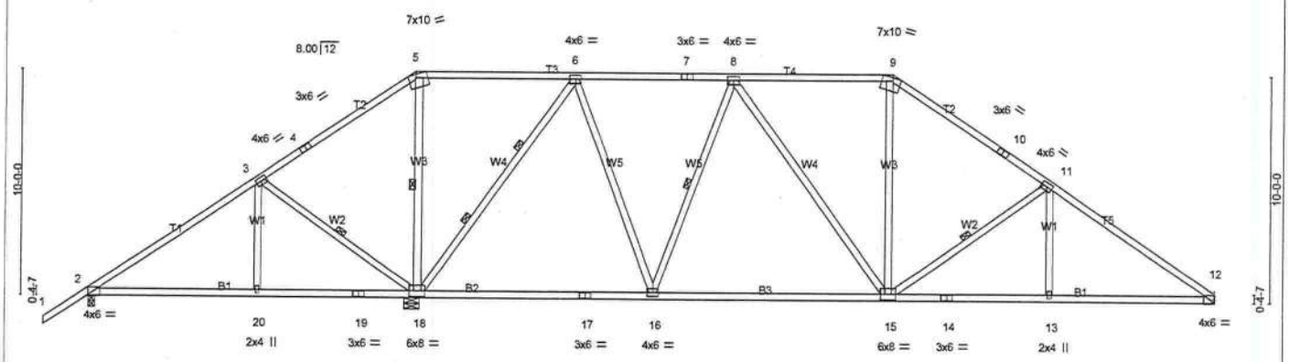
AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105503
L128901	T01	HIP	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:36 2005 Page 1

2-0-0	7-5-2	14-5-5	21-5-12	28-6-4	35-6-11	42-6-14	50-0-0
2-0-0	7-5-2	7-0-4	7-0-7	7-0-7	7-0-7	7-0-4	7-5-2

Scale = 1:89.3



7-5-2	14-5-5	25-0-0	35-6-11	42-6-14	50-0-0
7-5-2	7-0-4	10-6-11	10-6-11	7-0-4	7-5-2

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [5:0-4-0,Edge], [9:0-4-0,Edge], [12:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.23	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.39	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 293 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-18, 5-18, 8-16, 11-15
2 Rows at 1/3 pts 6-18

REACTIONS (lb/size) 2=278/0-3-8, 18=2720/0-8-0, 12=1298/Mechanical
Max Horz 2=436(load case 4)
Max Uplift2=-440(load case 5), 18=-1428(load case 4), 12=-489(load case 6)
Max Grav 2=344(load case 7), 18=2720(load case 1), 12=1319(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-57/426, 3-4=-363/661, 4-5=-333/851, 5-6=-210/635, 6-7=-719/533, 7-8=-719/533, 8-9=-1101/718, 9-10=-1238/740, 10-11=-1428/710, 11-12=-1992/860
BOT CHORD 2-20=-322/394, 19-20=-322/394, 18-19=-322/394, 17-18=-246/380, 16-17=-246/380, 15-16=-406/937, 14-15=-577/1567, 13-14=-577/1567, 12-13=-577/1567
WEBS 3-20=-365/226, 3-18=-572/779, 5-18=-786/486, 6-18=-1646/916, 6-16=-386/1020, 8-16=-667/469, 8-15=-198/306, 9-15=-99/359, 11-15=-576/487, 11-13=0/241

JOINT STRESS INDEX
2 = 0.69, 3 = 0.32, 4 = 0.33, 5 = 0.77, 6 = 0.65, 7 = 0.27, 8 = 0.65, 9 = 0.77, 10 = 0.33, 11 = 0.32, 12 = 0.69, 13 = 0.34, 14 = 0.51, 15 = 0.31, 16 = 0.65, 17 = 0.44, 18 = 0.31, 19 = 0.51 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 2, 1428 lb uplift at joint 18 and 489 lb uplift at joint 12.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105504
L128901	T01G	HIP	1	1	Job Reference (optional)

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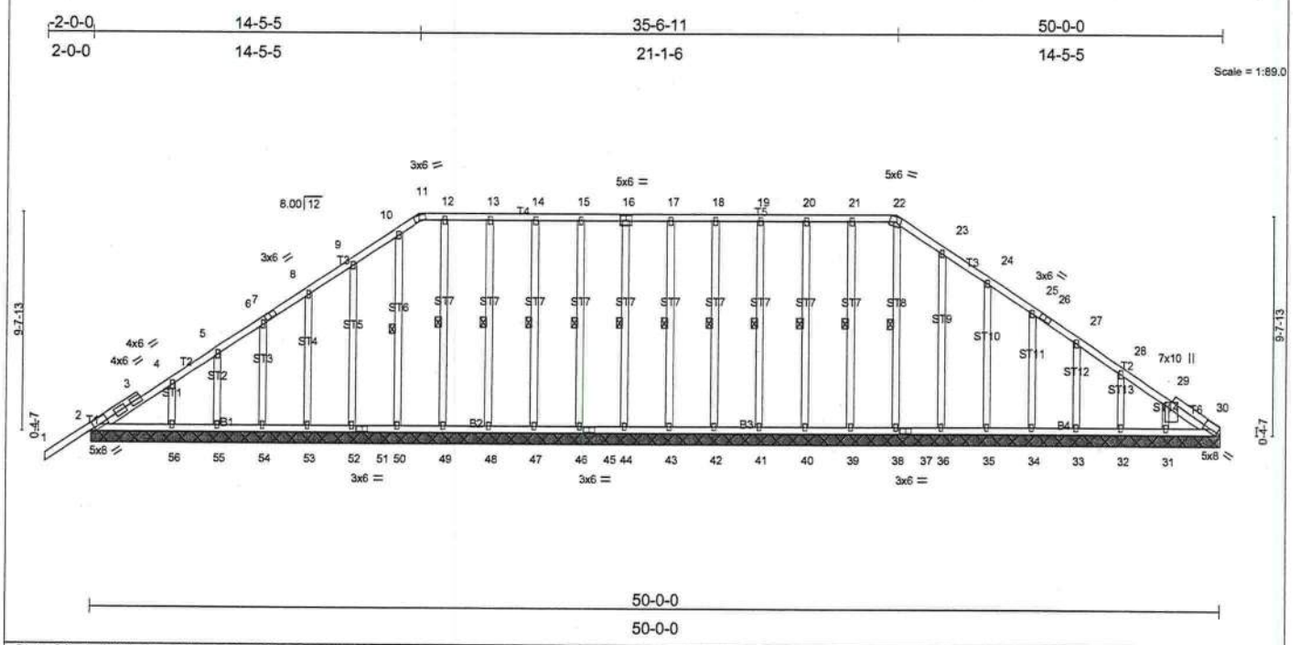

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [16:0-3-0,0-3-0], [22:0-2-0,Edge], [29:0-2-15,0-2-4], [30:Edge,0-3-4]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	0.00	1	n/r	90	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	1	n/r	80		
BCLL	10.0	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.02	30	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 401 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 22-38, 21-39, 20-40, 19-41, 18-42, 17-43, 16-44, 15-46, 14-47, 13-48, 12-49, 10-50

REACTIONS (lb/size) 2=279/50-0-0, 38=161/50-0-0, 39=168/50-0-0, 40=168/50-0-0, 41=168/50-0-0, 42=168/50-0-0, 43=168/50-0-0, 44=168/50-0-0, 46=168/50-0-0, 47=168/50-0-0, 48=169/50-0-0, 49=164/50-0-0, 50=163/50-0-0, 52=169/50-0-0, 53=167/50-0-0, 54=173/50-0-0, 55=146/50-0-0, 56=237/50-0-0, 36=168/50-0-0, 35=168/50-0-0, 34=167/50-0-0, 33=170/50-0-0, 32=161/50-0-0, 31=209/50-0-0, 30=94/50-0-0
Max Horz 2=421(load case 4)
Max Uplift2=-128(load case 3), 38=-18(load case 4), 39=-106(load case 4), 40=-103(load case 3), 41=-100(load case 4), 42=-100(load case 3), 43=-100(load case 4), 44=-100(load case 3), 46=-100(load case 3), 47=-100(load case 4), 48=-113(load case 3), 49=-92(load case 4), 50=-73(load case 4), 52=-144(load case 5), 53=-129(load case 5), 54=-126(load case 5), 55=-147(load case 5), 56=-82(load case 5), 36=-122(load case 6), 35=-134(load case 6), 34=-129(load case 6), 33=-131(load case 6), 32=-111(load case 6), 31=-154(load case 6), 30=-16(load case 4)
Max Grav 2=279(load case 1), 38=161(load case 8), 39=171(load case 7), 40=168(load case 1), 41=168(load case 8), 42=168(load case 7), 43=168(load case 1), 44=168(load case 8), 46=168(load case 8), 47=168(load case 7), 48=170(load case 8), 49=165(load case 7), 50=164(load case 7), 52=169(load case 1), 53=167(load case 7), 54=173(load case 1), 55=146(load case 7), 56=237(load case 1), 36=168(load case 8), 35=168(load case 1), 34=167(load case 8), 33=170(load case 1), 32=161(load case 1), 31=209(load case 8), 30=94(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/58, 2-3=-335/238, 3-4=-324/253, 4-5=-275/240, 5-6=-215/234, 6-7=-157/218, 7-8=-152/226, 8-9=-98/230, 9-10=-44/288, 10-11=-42/280, 11-12=-11/280, 12-13=-11/280, 13-14=-11/280, 14-15=-11/280, 15-16=-11/280, 16-17=-11/280, 17-18=-11/280, 18-19=-11/280, 19-20=-11/280, 20-21=-11/280, 21-22=-12/279, 22-23=-44/294, 23-24=-44/217, 24-25=-44/131, 25-26=-5/55, 26-27=-44/50, 27-28=-78/60, 28-29=-152/63, 29-30=-246/74
BOT CHORD 2-56=-61/243, 55-56=-61/243, 54-55=-61/243, 53-54=-61/243, 52-53=-61/243, 51-52=-61/243, 50-51=-61/243, 49-50=-61/243, 48-49=-61/243, 47-48=-61/243, 46-47=-61/243, 45-46=-61/243, 44-45=-61/243, 43-44=-61/243, 42-43=-61/243, 41-42=-61/243, 40-41=-61/243, 39-40=-61/243, 38-39=-61/243, 37-38=-61/243, 36-37=-61/243, 35-36=-61/243, 34-35=-61/243, 33-34=-61/243, 32-33=-61/243, 31-32=-61/243, 30-31=-61/243
WEBS 22-38=-101/30, 21-39=-111/118, 20-40=-108/115, 19-41=-108/112, 18-42=-108/112, 17-43=-108/112, 16-44=-108/112, 15-46=-108/112, 14-47=-108/112, 13-48=-110/125, 12-49=-105/104, 10-50=-104/85, 9-52=-109/156, 8-53=-107/140, 6-54=-110/140, 5-55=-98/149, 4-56=-147/116, 23-36=-108/135, 24-35=-108/146, 25-34=-108/141, 27-33=-109/143, 28-32=-105/124, 29-31=-134/160

JOINT STRESS INDEX
2 = 0.58, 3 = 0.00, 3 = 0.21, 3 = 0.21, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.15, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.26, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.20, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.22, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.15, 27 = 0.34, 28 = 0.34, 29 = 0.61, 30 = 0.41, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.15, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 45 = 0.15, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 51 = 0.15, 52 = 0.34, 53 = 0.34, 54 = 0.34, 55 = 0.34 and 56 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.

Continued on page 2

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105505
L128901	T01G	HIP	1	1	Job Reference (optional)

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NOTES

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 128 lb uplift at joint 2, 18 lb uplift at joint 38, 106 lb uplift at joint 39, 103 lb uplift at joint 40, 100 lb uplift at joint 41, 100 lb uplift at joint 42, 100 lb uplift at joint 43, 100 lb uplift at joint 44, 100 lb uplift at joint 46, 100 lb uplift at joint 47, 113 lb uplift at joint 48, 92 lb uplift at joint 49, 73 lb uplift at joint 50, 144 lb uplift at joint 52, 129 lb uplift at joint 53, 126 lb uplift at joint 54, 147 lb uplift at joint 55, 82 lb uplift at joint 56, 122 lb uplift at joint 36, 134 lb uplift at joint 35, 129 lb uplift at joint 34, 131 lb uplift at joint 33, 111 lb uplift at joint 32, 154 lb uplift at joint 31 and 16 lb uplift at joint 30.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105506
L128901	T02	SPECIAL	15	1	Job Reference (optional)

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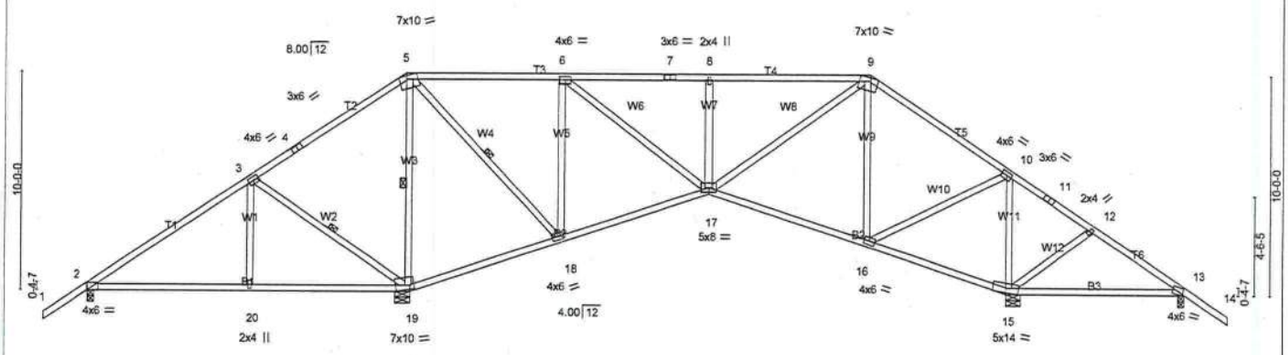
2-0-0	7-5-2	14-5-5	21-5-12	28-6-4	35-6-11	41-10-0	45-8-9	50-0-0	52-0-0
2-0-0	7-5-2	7-0-4	7-0-7	7-0-7	7-0-7	6-3-5	3-10-9	4-3-7	2-0-0

Scale = 1:91.7



7-5-2	14-4-0	14-8-0	21-5-12	28-3-0	35-6-11	41-10-0	42-2-0	50-0-0
7-5-2	6-10-14	0-4-0	6-9-12	6-9-4	7-3-11	6-3-5	0-4-0	7-10-0

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [5:0-4-0,Edge], [9:0-4-0,Edge], [13:0-1-1,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.19	13-15	>516	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	0.16	13-15	>613	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.06	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 294 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
10-0-0 oc bracing: 17-18,16-17.
WEBS 1 Row at midpt 3-19, 5-19, 5-18

REACTIONS (lb/size) 2=348/0-3-8, 15=1717/0-8-0, 13=112/0-3-8, 19=2230/0-8-0
Max Horz 2=-409(load case 3)
Max Uplift2=-498(load case 5), 15=-885(load case 3), 13=-433(load case 6), 19=-1424(load case 4)
Max Grav 2=365(load case 7), 15=1745(load case 8), 13=112(load case 1), 19=2230(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-140/407, 3-4=-444/557, 4-5=-414/740, 5-6=-232/310, 6-7=-803/516, 7-8=-803/516, 8-9=-804/516, 9-10=-495/311, 10-11=-272/630, 11-12=-288/526, 12-13=-304/479, 13-14=0/59
BOT CHORD 2-20=-256/371, 19-20=-256/371, 18-19=-629/632, 17-18=-134/423, 16-17=-93/381, 15-16=-564/403, 13-15=-362/246
WEBS 3-20=-358/246, 3-19=-587/771, 5-19=-1498/947, 5-18=-572/1073, 6-18=-827/605, 6-17=-368/730, 8-17=-395/424, 9-17=-406/580, 9-16=-500/353, 10-16=-389/940, 10-15=-1231/536, 12-15=-207/236

JOINT STRESS INDEX
2 = 0.43, 3 = 0.32, 4 = 0.25, 5 = 0.76, 6 = 0.31, 7 = 0.20, 8 = 0.34, 9 = 0.67, 10 = 0.40, 11 = 0.20, 12 = 0.34, 13 = 0.81, 15 = 0.73, 16 = 0.37, 17 = 0.75, 18 = 0.55, 19 = 0.94 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 2, 885 lb uplift at joint 15, 433 lb uplift at joint 13 and 1424 lb uplift at joint 19.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105507
L128901	T02G	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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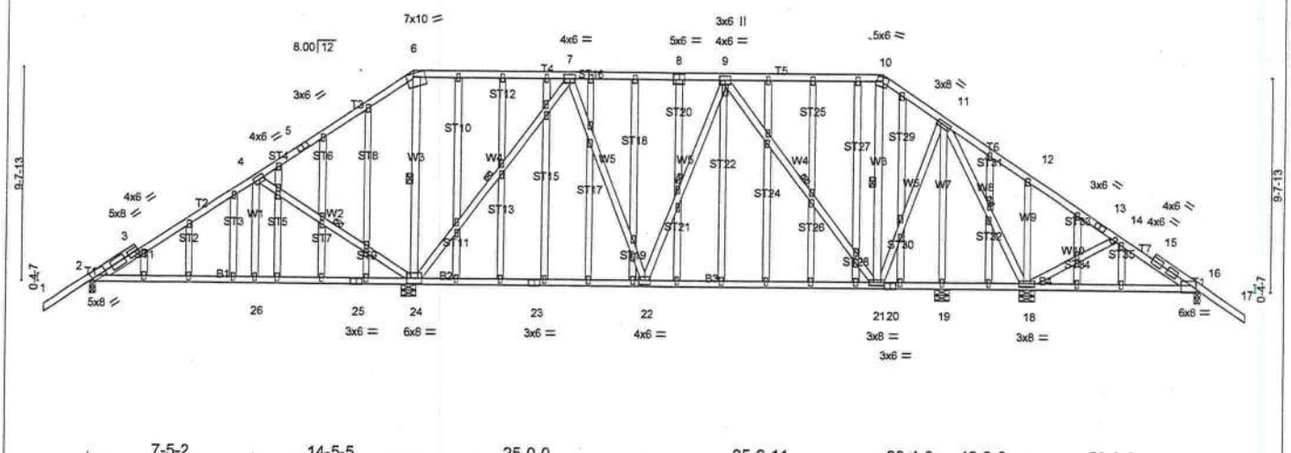
2-0-0	7-5-2	14-5-5	21-5-12	28-6-4	35-6-11	42-6-14	50-0-0	52-0-0
2-0-0	7-5-2	7-0-4	7-0-7	7-0-7	7-0-7	7-0-4	7-5-2	2-0-0

Scale = 1:90.5



7-5-2	14-5-5	25-0-0	35-6-11	38-4-0	42-2-0	50-0-0
7-5-2	7-0-4	10-6-11	10-6-11	2-9-5	3-10-0	7-10-0

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [6:0-4-0,Edge], [8:0-3-0,0-3-0], [9:0-2-13,0-1-8], [16:0-2-9,Edge], [76:0-1-15,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.17	21-22	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.30	21-22	>960	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.02	16	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 527 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2D
BOT CHORD	2 X 4 SYP No.2D
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 4-24, 6-24, 7-24, 9-22, 9-21, 10-21, 11-18

REACTIONS (lb/size) 2=437/0-3-8, 24=2075/0-8-0, 19=701/0-8-0, 18=850/0-8-0, 16=344/0-3-8
Max Horz 2=-394(load case 3)
Max Uplift2=-423(load case 5), 24=-1402(load case 4), 19=-296(load case 3), 18=-568(load case 6), 16=-360(load case 6)
Max Grav 2=441(load case 7), 24=2075(load case 1), 19=725(load case 8), 18=871(load case 8), 16=350(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/59, 2-3=-192/241, 3-4=-106/268, 4-5=-428/396, 5-6=-399/585, 6-7=-264/412, 7-8=-488/450, 8-9=-488/450, 9-10=-303/341, 10-11=-382/340, 11-12=-54/208, 12-13=-79/221, 13-14=-96/112, 14-15=-63/67, 15-16=-108/44, 16-17=0/59

BOT CHORD 2-26=-209/252, 25-26=-209/252, 24-25=-209/252, 23-24=-143/391, 22-23=-143/391, 21-22=-271/522, 20-21=0/304, 19-20=0/304, 18-19=0/304, 16-18=-27/125

WEBS 4-26=-365/225, 4-24=-578/789, 6-24=-640/520, 7-24=-1063/659, 7-22=-172/509, 9-22=-156/257, 9-21=-370/302, 10-21=-116/161, 11-21=-347/709, 11-19=-734/341, 12-18=-231/295, 11-18=-371/171, 14-18=-220/260

JOINT STRESS INDEX

2 = 0.79, 3 = 0.25, 3 = 0.34, 3 = 0.00, 3 = 0.34, 4 = 0.32, 5 = 0.21, 6 = 0.72, 7 = 0.34, 8 = 0.20, 9 = 0.34, 9 = 0.60, 10 = 0.64, 11 = 0.87, 12 = 0.34, 13 = 0.15, 14 = 0.34, 15 = 0.00, 15 = 0.17, 15 = 0.17, 16 = 0.69, 18 = 0.62, 19 = 0.34, 20 = 0.52, 21 = 0.87, 22 = 0.34, 23 = 0.55, 24 = 0.24, 25 = 0.39, 26 = 0.34, 27 = 0.34, 28 = 0.34, 28 = 0.34, 29 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34, 41 = 0.34, 41 = 0.34, 42 = 0.34, 43 = 0.34, 44 = 0.34, 44 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34, 50 = 0.34, 50 = 0.34, 51 = 0.34, 52 = 0.34, 53 = 0.34, 54 = 0.34, 55 = 0.34, 56 = 0.34, 57 = 0.34, 58 = 0.34, 59 = 0.34, 60 = 0.34, 61 = 0.34, 61 = 0.34, 62 = 0.34, 63 = 0.34, 64 = 0.34, 64 = 0.34, 65 = 0.34, 66 = 0.34, 67 = 0.34, 67 = 0.34, 68 = 0.34, 69 = 0.34, 70 = 0.34, 70 = 0.34, 71 = 0.34, 72 = 0.34, 73 = 0.34, 73 = 0.34, 74 = 0.34, 75 = 0.34, 76 = 0.35, 76 = 0.34, 77 = 0.34 and 78 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 2, 1402 lb uplift at joint 24, 296 lb uplift at joint 19, 568 lb uplift at joint 18 and 360 lb uplift at joint 16.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

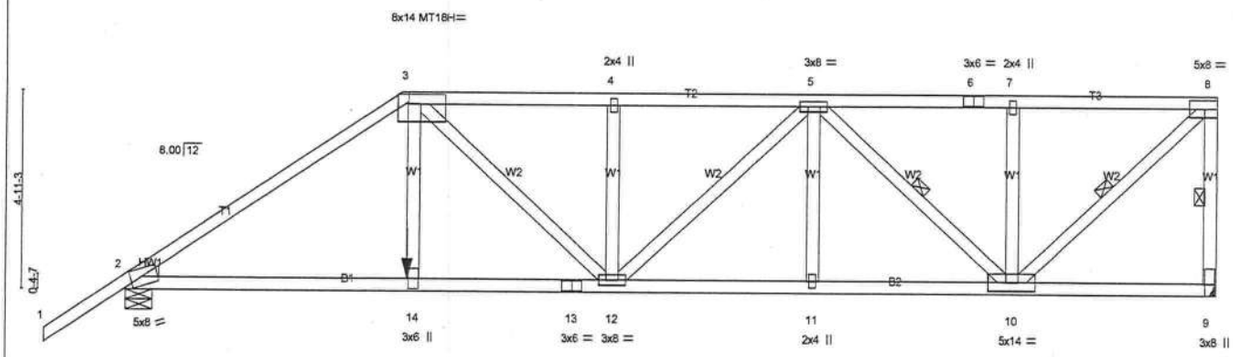
Job	Truss	Truss Type	Qty	Ply	Dwg.#083105508
L128901	T03	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:40 2005 Page 1

-2-0-0 | 3-8-7 | 7-0-0 | 12-1-2 | 17-0-8 | 21-11-14 | 27-1-0

2-0-0 | 3-8-7 | 3-3-9 | 5-1-2 | 4-11-6 | 4-11-6 | 5-1-2

Scale = 1:49.9



3-8-7 | 7-0-0 | 12-1-2 | 17-0-8 | 21-11-14 | 27-1-0

3-8-7 | 3-3-9 | 5-1-2 | 4-11-6 | 4-11-6 | 5-1-2

Plate Offsets (X,Y): [2:0-2-5,0-0-14], [3:0-7-8,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	0.22	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.31	11-12	>999	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(TL)	0.10	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-10 oc bracing.
WEBS 1 Row at midpt 8-9, 5-10, 8-10

REACTIONS (lb/size) 9=2383/Mechanical, 2=2326/0-8-0
Max Horz 2=350(load case 4)
Max Uplift9=-1799(load case 2), 2=-1524(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-3486/2387, 3-4=-3541/2612, 4-5=-3539/2608, 5-6=-2153/1619, 6-7=-2153/1619, 7-8=-2153/1619, 8-9=-2226/1807
BOT CHORD 2-14=-2016/2778, 13-14=-2030/2803, 12-13=-2030/2803, 11-12=-2457/3306, 10-11=-2457/3306, 9-10=-46/60
WEBS 3-14=-456/812, 3-12=-954/999, 4-12=-588/839, 5-12=-271/320, 5-11=0/310, 5-10=-1580/1150, 7-10=-576/779, 8-10=-2156/2870

JOINT STRESS INDEX
2 = 0.86, 3 = 0.98, 4 = 0.47, 5 = 0.89, 6 = 0.58, 7 = 0.44, 8 = 0.70, 9 = 0.70, 10 = 0.98, 11 = 0.34, 12 = 0.98, 13 = 0.93 and 14 = 0.26

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 3.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1799 lb uplift at joint 9 and 1524 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 506 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-112(F=-59), 2-14=-30, 9-14=-62(F=-33)
Concentrated Loads (lb)
Vert: 14=-539(F)

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

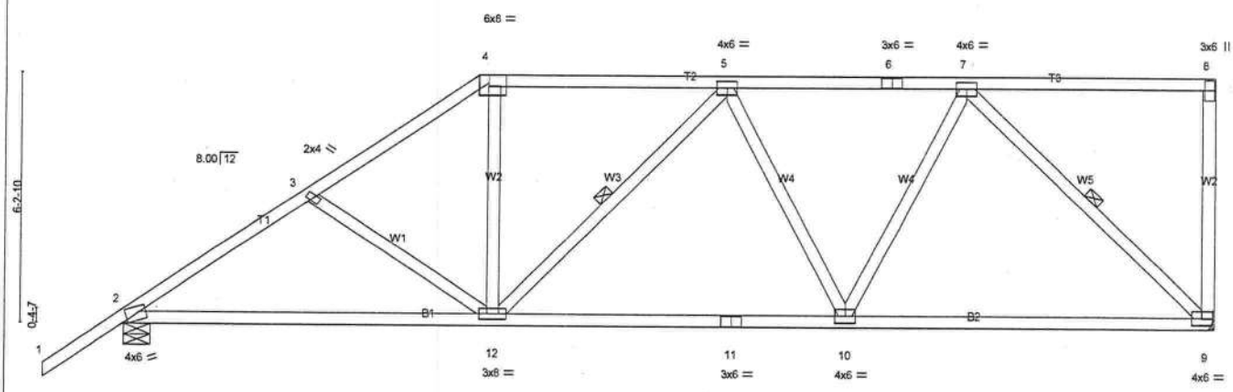
Job	Truss	Truss Type	Qty	Ply	Dwg.#083105509
L128901	T04	MONO HIP	1	1	

Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:41 2005 Page 1

-2-0-0	4-8-7	9-0-0	14-11-3	20-10-5	27-1-0
2-0-0	4-8-7	4-3-9	5-11-3	5-11-3	6-2-11

Scale = 1:49.9



9-0-0	17-10-12	27-1-0
9-0-0	8-10-12	9-2-4

Plate Offsets (X,Y): [2:0-1-5,Edge], [4:0-5-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.13	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.23	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.05	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-14 oc bracing.
WEBS 1 Row at midpt 5-12, 7-9

REACTIONS (lb/size) 9=1112/Mechanical, 2=1249/0-8-0
Max Horz 2=425(load case 5)
Max Uplift9=-523(load case 4), 2=-528(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1568/686, 3-4=-1400/643, 4-5=-1135/614, 5-6=-1139/569, 6-7=-1139/569, 7-8=-44/10, 8-9=-156/129
BOT CHORD 2-12=-752/1231, 11-12=-677/1248, 10-11=-677/1248, 9-10=-490/892
WEBS 3-12=-177/222, 4-12=-101/458, 5-12=-243/168, 5-10=-243/242, 7-10=-178/551, 7-9=-1185/670

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.62, 5 = 0.31, 6 = 0.24, 7 = 0.32, 8 = 0.34, 9 = 0.47, 10 = 0.32, 11 = 0.51 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 523 lb uplift at joint 9 and 528 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105510
L128901	T05	MONO HIP	1	1	

Job Reference (optional)

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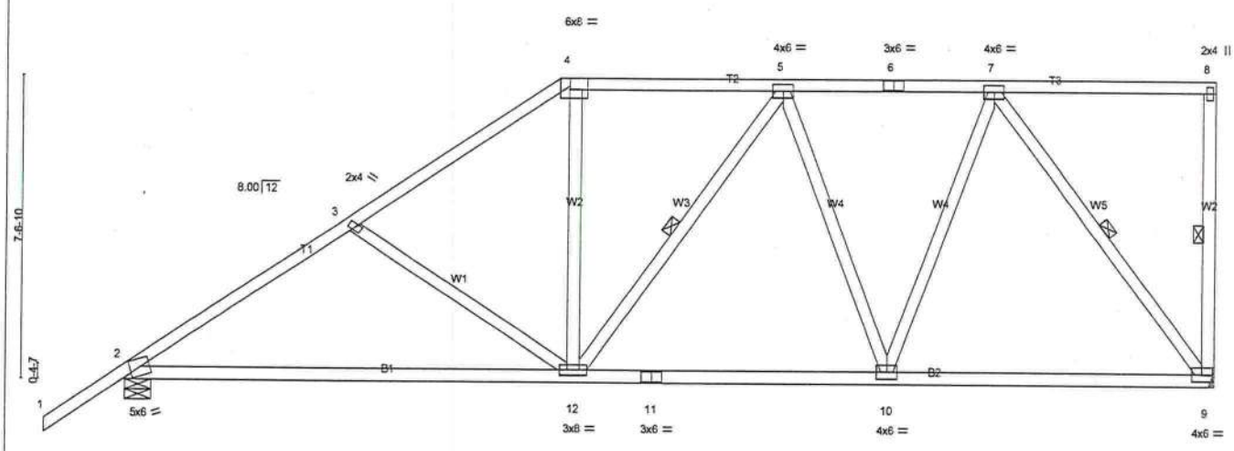
-2-0-0	5-8-7	11-0-0	16-3-3	21-6-5	27-1-0
2-0-0	5-8-7	5-3-9	5-3-3	5-3-3	5-6-11

Scale = 1:49.9



11-0-0	18-10-12	27-1-0
11-0-0	7-10-12	8-2-4

Plate Offsets (X,Y): [2:0-2-5,0-0-14], [4:0-5-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.27	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.48	2-12	>669	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 166 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.
WEBS 1 Row at midpt 8-9, 5-12, 7-9

REACTIONS (lb/size) 9=1112/Mechanical, 2=1249/0-8-0
Max Horz 2=499(load case 5)
Max Uplift9=-516(load case 4), 2=-540(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1526/653, 3-4=-1290/580, 4-5=-1018/568, 5-6=-868/454, 6-7=-868/454, 7-8=-27/5, 8-9=-137/112
BOT CHORD 2-12=-786/1206, 11-12=-549/988, 10-11=-549/988, 9-10=-375/667
WEBS 3-12=-266/309, 4-12=-61/390, 5-12=-119/169, 5-10=-351/279, 7-10=-229/592, 7-9=-1071/619

JOINT STRESS INDEX
2 = 0.87, 3 = 0.34, 4 = 0.65, 5 = 0.34, 6 = 0.25, 7 = 0.38, 8 = 0.87, 9 = 0.38, 10 = 0.38, 11 = 0.44 and 12 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 516 lb uplift at joint 9 and 540 lb uplift at joint 2.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105511
L128901	T06	HIP	1	1	Job Reference (optional)

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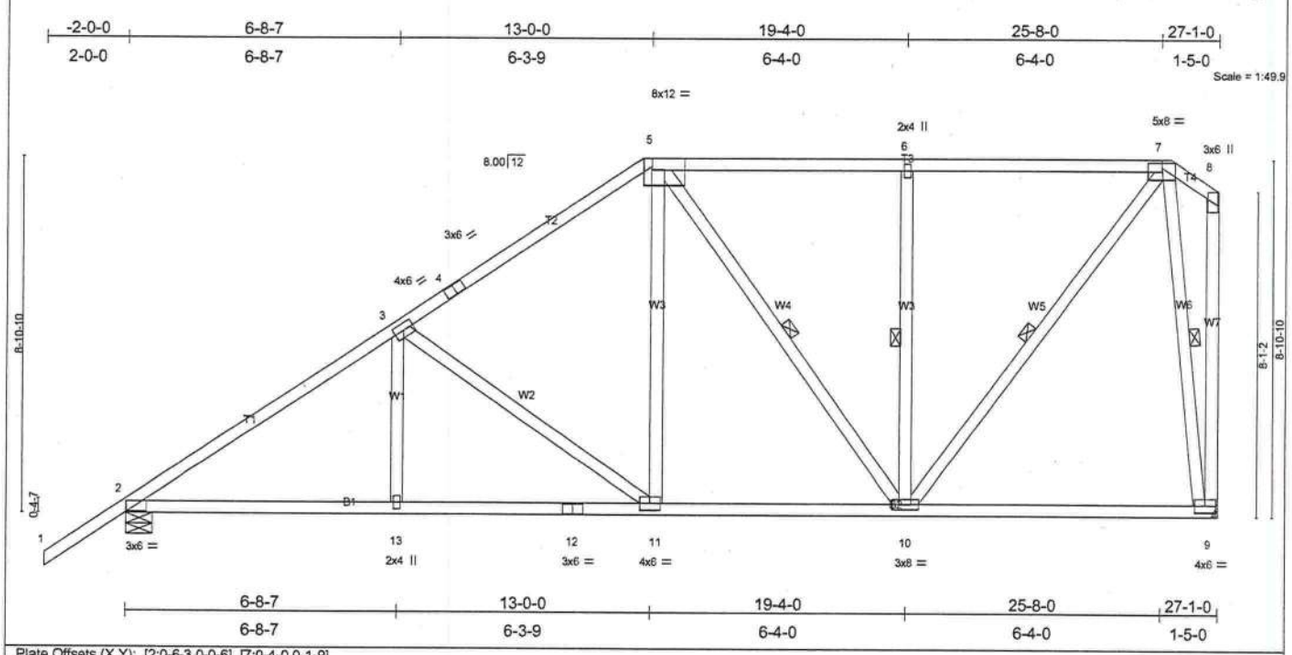

Plate Offsets (X,Y): [2:0-6-3,0-0-6], [7:0-4-0,0-1-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.08	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.14	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.03	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-13 oc bracing.
WEBS 1 Row at midpt 5-10, 6-10, 7-10, 7-9

REACTIONS (lb/size) 2=1249/0-8-0, 9=1112/Mechanical
Max Horz 2=549(load case 5)
Max Uplift2=-551(load case 5), 9=-547(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1612/593, 3-4=-1138/513, 4-5=-1041/538, 5-6=-741/436, 6-7=-741/436, 7-8=-60/26, 8-9=-113/36
BOT CHORD 2-13=-759/1238, 12-13=-759/1238, 11-12=-759/1238, 10-11=-504/879, 9-10=-99/167
WEBS 3-13=0/211, 3-11=-448/360, 5-11=-174/434, 5-10=-238/235, 6-10=-385/404, 7-10=-521/970, 7-9=-1024/687

JOINT STRESS INDEX
2 = 0.76, 3 = 0.32, 4 = 0.18, 5 = 0.76, 6 = 0.34, 7 = 0.39, 8 = 0.23, 9 = 0.59, 10 = 0.99, 11 = 0.25, 12 = 0.49 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 551 lb uplift at joint 2 and 547 lb uplift at joint 9.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105512
L128901	T07	HIP	6	1	Job Reference (optional)

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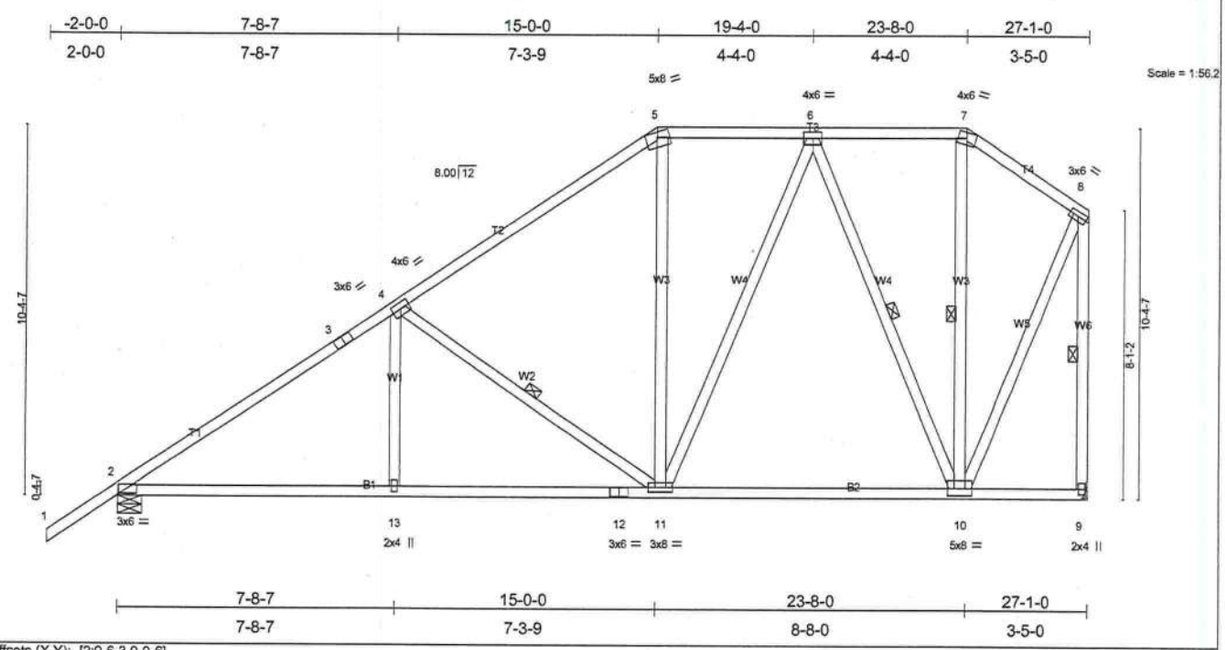

Plate Offsets (X,Y): [2:0-6-3,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.11	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.19	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-12 oc bracing.
WEBS 1 Row at midpt 4-11, 6-10, 7-10, 8-9

REACTIONS (lb/size) 2=1249/0-8-0, 9=1112/Mechanical
Max Horz 2=583(load case 5)
Max Uplift2=-563(load case 5), 9=-413(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1577/561, 3-4=-1374/583, 4-5=-1020/495, 5-6=-756/519, 6-7=-349/279, 7-8=-458/265, 8-9=-1097/571
BOT CHORD 2-13=-733/1204, 12-13=-733/1204, 11-12=-733/1204, 10-11=-315/592, 9-10=-5/4
WEBS 4-13=0/236, 4-11=-550/442, 5-11=-30/204, 6-11=-292/433, 6-10=-650/419, 7-10=-22/96, 8-10=-404/865

JOINT STRESS INDEX
2 = 0.75, 3 = 0.26, 4 = 0.32, 5 = 0.58, 6 = 0.34, 7 = 0.39, 8 = 0.69, 9 = 0.40, 10 = 0.48, 11 = 0.64, 12 = 0.50 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 2 and 413 lb uplift at joint 9.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105513
L128901	T07A	HIP	1	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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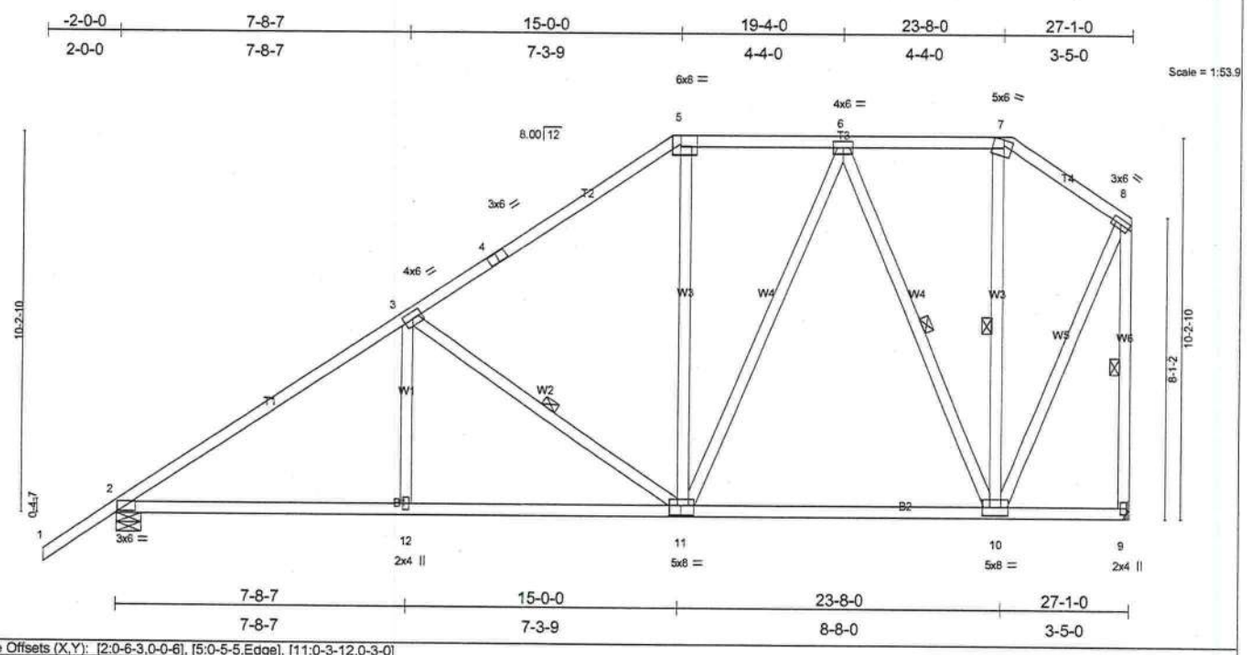

Plate Offsets (X,Y): [2:0-6-3,0-0-6], [5:0-5-5,Edge], [11:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.12	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.20	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 194 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-3 oc bracing.
WEBS 1 Row at midpt 3-11, 6-10, 7-10, 8-9

REACTIONS (lb/size) 2=1249/0-8-0, 9=1112/Mechanical
Max Horz 2=581(load case 5)
Max Uplift2=-563(load case 5), 9=-411(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1581/580, 3-4=-1026/468, 4-5=-908/497, 5-6=-763/517, 6-7=-353/275, 7-8=-456/262, 8-9=-1098/571
BOT CHORD 2-12=-734/1209, 11-12=-734/1209, 10-11=-322/601, 9-10=-5/4
WEBS 3-12=0/237, 3-11=-549/440, 5-11=-30/217, 6-11=-282/414, 6-10=-658/422, 7-10=-21/96, 8-10=-409/873

JOINT STRESS INDEX
2 = 0.75, 3 = 0.32, 4 = 0.34, 5 = 0.83, 6 = 0.33, 7 = 0.67, 8 = 0.69, 9 = 0.40, 10 = 0.48, 11 = 0.56 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 2 and 411 lb uplift at joint 9.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105514
L128901	T08	HIP	4	1	Job Reference (optional)

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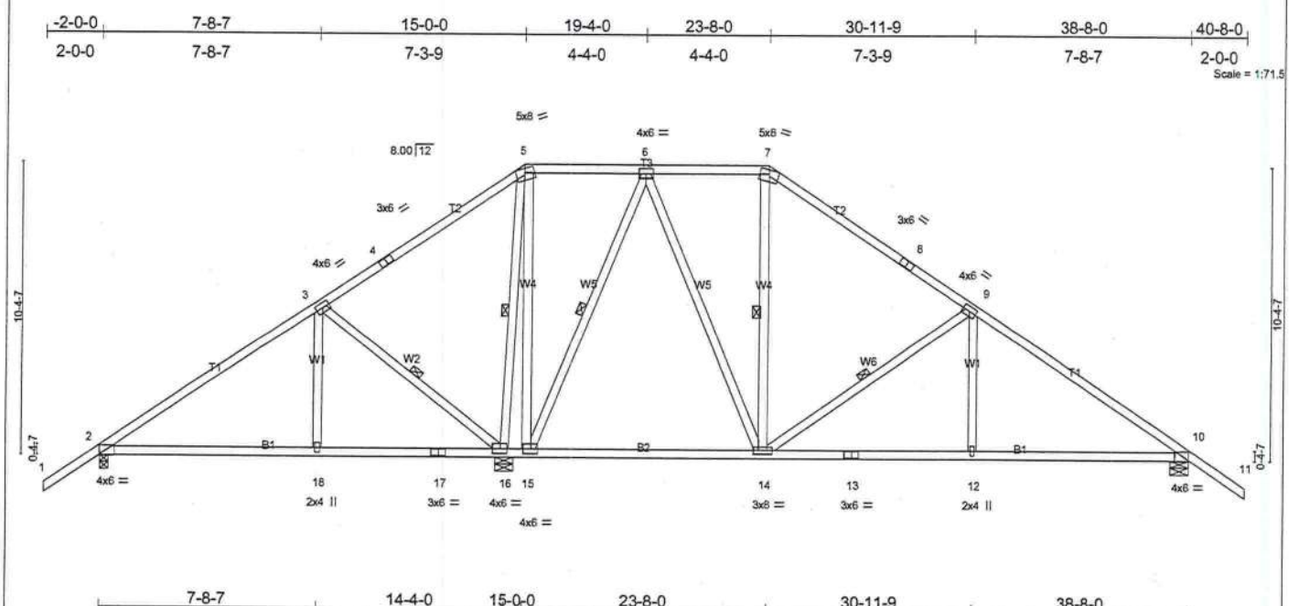

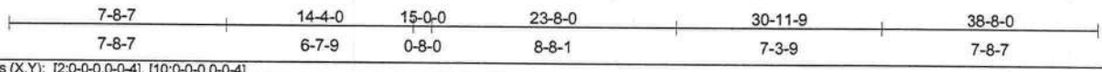

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [10:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.13	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.22	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.03	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 15-16.
WEBS 1 Row at midpt 3-16, 6-15, 7-14, 9-14, 5-16

REACTIONS (lb/size) 2=600/0-3-8, 16=1780/0-8-0, 10=1070/0-8-0
Max Horz 2=-425(load case 3)
Max Uplift2=-325(load case 5), 16=-634(load case 5), 10=-583(load case 6)
Max Grav 2=628(load case 7), 16=1780(load case 1), 10=1094(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-522/208, 3-4=-71/220, 4-5=-52/284, 5-6=0/253, 6-7=-528/496, 7-8=-635/469, 8-9=-751/438, 9-10=-1313/553, 10-11=0/61
BOT CHORD 2-18=-280/359, 17-18=-280/359, 16-17=-280/359, 15-16=-79/415, 14-15=-132/306, 13-14=-230/990, 12-13=-230/990, 10-12=-230/990
WEBS 3-18=0/232, 3-16=-582/455, 5-15=-265/796, 6-15=-812/403, 6-14=-276/632, 7-14=-9/60, 9-14=-564/443, 9-12=0/237, 5-16=-1214/459

JOINT STRESS INDEX
2 = 0.34, 3 = 0.32, 4 = 0.29, 5 = 0.65, 6 = 0.39, 7 = 0.59, 8 = 0.34, 9 = 0.32, 10 = 0.47, 12 = 0.34, 13 = 0.41, 14 = 0.75, 15 = 0.37, 16 = 0.29, 17 = 0.16 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 325 lb uplift at joint 2, 634 lb uplift at joint 16 and 583 lb uplift at joint 10.

LOAD CASE(S) Standard

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105515
L128901	T09	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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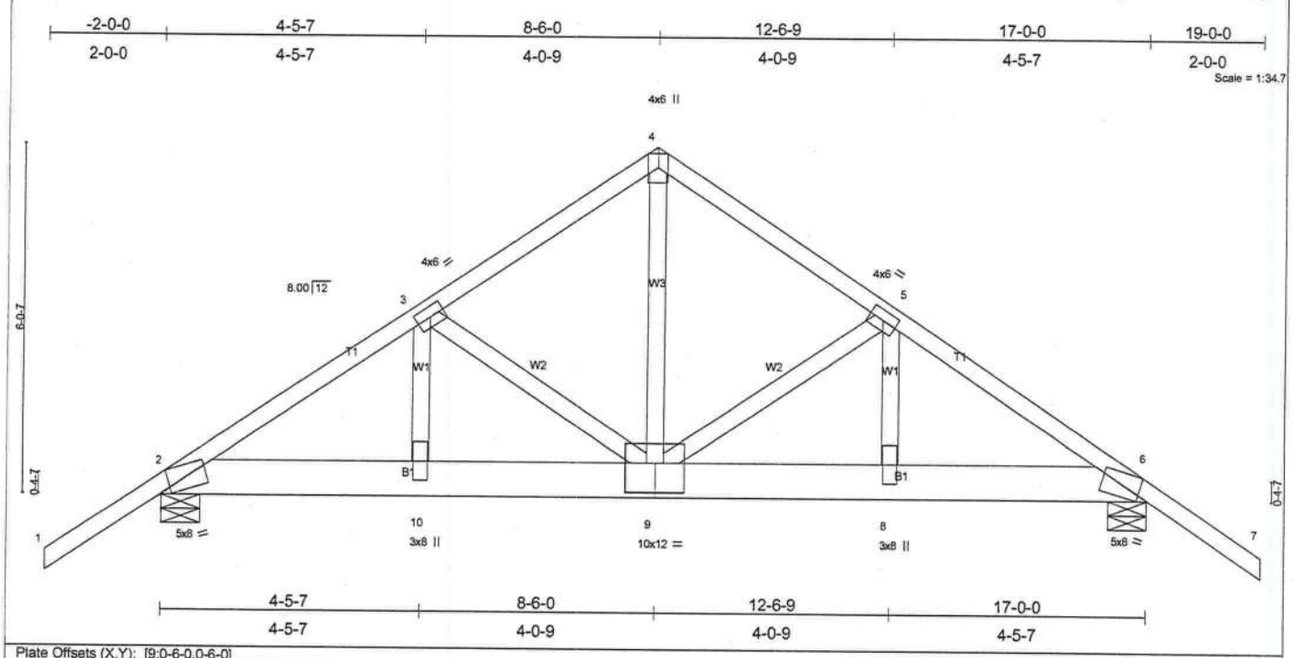

Plate Offsets (X,Y): [9:0-6-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.10	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.16	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 238 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=6529/0-8-0, 6=6529/0-8-0
Max Horz 2=239(load case 3)
Max Uplift2=-3282(load case 4), 6=-3282(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/66, 2-3=-8489/4057, 3-4=-5913/2908, 4-5=-5913/2909, 5-6=-8489/4054, 6-7=0/66
BOT CHORD 2-10=-3318/6932, 9-10=-3318/6932, 8-9=-3244/6932, 6-8=-3244/6932
WEBS 3-10=-1326/2837, 3-9=-2511/1297, 4-9=-3035/6237, 5-9=-2511/1300, 5-8=-1328/2837

JOINT STRESS INDEX
2 = 0.81, 3 = 0.80, 4 = 0.74, 5 = 0.80, 6 = 0.81, 8 = 0.46, 9 = 0.59 and 10 = 0.46

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3282 lb uplift at joint 2 and 3282 lb uplift at joint 6.
6) Girder carries tie-in span(s): 36-0-0 from 0-0-0 to 17-0-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-6=-730(F=-700)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105516
L128901	T09G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:46 2005 Page 1

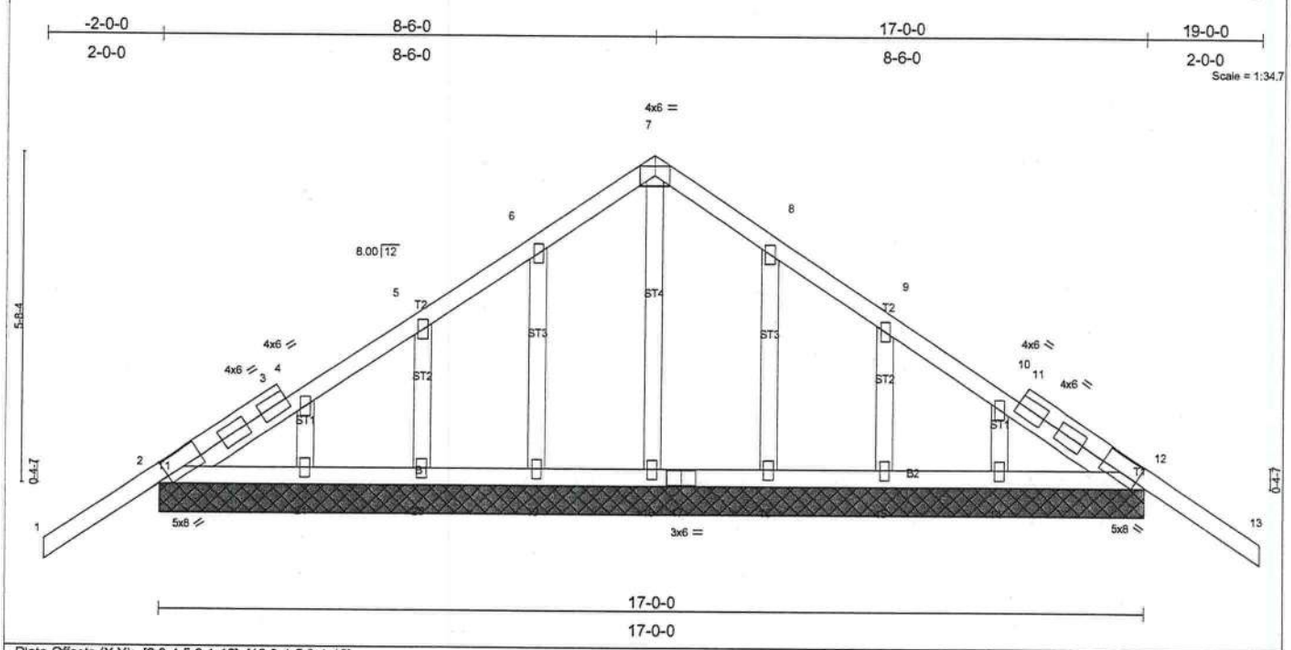

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [12:0-4-5,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.03	13	n/r	90	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.05	13	n/r	80		
BCLL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=394/17-0-0, 12=394/17-0-0, 18=199/17-0-0, 19=231/17-0-0, 20=244/17-0-0, 21=200/17-0-0, 16=231/17-0-0, 15=244/17-0-0, 14=200/17-0-0
Max Horz 2=-230(load case 3)
Max Uplift2=-266(load case 5), 12=-301(load case 6), 18=-2(load case 4), 19=-152(load case 5), 20=-186(load case 5), 21=-71(load case 6), 16=-148(load case 6), 15=-188(load case 6), 14=-64(load case 5)
Max Grav 2=394(load case 1), 12=394(load case 1), 18=199(load case 1), 19=236(load case 7), 20=244(load case 1), 21=200(load case 1), 16=236(load case 8), 15=244(load case 1), 14=200(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/94, 2-3=-154/151, 3-4=-156/152, 4-5=-117/138, 5-6=-82/171, 6-7=-81/212, 7-8=-81/208, 8-9=-82/134, 9-10=-78/49, 10-11=-72/68, 11-12=-104/66, 12-13=-8/94
BOT CHORD 2-21=-25/194, 20-21=-25/194, 19-20=-25/194, 18-19=-25/194, 17-18=-25/194, 16-17=-25/194, 15-16=-25/194, 14-15=-25/194, 12-14=-25/194
WEBS 7-18=-139/13, 6-19=-176/167, 5-20=-181/188, 4-21=-147/93, 8-16=-176/162, 9-15=-181/190, 10-14=-147/98

JOINT STRESS INDEX
2 = 0.65, 3 = 0.00, 3 = 0.25, 3 = 0.25, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.28, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.00, 11 = 0.25, 11 = 0.25, 12 = 0.65, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.15, 18 = 0.34, 19 = 0.34, 20 = 0.34 and 21 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2, 301 lb uplift at joint 12, 2 lb uplift at joint 18, 152 lb uplift at joint 19, 186 lb uplift at joint 20, 71 lb uplift at joint 21, 148 lb uplift at joint 16, 188 lb uplift at joint 15 and 64 lb uplift at joint 14.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-87(F=-33), 7-13=-87(F=-33), 2-12=-30

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105517
L128901	T10	COMMON	7	1	

Builders FirstSource, Lake City, Fl 32055 — Job Reference (optional)

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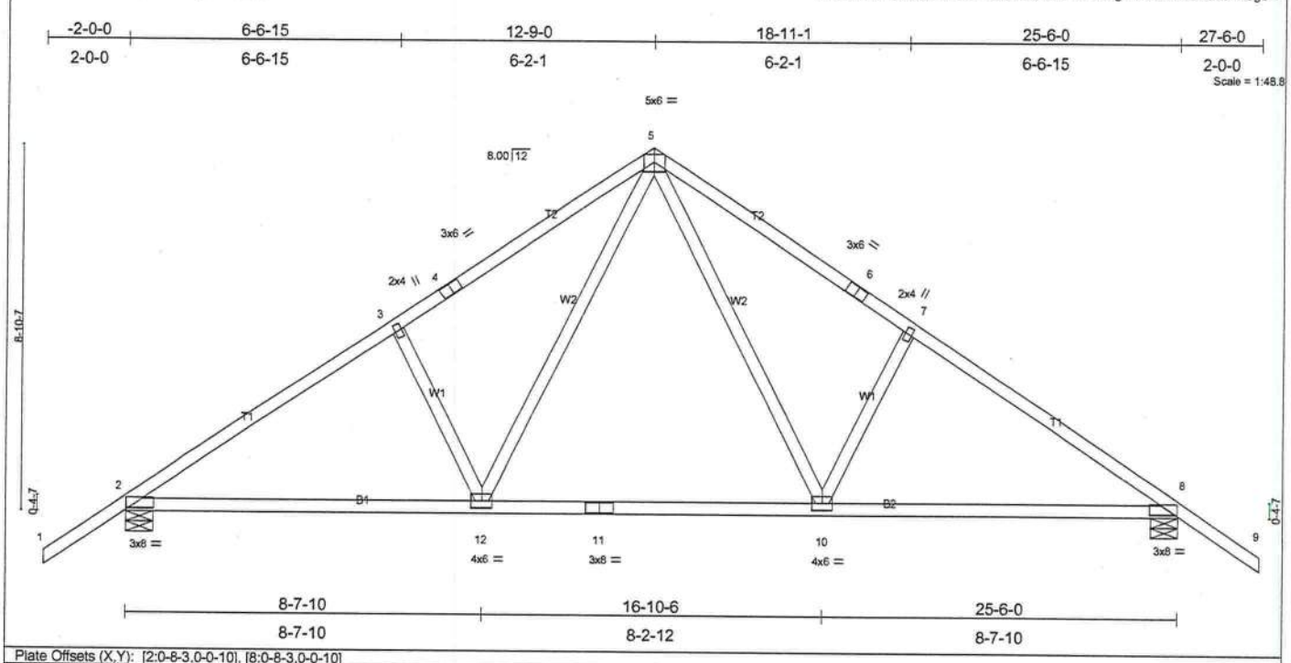

Plate Offsets (X,Y): [2:0-8-3,0-0-10], [8:0-8-3,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.30	10-12	>983	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.49	10-12	>609	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-12 oc bracing.

REACTIONS (lb/size) 2=1375/0-8-0, 8=1375/0-8-0
Max Horz 2=-361(load case 3)
Max Uplift2=-661(load case 5), 8=-661(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/61, 2-3=-1828/782, 3-4=-1670/832, 4-5=-1578/859, 5-6=-1578/859, 6-7=-1670/832, 7-8=-1828/782, 8-9=0/61
BOT CHORD 2-12=-589/1415, 11-12=-257/973, 10-11=-257/973, 8-10=-425/1415
WEBS 3-12=-256/352, 5-12=-414/802, 5-10=-414/802, 7-10=-256/352

JOINT STRESS INDEX
2 = 0.82, 3 = 0.34, 4 = 0.30, 5 = 0.64, 6 = 0.30, 7 = 0.34, 8 = 0.82, 10 = 0.47, 11 = 0.85 and 12 = 0.47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 661 lb uplift at joint 2 and 661 lb uplift at joint 8.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-9=-54, 2-12=-30, 10-12=-80(F=-50), 8-10=-30

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105518
L128901	T10G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Aug 30 09:40:47 2005 Page 1

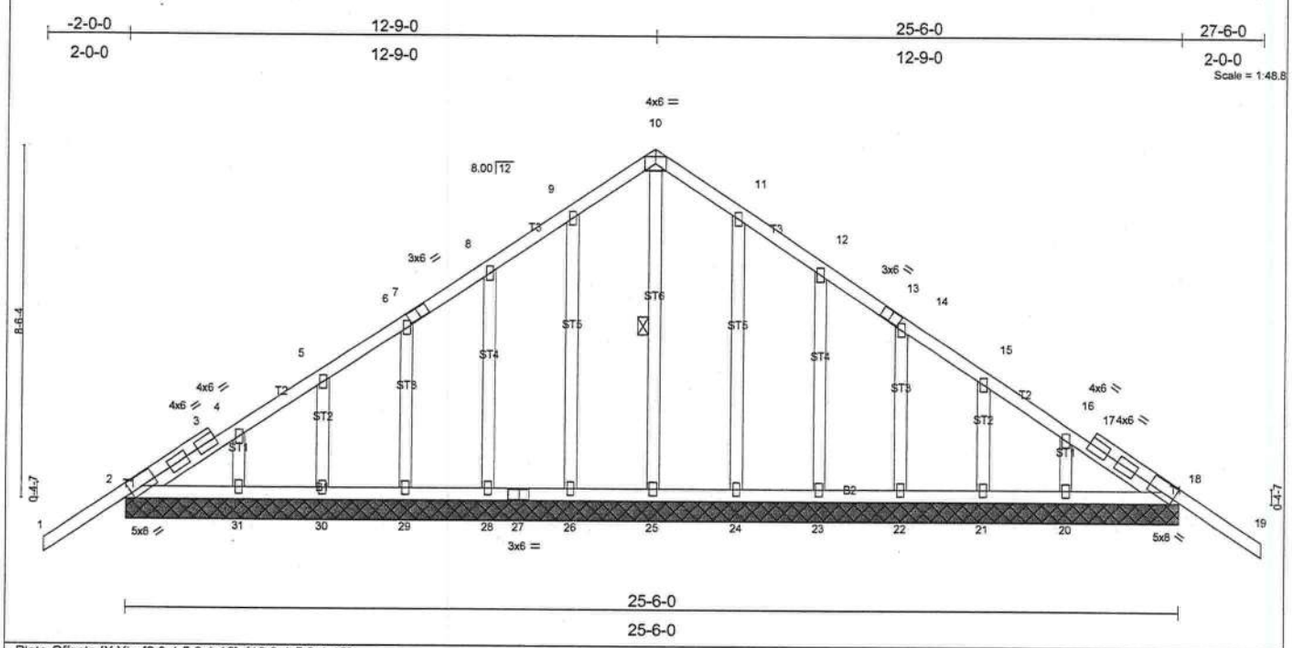

Plate Offsets (X,Y): [2:0-4-5,0-1-12], [18:0-4-5,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.03	19	n/r	90	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.05	19	n/r	80		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	0.01	18	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 10-25

REACTIONS (lb/size) 2=401/25-6-0, 25=200/25-6-0, 26=232/25-6-0, 28=235/25-6-0, 29=233/25-6-0, 30=239/25-6-0, 31=225/25-6-0, 24=232/25-6-0, 23=235/25-6-0, 22=233/25-6-0, 21=239/25-6-0, 20=225/25-6-0, 18=401/25-6-0
Max Horz 2=-347(load case 3)
Max Uplift 2=-212(load case 5), 26=-148(load case 5), 28=-169(load case 5), 29=-155(load case 5), 30=-185(load case 5), 31=-81(load case 6), 24=-141(load case 6), 23=-171(load case 6), 22=-154(load case 6), 21=-184(load case 6), 20=-71(load case 5), 18=-258(load case 6)
Max Grav 2=401(load case 1), 25=207(load case 6), 26=237(load case 7), 28=235(load case 1), 29=233(load case 7), 30=239(load case 7), 31=225(load case 1), 24=237(load case 8), 23=235(load case 1), 22=233(load case 8), 21=239(load case 8), 20=225(load case 1), 18=401(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/94, 2-3=-263/214, 3-4=-264/215, 4-5=-225/205, 5-6=-164/199, 6-7=-105/210, 7-8=-106/203, 8-9=-81/265, 9-10=-80/301, 10-11=-80/301, 11-12=-81/232, 12-13=-33/137, 13-14=-81/143, 14-15=-82/68, 15-16=-94/74, 16-17=-140/91, 17-18=-138/89, 18-19=-8/94
BOT CHORD 2-31=-46/273, 30-31=-46/273, 29-30=-46/273, 28-29=-46/273, 27-28=-46/273, 26-27=-46/273, 25-26=-46/273, 24-25=-46/273, 23-24=-46/273, 22-23=-46/273, 21-22=-46/273, 20-21=-46/273, 18-20=-46/273
WEBS 10-25=-196/5, 9-26=-177/160, 8-28=-175/180, 6-29=-173/169, 5-30=-179/187, 4-31=-165/102, 11-24=-177/153, 12-23=-175/183, 14-22=-173/169, 15-21=-179/186, 16-20=-165/108

JOINT STRESS INDEX
2 = 0.65, 3 = 0.00, 3 = 0.25, 3 = 0.25, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.15, 8 = 0.34, 9 = 0.34, 10 = 0.28, 11 = 0.34, 12 = 0.34, 13 = 0.15, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.00, 17 = 0.25, 17 = 0.25, 18 = 0.65, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.15, 28 = 0.34, 29 = 0.34, 30 = 0.34 and 31 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 2, 148 lb uplift at joint 26, 169 lb uplift at joint 28, 155 lb uplift at joint 29, 185 lb uplift at joint 30, 81 lb uplift at joint 31, 141 lb uplift at joint 24, 171 lb uplift at joint 23, 154 lb uplift at joint 22, 184 lb uplift at joint 21, 71 lb uplift at joint 20 and 258 lb uplift at joint 18.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-10=-87(F=-33), 10-19=-87(F=-33), 2-18=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	Dwg.#083105519
L128901	T11	COMMON	1	2	Job Reference (optional)

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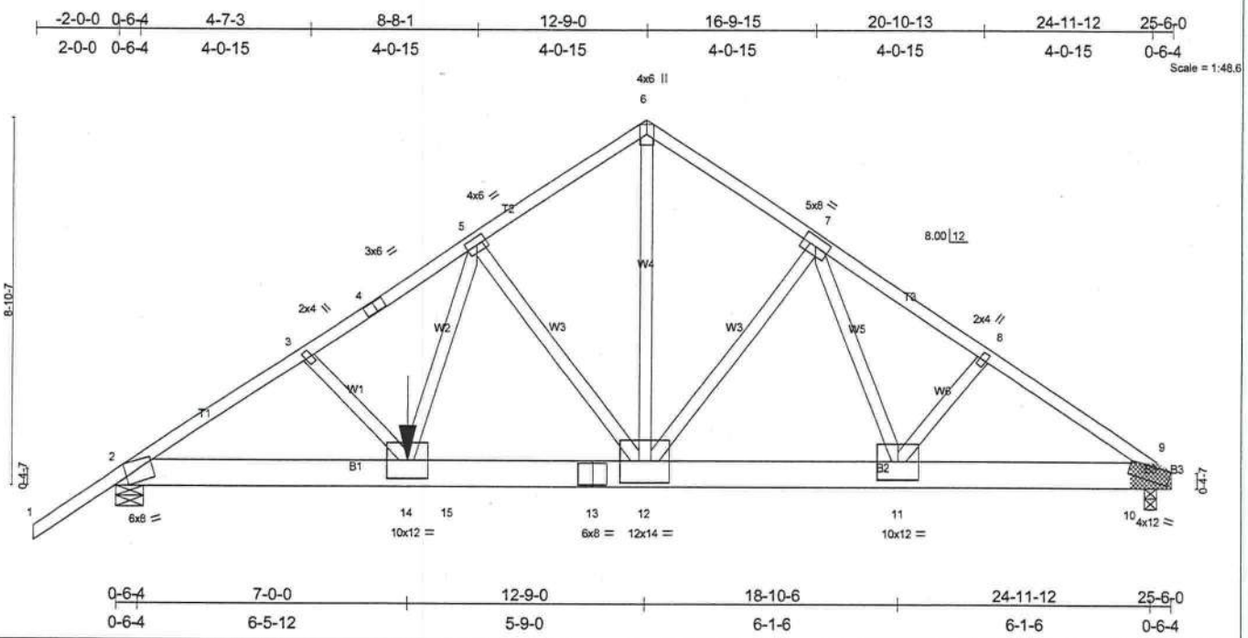

Plate Offsets (X,Y): [2:0-3-7,Edge], [9:1-1-9,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.20	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.32	12-14	>938	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.77	Horz(TL)	-0.07	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 384 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-7 oc bracing.

REACTIONS (lb/size) 9=7582/0-3-13 (0-3-8 + bearing block), 2=6064/0-8-0
Max Horz 9=386(load case 3)
Max Uplift9=-3646(load case 5), 2=-2973(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=-7129/3517, 7-8=-10586/5136, 8-9=-10716/5130, 1-2=0/66, 2-3=-10019/4704, 3-4=-9882/4695, 4-5=-9834/4707, 5-6=-7143/3532
BOT CHORD 2-14=-3759/8164, 14-15=-3315/7244, 13-15=-3315/7244, 12-13=-3315/7244, 11-12=-3518/7390, 10-11=-4360/8904, 9-10=-4360/8904
WEBS 8-11=-259/283, 7-11=-1998/4028, 7-12=-2550/1418, 6-12=-3723/7581, 5-12=-2301/1238, 5-14=-1557/3326, 3-14=-131/153

JOINT STRESS INDEX
2 = 0.91, 3 = 0.34, 4 = 0.84, 5 = 0.90, 6 = 0.91, 7 = 0.68, 8 = 0.34, 9 = 0.82, 9 = 0.00, 9 = 0.00, 10 = 0.00, 10 = 0.00, 10 = 0.00, 11 = 0.38, 12 = 0.47, 13 = 0.60 and 14 = 0.34

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 8 SYP No.1D bearing block 12" long at jt. 9 attached to each face with 4 rows of 0.131"x3" Nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3646 lb uplift at joint 9 and 2973 lb uplift at joint 2.
7) Girder carries tie-in span(s): 27-1-0 from 8-0-0 to 25-6-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2383 lb down and 1148 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 6-9=-54, 1-6=-54, 2-15=-30, 9-15=-551(F=-521)
Concentrated Loads (lb)
Vert: 14=-2383(F)

AUGUST 31, 2005 TRUSS DESIGN ENGINEER:
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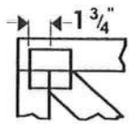
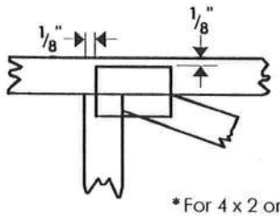
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

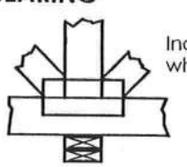
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

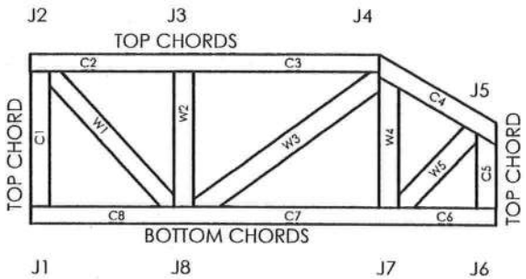
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek® TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0509-81 Date Received LH By LH Permit # 853/23741

Application Approved by - Zoning Official BLK Date 9.19.05 Plans Examiner OK JTH Date 9-29-05

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments

Applicants Name Katie Reed Phone 386-752-4072

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Owners Name David and Judith Onorati Phone 386-752-4072

911 Address 198 SW Stillview Glen Fort White, FL 32038

Contractors Name Don Reed Construction, Inc. Phone 386-752-4072

Address 2230 SE Baya Drive Suite 101 Lake City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disosway P.E. P.O. Box 868 Lake City, FL 32056

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 11-6S-16-03815-120 Estimated Cost of Construction $189,300.00

Subdivision Name Cardinal Farms Lot 20 Block ____ Unit ____ Phase ____

Driving Directions 47S To Herlong Rd; TL on Herlong; TR onto Skyline Loop;
TL on Stillview; 2nd lot on the right

Type of Construction single family dwelling Number of Existing Dwellings on Property 1

Total Acreage 10.010 Lot Size _____ Do you need a - Culvert Permit or (Culvert Waiver) or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 250' Side 196' Side 144' Rear 687'

Total Building Height 23' Number of Stories 1 Heated Floor Area 2,226 Roof Pitch 8/12

Porches 1023 GARAGE 616 TOTAL 3,865

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

Don Reed
Contractor Signature

Contractors License Number CGC036224

Competency Card Number

NOTARY STAMP/SEAL

Notary Public State of Florida
Ingrid Geiger
My Commission DD385312
Expires 01/26/2009

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 28th day of September 2005.

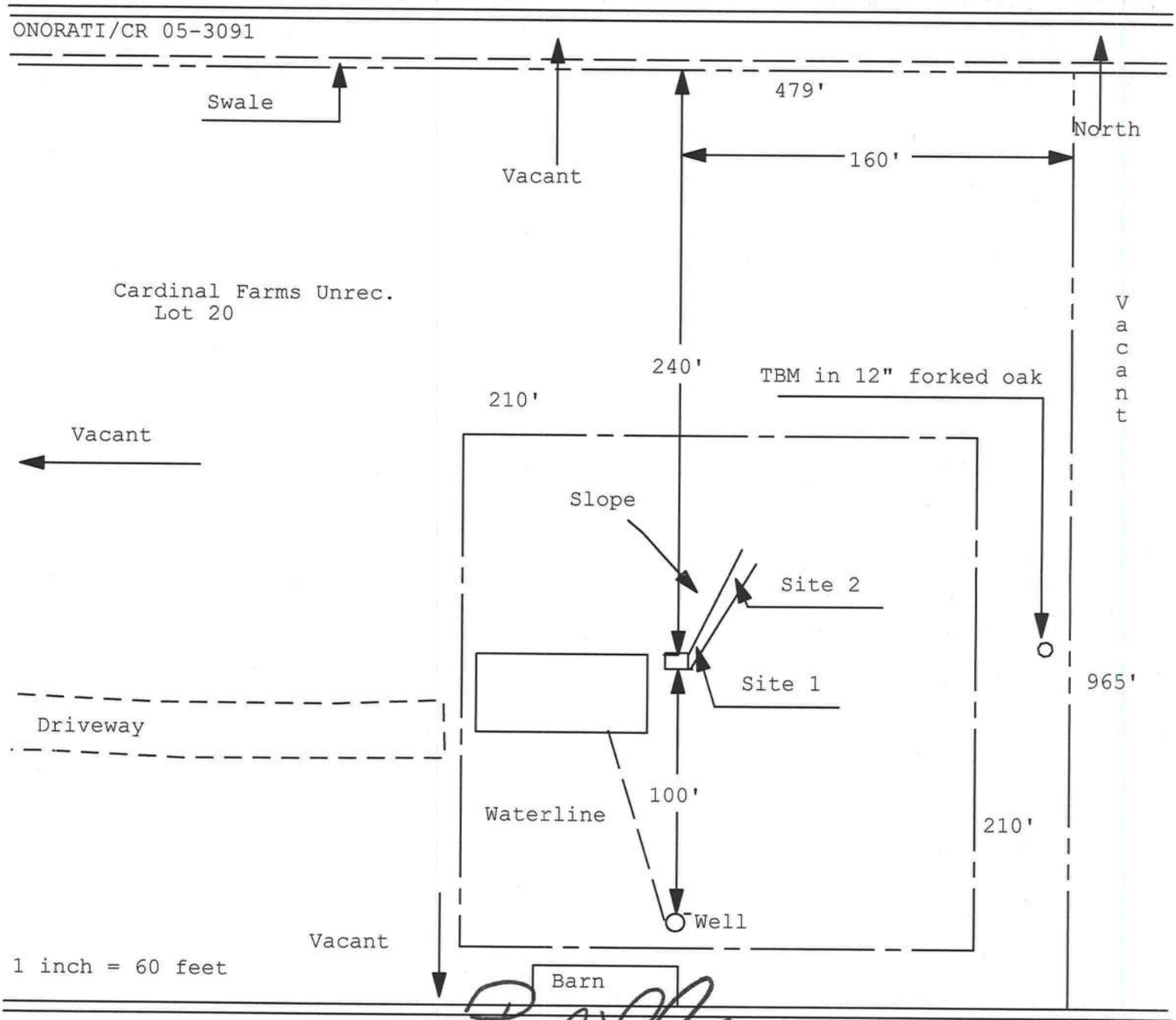
Personally known ✓ or Produced Identification ____

Ingrid Geiger
Notary Signature

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-0963N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

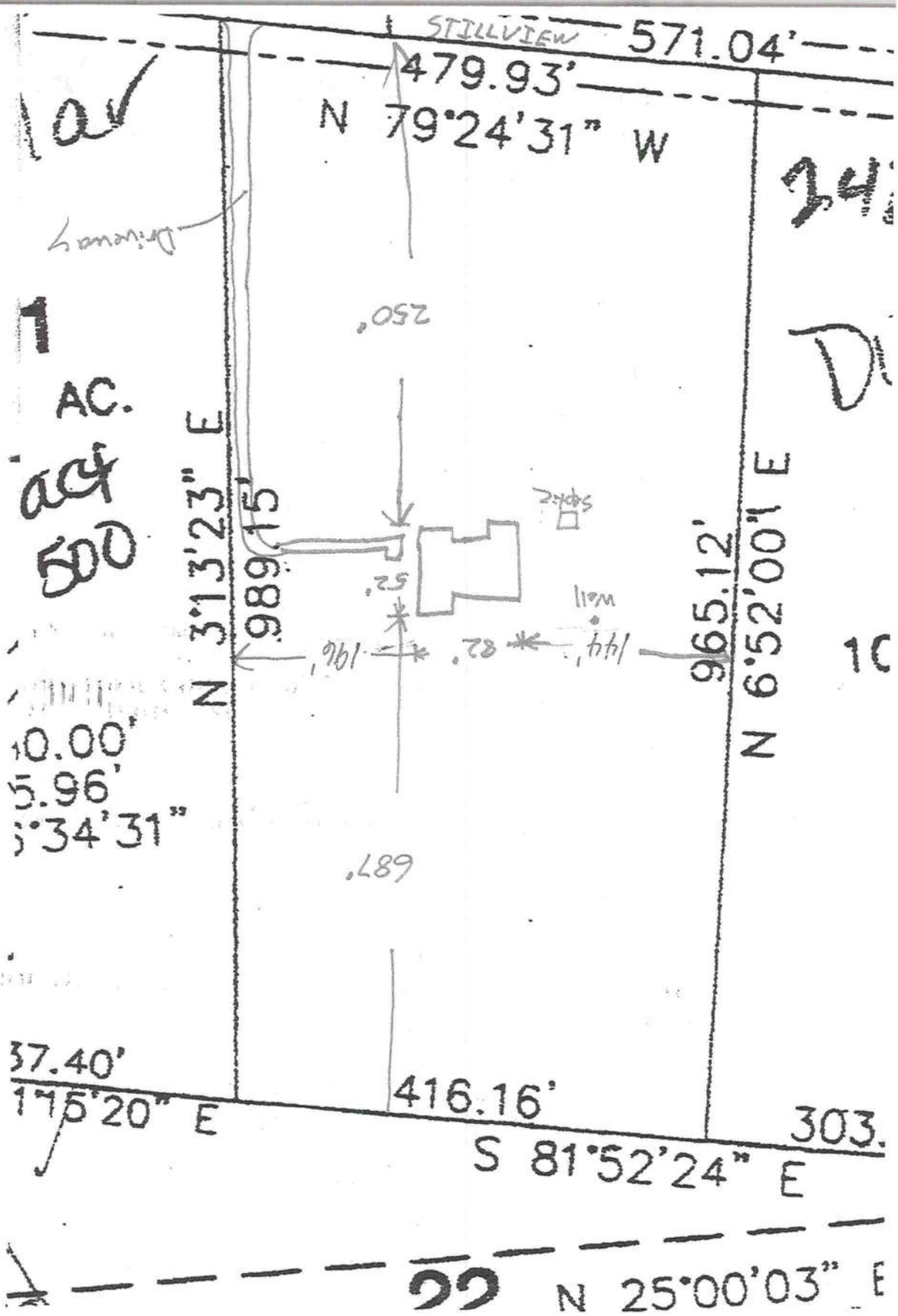


Site Plan Submitted By Paul Long **Date** 9/15/05

Plan Approved ✓ **Not Approved** ____ **Date** 9/26/05

By ____ Columbia **CPHU**

Notes: ____


STILLVIEW
571.04'
479.93'
N 79°24'31" W
Driveway
250'
AC.
N 3°13'23" E
989.15'
965.12'
N 6°52'00"1 E
687'
416.16'
37.40'
S 81°52'24" E
N 25°00'03" E
N

11-6S-16-03815-120

LOT 20 CARDINAL FARMS UNREC:
COMM SE COR & RUN W 3266.86 FT
N 22 DG E 510.42 FT, N 915.56
FT, N 22 DG E 1397.36 FT,

ONORATI DAVID & JUDITH
3392 CUSTER AVE
LAKE WORTH FL 33467

11-6S-16-03815-120

Columbia County 2005 R
CARD 001 of 001
PRINTED 6/02/2005 9:55 BY JEFF
APPR 2/06/2004 TW

USE		AE?	HTD AREA	.000	INDEX	11616.00	NBHD	PROP USE	009900 NO AG ACREAGE
MOD	BATH		EFF AREA		E-RATE	.000	INDX	STR 11- 6S- 16	
EXW	FIXT		RCN				AYB	MKT AREA 02	0 BLDG
%	BDRM		%GOOD		BLDG VAL		EYB	(PUD1	0 XFOB
RSTR	RMS							AC 10.010	42,042 LAND
RCVR	UNTS		FIELD CK:					NTCD	0 AG
%	C-W%		LOC:					APPR CD	0 MKAG
INT	HGHT							CNDO	42,042 JUST
%	PMTR							SUBD	0 CLAS
FLR	STYS							BLK	
%	ECON							LOT	0 SOHD
HTTP	FUNC							MAP# 76	0 ASSD
A/C	SPCD								0 EXPT
QUAL	DEPR							TXDT 003	0 COTXBL
FNDN	UD-1								
SIZE	UD-2							BLDG TRAVERSE	
CEIL	UD-3								
ARCH	UD-4								
FRME	UD-5								
KTCH	UD-6								
WNDO	UD-7								
CLAS	UD-8								
OCC	UD-9								
COND	%								

SUB A-AREA % E-AREA SUB VALUE

PERMITS

NUMBER	DESC	AMT	ISSUED

SALE

BOOK	PAGE	DATE		PRICE
1043	1556	4/11/2005	Q V	100000
GRANTOR CARMEN FAVORITO				
GRANTEE DAVID & JUDITH ONORATI				
1024	508	8/06/2004	U V	50000
GRANTOR SUBRANDY LIMITED PARTNERSHIP				
GRANTEE CARMEN P FAVORITO				

TOTAL

FIELD CK:

-------EXTRA FEATURES----------

AE	BN	CODE	DESC	LEN	WID	HGHT	QTY	QL	YR	ADJ	UNITS	UT	PRICE	ADJ UT PR	SPCD %	%GOOD	XFOB VALUE

AE	LAND CODE	DESC	ZONE	TOPO	ROAD	UTIL	{UD1	{UD2	{UD3	{UD4	FRONT	BACK	DEPTH	DT	FIELD CK: ADJUSTMENTS	UNITS	UT	PRICE	ADJ UT PR	LAND VALUE
Y	009900	AC NON-AG	A-1	0002	0007	0003					1.00	1.00	1.00	1.00		10.010	AC	4200.000	4200.00	42,042

L001 - LOT 20
2005

Permit No. _____ Tax Parcel No. _____

COLUMBIA COUNTY NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. **Description of property:** (legal description of the property, and street address if available.)

See Attached

2. **General description of improvement:** Single family dwelling

3. **Owner information:**

A. **Name and address:**

David and Judith Onorati

P.O. Box 400 Fort White, FL 32038

B. **Interest in property:**

100%

C. **Name and address of fee simple titleholder (if other than owner):**

N/A

4. **Contractor:** (name and address)

Don Reed Construction, Inc.

2230 SE Baya Drive Suite 101 Lake City, FL 32025

5. **Surety**

A. **Name and address:** N/A

B. **Amount of bond:**

N/A

6. **Lender:** (name and address) N/A

7. **Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 718.13 (1) (a) 7., Florida Statutes:** (name and address) _____

Inst:200501898 3 Date:08/08/2005 Time:13:42
DC,P.Dewitt Cason,Columbia County B:1054 P:910

8. In addition to himself, owner designates _____ of _____ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (a) 7., Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

_____
(Signature of Owner)

SWORN TO and subscribed before me this 5th day of August 2005.

_____
Notary Public

(NOTARIAL SEAL)

My Commission Expires: 1/24/2009

Notary Public State of Florida
Ingrid Geiger
My Commission DD385312
Expires 01/26/2009

Inst:2005018983 Date:08/08/2005 Time:13:42
_____DC,P.DeWitt Cason,Columbia County B:1054 P:911

Inst:2005018983 Date:08/08/2005 Time:13:42
DC,P.DeWitt Cason,Columbia County B:1054 P:912

This Instrument Prepared by & return to:
Name: ***JOYCE KIRPACH, an employee of TITLE OFFICES, LLC***
Address: ***1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025***
File No. 05Y-03186JK

Inst:2005008821 Date:04/18/2005 Time:09:38
Doc Stamp-Deed : 700.00
DC,P.DeWitt Cason,Columbia County B:1043 P:1556

Parcel I.D. #: ***03815-000***

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the 11th day of April, A.D. 2005, by*

CARMEN P. FAVORITO, married *hereinafter called the grantor, to*

DAVID ONORATI and JUDITH ONORATI, HIS WIFE, *whose post office address is*

3392 CUSTER AVE., LAKE WORTH, FL. 33467, *hereinafter called the grantees:*

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in* ***Columbia County, State of FLORIDA,*** *viz:*

LOT 20 OF AN UNRECORDED SUBDIVISION KNOWN AS CARDINAL FARMS

A PARCEL OF LAND IN SECTION 11, TOWNSHIP 6 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHEAST CORNER OF SECTION 11, TOWNSHIP 6 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE SOUTH 88°19'59" WEST ALONG THE SOUTH LINE OF SAID SECTION 11 A DISTANCE OF 3266.86 FEET; THENCE NORTH 22°15'30" EAST A DISTANCE OF 510.42 FEET; THENCE NORTH 01°40'01" WEST A DISTANCE OF 915.56 FEET; THENCE NORTH 22°03'23" EAST A DISTANCE OF 1397.36 FEET; THENCE NORTH 25°00'03" EAST A DISTANCE OF 2.82 FEET TO A POINT ON THE SOUTH LINE OF THE NORTH ½ OF SECTION 11; THENCE CONTINUE NORTH 25°00'03" EAST A DISTANCE OF 36.48 FEET; THENCE NORTH 81°52'24" WEST A DISTANCE OF 303.59 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE NORTH 81°52'24" WEST A DISTANCE OF 416.16 FEET; THENCE NORTH 03°13'23" EAST A DISTANCE OF 989.15 FEET; THENCE NORTH 79°24'31" WEST A DISTANCE OF 479.93 FEET; THENCE SOUTH 06°52'00" WEST A DISTANCE OF 965.12 FEET TO THE POINT OF BEGINNING.

Subject to declaration of covenants, conditions and restrictions as recorded in Official Records Book 1012 Page 905, but omitting any covenant or restrictions as to race, color, religion, sex, handicap, familial status or national origin.

Easement in favor of CLAY ELECTRIC COOPERATIVE, INC., recorded in Official Records Book 836, Page 1284, of the Public Records of Columbia County, FLORIDA.

Subject to power line easement, per instrument recorded in Official Records Book 1024, Page 508.

Subject to roadway easement to COLUMBIA COUNTY, FLORIDA, per instrument recorded in Official Records Book 1024, Page 508.

The above described property is not the homestead property of Grantor.

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

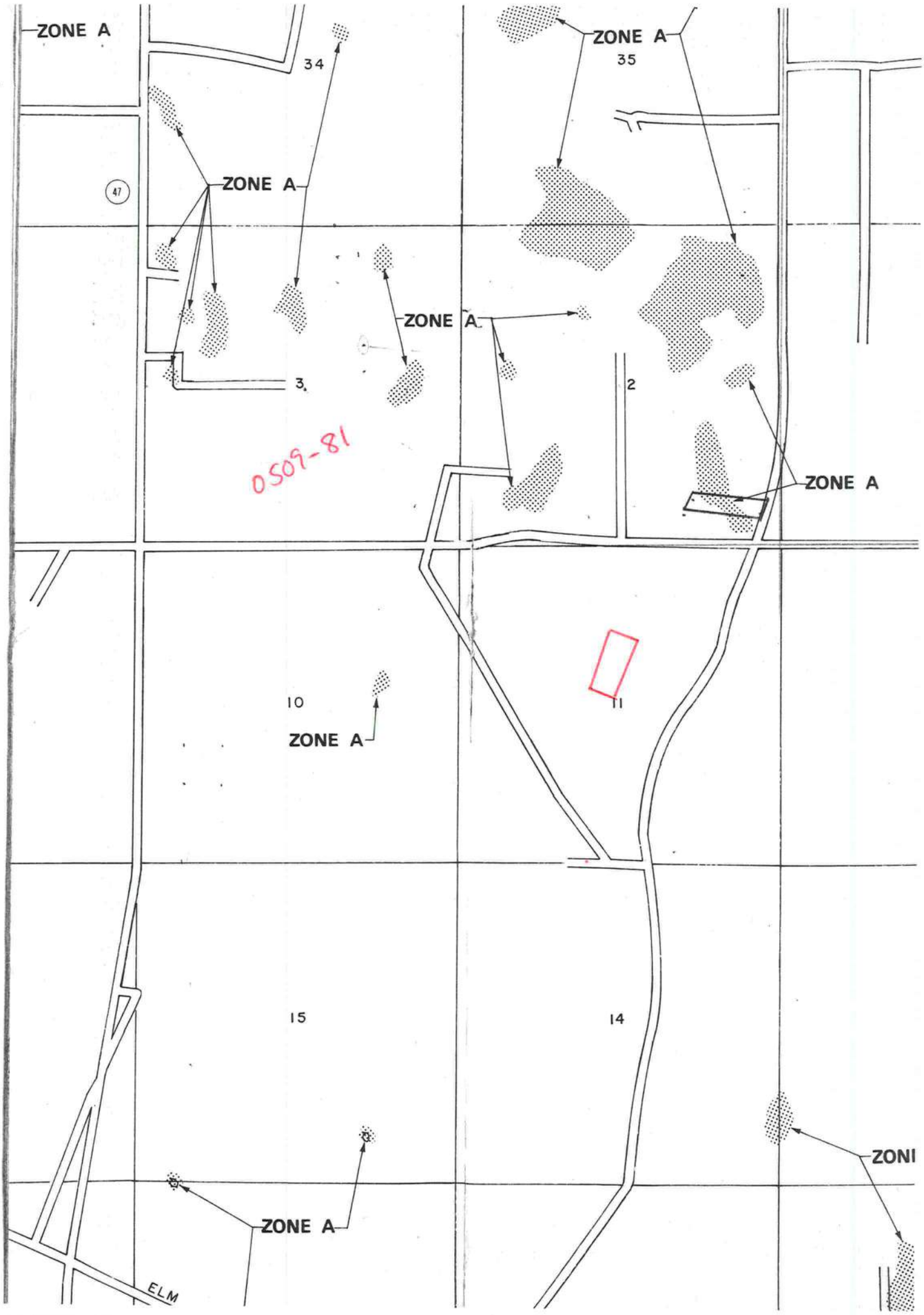
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk


ZONE A
34
ZONE A
35
ZONE A
47
ZONE A
3
2
0509-81
ZONE A
10
ZONE A
11
15
14
ZONE A
ZONI
ELM

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Onorati Residence**	Builder:	**Don Reed.**
Address:	**Stillview Rd.**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	23741
Owner:	**David & Judith Onorati**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	3	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2226 ft²	
7.	Glass area & type		
	a. Clear - single pane	0.0 ft²	
	b. Clear - double pane	225.0 ft²	
	c. Tint/other SHGC - single pane	0.0 ft²	
	d. Tint/other SHGC - double pane	0.0 ft²	
8.	Floor types		
	a. Slab-On-Grade Edge Insulation	R=0.0, 267.0(p) ft	
	b. N/A		
	c. N/A		
9.	Wall types		
	a. Frame, Wood, Adjacent	R=13.0, 360.0 ft²	
	b. Concrete, Int Insul, Exterior	R=13.0, 1784.0 ft²	
	c. N/A		
	d. N/A		
	e. N/A		
10.	Ceiling types		
	a. Under Attic	R=30.0, 2226.0 ft²	
	b. N/A		
	c. N/A		
11.	Ducts		
	a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 154.0 ft	
	b. N/A		

12.	Cooling systems		
	a. Central Unit	Cap: 30.0 kBtu/hr	
		SEER: 10.00	
	b. N/A		
	c. N/A		
13.	Heating systems		
	a. Electric Heat Pump	Cap: 36.0 kBtu/hr	
		HSPF: 6.80	
	b. N/A		
	c. N/A		
14.	Hot water systems		
	a. Electric Resistance	Cap: 50.0 gallons	
		EF: 0.90	
	b. N/A		
	c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.10

Total as-built points: 28907

Total base points: 32866

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 8-10-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA — IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,**	PERMIT #:

BASE				AS-BUILT								
WATER HEATING												
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X	Credit Multiplier	= Total
3		2746.00	8238.0	50.0	0.90	3		1.00	2684.98		1.00	8054.9
				As-Built Total:								**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
12179	**12449**	**8238**	**32866**	**9421**	**11430**	**8055**	**28907**

PASS

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,**	PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**19841.7**	**Winter As-Built Points:**						**18239.9**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
19841.7	**0.6274**	**12448.7**	18239.9 **18239.9**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.501 **0.501**	1.000 **1.000**	11430.3 **11430.3**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2226.0	12.74	5104.7	Double, Clear	E	1.5	8.0	30.0	9.09	1.02	278.1	
				Double, Clear	E	8.0	6.5	60.0	9.09	1.34	733.4	
				Double, Clear	E	1.5	6.0	10.0	9.09	1.04	94.1	
				Double, Clear	N	1.5	5.0	16.0	14.30	1.00	229.7	
				Double, Clear	W	1.5	6.0	30.0	10.77	1.02	330.5	
				Double, Clear	W	14.0	4.0	9.0	10.77	1.24	119.9	
				Double, Clear	W	14.0	8.0	40.0	10.77	1.22	524.1	
				Double, Clear	W	14.0	6.0	30.0	10.77	1.23	397.0	
				As-Built Total:				**225.0**			**2706.9**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	360.0	3.60	1296.0	Frame, Wood, Adjacent			13.0	360.0	3.30		1188.0	
Exterior	1784.0	3.70	6600.8	Concrete, Int Insul, Exterior			13.0	1784.0	2.72		4861.4	
Base Total:	**2144.0**		**7896.8**	**As-Built Total:**				**2144.0**			**6049.4**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	230.0	Exterior Wood				40.0	12.30		492.0	
Exterior	80.0	12.30	984.0	Adjacent Wood				20.0	11.50		230.0	
				Exterior Wood				40.0	12.30		492.0	
Base Total:	**100.0**		**1214.0**	**As-Built Total:**				**100.0**			**1214.0**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	2226.0	2.05	4563.3	Under Attic			30.0	2226.0	2.05 X	1.00	4563.3	
Base Total:	**2226.0**		**4563.3**	**As-Built Total:**				**2226.0**			**4563.3**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	267.0(p)	8.9	2376.3	Slab-On-Grade Edge Insulation			0.0	267.0(p	18.80		5019.6	
Raised	0.0	0.00	0.0									
Base Total:			**2376.3**	**As-Built Total:**				**267.0**			**5019.6**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	2226.0	-0.59	-1313.3					2226.0	-0.59		-1313.3	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,**	PERMIT #:

BASE				AS-BUILT							
Summer Base Points:			**28549.9**	**Summer As-Built Points:**							**22079.6**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points	
28549.9	**0.4266**		**12179.4**	22079.6 **22079.6**	1.000 **1.00**	(1.090 x 1.147 x 1.00) **1.250**	0.341 **0.341**	1.000 **1.000**		9421.5 **9421.5**	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Stillview Rd., Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2226.0	20.04	8029.6	Double, Clear	E	1.5	8.0	30.0	40.22	0.96	1155.5	
				Double, Clear	E	8.0	6.5	60.0	40.22	0.47	1123.3	
				Double, Clear	E	1.5	6.0	10.0	40.22	0.91	367.1	
				Double, Clear	N	1.5	5.0	16.0	19.22	0.92	281.5	
				Double, Clear	W	1.5	6.0	30.0	36.99	0.91	1013.5	
				Double, Clear	W	14.0	4.0	9.0	36.99	0.37	124.7	
				Double, Clear	W	14.0	8.0	40.0	36.99	0.42	622.8	
				Double, Clear	W	14.0	6.0	30.0	36.99	0.39	436.8	
				As-Built Total:				**225.0**			**5125.2**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	360.0	0.70	252.0	Frame, Wood, Adjacent	13.0	360.0	0.60	216.0
Exterior	1784.0	1.70	3032.8	Concrete, Int Insul, Exterior	13.0	1784.0	0.35	624.4
Base Total:	**2144.0**		**3284.8**	**As-Built Total:**		**2144.0**		**840.4**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	20.0	2.40	48.0	Exterior Wood	40.0	6.10	244.0
Exterior	80.0	6.10	488.0	Adjacent Wood	20.0	2.40	48.0
				Exterior Wood	40.0	6.10	244.0
Base Total:	**100.0**		**536.0**	**As-Built Total:**	**100.0**		**536.0**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	2226.0	1.73	3851.0	Under Attic	30.0	2226.0	1.73 X 1.00	3851.0
Base Total:	**2226.0**		**3851.0**	**As-Built Total:**		**2226.0**		**3851.0**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	267.0(p)	-37.0	-9879.0	Slab-On-Grade Edge Insulation	0.0	267.0(p	-41.20	-11000.4
Raised	0.0	0.00	0.0					
Base Total:			**-9879.0**	**As-Built Total:**		**267.0**		**-11000.4**

INFILTRATION	Area X	BSPM	= Points		Area X	SPM	= Points
	2226.0	10.21	22727.5		2226.0	10.21	22727.5

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.7

The higher the score, the more efficient the home.

David & Judith Onorati, Stillview Rd., Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	3	b.	N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	2226 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	225.0 ft²	a.	Electric Heat Pump	Cap: 36.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 267.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Adjacent	R=13.0, 360.0 ft²			EF: 0.90
b.	Concrete, Int Insul, Exterior	R=13.0, 1784.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 2226.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 154.0 ft		RB-Attic radiant barrier,	
b.	N/A			MZ-C-Multizone cooling,	
				MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

EnergyGauge® (Version: FLRCPB v3.2)

Weggie

Columbia County Building Department
Culvert Waiver

Culvert Waiver No. 000000853

DATE: 10/20/2005 BUILDING PERMIT NO. 23741

APPLICANT KATIE REED PHONE 752-4072

ADDRESS 2230 SE BAYA DRIVE LAKE CITY FL 32025

OWNER DAVID & JUDITH ONORATI PHONE 752-4072

ADDRESS 198 SW STILLWIEW GLEN FT. WHITE FL 32038

CONTRACTOR DON REED PHONE 752-4072

LOCATION OF PROPERTY 47S, TL ON HERLONG,TR ON SKYLINE LOOP, TL ON STILLVIEW,

2ND LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT CARDINAL FARMS 20

PARCEL ID # 11-6S-16-03815-120

I HEREBY CERTIFY THAT I UNDERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF THE COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROPOSED APPLICATION.

SIGNATURE: Katie Reed

A SEPARATE CHECK IS REQUIRED
MAKE CHECKS PAYABLE TO BCC

Amount Paid 50.00

PUBLIC WORKS DEPARTMENT USE ONLY

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINED THAT THE CULVERT WAIVER IS:

✓ APPROVED _____ NOT APPROVED - NEEDS A CULVERT PERMIT

COMMENTS: CULVERT ALREADY INSTALLED, WINGER ENDS NOT CONCRETED OR ARMORED

SIGNED: Perry Little DATE: 10/27/05

RECEIVED
OCT 24 2005
BY:

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

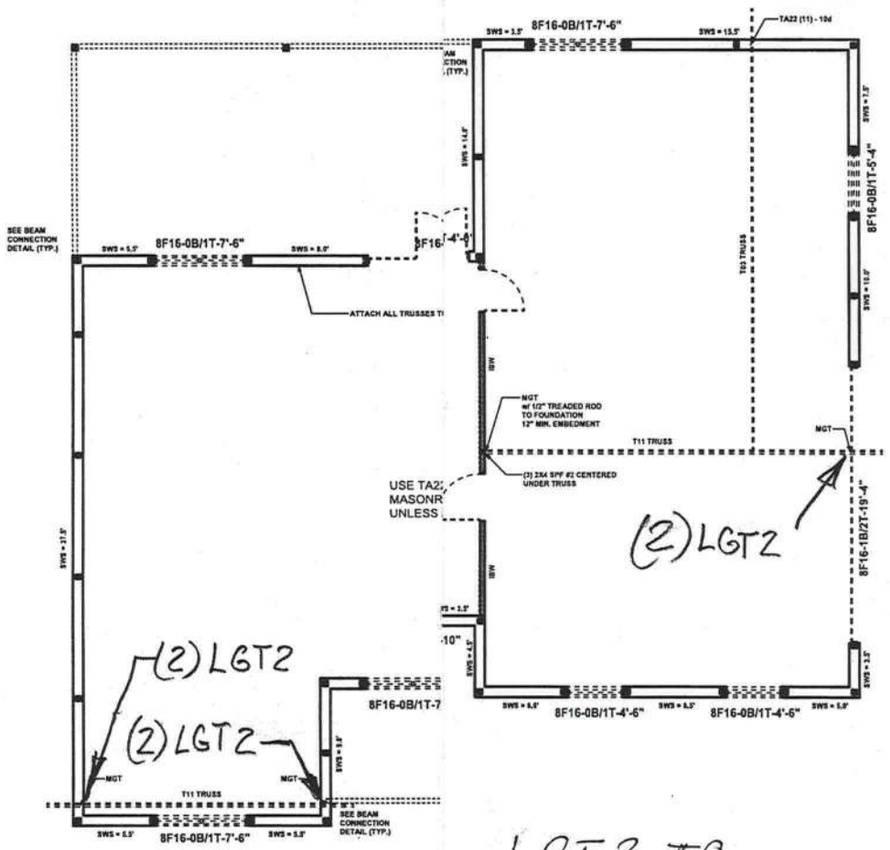
135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

COLUMBIA COUNTY · GATEWAY TO FLORIDA ·

23741

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE



ADDENDUM TO REPLACE MGT WITH (2) LGT2

LGT2 TO MASONRY BONDBEAM
(7) 1/4 x 2 1/4 TAPCONS
(16) 16d SINKER .148 X 3"

NG RD
INE 3RDRD
LEFT

CONNECTIONS, WALL, & ON REACTIONS & UPLIFT FURNISHED BY BUILDE (JOB #L128901)

STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT, REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1606, florida building code 2001, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

SEAL

06 JAN 06

David & Judith Onorati Residence

ADDRESS: Columbia County, Florida

Mark Disosway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: September 14, 2005

DRAWN BY: David Disosway | CHECKED BY:

FINALS DATE: 14 / Sep / 05

JOB NUMBER: 509064

DRAWING NUMBER S-3

OF 3 SHEETS

Residential System Sizing Calculation

Summary

David & Judith Onorati
Stillview Rd.
Lake City, FL

Project Title:
Onorati Residence

Code Only
Professional Version
Climate: North

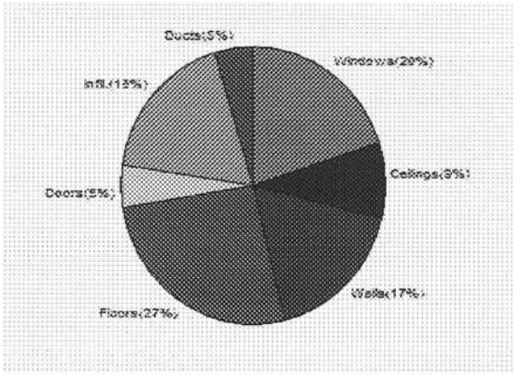
8/10/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**31807**	**Btuh**	**Total cooling load calculation**	**27055**	**Btuh**
Submitted heating capacity	36000	Btuh	Submitted cooling capacity	30000	Btuh
Submitted as % of calculated	113.2	%	Submitted as % of calculated	110.9	%

WINTER CALCULATIONS

Winter Heating Load (for 2226 sqft)

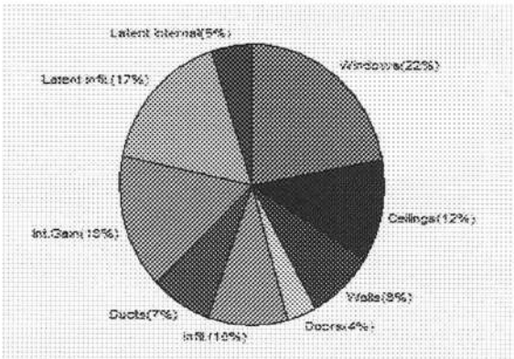
Load component			Load	
Window total	225	sqft	6368	Btuh
Wall total	2144	sqft	5393	Btuh
Door total	100	sqft	1619	Btuh
Ceiling total	2226	sqft	2894	Btuh
Floor total	267	ft	8437	Btuh
Infiltration	130	cfm	5582	Btuh
Subtotal			**30292**	**Btuh**
Duct loss			1515	Btuh
TOTAL HEAT LOSS			**31807**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 2226 sqft)

Load component			Load	
Window total	225	sqft	6001	Btuh
Wall total	2144	sqft	2203	Btuh
Door total	100	sqft	998	Btuh
Ceiling total	2226	sqft	3161	Btuh
Floor total			0	Btuh
Infiltration	130	cfm	2576	Btuh
Internal gain			4300	Btuh
Subtotal(sensible)			**19239**	**Btuh**
Duct gain			1924	Btuh
Total sensible gain			**21163**	**Btuh**
Latent gain(infiltration)			4512	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5892**	**Btuh**
TOTAL HEAT GAIN			**27055**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY:
DATE: 8-10-05

System Sizing Calculations - Winter

Residential Load - Component Details

David & Judith Onorati
Stillview Rd.
Lake City, FL

Project Title:
Onorati Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

8/10/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	30.0	28.3	849 Btuh
2	2, Clear, Metal, DEF	N	60.0	28.3	1698 Btuh
3	2, Clear, Metal, DEF	N	10.0	28.3	283 Btuh
4	2, Clear, Metal, DEF	W	16.0	28.3	453 Btuh
5	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
6	2, Clear, Metal, DEF	S	9.0	28.3	255 Btuh
7	2, Clear, Metal, DEF	S	40.0	28.3	1132 Btuh
8	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
	Window Total		225		6368 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	360	1.6	576 Btuh
2	Concrete - Exterior	13.0	1784	2.7	4817 Btuh
	Wall Total		2144		5393 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		40	17.9	718 Btuh
2	Wood - Adjac		20	9.2	184 Btuh
3	Wood - Exter		40	17.9	718 Btuh
	Door Total		100		1619Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	2226	1.3	2894 Btuh
	Ceiling Total		2226		2894Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	267.0 ft(p)	31.6	8437 Btuh
	Floor Total		267		8437 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.35	22260(sqft)	130	5582 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			130	5582 Btuh

Totals for Heating	**Subtotal**	**30292 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1515 Btuh**
	Total Btuh Loss	**31807 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

Manual J Summer Calculations

Residential Load - Component Details (continued)

David & Judith Onorati
Stillview Rd.
Lake City, FL

Project Title:
Onorati Residence

Code Only
Professional Version
Climate: North

8/10/2005

Totals for Cooling	**Subtotal**	**19239**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**1924**	**Btuh**
	Total sensible gain	**21163**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**4512**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**27055**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

System Sizing Calculations - Summer

Residential Load - Component Details

David & Judith Onorati
Stillview Rd.
Lake City, FL

Project Title:
Onorati Residence

Code Only
Professional Version
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 8/10/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	8	30.0	0.0	30.0	22	22	660	Btuh
2	2, Clear, DEF, N, N	N	8	6.5	60.0	0.0	60.0	22	22	1320	Btuh
3	2, Clear, DEF, N, N	N	1.5	6	10.0	0.0	10.0	22	22	220	Btuh
4	2, Clear, DEF, N, N	W	1.5	5	16.0	1.0	15.0	22	72	1103	Btuh
5	2, Clear, DEF, N, N	S	1.5	6	30.0	30.0	0.0	22	37	660	Btuh
6	2, Clear, DEF, N, N	S	14	4	9.0	9.0	0.0	22	37	198	Btuh
7	2, Clear, DEF, N, N	S	14	8	40.0	20.0	20.0	22	37	1180	Btuh
8	2, Clear, DEF, N, N	S	14	6	30.0	30.0	0.0	22	37	660	Btuh
	Window Total				225					6001	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Adjacent	13.0	360.0	1.0	374	Btuh
2	Concrete - Exterior	13.0	1784.0	1.0	1829	Btuh
	Wall Total		2144.0		2203	Btuh

Doors	Type	Area	HTM	Load	
1	Wood - Exter	40.0	10.0	399	Btuh
2	Wood - Adjac	20.0	10.0	200	Btuh
3	Wood - Exter	40.0	10.0	399	Btuh
	Door Total	100.0		998	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	2226.0	1.4	3161	Btuh
	Ceiling Total		2226.0		3161	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	267.0 ft(p)	0.0	0	Btuh
	Floor Total		267.0		0	Btuh

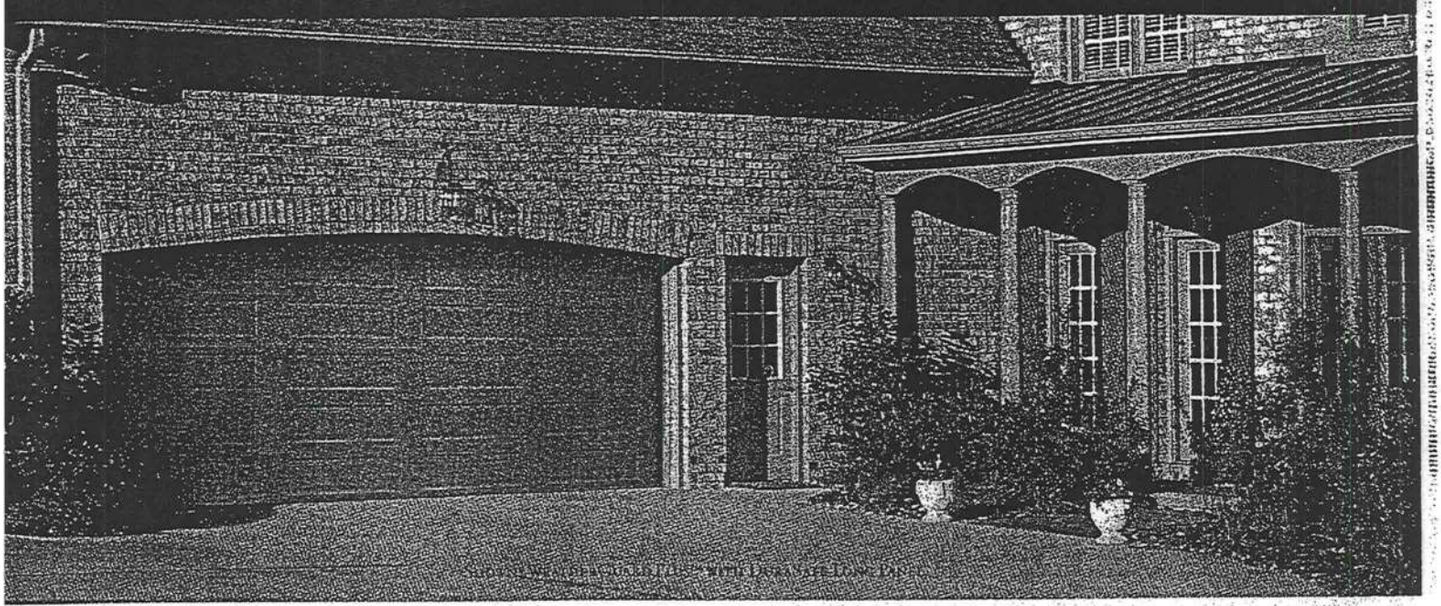
Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	22260	130.1	2576	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			130	2576	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	2500	4300	Btuh

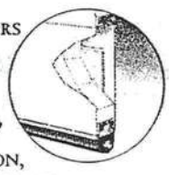
Amarr GARAGE DOORS

BEST

WeatherGuard™ Series
featuring our DuraSafe System



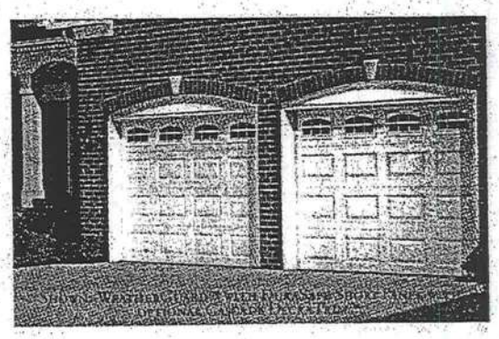
WeatherGuard Plus™ with DuraSafe



The WeatherGuard Plus offers discerning homeowners a masterful combination of premium features. Superior triple-layer construction, 2" (5.1 cm) polystyrene insulation, an R-value of 8.34, and unmatched beauty put the WeatherGuard Plus at the top of its class.

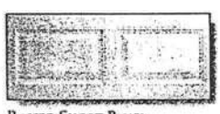
WeatherGuard™ with DuraSafe

Top-quality triple-layer construction and 1 3/8" (3.5 cm) polystyrene insulation make our WeatherGuard steel door strong, quiet, and energy efficient. Featuring an R-value of 5.73, the WeatherGuard is the perfect addition to your home for years of trouble free service and great looks.

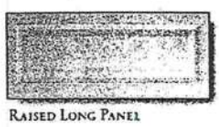


Design Elements

The WeatherGuard Series doors are available with a raised short, raised long, or flush panel design in your choice of four colors.*

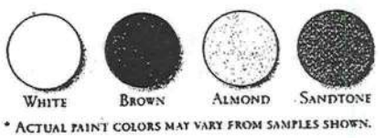


Raised Short Panel



Raised Long Panel

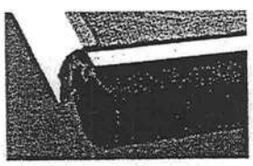
Flush Panel



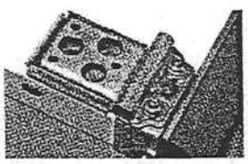
White Brown Almond Sandtone

* Actual paint colors may vary from samples shown.

Bottom Seal
New aluminum bottom seal means easy and fast installation and maintenance... as well as a better seal against the elements.

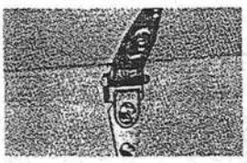


Bottom Bracket
New tamper resistant bottom bracket helps prevent accidents, yet allows for roller maintenance/change without disassembly. Full length roller tube prevents slip-outs.

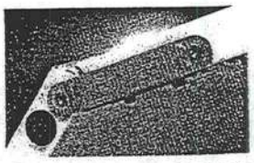


Door Sections
The section joint of the future: today. New section profile assures pinch resistance both inside and out, exceeding industry standards - neither fingers nor weather gets in.

Center Hinge
Flush mount inboard design center hinges provide pinch resistant protection and a low profile clean look on the inside of the door.



End Hinge
With most of its action hidden inside the door, our re-engineered end hinges leave no room for even the smallest fingers.

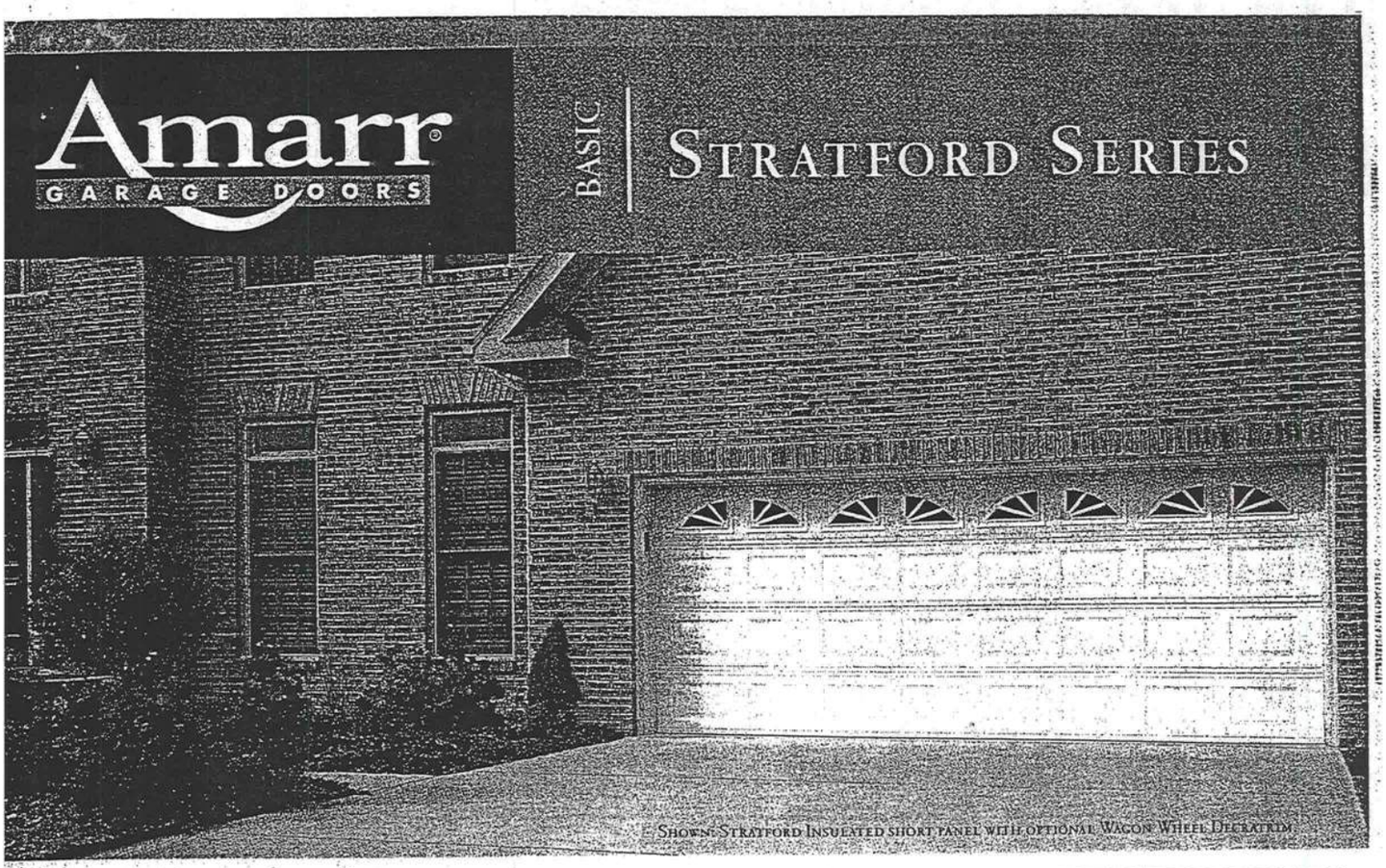


Amarr DuraSafe doors under 8'9" will be supplied with DuraSafe hardware. DASMA standards for pinch-resistance do not apply to doors over 8' high since the potential pinch points are above typical grasping heights; Amarr doors over 8'9" are supplied with conventional hardware. The Bottom Bracket, Door Sections, Center Hinge and End Hinge shown above are patented. Doors shown are electrically operated. Non-electrically operated doors should have exterior and interior lift handles attached to the door.

Amarr
GARAGE DOORS

BASIC

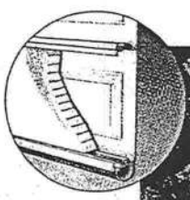
Stratford Series



Shown: Stratford Insulated short panel with optional Wagon Wheel Decorative

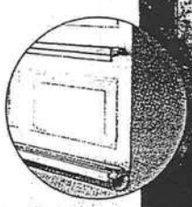
Stratford Insulated

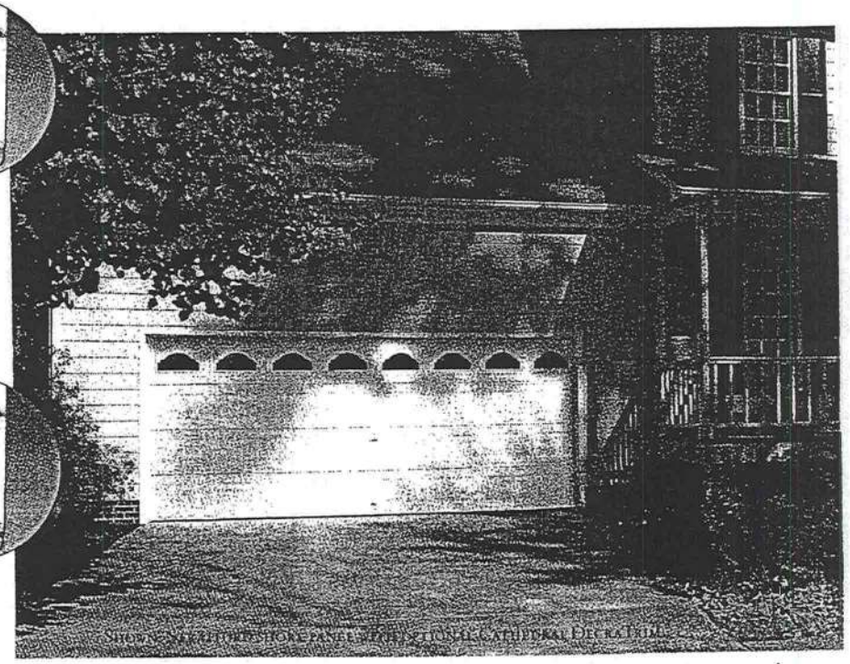
The 2" (5.1 cm) thick Stratford Insulated provides homeowners excellent thermal protection and handsome good looks. Features include double-layer construction of sturdy 25-gauge steel, and 1 7/16" (3.7 cm) polystyrene insulation with laminated backing and an R-value of 5.65.



Stratford

A superlative addition to any home, the Stratford's durable single-layer construction, 25-gauge steel, and attractive design provide homeowners with exceptional value.

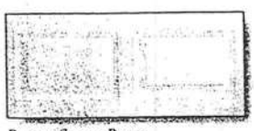




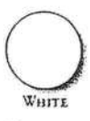
Shown: Stratford short panel with optional Decorative

Design Elements

The Stratford Series doors are available with a raised short panel design in your choice of three colors.*



Raised Short Panel



White

Almond

Sandtone

* Actual paint colors may vary from samples shown.

CAPITOL
WINDOWS AND DOORS

- Series 165 Single Hung and Fixed Windows
- Series 650 Single Hung and Fixed Windows
- Series 168 Horizontal Slider and Fixed Windows
- Series 680 Horizontal Slider and Fixed Windows

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" ALUMINUM FIN WINDOWS

Capitol Windows & Doors appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition – proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1. Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin. Place a continuous bead of caulk on the back side of nail fin (mounting flange).
2. Set unit plumb and square into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit frequently as fasteners are set.
3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DPs (design pressures) up to 35, do not exceed 24" spacing of additional screws. For DPs from 35.1 to 50, do not exceed 18" spacing.
4. Caulk entire perimeter of fin to mounting surface joint and caulk over screw heads.
   Note: this step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd., head 3rd.).
5. Fill voids between frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of expanding aerosol type insulating foam, which can bow the frame, waives all stated warranties.
6. Remove plaster, mortar, paint, and debris that has collected on the unit and make sure that sash/vent tracks and interlocks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress and structural / energy performance.

Corporate Headquarters:
M.I. Home Products
650 West Market St.
Gratz, PA 17030-0370
(717) 365-3300

www.mihp.com

STEVEN M. URICH
CERTIFICATE
No. 57705
STATE OF FLORIDA
PROFESSIONAL ENGINEER

JL 221
JULY 29, 2003

Rev. 7-24-03

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 450/650/850 Drop In Glazing
TYPE: Aluminum Single Hung Window

Title	Summary of Results
AAMA Rating	H-LC30 53 x 90
Operating Force	24 lb max.
Air Infiltration	0.11 cfm/ft^2
Water Resistance Test Pressure	6.75 psf
Uniform Load Deflection Test Pressure	+32.8 psf -47.2 psf
Uniform Load Structural Test Pressure	+49.2 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-42487.01 for complete test specimen description and data.

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-42487.01
Test Date: 08/14/02
And: 08/15/02
Report Date: 10/02/02
Expiration Date: 08/15/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 450/650/850 Drop In Glazing, aluminum single hung window at their facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-LC30 53 x 90 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 450/650/850 Drop In Glazing

Type: Aluminum Single Hung Window

Overall Size: 4' 5-1/8" wide by 7' 5-5/8" high

Interior Sash Size: 4' 2-3/4" wide by 3' 8-7/8" high

Fixed Daylight Opening Size: 4' 0" wide by 3' 5-3/8" high

Screen Size: 4' 0-3/4" wide by 3' 8-3/4" high

Finish: The unit was white.

Glazing Details: The specimen utilized 5/8" thick, sealed insulating glass constructed from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description (Continued)

Weatherstripping:

Description	Quantity	Location
0.190" high by 0.187" polypile with center fin	1 Row	Fixed meeting rail interlock
0.190" high by 0.187" polypile with center fin	2 Rows	Interior sash stiles
1/4" vinyl foam-filled bulb seal	1 Row	Interior sash bottom rail
5/8" wide by 7/8" long polypile plug	4 Pieces	Interior sash, all corners

Frame Construction: The frame was constructed of extruded aluminum. Each corner was coped, butted, sealed, and fastened with two #8 x 1" screws per corner through the head and sill into jamb screw boss. End caps were utilized on the ends of the meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was then secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum. Each corner was coped, butted, and fastened with one #8 x 1-1/4" screw per corner.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock	2	Interior sash, 6-1/2" from top rail ends
Spring-loaded coil balance	2	One per jamb
Plastic tilt latch	2	Interior sash top rail ends
Metal tilt latch pin	2	Interior sash bottom rail ends
Screen spring-loaded retainer pin	2	6-3/4" from rails on stiles

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The specimen was installed into a #2 2 x 8 Spruce-Pine-Fir wood buck. #8 x 1-5/8" drywall screws were placed 3" from corners and 15" on center around nailing fin. Polyurethane was used a s a sealant around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	24 lbs	35 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 3.75 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	@ 25.0 psf (positive)	0.64"*	0.29" max.
	@ 25.0 psf (negative)	0.54"*	0.29" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 37.5 psf (positive)	0.04"	0.20" max.
	@ 37.5 psf (negative)	0.03"	0.20" max.

Test Results:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Interior sash meeting rail	0.12"/25%	0.50"/100%
	Interior sash bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Interior sash right stile	0.06"/12%	0.50"/100%
	Interior sash left stile	0.06"/12%	0.50"/100%
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.75 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 32.8 psf (positive)	0.85"*	0.29" max.
	@ 47.2 psf (negative)	0.87"*	0.29" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 49.2 psf (positive)	0.09"	0.20" max.
	@ 70.8 psf (negative)	0.12"	0.20" max.

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen. N. Reeves, P.E.
Director - Engineering Services
11 OCTOBER 2002

MAH:mlb
01-42487.01

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Picture Window

Title of Test	Results
Rating	F-R45 60 x 80
Overall Design Pressure	+45.0 psf -47.2 psf
Air Infiltration	0.04 cfm/ft²
Water Resistance	8.25 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41135.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:mlb

Allen N. Rowe
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41135.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650, aluminum picture window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a F-R45 60 x 80 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 650

Type: Aluminum Picture Window

Overall Size: 5' 0" wide by 6' 8" high

Daylight Opening Size: 4' 9-1/4" wide by 6' 5-1/4" high

Finish: All aluminum was white.

Glazing Details: The test specimen utilized 7/8" thick, sealed insulating glass constructed from two sheets of 3/16" thick, clear annealed glass and a metal reinforced butyl spacer system. The glass was interior glazed against double-sided adhesive foam tape and secured with aluminum snap-in glazing beads.

130 Derry Court
York, PA 17402-9409
phone: 717-764-7700
fax: 717-764-8129
www.archtest.com

Allen N. Reum
1 APRIL 2002

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss.

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck. #8 x 2-1/2" installation screws were utilized 18" on center around the interior perimeter. Polyurethane was utilized to seal the exterior.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.2	Air Infiltration (ASTM E 283-91)		
	@ 1.57 psf (25 mph)	0.04 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance (ASTM E 547-00)		
	WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.01"	0.41" max.
	@ 34.7 psf (negative)	0.01"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the jamb)		
	(Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.0"	0.29" max.
	@ 52.1 psf (negative)	0.01"	0.29" max.

Allen N. Reeves
1 APRIL 2002

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: D Grade: 10		
	Hand and Tool Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) WTP – 8.25 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.02"	0.41" max.
	@ 47.2 psf (negative)	0.02"	0.41" max.
4.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the jamb) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.01"	0.29" max.
	@ 70.8 psf (negative)	0.02"	0.29" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

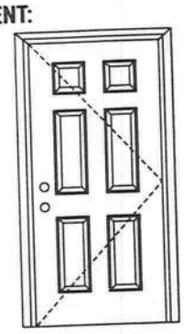
MAH:nb
01-41135.01

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

X
Opaque Inswing Unit

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+76.0/-76.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

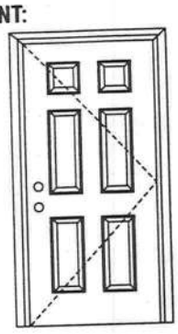
Masonite®

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

X
Opaque Outswing Unit

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door
Maximum unit size = 3'0" x 6'8"

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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MINIMUM ASSEMBLY DETAIL:

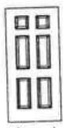
Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0011-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

Masonite®

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

X
Opaque Outswing Unit

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft
Wood-Grain & Textured
FIBERGLASS ENTRY DOORS

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X
Opaque Inswing Unit

COP-WL-MA0101-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

NCTL 210-1973-1, 2, 3

Certifying Engineer and License Number: Barry Portney, P.E. 16258

CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996.

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthz

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

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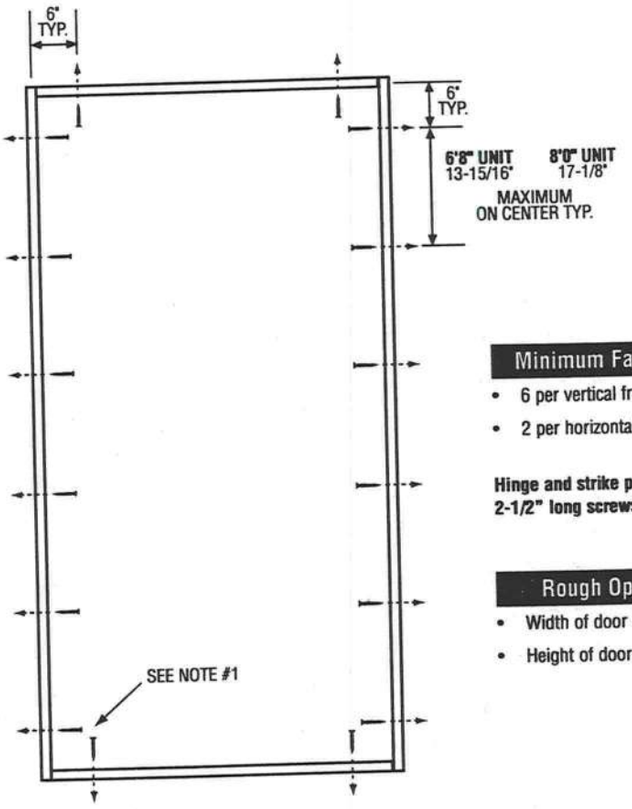
SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #30264478-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0246*, 0266*, 3241*, 3246, 3261* or 3266**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

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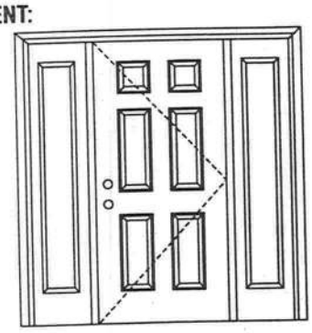
Masonite®

OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0014-02 or MAD-WL-MA0017-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

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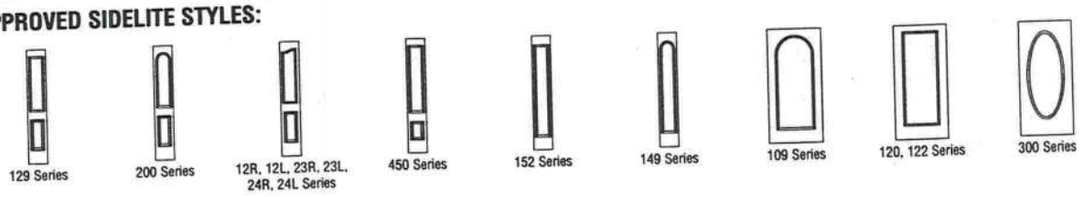
OXO
Opaque Outswing Unit

COP-WL-MA0124-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etisemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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ARTEK™ Non-Textured Fiberglass Entry Doors

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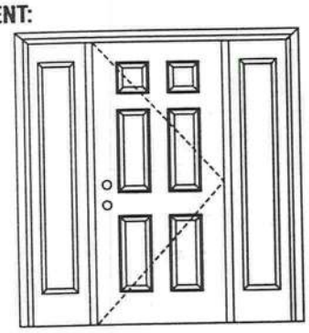
OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Single Door with 2 Sidelites
Maximum unit size = 5'4" x 6'8"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED on opaque panel, but is required on glazed panels.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

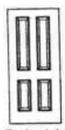
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

Masonite®

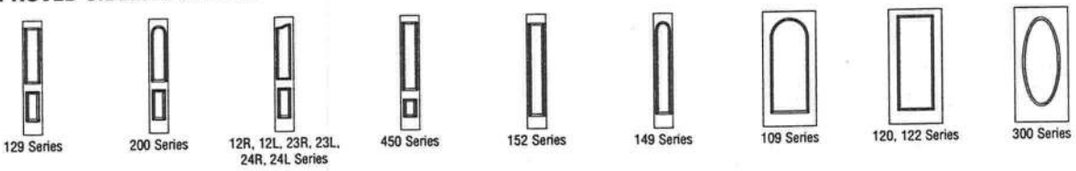
March 10, 2003

OXO
Opaque Inswing Unit

COP-WL-MA0104-02

FIBERGLASS DOORS

APPROVED SIDELITE STYLES:



129 Series | 200 Series | 12R, 12L, 23R, 23L, 24R, 24L Series | 450 Series | 152 Series | 149 Series | 109 Series | 120, 122 Series | 300 Series

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panel glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

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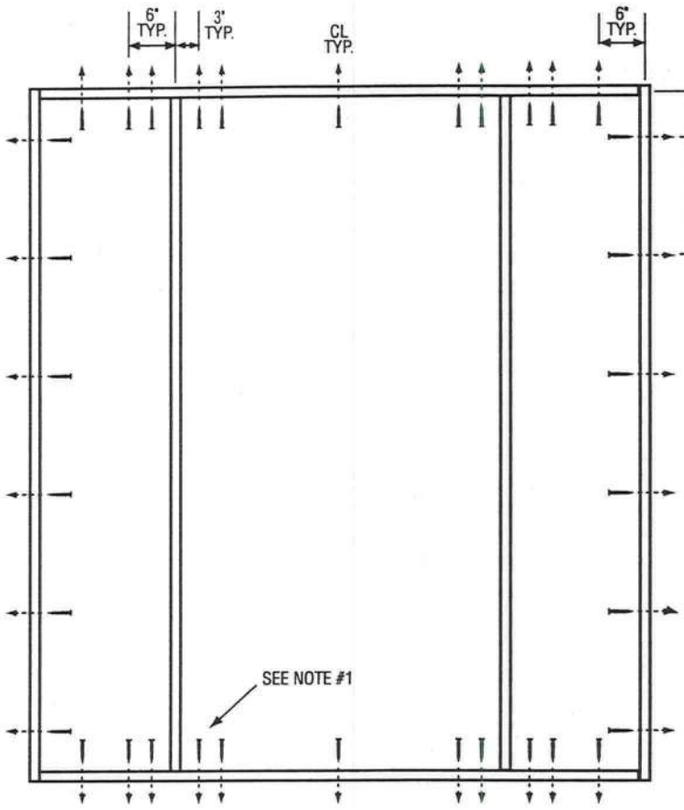
Masonite®

OXO
Unit

MID-WL-MA0004-02

SINGLE DOOR WITH 2 SIDELITES



Minimum Fastener Count

- 6 per vertical framing member
- 11 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0249*, 0269*, 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

Mrach 10, 2003
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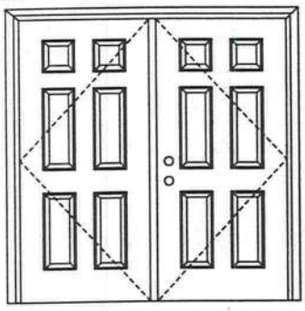
Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure

+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0012-02.

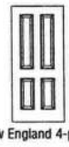
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

Oakcraft™ Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

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Masonite®

XX
Opaque Outswing Unit

COP-WL-MA0122-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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ARTEK™ Non-Textured Fiberglass Entry Doors

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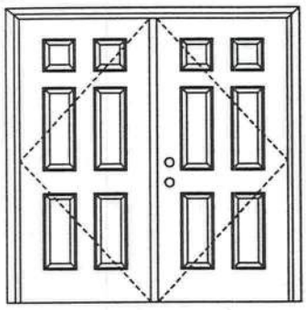
XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

APPROVED ARRANGEMENT:



Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+55.0/-55.0
limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02.

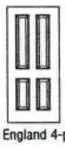
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:


Flush


6-panel


New England 4-panel


Eyebrow 4-panel


9-panel


Eyebrow 5-panel with scroll

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Masonite®

XX
Opaque Inswing Unit

COP-WL-MA0102-02

FIBERGLASS DOORS

CERTIFIED TEST REPORTS:

CTLA-772W-2; CTLA-1051W

Certifying Engineer and License Number: Ramesh Patel, P.E./20224

Unit Tested in Accordance with Miami-Dade BCCO PA202, ASTM E1886 and ASTM E1996

Door panels constructed from 0.075" minimum thick fiberglass skins. Both stiles constructed of 1-5/8" laminated lumber. Top end rails constructed of 31/32" wood. Bottom end rails constructed of 31/32" wood composite. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO PA201, PA202 & PA203
OR ASTM E1996, MIAMI-DADE PA202,
AND ASTM E1886
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey WH®

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003; #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Oakcraft Wood-Grain Textured FIBERGLASS ENTRY DOORS

ARTEK™ Non-Textured Fiberglass Entry Doors

March 10, 2003
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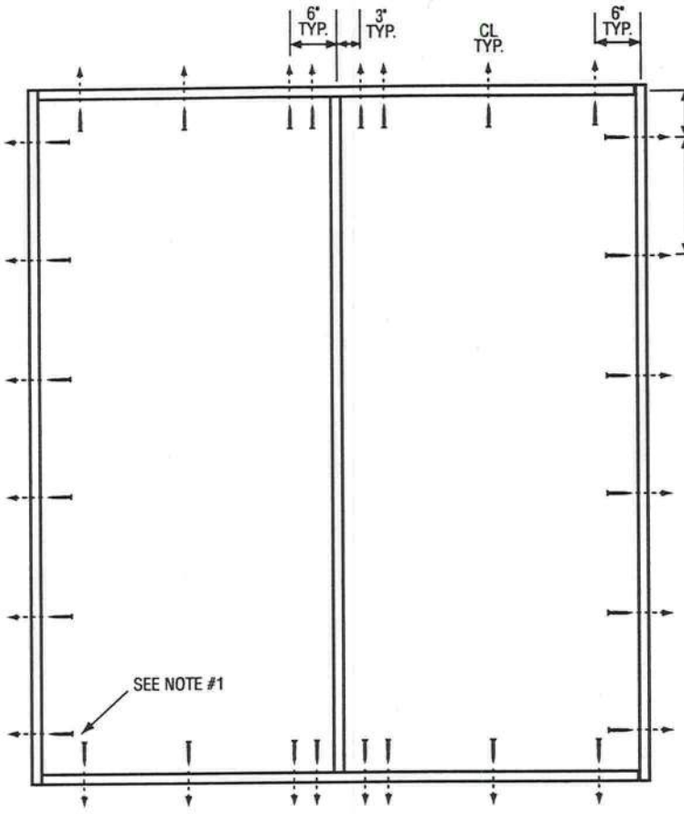
Masonite®

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Warnock Hersey

Test Data Review Certificate #3026447A; #3026447B; #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, 004; #3026447B-001, 002, 003, 004; #3026447C-001, 002, 003, 004 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 0247*, 0267*, 3242*, 3247, 3262* or 3267**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Jamb and head fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons. Threshold fasteners analyzed for this unit include #8 and #10 wood screws, 3/16" Tapcons, or Liquid Nails Builders Choice 490 (or equal structural adhesive).
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

March 10, 2003
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Masonite®

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001
ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA. OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☑	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☐	☑	**Elevations including:**
☑	☑	a) All sides
☑	☑	b) Roof pitch
☑	☑	c) Overhang dimensions and detail with attic ventilation
☐	☐	d) Location, size and height above roof of chimneys
☐	☑	e) Location and size of skylights
☐	☑	f) Building height
☐	☑	e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned

b) Shear walls

c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)

d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth

e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails

f) Must show and identify accessibility requirements (accesssable bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing

b) All posts and/or column footing including size and reinforcing

c) Any special support required by soil analysis such as piling

d) Location of any vertical steel

Roof System:

a) Truss package including:

1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:

1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☑ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☑ f) Appliances and HVAC equipment

HVAC information

☑ ☑ a) Manual J sizing equipment or equivalent computation

☑ ☑ b) Exhaust fans in bathroom

☑ ☑ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

This Instrument Prepared by & return to:
Name: ***JOYCE KIRPACH, an employee of TITLE OFFICES, LLC***
Address: ***1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025***
File No. 05Y-03186JK

Inst:2005008821 Date:04/18/2005 Time:09:38
Doc Stamp-Deed : 700.00
DC,P.DeWitt Cason,Columbia County B:1043 P:1556

Parcel I.D. #: ***03815-000***

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the* ***11th*** *day of* ***April****, A.D.* ***2005****, by* ***CARMEN P. FAVORITO,*** married *hereinafter called the grantor, to* ***DAVID ONORATI*** *and* ***JUDITH ONORATI, HIS WIFE,*** *whose post office address is* ***3392 CUSTER AVE., LAKE WORTH, FL. 33467,*** *hereinafter called the grantees:*

(Wherever used herein the terms "grantor" and "grantees" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantees all that certain land situate in* ***Columbia County, State of FLORIDA****, viz:*

LOT 20 OF AN UNRECORDED SUBDIVISION KNOWN AS CARDINAL FARMS

A PARCEL OF LAND IN SECTION 11, TOWNSHIP 6 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHEAST CORNER OF SECTION 11, TOWNSHIP 6 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE SOUTH 88°19'59" WEST ALONG THE SOUTH LINE OF SAID SECTION 11 A DISTANCE OF 3266.86 FEET; THENCE NORTH 22°15'30" EAST A DISTANCE OF 510.42 FEET; THENCE NORTH 01°40'01" WEST A DISTANCE OF 915.56 FEET; THENCE NORTH 22°03'23" EAST A DISTANCE OF 1397.36 FEET; THENCE NORTH 25°00'03" EAST A DISTANCE OF 2.82 FEET TO A POINT ON THE SOUTH LINE OF THE NORTH ½ OF SECTION 11; THENCE CONTINUE NORTH 25°00'03" EAST A DISTANCE OF 36.48 FEET; THENCE NORTH 81°52'24" WEST A DISTANCE OF 303.59 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE NORTH 81°52'24" WEST A DISTANCE OF 416.16 FEET; THENCE NORTH 03°13'23" EAST A

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Martha Bryan
Witness Signature
MARTHA BRYAN
Printed Name

Joyce Kirpach
Witness Signature
Joyce Kirpach
Printed Name

Carmen P. Favorito L.S.
CARMEN P. FAVORITO
Address:
3112 SW HERLONG ST., FORT WHITE, FL 32038

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 12th *day of* ***April, 2005****, by* ***CARMEN P. FAVORITO****, who is known to me or who has produced* D. License *as identification.*

Martha Bryan
Notary Public
My commission expires

Martha Bryan
Commission # DD232534
Expires August 10, 2007
Bonded Troy Fain - Insurance, Inc. 800-385-7019

Inst:2005008821 Date:04/18/2005 Time:09:38
Doc Stamp-Deed : 700.00
DC,P.DeWitt Cason,Columbia County B:1043 P:1557

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 15-4S-16-03815-120 **Building permit No.** 000023741

Use Classification SFD,UTILITY **Fire:** 49.56

Permit Holder DON REED **Waste:** 73.50

Owner of Building DAVID & JUDITH ONORATI **Total:** 123.06

Location: 198 SW STILLVIEW GLEN(CARDINAL FARMS,LOT 20)

Date: 04/14/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

BRITT SURVEYING
830 West Duval Street • Lake City, FL 32055
Phone (386) 752-7163 • Fax (386) 752-5573

Land Surveyors and Mappers

11/01/05

L-16736

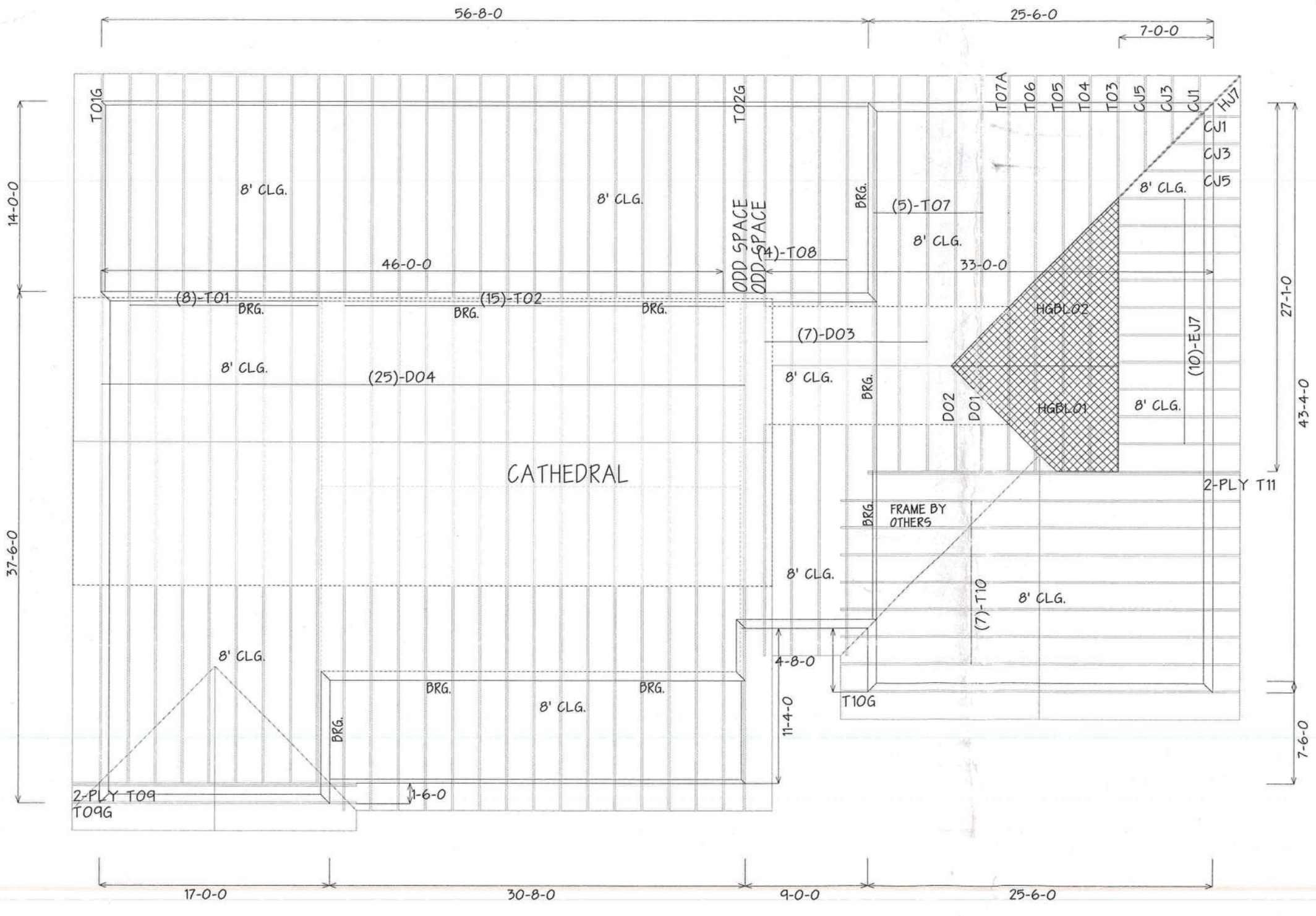
To Whom It May Concern:

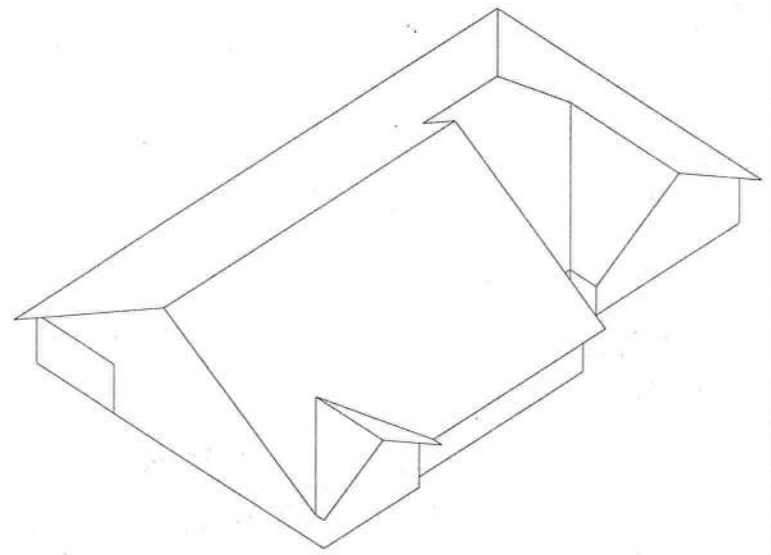
C/o: Don Reed Construction

Re: Lot 23 Woodborough Phase 1 (Permit #23692)

The elevation of the foundation is found to be 148.58 feet. The floor elevation, as per plat of record, is shown to be 139.20 feet on the plat of record. The highest adjacent grade is 146.54 feet and the lowest adjacent grade is 139.34 feet.

L. Scott Britt
PLS #5757





BEARING HEIGHT SCHEDULE

8'-0"

ROOF PITCH 8/12

OVERHANG
1'-6" EAVE
1'-0" GABLE

HANGER SCHEDULE
(17) HTU26

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HU526 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

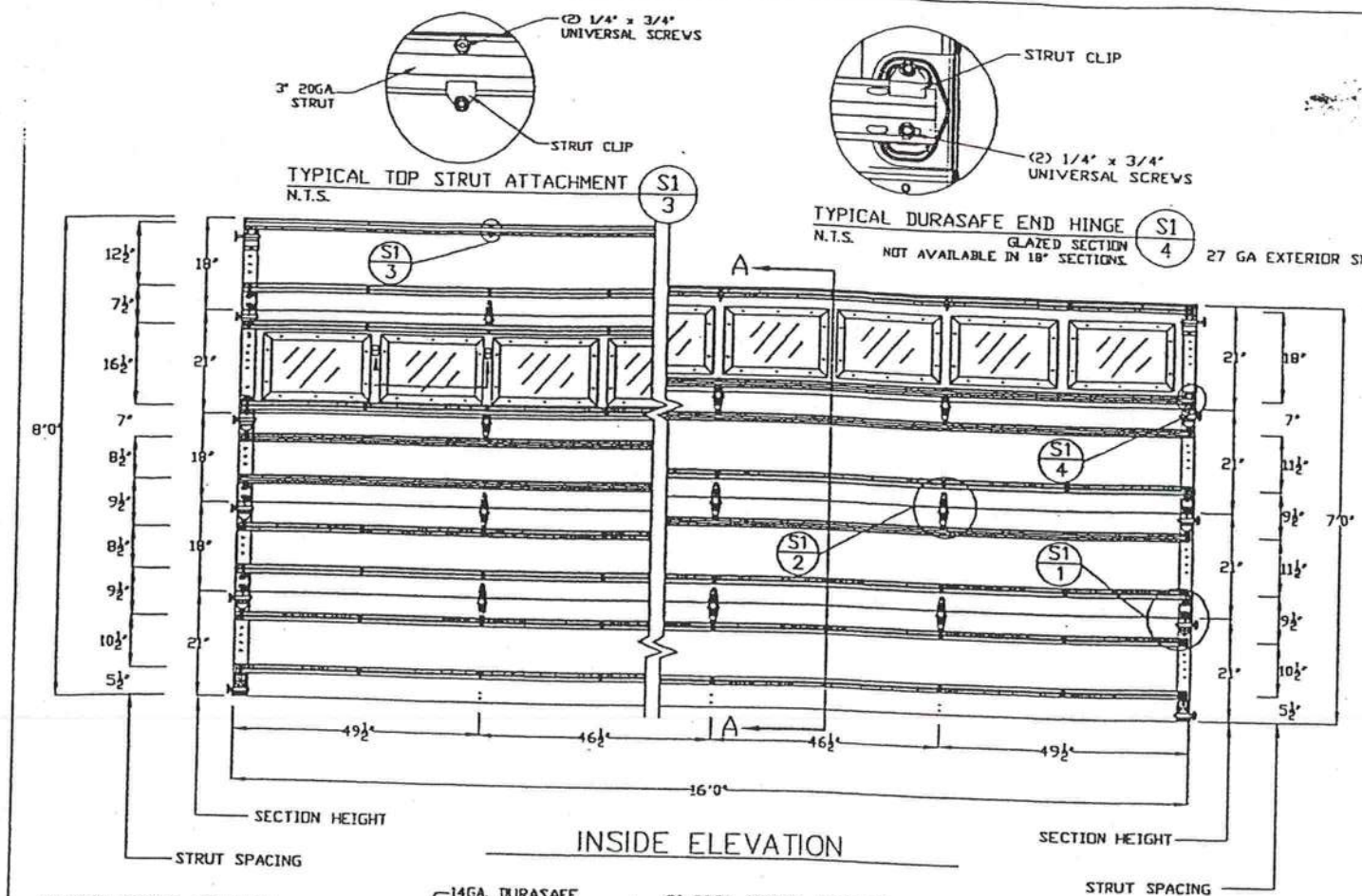
BUILDER: DON REED CONST.

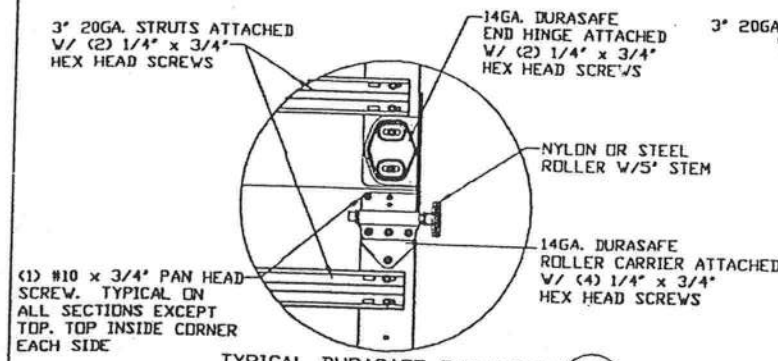
LEGAL ADDRESS: COLUMBIA CO.

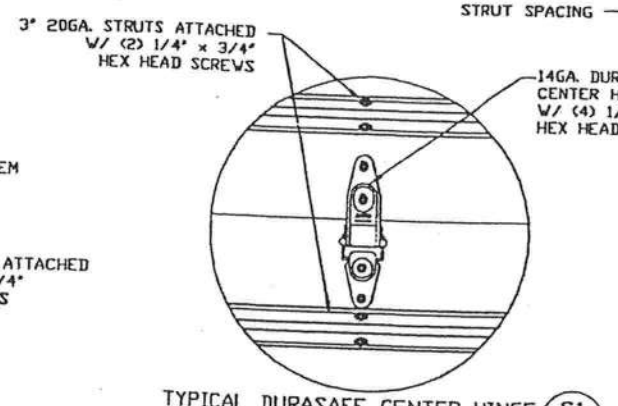
MODEL: ONORATI RES. REVISION:

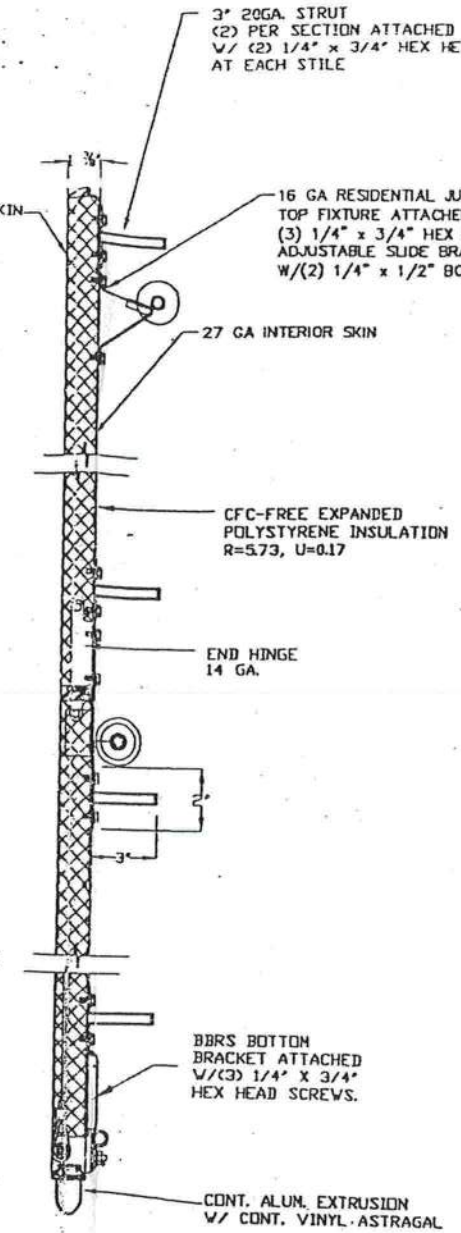
SCALE: NTS

DATE: 8/26/05 DRAWN BY: ROBERT CULOTTI JOB #: L128901



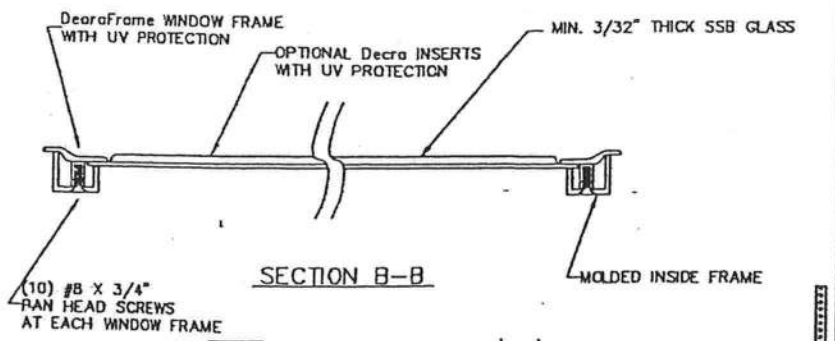


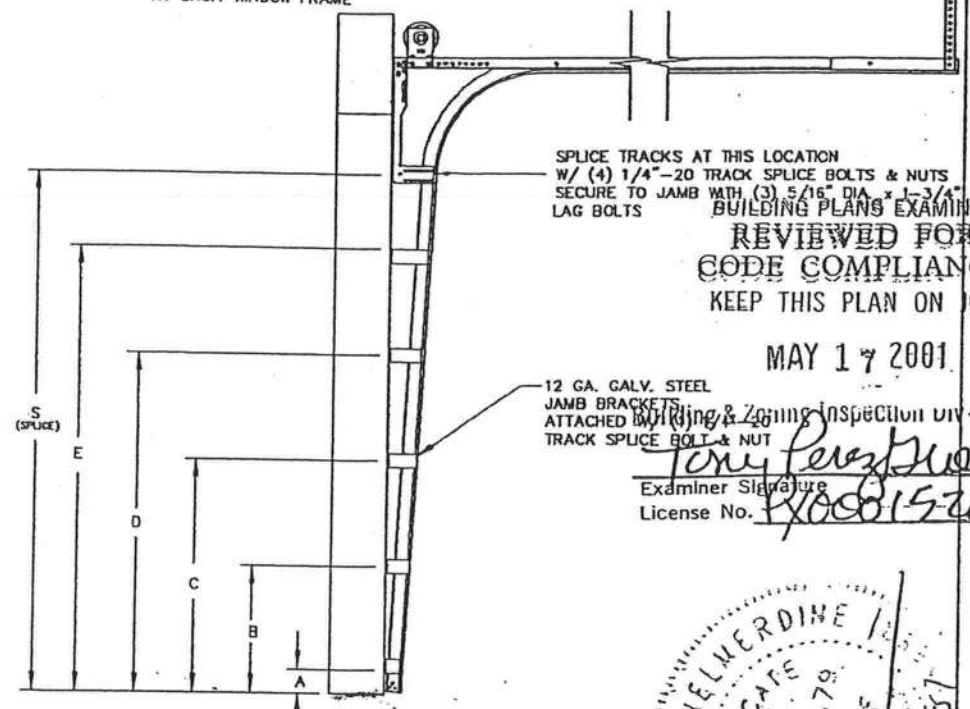




GLAZING OPTION CROSS SECTION

TEST No. SBC-580-020 ON MAY 24, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISION, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' x 7' AND 16' x 8' MODEL 1500-D DOORS.

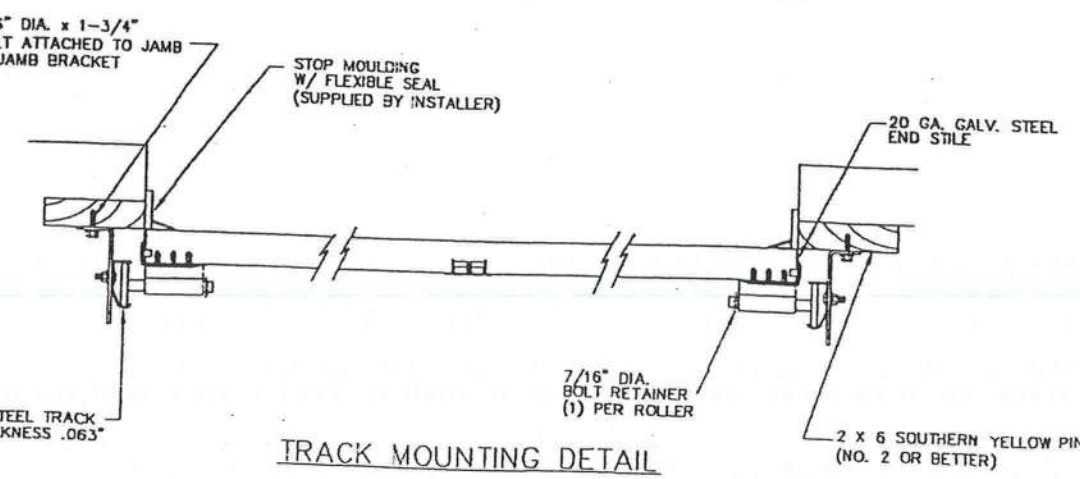




TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6'	4'	21-1/2"	39"	57"		70"
7'-0"	4'	21-1/2"	42"	63"		76"
7'-6'	4'	18-1/2"	36"	54"	72"	82"
8'-0"	4'	21-1/2"	39"	57"	75"	88"



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,700 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 8" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 8" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 8" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.018") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
8. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1608 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div - Jax., FL.
Examiner Signature
License No. RA0081520

HOWARD L. SHELMERDINE II
No. 48379
STATE OF FLORIDA
REGISTERED
5/21/01

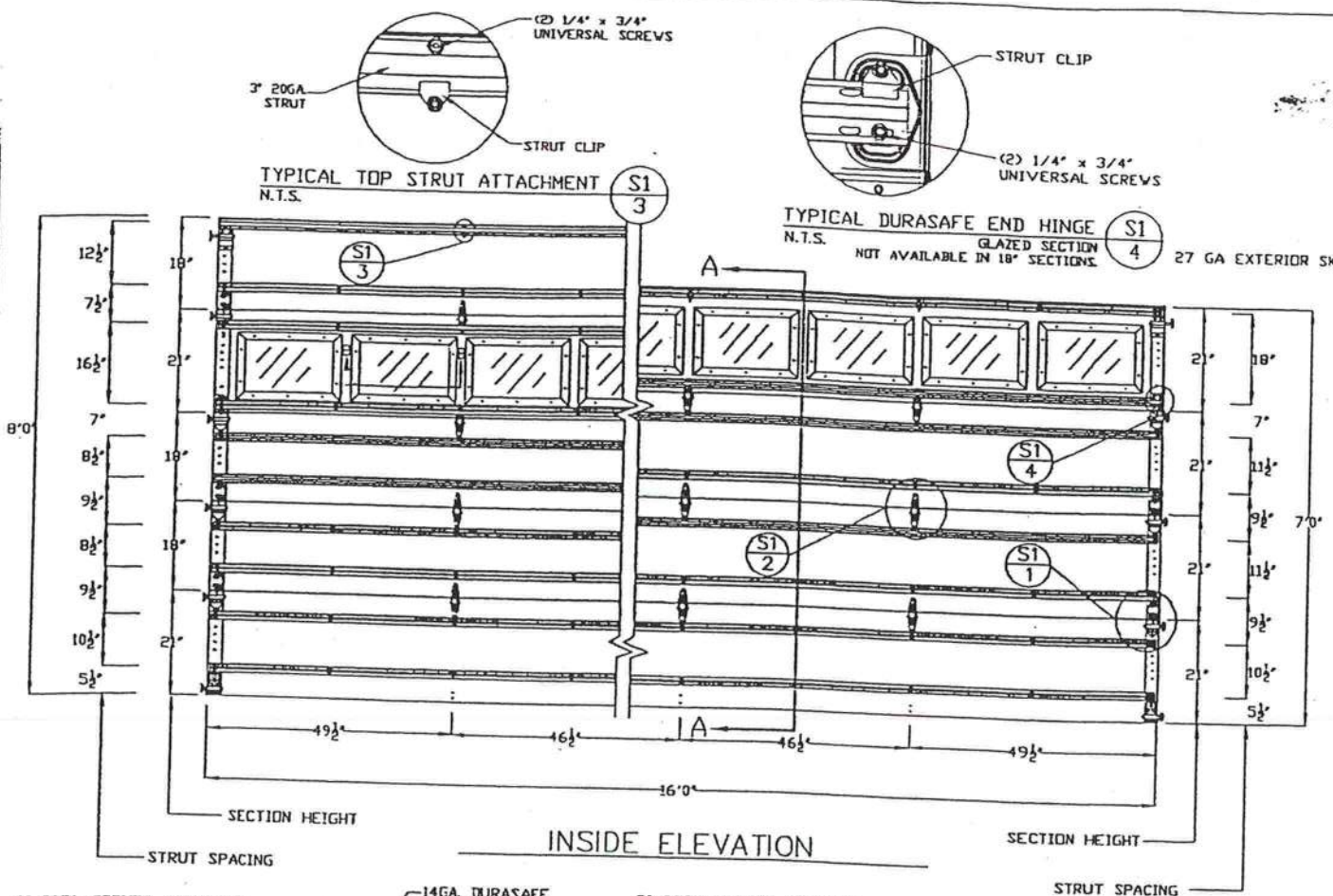
MAX SIZE	16' x 8'
DESIGN LOADS	+29.5 PSF -29.5 PSF
TEST LOADS	+43.3 PSF -43.3 PSF

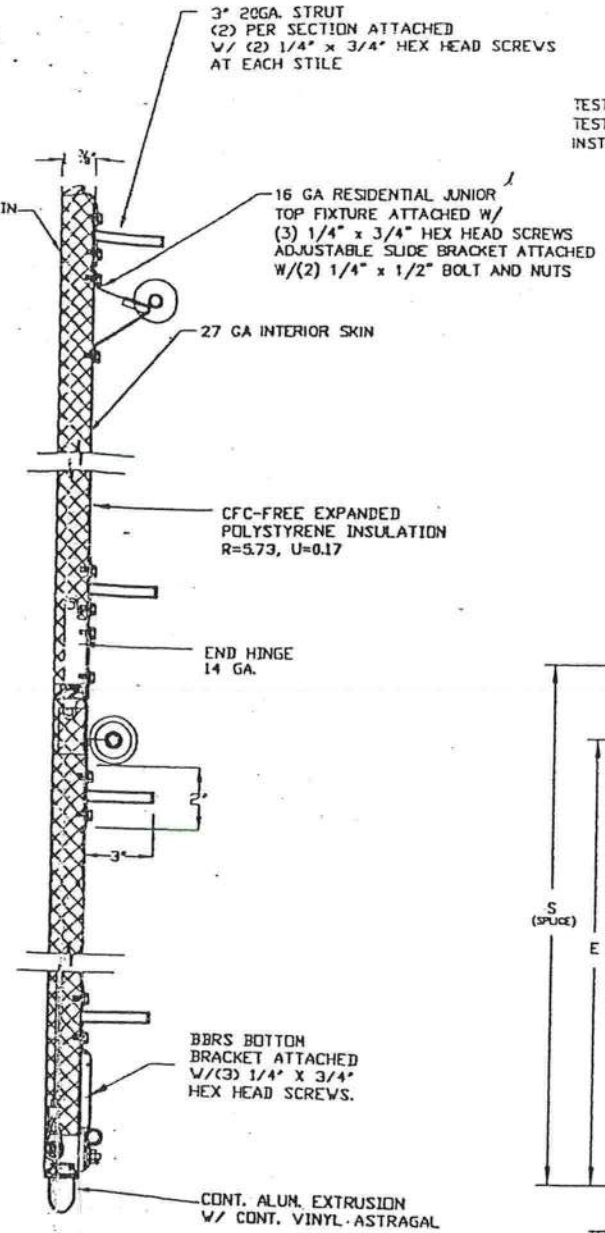
REV	DESCRIPTION OF REVISIONS	DATE	BY

Amarr GARAGE DOORS
5501 GRANBY CREEK BLVD WINSTON-SALEM, N.C. 27105

MODEL #1500 WeatherGuard

SIZE	DRAWN BY	DLJ	DATE	04/16/01	DRAWING NUMBER
B	CHECKED BY		DATE		SBC-580-019-J
SCALE: NOT TO SCALE					SHEET 1 OF 1





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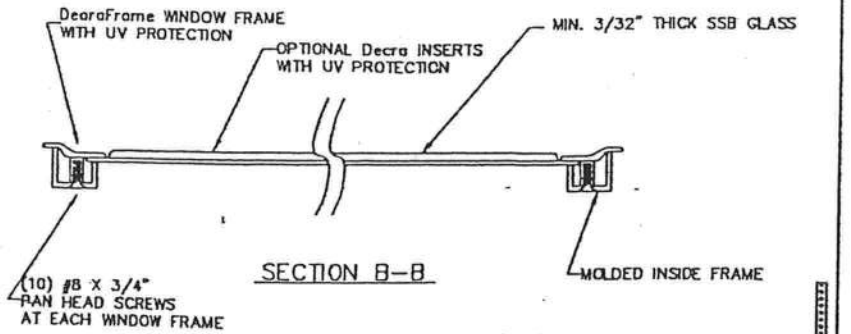

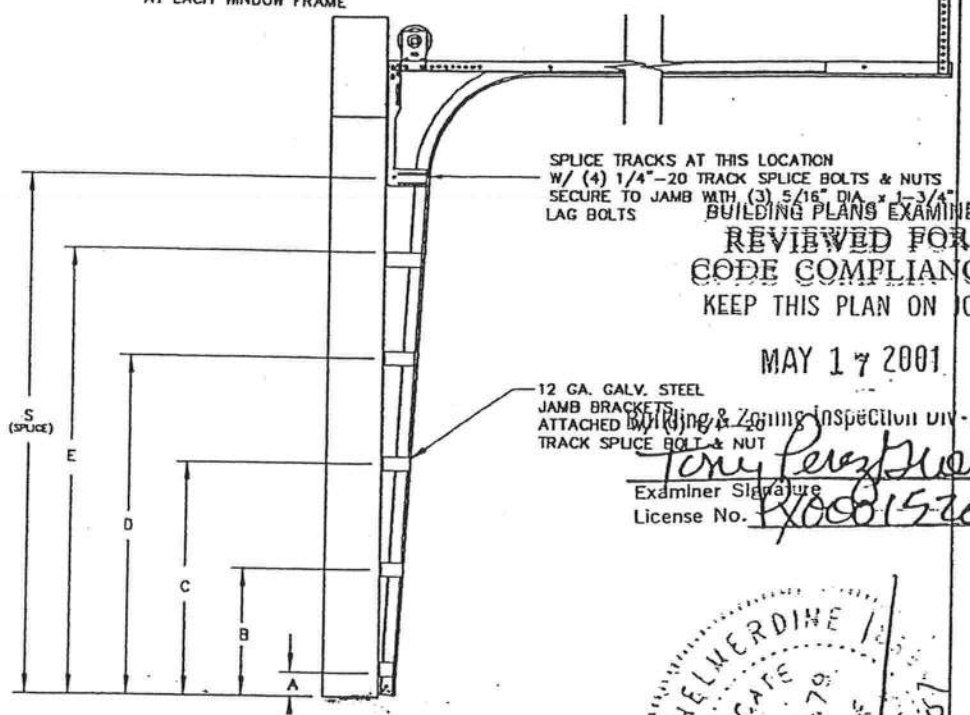

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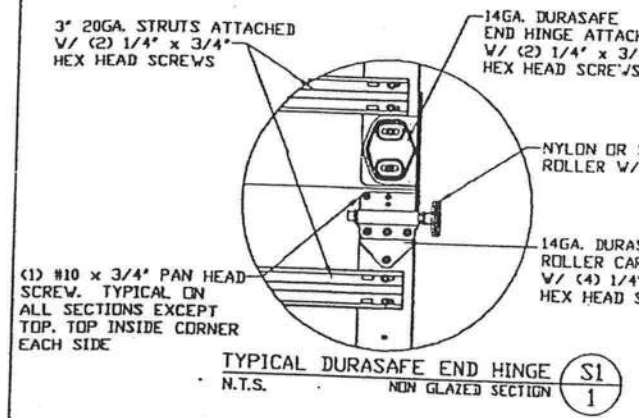
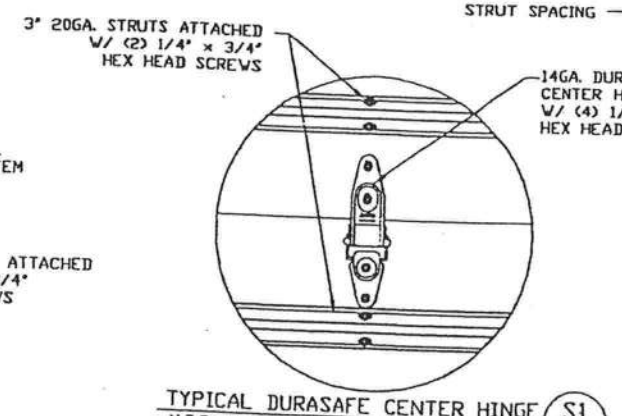
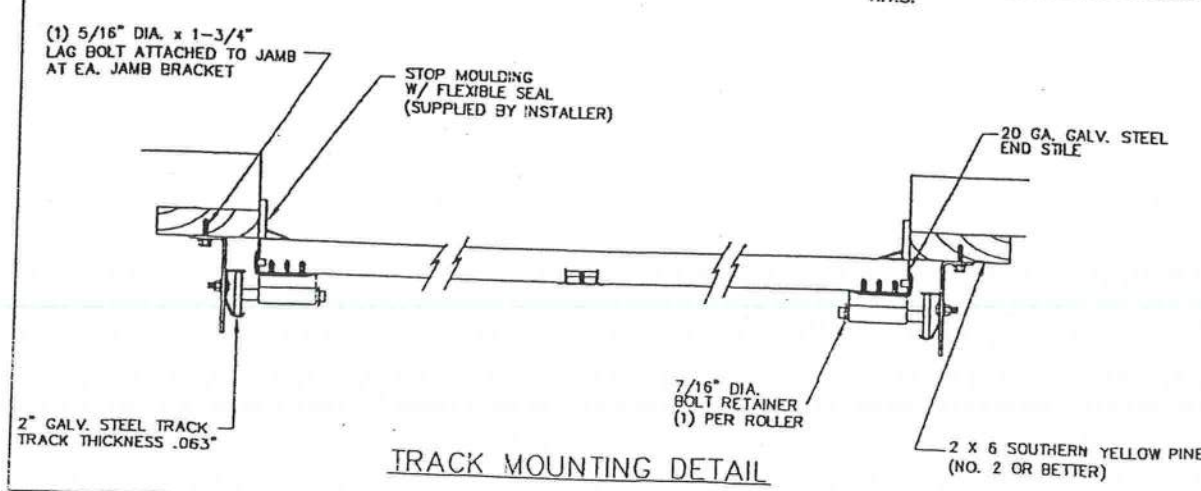
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"







WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,200 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 8" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 8" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 8" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.018") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB

MAY 17 2001

Building & Zoning Inspection Div - Jax., FL.
Examiner Signature: Tony Perez
License No. EX0001520

MAX SIZE	16' x 8'
DESIGN LOADS	+29.5 PSF -29.5 PSF
TEST LOADS	+43.3 PSF -43.3 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY

Amarr GARAGE DOORS
5921 GLADSTY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #1500 WeatherGuard

SIZE	DRAWN BY DLJ	DATE 04/18/01	DRAWING NUMBER
B	CHECKED BY	DATE	SBC-580-019-J
SCALE: NOT TO SCALE			SHEET 1 OF 1

Notice of Treatment 11753

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYA AVE
City LC Phone 752-1703

Site Location: Subdivision Cardinal Farms
Lot # 20 Block# Permit # 23701
Address 198 SW Stillview Cir

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	3865	876	5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

1/4/06 Date | 1000 Time | Fesh Gummy Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©