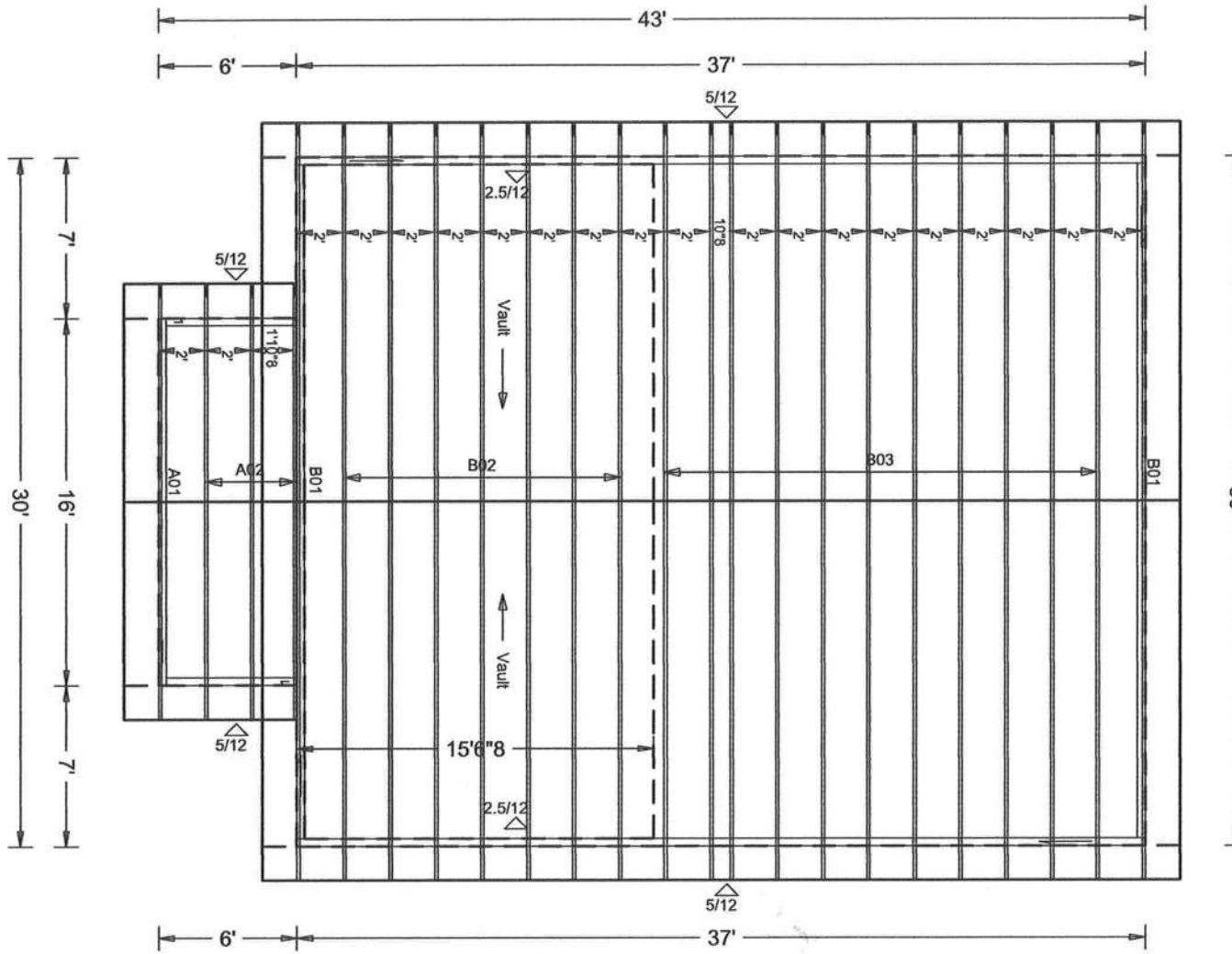


44413



W.B. Howland Truss Co.
610 11th St. SW
Live Oak, FL 32064
(386) 362-1235
(386) 362-7124 (Fax)
howlandtruss@gmail.com

ROOF PITCH: 5/12
OVERHANG: 18" (Verify)
CEILING: 8' w/Vault
EXT. WALLS: 4"
LOADING: 40psf
WIND LOAD: 130mph
EXPOSURE: C

DATE: 4/25/22



JOB NAME: Bruce Park Customer: K&H FRAMING Designer: Kelly Caudill ADDRESS: SALESMAN: WH <Not Found>	JOB #: 22-7472	
JOB NO: 22-7472	PAGE NO: 1 OF 1	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpininst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



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Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999

04/25/2022

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-7472
Job Description: Bruce Park	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 20.01.01A JRef #: 1Xf02150008
Wind Standard: ASCE 7-16	Wind Speed (mph): 130
Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 5 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	115.22.1403.48248	A01
3	115.22.1403.48293	B01
5	115.22.1403.48326	B03
7	GBLLETIN0118	

Item	Drawing Number	Truss
2	115.22.1403.48325	A02
4	115.22.1403.48371	B02
6	A14015ENC160118	





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04/25/2022

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-7472
Job Description: Bruce Park	
Address: FL	

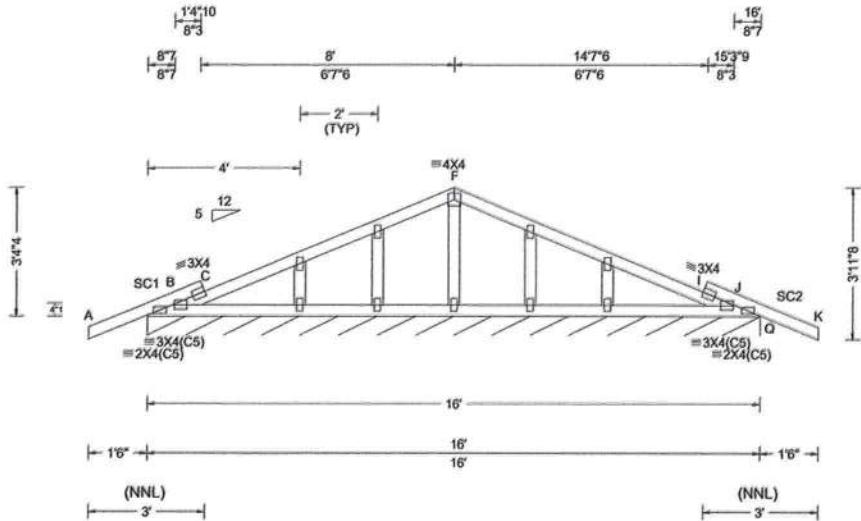
Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 20.01.01A JRef #: 1Xf02150008
Wind Standard: ASCE 7-16	Wind Speed (mph): 130
Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 5 truss drawing(s) and 2 detail(s).

Item	Drawing Number	Truss
1	115.22.1403.48248	A01
3	115.22.1403.48293	B01
5	115.22.1403.48326	B03
7	GBLLETIN0118	

Item	Drawing Number	Truss
2	115.22.1403.48325	A02
4	115.22.1403.48371	B02
6	A14015ENC160118	

SEQN: 614219 / FROM: CDM	GABL Ply: 1 Qty: 1	Job Number: 22-7472 Bruce Park Truss Label: A01	Cust: R 215 JRef: 1X102150008 T2 / DrwNo: 115.22.1403.48248 / YK 04/25/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF					
TCLL:	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity			
TCDL: 20.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.003 C 999 240	Loc R+ /R- /Rh /Rw				/U /RL	
BCLL: 10.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.006 C 999 180	Q* 94 /- /- /47 /- /2				Wind reactions based on MWFRS	
BCDL: 0.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 C - -	Q Brg Width = 192 Min Req = -				Bearing B is a rigid surface.	
BCDL: 10.00	EXP: C Kz: NA		HORZ(TL): 0.002 C - -	Members not listed have forces less than 375#					
Des Ld: 40.00	Mean Height: 15.00 ft		Creep Factor: 2.0						
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Max TC CSI: 0.230						
Soffit: 2.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max BC CSI: 0.079						
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max Web CSI: 0.035						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes							
	Loc. from endwall: Any	FT/RT:20(0)/10(0)							
	GCpi: 0.18	Plate Type(s):							
	Wind Duration: 1.60	WAVE		VIEW Ver: 20.01.01A.0724.11					

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 3-4-4.



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

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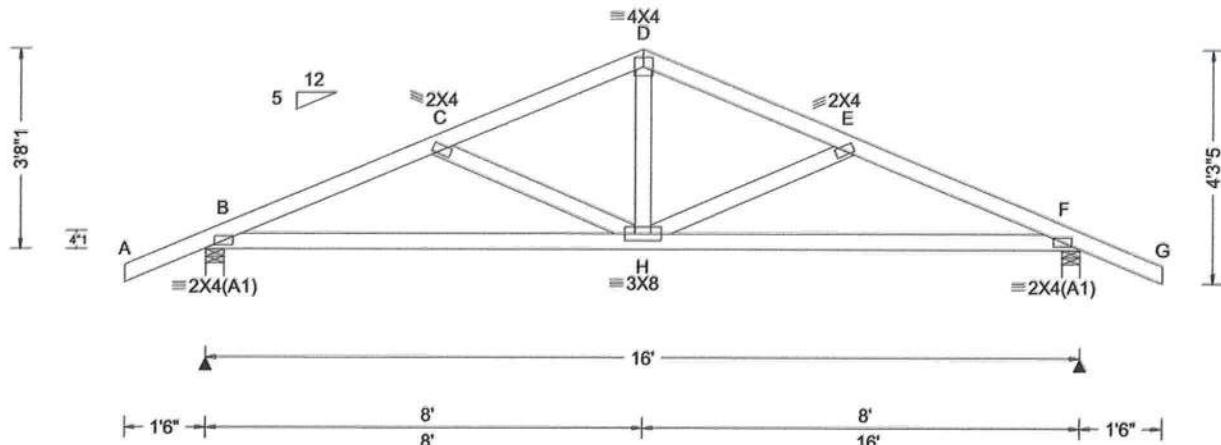
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 614218 / FROM: CDM	COMM Ply: 1 Qty: 3	Job Number: 22-7472 Bruce Park Truss Label: A02	Cust: R 215 JRef: 1Xf02150008 T1 / DrwNo: 115.22.1403.48325 / YK 04/25/2022
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4'3"14 8' 11'8"2 16' 4'3"14

4'3"14 3'8"2 3'8"2 4'3"14



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL:	Wind Std: ASCE 7-16	Pg: NA Cl: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 20.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.033 H 999 240	B	752	/-	/-	/441	/141	/104
TCDL: 10.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.066 H 999 180	F	752	/-	/-	/441	/141	/-
BCLL: 0.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.013 H - -	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: C Kz: NA	Building Code: FBC 7th Ed. 2020 Res.	HORZ(CL): 0.025 H - -	B	Brg Width = 4.0					Min Req = 1.5
Des Ld: 40.00	Mean Height: 15.00 ft	TPI Std: 2014	Creep Factor: 2.0	F	Brg Width = 4.0					Min Req = 1.5
NCBCLL: 10.00	TCDL: 5.0 psf	Rep Fac: Yes	Max TC CSI: 0.198	Bearings B & F are a rigid surface.						
Soffit: 2.00	BCDL: 5.0 psf	FT/RT: 20(0)/10(0)	Max BC CSI: 0.642	Members not listed have forces less than 375#						
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	Plate Type(s):	Max Web CSI: 0.172	Maximum Top Chord Forces Per Ply (lbs)						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	WAVE	VIEW Ver: 20.01.01A.0724.11	Chords	Tens.Comp.	Chords	Tens. Comp.			
	Loc. from endwall: Any			B - C	656 - 1153	D - E	490 - 877			
	GCpi: 0.18			C - D	490 - 877	E - F	656 - 1153			
	Wind Duration: 1.60									

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3'-8-1".



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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

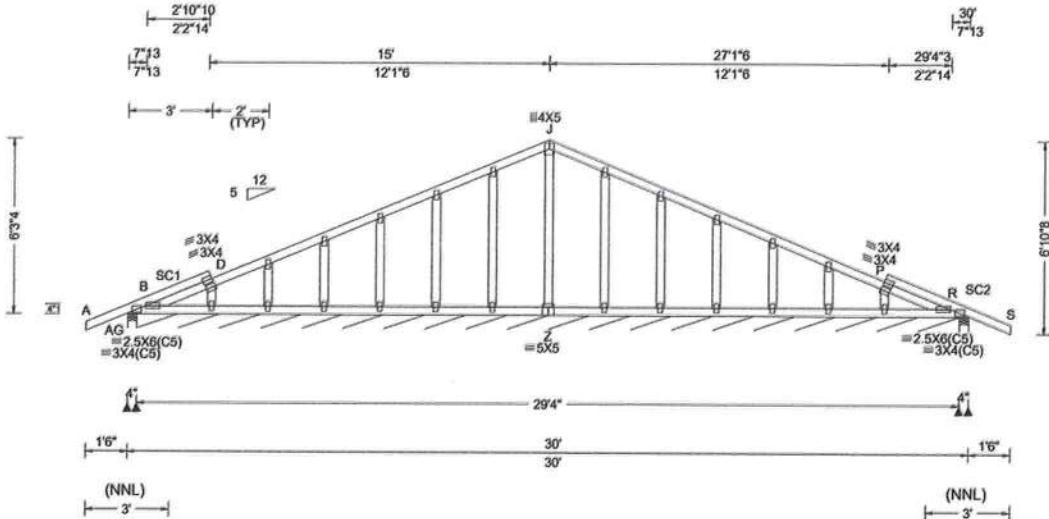
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

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SEQN: 614217 / FROM: CDM	GABL Ply: 1 Qty: 2	Job Number: 22-7472 Bruce Park Truss Label: B01	Cust: R 215 JRef: 1Xf02150008 T4 / DrwNo: 115.22.1403.48293 / YK 04/25/2022
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF											
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity									
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 K 999 240												
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.002 T 999 180												
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 Q - -												
Des Ld:	40.00	EXP: C Kz: NA		HORZ(TL): 0.000 Q - -												
NCBCLL:	10.00	Mean Height: 15.00 ft		Creep Factor: 2.0												
TCDL:	5.0 psf	BCDL: 5.0 psf		Max TC CSI: 0.226												
Soffit:	2.00	MWFRS Parallel Dist: 0 to h/2		Max BC CSI: 0.038												
Load Duration: 1.25		C&C Dist a: 3.00 ft		Max Web CSI: 0.074												
Spacing: 24.0 "		Loc. from endwall: Any														
		GCpi: 0.18														
		Wind Duration: 1.60														
Lumber																
Top chord: 2x4 SP #2;																
Bot chord: 2x4 SP #2;																
Webs: 2x4 SP #3;																
Stack Chord: SC1 2x4 SP #2;																
Stack Chord: SC2 2x4 SP #2;																
Plating Notes																
All plates are 2X4 except as noted.																
Loading																
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.																
Wind																
Wind loads based on MWFRS with additional C&C member design.																
Wind loading based on both gable and hip roof types.																
Additional Notes																
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.																
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals.																
Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.																
The overall height of this truss excluding overhang is 6-3-4.																



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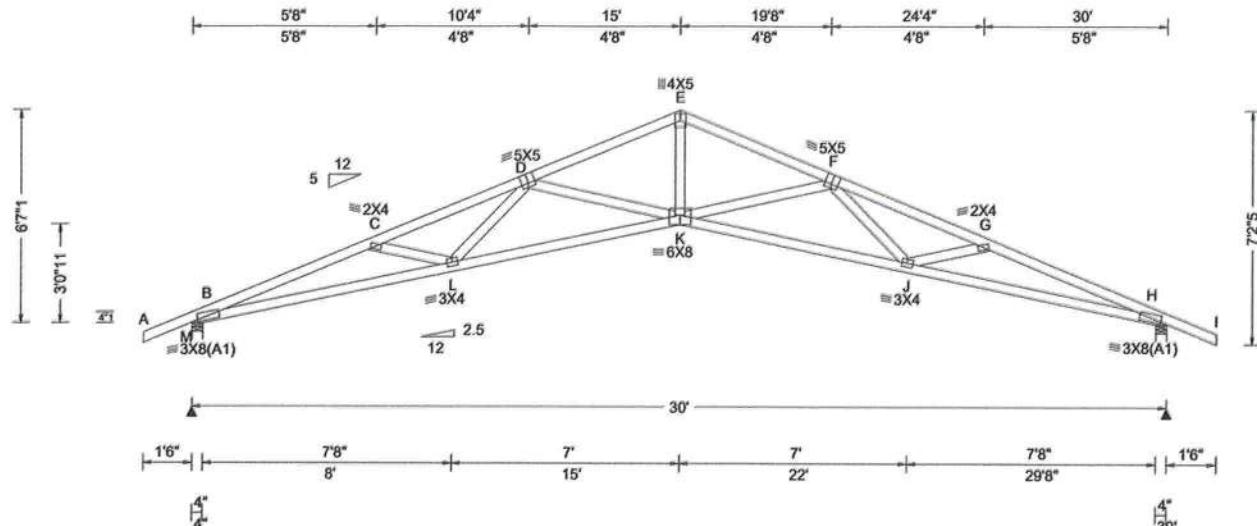
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SEQN: 626346 / FROM: CDM	COMM Ply: 1 Qty: 7	Job Number: 22-7472 Bruce Park Truss Label: B02	Cust: R 215 JRef: 1Xf02150008 T5 / DrwNo: 115.22.1403.48371 / YK 04/25/2022
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Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)			
TCLL: 20.00		Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity	Non-Gravity		
TCDL: 10.00		Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.432 K 825 240	Loc	R+	/ R-	/ Rh
BCLL: 0.00		Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.870 K 409 180	M	1330	/ -	/ 755 /243 /170
BCDL: 10.00		Risk Category: II	Snow Duration: NA	HORZ(LL): 0.249 J - -	H	1330	/ -	/ 755 /243 / -
Des Ld: 40.00		EXP: C Kzt: NA		HORZ(CL): 0.503 J - -	Wind reactions based on MWFRS			
NCBCLL: 10.00		Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	M	Brg Width = 4.0	Min Req = 1.5	
TCDL: 5.0 psf		TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.695	H	Brg Width = 4.0	Min Req = 1.5	
Soffit: 2.00		BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.507	Bearings M & H are a rigid surface.			
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.794	Members not listed have forces less than 375#			
Spacing: 24.0 "		C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)			
		Loc. from endwall: Any	Plate Type(s):		Chords	Tens.Comp.	Chords	Tens. Comp.
		GCpi: 0.18	WAVE		B - C	1808 -4544	E - F	1239 -3133
		Wind Duration: 1.60			C - D	1620 -4207	F - G	1602 -4207
					D - E	1220 -3133	G - H	1796 -4544

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 6'-7-1".



FL REG# 278, Yoonhwak Kim, FL PE #86367
File# 046Q021 Certificate of Product Approval #FL 1999

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

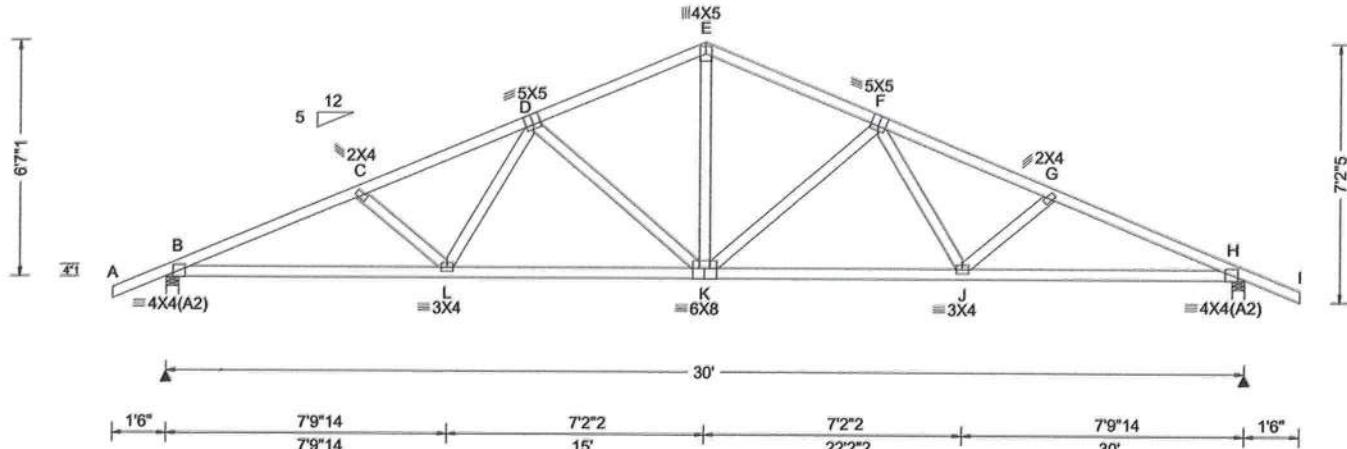
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 614215 / FROM: CDM	COMM Qty: 11	Ply: 1 Job Number: 22-7472 Bruce Park Truss Label: B03	Cust: R215 JRef: 1Xf02150008 T3 DrwNo: 115.22.1403.48326 / WHK 04/25/2022
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5'5"3 10'2"10 15' 19'9"6 24'6"13 30'
5'5"3 4'9"6 4'9"6 4'9"6 4'9"6 5'5"3



1'6" 7'9"14 7'2"2 7'2"2 7'9"14 1'6"
7'9"14 15' 22'2" 30'

Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)			
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in: loc L/defl L/#	Gravity Loc	R+	/R-	Non-Gravity /Rh /Rw /U /RL
TCDL:	10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.134 K 999 240	B	1324	/ -	/756 /243 /170
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.269 K 999 180	H	1324	/ -	/756 /243 / -
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.050 J - -	Wind reactions based on MWFRS			
Des Ld:	40.00	EXP: C Kzt: NA		HORZ(TL): 0.099 J - -	B	Brg Width = 4.0	Min Req = 1.6	
NCBLL:	10.00	Mean Height: 15.00 ft	Building Code: FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	H	Brg Width = 4.0	Min Req = 1.6	
Soffit:	2.00	BCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.299	Bearings B & H are a rigid surface.			
Load Duration: 1.25		MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max BC CSI: 0.755	Members not listed have forces less than 375#			
Spacing: 24.0 "		C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Max Web CSI: 0.460	Maximum Top Chord Forces Per Ply (lbs)			
		Loc. from endwall: Any	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.11	Chords	Tens. Comp.	Chords	Tens. Comp.
		GCap: 0.18			B - C	1057 -2525	E - F	806 -1629
		Wind Duration: 1.60			C - D	982 -2293	F - G	982 -2293

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 6'-7-1".



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Fl 04/25/2020 Certificate of Product Approval #FL 1999

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**

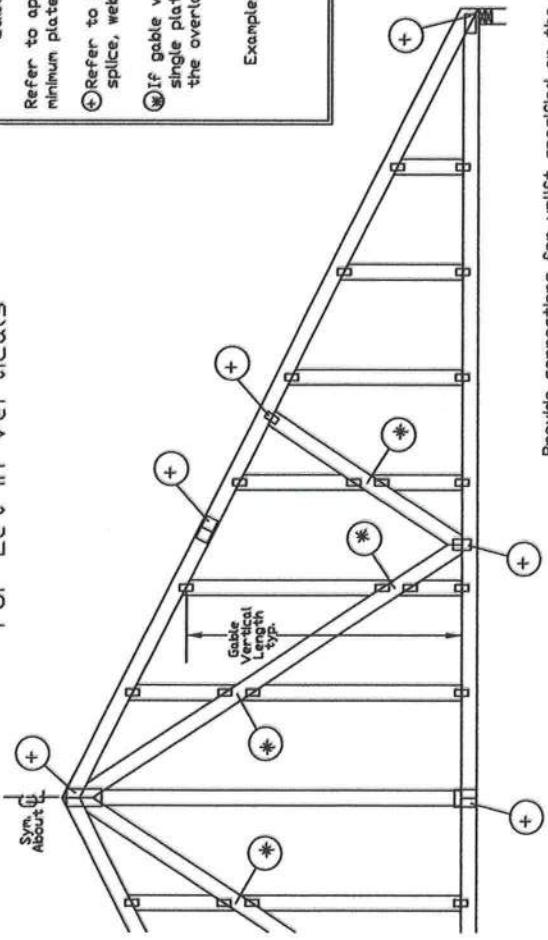
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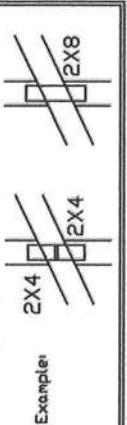
Gable Detail
For Let-in Verticals



Geopolitical Trends Page Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- Refer to Engineered truss design for peak, splice, web, and heel plates.
 - If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with
End Driven Nails.
10d Common (0.148" x 3.5" min) Nails at 4"

Toenail Nails[®]
10d Common (0.148" x 3", min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dri 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dri 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Dri 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing	Brace Species	Grade	Brace No.	Group A			Group B			Group A			Group B			Group A			Group B			Group A				
				(1) 2x4 1" Brace	(1) 2x4 1" Brace	(1) 2x4 1" Brace	(2) 2x4 1" Brace	(2) 2x4 1" Brace	(2) 2x4 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(2) 2x6 1" Brace	(2) 2x6 1" Brace	(2) 2x6 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(2) 2x6 1" Brace	(2) 2x6 1" Brace	(2) 2x6 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(1) 2x6 1" Brace	(2) 2x6 1" Brace	(2) 2x6 1" Brace
24" O.C.	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Std	4' 1"	6' 7"	7' 1"	8' 6"	8' 6"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	Std	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF	#1 / #2	8' 4"	8' 8"	8' 11"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	Std	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#2	4' 11"	8' 4"	8' 8"	9' 8"	10' 10"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	Std	4' 9"	7' 4"	7' 9"	9' 9"	10' 9"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	4' 8"	6' 11"	6' 5"	6' 11"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF	#1	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	Std	5' 1"	9' 0"	9' 2"	9' 4"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#2	5' 11"	8' 4"	8' 8"	9' 10"	10' 10"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	#3	5' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	Std	5' 9"	7' 4"	7' 9"	9' 9"	10' 8"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	5' 8"	5' 5"	5' 2"	5' 6"	6' 10"	9' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SPF	#1 / #2	5' 5"	5' 8"	9' 2"	9' 4"	10' 8"	11' 3"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	HF	Std	5' 1"	9' 0"	9' 2"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	SP	#1	5' 8"	9' 3"	9' 6"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	#2	5' 5"	9' 2"	9' 5"	9' 8"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	#3	5' 3"	8' 5"	8' 8"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	Std	5' 3"	8' 3"	8' 6"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	Standard	5' 1"	7' 5"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	

Max Gable Vertical Length

Bracing Group Species and Grades:		
Group A:	Spruce-Pine-Fir	Hem-Fir
#1 / #2	Standard	#2 Standard
#3	Std	#3 Standard
Douglas Fir-Larch		
	#3	#3 Standard
	Std	Standard

Group B:		
Hem-Fir	#1 & #2	#1 & #2
	Std	Std
Douglas Fir-Larch	#1	#1

Group C:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group D:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group E:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group F:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group G:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group H:		
Southern Pine	#1	#1
	#2	#2
Douglas Fir-Larch	#1	#1

Group
