BUILDING PROFILE

Width (ft) = 40Eave Height (ft) = 16.67 H/SRoof Slope (Rise/12) = 2.0:12Length (ft) = 40

BUILDING LOADS

- A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY ______ STATE COLUMN STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY _____ STATE COLUMN STATE COLU
- B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE FRECTION AND INSPECTION OF THE BUILDING, THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY II - Normal <u>ls 1.0000 le 1.00</u> ULTIMATE 120 MPH NOMINAL 92.95 MPH WIND EXPOSURE C WIND LOAD <u>-0.18 / 0.18</u> Enclosed INTERNAL WIND COEF. CLOSURE TYPE ROOF SNOW LOAD 0.00 PSF Ce 1.0000 Ct 1.2000 0.00 PSF GROUND SNOW LOAD PER CODE SNOW BANKING LOADS COLLATERAL DEAD LOAD 5.00_ PSF

20.00 PSF (REDUCIBLE Yes

DEAD LOAD	2.0	O PSF (FOR	ROOF PANELS AND	PURLINS)
SEISMIC				
CDECTRAL DECRONCE	c- 0.1200	C4 0 0580	C4= 0.1067	C41 0 080

SPECIFIAL RESPONSE S	3 0.1200 ST 0.0000
SITE CLASS D	DESIGN RISK CATEGORY B Cs 0.0356
RESPONSE MODIFICATION FACT	OR, R 3.00* FRAMES 3.00* BRACING
BASIC SEISMIC FORCE RESIST	NG SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES
BASIC SEISMIC FORCE RESIST	NG SYSTEM (ENDWALLS) = ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
BASIC SEISMIC FORCE RESIST	NG SYSTEM (BSW) = ORDINARY STEEL CONC. BRACED FRAMES
BASIC SEISMIC FORCE RESIST	NG SYSTEM (FSW) = ORDINARY STEEL MOMENT FRAMES
ANALYSIS PROCEDURE	= EQUIVALENT LATERAL FORCE PROCEDURE

SERVICEABILITY CRITERIA

ROOF LIVE LOAD

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR

MINIMUM DESIGN DEFLECTIONS									
Endwall Column	=	180	Roof Panel (Live)	=	60				
Endwall Rafter (Live)	=	180	Roof Panel (Wind)	100	60				
Endwall Rafter (Wind)	22.	180	Rigid Frame (Horz)	=	60				
Wall Girt	=	90	Rigid Frame (Vert)	=	180				
Roof Purlin (Live)	=	150	Rigid Frame (Seismic)	=	50				
Roof Purlin (Wind)	=	150							
Wall Panel	=	60							

GENERAL NOTES

- A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER.
- THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

 B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.
- 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
- AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUIDLING CODE
- REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING
- CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER
- PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572 . FLANGES AND WER MATERIAL CONFORMS TO ASTM-A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi. 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIELD
- POINT OF 42,000 psi. 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM
- YIELD POINT OF 46,000 psi.

 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 WITHMINIMUM YIELD POINT OF 50,000 PSI, HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI.
- 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.
 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3
- WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY
- 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT
- B) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.
- IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS". ALL A-325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF:
- BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC—BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
- b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
- c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS REVERSALS ON THE CONNECTIONS.
- d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 SC".

- 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.
- 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307.
- D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.
- E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

- A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
- 1) BE MADE IN CONTRASTING INK.
- 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
- 3) BE LEGIBLE AND UNAMBIGUOUS.
- B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.
- C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.
- D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED
- E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

- A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.
- IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.
- LOCAL STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE C) WORKER SAFETY.
- MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.
- DAILY MEFTINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

- A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.
- B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS
- C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS.
- (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)

 D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

 E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING
- MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT-URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED. F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH
- THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

 G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT, HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
- IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
- THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
- H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE FRECTOR AS A PERMANENT PART OF THE STRUCTURE.
- I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)
- J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT. BEARING VALUES. TIE RODS AND OTHER ASSOCIATED (TEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)
- K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored) Wall Field Values = 28.674 psf / -31.063 psf Wall Edge Values = 28.674 psf / -38,231 psf



BUILDINGS AND MORE

LORIDA PRODUCT APPROVAL NUMBER PRR ROOF PANEL 36875.1 PBR WALL PANEL 36876. 3070 WALKDOORS +50/-50 PSF 22211.4

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT. HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT

FRAMING COLORS

Rügid Frame: RO	RO Red Oxide
Flange braces: RO	GP Grey Primer
Angle: RO	GZ Galvanized

						End	Wall
	Grt	Pur	EvSt	Jmb	BB	Col	Rof
u section:	RO	RO	RO	R0	RO	RO	RO
C SECTION	RO I	R0	RO	RO	RO	RO	RO
D SECTION:	RO	RO	RO	RO	RO	RO	RO
Z SECTION	RO	RO :	RO	RO	RO	RO	RO
E SECTION:	RO	RO	RO	RO	RO	RD:	RO
R SECTION:	RO	R0	RO	RO	RO	RO	RO
W SECTION:	RO	R0	RO	RO	RO	RD	RO

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT-UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE-GALVANIZED

WALLS:

CABLE

EAVE:

BASE

CORNER:

FRAMED OPENING

		DI	RAWING INDEX					
	REV.	PAGE	DESCRIPTION					
		0	COVER PAGE					
		1	ANCHOR BOLT LAYOUT					
		1.1	ANCHOR BOLT DETAILS					
		1.2	ANCHOR BOLT REACTIONS					
		2	ROOF FRAMING LAYOUT					
		2.1-2.2	RIGID FRAME CROSS SECTION					
i		3-3.1	ENDWALL FRAMING/SHEETING LAYOUT					
		4-4.1	SIDEWALL FRAMING/SHEETING LAYOU					
ì		5-5.4	FRAMING DETAILS					
		6	ROOF PANELS & TRIM					
		6.1	ROOF PANEL DETAILS					
		7	SIDEWALL PANEL DETAILS					
		8	ENDWALL PANEL DETAILS					
		9	SPECIAL DETAILS					
	IC DECIG	MED 10 M	FUOLOGED DURING ACCESSORIES					

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE.

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS.



BUILDING DESIGNED & MANUFACTUR BY AN IAS ACCREDITED FACILITY.

ED					
		BUILDINGS AND MORE	792 SW BASCOM NORRIS DR.	LAKE CITY, FL 32025	
	FROM:	BUILD	792	LAKE	

CONTAINER

WOODS

띺 149

GLN

WOOLSEY

SW

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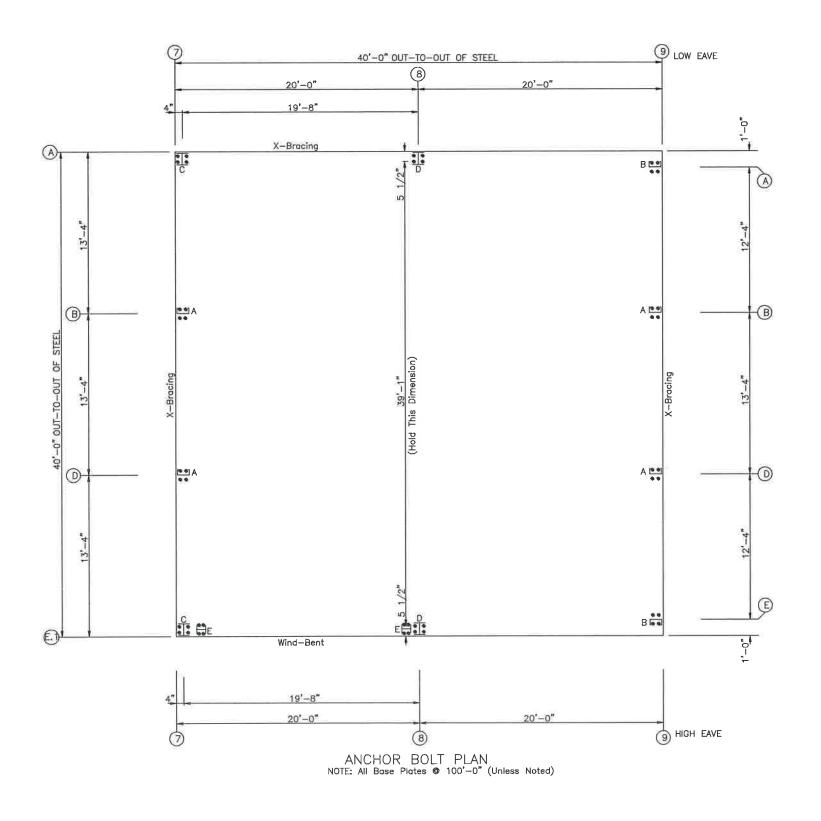
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NONE

JOBSI

		ш ш	7 7	
	DRAWING STATUS	JOB NO :	8967B	
OLORS:	FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL	DATE :	1/ 2	2/25
BLACK BLACK	REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION"	BY: BJC	SC	ALE :
BLACK	CAN BE CONSIDERED AS COMPLETE. FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION	TITLE :	OVER F	'AGE
BLACK	NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED, ONLY DRAWINGS ISSUED FOR CONSTRUCTION "CAN BE CONSIDERED AS COMPLETE.	NUMBER :		
BLACK BLACK	FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION	PA	4GE	(



BUILDINGS AND MORE

CUSTOMER:
THE WOODS CONTAINER PARK

JOB NO:
8967B

LOCATION:
LAKE CITY, FL. 32024

DRAWING MAKE:
ANCHOR BOLT LAYOUT

BJC

DET CHK DATE

SPW SCALE:

NONE

ISSUE

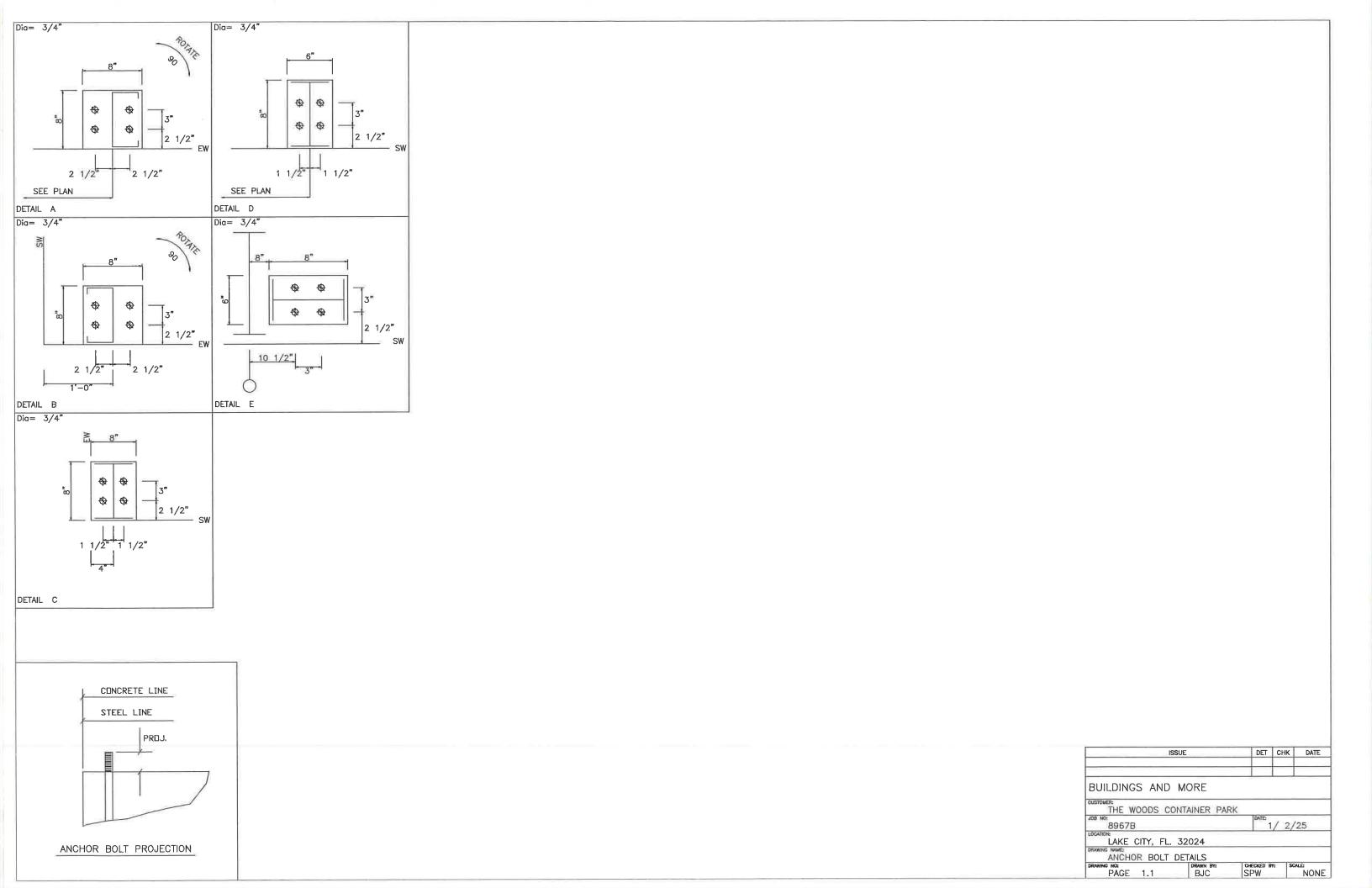
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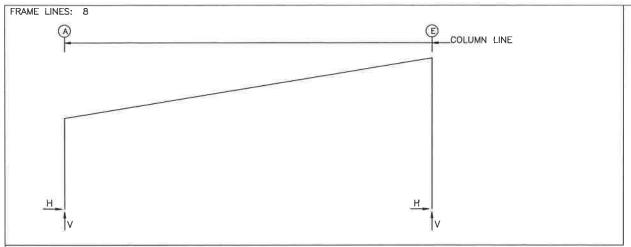
NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE:

(1) 6' 4" ** 7' 2" FRAMED OPENING

FIELD LOCATE:
(1) 6'-4" x 7'-2" FRAMED OPENING
(2) 3'-4" x 7'-2" FRAMED OPENING





RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col	_Bolt	Base_	_Plate (i	n)	Grout
Line	Line	Dia	Width	Length	Thick	(in)
8	A E	0.750 0.750			0.375 0.375	0.0 0.0

L									
ĺ	END	WALL	COI	LUMN:	ANCI	HOR BOL	TS & B	ASE PLA	TES
	Frm Line	Col Line	Anc. Qty	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)	
	7	Α	4	0.750	8.000	8.000	0.375	0,0	
١	7	В	4	0.750	8.000	8.000	0.250	0.0	
ı	7	D	4	0.750	8.000	8,000	0.250	0.0	
١	7	E	4	0.750	8.000	8.000	0.375	0.0	
I	9	Ε	4	0.750	8.000	8.000	0.250	0.0	
ı	9	D	4	0.750	8.000	8.000	0.250	0.0	
ı	9	В	4	0.750	8.000	8,000	0.250	0.0	
	9	Α	4	0.750	8.000	8.000	0.250	0.0	

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Туре	Proj (in)
₩ 32	Endwall	3/4"	GR36	1.50
₩ 8	Frame	3/4"	GR36	2.50
₩ 8	WindCol	3/4"	GR36	2.50

GENERAL NOTES

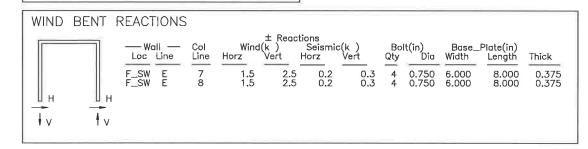
- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUIL	DIN	G BRA	ACINO	RE	ACTIO	SNC					
Loc	Line		± Wi Horz	React nd — Vert	ions(k —Sei Horz) smic — Vert	Panel_ (lb/ Wind	Shear ft) Seis	Note		
L_EW	7	B,D	3.0	2.9	0.2	0.2			(a)		
L_EW F_SW R_EW B_SW	9 A	7,8 D,B 8,7	3.0 2.4	2.9 1.0	0.2 0.3	0.2 0.1			(u)		
(a)Win	d ben	it in ba	у								
Reacti Reacti	Reactions for seismic represent shear force, Eh Reaction values shown are unfactored										

RIGIE	FRAN	ИЕ: в	BASIC COL	UMN REA	CTIONS (k)								
Frame Line 8 8	Column Line A E	Horz 0.7 -0.7	-Dead Vert 1.4 1.5	-—-Colla Horz 1.3 -1.3	teral- Vert 2.5 2.6	Horz 3.2 -3.2	Live——— Vert 5.9 6.1	Wind Horz -8.1 2.7	_Left1- Vert -12.0 -11.1	-Wind_ Horz 1.1 7.7	Right1- Vert -5.8 -9.6	Wind. Horz -7.3 2.0	_Left2- Vert -7.2 -6.4	
Frame Line 8 8	Column Line A E	-Wind_ Horz 2.2 6.6	Right2- Vert -1.0 -4.8	Wind Horz -3.1 4.1	_Long1- Vert -11.2 -11.9	Wind Horz -1.6 1.3	_Long2 Vert -7.4 -7.2	-Seism Horz -0.2 -0.1	ic_Left Vert -0.1 0.1	Seismic Horz 0.2 0.1	_Right Vert 0.1 -0.1			

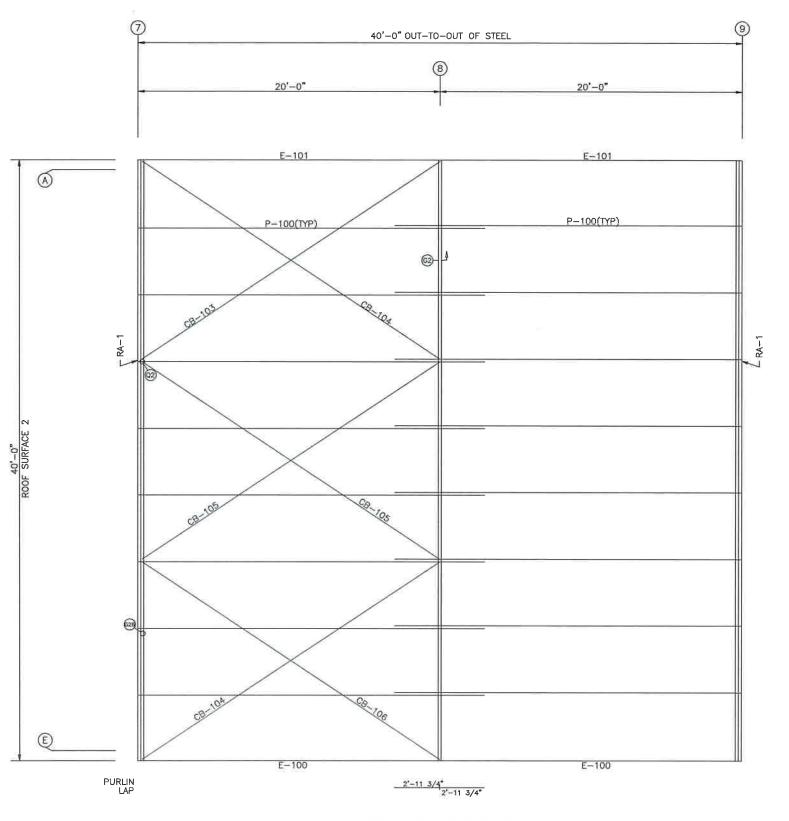
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EN	DWAL	L COL	_UMN:	BASIC C	OLUMN R								
Frm Line 7 7 7 7 7 9 9		Dead Vert 0.2 0.4 0.4 0.3 0.2 0.4 0.4 0.4	Collat Vert 0.3 0.7 0.7 0.3 0.3 0.7 0.7	Live Vert 1.1 2.9 2.9 1.2 1.2 2.9 2.9	Wind Left1 Vert -1.6 -4.4 -1.4 -1.3 -2.7 -2.2 -1.2	Wind Right1 Vert -1.2 -2.2 -2.7 -1.3 -1.4 -4.6 -4.4 -1.6	Wind Left2 Vert -0.9 -3.0 -3.2 -1.0 -0.8 -1.3 -0.9	Wind Right2 Vert -0.6 -0.9 -1.3 -0.8 -1.0 -3.2 -3.0 -0.9	Wind Press Horz -1.0 -1.6 -2.0 -1.8 -1.2 -2.0 -1.6 -0.7	Wind Suct Horz 1.2 1.8 2.2 2.0 1.4 2.2 1.8 0.8	Wind Long1 Vert -1.9 -4.2 -4.4 -1.5 -4.4 -4.2 -1.9	Wind Long2 Vert 1.2 2.5 0.8 0.8 2.6 2.5 1.2	Seis Left Vert 0.0 0.0 0.0 0.0 0.0 0.0
Frm Line 7 7 7 7 9 9		Seis Right Vert 0.0 0.0 0.0 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0 0.0 0.0 0.0	N									

Building reactions are based on the following building data: Width Length Eave Height (ft) = 40.0 Eave Height (ft) = 10.0/ 16.7 Roof Slope Roof Dead Load Wall Dead Load Left Endwall Right Endwall Right Endwall Back Sidewall Back Sidewall Roof Live Load Frame Live Load Collateral Load Wind Speed Wind Code Exposure Closure Internal Wind Coeff Risk Category Importance — Wind Importance — Wind Importance — Seismic Seismic Coeff (ft) = 40.0 (ft) = 40.0 (ft) = 40.0 (ps) = 40.0 (ps) = 2.0 (ps) =



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STOMER: THE WOODS CON	TAINER PAF	RK				
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ANCHOR BOLT RE	ACTIONS					
PAGE 1.2	BJC	SPW	: SC	NONE		

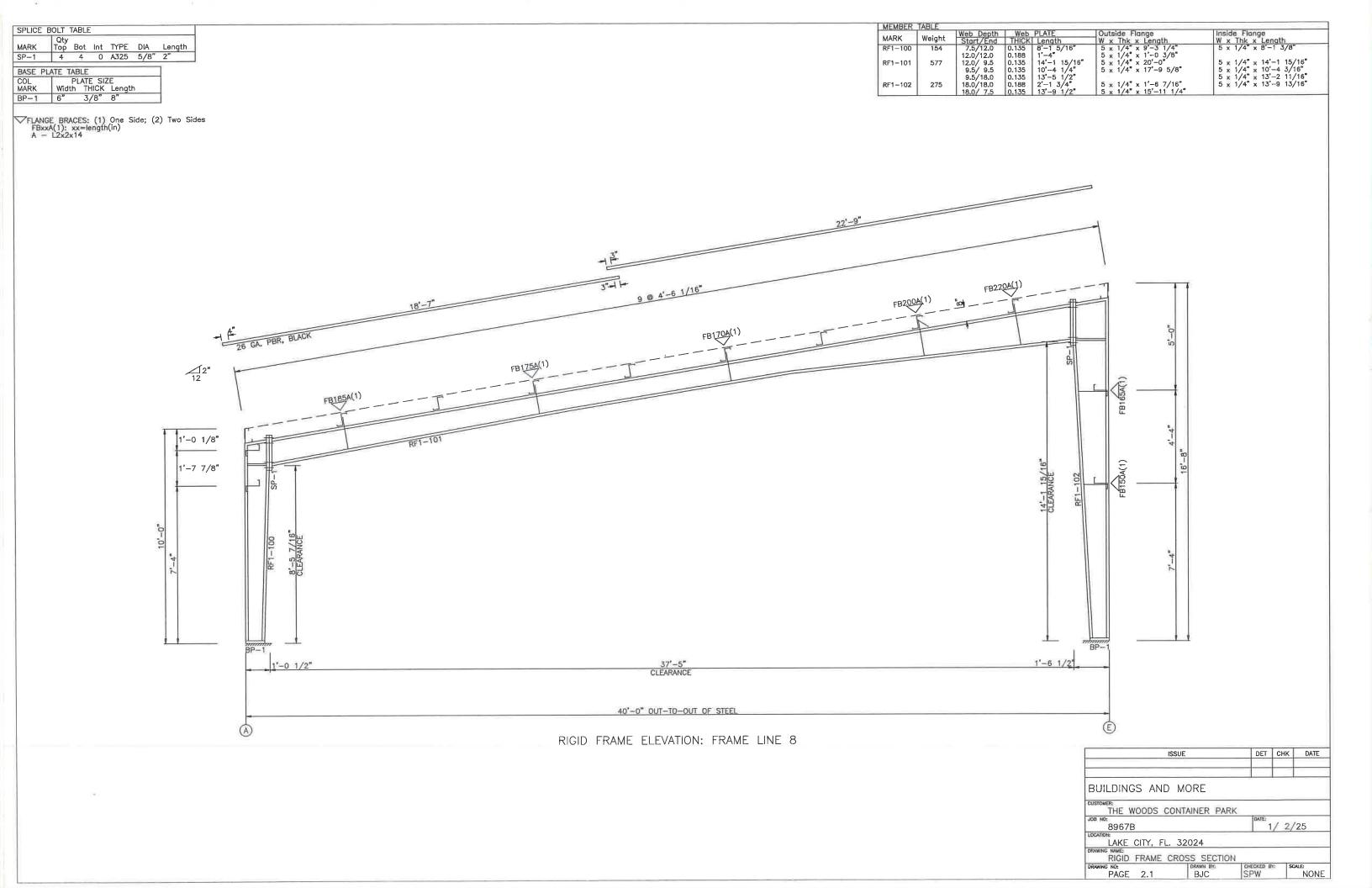
NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.



ROOF FRAMING PLAN

MEMBER	TABLE	
ROOF P	LAN	
MARK	PART	LENGTH
P-100	8x25Z16	22'-11 1/2"
		19'-11 1/2"
E-101	8LE14@2	19'-11 1/2"
CB-103	1/4 CBL	23'-5"
CB-104	1/4 CBL	23'-3"
CB-105	1/4 CBL	24'-0"
CB-106	1/4 CBL	23'-8"

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ROOF FRAMING L				
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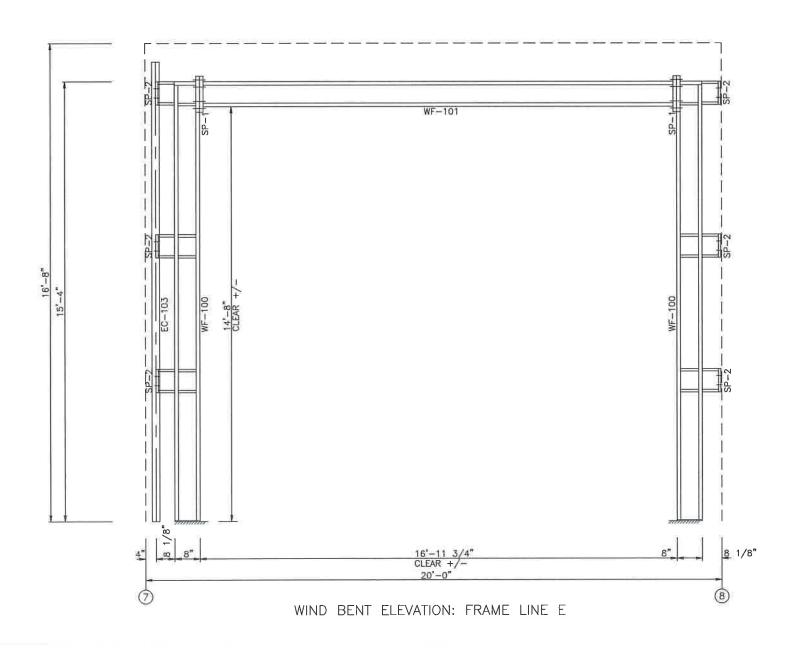


MEMBER SIZE TABLE

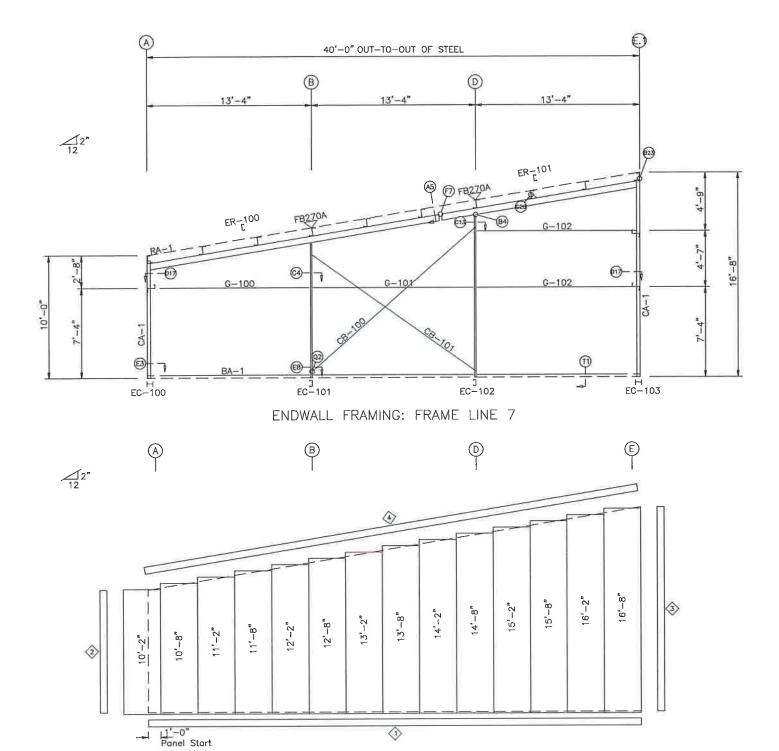
MARK MEMBER LENGTH

WF-101 B08541 16'-11 1/2"

WF-100 B08541 15'-4"



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RIGID FRAME CROS	SS SECTIO	N		
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ENDWALL SHEETING & TRIM: FRAME LINE 7
PANELS: 26 GA. PBR - BLACK

ISSUE DET CHK DATE

BUILDINGS AND MORE

CUSTOMER
THE WOODS CONTAINER PARK

JOB NOT
8967B

LOCATIONE
LAKE CITY, FL. 32024

DRAWING NAME:
ENDWALL FRAMING & SHEETING LAYOUT

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PAGE 3

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NONE

BOLT TABLE FRAME LINE 7 LOCATION

Cor_Column/Raf ER-100/ER-101

Int_Column/Raf
TRIM TABLE
FRAME LINE 7

MEMBER TABLE

FRAME LINE 7
MARK | PART

EC-100 W8X10 EC-101 8X35C16 EC-102 8X35C14 EC-103 W8X10

ER-100 8X35C14 ER-101 8X35C14 G-100 8x25Z16

G-100 8x25Z16 G-101 8x25Z16 G-102 8x25Z16 CB-100 1/4 CBL CB-101 1/4 CBL DIA LENGTH
5/8" 2"
5/8" 2"
5/8" 2"

DETAIL
TRIM_16
TRIM_5
TRIM_5
TRIM_3

LENGTH

9'-6 9/16"

10'-8 7/16"

12'-11 1/8"

15'-10 9/16"

22'-11"

15'-0 3/4"

11'-7 5/8"

12'-11 1/2"

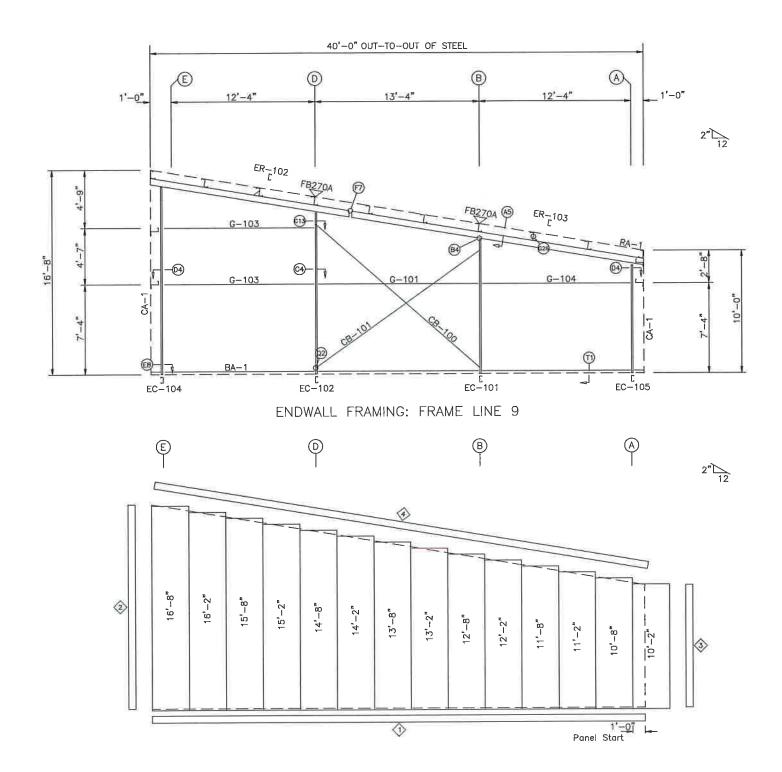
11'-11 5/8"

18'-6"

17'-2"

QUAN TYPE DIA 4 A325 5/ 8 A325 5/ 2 A325 5/

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



ENDWALL SHEETING & TRIM: FRAME LINE 9

PANELS: 26 GA. PBR - BLACK

ISSUE DET CHK DATE

BUILDINGS AND MORE

CUSTOMER: THE WOODS CONTAINER PARK

JOB NO: 8967B

LOCATION: DATE

LAKE CITY, FL. 32024

DRAWING NAME: ENDWALL FRAMING & SHEETING LAYOUT

DRAWING NO: DRAWIN BY: CHECKED BY: SCALE:
PAGE 3.1 BJC SPW NONE

BOLT TABLE FRAME LINE 9 LOCATION ER-102/ER-103

TRIM TABLE
FRAME LINE 9

◇ID PART LENGTH

1 DRIP BASE 13'-7"
2 O/S CORN 16'-10"
3 O/S CORN 10'-2"
4 RAKE TRM 13'-9"

MEMBER TABLE FRAME LINE 9 MARK | PART

MARK PART

EC-101 8X35C16

EC-102 8X35C14

EC-104 8X35C16

EC-105 8X35C16

ER-102 8X35C14

ER-103 8X35C14

G-101 8x25Z16

G-104 8x25Z16

G-104 8x25Z16

CB-100 1/4 CBL

CB-101 1/4 CBL

Columns/Raf

5/8" 2" 5/8" 2"

> DETAIL TRIM_16 TRIM_5 TRIM_5

TRIM_3

LENGTH

10'-8 7/16"

12'-11 1/8"

14'-11 3/4"

8'-7 3/4"

16'-4 5/8"

24'-1 1/2"

12'-11 1/2"

12'-3 1/2"

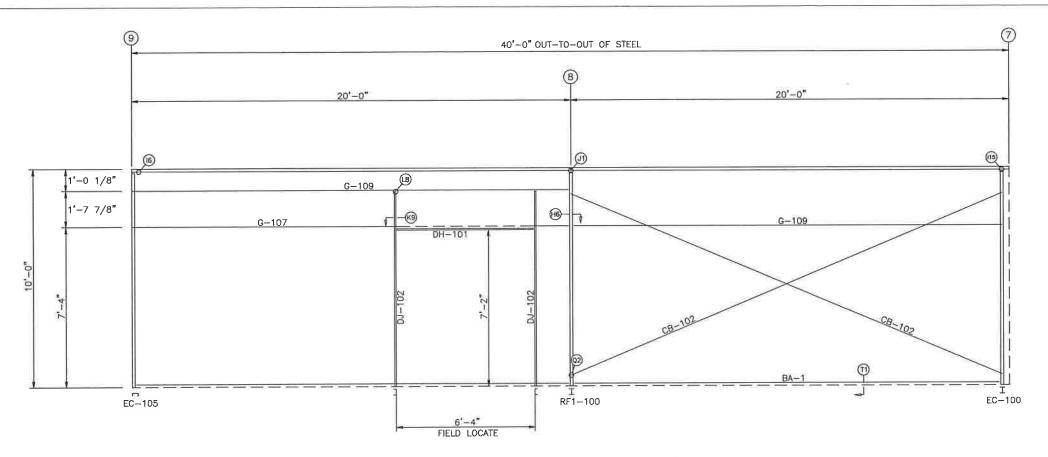
11'-11 1/2"

18'-6"

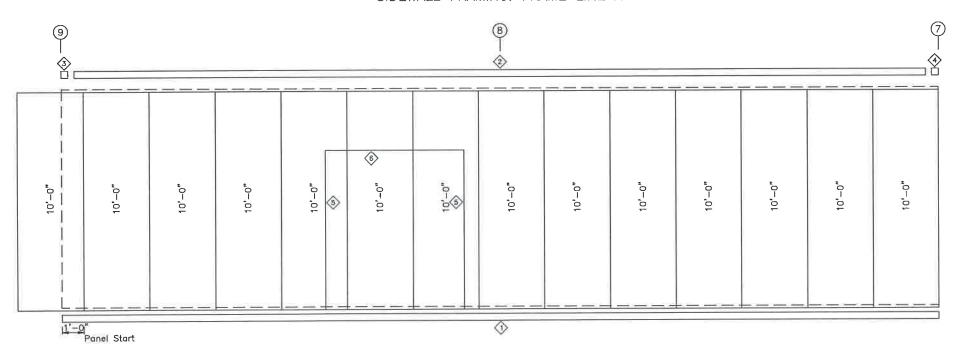
17'-2"

QUAN TYPE DIA 8 A325 5/ 2 A325 5/

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.



SIDEWALL FRAMING: FRAME LINE A

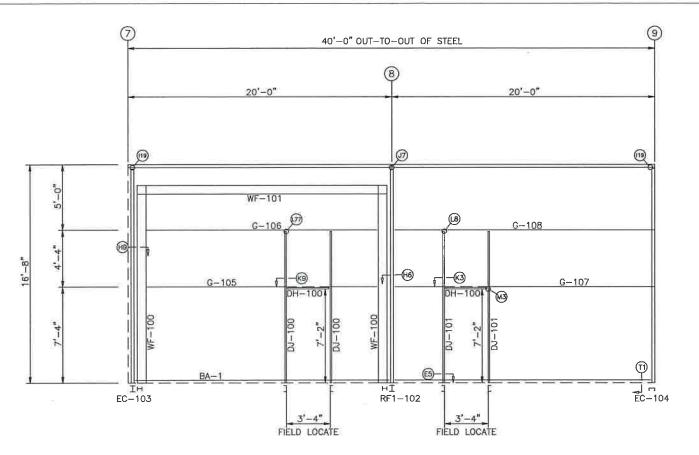


SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 GA. PBR - BLACK

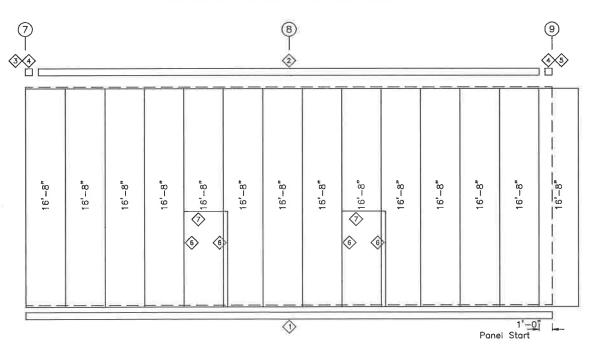
TRIM	ME LINE A		
OID	PART	LENGTH	DETAIL
1	DRIP BASE	13'-7"	TRIM_16
2	EAVE TRM	20'-2"	TRIM_12
3	R END LH	6"	TRIM_13
4	R END RH	6"	TRIM_13
5	R JAMB	7'-5"	TRIM_8
6	R HEAD	6'-7"	TRIM_61

1	MEMBER	TABLE	
	FRAME I	INE A	
	MARK	PART	LENGTH
	DJ-102	8X25C16	8'-11 7/8"
	DH-101	8X25C16	6'-4"
	G⊸107	8x25Z16	18'-11 1/2"
	G-109	8x25Z14	18'-11 1/2"
	CB-102	1/4 CBL	22'-1"

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SIDEWALL FRAMING: FRAME LINE E

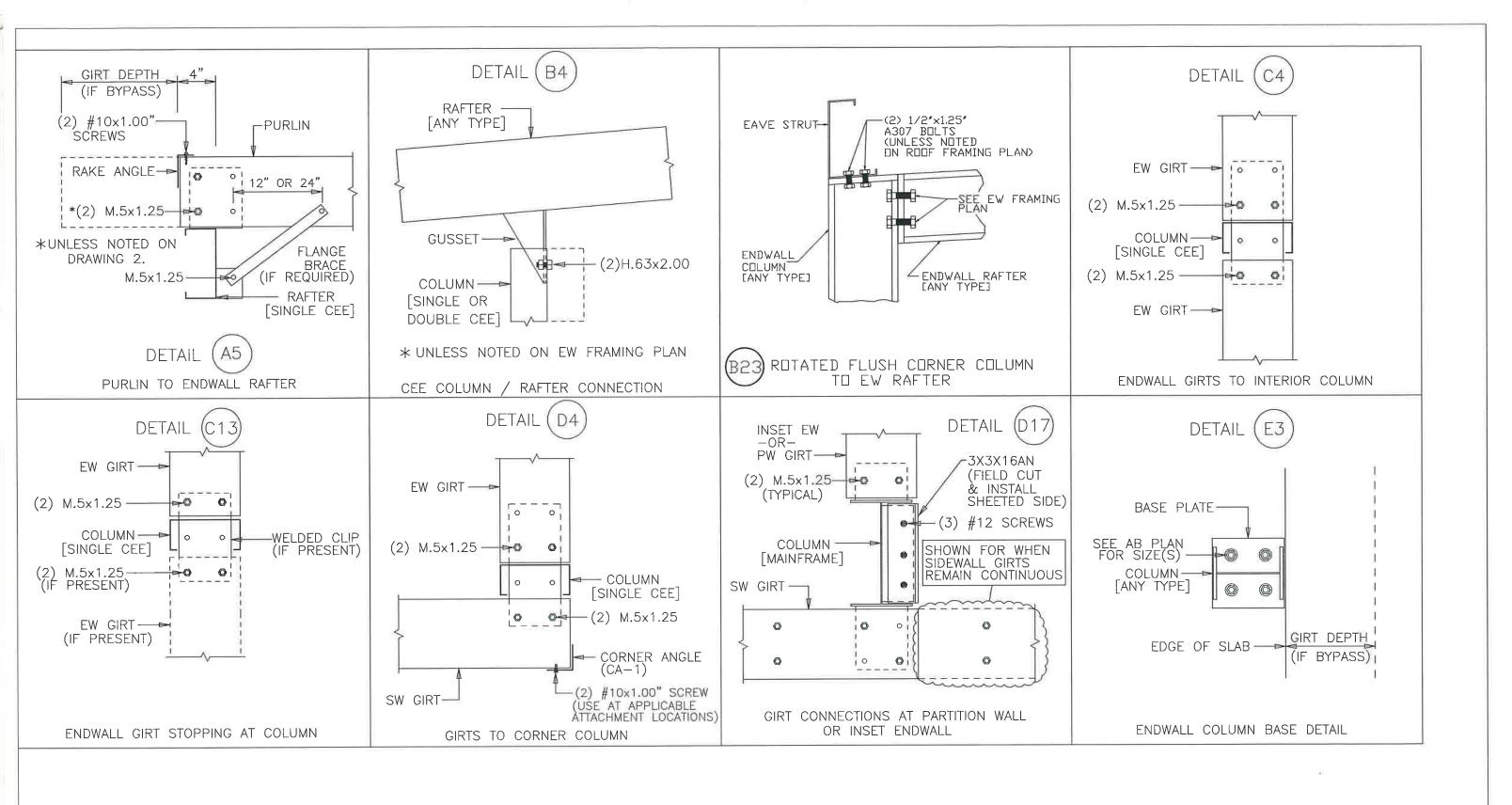


SIDEWALL SHEETING & TRIM: FRAME LINE E
PANELS: 26 GA. PBR - BLACK

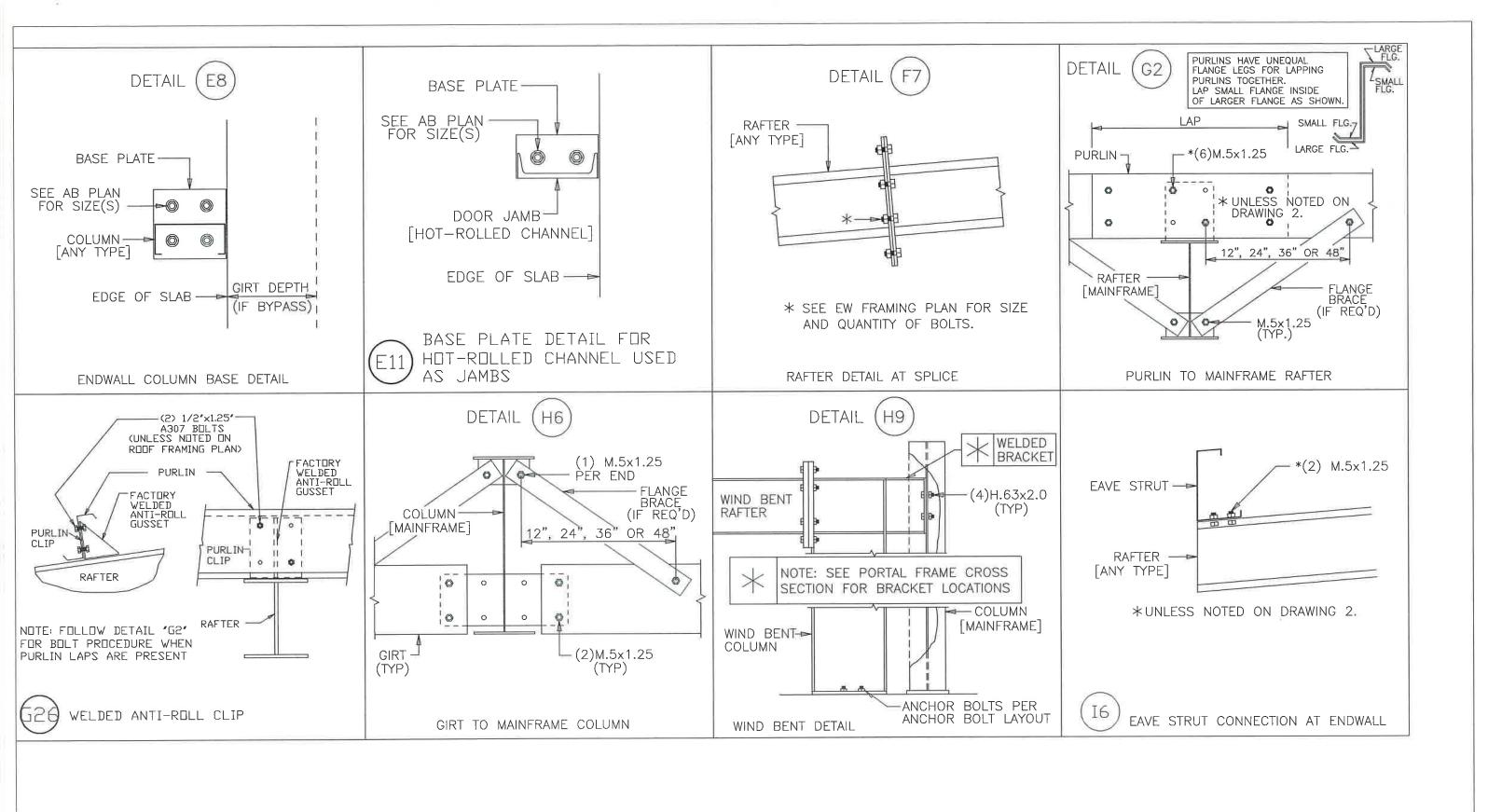
l RC)LI	IARL	E								
FR	AME	LIN	ΕE								
LO	CAT	ION			QUAN	1	TYPE	DIA		LEN	IGTH
WF-	-10	0 —	WF-10)1	8	Т	A325	5	/8" /8"	2"	
WF-	-10	0 —	EC-10	3	12	2	A325	5	/8"	2"	
WF-	-10	0 -	RF1-1	02	12)	A325	- 5	/8"	2" 2"	
-									-		
		TAE									
	<u>FRAI</u>	<u>ME L</u>	INE E								
K	>ID	PAR		LE	NGTH				DETAI	L	
T.	1	DRIP	BASE	13				- 1-	ΓRIM_{-}	_16	
- 1	2	H/S	EAVE	20)'-2"				TRIM_	_14	
	2	ΗŚ	COR L	11'	-0 ["]			-	TRIM	15	
	4	RAKI	E BOX	11	-4"						
	5		CORR	۱۱٬	-o"			-	TRIM_	15	
	6	R JA		ַל	~5 "				TRIM_		
	7		EAD	31	_7 "				TRIM.		
_		18 11			,				LIXHVI	_01	
			MEMDE	T C	VDI E						

MEMBER FRAME L	TABLE INE E	
MARK	PART	LENGTH
WF-100	B08541	15'-4"
WF-101	B08541	16'-11 1/2"
DJ-100	8X25C16	11'-8"
DJ-101	8X25C16	11'–8"
DH-100	8X25C16	3'-4"
G-105	8x25Z16	16'-11 1/4"
G-106	8x25Z12	16'-11 1/4"
G-107	8x25Z16	19'-7 1/2"
G-108	8x25Z12	19'-7 1/2"

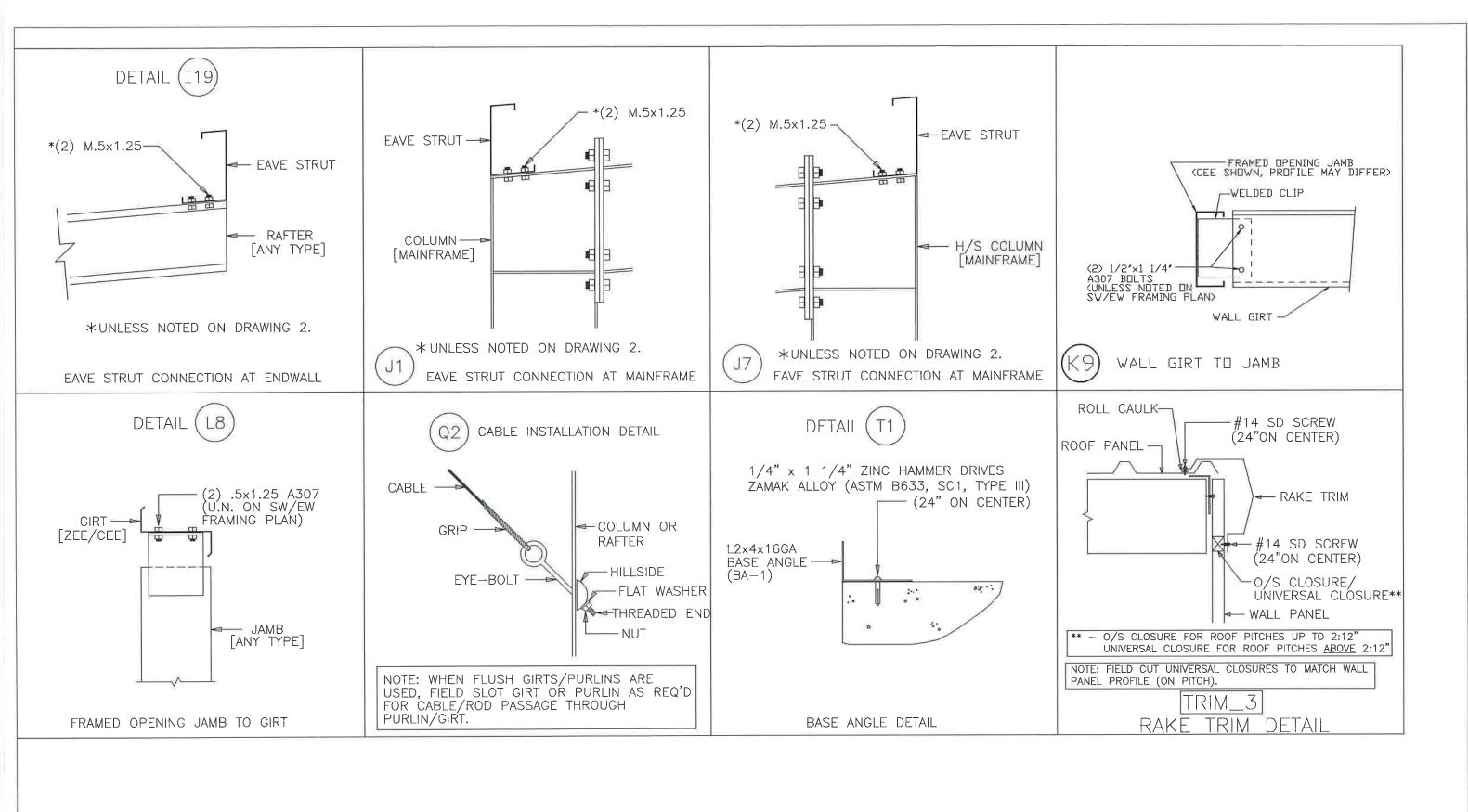
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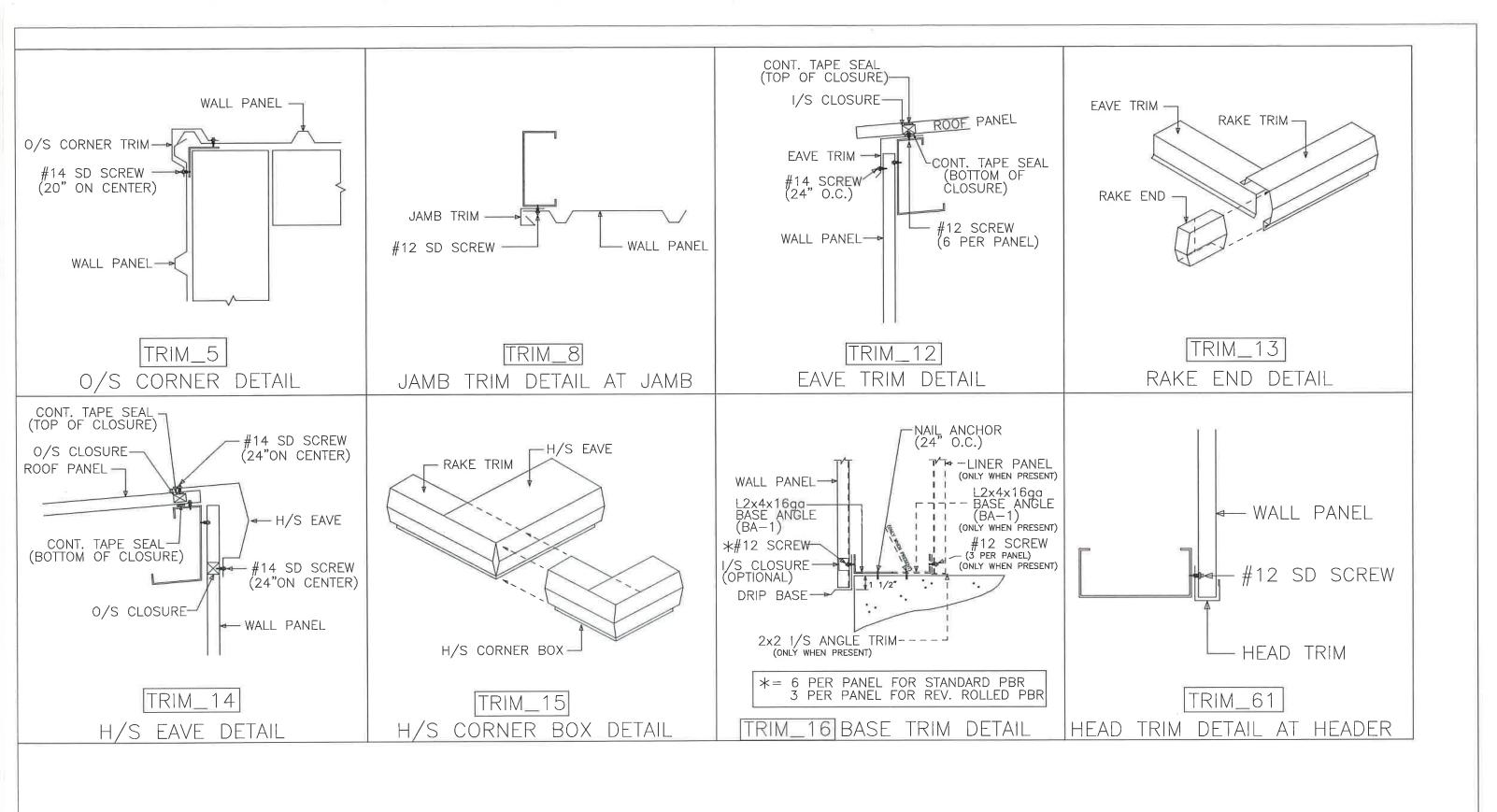
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STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

- [1] SEAL TRIM SPLICES WITH TUBE CAULK.
- [2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
- [3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
- [4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
- [5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.

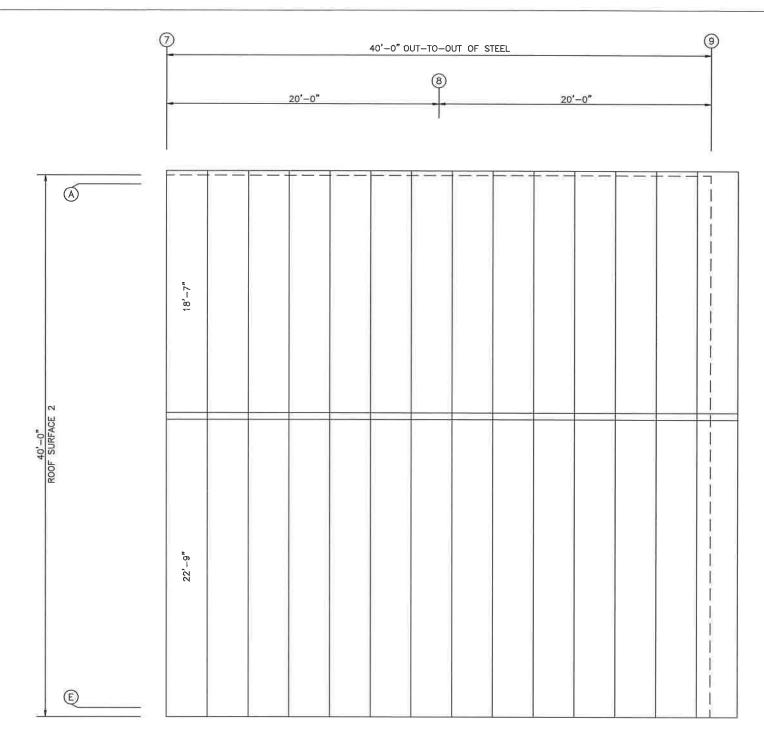
GENERAL SKYLIGHT NOTES

- 1) LTP'S (LIGHT TRANSMITTING PANELS) SHALL NOT BE LOCATED END—TO—END AND SHALL NOT BE LOCATED SIDE—TO—SIDE. IT IS RECOMMENDED THAT A MINIMUM OF (3) METAL ROOF PANELS BE INSTALLED BETWEEN EACH LTP SIDELAP.
- 2) METAL ROOF PANELS ARE NOT TO BE INSTALLED USING A SIMPLE—SPAN CONDITION. IT IS RECOMMENDED THAT A MIN. OF (3) ROOF FRAMING MEMBERS, PREFERABLY (4), SUPPORT ALL METAL ROOF PANELS WHERE ROOF PANELS HAVE BEEN CUT (FACTORY OR FIELD) TO ALLOW FOR LTP INSTALLATION.
- 3) LTP'S SHALL NOT BE INSTALLED EAVE—TO—PEAK <u>OR</u> EAVE—TO—EAVE (ONE LTP PER SINGLE "RUN" OF SHEETING).
- 4) BUILDINGS WITH LESS THAN ~60'-0" OF ROOF PANELS-IN A SINGLE "RUN" TYPICALLY ARE ONLY ALLOWED (1) LTP PER "RUN".
- 5) ANY INSTALLATIONS FOR LTP'S OUTSIDE OF THESE GUIDLINES AND THE DETAILS PROVIDED BY THE M.B.M. REMOVE SAID M.B.M. FROM ANY LIABILITIES OR FAULTS DESPITE CLAIMS AGAINST M.B.M.
- 6) FOR ALL LTP'S PROVIDED BY M.B.M., LIGHT STONE FASTENERS WILL BE PROVIDED FOR A CLOSE MATCH TO LTP COLOR. THIS INCLUDES WALL & ROOF LTP'S.
- 7) DO NOT STEP ON LTP'S ONCE THEY HAVE BEEN INSTALLED! STEPPING ON LPT'S AFTER INSTALLATION MAY RESULT IN INJURY OR DEATH!

BUILT-UP MEMBER LEGEND

	V			
BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
\Box	08	5	4	1
BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

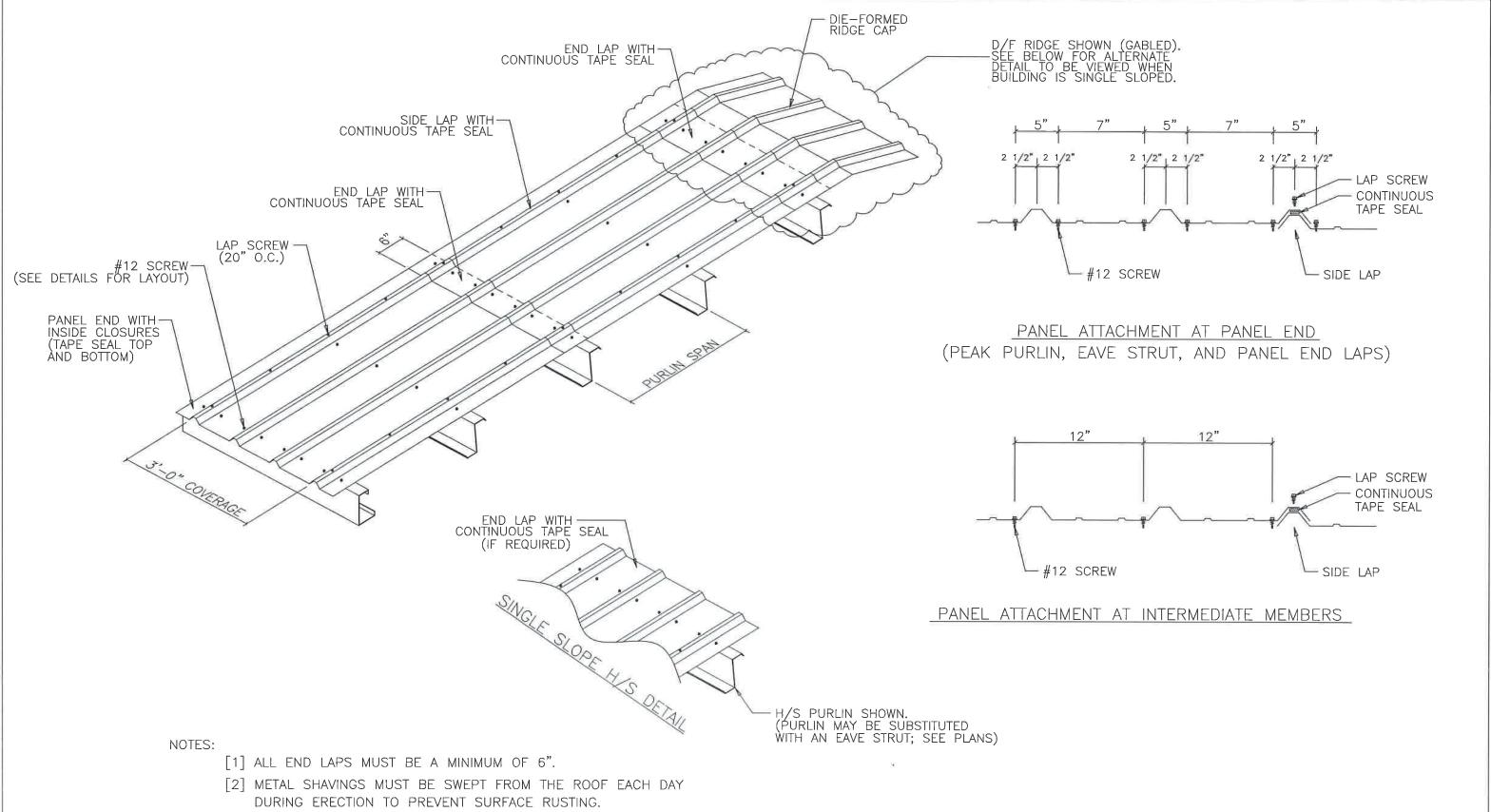
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CUSTOMER: THE WOODS CON	NTAINER PAR	RK		
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LAKE CITY, FL.	32024			
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1'-0" |-Panel Start

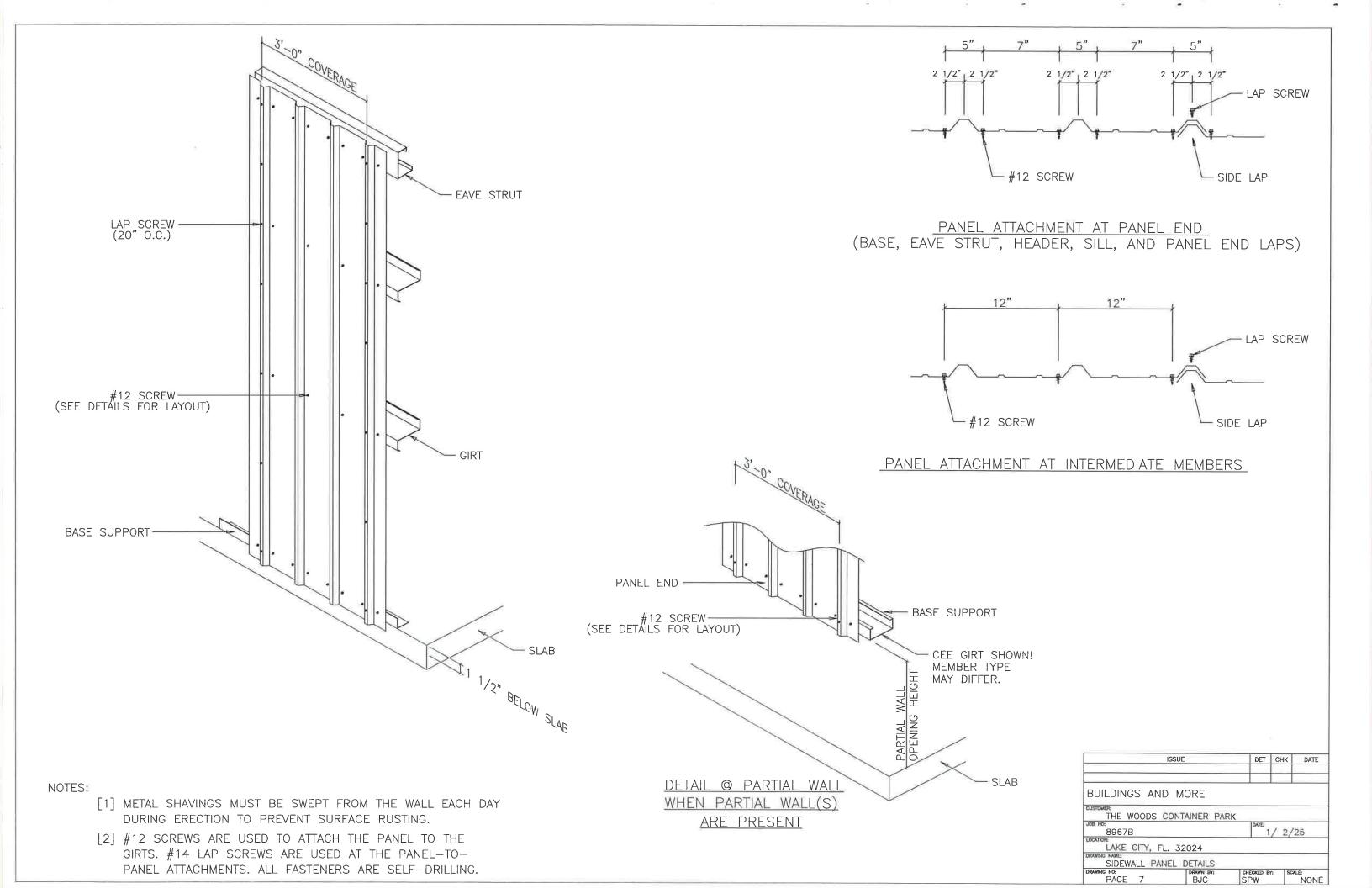
ROOF SHEETING PLAN PANELS: 26 GA. PBR – BLACK

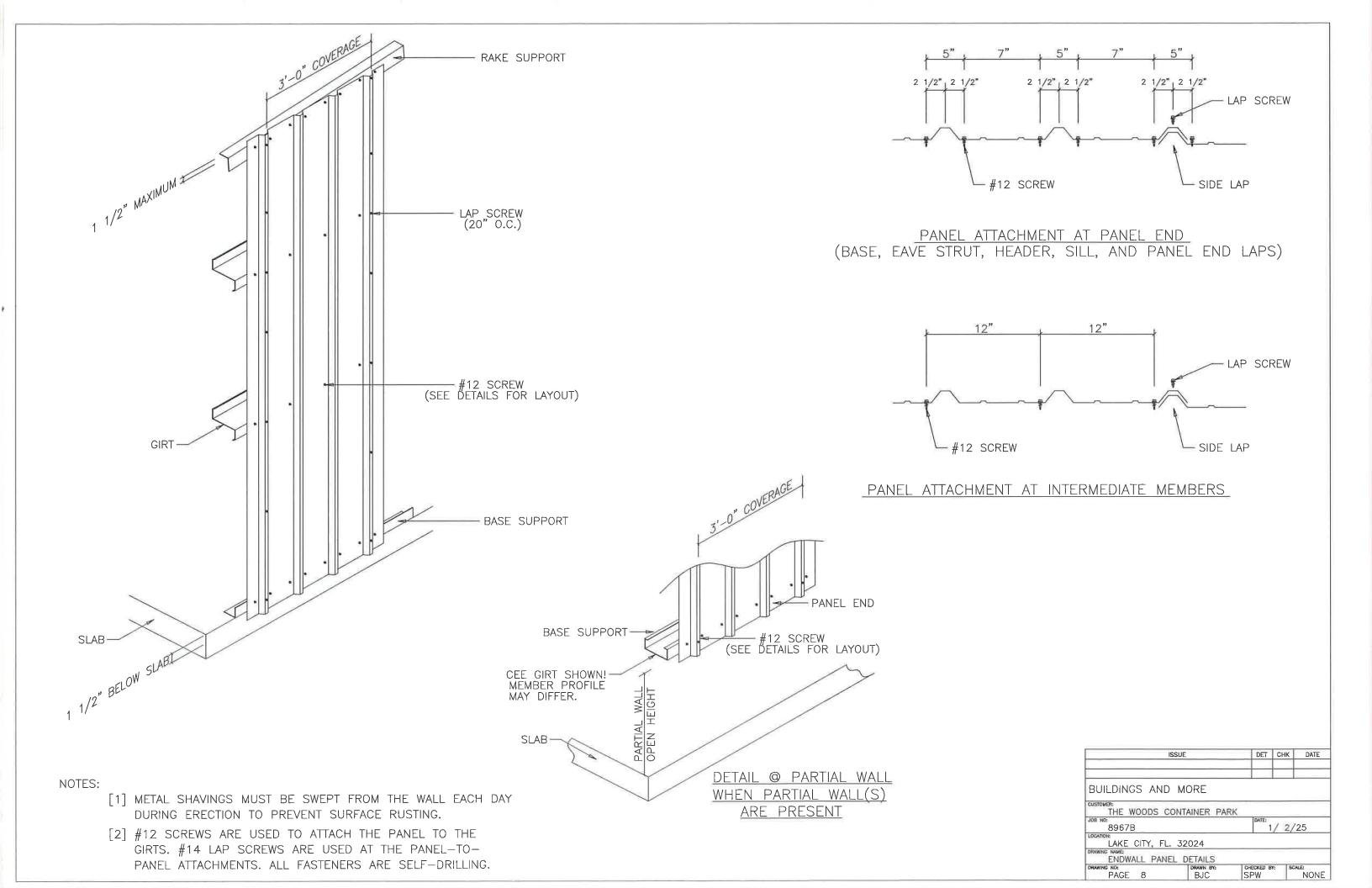
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THE WOODS CONT	AINER PAR	К			
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LAKE CITY, FL. 32	2024				
ROOF PANELS &	TRIM				
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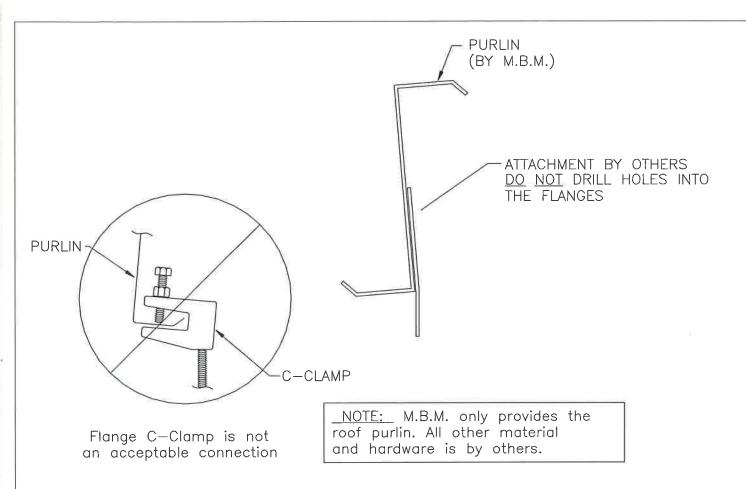


- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

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CUSTOMER: THE WOODS CON	NTAINER PAF	RK		
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Recommended Connection Detail

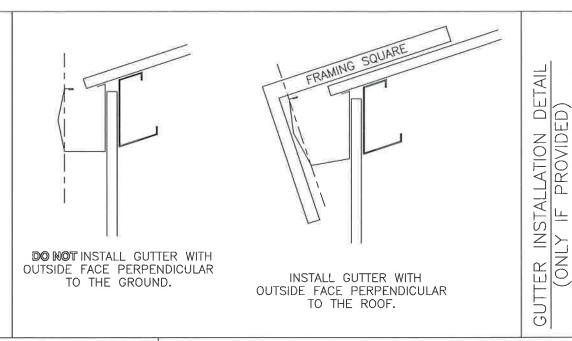
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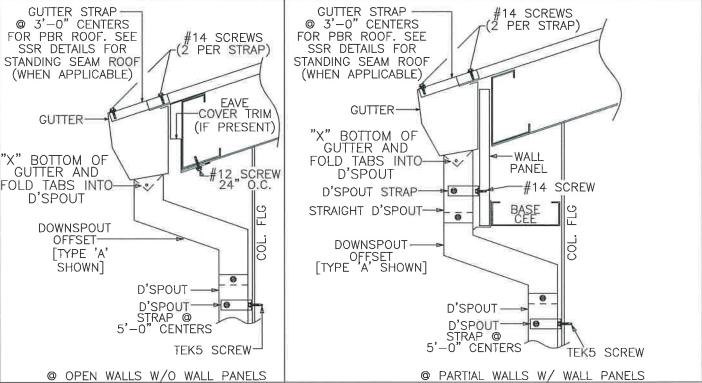
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS





NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

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