JULIUS LEE PE.

RE 530670 - MIKE ROBERTS - CANNON CREEK

1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435

Site Information:
Project Customer Mike Roberts Project Name 530670 Model Custom
Lot/Block TBD Subdivision Cannon Creek
Address
City Columbia Cty State FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name Unknown at time of seal License # Unknown at time of seal
Address Unknown at time of seal
City Unknown at time of seal State Unknown at time of seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code FBC2010/TPI2007 Design Program MiTek 20/20 7.3
Wind Code. ASCE 7-10 Wind Speed· 130 mph Floor Load N/A psf
Roof Load. 32 0 psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules
This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No	Seal#	Truss Name	Date	No	Seal#	Truss Name	Date
1	I7422687	CJ01	10/25/013	18	I7422704	T06	10/25/013
2	I7422688	CJ02	10/25/013	19	I7422705	T07	10/25/013
3	I7422689	CJ03	10/25/013	20	I7422706	T08	10/25/013
4	I7422690	CJ04	10/25/013	21	I7422707	T09	10/25/013
5	I7422691	CJ05	10/25/013	22	I7422708	T10	10/25/013
6	I7422692	EJ01	10/25/013	23	I7422709	T11	10/25/013
7	I7422693	EJ02	10/25/013	24	I7422710	T12	10/25/013
8	I7422694	EJ03	10/25/013	25	I7422711	T13	10/25/013
9	I7422695	EJ04	10/25/013	26	I7422712	T14	10/25/013
10	I7422696	EJ05	10/25/013	27	I7422713	T15	10/25/013
11	I7422697	HJ01	10/25/013	28	I7422714	T16	10/25/013
12	I7422698	HJ02	10/25/013	29	I7422715	T17	10/25/013
13	I7422699	T01	10/25/013	30	I7422716	T18	10/25/013
14	I7422700	T02	10/25/013	31	I7422717	T18G	10/25/013
15	I7422701	T03	10/25/013	32	I7422718	T19	10/25/013
16	I7422702	T04	10/25/013				
17	I7422703	T05	10/25/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name Julius Lee

My license renewal date for the state of Florida is February 28, 2015

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

COUNTY BUILDING DEPARTMENT Received for Julius Lee FILE COPY Code Compliance PLANS EXAMINER

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	CJ01	Jack-Open Truss	8	1	I7422687
					Job Reference (optional)

Builders FirstSource Lake City FL 32055

7.350 s Sep 27 2012 MiTek Industries Inc. Fri Oct 25 11:42:16 2013 Page 1

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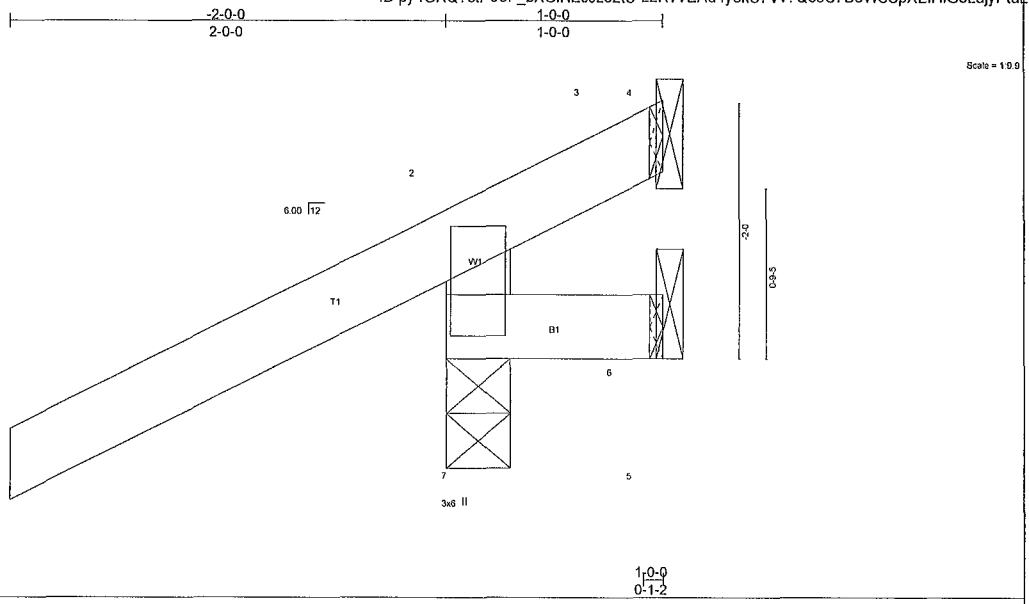


Plate Offsets (X,Y): [7 0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0 00	7	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 07	Vert(TL)	0 00	7	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0 00	3	n/a	n/a		
BCDL	5 0	Code FBC2010/TPI2007		(Matrix-M)							Weight 7 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

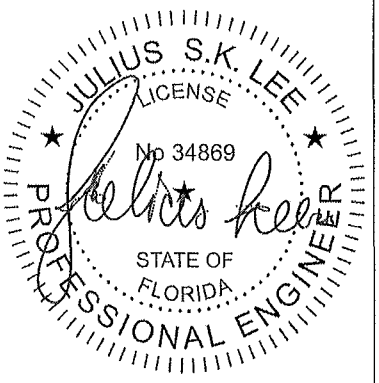
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=248/0-3-8 (min 0-1-8), 3=-77/Mechanical 6=-32/Mechanical
Max Horz 7=66(LC 12)
Max Uplift7=-171(LC 12) 3=-94(LC 2), 6=-40(LC 2)
Max Grav 7=303(LC 2), 3=50(LC 12) 6=20(LC 12)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown.
TOP CHORD 2-7=-276/363

NOTES (7-9)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3 0psf h=18ft, Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 7 94 lb uplift at joint 3 and 40 lb uplift at joint 6.
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
9) Truss Design Engineer Julius Lee PE Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422689
530670	CJ03	Jack-Open Truss	8	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7 350 s Sep 27 2012 MiTek Industries Inc. Fri Oct 25 11:42:18 2013 Page 1

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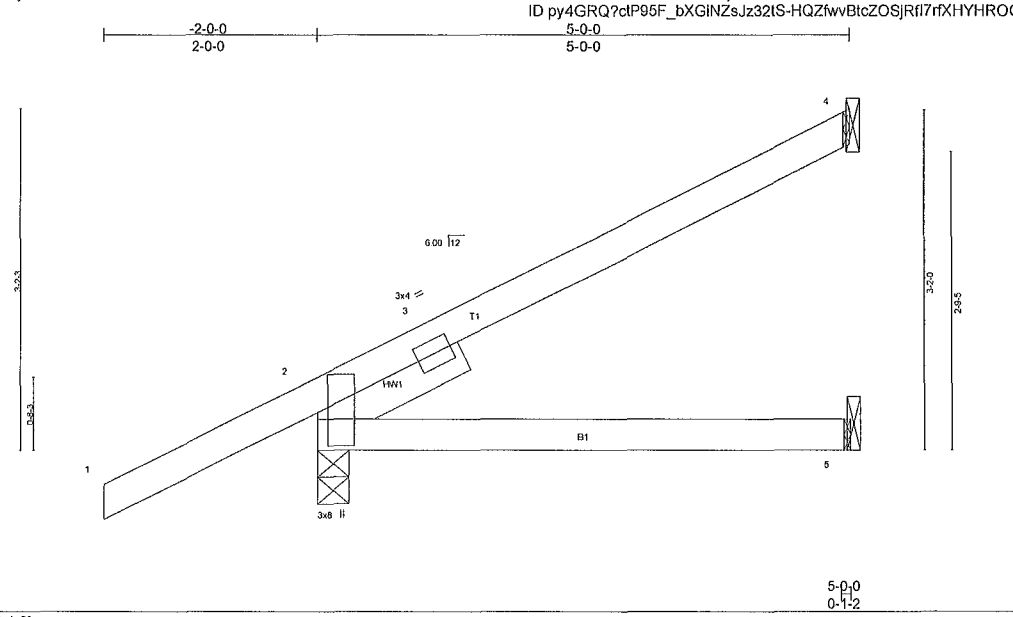


Plate Offsets (X,Y) [2.0-3-0,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1 25	TC	0.42	Vert(LL)	0 12	5-8	>475	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 37	Vert(TL)	0 11	5-8	>538	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	-0 03	4	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 22 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No 3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

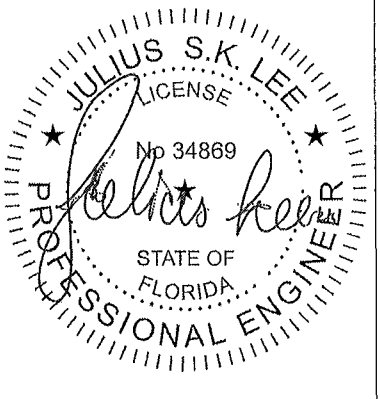
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=85/Mechanical 2=239/0-3-8 (min 0-1-8) 5=31/Mechanical
Max Horz 2=162(LC 12)
Max Uplift 4=-102(LC 12) 2=-126(LC 12) 5=-47(LC 9)
Max Grav 4=103(LC 2) 2=288(LC 2) 5=65(LC 3)

FORCES (lb) Max. Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-245/266

NOTES (7-9)
1) Wind. ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf· h=18ft, Cat. II Exp C; Encl GCpi=0 18· MWFRS (envelope) gable end zone and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 4 126 lb uplift at joint 2 and 47 lb uplift at joint 5.
6) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer· Julius Lee PE. Florida P E License No. 34869· Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



October 25,2013

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422691
530670	CJ05	Jack-Open Truss	1	1	Job Reference (optional)	

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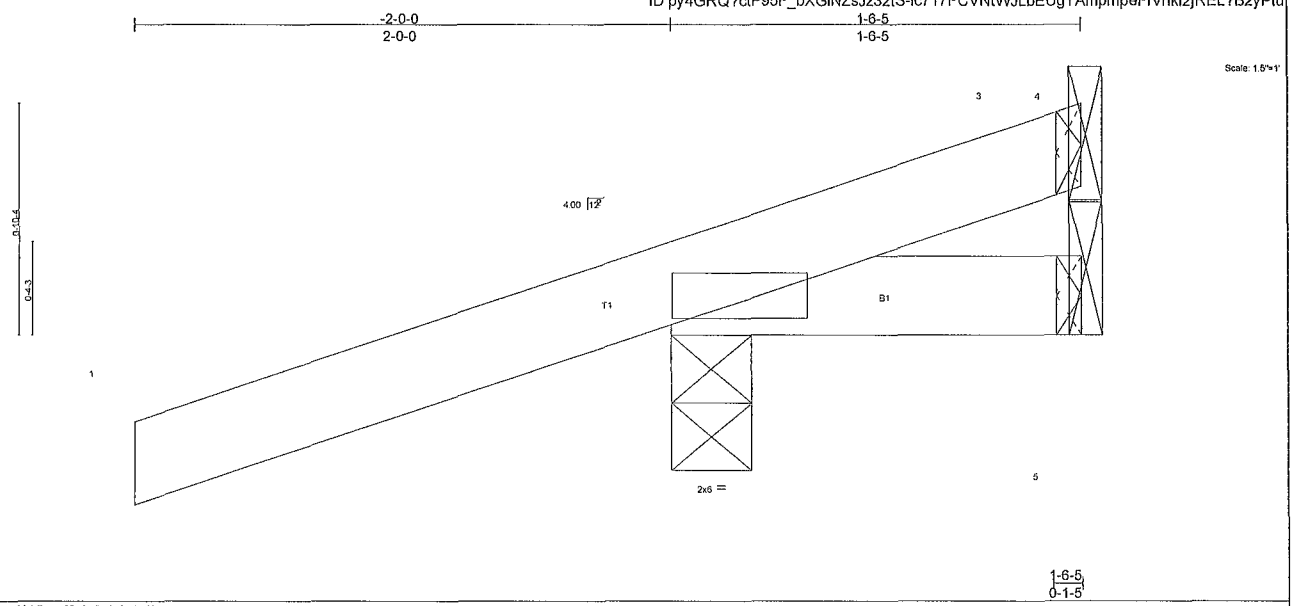


Plate Offsets (X,Y): [2:0-6-1,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0 28	Vert(LL)	0 00	8	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 05	Vert(TL)	0 00	8	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 00	Horz(TL)	0 00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 8 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-6-5 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

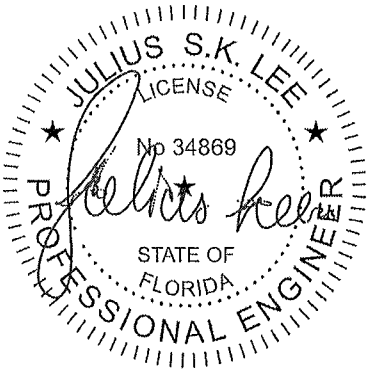
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=174/0-3-8 (min. 0-1-8) 5=-8/Mechanical 3=1/Mechanical
Max Horz 2=68(LC 8)
Max Uplift2=-195(LC 8) 5=-11(LC 2) 3=-8(LC 12)
Max Grav 2=212(LC 2) 5=22(LC 8) 3=9(LC 3)

FORCES (lb) Max. Comp./Max Ten All forces 250 (lb) or less except when shown

NOTES (7-9)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph, TCDL=4.2psf BCDL=3 0psf h=18ft Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 2 11 lb uplift at joint 5 and 8 lb uplift at joint 3.
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
8) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
9) Truss Design Engineer Julius Lee PE Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



October 25,2013

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422693
530670	EJ02	Jack-Partial Truss	1	1	Job Reference (optional)	

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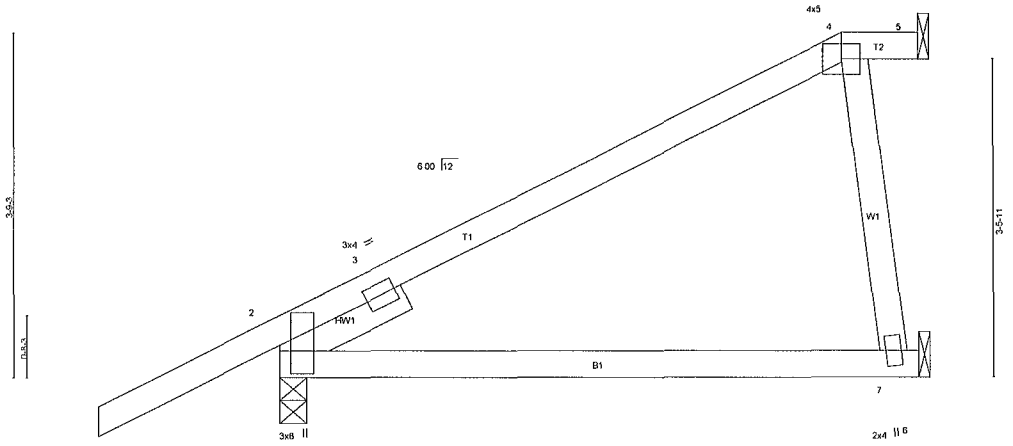
Scale 1/2"=1"



7-0-0
7-0-0

Plate Offsets (X,Y) [2 0-3-12,0-1 7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0 48	Vert(LL)	0.15	7 10	>554	240	MT20	244/190
TCDL	7 0	Lumber Increase	1 25	BC	0 40	Vert(TL)	-0 16	7 10	>502	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 06	Horz(TL)	-0 03	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 32 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=17/Mechanical 2=284/0-3-8 (min 0-1-6) 7=163/Mechanical
Max Horz 2=132(LC 12)
Max Uplift 5= 10(LC 8) 2=-78(LC 12) 7=-78(LC 12)
Max Grav 5=21(LC 2) 2=340(LC 2) 7=192(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-556/260
BOT CHORD 2-7=-293/197

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=18ft; Cat. II Exp C; Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration
4) Provide adequate drainage to prevent water ponding
5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 5, 78 lb uplift at joint 2 and 78 lb uplift at joint 7
9) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
11) Note· Visually graded lumber designation SPp represents new lumber design values as per SPIB
12) Truss Design Engineer· Julius Lee PE Florida P E License No 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

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1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	17422695
530670	EJ04	Half Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055 — 7 350 s Sep 27 2012 MiTek Industries Inc. Fri Oct 25 11:42:22 2013 Page 1

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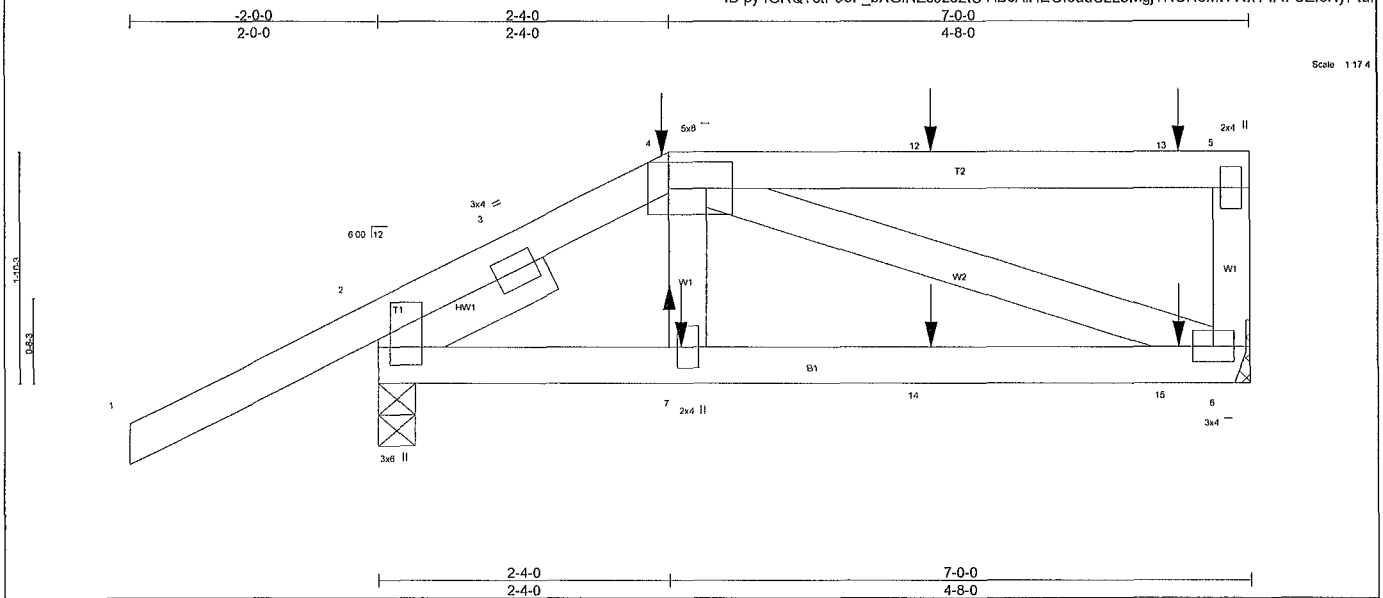


Plate Offsets (X,Y) [2 0-2 8,0-1-3], [4,0-6-0,0-2-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0 02	6-7	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 24	Vert(TL)	-0 04	6-7	>999	180		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0 11	Horz(TL)	0 00	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 37 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No 3
SLIDER Left 2x4 SP No 3 1-6-0

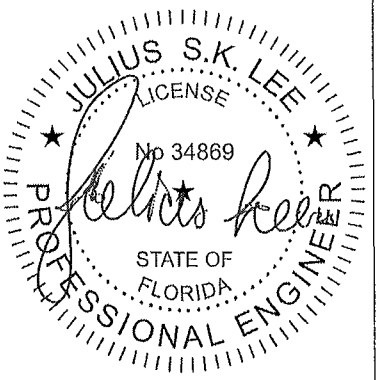
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=322/0-3-8 (min 0-1-8) 6=243/Mechanical
Max Horz 2=67(LC 8)
Max Uplift2=-138(LC 8) 6=-138(LC 5)
Max Grav 2=387(LC 2) 6=287(LC 2)

FORCES (lb) Max Comp./Max Ten. All forces 250 (lb) or less except when shown
TOP CHORD 3-4=-304/115
WEBS 4-6=-261/108

NOTES (11 14)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=18ft, Cat. II Exp C, Encl. GCpi=0 18, MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 2 and 138 lb uplift at joint 6.
8) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 31 lb down and 77 lb up at 2-4-0 and 31 lb down and 77 lb up at 4-5-2 and 31 lb down and 77 lb up at 6-5-2 on top chord and 26 lb down and 16 lb up at 2-4-0, and 21 lb down at 4-5-2 and 21 lb down at 6-5-2 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
13) Truss Design Engineer· Julius Lee PE. Florida P E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435
14) Use Simpson HTU26 to attach Truss to Carrying member



LOAD CASE(S) Standard
1) Regular· Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44 4-5=-44 6-8=-10
Concentrated Loads (lb)
Vert. 4=-26(B) 7=-6(B) 12=-26(B) 13=-26(B) 14=-12(B) 15=-12(B)

October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422697
530670	HJ01	Diagonal Hip Girder	4	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

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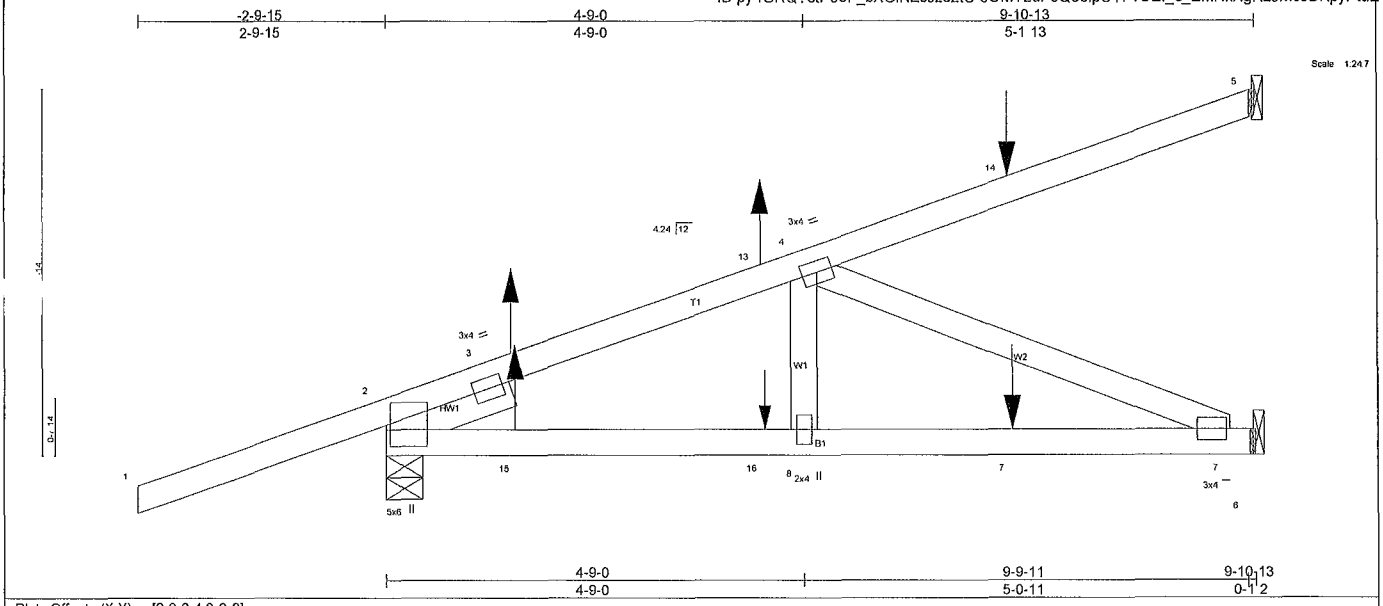


Plate Offsets (X,Y) [2.0-2-4,0-0-8]

LOADING (psf)		SPACING	2 0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0.66	Vert(LL)	0 13	7-8	>894	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 51	Vert(TL)	-0 13	7 8	>938	180		
BCLL	0 0 *	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0 02	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 47 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No 3
SLIDER Left 2x4 SP No.3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 7-4-14 oc bracing

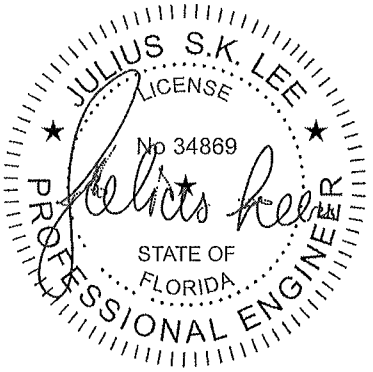
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=131/Mechanical 2=345/0-4-15 (min 0-1-8) 6=188/Mechanical
Max Horz 2=234(LC 4)
Max Uplift 5=-152(LC 4), 2=-398(LC 4) 6=-278(LC 4)
Max Grav 5=159(LC 2), 2=430(LC 2), 6=234(LC 3)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-219/350 3-13=-488/472, 4-13=-436/464
BOT CHORD 2 15=-531/429 15-16=-531/429 8-16=-531/429 8-17=-531/429 7 17=-531/429
WEBS 4-7=-465/576

NOTES (9-11)
1) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) gable end zone porch left and right exposed Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 5 398 lb uplift at joint 2 and 278 lb uplift at joint 6.
6) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 46 lb down and 76 lb up at 1 5-12 46 lb down and 76 lb up at 1-5-12 6 lb down and 53 lb up at 4-3-11 6 lb down and 53 lb up at 4-3-11 and 49 lb down and 103 lb up at 7 1 10 and 49 lb down and 103 lb up at 7 1 10 on top chord and 15 lb down and 32 lb up at 1-5-12 15 lb down and 32 lb up at 1-5-12 5 lb down and 36 lb up at 4-3-11 5 lb down and 36 lb up at 4-3-11 and 35 lb down and 57 lb up at 7 1 10, and 35 lb down and 57 lb up at 7 1 10 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB
11) Truss Design Engineer: Julius Lee PE. Florida P E License No. 34869; Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1 25 Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-44 6-9=-10



Continued on page 2

October 25,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422698
530670	HJ02	Diagonal Hip Girder	1	1	Job Reference (optional)	

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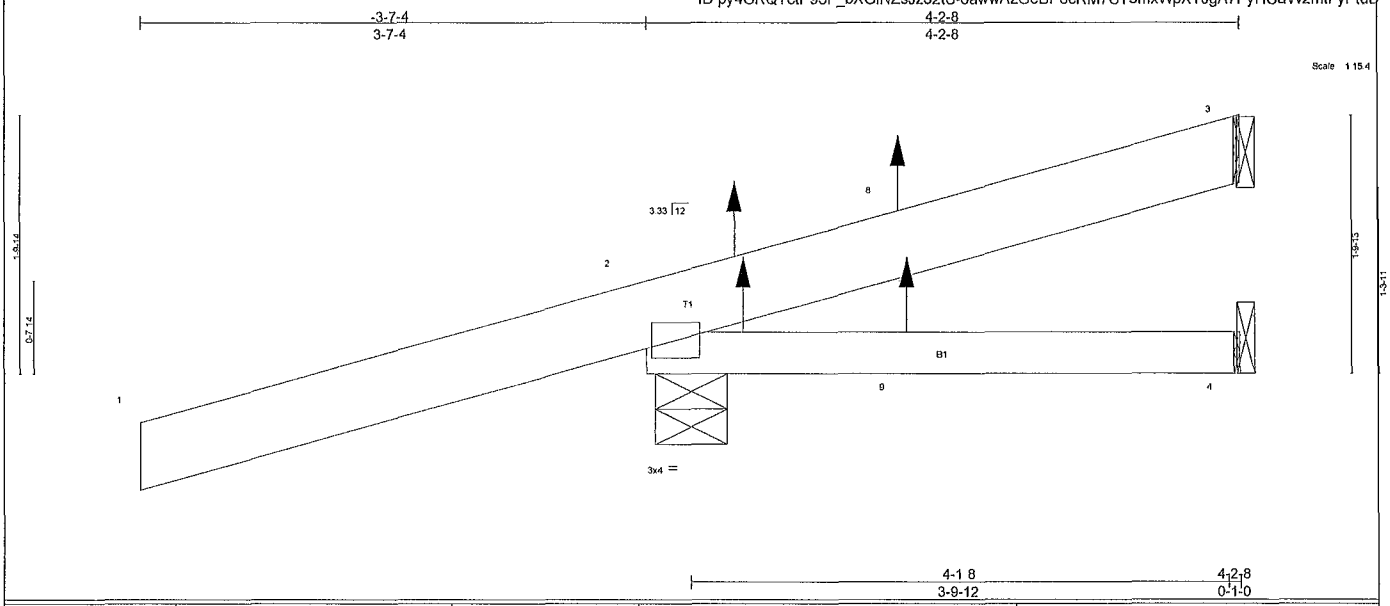


LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1 25	TC 0.39	Vert(LL) -0 03 4-7 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 17	Vert(TL) 0 02 4-7 >999 180	
BCLL 0 0 *	Rep Stress Incr NO	WB 0 00	Horz(TL) 0 01 2 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)		Weight 25 lb FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2
BOT CHORD 2x4 SP No.2

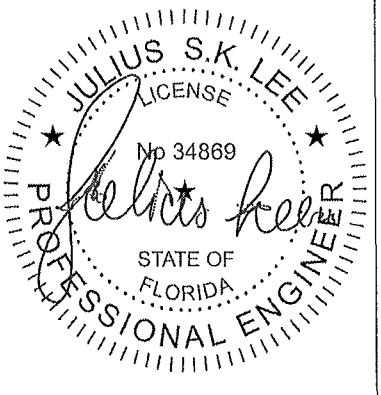
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-8 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=-64/Mechanical 2=179/0-6-0 (min. 0-1-8) 4=-8/Mechanical
Max Horz 2=65(LC 6)
Max Uplift 3=-77(LC 2) 2=-208(LC 4) 4=-11(LC 2)
Max Grav 3=100(LC 4) 2=221(LC 2) 4=37(LC 6)

FORCES (lb) Max Comp./Max Ten. All forces 250 (lb) or less except when shown
TOP CHORD 2-8=-487/450
BOT CHORD 2-9=-257/264

NOTES (9-11)
1) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=18ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope) gable end zone Lumber DOL=1.60 plate grip DOL=1 60
2) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 3 208 lb uplift at joint 2 and 11 lb uplift at joint 4
6) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 49 lb down and 53 lb up at 0-8-2 and 53 lb down and 40 lb up at 1 10-1 on top chord, and 20 lb up at 0-8-2 and 4 lb down and 39 lb up at 1 10-1 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note· Visually graded lumber designation SPp, represents new lumber design values as per SPIB
11) Truss Design Engineer· Julius Lee, PE Florida P E. License No 34869· Address 1109 Coastal Bay Blvd Boynton Beach FL 33435



LOAD CASE(S) Standard
1) Regular· Lumber Increase=1.25 Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-44 4-5=-10
Concentrated Loads (lb)
Vert. 6=42(B) 7=9(B) 8=16(F) 9=25(F)

October 25,2013

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T01	Half Hip Truss	1	1	I7422699
					Job Reference (optional)

NOTES (11 14)
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer Julius Lee PE Florida P E License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach FL 33435
14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Regular Lumber Increase=1 25, Plate Increase=1 25
Uniform Loads (plf)
Vert: 1-4=-44 4-11=-44, 12-20=-10
Concentrated Loads (lb)
Vert: 4=-83(F) 7=-83(F) 19=-216(F) 6=-83(F) 16=-38(F) 15=-38(F) 14=-38(F) 24=-83(F) 25=-83(F) 26=-83(F) 27=-83(F) 28=-83(F) 29=-83(F) 30=-83(F) 31=-83(F) 32=-83(F) 33=-83(F) 34=-83(F) 35=-38(F) 36=-38(F) 37=-38(F) 38=-38(F) 39=-38(F) 40=-38(F) 41=-38(F) 42=-38(F) 43=-38(F) 44=-38(F)

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1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422701
530670	T03	Hip Truss	1	1	Job Reference (optional)	

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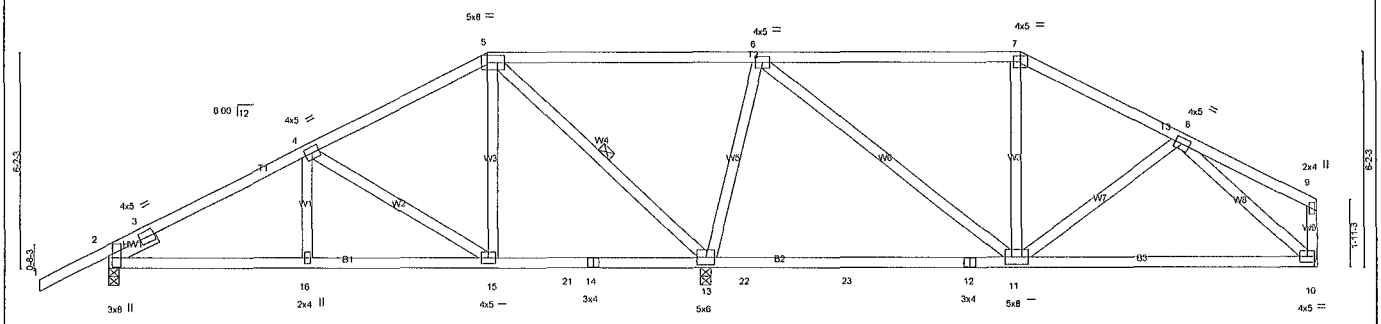
ID py4GRQ?ctP95F_bXGiNZsJz32tS-_L9R0KJ8Ef1wzQDixqthfh3ulSvLd_2V80_00yPtu9

-2-0-0	5-9-3	11-0-0	18-10-12	26-4-8	30-10-10	34-10-8
2-0-0	5-9-3	5-2-14	7 10-12	7-5-12	4-6-2	3-11 15

Scale = 1 62 5



5-9-3	11-0-0	17 2-12	26-4-8	34-10-8
5-9-3	5-2-14	6-2-12	9-1 12	8-6-0

Plate Offsets (X,Y). [2 0-4-0,0-1-3], [5.0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1 25	TC	0 73	Vert(LL)	-0 12	11 13	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 50	Vert(TL)	-0 20	10-11	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 60	Horz(TL)	0 02	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 195 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing Except
6-0-0 oc bracing· 11 13.
WEBS 1 Row at midpt 5-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=481/0-3-8 (min 0-1-8) 13=1254/0-3-8 (min 0-1 12) 10=383/Mechanical
Max Horz 2=124(LC 12)
Max Uplift2=-159(LC 12) 13=-301(LC 9) 10=-123(LC 13)
Max Grav 2=572(LC 27), 13=1320(LC 2) 10=486(LC 28)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-256/0 3-4=-643/359 4-5=-330/224 5-6=-65/296 6-7=-344/294 7-8=-431/273
BOT CHORD 2-16=-301/551 15-16=-301/551 10-11=-214/396
WEBS 4-15=-403/313 5-15=-127/302 5-13=-748/418, 6-13=-847/552 6-11=-276/538 8-10=-502/290

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=18ft; Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone·C-C for members and forces & MWFRS for reactions shown· Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5 0psf
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 2 301 lb uplift at joint 13 and 123 lb uplift at joint 10.
8) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB.
11) Truss Design Engineer· Julius Lee PE Florida P E. License No 34869· Address. 1109 Coastal Bay Blvd Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

October 25,2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T05	Hip Truss	1	1	I7422703
					Job Reference (optional)

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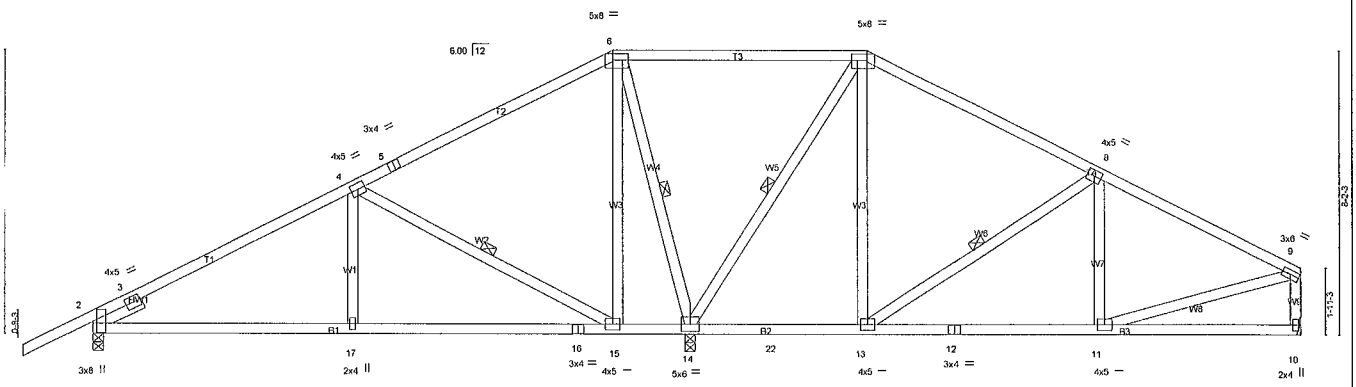
ID:py4GRQ?ctP95F_bXGiNZsJz32tS-xkHBR0KPnFvl9HacqMsLm4mQg599pbVLzSV44vyPtu7

-2-0-0	7-5-12	15-0-0	22-4-8	29-0-15	34-10-8
2-0-0	7-5-12	7-6-5	7-4-8	6-8-7	5-9-9

Scale 1:62.7



7-5-12	15-0-0	17-2-12	22-4-8	29-0-15	34-10-8
7-5-12	7-6-5	2-2-12	5-1-12	6-8-7	5-9-9

Plate Offsets (X,Y): [2,0-4-0,0-1-3], [6 0-5-8,0-2-4], [7 0-5-8,0-2-4]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plates Increase 1.25	TC 0.65	Vert(LL) 0 07 15-17 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1.25	BC 0.38	Vert(TL) -0 12 15-17 >999 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 34	Horz(TL) 0 02 10 n/a n/a	
BCDL 5 0	Code FBC2010/TPI2007	(Matrix-M)		Weight 209 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing Except 6-0-0 oc bracing 14-15.
WEBS 1 Row at midpt 4-15, 6-14 7 14 8-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=460/0-3-8 (min 0-1-8) 14=1189/0-3-8 (min 0-1 11) 10=392/Mechanical
Max Horz 2=149(LC 12)
Max Uplift 2=-167(LC 12), 14=-254(LC 12) 10=-138(LC 13)
Max Grav 2=577(LC 27) 14=1331(LC 2) 10=499(LC 28)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-328/54 3-4=-591/344 6-7=0/265, 7 8=-287/220 8-9=-560/316 9-10=-473/292
BOT CHORD 2 17=-269/478 16-17=-269/478 15-16=-269/478 12-13=-210/450 11 12=-210/450
WEBS 4-15=-641/484 6-15=-172/355, 6-14=-777/470 7 14=-753/418, 7 13=-142/331 8-13=-388/335
9-11=-188/427

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4.2psf BCDL=3.0psf h=18ft, Cat II Exp C Encl GCpi=0 18, MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1 60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5 0psf
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 2 254 lb uplift at joint 14 and 138 lb uplift at joint 10
8) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
11) Truss Design Engineer Julius Lee PE Florida P E License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
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Julius Lee PE,
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T07	Hip Truss	1	1	I7422705

Job Reference (optional)

Builders FirstSource Lake City FL 32055

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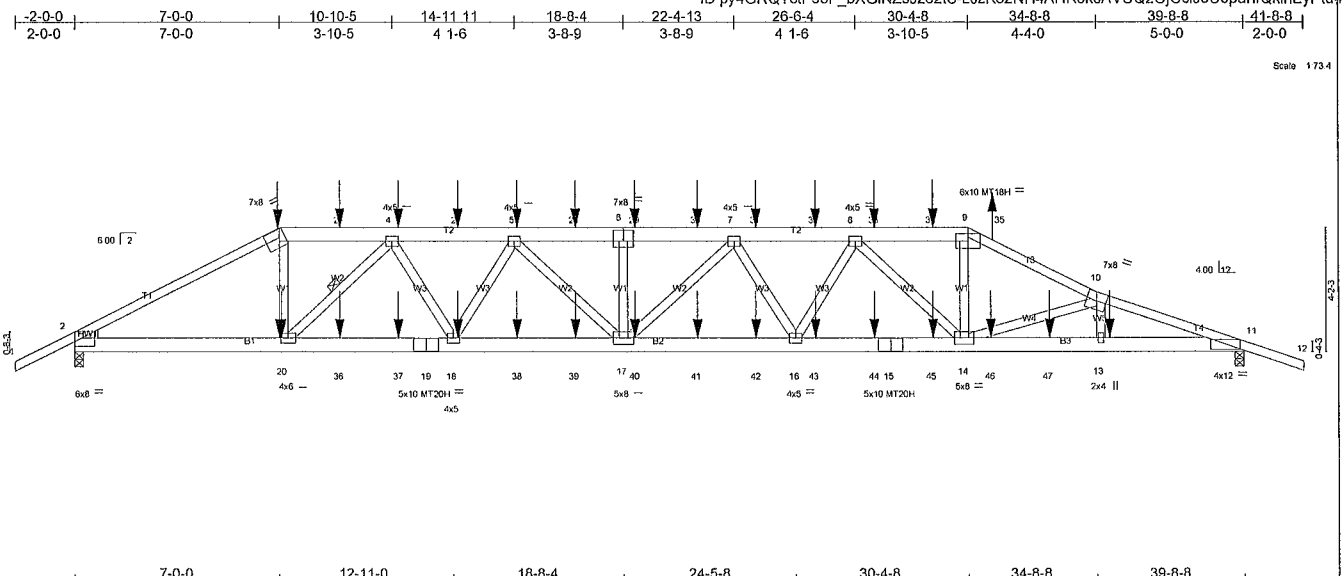

Plate Offsets (X,Y) [2 0-0-0,0-2-2], [3 0-5-14,0-3-4], [6.0-4-0,0-4-8], [9 0-5-0,0-1 7], [11 0-6-0,0-1 11]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0 99	Vert(LL)	0 77	16-17	>618	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 73	Vert(TL)	1 10	16-17	>433	180	MT20H	187/143
BCLL	0 0 *	Rep Stress Incr	NO	WB	0 96	Horz(TL)	0.24	11	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight 257 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2 *Except*
T1 2x4 SP M 31 T3,T4 2x4 SP No 1
BOT CHORD 2x6 SP SS
WEBS 2x4 SP No.3
WEDGE
Left. 2x4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 4-0-8 oc bracing
WEBS 1 Row at midpt 4-20

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2177/0-3-8 (min 0-3-1) 11=2290/0-3-8 (min 0-3-3)
Max Horz 2=58(LC 12)
Max Uplift2=-1532(LC 8) 11= 1459(LC 5)
Max Grav 2=2579(LC 2) 11=2716(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-4655/2999 3-26=-4134/2746 4 26=-4137/2747 4-27=-6318/4105 5-27=-6318/4105
5-28=-7191/4556, 6-28=-7191/4556, 6-29= 7191/4556 29-30=-7191/4556 7 30=-7191/4556
7-31=-6803/4206, 31-32=-6803/4206 8-32=-6803/4206 8-33=-5294/3104 33-34=-5290/3103
9-34=-5288/3103 9-35=-5767/3382 10-35=-5836/3368 10-11=-7587/4109
BOT CHORD 2-20=-2587/4067 20-36=-3613/5710 36-37=-3613/5710 19-37=-3613/5710 18-19=-3613/5710
18-38=-4244/6784 38-39=-4244/6784 17-39=-4244/6784 17-40=-4310/7098 40-41=-4310/7098
41-42=-4310/7098 16-42=-4310/7098 16-43=-3809/6437 43-44=-3809/6437 15-44=-3809/6437
15-45=-3809/6437 14-45=-3809/6437 14-46=-3849/7145 46-47=-3849/7145 13-47=-3849/7145
11 13=-3856/7174
WEBS 3-20=-1226/1760 4-20=-2296/1434 4-18=-813/1280 5-18=-982/537 5-17=-310/609
6-17=-339/195 7 16=-645/453 8-16=-625/793 8-14=-1680/1183, 9-14=-1414/2371
10-14=-2072/1015, 10-13=-77/326

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft; Cat. II Exp C, Encl GCpi=0 18, MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) All plates are MT20 plates unless otherwise indicated.
5) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
7) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1532 lb uplift at joint 2 and 1459 lb uplift at joint 11
9) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422706
530670	T08	Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055

7 350 s Sep 27 2012 MiTek Industries Inc. Fri Oct 25 11:42:34 2013 Page 1

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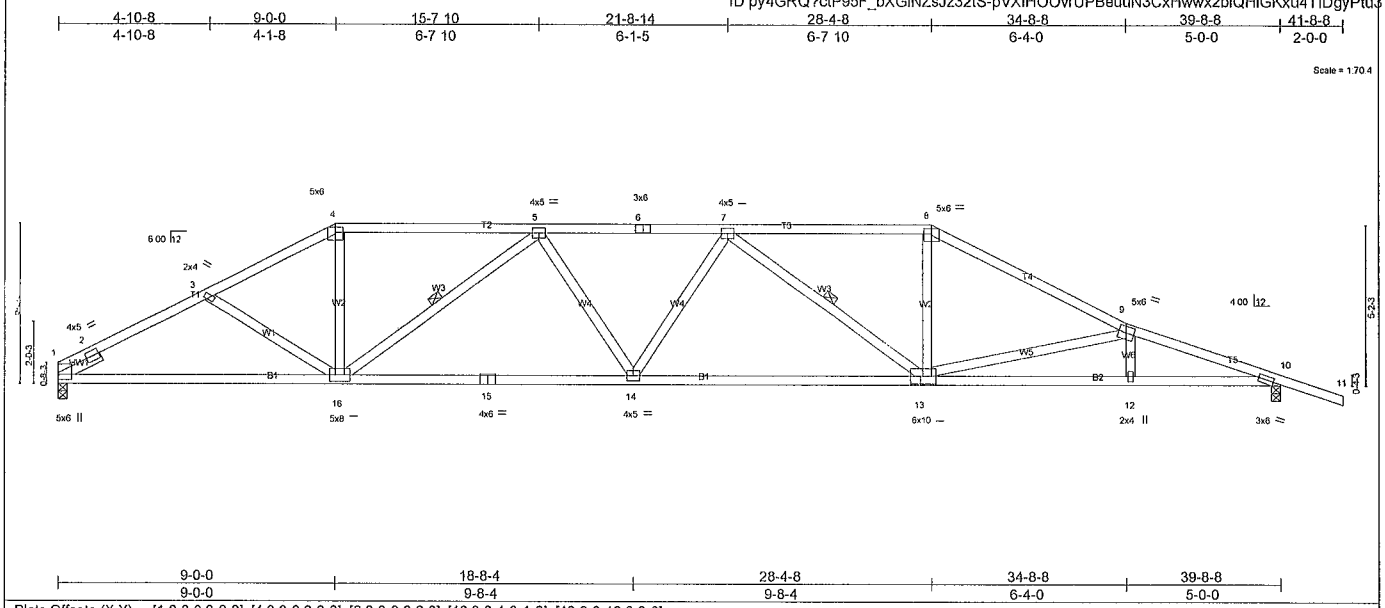


Plate Offsets (X,Y) [1.0-2-0,0-0-3], [4.0-3-0,0-2-0], [8.0-3-0,0-2-0], [10:0-3-1,0-1-8], [13 0-3-12,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plates Increase 1.25	TC 0 91	Vert(LL) 0 37 14-16 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 82	Vert(TL) -0.63 14-16 >762 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 92	Horz(TL) 0 18 10 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)		Weight. 200 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No.2 *Except*
B2. 2x4 SP No.1
WEBS 2x4 SP No 3
SLIDER Left 2x4 SP No 3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 4-9-3 oc bracing
WEBS 1 Row at midpt 5-16, 7 13

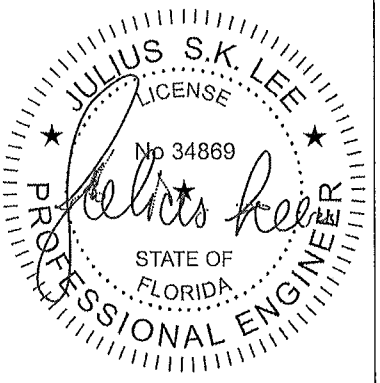
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1064/0-3-8 (min 0-1-9) 10=1169/0-3-8 (min 0-1 11)
Max Horz 1=-85(LC 13)
Max Uplift 1=-275(LC 9), 10=-351(LC 9)
Max Grav 1=1260(LC 2) 10=1389(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-808/305 2 3=-2289/1315, 3-4=-2139/1234, 4-5=-1926/1168, 5-6=-2642/1550
6-7=-2642/1550 7-8=-2224/1337 8-9=-2459/1380 9-10=-3346/1862
BOT CHORD 1 16=-1025/1961 15-16=-1254/2507 14 15=-1254/2507 13-14=-1300/2592 12-13=-1682/3133
10-12=-1675/3130
WEBS 4-16=-316/628, 5-16=-860/473, 7 13=-658/324 8-13=-330/706 9-13=-1058/683

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph; TCDL=4 2psf· BCDL=3 0psf· h=18ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown· Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 275 lb uplift at joint 1 and 351 lb uplift at joint 10
8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note· Visually graded lumber designation SPp represents new lumber design values as per SPIB.
11) Truss Design Engineer· Julius Lee PE. Florida P E License No. 34869· Address: 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T10	Hip Truss	1	1	I7422708
					Job Reference (optional)

Builders FirstSource Lake City FL 32055 — 7 350 s Sep 27 2012 MiTek Industries, Inc. Fri Oct 25 11:42 37 2013 Page 1

ID py4GRQ?ctP95F_bXGiNZsJz32tS-D4CrvPQo7PnmVMcykKU_YZZZlwSbykQNa1iyq?yPtu0

6-4-4	13-0-0	15-4-4	19-5-2	24-4-8	30-5-0	34-8-8	39-8-8	41-8-8
6-4-4	6-7 12	2-4-4	4-0-14	4-11-6	6-0-8	4-3-8	5-0-0	2-0-0

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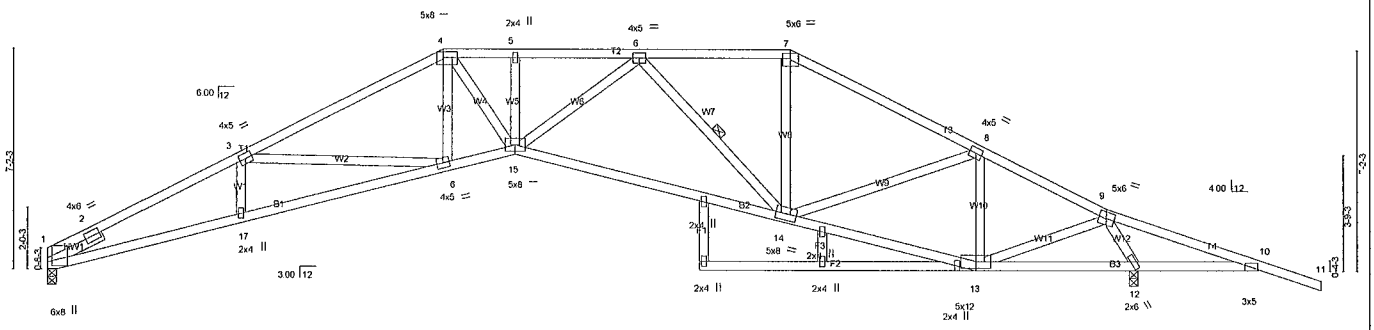


6-4-4	13-0-0	15-4-4	21-4-8	24-4-8	30-5-0	35-7-4	39-8-8
6-4-4	6-7 12	2-4-4	6-0-4	3-0-0	6-0-8	5-2-4	4-1-4

Plate Offsets (X,Y) [1 0-6-6,Edge], [4 0-5-8,0-2-4], [7:0-3-0,0-2-0], [10 0-0-5,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plates Increase 1 25	TC 0 92	Vert(LL) 0.40 16-17 >999 240	MT20 244/190
TCDL 7 0	Lumber Increase 1 25	BC 0 78	Vert(TL) -0 70 14-15 >615 180	
BCLL 0 0 *	Rep Stress Incr YES	WB 0 45	Horz(TL) 0 38 12 n/a n/a	
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)		Weight 224 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP No.1
BOT CHORD 2x4 SP No.2 *Except*
B1 2x4 SP M 31 F2: 2x4 SP No.3
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 2-0-0

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied or 5-11-6 oc bracing
WEBS 1 Row at midpt 6-14

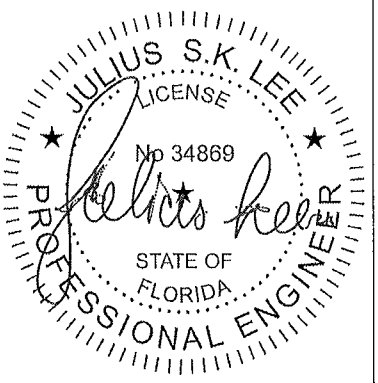
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=937/0-3-8 (min 0-1-8) 12=1296/0-3-8 (min 0-1 14)
Max Horz 1=-110(LC 13)
Max Uplift 1=-245(LC 12) 12=-412(LC 9)
Max Grav 1=1110(LC 2) 12=1540(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 1 2=-1315/622 2-3=-3352/1790 3-4=-2965/1489 4-5=-3192/1629 5-6=-3192/1629
6-7=-1669/957 7-8=-1895/990, 8-9=-1410/695 9-10=-1091/901
BOT CHORD 1 17=-1472/2975 16-17=-1489/3010 15-16= 1042/2609 14-15=-976/2426, 13-14=-451/1227
12-13=-20/495, 10-12=-830/1138
WEBS 3-16=-380/438, 4-15=-360/937 6-15=-365/951 6-14=-1149/571 7 14=-207/520 8-14=-152/461
8-13=-695/467 9-13=-912/1206 9-12=-1747/1288

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3.0psf· h=18ft, Cat. II Exp C, Encl GCpi=0 18· MWFRS (envelope) and C-C Exterior(2) zone, cantilever right exposed ·C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 1 and 412 lb uplift at joint 12
9) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
11) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB
12) Truss Design Engineer· Julius Lee PE Florida P E License No 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



October 25,2013

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Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component Applicability of design parameters and proper incorporation of component is responsibility of building designer not truss designer Bracing shown is for lateral support of individual web members only Additional temporary bracing to insure stability during construction is the responsibility of the erector Additional permanent bracing of the overall structure is the responsibility of the building designer For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422710
530670	T12	Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource, Lake City FL 32055 — 7 350 s Sep 27 2012 MiTek Industries, Inc. Fri Oct 25 11:42:40 2013 Page 1

ID py4GRQ?ctP95F_bXGiNZsJz32tS-efuzXRSgQK9KMpLXPS2hABBAq7R692BpG?wcQKyPttz

6-0-4	12-3-6	17-0-0	20-4-8	26-1-9	30-5-0	34-8-8	39-8-8	41-8-8
6-0-4	6-3-2	4-8-10	3-4-8	5-9-1	4-3-7	4-3-8	5-0-0	2-0-0

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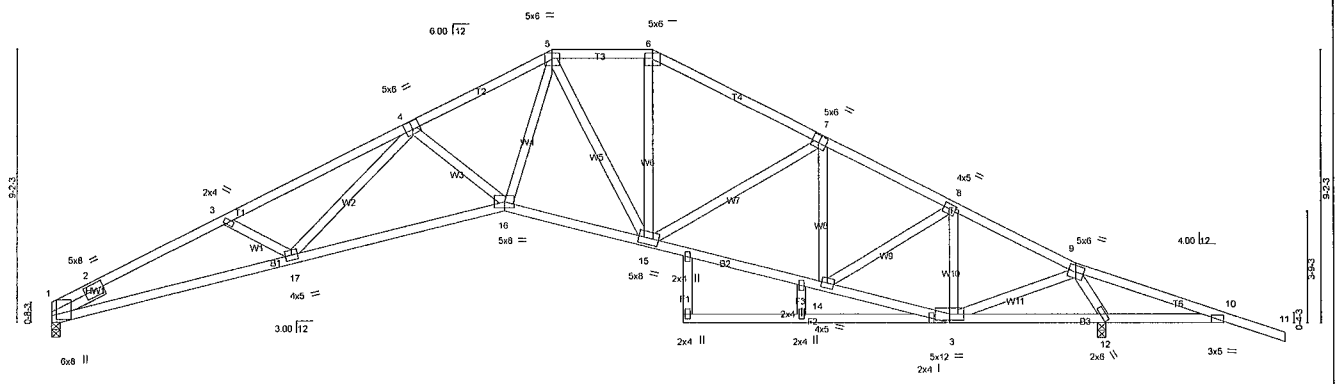

8-1-12	15-4-4	20-4-8	21-4-8	26-1-9	30-5-0	35-7-4	39-8-8
8-1-12	7-2-7	5-0-4	1-0-0	4-9-1	4-3-7	5-2-4	4-1-4

Plate Offsets (X,Y) [1.0-4-13,Edge], [4 0-3-0,0-3-4], [5:0-3-0,0-2-0], [6:0-3-0,0-2-0], [7.0-3-0,0-3-0], [10.0-0-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0 54	Vert(LL)	0.40	16-17	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 92	Vert(TL)	-0.67	16-17	>640	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 58	Horz(TL)	0 35	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight 233 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T1 2x4 SP M 31
BOT CHORD 2x4 SP No.2 *Except*
B1 2x4 SP No 1 F2 2x4 SP No 3
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No 3 2-0-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing
JOINTS 1 Brace at Jt(s) 14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=937/0-3-8 (min. 0-1-8) 12=1295/0-3-8 (min 0-1 14)
Max Horz 1=-137(LC 8)
Max Uplift 1=-268(LC 12) 12=-386(LC 13)
Max Grav 1=1110(LC 2) 12=1539(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 1 2=-1030/476 2-3=-3425/1879 3-4=-3234/1761 4-5=-2707/1435 5-6=-1632/989
6-7=-1853/1033 7-8=-1852/1011 8-9=-1414/703 9-10= 1080/882
BOT CHORD 1 17=-1553/3039 16-17=-1203/2742 15-16=-611/1830 14-15=-638/1622 13-14=-448/1216
12 13=-34/473 10-12=-811/1126
WEBS 3-17=-172/265 4-17=-191/296 4-16=-517/454 5-16=-747/1592 5-15=-604/232 6-15=-261/546
7 14=-297/203 8-14=-234/477 8-13=-684/454 9-13=-885/1171 9-12=-1759/1308

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft; Cat. II Exp C, Encl GCpi=0 18 MWFRS (envelope) and C-C Exterior(2) zone cantilever right exposed C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 268 lb uplift at joint 1 and 386 lb uplift at joint 12
9) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
11) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
12) Truss Design Engineer Julius Lee PE. Florida P E. License No 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
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Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

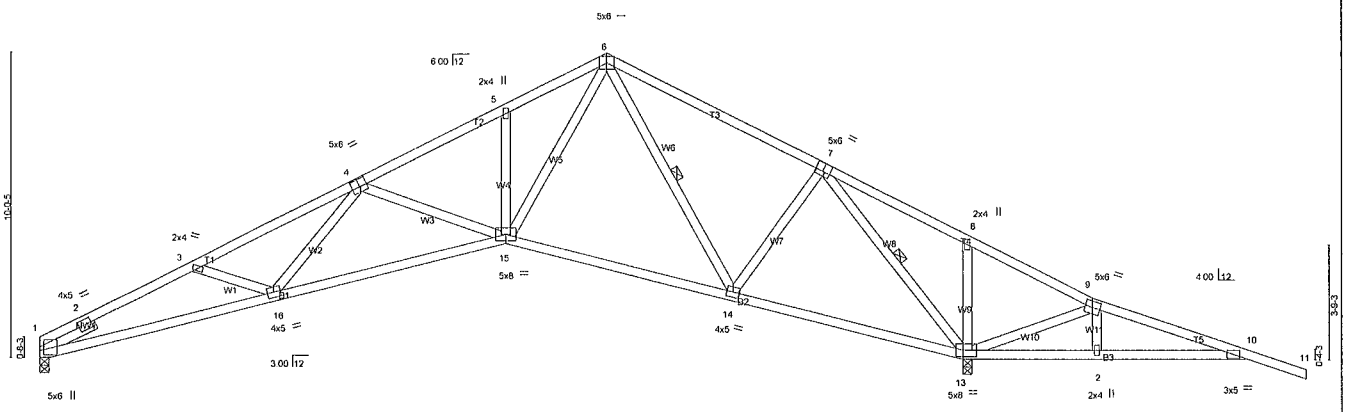
Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T14	Special Truss	2	1	I7422712
					Job Reference (optional)

Builders FirstSource Lake City FL 32055 — 7.350 s Sep 27 2012 MiTek Industries Inc. Fri Oct 25 11.42.43 2013 Page 1

ID py4GRQ?ctP95F_bXGiNZsJz32tS-2EZ69TVZjFYvDH454bbPoqpcgLWSMP3Fyz9G1fyPttw

5-1-11	10-6-12	15-4-4	18-8-4	25-8-8	30-5-0	34-8-8	39-8-8	41-8-8
5-1-11	5-5-0	4-9-8	3-4-0	7-0-4	4-8-8	4-3-8	5-0-0	2-0-0

Scale 1:71.4



7-8-7	15-4-4	22-10-0	30-5-0	34-8-8	39-8-8
7-8-7	7-7-12	7-5-12	7-7-0	4-3-8	5-0-0

Plate Offsets (X,Y) [1 0-2-11,0-1-9], [4 0-3-0,0-3-0], [7.0-3-0,0-3-4], [13.0-5-4,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	0.24	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.49	15-16	>745	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.24	13	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 211 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 2-0-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing
WEBS 1 Row at midpt 6-14, 7-13

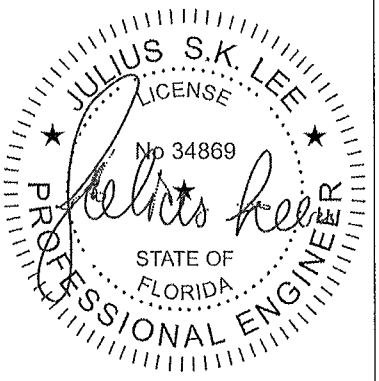
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=714/0-3-8 (min. 0-1-8), 13=1518/0-3-8 (min. 0-2-3)
Max Horz 1=-148(LC 8)
Max Uplift 1=-234(LC 12), 13=-506(LC 9)
Max Grav 1=845(LC 2), 13=1804(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown
TOP CHORD 1-2=-728/288, 2-3=-2526/1184, 3-4=-2343/1018, 4-5=-1672/569, 5-6=-1665/673, 6-7=-736/249, 7-8=-1301/1175, 8-9=-1409/1149, 9-10=-1055/878
BOT CHORD 1-16=-937/2232, 15-16=-600/1982, 14-15=-85/740, 13-14=-81/647, 12-13=-807/1120, 10-12=-804/1101
WEBS 3-16=-158/281, 4-16=-148/292, 4-15=-601/516, 6-15=-519/1311, 6-14=-549/424, 7-14=-280/524, 7-13=-1902/1409, 9-13=-212/403

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 1 and 506 lb uplift at joint 13.
8) Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE. Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



October 25,2013

WARNING *Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.*
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive; Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK
530670	T16	Hip Truss	1	1	I7422714
					Job Reference (optional)

Builders FirstSource Lake City FL 32055 — 7 350 s Sep 27 2012 MiTek Industries, Inc. Fri Oct 25 11:42:45 2013 Page 1

ID py4GRQ?ctP95F_bXGiNZsJz32tS-_dhsa8WpFsodSaDUC?etlFuyh8E1qQGYPHeN5XyPttu

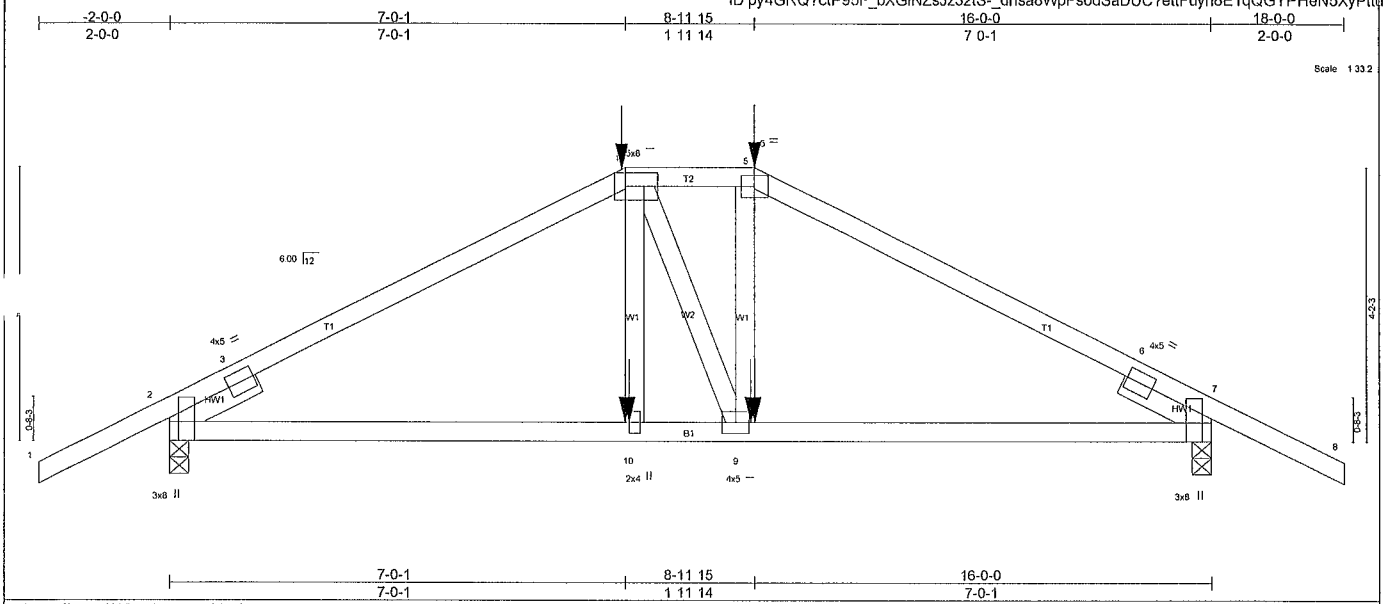


Plate Offsets (X,Y): [2 0-4-4,Edge], [4.0-6-0,0-2-8], [7:0-4-4,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20 0	Plates Increase 1.25	TC 0 79	Vert(LL) 0 11 9-10 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0 62	Vert(TL) -0.10 10-13 >999 180	
BCLL 0 0 *	Rep Stress Incr NO	WB 0 14	Horz(TL) -0 05 7 n/a n/a	
BCDL 5 0	Code FBC2010/TPI2007	(Matrix-M)		Weight. 78 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0 Right 2x4 SP No 3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-7 oc purlins
BOT CHORD Rigid ceiling directly applied or 5-4-7 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=857/0-3-8 (min 0-1-8) 7=868/0-3-8 (min. 0-1 8)
Max Horz 2=56(LC 12)
Max Uplift 2=-708(LC 5) 7=-719(LC 4)
Max Grav 2=1017(LC 2) 7=1030(LC 2)

FORCES (lb) Max Comp./Max Ten. All forces 250 (lb) or less except when shown
TOP CHORD 2 3=-611/598, 3-4=-1443/1184 4-5=-1247/1109 5-6= 1470/1213, 6-7=-613/620
BOT CHORD 2-10=-1014/1215 9-10=-1028/1225, 7-9=-1014/1239
WEBS 4-10=-383/374 5-9=-329/359

NOTES (11 13)
1) Unbalanced roof live loads have been considered for this design
2) Wind. ASCE 7 10, Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft Cat. II Exp C Encl GCpi=0 18; MWFRS (envelope); porch left and right exposed Lumber DOL=1 60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 708 lb uplift at joint 2 and 719 lb uplift at joint 7
8) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 103 lb up at 7-0-1 and 246 lb down and 262 lb up at 8-11 15 on top chord and 269 lb down and 362 lb up at 7-0-0, and 269 lb down and 362 lb up at 8-11-4 on bottom chord The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note. Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer Julius Lee PE. Florida P E. License No. 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard
1) Regular Lumber Increase=1 25 Plate Increase=1 25
Uniform Loads (plf)
Vert: 1-4=-44 4-5=-44 5-8=-44 11 15=-10
Concentrated Loads (lb)
Vert. 4=-83(B) 5=-170(B) 10=-216(B) 9=-216(B)

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

October 25,2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422716
530670	T18	Common Truss	2	1	Job Reference (optional)	

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ID py4GRQ?ctP95F_bXGlNZsJz32tS-x?pd?qY3nT2LiuNtJQgLyg_P6ywWlKHrtb7UAQyPtts

2-0-0	4-8-10	9-6-0	14-3-6	19-0-0
2-0-0	4-8-10	4-9-6	4-9-6	4-8-10

Scale 1 34 5

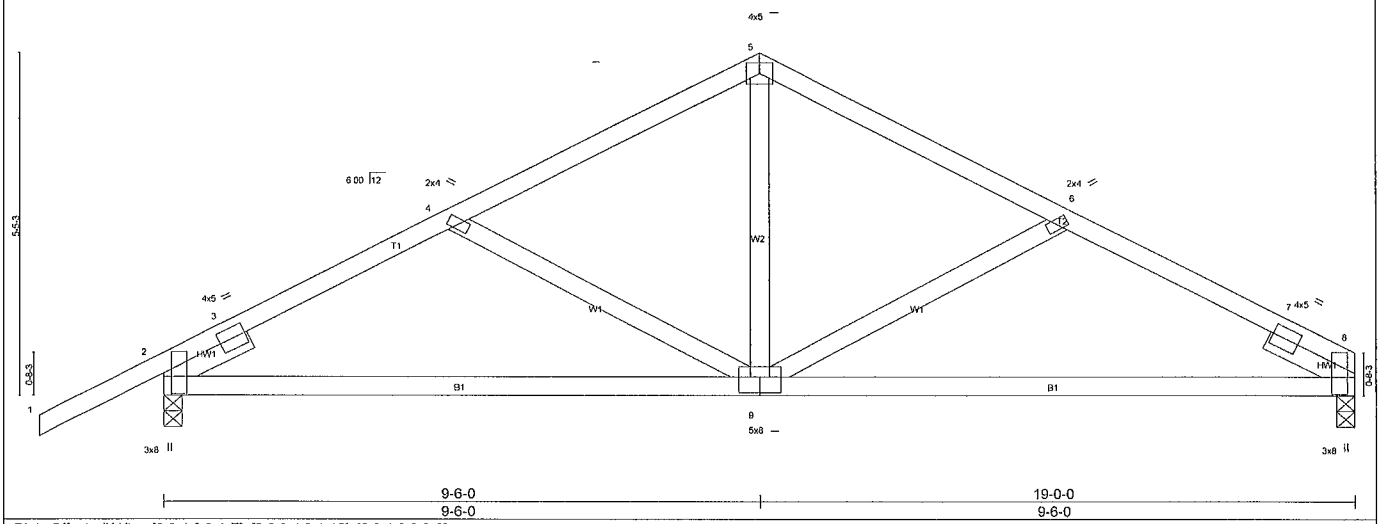


Plate Offsets (X,Y) [2 0-4-0,0-1 7], [6,0-3-4,0-1 15], [9,0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1 25	TC	0 35	Vert(LL)	-0 11	9-12	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 55	Vert(TL)	-0 19	9-12	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 17	Horz(TL)	0.03	8	n/a	n/a		
BCDL	5 0	Code FBC2010/TPI2007		(Matrix-M)							Weight 91 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0 Right 2x4 SP No.3 1-6-0

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-5 oc purlins
BOT CHORD Rigid ceiling directly applied or 8-6-2 oc bracing

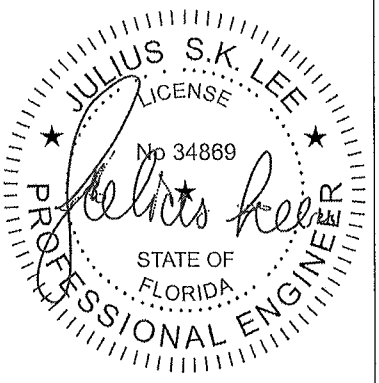
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=508/0-3-8 (min 0-1-8) 2=606/0-3-8 (min 0-1-8)
Max Horz 2=141(LC 12)
Max Uplift8=-260(LC 13) 2=-331(LC 12)
Max Grav 8=602(LC 2) 2=722(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-580/0 3-4=-987/601 4-5=-760/470 5-6=-761/471 6-7=-999/614 7 8=-622/130
BOT CHORD 2-9=-452/831 8-9=-471/847
WEBS 5-9=-190/371 6-9=-295/282 4-9=-277/260

NOTES (8-10)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10 Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf BCDL=3 0psf h=18ft, Cat. II Exp C, Encl. GCpi=0 18, MWFRS (envelope) gable end zone and C-C Exterior(2) zone C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1 60
3) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 8 and 331 lb uplift at joint 2
7) Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
9) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB.
10) Truss Design Engineer Julius Lee PE Florida P E License No 34869 Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard



October 25,2013

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

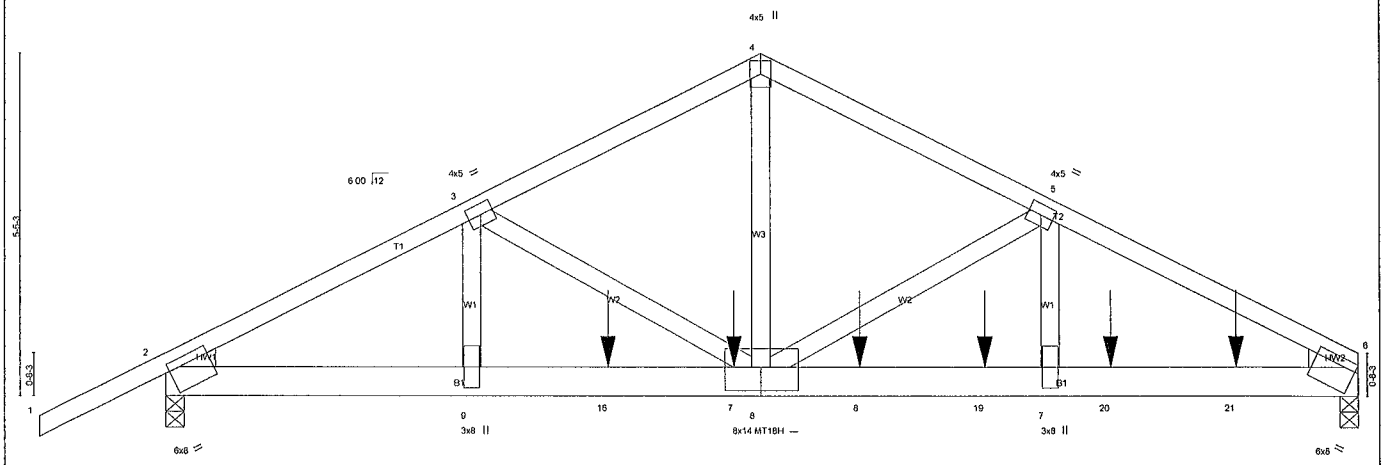
Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS CANNON CREEK	I7422718
530670	T19	Common Truss	1	1	Job Reference (optional)	

Builders FirstSource Lake City FL 32055 — 7.350 s Sep 27 2012 MiTek Industries, Inc. Fri Oct 25 11:42:50 2013 Page 1

ID py4GRQ?ctP95F_bXGiNZsJz32tS-LaUldsay3OQwZL6S?ZD2albr49wTVV8HZZL8mlyPttp

-2-0-0	4-10-12	9-6-0	14-1-4	19-0-0
2-0-0	4-10-12	4-7-4	4-7-4	4-10-12

Scale 1:34.5



4-10-12	9-6-0	14-1-4	19-0-0
4-10-12	4-7-4	4-7-4	4-10-12

Plate Offsets (X,Y) [2 0-0-12,0-1-8], [6 Edge,0-1-8], [8.0-7-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20 0	Plates Increase	1.25	TC	0 69	Vert(LL)	-0 16	8-9	>999	240	MT20	244/190
TCDL	7 0	Lumber Increase	1.25	BC	0 66	Vert(TL)	-0.29	8-9	>782	180	MT18H	244/190
BCLL	0 0 *	Rep Stress Incr	NO	WB	0 93	Horz(TL)	0 05	6	n/a	n/a		
BCDL	5 0	Code FBC2010/TPI2007		(Matrix-M)							Weight. 110 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP SS
WEBS 2x4 SP No.3
WEDGE
Left 2x4 SYP No.3 Right· 2x4 SYP No 3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-6-9 oc purlins
BOT CHORD Rigid ceiling directly applied or 6-9-14 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=2158/0-3-8 (min 0-3-1) 2=1709/0-3-8 (min 0-2-6)
Max Horz 2=86(LC 12)
Max Uplift6=-836(LC 9) 2=-768(LC 8)
Max Grav 6=2577(LC 2), 2=2032(LC 2)

FORCES (lb) Max Comp./Max Ten All forces 250 (lb) or less except when shown
TOP CHORD 2-3=-3540/1397 3-4=-3077/1163 4-5=-3090/1162 5-6=-4211/1388
BOT CHORD 2-9=-1250/3097 9-16=-1250/3097 16-17= 1250/3097 8-17=-1250/3097 8-18=-1188/3705
18-19=-1188/3705 7 19= 1188/3705 7-20=-1188/3705 20-21=-1188/3705 6-21=-1188/3705
WEBS 4-8=-936/2450 5-8=-1179/336, 5-7=-172/880 3-8=-470/342, 3-9=-195/286

NOTES (11 13)
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7 10· Vult=130mph (3-second gust) Vasd=101mph TCDL=4 2psf· BCDL=3 0psf· h=18ft; Cat. II Exp C Encl GCpi=0 18 MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated
4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=836, 2=768
8) 'Semi-rigid pitchbreaks including heels Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 904 lb down and 582 lb up at 7-0-12 465 lb down and 152 lb up at 9-0-12 476 lb down and 133 lb up at 11-0-12 484 lb down and 144 lb up at 13-0-12 and 489 lb down and 148 lb up at 15-0-12 and 468 lb down and 137 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
10) In the LOAD CASE(S) section loads applied to the face of the truss are noted as front (F) or back (B)
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
12) Note Visually graded lumber designation SPp represents new lumber design values as per SPIB
13) Truss Design Engineer· Julius Lee PE. Florida P E. License No. 34869· Address. 1109 Coastal Bay Blvd Boynton Beach FL 33435

LOAD CASE(S) Standard
1) Regular· Lumber Increase=1 25 Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44 4-6=-44 10-13=-10

Continued on page 2

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October 25,2013

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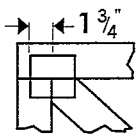
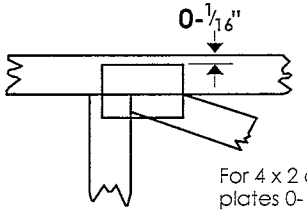
1109 Coastal Bay
Boynton Beach,FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated
Dimensions are in ft-in-sixteenths
Apply plates to both sides of truss and fully embed teeth



For 4 x 2 orientation locate plates 0- 1/16 from outside edge of truss

This symbol indicates the required direction of slots in connector plates

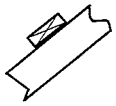
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

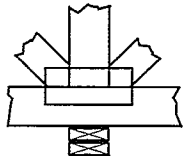
The first dimension is the plate width measured perpendicular to slots Second dimension is the length parallel to slots

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output Use T 1 or Eliminator bracing if indicated

BEARING

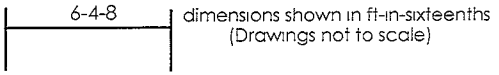


Indicates location where bearings (supports) occur Icons vary but reaction section indicates joint number where bearings occur

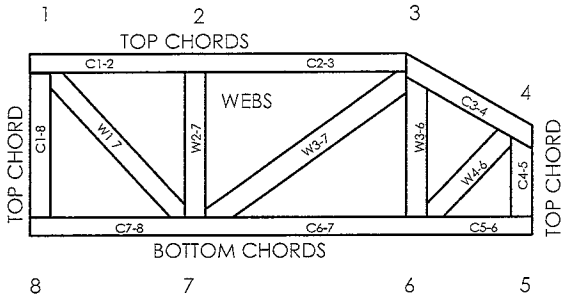
Industry Standards:

ANSI/TPI1 National Design Specification for Metal Plate Connected Wood Truss Construction
DSB-89 Design Standard for Bracing
BCSI1 Building Component Safety Information Guide to Good Practice for Handling Installing & Bracing of Metal Plate Connected Wood Trusses

Numbering System



dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC ES Reports

ESR 1311 ESR-1352, ER-5243 9604B
9730 95-43 96-31 9667A
NER-487 NER-561
95110 84-32, 96-67 ER-3907 9432A

Julius Lee PE
1109 Coastal Bay ,
Boynton Beach ,FL 33435

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1 Additional stability bracing for truss system e.g diagonal or X-bracing is always required See BCSI1

2 Truss bracing must be designed by an engineer For wide truss spacing individual lateral braces themselves may require bracing, or alternative T 1, or Eliminator bracing should be considered

3 Never exceed the design loading shown and never stack materials on inadequately braced trusses.

4 Provide copies of this truss design to the building designer erection supervisor property owner and all other interested parties

5 Cut members to bear tightly against each other

6 Place plates on each face of truss at each joint and embed fully Knots and wane at joint locations are regulated by ANSI/TPI 1

7 Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1

8 Unless otherwise noted moisture content of lumber shall not exceed 19% at time of fabrication

9 Unless expressly noted this design is not applicable for use with fire retardant preservative treated or green lumber

10 Camber is a non-structural consideration and is the responsibility of truss fabricator General practice is to camber for dead load deflection

11 Plate type size orientation and location dimensions indicated are minimum plating requirements.

12 Lumber used shall be of the species and size and in all respects equal to or better than that specified

13 Top chords must be sheathed or purlins provided at spacing indicated on design

14 Bottom chords require lateral bracing at 10 ft spacing or less if no ceiling is installed unless otherwise noted

15 Connections not shown are the responsibility of others

16 Do not cut or alter truss member or plate without prior approval of an engineer

17 Install and load vertically unless indicated otherwise

18 Use of green or treated lumber may pose unacceptable environmental health or performance risks. Consult with project engineer before use

19 Review all portions of this design (front back, words and pictures) before use Reviewing pictures alone is not sufficient

20 Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria

August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

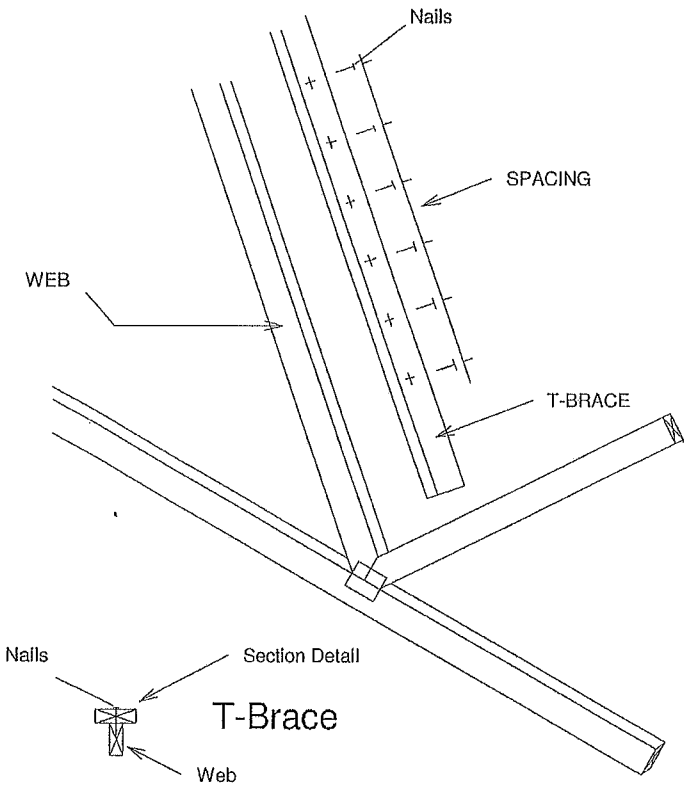
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o c.
Note Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

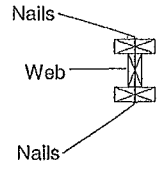
	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

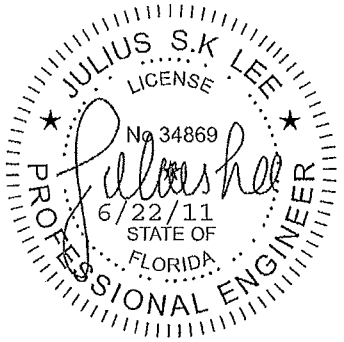
T-Brace / I-Brace must be same species and grade (or better) as web member.



T-Brace



I-Brace



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JANUARY 1, 2009	LATERAL TOE-NAIL DETAIL	ST-TOENAIL_SP

MiTek Industries, Inc

NOTES
1 TOE NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN
2 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD
3 ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

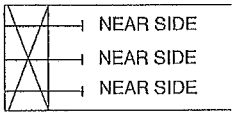
THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM	SYP	DF	HF	SPF	SPF-S
3.5" LONG	131	88 0	80 6	69.9	68 4	59 7
	135	93 5	85 6	74 2	72.6	63 4
	162	108 8	99 6	86 4	84 5	73 8
3.25" LONG	128	74 2	67.9	58.9	57 6	50 3
	.131	75 9	69 5	60 3	59 0	51 1
	148	81 4	74 5	64.6	63.2	52 5

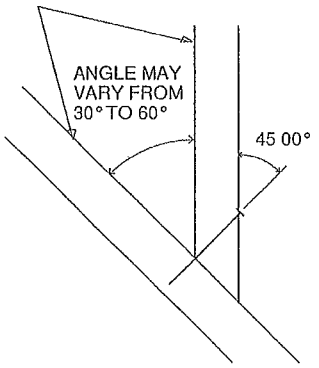
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

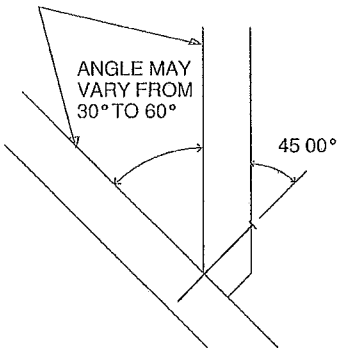
SIDE VIEW

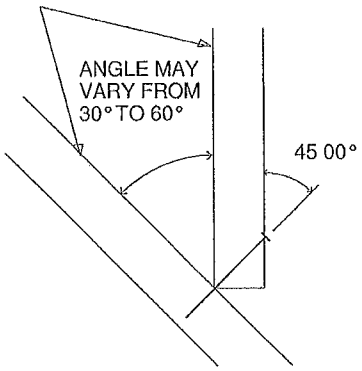


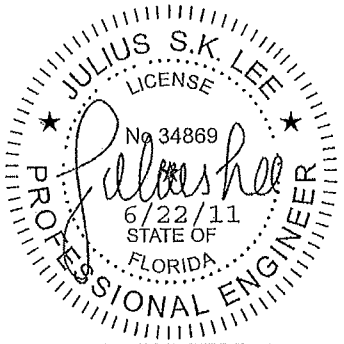
EXAMPLE
(3) 16d NAILS (162" diam x 3 5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1 15
3 (nails) X 84 5 (lb/nail) X 1 15 (DOL) = 291 5 lb Maximum Capacity









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FEBRUARY 14, 2012	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY-7-10

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C CATEGORY II BUILDING EXPOSURE B or C ASCE 7 10 DURATION OF LOAD INCREASE 1 60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES) ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED

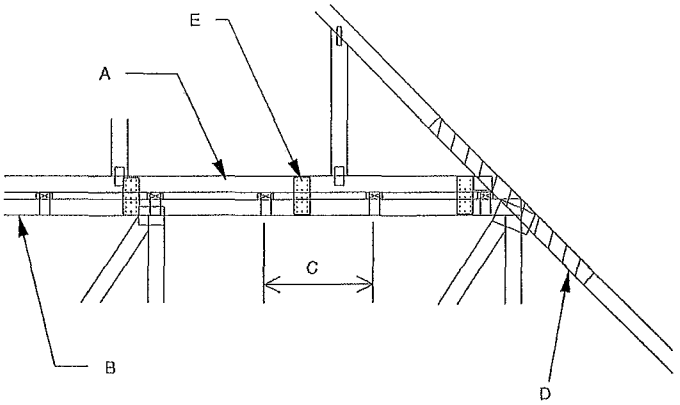
A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0 131" X 3.5 TOE NAILED

B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24 O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131 X 3.5" NAILS EACH.

D 2 X__ X 4' 0' SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4 O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT IN BOTH DIRECTIONS AND
1 WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft

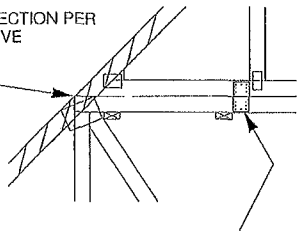
E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C W/ (4) 0 131 X 1.5 PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5' EDGE DISTANCE. (MIN 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



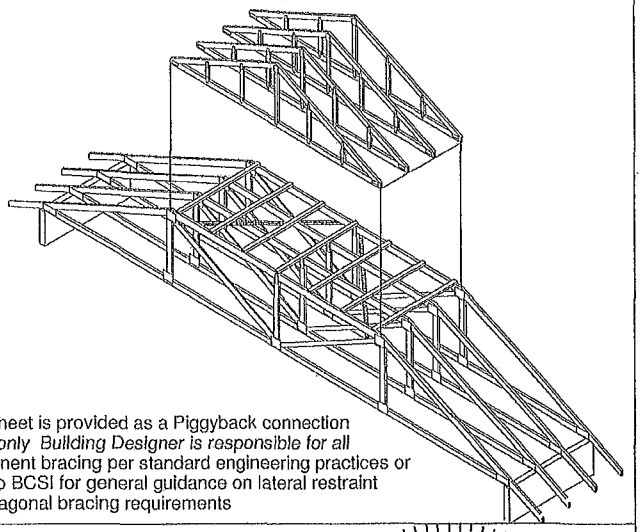
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

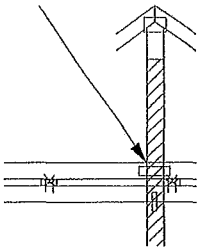


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48' O C W/ (4) 0 131" X 1.5 PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5' EDGE DISTANCE.



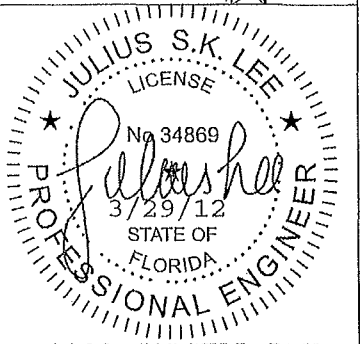
This sheet is provided as a Piggyback connection *detail only Building Designer is responsible for all* permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4 0' SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0 131" X 3") NAILS SPACED 4 O C FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) *THIS CONNECTION IS ONLY VALID FOR A MAXIMUM* CONCENTRATED LOAD OF 4000 LBS (@1 15) REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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FEBRUARY 14, 2012	Standard Gable End Detail	SHEET 2

MiTek Industries, Chesterfield, MO

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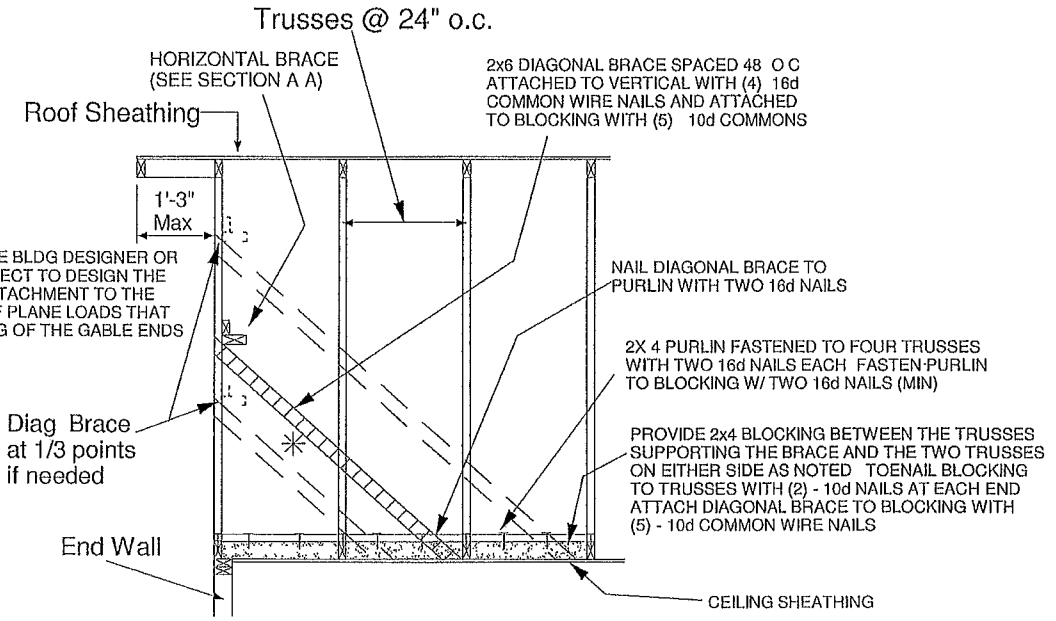
ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED

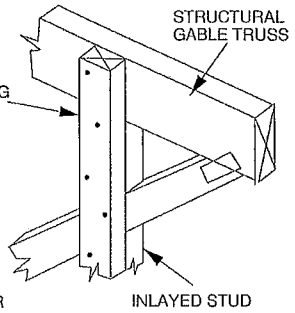
METHOD 1 ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 ATTACH 2X_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

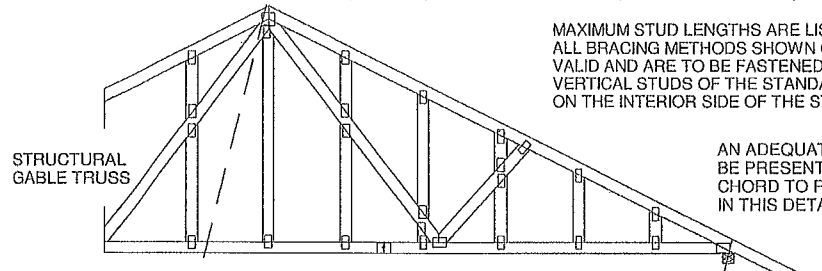
NAILING SCHEDULE

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131 X 3") NAILS SPACED 6 O C

FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7 10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3') NAILS SPACED 6 O C (2X 4 STUDS MINIMUM)

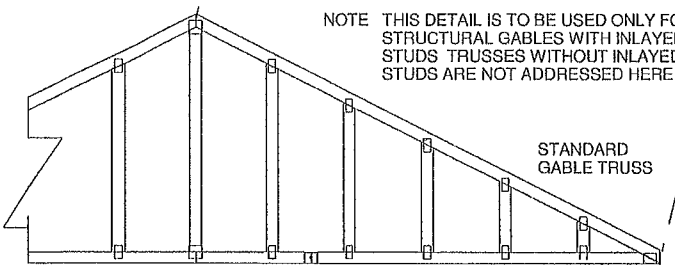


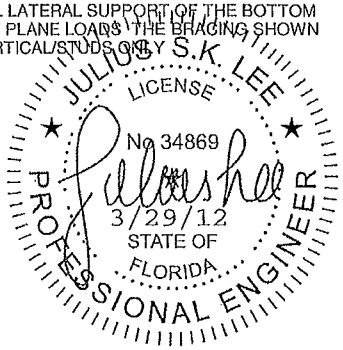
MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1 ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY

NOTE THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE





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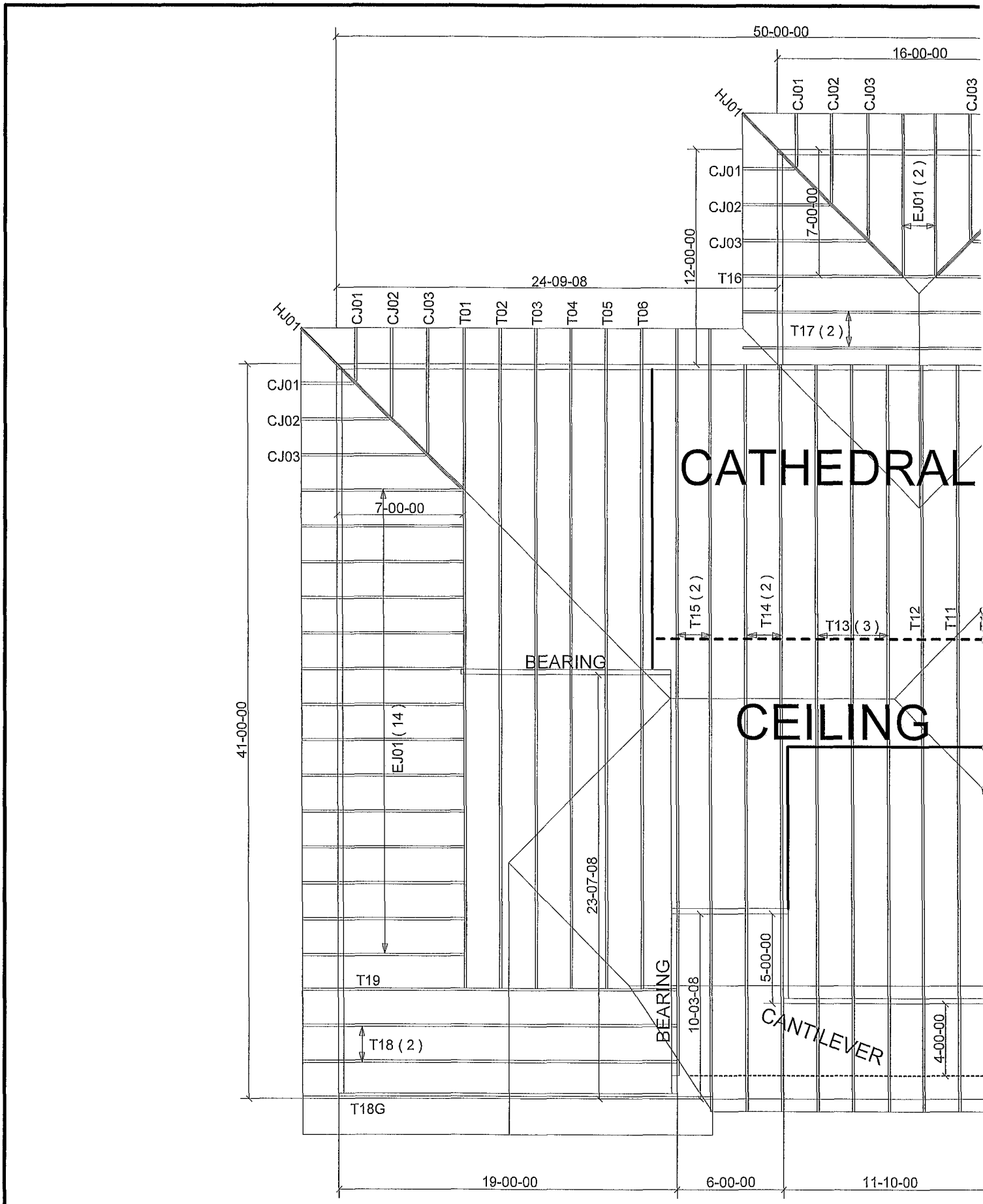

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERH