Top chord Bot chord chord 2x4 SP chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

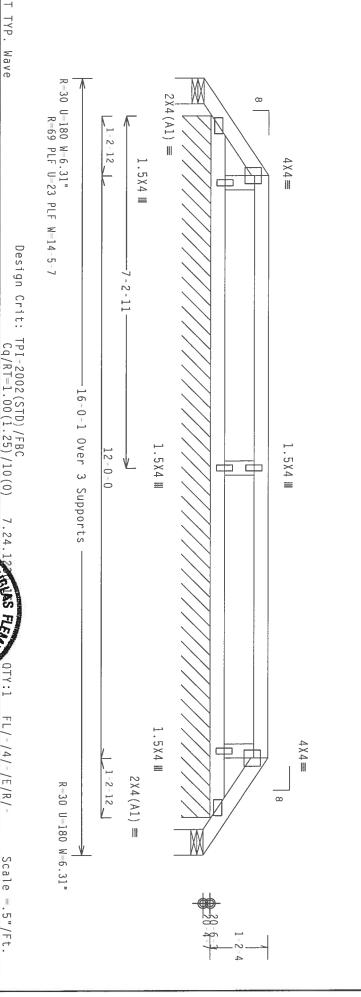
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

110 mph wind, 21.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



\*\*WARNING\*\* TRUSSES REQUIRE TXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RECER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PHRELISED BY TPI (TRUSS PLATE INSTITUTE, 21B HORTH LEE STRETE, SHITE LIST, ALEXANDRA, VA, 22314) AND MICA (MONDO TRUSS COUNCIL O AMERICA, 6300 ENTREPRENT AND SON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PREFERRENCE HORTHOG THESE FUNCTIONS. UNLESS OTHERWISE HOLDING MACHED FOR COMBO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

ALPINE

PLT TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG. THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. FAT FAILURE 10 BUILD THE RUSS IN COMPORNANCE WITH TP: OR FABBLECATHO. HANDLING, SHAPPING. HASTALLING A BRACHEG OF RUSSES.

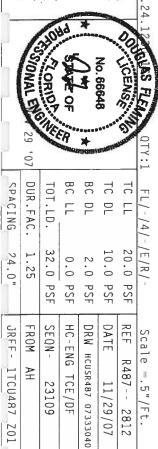
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ATAFA) AND TPI. ITM RCG CONNECTOR PLATES ARE MADE OF ZO/18/16GA (M.H/SS/K), ACHT MGS3 GRADE 40/16G (M. X/M.SS) GALV STEEL. APPLY PLATES TO EACH TACE OF TRUSS AND. UNLESS ON HEMISEL COCKING TOR PLATES AND MPRES AND UNLESS ON THE MESS ON THE MESS

DRAWTHG LUDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. DUSTBILLITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE

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Haines City, FL 33844

Grate City Figure 73844



TCE / DF 23109

1TCU487

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R487 - -

11/29/07 2812

Bot chord 2x4 SP Webs 2x4 SP #27 Dense Dense

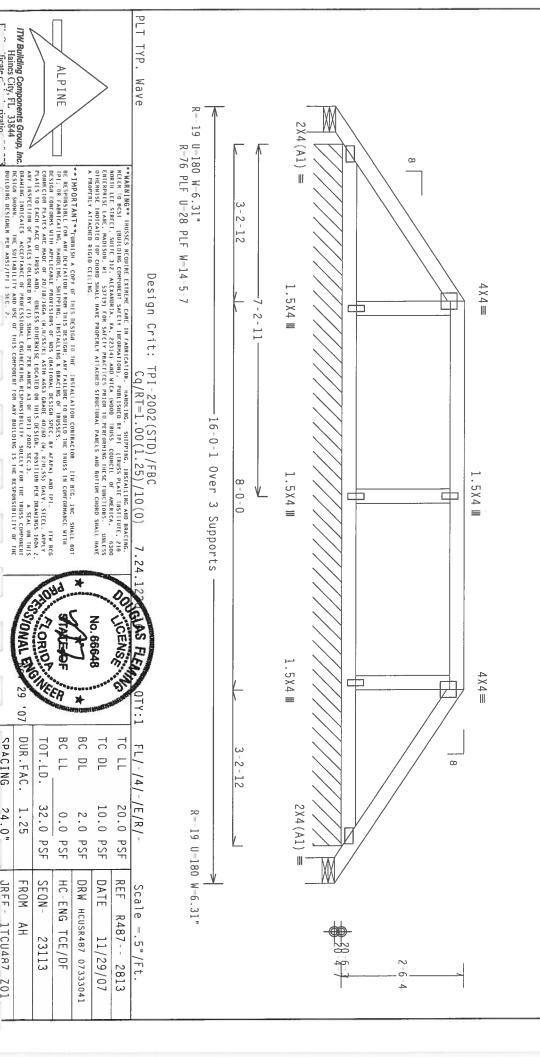
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for details. Portion of truss under piggyback is braced @ 24" oc unless otherwise specified. to piggyback

110 mph wind, 21.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



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ITW Building Components Group, Inc. Haines City, FL 33844

07

DUR.FAC

SPACING

24.0" 1.25

JRFF -

1TCU487

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FROM

Bot chord 2x4 SP Webs 2x4 SP #72 Dense Dense

110 mph wind, 22.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT iI, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

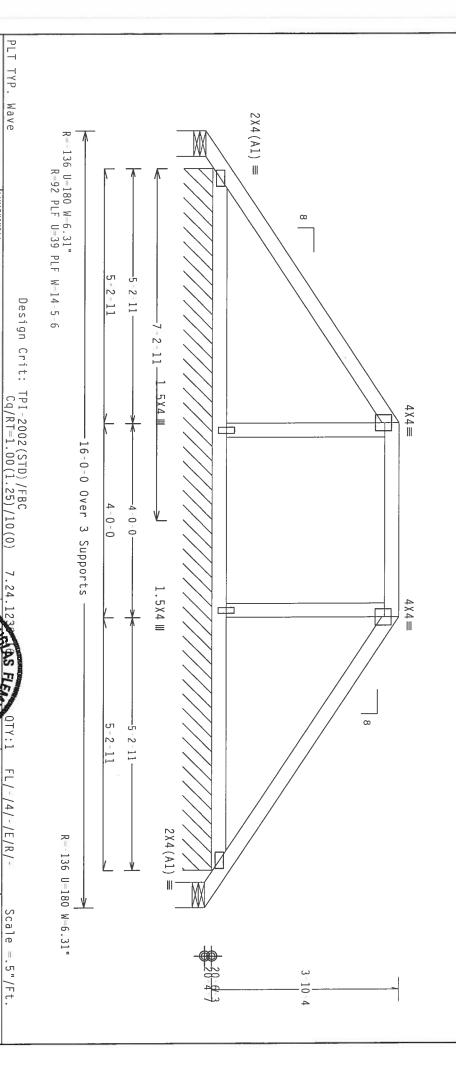
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. piggyback details.

> Bearing reactions of  $\pm 136 \#$  at (0-0-0, 20-4-7),  $\pm 136 \#$  at (15-5-11, 20-4-7), require special connection to resist uplift from loads other than wind.

Wind reactions based on MWFRS pressures

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.



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Haines City, FL 33844

ALPINE

\*\*IMPORTANT\*\*THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BC RESPONSIBLE FOR ANY DETIALTON FROM HIRS DESIGN, ANY FAILBER TO BHILD THE BRASS IN COMPORMANCE WITH FDI: ON TANKEALTHO. MANDLING. SHEPPING. INSTALLING A BRACLING OF TRUSSES.

DESIGN CONFIDENCE HIT APPLICABLE PROVISIONS OF HIDS (MATIONAL DESIGN SPEC. BY AFAPA) AND FPI. CONTROL OF THE BCG CONNECTION FLATES ARE HAVE OF 707/19 19GA (M. 1957/2) ASTALLING A BRACLING SPEC. BY AFAPA) AND FPI. SPECE APPLY PLATES TO EACH FACE OF TRUSS AND. HILLESS OHERWISE LOCALED ON HIS DESIGN, POSITION FER DRAWHIGS 160A Z. ANY HISPECTION OF PLATES COLUMNED BY (1) SHALL BE FER AHER AS OF FPII 2002 SEC.3. A SEAL ON HILLS DRAWHIG INDICALES ACCEPTANCE OF PROFESSIONAL ENGLINEER HIS RESPONSIBILITY SOLELY FOR THE BUSS COMPONENT DESIGN SHOWN. HIS DESIGN SHOWN. HIS DESIGN SHOWN. HIS DESIGN SHOWN. HIS SUITABLILLY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BULLDING COMPONERS SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, ZIB HOBERL SULE TAZZ. ALEXANDRA, YA, 22314) AND HIGH CAR (ADDO TRUSS COUNCIL OF AHERIA, 6300 ENTERPRISE LINE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE THOSCOPED HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE THOSCOPED HADISON, HI MAYE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

SOULCENSE

TC

FL/=/4/-۲

Scale

=.5"/Ft.

R487--

2814

No. 66648

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BC DL TC DL

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PSF PSF

HC-ENG

DF / DF 24342

DRW HCUSR487 07332038

10.0 20.0 /E/R/-

PSF PSF

DATE

11/28/07

07

TOT.LD.

SPACING DUR.FAC

24.0" 1.25 32.0

JRFF-

1TCU487 Z01

FROM SEQN- Wave

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified. Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. In lieu of rigid ceiling use purlins to brace BC @ 24" OC. 110 mph wind, 23.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 Fi Cate Contaction and Lines City, FL 33844

Fi Cate Contaction and Transfer and Tr ALPINE Wave ### 32° 2X4(A1) 73 U=112 W=6.31" R=83 PLF U=34 Dense -83 PLF U=34 PLF W=14-5-6 \*\*IMPORTANT\*\*rubhish a copy of this design to the Installation contractor. The BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BHILD THE TRUSS IN COMPORMANCE HITH FPI: OR FAREICALING, ANDICHIG. SHEPPING, HISTALLING A BRACHEG OF TRUSSES, DESIGN CONTROLLING. SHEPPING, HISTALLING A BRACHEG OF TRUSSES, DESIGN CONTROLLING SHEPPING, THE SECOND SOLID SHEPPING, THE SECOND SHEPPING, THE SECOND SHEPPING, THE BCG CONNECTION FOR THE AREA SHEPPING TO PAIRES ARE AND LUNCESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR BRANHINGS 160A Z. ANY INSPECTION OF FARES FOLUMED BY (1) SHALL BE PER ANNEX AS OF FPII 2002 SEC. 3. A SEAL ON THIS DRAWING HOLD AND THE SECOND SHEPPING SHE \*\*\*MARNIG\*\* RUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, HISTALLING AND BRACING, REFER 10 BCSI (BUILDING COMPONEN SAFETY INFORMATION), PUBLISHED BY TPI LIRINS PLATE INSTITUTE, 218 MORTH LE SIREET, SUITE 317. ALEXANDRIA, VA. Z2314) AND MICA (MODD TRUSS COUNCIL OF AREAGA. 6300 CHILERRISE LINE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERRISE INDICATED NO FORODS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. œ Design Crit: -2 - 11-2 - 111.5X4 III 1.5X4 III TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 6-0-0 Over 3 Supports 1.5X4 Ⅲ 4 X 4 == 中 Wind reactions based on MWFRS pressures. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. SPECIAL LUAUS --(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
From 64 PLF at 0.00 to 64 PLF at 8.00
From 64 PLF at 8.00 to 64 PLF at 16.00
From 4 PLF at 0.00 to 4 PLF at 16.00 .35.03 SOUCENSE 1.5X4 III 1.5X4 III 卣 07 BC LL DUR.FAC. BC DL TC DL TC LL SPACING TOT.LD. FL/-/4/-8 /E/R/ 32.0 1.25 10.0 PSF 20.0 PSF 2X4(A1) =24.0" 0.0 2.0 73 U 43 W 6.309" PSF PSF PSF JRFF-REF FROM SEQN DATE HC-ENG DRW HCUSR487 07333036 Scale = .5"/Ft R487--1TCU487 TCE / DF 13870 11/29/07 2815 Z01 REV

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. 110 mph wind, 23.10 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 Bot chord 2x4 SP #2
Webs 2x4 SP #3 In lieu of rigid ceiling use purlins to brace PLT TYP. Haines City, FL 33844 1.5X4(\*\*) III 0 - 9 - 3ALPINE Wave rizatio " ^ ~ 3 M R-0 U-180 W-3.5" R=78 PLF U=30 PLF W=13-4-12 3X4(\*\*) Dense \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THIS DESIGN. OR FABRICATION. HANDLING, SHIPPIDE, HISALLING A BRACHES OF TRUSSES. BY AFRY AND IPI. IN BCG CONNECTOR PRICE AND FAILURE PROVISIONS OF UNDS. (MATIONAL DESIGN SPCC. BY AFRY) AND IPI. IN BCG CONNECTOR PLATES AND EMPLICABLE PROVISIONS OF UNDS. (MATIONAL DESIGN SPCC. BY AFRY) AND FOR DRAWINGS 160A 2. ANY INSPECTION OF TRUSS AND. UNICES OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLOHOLD BY (1) SHALL BE PER ANNEX AS OF IPI1 2002 SEC. 3. A SEAL ON THIS DESIGN AND INTERNAL SHOWN. THE SUBJECTION OF PLATES FOLOHOLD BY (1) SHALL BE PER ANNEX AS OF IPI1 2002 SEC. 3. BEAL ON THIS DESIGN SHOWN. THE SUBJECTION OF PLATES FOLOHOLD BY (1) SHALL BE PER ANNEX AS OF IPI1 2002 SEC. 3. BEAL ON THIS DESIGN SHOWN. THE SUBJECTION OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE \*\*MARNING\*\* IRUSE'S REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO ESCI. (BUILDING COMPONENT SACTEY INFORMATION), PUBLISHON BY FPI (IRUSS PLANE INSTITUTE, 218 MORTH LEE SIRTET, SUITE 312, ALEXANDRIA, NA, 2231) AIDN HETAC (MOOD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAME, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DUECSS OFFICENTISE HOLOGODS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANIELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT CELLING. DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. Ш 4-0-6 4 0 6  $\infty$ 6-7-14 -6-11-14×4 III Design Crit: 1.5X4 III B C Ф **@** TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7-8 8 14-5-9  $4 \times 4 =$ 0ver .5X4 W Supports 2-7-8 -7-8 Deflection meets  $L/240\ \text{live}$  and  $L/180\ \text{total}$  load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. Bearing reaction of -/9# at (13-11-4, 20-6-0), requires special connection to resist uplift from loads other than wind. .5X4 **||** .5X4 III 7.24.123 歫 SOUS AS FLE \* -0-6 No. 66648 4-4-14 8 107 2X4(A1) = BC LL ВС TC LL DUR.FAC TC SPACING TOT.LD. FL/-/4/-/E/R/-민 79 U-180 W-6.309" 1.25 32.0 24.0" 10.0 PSF 20.0 PSF 2.0 0.0 PSF PSF PSF DATE REF JRFF-FROM SEQN-DRW HCUSR487 07332036 HC-ENG DF/DF Scale =.5"/Ft. R487--1TCU487 Z01 24351 11/28/07 S 2816

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. In lieu of rigid ceiling use purlins to brace 110 mph wind, 23.10 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. ITW Building Components Group, Inc.
Haines City, FL 33844

F' Carificate Cariff rization " Cary 1.5X4(\*\*) Ⅲ 0 - 9 - 3ALPINE Wave R=0 U=180 W=3.5" R=75 PLF U=28 PLF W=13-5-2 W #32 3 X 4 (\*\*) Dense \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD HE TRUSS IN COMPORMANCE WITH IP: OR FARREACHING, MANDLING, SHAPPING, HISFALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY ATRPA) AND IPI. ITM BCG CONNECTOR PLANES ARE MADE OF 20/18/1664 (M.H/SSK), ASIA MASS GRADE 40/160 (M. KM.SS) GALV. SITECA APPLY PLANES TO EACH FACE OF TRUSS AND. MURESS ON THEMSES OF LACE FACE OF THUSS AND. MURESS ON THE STREAM OF THE DEATH OF THE STREAM DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI I SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY \*\*WARNING\*\* IRUSSES BEOUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO SECTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TET (TRUSS PLATE INSTITUTE, ZIB HORTH LEE SHREET, SUHE 312, ALEXANDRALA, N., Z2314) AND NICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRENENT LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OFHERISE LANCE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OFHERISE LINGUISTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE 4 0 6 4-0-6 8 .7 - 0 - 15X4 III Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -14 1.5X4 W BC @ 6 000 7-8 8 14-5-9 WILD TING ANY  $4 \times 4 =$ Over SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE .5X4 3 Supports 3-4-4 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. bearing reaction of "38# at (14-Z-1, ZU-b-U), requires special connection to resist uplift from loads other than wind. .24. 1.5X4 **■** 1.5X4 III OUBLAS FLEE 7-0-12  $\Rightarrow$ CENS No.66648 REINEER ယ 8 ά œ ω ά 07 2X4(A1) =BC LL 8 C TC DL TC LL DUR.FAC SPACING TOT.LD. FL/-/4/-/E/R/-믿 39 U=180 W=3.5" 32.0 24.0" 1.25 20.0 PSF 10.0 PSF 0.0 2.0 PSF PSF PSF JRFF-DATE REF FROM SEQN-HC-ENG DF/DF DRW HCUSR487 07332044 Scale =.5"/Ft. R487--1TCU487 24357 11/28/07 5 - 0 - 112817 201

Bot chord 2x4 SP
Webs 2x4 SP #2 Dense #3

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

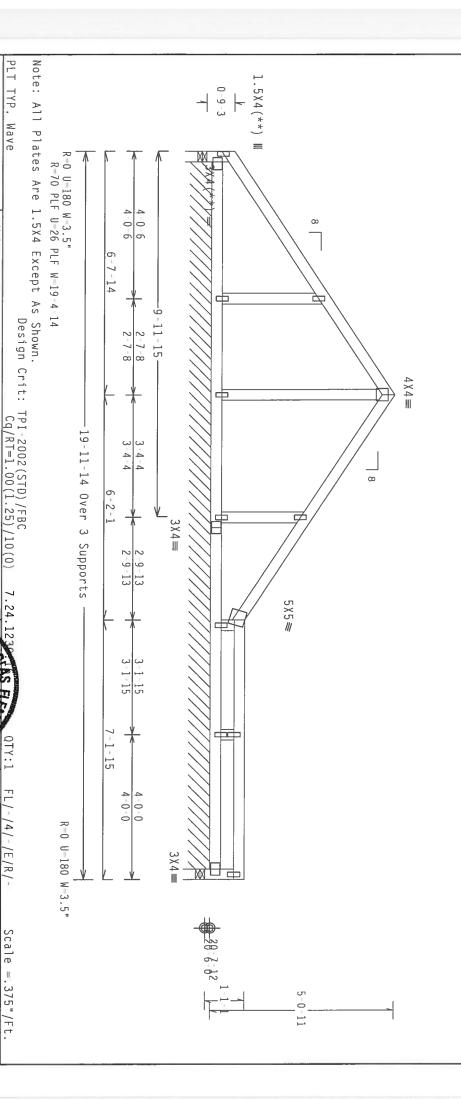
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 23.49 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures



ITW Building Components Group, Inc.
Haines City, FL 33844

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DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/IPI I SEC. 2.

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\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH IPT: OR FARRICATION, HANDLING. SUMPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFRA) AND IPT.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFRA) AND IPT.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFRA) AND IPT.

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PLATES TO EACH FACE OF TRUSS AND. UNLESS ON THE SPEN ANNEX AS OF TPTI 2002 SEC.3.

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O (W. K/H.SS) GALV. SIEEL. APPLY
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\*\*\*MARNING\*\* RUSSES REQUIRE EXTREME CARE IN FABRICATION, IMADILINE, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (RUSS PLATE INSTITUTE, 210 MORTH LEE STREET, SUITE 31Z, ALEXANDRIA, VA, Z2314) AND MICA (MOUD TRUSS COUNCIL OT AMERICA, 6300 CHICENES LAKE, MADISON, HI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNLESS OTHERNISE HOLDER TO AMERICA, BUTCHES AND TO PERFORMING INESE FUNCTIONS. UNLESS OTHERNISE HOLDER TO AMERICA, BUTCHES AND AMERICA, BUTCHES AND AMERICA, BUTCHES AND AMERICA CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

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11/28/07

No. 66640

Bot chord 2x4 SP
Webs 2x4 SP #27 Dense

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24  $^{\circ}$  OC, all BC @ 24  $^{\circ}$  OC.

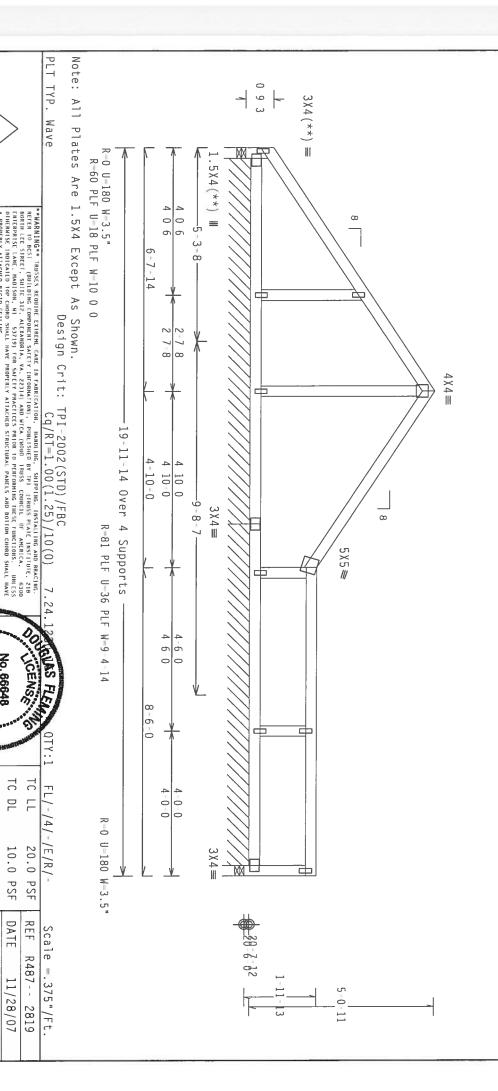
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 23.49 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures



Haines City, FL 33844
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DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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No. 66648

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DRW HCUSR487 07332039

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HC-ENG

DF / DF 24374

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG. THC. SHALL NOT BUT RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IPI: OR FARRICATHIG. HANDIJIG. SHIPPIDIG. HISTALLIG & BRACHIGO FIRUSSES; BUT OR FARRICATHIG. HANDIJIG. SHIPPIDIG. HISTALLIG & BRACHIGO FIRUSSES; BUT OF THE SECONDERS WITH APPLICABLE PROVISIONS OF THIS SHIPPIDIAL BUT OF THE BEAUTY OF T

110 mph wind, 21.70 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Bot chord 2x4 SP
Webs 2x4 SP 327 Dense Dense 8777 SPECIAL LOADS From From From (LUMBER 64 PLF at 64 PLF at 64 PLF at 4 PLF at 11.34 0.00 4.00 to to to

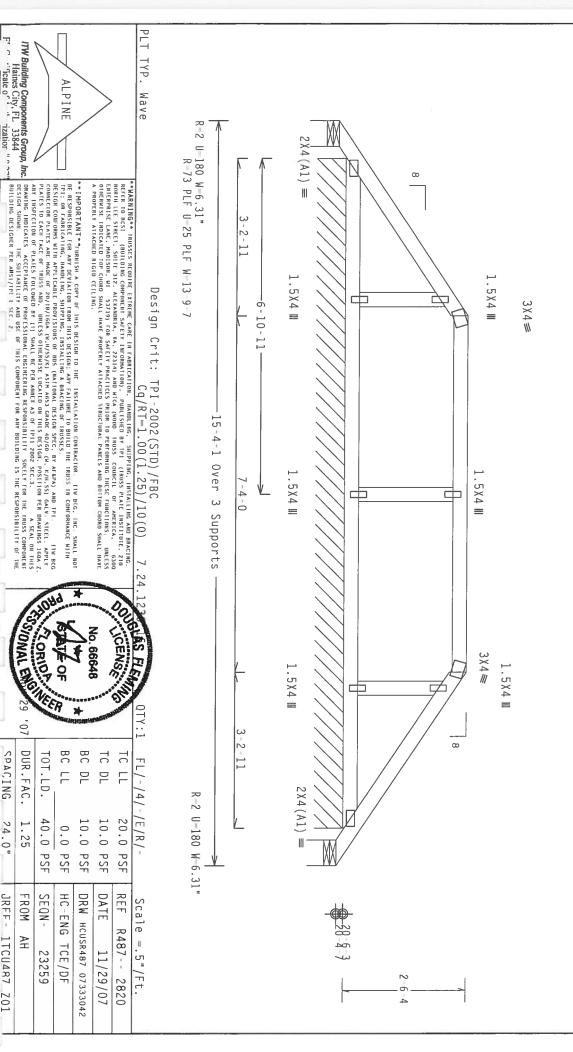
Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{cm}$ 

DUR.FAC.-1.25 / PLATE DUR.FAC.-1.25) 64 PLF at 4.00 64 PLF at 11.34 64 PLF at 15.34 4 PLF at 15.34

In lieu of 24° OC. structural panels use purlins to brace all flat TC @



Haines City, FL 33844

F' 'scale o' 'zatior'

DRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONEN BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

THE SUITABILITY AND USE OF THIS COMPONENT FOR

DHSIBILITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE

07

DUR.FAC.

FROM SEQN-

TOT.LD.

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PSF

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SPACING

24.0"

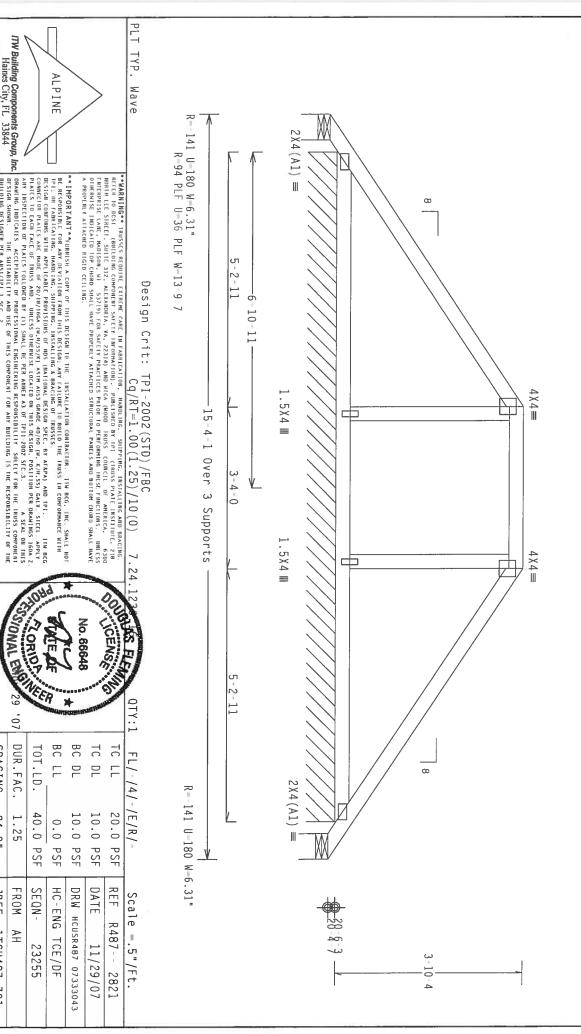
JRFF -

1TCU487 Z01

Bot chord 2x4 SP
Webs 2x4 SP 110 mph wind, 22.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18 Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures. #2 Dense #3 In lieu of 24" OC. TC TC BC SPECIAL LUAUS

From 64 PLF at 0.00 to 64 PLF at From 64 PLF at 6.00 to 64 PLF at From 64 PLF at 6.00 to 64 PLF at From 64 PLF at 9.34 to 64 PLF at From 4 PLF at 0.00 to 4 PLF at 6.00 9.34 15.34 15.34

structural panels use purlins to brace all flat TC @



Haines City, FL 33844
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DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESLINE SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER ANSI/FFI I SEC. Z.

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DUR.FAC.

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FROM SEQN

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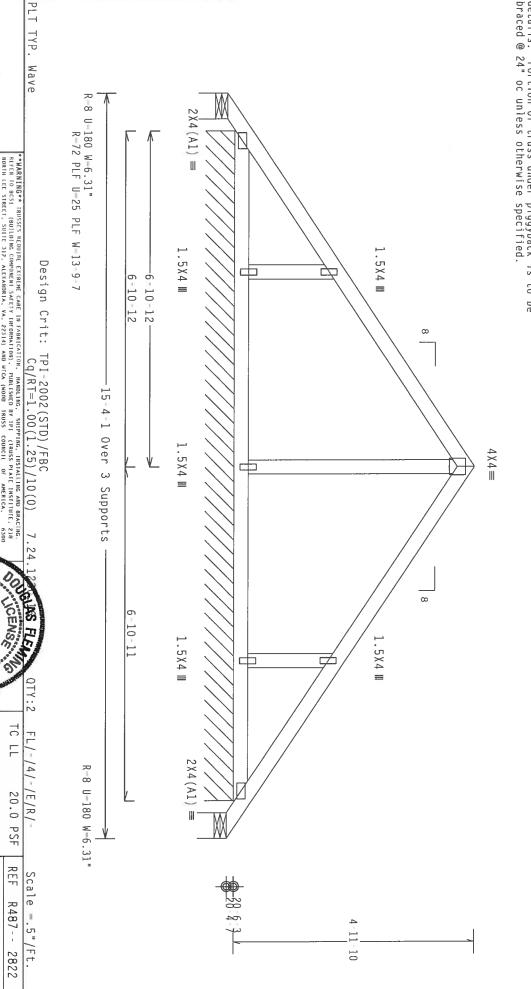
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Bot chord 2x4 SP
Webs 2x4 SP Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyt details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified. 110 mph wind, 22.93 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures #2 Dense #3 piggyback Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. SPECIAL LUAUS From From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
room 64 PLF at 0.00 to 64 PLF at 7.67
room 64 PLF at 7.67 to 64 PLF at 15.34
room 4 PLF at 0.00 to 4 PLF at 15.34 64 PLF at 7.67 64 PLF at 15.34 4 PLF at 15.34



Haines City, FL 33844
F\* C--i-ficate of A--i-rization # 0 7-9

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, NOT FALLER TO BUILD THE TRUSS IN COMPORANCE WITH FPI; OR FARRICATHO, HANDLING, SHEPPIG, INSTALLING & BRACHIG OF TRUSSES, OR FARRY AND IPI.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF 1005 (MALIONAL DESIGN SPEC, BY ATRA) AND IPI.

CONNECTOR PALES ARE HAND OF 20718J JACA (M-14558), ASTH AGS GRADE 40760 (M-27H-S) AGLY. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND. JUNESS OTHERNISE, LOCATED ON THIS DESIGN, POSITION PER BRANHOS 160A 2.

ANY HISPECTION OF PALES FOLLOWED BY (1) SHALL BE PER ADMEX AS OF IPI1-2002 SCC.3.

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ASSA. ON THE TRUSS COMPONENT

No. 66648

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DRW HCUSR487 07333044

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HC-ENG

TCE/DF 23249

TC LL

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REF

R487-- 2822

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DUR.FAC. SPACING

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A PROPERLY ATTACHED RIGID CEILING.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. INF. SULFABILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER AMSI/IPI I SEC. 2.

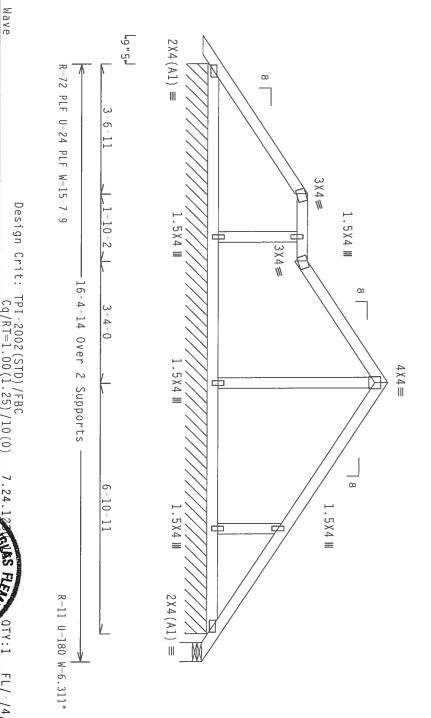
Bot chord 2x4 SP
Webs 2x4 SP 110 mph wind, 22.93 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18 #2 Dense #3

Wind reactions based on MWFRS pressures. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

877777 SPECIAL LUAUS From From 64 PIF at 25 From From 64 PLF 64 PLF 64 PLF 64 PLF 4 PLF 0.785.40 .74 .78 / PLATE DUR.FAC.\*
to 64 PLF at
to 64 PLF at 0.10 3.56 5.40 8.74 16.41 16.41

In lieu of 24" OC. structural panels use purlins to brace all flat TC @





TC LL FL/-/4/-/E/R/-20.0 PSF REF Scale =.375"/Ft. R487---2823

7.24

SOURIAS FLE

No. 66648

TC DL

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11/29/07

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HC-ENG

TCE / DF 23245

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DRW HCUSR487 07333045

TOT.LD.

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SEQN

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TP: OR FARBLEATHG. HANDLING, MINEPIDE. HISTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MAILUNAL DESIGN SPIC. BY AFAPA) AND TPI. ITH BCG CONNICTOR PLATES ARE MADE OF ZO/10/156A (M.H/SS/K), ASH AGS3 GRADU 40/60 (M.K/M.SS) GALV. SITEL APPLY PLATES TO EACH TACE OF TRUSS AND. MULESS ON THAT SET CONTROL ON THIS DESIGN, POSITION PER DRAWHINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF 1P11/2002 SEC.3. A SEAL ON THIS AND MINES AND M \*\*MARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RELIER TO BOSI. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE STREET, SUIT CITZ, ALEXANDRAIN, NA, 22314) AND VIEW (HORD TRUSS COUNCIL OF AMERICA, 6300 CHICEPREISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDING. THE ORDER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

PLT TYP. Wave

DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RES DESIGN SMOWN. THE SUITABILITY AND USE OF THIS COMPONENT FO BUILDING DESIGNER FER ANSI/TPI I SEC. 2. 02 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT HE IS THE RESPONSIBILITY OF THE

Haines City, FL 3844

ALPINE

29 07 SPACING DUR.FAC. 24.0" 1.25

JREF FROM 1TCU487

Z01

Bot chord 2x4 SP Webs 2x4 SP ##2 Dense Dense

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

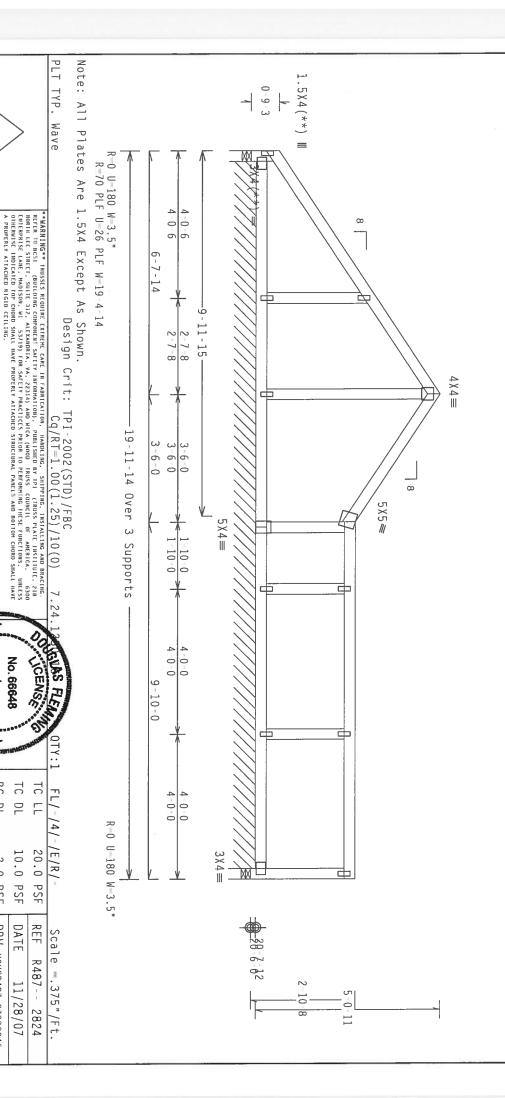
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

(\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 23.49 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



7: 2

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD!

ANY INSPECTION OF PLATES FOLLOWED BY (1)

SHALL BE PER ANNEX AS OF TP11-2002 SEC.3

OF TP11-2002 SEC.3. A SEAL ON THIS OWSIBILITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE

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HC-ENG

DF / DF 24381

DRW HCUSR487 07332046

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DATE

11/28/07

SPACING

24.0"

JRFF-

1TCU487

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ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGNA MY FAILURE TO BHILD THE TRUSS IN COMPORMANCE WITH FP: OR FAREACTING, HANDLING. SHAPPING, HISTALLING A BRACING OF TRUSSES, AND THE PRICE PROPERTY OF THE PRICE PRICE PROPERTY OF THE PRICE PRICE PROPERTY OF THE PRICE PRIC

No. 66648

Bot chord 2x4 SP Webs 2x4 SP ## 372 Dense

Bearing reactions of -53# at (0-0-0, 21-6-0), +138# at (13-3-3, 21-6-0), require special connection to resist uplift from loads other than wind.

110 mph wind, 23.49 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

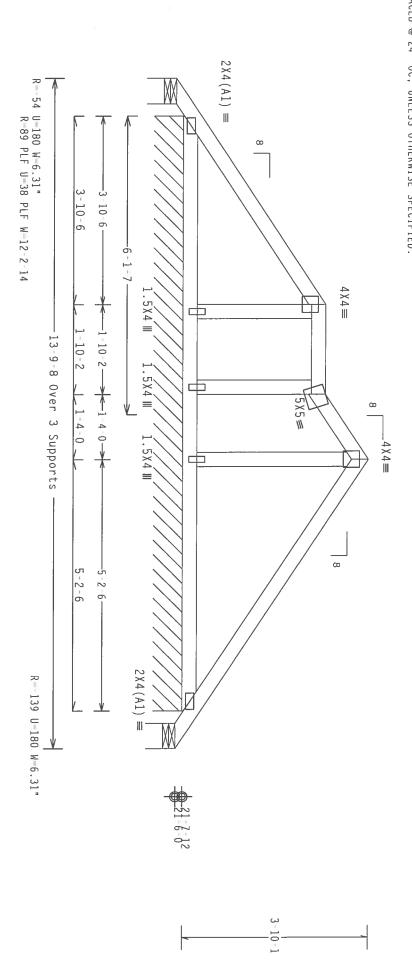
## COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d\_Common\_(0.148"x3.25",\_min.)\_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 0C.



PLT TYP.

Wave

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ITW Building Components Group, Inc. Haines City, FL 33844

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD

THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PI: OR FABRICATHO. HANDLING, SHEPTIGE, HISTALLING ABRAITION FRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF HDS (HAITONAL DESIGN SPEC. BY AFAPA) AND TPI. THE BCG CHINGCION PLAITS ARE MADE OF 20/10/16GA (W.H/SS/K) ASTH MADS DESIGN SPEC. BY AFAPA) FOR DEATHERS HOSE. AND FOR THE BOARDESS HOSE CONTRACTOR PLAITS ARE MADE OF 20/10/16GA (W.H/SS/K) ASTH MADS DESIGN SPEC. BY AFAPA (W. SIECEL. APPLY PLAITS TO EACH TACT OF TRUSS AND. UNITESS OF THE MADE AND ANY HIS DESIGN. POSITION PER BRANTICS 16GA 2. ANY HIS DE

\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BESS. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE ID.7. ALEXANDRIA, NA, 22214) AND HIGH ALG (MODO TRUSS COUNCIL OF AMERICA. 6300 CHIERREISE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INJECTION TO THE ORDERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.

Design Crit:

TPI-2002 (STD) /FBC Cq/RT=1.00 (1.25) /10 (0)

COUSTAS FLE

CENS No.66648

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R487--

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DRW HCUSR487 07332017

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DUR.FAC

SPACING

24.0" 1.25

JRFF-FROM SEQN-HC-ENG

1TCU487

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Bot In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC. Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. Haines City, FL 33844

F' Tcate o rization chord 2x4 SP Webs 2x4 SP / 0-9-3 1.5X4(\*\*) III ALPINE Wave R-0 U-180 W=3.5" R-70 PLF U-26 PLF W-19-4-15 #2 Dense #2 Dense #3 rization " . . . - -4-0 \*\*IMPORTANT\*\* FURBLES A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. HEC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRISS HE COMFORMANCE WITH PICT ON FARREACHING, MENDLING, SHIPPING, INSTALLING A BRACHM OF TRUSSES. HE COMFORMANCE WITH PICT ON FARREACHING, AND THE ANY ISSUES OF THE SECONDARY OF TRUSSES. HE ATAPA, AND THE ANY ISSUES OF THE SECONDARY OF TH 4-0-6 \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, HISTACLING BRACHEG, REFER TO BES! (BUILDING COMPORTER SAFETY INFORMATION), POBLISHED BY PP] (TRUSS PLAIE INSTITUTE, 218 HORTH LEE STREET, SUITE 137. ALEXANDRIA, VA. Z2214) AND NICA (NOOD TRUSS COUNCIL OF AMERICA, 6300 ERITEPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERNISE LUDICATED FOR PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING. ω 6 9-11-15 Design Crit: 2-7-8 8 4 X 4 = TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /10(0) 2-2-0 2-2-0 2 0 19-11-15 Over 3 Supports œ 5 X 5 🚚 3 - 2 - 13 X 4 ≡ Wind 110 mph wind, 23.49 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 Right end vertical not exposed to wind pressure.  $(\ensuremath{^{**}})$  2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. reactions based on MWFRS pressures. URIOT INE 4-0-0 4-0-0 11 - 2 - 107 TC LL DUR.FAC. ВC ВС  $^{\circ}$ SPACING TOT.LD. FL/-/4/-4-0-0 0 몬 R-0 U-180 W-3.5" /E/R/-32.0 3 \ 4 == 24.0" 10.0 PSF 20.0 PSF 1.25 0.0 2.0 PSF PSF PSF DATE REF FROM SEQN-DRW HCUSR487 07332047 JRFF-HC-ENG Scale =.375"/Ft. R487--1TCU4R7 24388 11/28/07 2826 Z01

Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. Wave Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Note: All Plates Are 1.5X4 Except As Shown. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC. Haines City, FL 33844
F' --- 'ficate o' ' 'rizatior " ----0-9-3 ALPINE 1.5X4(\*\*) III ##7  $^{\infty}$   $\circ$ U-180 W-3.5' 70 PLF U-26 Dense PLF U-26 PLF W-19-4-14 4-0 4-0-6 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MAY FALLURE FOR BUILD THE TRUSS IN COMPORNANCE WITH TP:: OR FABRELANTING, MANDIANG, SHEPPING, INSALLING & BRACLING OF TRUSSES.

DESIGN COMPORTS HITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFRA) AND TPI. ITM BCG. CONNECTOR PLANES ARE MADE OF ZO/JRJIGGA (M.H/SS/K) ASH MAGS GRANDE 40760 (M. K/H.SS) GALY. SITEFL. APPLY PLATES TO EACH FACE OF TRUSS AND, MURLESS OHERMISE COARDET ON THIS DESIGN, POSITION PER DRAWNINGS 160A.Z. ANY LISSECTION OF PLATES FOLLOWED BY (I) SHALL BE FER ANNEY A 3 OF TPI ZODZ SEC.3. A SCAL ON THIS BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY A PROPERLY ATTACHED RIGID CEILING. 6-7-14 9-11-15 Design Crit: 7 - 8 7 8 35.0 4 X 4 == THIS COMPONENT FOR 10 10" TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) გ×2. 19-11-14 Over 3 Supports OF THE TRUSS COMPONENT OF THE STATE OF THE S 6-0 6 0  $3 \times 4 \equiv$ Right end vertical not exposed to wind pressure bu ! M 110 mph wind, 23.49 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18  $(x^*)$  2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. 7.24 reactions based 200 Mas FLE 12-6-0 SIONAL ENGINEER 4-0-0 4-0-0 CENSE No. 66648 on MWFRS pressures 107 TC DL TC LL DUR.FAC. BC LL ВС SPACING TOT.LD. FL/-/4/ 4-0-0 2 0 R=0 U=180 W=3.5" 32.0 /E/R/-1.25 10.0 PSF 20.0 PSF 24.0" 3 \ 4 ≡ 0.0 2.0 MM PSF PSF PSF JRFF-REF FROM SEQN DATE HC-ENG DRW HCUSR487 07332002 Scale =.375"/Ft. R487--1TCU487 DF / DF 24395 11/28/07 2827 Z01

Bot chord 2x4 SP
Webs 2x4 SP #27 Dense Dense

In lieu of structural all flat TC @ 24" OC, panels or rigid ceiling use purlins to brace all BC @  $24\,^{\circ}$  OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

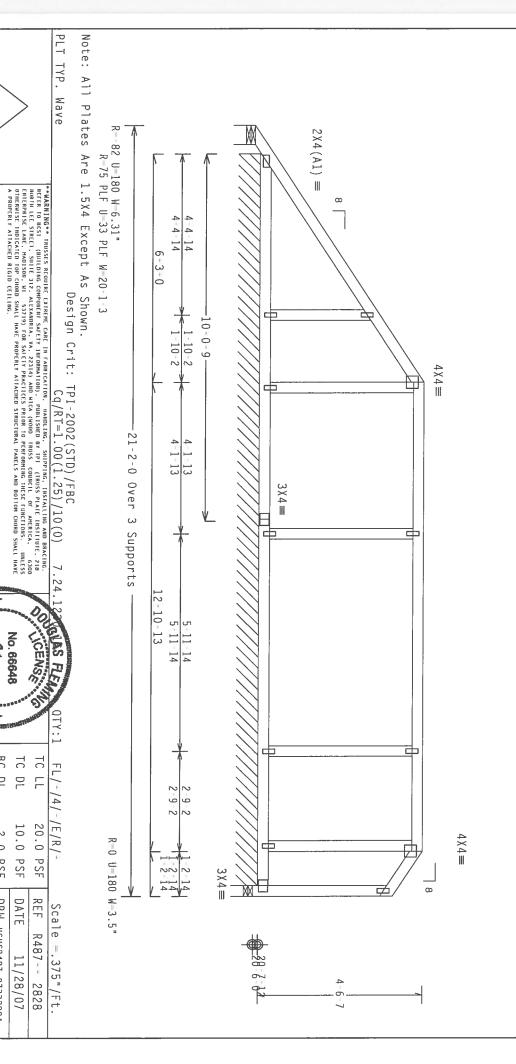
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

Bearing reaction of -82# at (0-0-0, 20-6-0), requires special connection to resist uplift from loads other than wind.

110 mph wind, 22.84 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



Haines City, FL 33844
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ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, ING. SHALL NOT DER RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN. FAILURE TO BUILD THE BRUSS IN COMPORMANCE WITH PIC OR FABRICATING, MANDLUNG, SHEPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONTORNS HITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY ATRPA) AND TP1. THE BCG CONNECTOR PLATES ARE HADE OF 20/18/1966 (M.H/SS/K), SAIH HASS GARAGE 40/60 (M. K/H.SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND. UNICESS ON THE MESS TO EACH FACE OF TRUSS AND. UNICESS ON THE MESS TO EACH FACE OF TRUSS AND. UNICESS ON THE MESS TO EACH FACE OF TRUSS AND. UNICESS ON THE MESS TO EACH FACE OF TRUSS AND. UNICESS ON THE MESS TO EACH OF PICE TO BE ADMITS AS THE ADMITS AND THE

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SPACING DUR.FAC

24.0" 1.25

JRFF- 1TCU487 Z01

No. 66648

BC DL TC DL

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DATE REF

11/28/07

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BUILDING DESIGNER PER ANSI/TPI I SEC.

Bot chord 2x4 SP
Webs 2x4 SP #2 Dense

In lieu of structural all flat TC @ 24" OC, panels or rigid ceiling use purlins to brace all BC @  $24\,^{\circ}$  OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

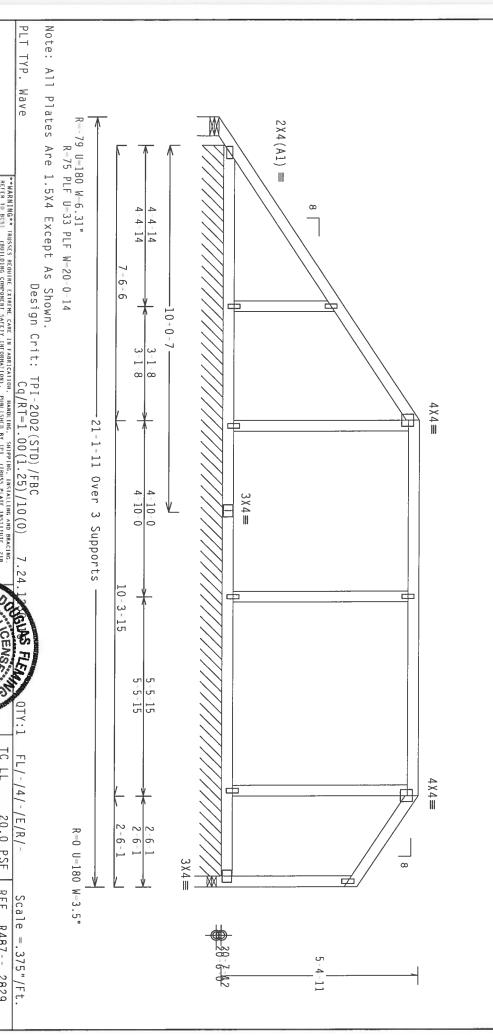
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

Bearing reaction of "/9# at (0-0-0, 20-6-0), requires special connection to resist uplift from loads other than wind.

110 mph wind, 23.27 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure



Fr Carificate of Anti-rization 40 779 ITW Building Components Group, Inc.

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH IP: OR FLAREFACTHO. HANDLING. SHIPPIDIG. HISTALLING A BRACHEO OF TRUSSES. DESIGN. ANY FLOW OF THE STATE OF THE STATE

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDEING, SHIPPING, INSTALLING AND BRACING.
RETER TO BEST (QUILIDING COMPONENT SAFETY INFORMATION), PUBLISHOED BY FIT (FRUSS PLATE INSTITUTE, 218)
HORTH LEE STREET, SUITE 137, ALEXANDRIA, VA, 221314) AND WICA (MODO TRUSS COUNCIL OF AMERICA, 6300
ERITERPRISE LARE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMHUG INESE FUNCTIONS. UNICESS
OTHERWISE LIDOLGALIED FOR CHARD SWALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE
A PROPERLY ATTACHED REGID CELLING.

CENS No. 66648

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DATE REF

11/28/07 2829

DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD

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DUR.FAC.

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FROM SEQN-HC-ENG

TOT.LD.

PSF PSF

SPACING

24.0"

JRFF-

1TCU487

201

BC LL

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ВС

PSF

DRW HCUSR487 07332012

DF / DF 24655

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / #2 Dense #2 Dense #3

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

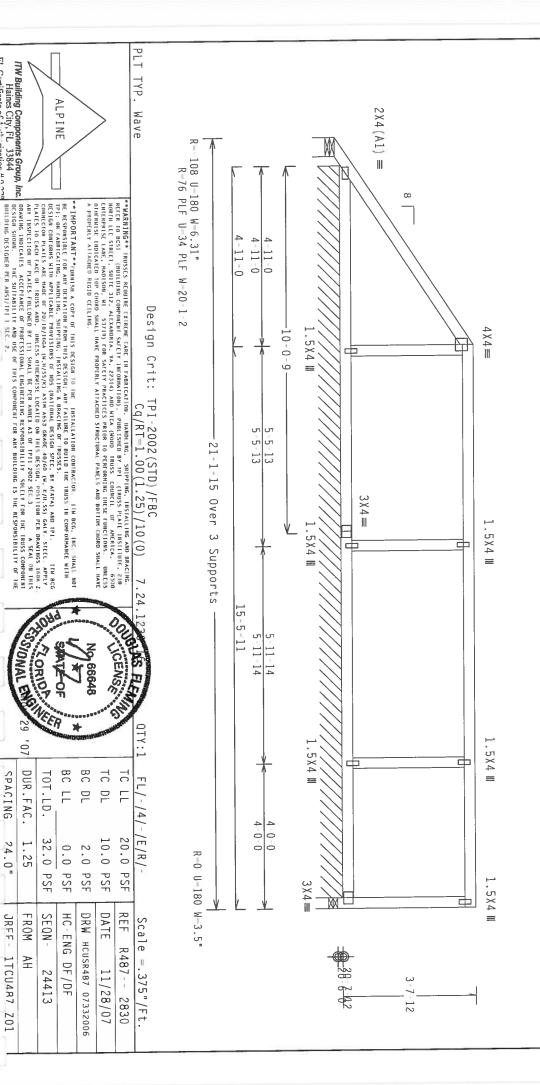
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

Bearing reaction of "10/# at (0-0-0, 20-5-0), requires special connection to resist uplift from loads other than wind.

110 mph wind, 22.40 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

₩ind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure



BUILDING DESIGNER PER

07

DUR.FAC.

1.25

FROM

SPACING

24.0"

JRFF-

1TCU487

Z01

Bot chord 2x4 SP
Webs 2x4 SP #2 Dense #2 Dense #3

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets  $L/240\,$  live and  $L/180\,$  total load. Creep increase factor for dead load is  $1.50.\,$ 

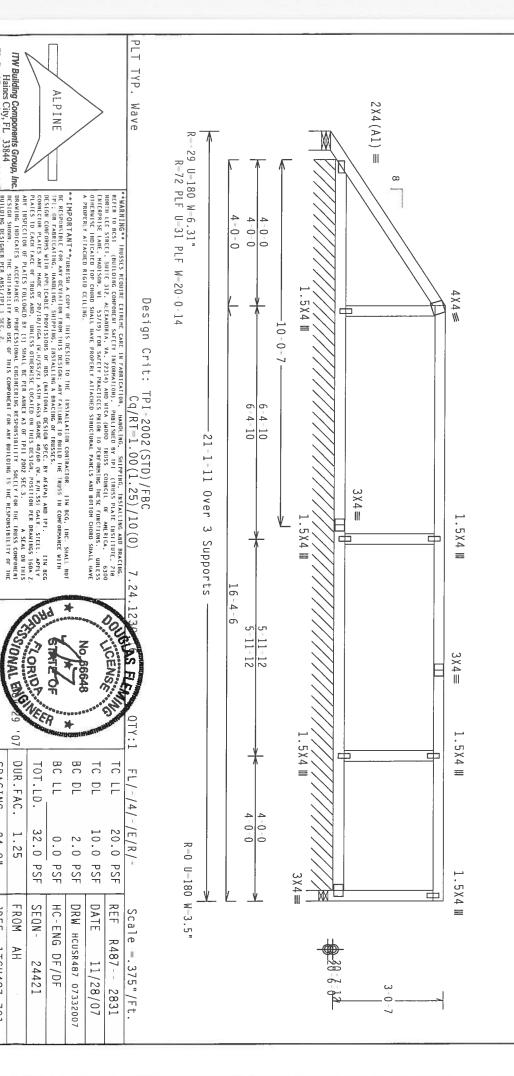
Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

Bearing reaction of -28# at  $(0\cdot 0\cdot 0$ ,  $20\cdot 6\cdot 0)$ , requires special connection to resist uplift from loads other than wind.

110 mph wind, 22.09 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Haines City, FL 33844
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BUILDING DESIGNER PER ANSI/TPI 1 SEC.

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24421

SPACING

24.0" 1.25

JRFF-FROM

1TCU487 Z01

l op Bot p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP #22 Dense Dense

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

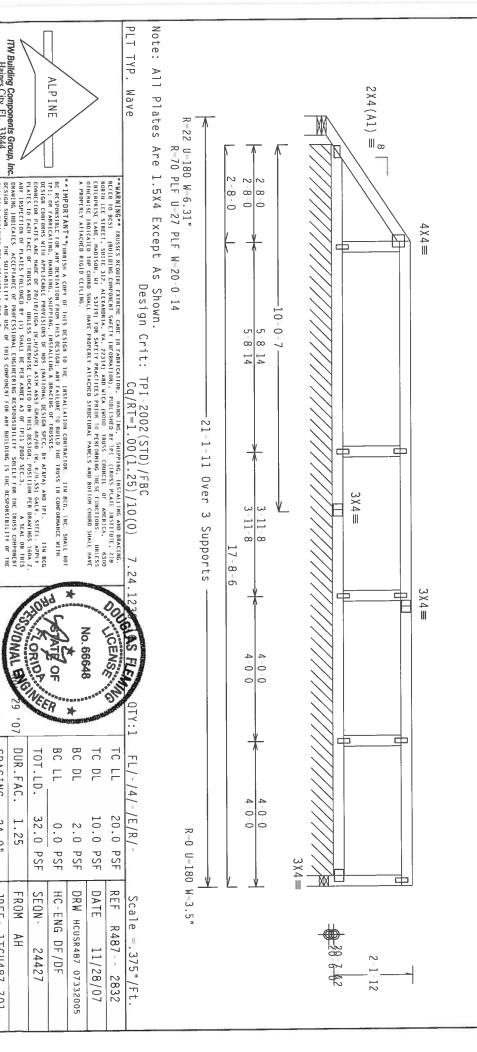
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

110 mph wind, 21.65 ft mean hgt, ASCE /-UZ, CLUSED bigg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.



Haines City, FL 33844

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BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

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SPACING

24.0"

JRFF-

1TCU487

201

Bot t chord 2x4 SP
Webs 2x4 SP #27 Dense

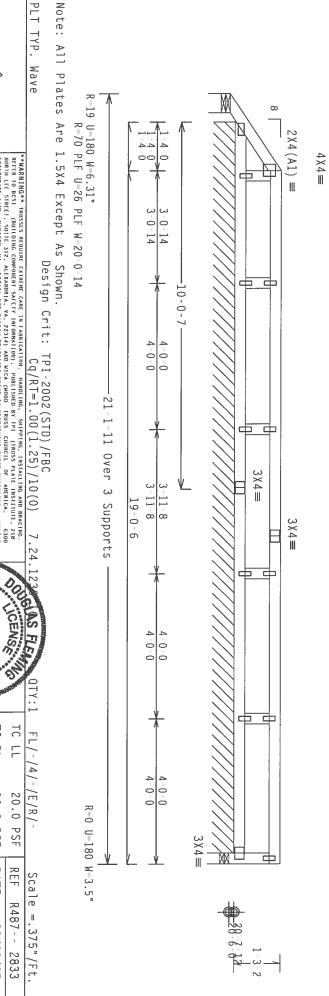
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 21.20 ft mean hgt, ASCE /-U2, CLUSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{.}$ 



Haines City, FL 33844
Fr Cate of ization ALPINE

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEEL (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, ZIB MORTH LET STREEL, SUITE IZZ ALEXANDRIAL, VA, 25214) AND MICHA (MODD TRUSS COUNCIL OF AMERICA. 6300 CHIERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED INFORMED SHALL HAVE PROPERLY ATTACHED RIGHD CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CENTROL SHALL HAVE

Vo. 66648

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R487--

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DATE REF

11/28/07 2833

DRW HCUSR487 07332031

8 C ВС TC DL TC

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PSF

DF / DF 24433

TOT.LD.

32.0

PSF

SEQN-HC-ENG

\*\*IMPORTANT\*\*FURNISH A COTY OF THIS DESIGN TO THE HISTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BC. RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. ANY TAILURE TO BUILD THE REVS IN COMPORMANCE HITH PIT: OR FLAREACHING, INAULING, SHAPPING, HISTALLING A BRACHING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF UNDS (MATIONAL DESIGN SPEC, BY ATRA) AND IPI. THE BCC COMMICTION PARTIES AND THE APPLICABLE PROVISIONS OF UNDS (MATIONAL DESIGN SPEC, BY ATRA) AND IPI. APPLY PLATES TO EACH FACE OF TRUSS AND, UNILESS OHERNIST LOCALED ON THIS DESIGN, POSITION PER DRAWHINGS 150A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OT IPI1 700S SEC.3. A SEAL ON THIS DRAWHING INFORMATION OF THE SECONDOLUTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OT IPI1 700S SEC.3. A SEAL ON THIS DRAWHING INFORMATION OF THE THUS COMPONENT OF THE PLATES FOR THE THUS COMPONENT OF THE PLATES FOR THE THUS COMPONENT OF THE PLATES FOR THE PL

DRAMING HUDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPO DESIGN SHOWN. HE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. ORIO PIER

SPACING DUR.FAC

> 24.0" 1.25

JRFF-FROM

1TCU487

Z01

Bot chord 2x4 SP
Webs 2x4 SP #2 Dense

110 mph wind, 21.04 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

In lieu of 24° OC.

structural panels use

purlins to brace all flat TC @

From From

64 PLF 64 PLF 62 PLF 4 PLF

0.00 2.00 12.29 0.00

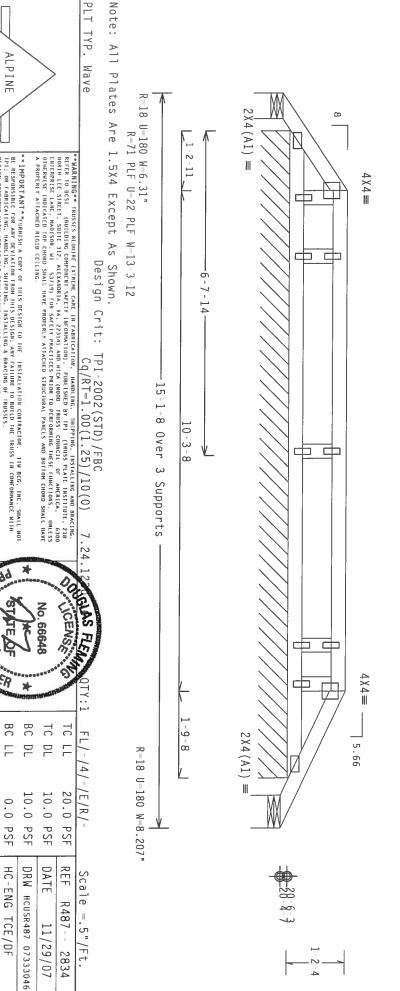
/ PLATE DUR.FAC.=1.25)
) to 64 PLF at 2.00
) to 64 PLF at 12.29
) to 62 PLF at 15.13
) to 4 PLF at 15.13

SPECIAL LUAUS

From 64 PLF at 0 no '

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{cm}$ 

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.



ITW Building Components Group, Inc.
Haines City, FL 33844
F\* Configuration (Configuration (Configuration)) ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. FOR FAILURE TO DULLD THE TRUSS IN COMPORMANCE WITH JP: OR FAREACHING, AMOUNTING, SHAPPING, INSTALLING A BRACHING OF TRUSSES.

DESIGN CONTRONS WITH APPLICABLE PROVISIONS OF DOS (NATIONAL DESIGN SPEC, BY ATREA) AND TPI. THE PROVISIONS OF DOS (NATIONAL DESIGN SPEC, BY ATREA) AND TPI. THE STREAM OF THE

29 07 SPACING DUR.FAC. ВС TOT.LD. 40.0 24.0" 1.25 0.0 PSF PSF JRFF-SEQN-FROM HC-ENG 1TCU487 23263

201

Bot chord 2x4 SP
Webs 2x4 SP #2 Dense

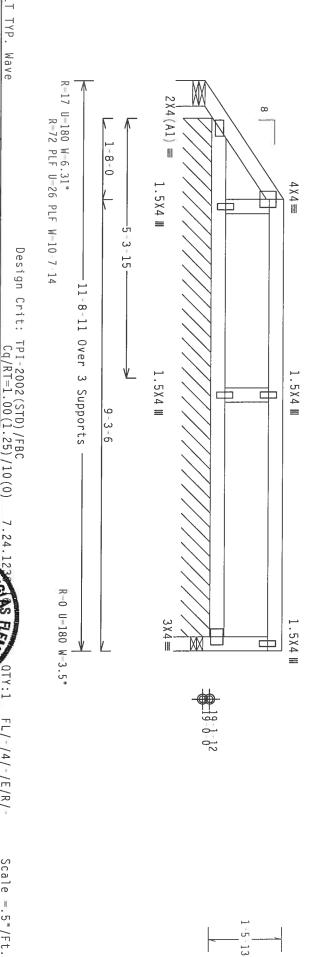
In lie lieu of structural panels or rigid ceiling use purlins ace all flat TC @ 24" OC, all BC @ 24" OC. to

Refer to Dwg PIGBACKA0207 or PIGBACKB0207 for piggyback details. Portion of truss under piggyback is to be braced @ 24" oc unless otherwise specified.

110 mpn wind, 19.81 ft mean ngt, ASCE /-U2, CLUSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/ )=0.18

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



A PROPERLY ATTACHED RIGID CEILING.

7.24.123

TC LL

20.0

PSF

REF

2835

11/29/07

FL/-/4/-

/E/R/-

Scale = .5"/Ft. R487--

PLT TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO SHILD THE TRUSS IN COMPORNANCE WITH TPI; OR FARRICATING, HANDLING, SHIPPING, HISTALLING A BRACHIG OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROPYSIONS OF DUS (MATIONAL DESIGN SPEC, BY ATRA) AND TPI. BCG CONNECTOR PLATES ARE HADE OF ZO/IN/16/GA (M.H/SS/K) ASTH A653 GRADE 40/GO (M. K/M.SS) GALV. SITEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNICES OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. PALES TO EACH FACE OF TRUSS AND. UNICES OTHERSISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPI] ZOOZ SEC.3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLHEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING DESIGNER PER AMS//FFT I SEC. Z. ANY BUILDING 02 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT HG IS THE RESPONSIBILITY OF THE

ITW Building Components Group, Inc. Haines City, FL 33844 Fr Carificate Communication (1979)

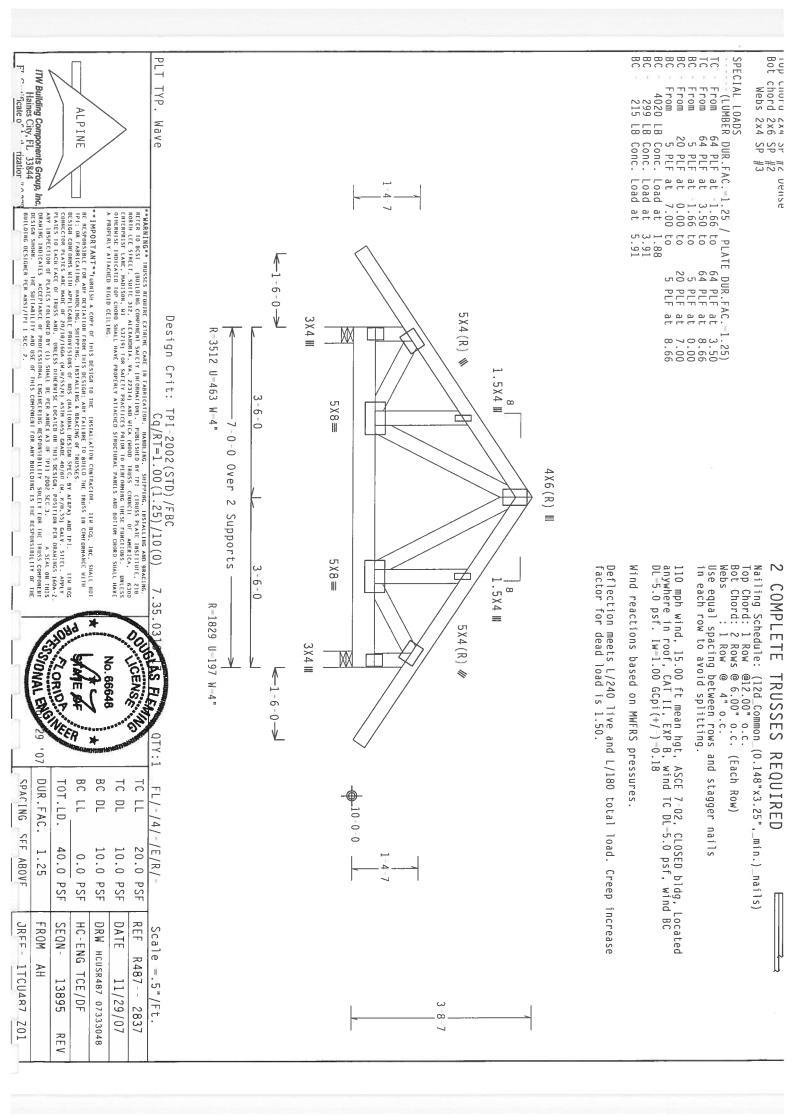
ALPINE

GODGLAS FLEAT CENSE No. 66648 ANE OF 107 DUR.FAC BC DL SPACING ВС TC DL TOT.LD. 32.0 24.0" 1.25 10.0 PSF 0.0 2.0 PSF PSF PSF DATE JRFF-FROM SEQN HC-ENG DRW HCUSR487 07333047 1TCU487

TCE / DF 23186

Z01

110 mph wind, 23.10 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf. Iw=1.00 GCpi(+/-)=0.18 PLT TYP. Refer to DWG PIGBACKB0207 for piggyback details. PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Bot chord 2x4 SP #2
Webs 2x4 SP #3 Haines City, FL 3844 ALPINE Wave  $2X4(A1) \equiv$ 74 U=180 W=6.31" R=84 PLF U=34 Dense -84 PLF U-34 PLF W-14-0-12 \*\*IMPORTANT\*\*rurrish a copy of this design to the installation contractor. The beg, inc. shall not be responsible for any deviation from this design, for falling to dull the frass in componance with fee responsible for any deviation from this design. The falling to dull the frass in componance with fee responsible from the fee responsibility and use of this design from the feet sold feeling from the feet sold feeling from the feet sold feeling from the feeling feeling from the feet sold feeling from the feet sold feeling from the feeling feeling from the feet sold feeling from the feeling from the feet sold feeling from the feeling f \*\*MARNING\*\* RRISES REQUIRE EXTRINE CARE IN FAMRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY HEFORALIDS), PROBLISHED BY TP) (TRUSS PLATE INSTITUTE, ZIB HORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. ZZ313) AND HETAC (MOOD TRUSS COUNCIL DE AMERICA. 6300 CHIEERRISE LAME, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICES OTHERWISE INDICATED FOR PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CEILING. DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. α Design Crit: -7-0-61<u>5X4</u>III 0 1.5X4 Ⅲ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 15-7-6 0ver 4 X 4 = 3 Supports 5 X 4 III Bearing reactions of -74# at (0-0-0, 20-6-0), -74# at (15-1-1, 20-6-0), require special connection to resist uplift from loads other than wind. In lieu of rigid ceiling use purlins to brace Wind reactions based on MWFRS pressures -2-7-8 .24.123 SOUS AS FLE 1.5X4 III 1.5X4 III ф No. 66648 70R10P 7-0-6 07 4-4-14 BC DL DUR.FAC. TC DL SPACING TOT.LD. œ FL/-/4/-2X4(A1) =ВС 32.0 20.0 /E/R/-24.0" 1.25 10.0 PSF 0.0 74 U=180 W=6.31" 2.0 PSF @ PSF PSF SEQN-DATE REF FROM HC-ENG JRFF-DRW HCUSR487 07332003 Scale =.5" R487--1TCU487 Z01 DF / DF 24647 11/28/07 2836



Bot chord 2x4 SP Webs 2x4 SP #2 Dense #2 Dense #3

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC  $0\ensuremath{\text{C}}\xspace.$ **@** 24"

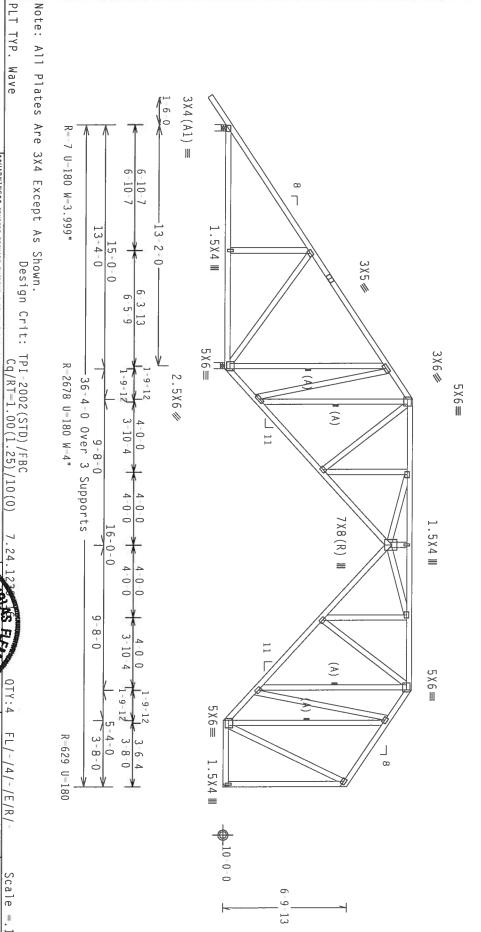
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{cm}$ 

Bearing reaction of  $\cdot 7\#$  at (0-0-0, 10-0-0), requires special connection to resist uplift from loads other than wind.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure



PLT TYP.

Wave

\*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE TH FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING, BETER TO BEST. (BUILDING COMPORTH SAFETY INFORMATION), PHALSHED BY FPT (TRUSS PLATE INSTITUTE, 218 HORRH LEE STREET, SUITE 127 ALEXANDRAIL, NA, 22214) AIDN HEACA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERREISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE LODGERIALD TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHT ETILING.

\*\*IMPORTANT\*\*THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURG TO BHILD THE TRUSS IN COMPORTANCE HITH PPI, OR FARRICALING, AND FOLLOWING, SHIPPING, THISTALLING A BRACHEG OF TRUSSES. OR A FARAL AND FILL THE BCG. CONTROLMS SHIP APPLICABLE PROVISIONS OF HIDS (MATIONAL DESIGN SPEC, BY AFRAL AND FILL APPLY DESIGN CONTROLMS ARE ARE ARE ASSOCIATED ON THE STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2 PLATES TO EACH FACE OF TRUSS AND.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 3844

F' 'ficate c' 'rizatio " 'S DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2 ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF 1911 2002 SEC.3. A SE THE SULLABILLITY AND ANY BUILDING DZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE

IIH BCG EEL. APPLY INGS 160A-Z. COURT STAN 07 BC DL DUR.FAC TC DL SPACING TOT.LD. FL/-/4/-/E/R 40.0 10.0 20.0 1.25 10.0 PSF 24.0" 0.0 PSF PSF PSF PSF DATE FROM SEQN-REF JRFF-HC-ENG DRW HCUSR487 07332016

DF / DF 24334

1TCU487 Z01

Scale =.1875"/Ft.

R487--

11/28/07 2838

In lieu of structural panels use purlins to brace all flat TC @  $24\mbox{\tt "}$  OC. Bot chord 2x4 SP
Webs 2x4 SP Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ ITW Building Components Group, Inc. Haines City, FL 33844 F' C--'ficate c' rizatior " 1779 TYP. ALPINE Wave #2 Dense #3 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BC RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, THE FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FLAREACHING, HANDLING, SHEPPING, HISTALLING A BRACHING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MUS (MATIONAL DESIGN SEC. B. WATERA) AND TPI.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MUS (MATIONAL DESIGN SEC. B. WATERA) AND TPI.

DESIGN SHOW, THE SECOND OF TRUSS AND MAKES OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHIGS IGNA Z. ANY HISTELTION OF TRACE OF TRUSS AND MAKES OTHERWISE LOCATED ON THIS DESIGN. POSITION FROM SHALL BE FER ANHREX AS OT TPI1-2002 SEC. 3. SEAL ON THIS DESIGN SHOWN.

DRAWHIG INDICATES ACCEPTANCE OF PROTESSIONAL TRUSHERS HISTELTS SOLLY FOR THE TRUSS COMPONENT DESIGN SHOWN.

THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESI DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/JPT 1 SEC. 2 \*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, NANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST QUILEDING COMPONENT SAFLY INFORMATION), PUBLISHED BY PT (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI \$3/19) FOR SAFETY PRACTICES BRIOR TO PERFORMING HICKS FUNCTIONS. QUILESS OFFICEN TO THE SAFETY PRACTICES FROM TO PERFORMING HICKS FUNCTIONS. A PROPERLY ATTACHED RIGID CEILING. 1 6 0 V  $2X4(A1) \equiv$ R-683 U-180 W-4" MM Design Crit: 8 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 1.5X4// 0-0 13-4-0 Over 2 Supports Right end vertical not exposed to wind pressure. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. iw=1.00 GCpi(+/ $^{\prime}$ )=0.18 Wind reactions based on MWFRS pressures 7.24.123  $4 \times 4 \equiv$ 4 X 8 ≡ CENSO No. 66648 ATH OF 4-4-0 R-546 U-180 W-4" 1.5X4 III 3 \ 4 ≡ BC DL DUR.FAC. 8C TC DL C SPACING TOT.LD. ₩Φ FL/-/4/-40.0 20.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 \_10-0-0 PSF PSF PSF 6 REF FROM SEQN-DATE DRW HCUSR487 07333024 JRFF -HC-ENG DAL/DF Scal le =.375"/Ft. R487--1TCU487 Z01 23091 11/29/07 2839

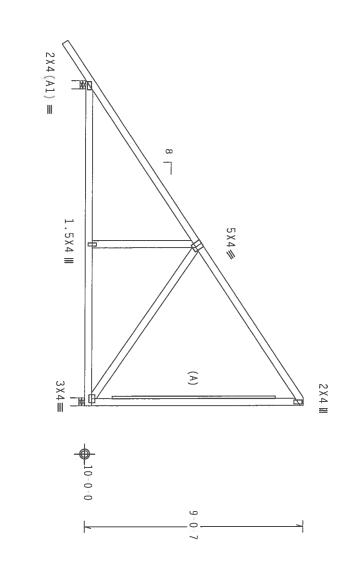
Bot chord 2x4 SP
Webs 2x4 SP PLT TYP. Deflection meets  $L/240\,$  live and  $L/180\,$  total load. Creep increase factor for dead load is 1.50. OC In ITW Building Components Group, Inc. Haines City, FL 3844 F' C--' ficate c' \* ...'- rizatior # ^ ~ ~ 9 lieu of ALPINE Wave structural panels use purlins to brace all flat TC @ #2 Dense #2 Dense #3 \*\*IMPORTANT\*\*TURNISH A COPY OF HHIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI: OR FLORESCENTING, HANDLING, SHEPPING, HISALLING A BRACHIGO OF TRUSSES. BUSION CONTRACT HING, AND THIS DESIGN SECONDECTOR OF THE SECOND SECO \*\*MARNING\*\* HRUSSIS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER 10 BCS1 (BUILDING COMPONENT SAFET INFORMATION), PUBLISHED BY TP) (TRUSS PLATE INSTITUTE, Z18 HORTH LEE STREET, SUITE 312, ALEXANDRÍA, WA, Z2313) AND HICA (MODO TRUSS COUNCIL OF ARREIGA. 6300 CHIEFRRISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIBE TO PERCORMING HEES FUNCTIONS. UNITESS OTHERWISE INDICACTED TOP CHORDS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANIELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGHD ELLING. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI I SEC. 2. 41-6-0×  $2X4(A1) \equiv$ -683 U-180 W-4" Design Crit: USE OF THIS COMPONENT FOR Φ ٦ ထ œ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 13-4-0 Over 1.5X4 III 1-0-0 3X4 / IZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT IN THE TRUSS TO THE THE SECOND IN THE SECOND 2 Supports 3 9 ىٰ 9 110 mph wind, 15.00 ft mean hgt, ASCL /-02, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures. .24. COUNTY FIRM  $4 \times 4 =$ 4 X 8 ≡ R=546 U=180 W=4" CENS 2-4-0 2-4-0 4-0 No. 66648 1.5X4  $3 \times 4 \equiv$ QTY:1 1071 SPACING DUR.FAC. BC DL TC DL ВС TC LL TOT.LD. FL/-/4/-/E/R/- $\infty$ 40.0 20.0 1.25 24.0" 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF JRFF SEON DATE REF FROM HC-ENG DRW HCUSR487 07332035 Scale = .3125"/Ft. R487--1TCU487 DF / DF 24318 11/28/07 2840 201

Bot chord 2x4 SP
Webs 2x4 SP (A)  $1\times 4$  #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{.}$ ##2 Dense

DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



1 6 0 R-683 U-180 W-4" 13-4-0 Over 2 Supports R-545 U-180 W-4"

\*\*WARNING\*\* IRUSSES REQUIRE EXTREME EARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FP1 (TRUSS PLATE INSTITUTE, 218 HORRIN LEE SIREET, SUITE 312, ALEXANDRIA, VA, 22314) AND HICACA (MODD TRUSS COUNCELL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNILESS OTHERWISE HOLDCALED TO PERFORM SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION COURSE. Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. HC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE FRUSS IN CONTORNANCE WITH TPI: OR FLAREATHING, HANDLING, SHEPPING, HISTALLING A BRACHEG OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DADS (MAIDDLA, DESIGN SOC. AVARA) AND TPI.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DADS (MAIDDLA, DESIGN SOC. AVARA) AND TPI.

PLATES IN EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR DAMPHOS 160A Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANHER AS OF TPI) 2002 SEC. 3.

A SEAL ON THIS

DRAHING INDICALES ACCEPTANCE OF PROFESSIONAL FROM THE ASSOCIATION OF THE TRUSS COMPONENT

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7.24. GOUGHAS FLEW WONAL BROWNER CENSE o. 66648 QTY:1107 SPACING BC LL TC LL DUR.FAC ВС  $\mathcal{I}$ TOT.LD. FL/-/4/-2 P 40.0 /E/R/-10.0 20.0 PSF 24.0" 1.25 10.0 PSF 0.0

PSF PSF

HC-ENG DAL/DF DRW HCUSR487 07333025 REF

Scale =.25"/Ft. R487--

DATE

11/29/07 2841

PSF

23104

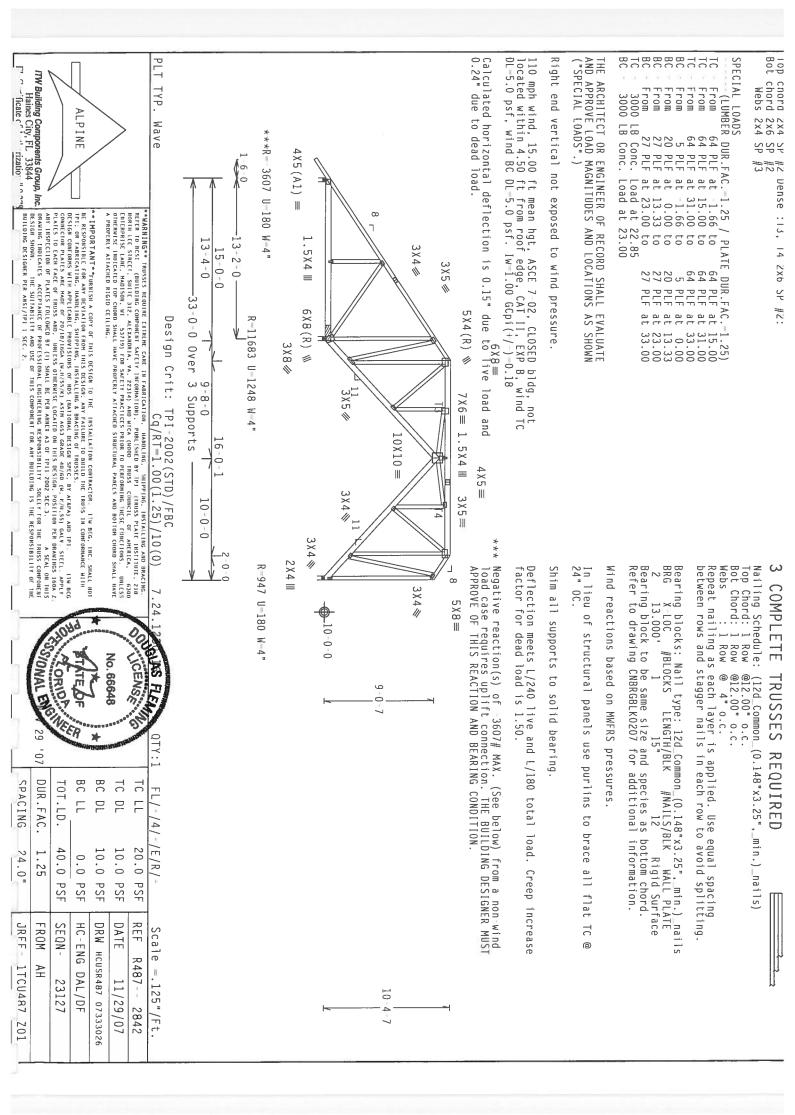
JRFF

1TCU487

Z01

FROM SEQN-

Haines City, FL 33844 ITW Building Components Group, Inc. ALPINE BUILDING DESIGNER PER ANSI/TPI 1 SEC.



Bot chord 2x6 SP #1 Dense :B2 2x8 SP SS: :B3 2x4 SP #2 Dense: B2 2x8 SP SS: Webs 2x4 SP #3 End verticals exposed to wind pressure. Deflection meets  $\ensuremath{\mathsf{L}}/240$  criteria for brittle and flexible wall coverings. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ Collar tie braced with continuous lateral bracing at 24\* rigid ceiling. PLT TYP. Haines City, FL 33844
F' 'ficate c' 'rizatior " 'Tizatior " 'Tizat ALPINE 20 Gauge HS, Wave 1 6 0 3×4 Ⅲ 5×6/ R-1974 U-180 W-4" \*\*IMPORTANT\*\*FURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH PI; OR FLARECRITHG, HANDLUNG, SHEPPIG, INSTALLING A BRACHIG OF TRUSSES, DESIGN CONTROLS OF THE PICABLE PROVISIONS OF THOS (MATIONAL DESIGN SEC.) A KARAN, AND THIS DESIGN CONTROLS ARE HADE OF 20/18/166A (M.H/SS/K) ASTH A653 GRADE 40/60 (M. K/H-SS) GALV. STEEL. APPLY FIARLS INDEAD OF THIS DESIGN, POSITION PER DRAWNINGS 160A Z. PIARLS TO EACH FACE OF TRUSS AND. UNLESS OHICRAIST LOCATED ON THIS DESIGN, POSITION PER DRAWNINGS 160A Z. PIARLS TO COMPRESSED OF THIS DESIGN. POSITION PER DRAWNINGS 160A Z. AND THE Z. A \*\*HARNING\*\* TRUSSES REQUIRE CIRTRE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFIER TO BCSI. (BUILDING COMPONENT SAFLY INFORMATION), PUBLISHED BY IPT (TRUSS PLATE INSTITUTE, 2218 MINDLING, 218 MINDLING, 218 MINDLING, 218 MINDLING, STATE INSTITUTE, 218 MINDLING, STATE AND SOME DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. 4 X 5 ≡ 3×6/ €X6= HS512 € 10 4X10(R) III 6-6 Design Crit: 12-6-0 3 X 9 Ⅲ 0C. TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -24-8-0 Over 2 Supports 82 3 X 4 ≡ 1.5X4 4X8(R) **Ⅲ** 12 NG IS THE RESPONSIBILITY OF THE 0-0 8 0 3 X 4 ≡ 12 T3 BC attic room floor loading: LL = 6-6-0 to 18-6-0. Calculated horizontal deflection is 0.12" 0.22 due to dead load. 110 mph wind, 15.00 ft mean hgt, ASCE /-02, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures 5-6-6 œ 4 X 10 (R) Sandas , 3 X 8 III CENS 6X6≡ = Vo. 66648 6X6// 4 X 5 = 4-8-0 4 X 5 // R=1880 U=180 07 5 X 5 // 3 X 4 III 40.00 psf; DL = 10.00 psf; from BC LL DUR.FAC. BC DL TC DL TC LL SPACING TOT.LD. FL/-/4/-/E/R/due to live load and 10.0 20.0 40.0 10.0 24.0" 1.25 0.0 0-0 PSF PSF PSF PSF PSF JRFF-FROM SEQN-DATE REF HC-ENG DRW HCUSR487 07333029 Scale = .25"/Ft. R487--1TCU487 DAL/DF 23137 11/29/07 10 2843 ω Z01

Bot chord 2x8 SP SS :B2 2x4 SP Webs 2x4 SP #3 Calculated horizontal deflection is 0.22" due to live load and 0.30" due to dead load. PLT TYP. Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling. End verticals not exposed to wind pressure. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ ITW Building Components Group, Inc. Haines City, FL 33844

F' 'Teate o' 'Tization' ALPINE Wave 1 6 0 2X4 III 3×6/ R=1348 U=180 W=4" \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BCRESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH IP: OR FLARESKYTHIG, INNOLLING, SHEPPING, HISTALLING A BRACHEG OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DIDS (MATIONAL DESIGN SPEC, BY ATER) AND IP!. THE BCG COMPORNS OF THE ADELOG TO THE STATE AND THE APPLY PARTES TO CAMPETOR PLATES ARE HADE OF TO 120/150/160A, OR HADESTON THE DESIGN SPEC, BY ATER) AND THE DESIGN SPEC, BY ATER AND THE STATE A \*\*HARNING\*\* RRUSE'S REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER 10 BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITUTE, ZIB HORRI LEE SIREET, SUITE JEZ, ALEXANDRIA, VA, 22:14) AND STACK (MODD TRUSS COUNCIL OF AMERIKA, 63:00 CHILERPE'SE LAME, HADISON, HI S3719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS COUNCIL CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. DRAWING INDICATES ACCEPTANCE OF PROFITSSIONAL ENGINEERING RESPONSIBILITY DESCRIPTIONS SHOWN IN SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILD BUILDING DESIGNER PER ANSI/PP 1 SEC. Z. #2 Dense: #2 Dense: 3 X 4 ≡ 3X4 // #9×9 10 -6-6 1.5X4 III Design Crit: 12-6-0 3 X 4 ≡ 13 - 2 - 0TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 24-8-0 Over 3 Supports  $1.5 \times 4 =$ DUSTBILLITY SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE 4 X 8 (R) 1.5X4 W 3-11-3 = ω R-1523 U-180 W-4" \_₩ 0 3-2-6 15 12  $1.5 \times 4 =$ œ 10 1-6-8X10₩ œ €X6= BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from  $6 \cdot 6 \cdot 0$  to  $22 \cdot 4 \cdot 8$ . In lieu of structural panels use purlins to brace all flat TC  $24\ensuremath{\text{m}}$  OC. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf, Iw-1.00 GCpi(+/ )-0.18 Wind reactions based on MWFRS pressures (A) Continuous lateral GOUGH S. FLE SIONAL ENGINEER 8-11-10 8-8-0 bracing equally spaced on member.  $\widehat{\mathbb{A}}$ 7 X 6 (R) 3 X 4 Ⅲ R=1535 U=180 07 3X4 III DUR.FAC. BC DL TC DL ВС TC SPACING TOT.LD. FL/-/4/-8X14(R) **Ⅲ** 20.0 40.0 10.0 1.25 10.0 PSF 24.0" 0.0 0-0 PSF PSF PSF PSF α 6-14 SEQN-DATE REF FROM HC-ENG DRW HCUSR487 07333030 JRFF -Scale R487--1TCU487 Z01 =.25"/Ft. DAL/DF 23148 11/29/07 10 2844

Bot chord 2x8 SP SS
Webs 2x4 SP #3 BC attic room floor loading: LL 6-6-0 to 22-4-8. In lieu of structural panels use purlins to brace all flat TC  $24\,\mbox{^{"}}$  OC. Calculated horizontal deflection is 0.30" due to live load and 0.46" due to dead load. End verticals not exposed to wind pressure. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Haines City, FL 33844
Fr Conficate of Australian Haines City (Training Training Haines City) ALPINE Wave 1-6-0 1.5X4 III 3×6/ R=1248 U=180 W=3.999' \*\* TMPORTANT\*\* TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BCE RESPONSEDLE FOR ANY DEFINATION FROM THIS DESIGN, ANY TAILURE TO BHILD THE BRUSS IN COMPORMANCE WITH FPI: ON FARECHING, MANDLING, SHIPPING, INSTALLING A BRACHING OF IRVISES.

DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY ATAFA) AND IPI. ITH BCG COMMICCIONS PLAILS ARE HADE OF ZO/USE/BCA (M. H/SSEX) ASTH AGAS GRADE GA/GO (M. K/M.SS) GALV. SHELL, APPLY PLAIES TO EACH FACE OF TRUSS, AND. UNLESS OTHERNISE LOCATED ON HIS DESIGN, POSITION FER BRAHMES 160A Z. ANY HISPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER AIMER AS OF THIS DESIGN, POSITION FER BRAHMES 160A Z. ANY HISPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER AIMER AS OF THIS DESIGN. AS SLA, ON THIS DESIGN SHOWN.

DESIGN SHOWN. HIS SHILABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIMST/TPI I SEC. 2. \*\*\*HARNING\*\* RINSSYS REQUIRE CYRRENE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
\*\*ETER 10 BCS1 (BUILDING COMPONENT SAFETY INFORMATION), HANDLING, STIPPI (TRUSS PLANE INSTITUT, 218
\*\*LORD HORRI LEE SIREET, SUITE 312. ALEXANDRIA, VA, Z2314) AND WICA (MODD TRUSS COUNCIL OF ARERICA. 6300
\*\*ETERPRISE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERCORNING THESE FUNCTIONS. UNLESS
\*\*OTHERWISE INDICATED FOR FUNDOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE
\*\*A PROPERLY ATTACHED RIGID CELLING. 4 X 5 ≡ 3X4/ ī 40.00 9×9 psf; Design Crit: 12-11-5  $3 \times 4 =$ -13 - 2 - 0DL 1 1.5X4 III 10.00 psf; from ω 24-8-0 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 0ver 3 Supports - W R-1735 U-180 W-4" 6 X 6 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Collar-tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Wind reactions based on MWFRS pressures 2x6~#3 or better "T" brace. 80% length of web member. Attach h 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC. 7.24.12 SOUGHAS FLE 1 - 8 - 11 No. 66648 8 7x6(R) III 3 X 4 III 29 R=1466 U=180 07 2×4 III DUR.FAC 8 C ВС TC DL  $^{\circ}$ SPACING TOT.LD. FL/-/4/-8X14(R) Ⅲ 믿 Ε  $\widehat{\mathbb{A}}$ 20.0 /E/R/-40.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF 1 0-0 SEQN-DATE REF JRFF-FROM HC-ENG DRW HCUSR487 07333033 Scale = R487 1TCU487 DAL/DF 23175 11/29/07 .25"/Ft. 2845 Z01

BC attic room floor loading: LL = 6-6-0 to 24-8-0. End Bot chord 2x8 SP SS
Webs 2x4 SP #3 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Collar tie braced with continuous lateral bracing at 24" OC. rigid ceiling. Calculated horizontal deflection is 0.33" due to live load and 0.44" due to dead load. PLT TYP. ITW Building Components Group, Inc. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY Haines City, FL 33844

FI Carificate of Amelia Zization 40 and Building designer per ansi/fri sec. 2. verticals not exposed to wind pressure ceiling. ALPINE Wave 1 6 0 1.5X4 III 3×6/ R-1241 U-180 W-4" \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN 10 THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, MAY FAILURE 10 BUILD THE TRUSS IN COMPORNANCE WITH FPI; OR FARRIEATHO. HANGLUIG. SHAPPIO. HISTALLING A BRACING OF TRUSSES.

DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND IPI. THE BCG CONNECTOR PLATES ARE MADE OF ZO/JBJ/16G, 40, H/55/KJ, ASH A653 GRADE 40/16G, POSITION PER DRAWHIGS 16GA 2. PLATES TO EACH FACE OF TRUSS AND. MULESS ON THIS DESIGN, POSITION PER DRAWHIGS 16GA 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF IPI1 Z00Z SEC.3. A SEAL ON THIS \*\*MARNING\*\* TRUSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
RETER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIP (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREET, SUITE 312, ALEXANDRAIA, NA, 22314) AND HICA (MODO TRUSS COUNCEL OF AMERICA. 6300
CHIERRE'SE, LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DNECSS
OTHERRE'SE, LOUICATED 109 CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING. 4 X 5 == 3×4/ Æ 40.00 psf; DL 例 9 X 9 Design Crit: 1.5X4 **Ⅲ** 3 X 4 == 13-4-10 13 - 2 - 0n. 10.00 psf; from TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 24-8-0 9 0ver BUILDING OZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE ယ €X8# Supports ₩ R-1752 U-180 W-4" € X 6 ≡ 110 mpn wind, 15.00 ft mean ngt, ASCE /-02, CLOSED bidg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures <u>în lieu of structural panels use purlins to brace all flat TC</u> (A) Continuous lateral bracing equally spaced on member 7.24.12 USTONAL BROWNER 3-6 (A)7 X 6 (R) Ⅲ 3 X 4 Ⅲ R-1550 U-180 8X14(R) III 07 2X4 🏢 TC LL SPACING DUR.FAC. ВС ВС TC DL TOT.LD. FL/-/4/-PL 8-0-0 /E/R/-20.0 40.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF 9 FROM DATE REF JRFF-SEQN-HC-ENG DRW HCUSR487 07333032 Scale = .25"/Ft. **@** R487--1.TCU487 DAL/DF 23181 11/29/07 2846 201

Bot chord 2x8 SP \$S Webs 2x4 SP #3 Calculated horizontal deflection is 0.30" due to live load and 0.46" due to dead load. End verticals not exposed to wind pressure Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ Collar-tie braced with continuous lateral bracing at 24  $^{\circ}$  OC. or rigid ceiling. TYP. ALPINE Wave 1 6 0 1.5X4 W 5 X 6 (R) / R-1294 U-180 W-3.999' \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG, THG. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PI; OR FARRICATHO, MANDLING. SHEPPING, INSTALLING & BRACHEG OF TRUSSES.

DESIGN CONFIGERS WITH APPLICABLE PROVISIONS OF 3DS (MATIONAL DESIGN SEC, B. WITEN) AND PP!. THE GOOD CONNECTION FOR THE APPLY PLATES TO EACH FACE OF TRUSS AND. UNICES OTHERS IS CHAFFED ON THIS DESIGN, POSITION PRE DRAWHINGS 160A. Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANHER AS OF TPIJ 2002 SEC, B. SEALON THIS BRANDALES ACCUMPONENT FOR THE SUBJECT OF THE SUB \*\*\*\*MARNING\*\* RUSSES BEQUIRE EXTREME CARE IN FARRICATION. IMADILING, SUIPPING, INSTALLING AND BRACING.
REFER TO BEST (BUILDING COMPONUIT SEATET) REFORMATION), PRICEISED BY 191 (FRUSS PAIR INSTITUTE, 218
ROBERT LEE STREET, SUITE 317, ALEXANDRIA, WA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA,
BOOTH LEE STREET, SUITE 317, ALEXANDRIA, WA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA,
BOOTH LEES STREET, SUITE 317, ALEXANDRIA, WA, 22314) AND WICE STREET PRACTICES PRIOR TO PERFORMENT UNICES STORY
OF THE PROPERTY ATTACKS.

OTHERWISE INDICALED OF CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARTE AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER A PROPERLY ATTACHED RIGID CEILING. 3X5≡ 3×4/ 11-4-10 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 3 X 4 ≡ 13-2-0 α 1.5X4 **Ⅲ** -24-8-0 Over 6×6≡ W Supports R-1661 U-180 W-4" €X6≡ BC attic room floor loading: LL = 40.00 psf;  $\vartheta$ L = 10.00 psf; from 6-6-0 to 22-4-8. LIU MPN WING, 15.00 IL MEAN NGL, ASKE /-UZ, KLUSED DIGG, LOCATED ANYWHERE IN ROOF, CAT II, EXP B, WIND TC DL=5.0 psf, WIND BC DL=5.0 psf, WIND BC Wind reactions based on MWFRS pressures In lieu of structural panels use purlins to brace all flat TC @ 24" OC. (A) Continuous lateral bracing equally spaced on member 2  $\Box$ COUCUAS FLE 3-6 SONAL ENGINEE CENS No. 66648  $\Xi$ 7X6(R) Ⅲ 3 X 4 Ⅲ R=1488 U=180 107 8X14(R) Ⅲ 3 X 4 Ⅲ SPACING DUR.FAC BC LL TC LL ВС TC DL TOT.LD. FL/-/4/-Р 40.0 10.0 /E/R/-10.0 20.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF 9 JRFF-REF 9 FROM SEQN DATE HC-ENG DAL/DF DRW HCUSR487 07333031 Scale = .25"/Ft. R487--1TCU487 23190 11/29/07 2847 201

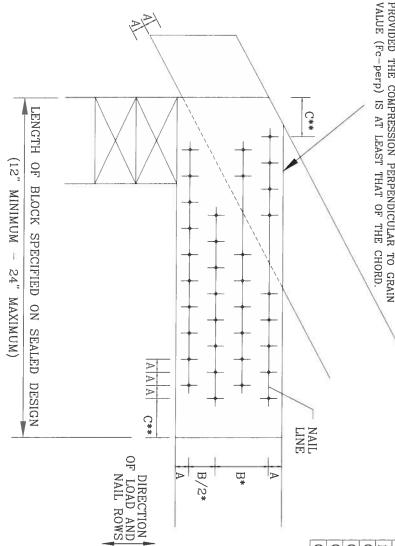
## BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- ROWS OF NAILS (6 NAIL DIAMETERS)
- CBA EDGE DISTANCE AND SPACING BETWEEN STAGGERED SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED
• SPACING MAY BE REDUCED BY 50%
• SPACING MAY BE REDUCED BY 33% BY THE AMOUNTS GIVEN BELOW:

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.



MAXIMUM N
NUMBER OF
NAIL
LINES
PARALLEL TO
TO
GRAIN

		CHC	CHORD SIZE	ZE	
NAIL TYPE	2X4	2X6	2X8	01X2	2X12
8d BOX (0.113"X 2.5", MIN)	ယ	6	9	12	15
10d BOX (0.128"X 3.", MIN)	ω	51	7	10	12
12d BOX (0.128"X 3.25", MIN)	ယ	5	7	10	12
16d BOX (0.135"X 3.5", MIN)	ယ	S	7	10	12
20d BOX (0.148"X 4.", MIN)	ಬ	4	5	6	8
8d COMMON (0.131"X 2.5", MIN)	သ	5	7	10	12
10d COMMON (0.148"X 3.", MIN)	2	4	6	8	10
12d COMMON (0.148"X 3.25", MIN)	∾	4	6	8	10
16d COMMON (0.162"X 3.5", MIN)	N	4	රි	8	10
GUN (0.120"X 2.5", MIN)	ယ	6	8	11	14
GUN (0.131"X 2.5", MIN)	ယ	5	7	10	12
GUN (0.120"X 3.",MIN)	ω	6	8	11	14
GUN (0.131"X 3.", MIN)	ယ	Q	7	10	12

# MINIMUM NAIL SPACING DISTANCES

	ZIU.	E.	DISTANCES		
NAIL TYPE	Α		₩*		C**
8d BOX (0.113"X 2.5",MIN)	3/4"	-	1 3/8"	-	1 3/4"
10d BOX (0.128"X 3.",MIN)	. ,	-	5/8"	i	ญู
12d BOX (0.128"X 3.25", MIN)		-	1 5/8"		ಬ್
16d BOX (0.135"X 3.5",MIN)	7/8"	1	5/8"	$\sim$	1/8"
20d BOX (0.148"X 4.",MIN)	1"	<del></del>	1 7/8"	N	2 1/4"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1	5/8"		ಬ್
10d COMMON (0.148"X 3.",MIN)	<u>-</u> "	⊢	1 7/8"	2	2 1/4"
12d COMMON (0.148"X 3.25", MIN)	1	1	7/8"	S	<b></b>
16d COMMON (0.162"X 3.5", MIN)	1,		2,	2	$\rightarrow$
GUN (0.120"X 2.5", MIN)	3/4"	_	1/2"	1	7/8"
GUN (0.131"X 2.5", MIN)	7/8"	<u>-</u>	1.5/8"		ಬ್ಬ
GUN (0.120"X 3.",MIN)	3/4"	1	1/2"	_	7/8"
GUN (0.131"X 3.",MIN)	7/8" 1 5/8"	<b>⊢</b>	5/8"		12

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

DATE REF

DRWG -ENG

SJP/KAR CNBRGBLK0207 2/23/07 BEARING BLOCK



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WITHDRIVANIES FUNKSH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL  NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS.  ONE DEMANCE UPITH FIJ DE FABRICATION, HANDLING, SHIPPINN, INSTALLING & BRACHING OF TRUSSES.  DESIGN CONTRIES VITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFFAN AND FRI-  STOCHMANCETOR PLATES ARE HADE OF 20/18/19/16/46 VAP/ASSAY) ASTH A633 GRADE 40/60 (VAP/ASSA)  LEVAL MORE THAN A THE REPORT OF THE THE SECTION OF PLATES THE PER THE STALL BE PER  MARKET OF THIS COMPONENT FOR SEC. 3. A SEAL ON THIS DRAWING MOIL ARES AND CONTROL THE SUBJECTION OF THE COMPONENT FER THE SECTION OF THE COMPONENT FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSISTENT SECTOR.
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ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

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"WAVARNINGS" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST GUILDING COMPONENT SAFETY IN KITOPRAMITONS, PUBLISHED BY TRI CIRUSS PLATE INSTITUTE, 218 MORTH LEE STR., SUITE 312 ALEXANDRIA, VA. 22314) AND VTCA VOCOD TRUSS COUNCIL PARKICA, 6300 ENTERPRISE LN, HANDSON, WI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICTION, TO PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL O'IONAL ENGLISH

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" O.C.
MAXIMUM SPACING. ATTACH TO EACH TOP CHORD WITH

(2) 16d COMMON (0.162"X 3.5",MIN) NAILS.
BRACING MATERIAL TO BE SUPPLIED AND ATTACHED
AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.

++ 2X4 SO. PINE #2 N OR SPF #1/#2 FILLER TOP CHORD.

+++ 2X4 SO. PINE #3 OR SPF #1/#2 VERTICAL WEBS SPACED 48" OC MAXIMUM.

\* 8/12 MAXIMUM PITCH.

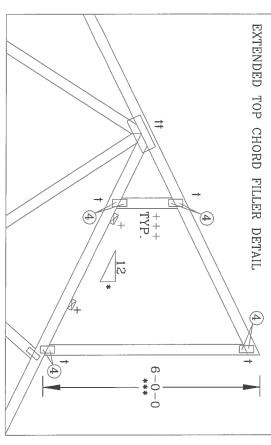
\*\* 2X8.25 PIGGYBACK SPECIAL PLATE. SEE DRAWING PIGBACKB0699 FOR PIGGYBACK SPECIAL PLATE INFORMATION.

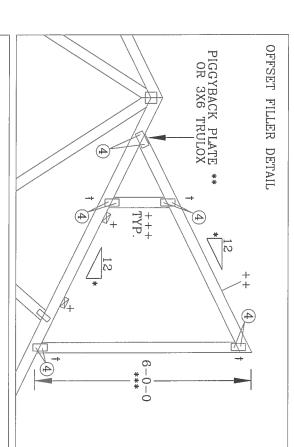
\*\*\* 6'0" MAXIMUM HEIGHT

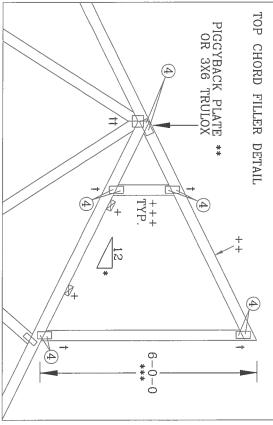
† W2X4 OR 3X6 TRULOX.

th REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

0.120"X 1.375" NAILS REQUIRED
FOR TRULOX PLATE ATTACHMENT. NAILS SPECIFIED
IN CIRCLES MUST BE APPLIED TO EACH FACE OF EACH TRUSS PLY.
SEE DWG. 160TL FOR NAILING AND TRULOX PLATE REQUIREMENTS







RACING. REFER TO REST (BUILDING COMPONENT SERTY) MEDBANTING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO REST (BUILDING COMPONENT SERTY) MEDBANTING, DUBLISHED BY PIG ITRISES PLAID RISTITUTE, 219 MORTH LEE STR. SUITE 312, ALEXANDRIA, VA. 22344 AND VICA COUDD TRUSS COUNCIL OF APERICA, 6300 ENTERPRISE LA, HANDLING, WISTATISH PER SHALL HAVE PRIDERLY ATTACHD STRUCTURAL FUNCTIONS. UNLESS DIMERVISE INDICATED, TOP CHIRD SHALL HAVE PRIDERLY ATTACHD STRUCTURAL PANELS AND BOITON CHIRACTER, INC., SHALL PANELS AND BOITON CHIRACTER TO PERFORMING THESE PANELS AND THE TRUSS IN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING OF TRUSSES. DESIGN CONFIDENCE VITH 761, OF ABRICATING, HANDLING, SHIPPING, INSTALLING TO BRACING ON HANDLING, SHIPPING, INSTALLING TO THE SUITABILITY AND UNITED AND THE TRUSS AND UNITED ASSOCIATION OF THE SUITABILITY AND UNITED ABOUNTS TOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, FER ANSI/TPI 1 SEC. 2.

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE



	TT D.I.	MAX	30	MAX 30 PSF REF	REF	TC-FILLER
	TC DL	MAX	15	PSF	MAX 15 PSF DATE	2/23/07
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V SER	SPACING	2	24.0"	2		
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THIS DRAWING REPLACES DRAWING 884,080

## BOTTOM CHORD FILLER DETAIL

SIZES (1X3 WAVE) MAY BE USED IF BEARING IS OMITTED. WEDGE OPTIONAL INTERIOR OR CANTILEVER BEARING. MINIMUM PLATE OR VERTICAL MEMBER MUST COINCIDE WITH BEARING LOCATION.

0.120" X 1.375", NAILS, REQUIRED FOR NAILING AND TRULOX PLATE REQUIREMENTS NAILS SPECIFIED IN CIRCLES MUST BE APPLIED FOR TRULOX PLATE ATTACHMENT. TO EACH FACE OF THE TRUSS. SEE DWG. 160TL

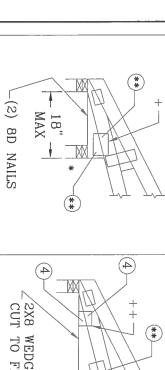
3X4 WAVE OR 4X8 TRULOX

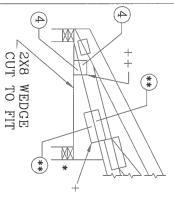
+ 2X4 WAVE OR 3X6 TRULOX

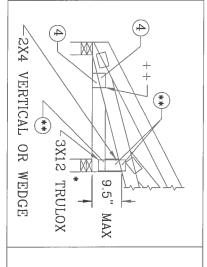
SHOWN DETAIL REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS FOR LUMBER, PLATES, AND OTHER INFORMATION NOT

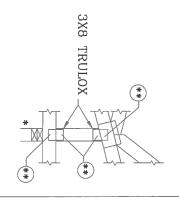
ALL TRULOX PLATES SHOWN ARE MINIMUMS. LARGER PLATES MAY BE REQUIRED TO ACCOMODATE REQUIRED NAILS (\*\*)

FILLER BOTTOM CHORD	MAXIMUM REACTION	EACTION	MINIMIM	** REQUIRED NA	D NAILS PE	R FACE WITH	ILS PER FACE WITH TRULOX PLATES	LATES
OR WEDGE SPECIES	DOWNWARD	UPLIFT	EΑ	1.00 D.O.L. 1.15		1.25 D.O.L.	1.33 D.O.L.	1.60 D.O.L.
DOUGLAS FIR-LARCH	3281#	1656#	1.5" X 3.5"	120		10		œ .
HEM-FIR	2126#	1095#	1.5" X 3.5"	9	8	7	7	ග
SPRUCE-PINE-FIR	2231#	1192#	1.5" X 3.5"	10	9	8	8	6
SOUTHERN PINE DENSE	3465#	1791#	1.5" X 3.5"	12	11	10	9	8
SOUTHERN PINE	2966#	1492#	1.5" X 3.5"	10	9	8	8	7
SOUTHERN PINE NON-DENSE	2520#	1343#	1.5" X 3.5"	9	8	7	7	ဘ









THIS DRAWING REPLACES DRAWINGS A115 A115/R & 884,132



\*\*\*WARNING\*\*\* "RUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACKING. REFER TO BEST GUULDING COMPONENT SAFETY INFORMATION, PUBLISHED BY PIT CIRKISS PLATINGTIVET. 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VTCA VOCODO TRUSS COUNCIL. AMERICA, 6300 ENTERPRISE LN, HANDSON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THE FUNCTIONS. UNLESS OTHER VIEW IN 1000 SHALL HAVE PRODERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

WHIPERFANIM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONFRACTOR ITY BCG. INC., SHALL CONFIDENCIAN, ANY FAILURG TO BUILD THE FRISS IN CONFIDENCIAL THE FIRE ANY EVALUATION CONFIDENCIAL TO BUILD THE FRISS IN FAILURG SEARCH OF THE STATE AND THE SECOND ANY FAILURG TO BUILD THE FRISS IN FAILURG SECOND THE SECOND THE SECOND THE SECOND THE SECOND SECOND THE SECOND THE SECOND SECOND THE SECOND SECOND THE SECOND THE SECOND SECOND SECOND THE SECOND THE SECOND SECOND SECOND THE SECOND THE SECOND SECOND SECOND THE SECOND SECOND SECOND SECOND THE SECOND SECOND

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SPACING 24.0"	DUR. FAC. 1.0/1.15/1.25/1.33	TOT. LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.15/1.29			10.0		
	5/1.33	PSF	PSF	PSF	PSF	PSF REF
			-ENG	10.0 PSF DRWG	DATE	REF
			-ENG DLJ/KAR	BCFILLER0207	2/23/07	BC FILLER

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

#### NOTES:

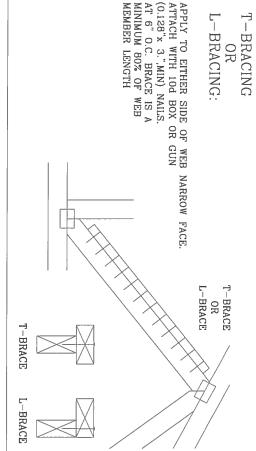
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

1-2X8 2-2X6(*)	2X6	1 ROW 2 ROWS	2X8
1-2X6	2X4	1 ROW	2X6
2-2X4(*	2X6	2 ROWS	2X6
1-2X4	2X4	1 ROW	2X3 OR 2X4
2-2X4	2X6	2 ROWS	2X3 OR 2X4
E BRACING	ALTERNATIVE BRACING T OR L-BRACE SCAB BR	SPECIFIED CLB	WEB MEMBER
SCAB BRACE		BRACING	SIZE

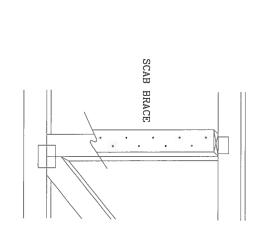
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

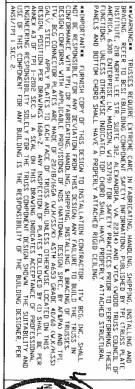


## SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM BOZ OF WEB MEMBER LENGTH

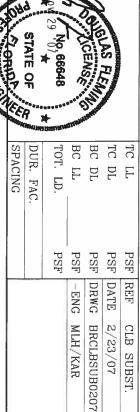


THIS DRAWING REPLACES DRAWING 579,640



ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE



														_	_	_			_	_									
DIAGONAL VERTICAL DOUBLED		1	M	A	X		(	<u> </u>	\ ]	3.	[_]	E		V	E	R	Τ.	ľ	C.	A	L		L	Е	N	1	чг Л.	Ή.	
DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL		1	2	"	(	0	.(	7,			1	6	"		0	.(	ζ.			2	4	,,		0	. (	3	•	SPACING	GABLE
OPTION: MAY BE AGONAL				<u>V.</u>	) j	TIT	I I	U I O	C J J		<u> </u>	j j	<u>V.</u>	)	TIT		777						) J	TIT	I	O'T'	777	SPECIES	2X4 GABLE VERTICAL
Ga	STANDARD	STUD	£#	#2	1#	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
GABLE TRUSS	4' 11"	5' 0"	5' 0"	5, 3,	5' 4"	4' 9"	4' 9"	4' 9"	4' 11"	4.	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4, 5,	3' 10"	4' 0"	4' 0"	4' 2"	4, 3,	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	NO .
	7' 5"	8' 5"	8' 5"	8 5	8 5"	7' 3"	8, 5,	8, 5,	8 5	6, 5,	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5' 3"	6' 1"	6.	6' 8"	6, 8,	5' 2"	6' 0"	6'0"	6' 8"	GROUP A	(1) 1X4 "L"
	7' 5"	8' 7"	8, 5,	9' 1"	9′ 1″	7' 3"	8' 5"	1	B' B"	6' 5"	7' 6"	7' 7"	8' 3"	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5' 3"	6' 1"	6, 2,	7' 2"	7' 2"	5' 2"	6' 0"	6' 0"	6' 10"	GROUP B	BRACE .
	9′ 10″	1	10' 0"	10' 0"	10' 0"	9' 7"	10'0"	10'0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6'11"	7'11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
ABOUT &	9' 10"	- 1	10' 6"	10' 9"	10′9″	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6'11"	8, 0,,	- 1	٦	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	BRACE *
* ¬/ ⊕	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	- 1		1	- 1	10' 10"	10' 10"				-1	9 5"	9' 1"	9,	9, 5,	9' 5"	GROUP A	(2) 2X4 "L"
<i>N</i> №	12' 3"	-1		12' 10"	٦	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	- 1	-	- 1		10' 10"	-1	~	- 1	_ I	- 1	10' 2"	9' 1"	9 5"	- 1	9' 8"	GROUP B	BRACE **
BETTER	14' 0"	٦,	14' 0"	14' 0"	- 1	14' 0"	14' 0"	14' 0"	14' 0"	- 1	- 1	14' 0"	- 1	-	- 1	- 1	- 1				- 1	- 1	12' 5"	-1		12' 4"	12, 5,	GROUP A	(1) 2X6 "L"
	-1	14' 0"	-1	-1	- 1	- "	- 1	14' 0"	٦,	13′ 3″	- 1	- 1	14' 0"	- 1	- L	- 1	14′0″	- F	10′ 10″									GROUP B	BRACE *
		14' 0"																- 1	- 1	14' 0"		14' 0"				14' 0"		GROUP A	(2) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **

DOUGLAS FIR-LARCH
#3
STUD

SOUTHERN PINE #3 STUD

STANDARD

STANDARD

GROUP HEM-FIR #1 & BTR

В

#1 / #2 STANDARD
#3 STUD

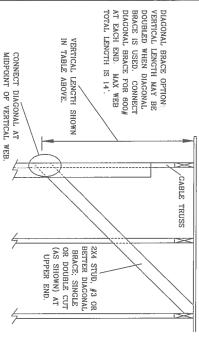
BRACING GROUP SPECIES AND GRADES:

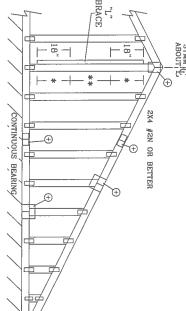
GROUP

A

HEM-FIR STUD

STANDARD





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG. CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

ATTACH EACH "L" BRACE WITH 10d NAILS.

FOR (1) "L" BRACE: SPACE NAILS AT 2 O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+			I		
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT	LESS THAN 4' 0"	VΕ	GABLE VERTICAL PLATE SIZES
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ESI	Ŋ	N	4	0	S
S G	2.5X4	2X4	×	Jds	ZE
FC			IX4 OR 2X3	NO SPLICE	Ω
≅				Ш	

BRACHUG. REFER TO BESS (GUILLIUNG CHAPEE) IN FABRICATING, HANDI, MG, SHIPPING, INSTALLING AND BRACHUG. REFER TO BESS (GUILLIUNG CHAPED IN FACETY) INCOMENTUM, PUBLISHED BY FIG CRUINS PLAIF INSTITUTE. 218 NURHH LEE STR. SUITE 312. ALEXANDRIA, VA. 22314 AND VICA CAUDD TRUSS COLNEIL OF AMERICA. 62300 ENLERPRISE IN, HADISON, WI 53793 PER SAFETY PRACTICES PRIDE TO FERTCHMING THESE TUNCTIONS. UNLESS COTHERVISE INDICATED, TOP CHORD SHALL HAVE A PROPERTY PRACTICES PRIDE TO FERTCHMING THESE TUNCTIONS. UNLESS COTHERVISE IN ADDISON, WI 53793 PER SAFETY PRACTICES PRIDE TO FERTCHMING THESE TUNCTIONS. TO HAVE A PROPERTY ATTACHED RIGHD CELLING.  ***IMPORTANT**** FURNISH COPY OF THIS DESIGN IN INSTALLATION CONTRACTOR. ITV BGC, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTRACTOR. ITV BGC, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTRACTOR PLATES ARE HADE OF 20/18/166A (V.H.Y.S.S.YA) ASST M A653 GRADE 40/60 (V.H.Y.S.S.YA). BESTIGN STREE, BY A FEAR AND THE TRUSS AND, UNLESS CHILEVED BY (D) SHALL BE PROPERSIONAL BESTIGN STREET, ELECTATED IN THIS DESIGN ON THE STATES TO EACH FACE OF TRUSS AND, UNLESS CHILEVED BY (D) SHALL BE PROPERSIONAL BESTIGN STREET LICATED IN THIS DESIGN STOWN. THE SUITABLITY AND LISE OF THIS CORPONENT FOR ANY BUILDING SIGNES, PER MASI/TPI I SEC. 2. A SEAL IN THIS SIGNAL STREAMS STOLD THE SUITABLITY AND LISE OF THE CORPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER MASI/TPI I SEC. 2.	BRACHAGE FRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACHAGE FOR SELECTIVE METROMATION, PUBLISHED BY TO CRISIS PLATE MISTITUTE, 280 MIRTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314-XMD WITCA (VOID TRUSS COLNICIL OF AMERICA, GOOD ENTERPRISE L. U., MADISON, V.) 537193 FOR SAFETY PRACTIES FROM TO PERFORMING THESE FUNCTIONS. UMEESS CHERVISE INDICATED, TIP CHIRD SHALL HAVE PRACTIES FROM TO PERFORMING THESE FUNCTIONS. UMEESS CHERVISE INDICATED, TIP CHIRD SHALL HAVE FORDERLY ATTACHD CELLING.  ***********************************
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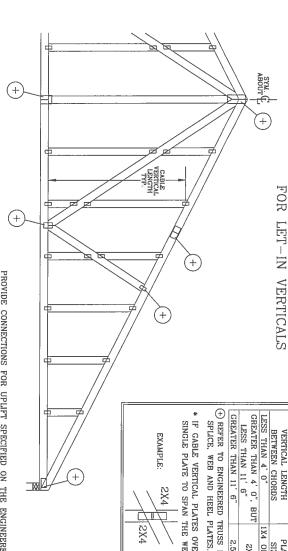
ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

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C.Co. Second	G ORION	STATE OF	₩v 29 '07★	7 / No. 66648	OF LICENSE TO	ISUAS FLEX	
	MAX.		MAX.				
1000000	MAX. SPACING		MAX. TOT. LD				
	Ω		1.9				

TOT. LD. 60 PSF				
SF	-ENG	DRWG	DATE	REF
		A11015EE0207	2/23/07	ASCE7-02-GAB11015

24.0"



VERTICAL LENGTH GABLE VERTICAL PLATE SIZES SIZE 1X4 OR 2X3 2.5X4 PLATE 2X4 IF PLATES OVERLAP\* 2.5XB **2XB** 2XB

GABLE

DETAIL

(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

\* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB



ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

HAND DRIVEN NAILS:

10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS (4) 18d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD. 8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

"T" REINFORCING-MEMBER

TOENAILS

RIGID SHEATHING

GABLE TRUSS

TOENAILS SPACED AT 4 O.C.

ASCE 7-93 GABLE DETAIL DRAWINGS

ASCE 7-98 GABLE DETAIL DRAWINGS A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207 A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207

ASCE 7-02 GABLE DETAIL DRAWINGS A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207 A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207

ASCE 7-05 GABLE DETAIL DRAWINGS A13015EE02207, A12015EE02207, A11015EE02207, A10015EE02207, A08515EE02207, A13030EE02207, A12030EE02207, A11030EE02207, A1030EE02207, A12030EE02207, A13030EE02207, A13030EE0207, A13030EE0207, A13030EE0207, A13000EE0207, A13000EE020

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207 A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207

4 TOENAILS

CEILING

\*\*AVARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HADDING, SHIPPING, INSTALLING AND BRACKING. REFER TO BESS (BUILDING CURPERTY INFORMATION), PUBLISHED BY TEL CIRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VTCA CVOIDD TISUS COUNCIL MARKICA, 6300 ENTERPRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIGR TO PERCORNING THESE FUNCTIONS. UNLESS DITHAVISE NIDICATED. TOP CHOOD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANGLS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL

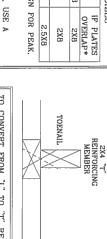
VERTICAL LENGTH.

WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE

WHIPERFANITH FUNNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BCG, INC., SHALL NOT BE RESENSIBLE CIPE AND EXCHAIGUS FOR HIS DESIGN, ANY FAILURE ID BUILD THE RISS. NI CONTRIBANCE. VITH 171 DR FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF RISSESS. DESIGN CONTRIBET OF REPLICABLE FRONTY OF THIS NAME AND FOLL SIDE AND THE SHORT OF THE SHORT OF THIS SHORT OF THE STORM SHORT OF THE SHORT OF

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE



TOENAIL

ZAO 1 REINFORCING MEMBER

SBCCI WIND LOAD. 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR VERTICAL SPECIES, GRADE AND SPACING) FOR (1)

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

			_				_					_	_		_						_
30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH		WIND SPEED
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE	"T" REINE												
10 %	10 %	0 %	0 %	20 %	20 %	2, 01	2 01	30 %	10 %	20 %	20 %	40 %	2 01	30 %	2 01	50 %	2 01	40 %	2 01	SBCCI	
30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	2 01	40 %	2 01	40 %	2 01	50 %	2 01	50 %	2 01	2 09	10 %	ASCE	

GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) ZX4 "L" BRACE LENGTH = 6' 7" MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH MEAN ROOF HEIGHT = 30 FT ASCE WIND SPEED = 100 MPH  $1.10 \times 6' 7'' = 7' 3''$ 

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

N.	d	N.				
2 ORION	STATE OF	N W 29 '07 *	No occaso	CENS	SAS FEE	
MAX SPACING 24.0"	STATE OF DUR. FAC. ANY	MAX TOT. LD. 60 PSF				
			-ENG	DRWG	DATE	REF
			-ENG DLJ/KAR	DRWG GBLLETIN0207	DATE 2/23/07	LET-IN VERT

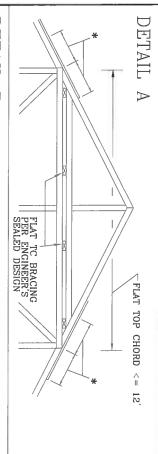
# PIGGYBACK DETAIL

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

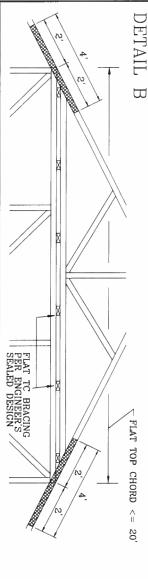
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS. TRUSSES MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

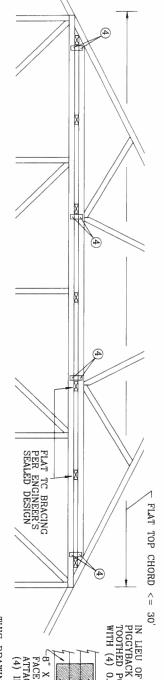
\* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.



DETAIL

CAP TRUSS TOENAILED TO TOP CHORD CIRCLED NUMBER INDICATES REQUIRED

PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.



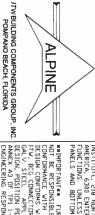
BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.

IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

(4) Bd COMMON NAILS (0.131"X2.5")

CB" X B" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) Bd COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860



\*\*MYARMING\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST GUILLIDING COMPODENT SAFETY INDEMANTIDIN, PUBLISHED BY FI CTRUSS PLATE INSTITUTE, 218 NIGHTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VITA CYCODD TRUSS COUNCIL DE MARRICA, 6300 ENTERPRISE LN, HANISDN, VI 35759; FDR SAFETY PRACTICES PRIGR TO PERFORMING THESE FUNCTIONS. UNICESS DIMENTALE INDICATION, TOP CORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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	TC LL	PSF REF	REF	PIGGYBACK
E	TC DL	PSF	PSF DATE	2/23/07
Single	BC DL	PSF	DRWG	DRWG PIGBACKA0207
NAZILA NAZILA	BC LL	PSF	-ENG	-ENG DLJ/KAR
#	TOT. LD. MAX 60 PSF	30 PSF		
OF CR	DUR. FAC. 1.15	15		
OFFICE	SPACING 24	24.0"		

## PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

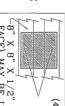
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTON CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



(4) 6d BOX (0.099"X 2.", MIN) NAILS.

 $\leftarrow$ 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES. ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET. (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC

TYPE H U C Щ A 4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY 4X6 5X4 .5X3 2X4 30 1.5X4 2.5X4 SPANS UP 5X5 5X6 34 2.5X4 1.5X4 5X5 5X6 38 To 1.5X4 5X6 3X5 52

OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION. ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS

	C	C	海 C	∫œ. C	C		ra C	C					
•	0	$\frac{1}{\circ}$	(	o°	APPLY PIGGYBACK AND SPACE 4' OC	ATTACH TEET FABRICATION.		10' TO 14'		7'9" TO 10'	0' TO 7'9"	WEB LENGTH	
	0	)	0	•	SCA	H TO T	* PIGGYB	2x4 T BRA MEMBER, OR MEMBER. A' (0.135"X 3.5	'n	1x4 "T" BRACE. MEMBER, OR BI	NO BRACING		WEB B
3	0	0	0	0	SPECIAL PLATE TO EACH TRUSS FACE OR LESS.	HE PIGGYBACK AT THE TIME OF CH TO SUPPORTING TRUSS WITH	* PIGGYBACK SPECIAL PLATE	BRACE. SAME GRADE, OR BETTER, AND 80% ATTACH WITH 164 BC 3.5",MIN) NAILS AT 4"	2.5",MIN) NAILS AT 4"	RACE. SAME GRADE, S OR BETTER, AND 80% ATTACH WITH 84 ROX		REQUIRE	BRACING CHART
	0	0	0	0	TO EACH T	AT THE TI	AL PLATE			~		REQUIRED BRACING	RT
		0		0	RUSS FACE	ME OF WITH		ECIES AS	. 1	SPECIES AS LENGTH OF		•	
	-	ľ	ų.	-				WEB		WEB			

NO 86648 CENS 1.33 DUR. FAC. .25 DUR. FAC MAX LOADING 47 PSF AT 50 PSF AT 55 PSF AT DRWG DATE PIGBACKB0207 DLJ/KAR **PIGGYBACK** 2/23/07

THIS DRAWING

REPLACES DRAWINGS

634,016 634,017 & 847,045

EITHER PLATE LOCATION IS ACCEPTABLE MAX \ 20 \*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE 20 FLAT TOP H TYP. В 公姓 SPLICE CHORD MAX SPAN 40 和 MAX SIZE OF 2X12 #2 OR BETTER ₩ >C-TYP 4 æ D-SPLICE



\*\*AVARNING\*\* "PUISSES REQUIPE EXTREME CARE IN FABRICATING, HANDLING, SMEPPING, INSTALLING AND BRACING. REFER TO BESS (BUILDING COMPENENT SAFETY INFORMATION), PUBLISHED BY TRI CTRUSS PLATE INSTITUTE, 218 MORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND "VICA (VOUDD TRUSS COUNCIL CAMERICA, 6300 ENTERPRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO ERETORNING THESE FUNCTIONS. UNLESS OTHERVISE INDICATED, 10P CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL

WHEREKANTH FURNISH COPY OF THIS DESIGN TO INSTALLATION CONFRACTOR TITY BCG, INC., SMALL
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### ASCE 170-1 OTT MTH WIND SPEED, 70 MEAN HEIGHT, ENCLOSED, ||1.00, EXPOSURE C

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#3 STUD

STANDARD

BRACING GROUP SPECIES

AND GRADES:

GROUP

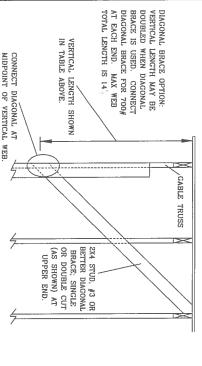
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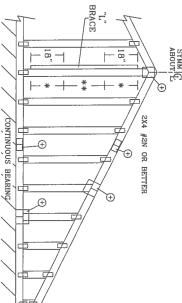
DOUGLAS FIR-LARCH

SOUTHERN PINE #3 STUD STANDARD

STUD STANDARD

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	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 7"	4' 9"	4' 9"	4' 11"	5 1"	4' 6"	4' 6"	4' 6"	4' 7"	4' 2"	4' 4"	4' 4"	4' 6"	4' 7"	4' 1"	4' 1"	4' 1"	4 2"	3' 8"	3' 9"	3' 9"	3' 11"	4' 0"	3' 7"	3' 7"	3' 7"	3' 8"	BRACES	S
	6' 9"	7' 9"	7' 11"	8' 0"	1 -	"	7' 8"	7' 8"	8' 0"	5' 10"	6' 9"	6' 10"	٠.	7' 3"	5' 8"		6' 8"	7: 3"	4' 9"	5' 6"	5' 7"	6' 4"	6' 4"	4' 8"	5' 5"	5' 5"	6' 4"	GROUP A	(1) 1X4 "L"
	6' 9"	7' 9"	7'11"	8' 7"	8' 7"	6' 7"	7' 8"	7' 8"	8' 2"	5' 10"	6'9"	6' 10"	7'9"	7' 9"	5' 8"	8'0"		7' 5"	4' 9"	5' 6"	5' 7"	6' 10"	6' 10"	4' 8"	5' 5"	5' 5"	6' 6"	GROUP B	BRACE *
	8' 10"	9' 5"	9' 5"	9' 5"	9' 5"	8' 8"	9' 5"	9'5"	9' 5"	7' 8"	8' 7"	8' 7"	8' 7"	8' 7"	7' 6"	8' 7"	8' 7"	8' 7"	6° 3″	7' 3"	7' 4"	7' 6"	7' 6"	6 1"	7' 1"	7' 2"	7' 6"	GROUP A	(1) 2X4 "L"
DI MMAS	8' 10"	9' 11"	9' 11"	10' 2"	10' 2"	8' 8"	9' 5"	9'5"	9' 8"	7' 8"	8' 11"	9' 0"		9' 3"	7' 6"	8' 7"	8' 7"	8' 10"	6' 3"	7' 3"	7' 4"	8' 1"	8' 1"	6' 1"	7' 1"	7' 2"	7' 8"	GROUP B	BRACE *
	11' 3"	11' 3"	11' 3"	11' 3"	11′ 3″	11' 3"	11' 3"	11' 3"	11, 3,	10' 3"	10' 3"	10' 3"	10' 3"	10' 3"	10' 1"	10' 3"	10′ 3″	10' 3"	8' 5"	8' 11"	8' 11"	8' 11"	8' 11"	8' 3"	8' 11"	8' 11"	8' 11"	GROUP A	(2) 2X4 "L"
	11' 7"	11' 10"	11' 10"	12' 1"	12' 1"	11' 3"	11' 3"	11' 3"	11' 7"	10' 4"	10' 9"	10' 9"	11' 0"	11' 0"	10' 1"	10′ 3″	10' 3"	10' 6"	8' 5"	9'5"	9. 5."	9' 7"	9' 7"	-	8' 11"	8' 11"	9' 2"	GROUP B	BRACE **
	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	13 5"	13' 5"	13' 5"	13' 5"	11' 8"		13' 5"	13' 5"	9' 9"	11' 4"	11' 5"	11' 9"	11' 9"	9' 6"	11' 1"	11' 2"	11' 9"	GROUP A	(1) 2X6 "L"
	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	1	14' 0"	14' 0"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	11' 8"		13' 5"	13' 10"	9' 9"	11' 4"	. !	12' 8"	1	9' 6"	11' 1"	11' 2"	12' 1"	GROUP B	BRACE *
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	- 1	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14' 0"	14' 0"	14′0″	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	- "	- 1	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH

GABLE
TRUSS
DETAIL
NOTES

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

HEM-FIR #1 & BTR #1 GROUP

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#2

GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG. CONTINUOUS BEARING (5 PSF TC DEAD LOAD). OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH

VERTICAL LENGTH IN SPLICE  VERTICAL LENGTH IN SPLICE  ESS THAN 4 0" IX4 OR 2X3  IREATER THAN 41 0", BUT 2X4  LESS THAN 11 6" 2X4  IREATER THAN 111 6" 2.5X4  IREATER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.
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WHITER FAMILY FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR THY BCG, INC., SMAIL

NOT BE RESENDANCE VITH 171 OR FABRICATING HEM THIS DESIGN, ANY FAILURE OR BALLO THE TRUSS IN

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ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

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	MAX.	MAX. TOT. LD. 60 PSF	LD.	60	PSF		
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THE	MAX.	MAX. SPACING 24.0"	ING	2,5	1.0"		