

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT

3" MIN. SPACING WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C. 1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C.

1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W 3/16" TAPCONS @ 12" O.C.

1/4" TAPCONS @ 18" O.C. GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C. (2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C. (2) 1/4" TAPCONS & 12" O.C.

STRUCTURAL PLAN NOTES

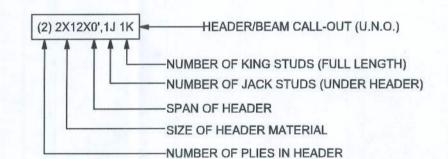
ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

HEADER LEGEND



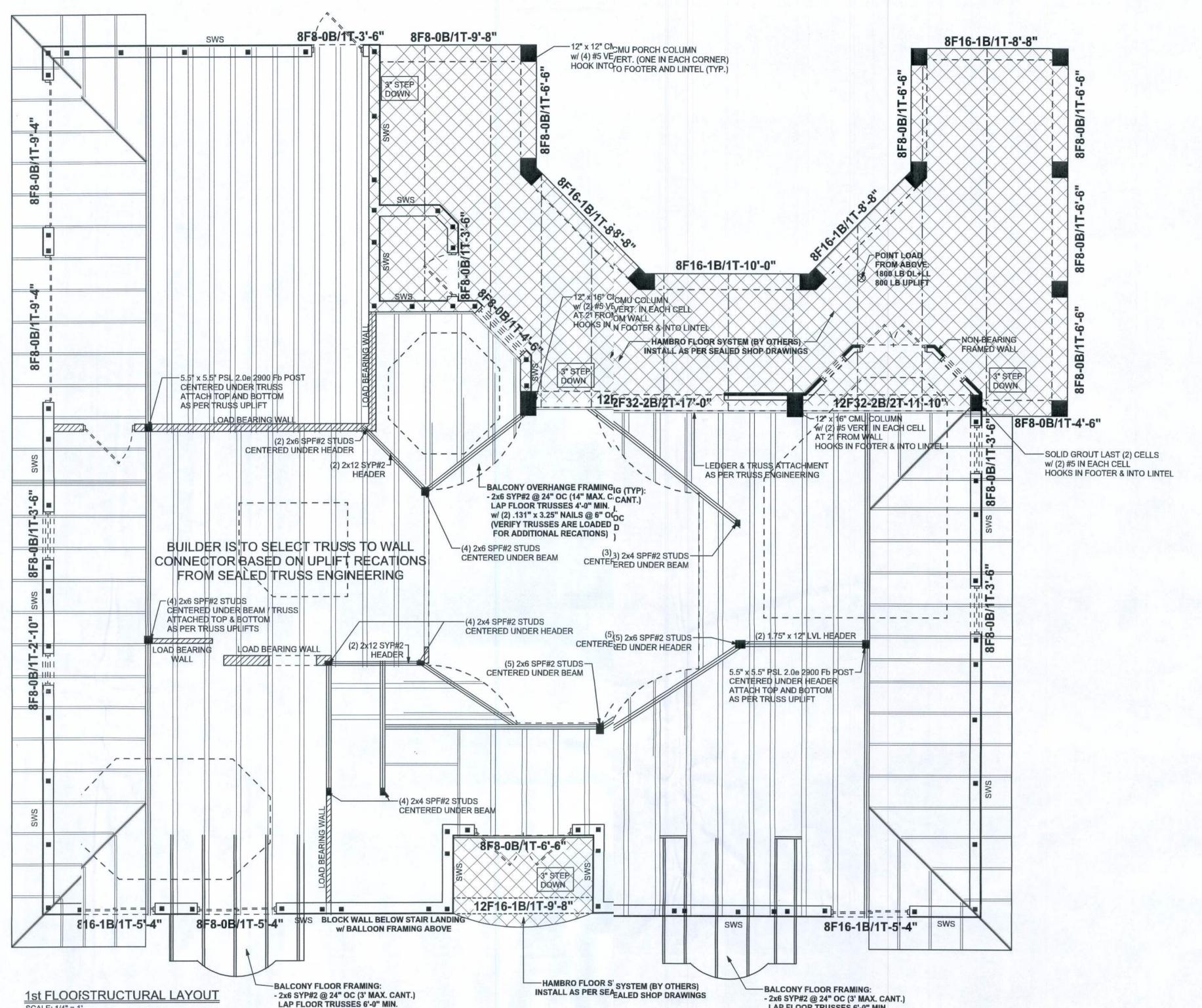
TOTAL SHEAR WALL SEGMENTS

SWS = INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	47.4'	80.5'
LONGITUDINAL	47.4'	56.3'

2ND FLOOR WALL LEGEND

 EXTERIOR FRAMED WALL
BLOCK WALL
INTERIOR LOAD BEARING WALL w/ NO UPLIFT
INTERIOR LOAD BEARING WALL w/ UPLIFT



w/ (2) .131" x 3.25" NAILS @ 6" OC

(VERIFY TRUSSES ARE LOADED

FOR ADDITIONAL RECATIONS)

LAP FLOOR TRUSSES 6'-0" MIN.

(VERIFY TRUSSES ARE LOADED

FOR ADDITIONAL RECATIONS)

w/ (2) .131" x 3.25" NAILS @ 6" OC

SINCE THE TRUSS ENGINEERING WAS NOT COMPLETE T THE TIME OF THIS SEAL BUILDER IS TO FURNISH SEALED SHOP DRAWINGS OF TRUSSES, PLACEMENT PLANS, CONECTION DETAILS, LEDGER OR SILL CONNECTIONS TO WALL, CURBS AND REINFORCEMENT, LEDGER TO BEAM AND WALL CONNECTIONS. DO NOT ORDER MATERIALS OR DO ANY WORK PRIOR TO APPROVAL OF THE SHOP DRAWINGS BY ENGINEER OF RECORD AND BUILDING DEPARTMENT.

REVISONS SOFTPIAN

Mark Disoswa, PE No.53915, POI 868, Lake City, FL 32056, 386-754-5419 Stated dimensons supercede scaled dimensions. Rifer all questions to Mark Disoswa; P.E. for resolution. Do not procee without clarification. COPYRIGHTSAND PROPERTY RIGHTS: Mark Disosway P.E. hereby expressly reserves its common law copyrights and property right i these instruments of service. This documents not to be reproduced, altered or copied in an form or manner without first the express wrten permission and consent CERTIFICATION: I hereby certify that I have examined this lan, and that the applicable portions of the lan, relating to wind engineering comply with section R301.2.1, floria building code residential 200 and 2009 Supplements, to the best of ny knowledge. LIMITATION: his design is valid for one building, at spelfied location. MRK DISOSWAY P.E. 53915

> Chris & Dora **Martin**

SEAL

ADDRESS: Columbia County, Florida

MarkDisosway P.E. PO. Box 868 Lake Cty, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

IRINTED DATE: Apil 20, 2010 DRAWN BY: STRUCTURAL BY: David Disosvay David Disosway

FINALS DA'E: Dec. 30, 209

> JOB NUMBER: 811211 DRAWING NUMBER

> > **S-3** F 11 SHEETS

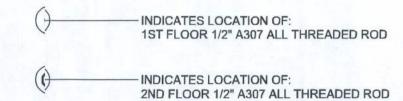


SRUCTURAL PLAN NOTES

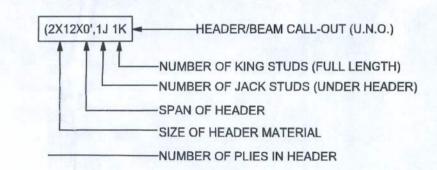
- S-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.N.O.)
- S-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- S-3
 DIMENSIONS ON STRUCTURAL SHEETS
 ARE NOT EXACT. REFER TO ARCHITECTURAL
 FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

TREADED ROD LEGEND



HADER LEGEND



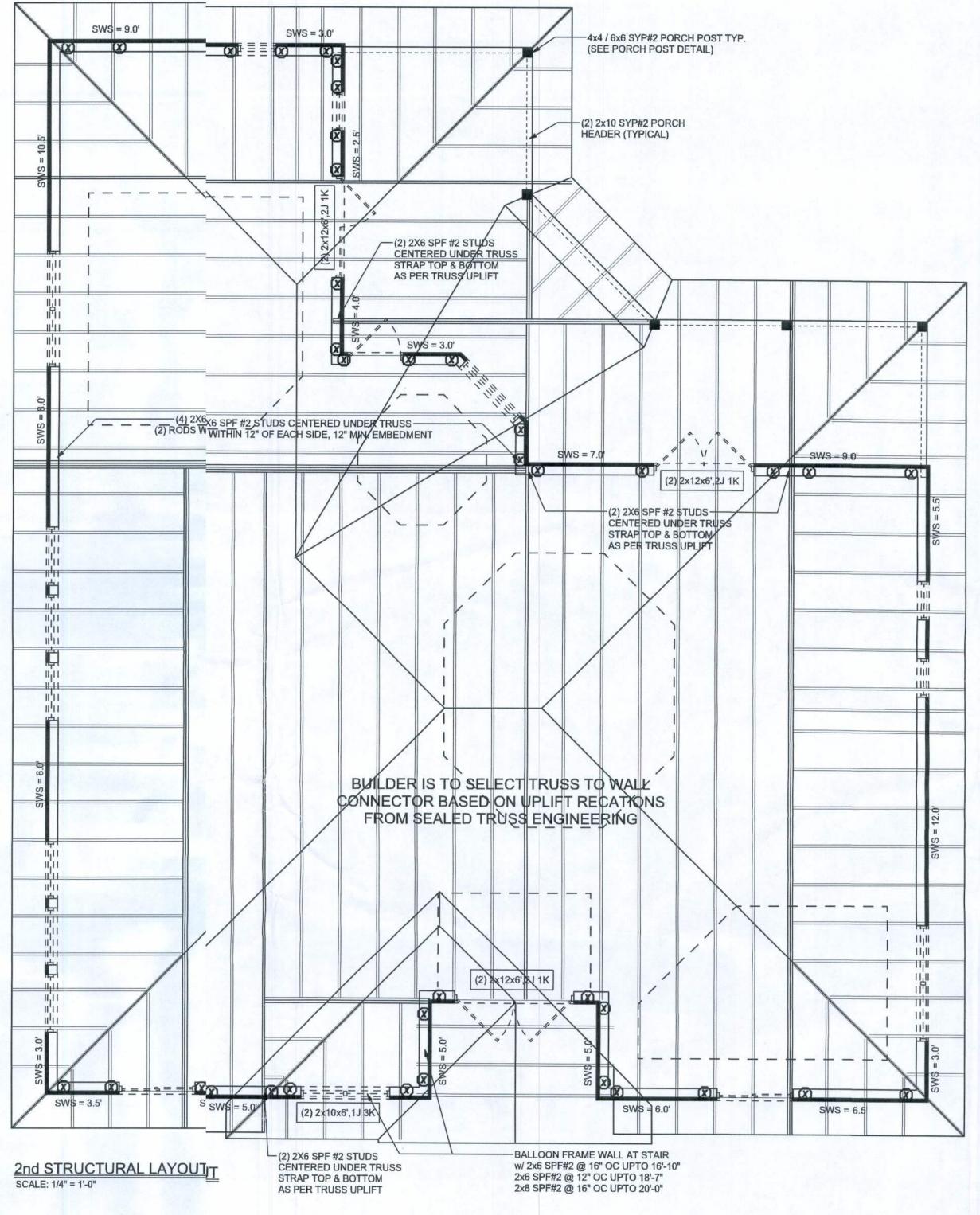
DTAL SHEAR WALL SEGMENTS

REQUIRED ACTUAL
TRANSVERSE 27.6' 64.5'

LONGITUDINAL 35.1' 52.0'

ND FLOOR WALL LEGEND

<u> </u>	EXTERIOR FRAMED WALL
	BLOCK WALL
ZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZZ	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT



WINDLOAD ENSINEER: Mark DisoswayPE No.53915, POE868, Lake City, FL 32056, 386-754-5419

REVISIONS

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Reer all questions to Mark Disosway P.E. for resolution.
Do not proceedwithout clarification.

COPYRIGHTS IND PROPERTY RIGHTS: Mark Disosway P.E. hereby expressly

reserves its cormon law copyrights and property right inhese instruments of service. This document is not to be reproduced, altered or copied in anyform or manner without first the express writen permission and consent of Mark Disoswy.

CERTIFICATIOI: I hereby certify that I have

CERTIFICATIOI: I hereby certify that I have examined this pan, and that the applicable portions of the pan, relating to wind engineerin comply with section R301.2.1, florid building code residential 2007and 2009 Supplements, to the best of m knowledge.

LIMITATION: Tils design is valid for one building, at specified location.

building, at spedied location.

M/Rix DISOSWAY
P.E. 53915

Chis & Dora Martin

ADDRESS: Columia County, Florida

Mark Disosway P.E. P.0. Box 868 Lake Cily, Florida 32056 Phone: 386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
Apri 20, 2010

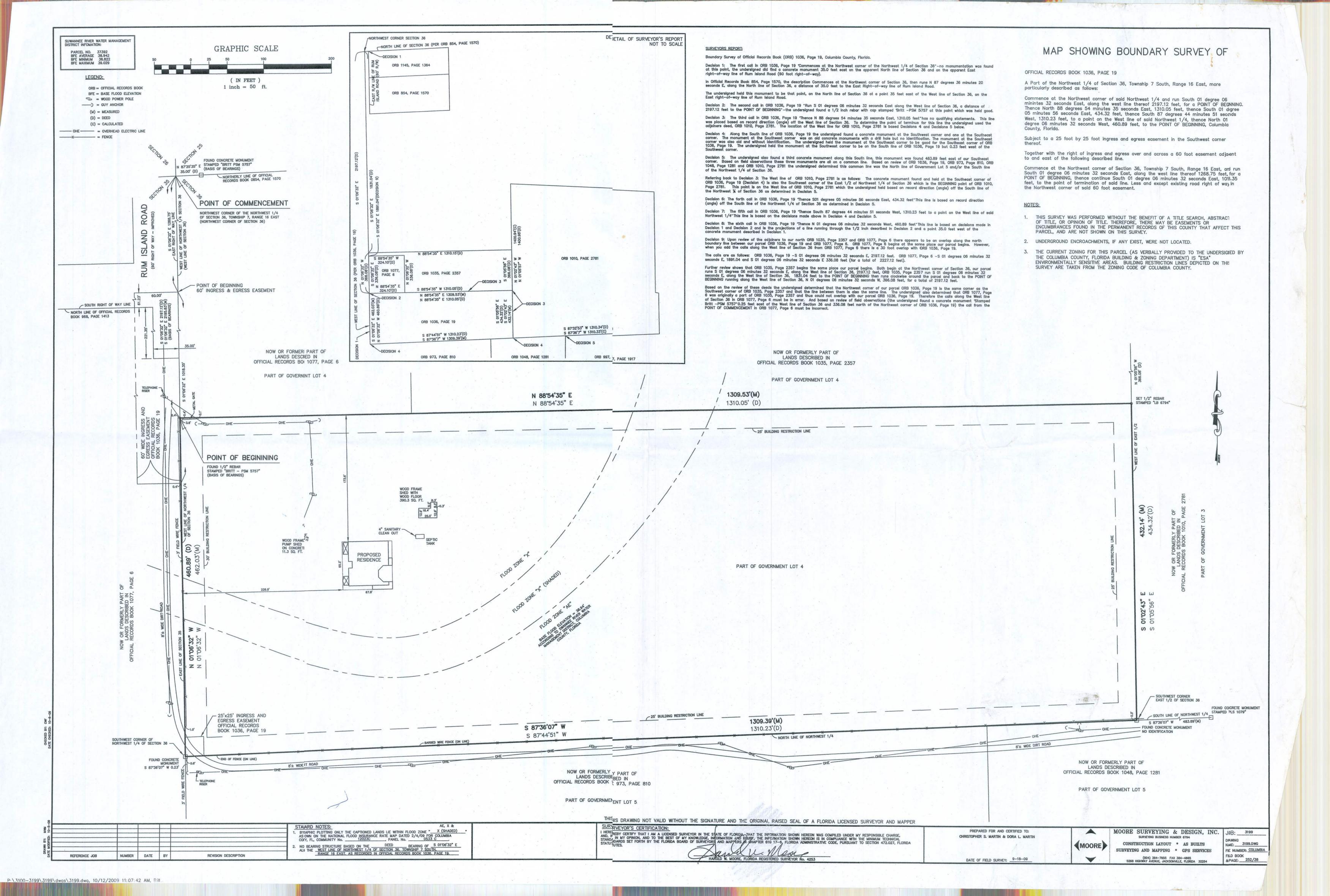
DRAWN BY STRUCTURAL BY:
David Disosway David Disosway

FINALS DATE:

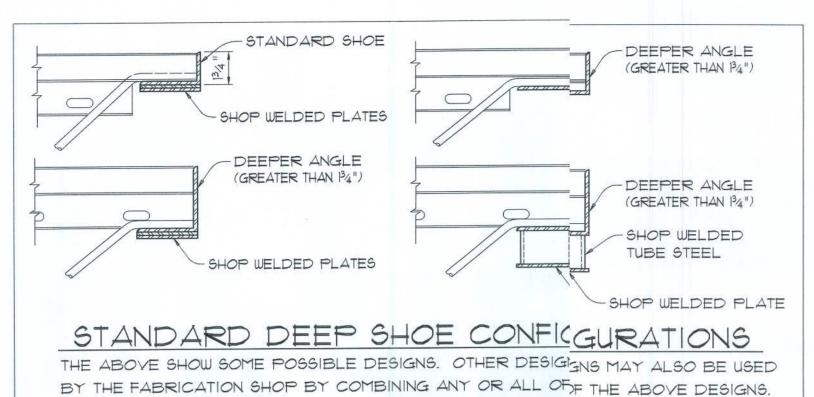
JOBNUMBER: 311211

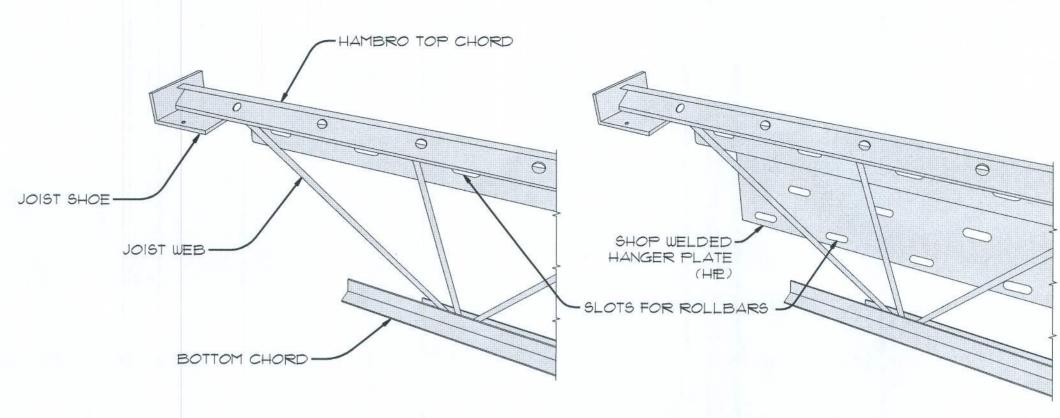
S-4 CF 11 SHEETS

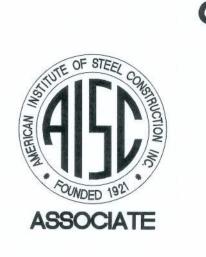
DRAVING NUMBER











ALL JOISTS SHALL BE ERECTED IN SUCH A MANNER SO THAT THEY ARE VERTICAL, LEVEL AND PLUMB

AND AT PROPER ELEVATIONS. ANY SHIMMING THAT

INSTALLATION SHALL BE IN ACCORDANCE WITH THE

THE FIELD USE SET OF PLANS.

SEVEN PALMS CONSTRUCTION

DISOSWAY DESIGN GROUP, INC.

CUSTOMER:

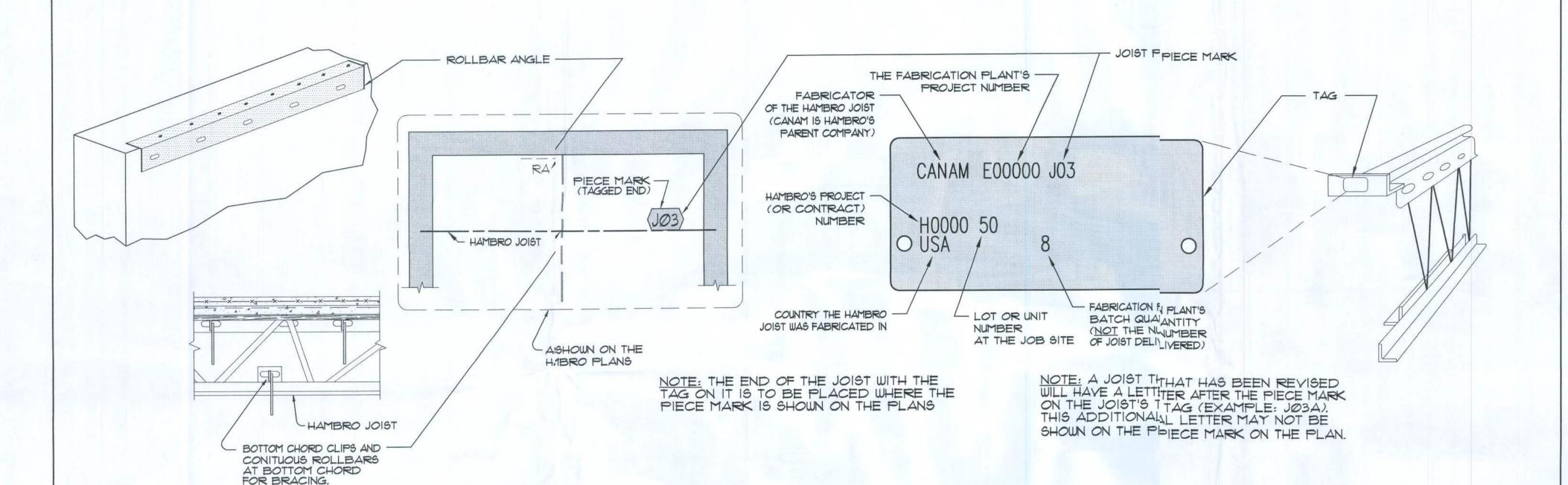
ARCHITECT:

ENGINEER:

MANUFACTURER'S RECOMMENDATIONS AND WITH THE "INSTALLATION MANUAL FOR THE HAMBRO D500 COMPOSITE FLOOR SYSTEM" AND / OR SHEET HD51 OF

MAY BE REQUIRED SHALL BE DONE WITH METAL.

HAMBRO JOIST TERMINOLOGY

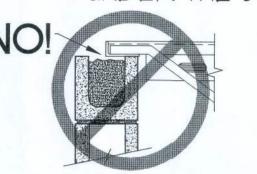


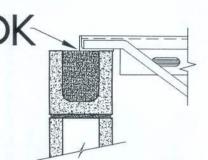
4" = FLANGE WIDTH

10 = POUNDS PER LINEAR FOOT

HAMBRO JOISTCOMPONENTS AND ACCESSORIES

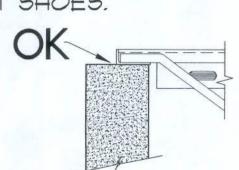






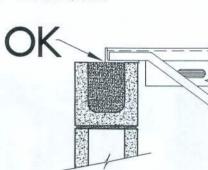
HAMBRO JOISTS MUST HAVE <u>LEVEL</u> BEARING UNDER THE JOIST SHOES.



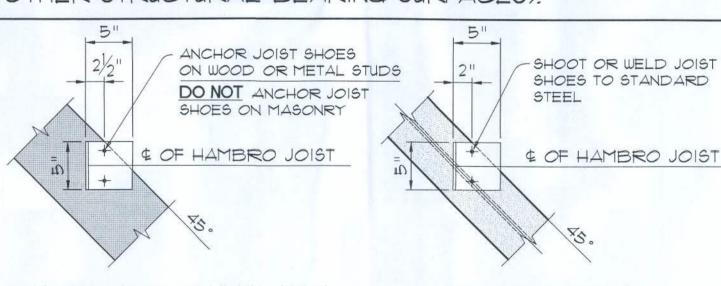


HAMBRO JOISTS MUST HAVE SOLID BEARING UNDER THE JOIST SHOES.





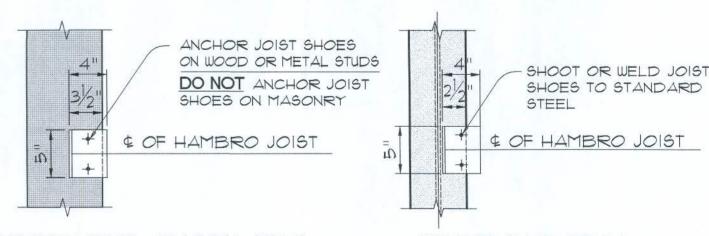
AVOID COMPROMISING THE INTEGRITY OF THE BEARING BY DOING ANYTHING TO DAMAGE THE MASONRY (OR OTHER STRUCTURAL BEARING SURFACES).



MASONRY, WOOD, OR METAL STUDS

WIDE FLANGE BEAM

5" SHOES ON SKEWED BEARING



MASONRY, WOOD, OR METAL STUDS

WIDE FLANGE BEAM

4" (STANDARD) SHOES ON PERPENDICULAR BEARING

IF YOU CAN NOT ACHIEVE PROPER BEARING, OR HAVING ANY QUESTIONS, PLEASE CALL HAMBRO BEFORE PROCEEDING

MINIMUM BEARING OF JOIST SHOES

SEE SHEET "HSØ-1" FOR MORE INFORMATION

NOTE: CERTIFICATION
EXTENDS ONLY TO
MATERIAL DESIGNED
AND FURNISHED BY
CANAM STEEL

IAN H. YAP, P.E. FL # 59557

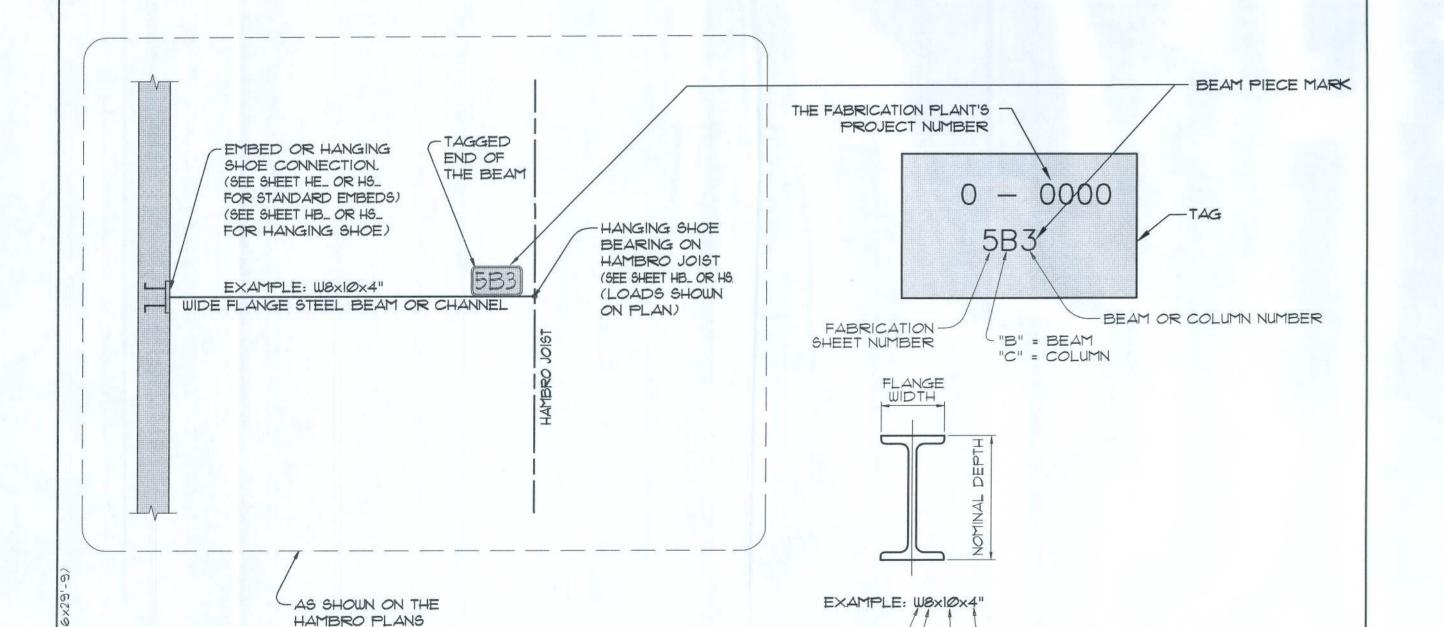


HAMBRO
Structural Systems
A Division of Canam Steel Corp.

450 E. Hillsboro Blvd. * Deerfield Beach, FL. 33441 (Phone) 954-571-3030 * 800-546-9008 * (FAX) 954-5713031 Email: Hambroeng@Hambro.ws * Web Site: www.Hambro.ws

DRAWN BY: DATE: CHECKED BY: DATE
SDW 11/2/09 LA 53-10

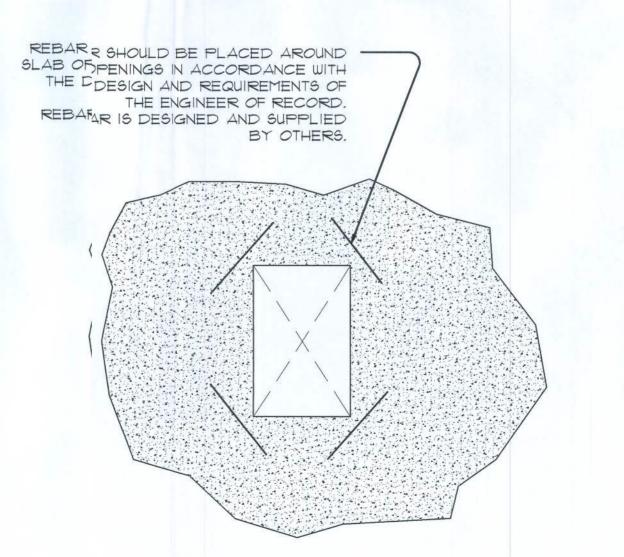
JOB NUMBER H9060 DRAWING NUMBER HN0-2



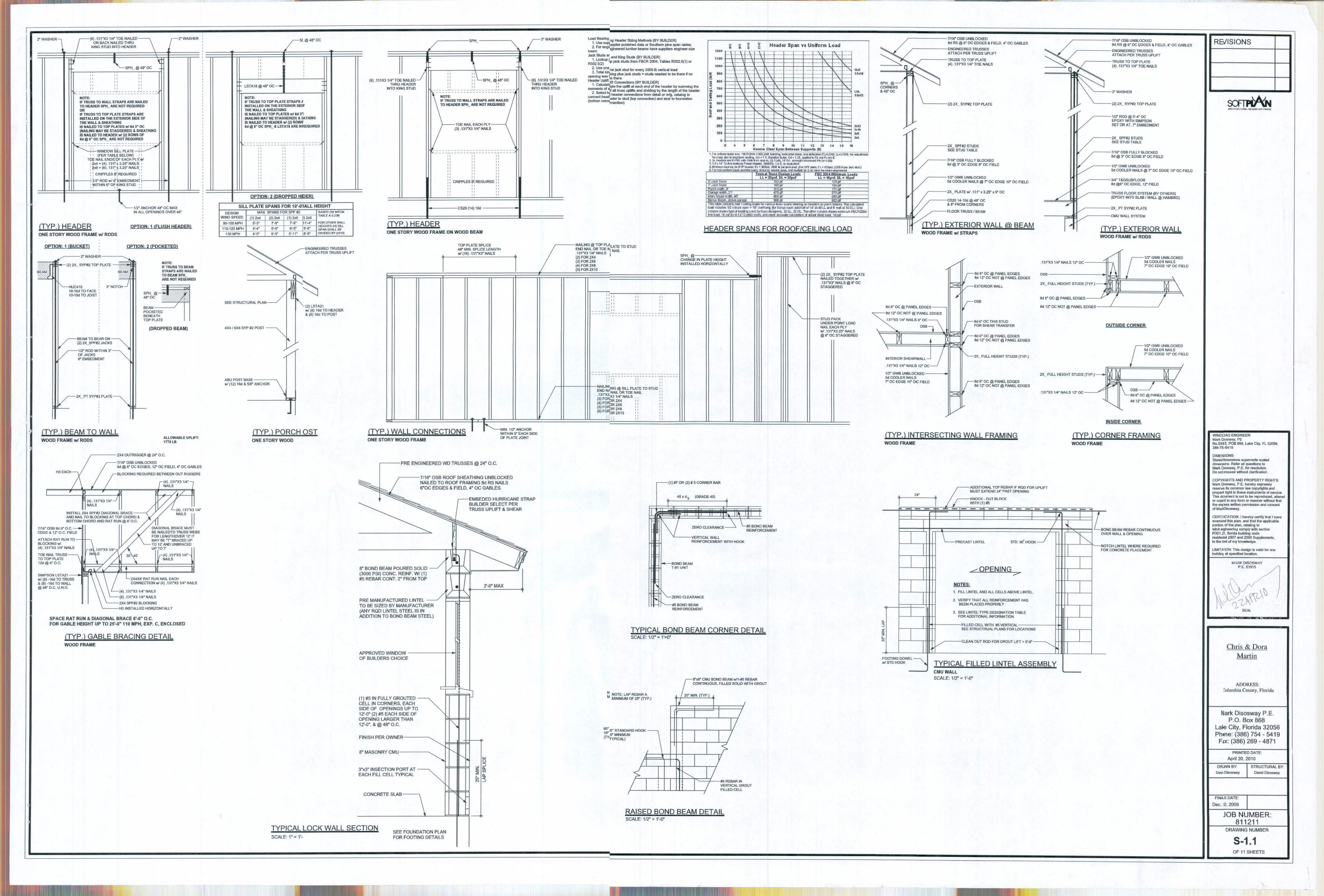
STRUCTURAL STEEL NOTIENCLATURE

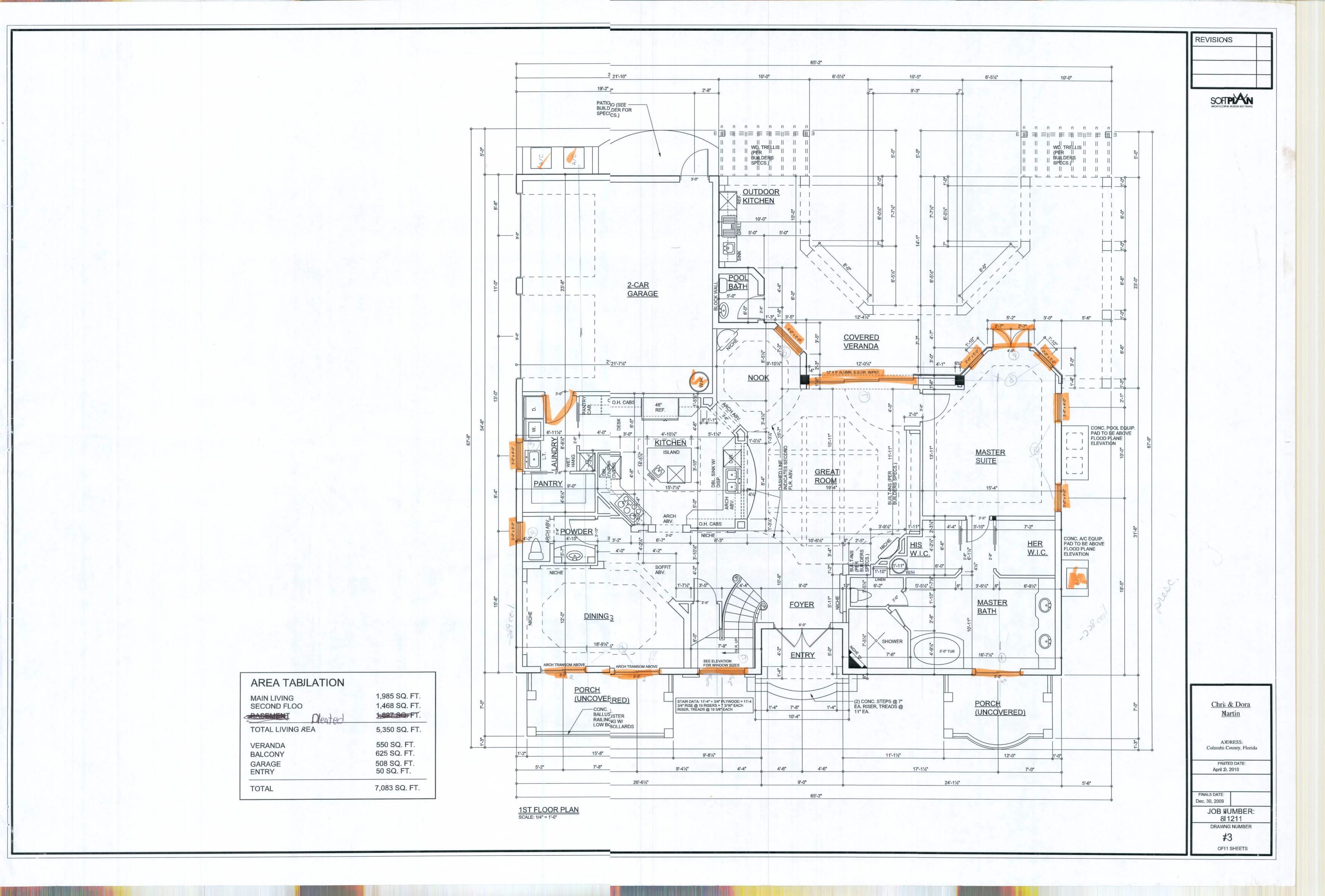
(APPLIES ONLY FOR STRUCTURAL STEEL SUPPLIED BY CANAM STEEL (HAMBRO))

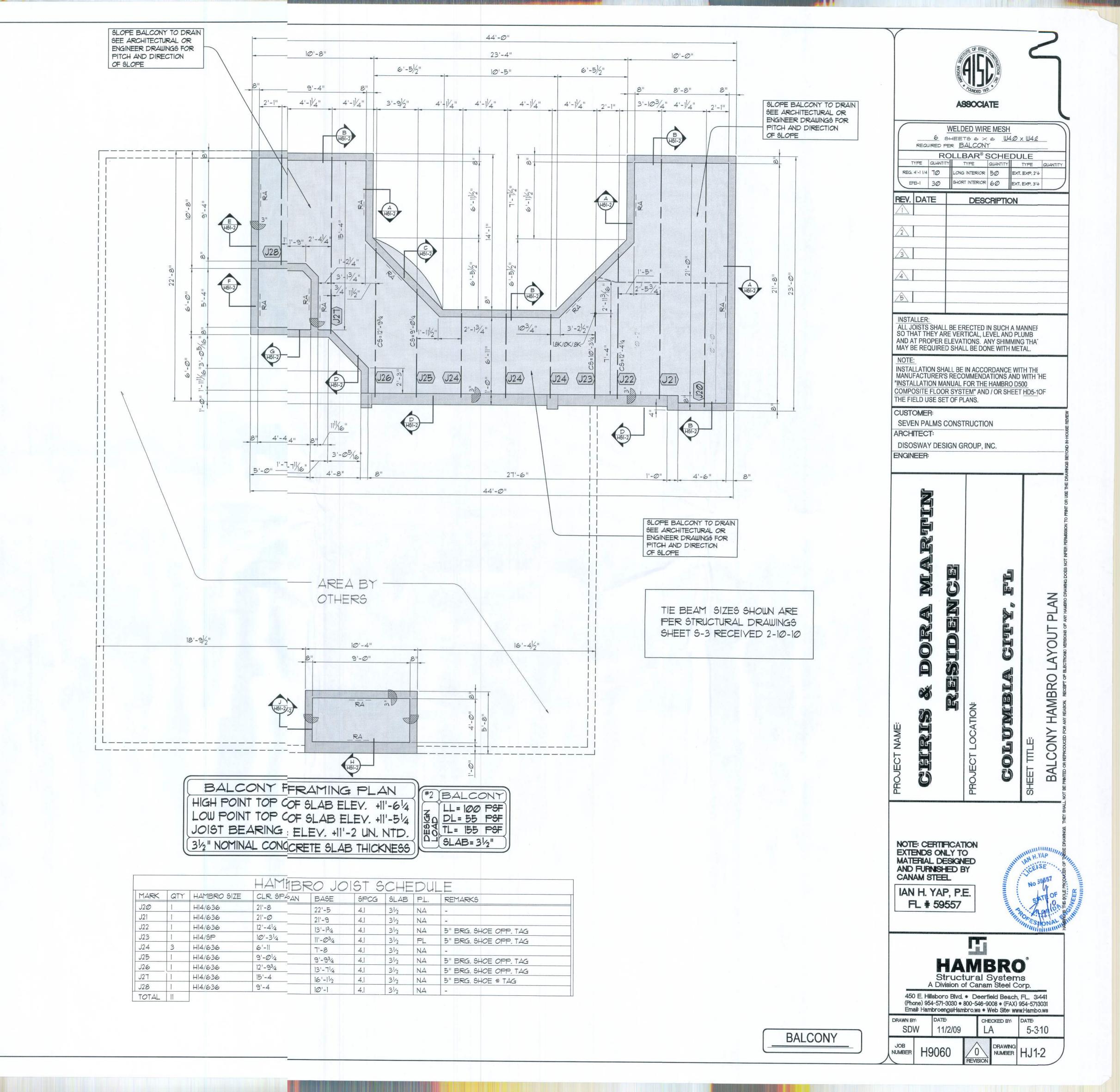
W = DE FLANGE BEAM -



REINFORRCING AT OPENINGS IN THE SLAB







LEGEND

HAMBRO JOISTS AND MATERIAL

NOTE: HAMBRO JOISTS ARE NOT DESIGNED TO
SUPPORT ROOF LOADS (UNLESS NOTED)

DENOTES TAGGED END OF JOIST (PIECE MARK)

RA DENOTES FIELD ATTACHED ROLLBAR® ANGLE (AKA. MH OR FH)

STRUCTURAL STEEL THAT IS SUPPLIED BY CANAM STEEL

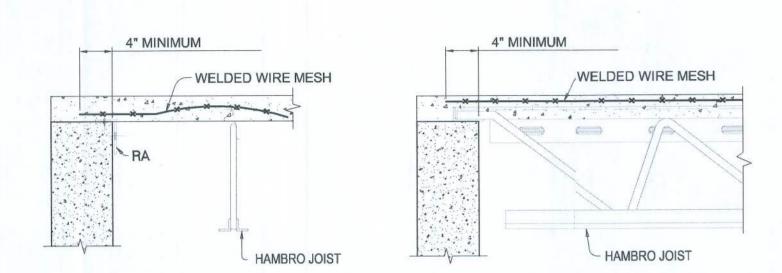
NOTE: ALL STRUCTURAL STEEL NOT SHOWN ON HAMBRO PLAN,
OR NOT MARKED WITH A PIECE MARK (SHOWN ABOVE),
IS SUPPLIED BY OTHERS.

BEARING WALLS OR HEADERS (DESIGNED & SUPPLIED BY OTHERS)

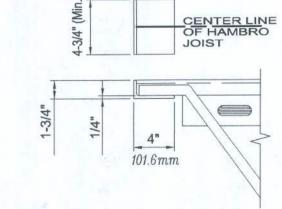
INDICATES LOAD BEARING 8" OR 12" MASONRY WALL OR CONC. BEAM

SPECIFICATIONS

- HAMBRO® DRAWINGS should always be used in cojuntion with approved Architectural, Structural and Mechanial plans.
- 2. 3 1/2 in. nominal concrete slab, fc=3000 psi. at 28 das, u/n. Total design load = 155 psf.
- 3. Welded wire mesh $6 \times 6 \times W4.0 \times W4.0$, fy = 60 ksi min.), u/n. Laps to follow ACI 318 provisions. At bearing, mesh over shall extend a minimum of 4", u/n.

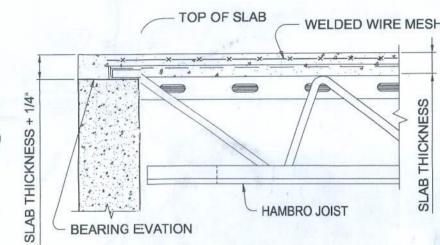


Standard joist shoe to be 4" D x 1-3/4" H x 1/4" T x 4-3/4" (Min.) W, u/n.



5. Bearing elevation to be finished floor elevation less thekness of slab and less 1/4" T for joist shoe.

POST SHORING NOT REQUIRED FOR STANDARD BEARING CONDITION (SEE ITEM #24 FOR NONSTANDARD BEARING)



- 6. Provide solid bearing as specified, without voids, uner joist shoes.
- 7. Joist to have ceiling extensions, u/n and 1 shop coat tandard (rust inhibitor) primer.
- 8. Rollbar® Angle (RA), are unpainted, non-structural istallation accessories once the concrete slab reaches specied strength

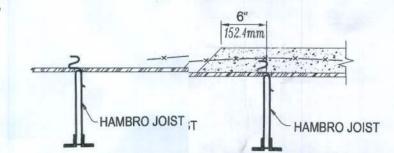
THAMB; RO® GENERAL NOTES

INSTALLATION

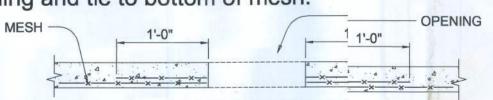
9. Installation shall be in accordance withh the Manufacturer's recommendations and with the "INSTALLATION / ERECTION MANUJAL for the HAMBRO® D500 COMPOSITE FLOOR SYSTEM".



- 10. All top chord 3 sections must face the SAME DIRECTION for proper installation of plywood formas.
- 11. Avoid, if possible (for ease of removal), inserting two "ROLLBARS®" in the same slot.
- 12. DO NOT WELD any joists (if required alat bearing), without first having inserted a "ROLLBAR" at each eend of every joist in unit to ensure proper spacing.
- 13. In areas where the slab is thickened, ""ROLLBAR®" spacing should be reduced.
- 14. BUNDLES OF FORMS AND MESH should not be placed on the joist system, but rather on the supporting walls or beams.
- 15. Construction joints shall be made paraallel to, and no closer than 6" to top chord.

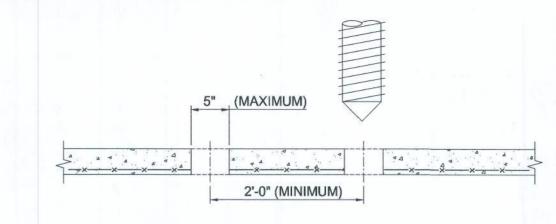


- 16. NO CONSTRUCTION LOADS shall be placed on the joist system until a min. of 4 consecutive jooists have been laterally braced with all required "ROLLBARS®" AT BOTH TOP AND BOTTOM CHORDS, AND WITH ALL FORMS IN PLACE.
- 17. If mesh has been cut to allow for an oppening, reinforce these areas with an additional layer of mesh 1. Jap 1'-0" all around opening and tie to bottom of mesh.



CONCRETE POUR

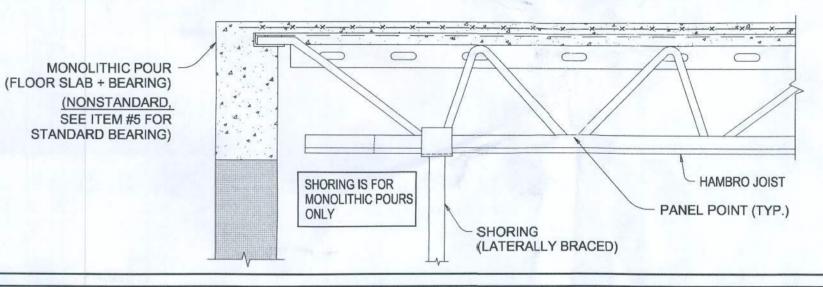
18. If core drilling, maximum hole diameter is 5 inches. Make sure HAMBRO® Joists are not drilled. Minimum center to center of drilled holes is 2'-0. Only one strand of tensile mesh may be cut per hole.



- 19. DO NOT DROP BUCKET LOADS OF CONCRETE FROM EXCESSIVE HEIGHTS AND IN CONCENTRATED AREAS ON THE JOIST SYSTEM
- DO NOT FINISH CONCRETE IN EXCESS OF THE DESIGN SLAB THICKNESS, u/n. During construction, the non-composite capacity of the joist is 279 PLF, u/n.
- 21. As needed, clean over pour at lower levels after each pour.

OTHER TRADES

- 22. DO NOT OVERLOAD (i.e. loads from blocks, drywall, lumber, sand, etc.) joist system or slab at any phase of construction.
- 23. DO NOT ALTER, drill, powder actuate fasten (except as noted on the HAMBRO® plans), burn or cut any HAMBRO® Structural Component as this will reduce it's designed capacity.
- 24. If shoring is used, place shores at the first panel point of the bottom chord.



FIRE RATINGS

25. Refer to the latest UNDERWRITERS LABORATORIES, INC. FIRE RESISTANCE DIRECTORY. For drywall ceilings use LISTING NO. G524

INSTALLER: ALL JOISTS SHALL BE ERECTED IN SUCH A MANNER SO THAT THEY ARE VERTICAL, LEVEL AND PLUMB AND AT PROPER ELEVATIONS. ANY SHIMMING THAT MAY BE REQUIRED SHALL BE DONE WITH METAL

ASSOCIATE

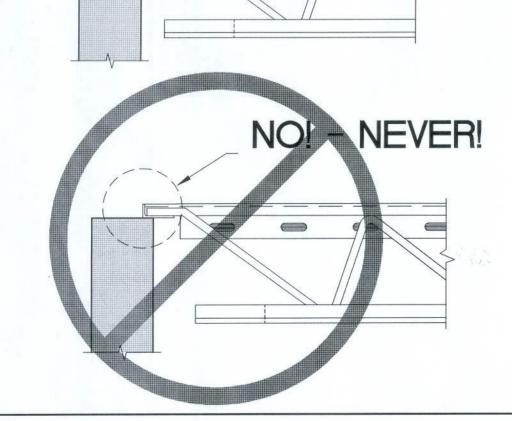
INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND WITH THE COMPOSITE FLOOR SYSTEM" AND / OR SHEET HD5-1 OF

THE FIELD USE SET OF PLANS CUSTOMER: SEVEN PALMS CONSTRUCTION

ARCHITECT: DISOSWAY DESIGN GROUP, INC. ENGINEER:

Please contact a HAMBRO® Representative if you require additional information.

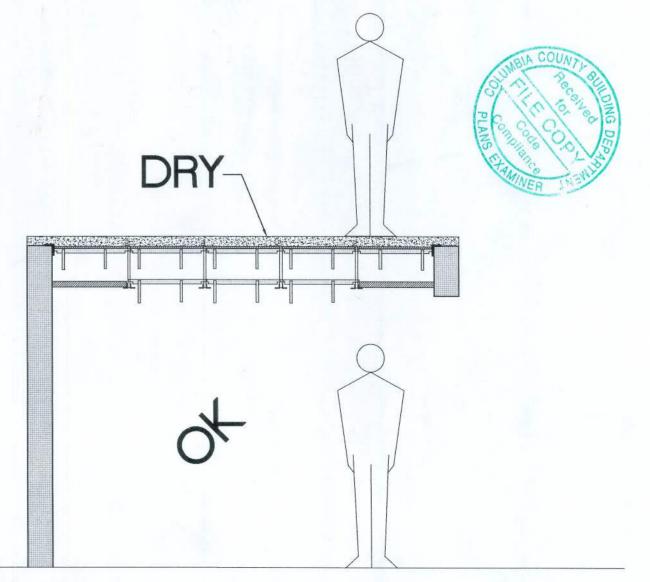


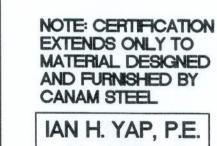


3 1/2" MIMIMUM BEARING ON MASONRY, WOOD, OR METAL STUDS

1/2" MIMIMUM BEARING ON STANDARD STEEL







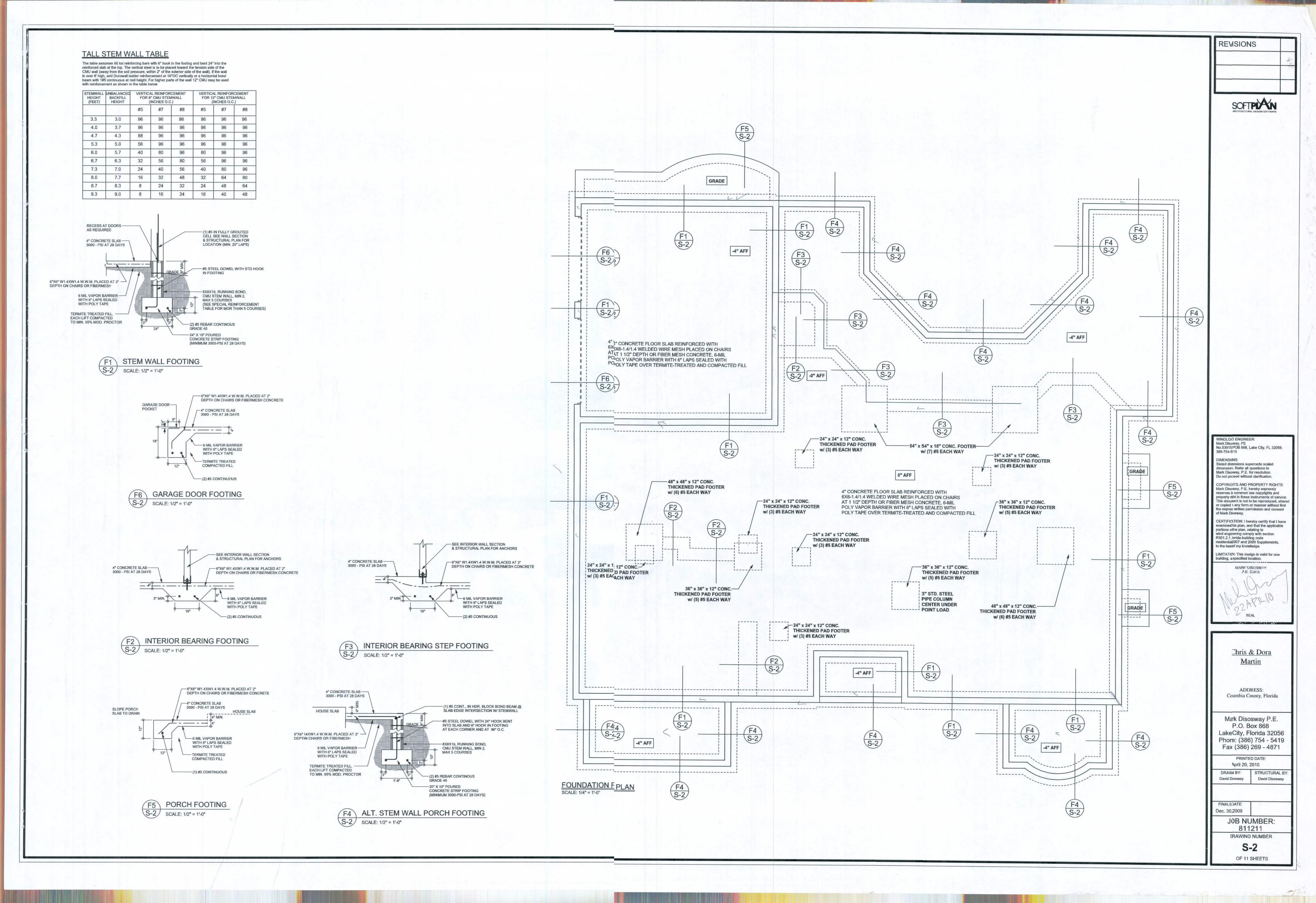


HAMBRO



450 E. Hillsboro Blvd, * Deerfield Beach, FL. 3344 (Phone) 954-571-3030 * 800-546-9008 * (FAX) 954-571-3031 Email: Hambroeng@Hambro.ws * Web Site: www.Hambro.ws

5-3-10 0 DRAWING HNO-1



MATERIALS

1. fc 8" precast lintel = 3500 psi 2. fc prestressed lintel = 6000 psi 3. Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump 4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi Rebar per ASTM A615 grade 60 6. Prestressing strand per ASTM A416 grade 270 low relaxation 7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints. Shore filled lintels as required. 3. Installation of lintel must comply with the architectural and/or structural documents.

4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting. 5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and ionger with a nominal height of 8" meet or exceed L/180 deflection. 6. Bottom field added rebar to be located at the bottom

of the lintel cavity. 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage. 8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

Safe load rating based on rational design analysis per ACI 318 and ACI 530 10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05

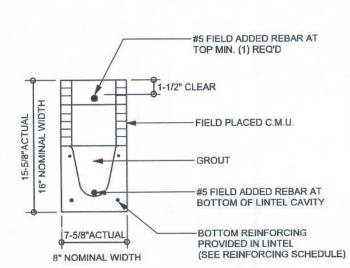
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. 12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

Applied vertical load Safe vertical load Safe horizontal load Safe horizontal load ≤1.0 Safe vertical load Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED / S = SOLID

OUANTITY OF #5 FIELD ADDED REBAR AT BOTTOM OF LINTEL CAVITY 8F16-1B/1T-4'-0" NOMINAL WIDTH ---**-OUANTITY OF #5 FIELD** NOMINAL HEIGHT -ADDED REBAR AT TOP -#5 FIELD ADDED REBAR AT



SAFE LOAD TABLE NOTES 1. All values based on minimum 4 inch

nominal bearing.

be reduced by 20% if bearing length is less than 6 1/2 inches. 2. N.R. = Not Rated 3. Safe loads are superimposed allowable loads. 4. Safe loads based on grade 40 or grade 60 field rebar. 5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only 6. The designer may evaluate concentrated

Exception: Safe loads for unfilled lintels must

loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support. 7. For composite lintel heights not shown,

use safe load from next lower height shown.

8. For lintels lengths not shown, use safe load from next longest length shown 9. All safe loads in units of pounds per linear

All safe loads based on simply supported

11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar. Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction 6472 ⇒ (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED LINTELS

<) ASTI-	(Centeria		SAFE	LOAD -	POUNE	S PER L	JIAR I	FOOT	
		TYPE	277000000	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8I-0B	8F28-0B	8F32-0B
ENGTH	Н		8U8	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8I-1B	8F28-1B	8F32-1B
				3069	4605	6113	7547	74	10394	11809
2'-10"	(34")	PRECAST	2231	3069	4605	6113	7547	74	10394	11809
				3069	3719	5163	6607	54	9502	10951
3'-6"	(42")	PRECAST	2231	3069	4605	6113	7547	74	10394	11809
				2561	2751	3820	4890	161	7034	8107
4'-0"	(48")	PRECAST	1966	2693	4605	6113	7547	74	10394	11809
	/E 493	PPEGART		1969	2110	2931	3753	76	5400	6224
4'-6"	(54")	PRECAST	1599	2189	4375	6113	7547 (7)	72	10294	11809
500 1010/01	Table State Conf.			1349	1438	1999	2560	23	3686	4249
5'-4"	(64")	PRECAST	1217	1663	3090	5365	7547(36)	2 (19)	8733 (19)	10127(19
	*****			1105	1173	1631	2090	49	3009	3470
5'-10"	(70")	PRECAST	1062	1451	2622	4360	7168 (45)	36 (19	7181 (19)	8328 (20
et ell	(770H)	DDECAST		1238	2177	3480	3031	07	4383	5061
6'-6"	(78")	PRECAST	908	1238	. 2177	3480	5381	60	10394(37)	8825 (14
	/ M			1011	1729	2632	2205	98	3191	3685
7'-6"	(90")	PRECAST	743	1011	1729	2661	3898	i81	8467 (44)	6472 (15
01.41	(44011)	DDECAST		699	1160	1625	2564	-86	2818	3302
9'-4"	(112")	PRECAST	554	752	1245	1843	2564	-86	4705(37)	6390 (47
101.00	/4000	POFOAGT		535	890	1247	2093	77	2163	2536
10'-6"	(126")	PRECAST	475	643	1052	1533	2093	'81	3643 (38	4754 (45
	/ / / / / / / / / / / / / / / / / / / /	2222427		582	945	1366	1846	123	3127	4006
11'-4"	(136")	PRECAST	362	582	945	1366	1846	123	3127	4006
4 51 61	(4.4.4M)	DDEGLOT	227	540	873	1254	1684	193	2805	3552
12'-0"	(144")	PRECAST	337	540	873	1254	1684	193	2805	3552
401.48	(4501)	DDECAST	noc	471	755	1075	1428	138	2316	2883
13'-4"	(160")	PRECAST	296	471	755	1075	1428	138	2316	2883
441.011	(4008)	PPECART	070	424	706	1002	1326	i97	2127	2630
14'-0"	(168*)	PRECAST	279	442	706	1002	1326	597	2127	2630
4 41 011	(4701)	PRESTRESSED	ND	NR	NR	NR	NR	1R	NR	NR
14'-8"	(176")	PRESTRESSED	N.R.	458	783	1370	1902	245	2517	2712
451.41	/4048\	PRESTRESSED	ND	NR	NR	NR	NR	√R	NR	NR
15'-4"	(184")	TREBTREBOED	N.R.	412	710	1250	1733	158	2320	2513
17'-4"	(208")	PRESTRESSED	N.R.	NR	NR	NR	NR	√R	NR	NR
17.79	(200)	TRESTRESSES	N.K.	300	536	950	1326	509	1849	2047
19'-4"	(232")	PRESTRESSED	N.R.	NR	NR	NR	NR	1R	NR	NR
	(IN.IX.	235	418	750	1037	282	1515	1716
21'-4"	(256")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR.	NR	NR
TE 10.	(===)		IV.IX.	180	340	598	845	114	1359	1468
22'-0"	(264")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR.	NR	NR
No. of the Control of	V-2.1	New York Control of Control Co	H.IX.	165	315	550	784)47	1285	1399
24'-0"	(288")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR -	NR	NR
	(1.4.4.4	170	250	450	654	84	1092	1222

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-NTELS

	A				- POUN			8-1T	
ENO:	TIL	TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T		8F32-1T
ENG	IH		8F8-2T	8F12-2T	8F16-2T	8F20-2T	8F24-2T	8-2T	8F32-2T
2'-10"	(34")	PRECAST	1972	3173	4460	5747	7034	321	9608
-10	(04)	THEONOT	1972	3173	4460	5747	7034	321	9608
3'-6"	(42")	PRECAST	1569	2524	3547	4569	5591	613	7636
ס-כ	(42)	PRECASI	1569	2524	3547	4569	5591	613	7636
4'-0"	(48")	PRECAST	1363	2192	3079	3966	4853	740	6627
+-0	(40)	TRECAUT	1363	2192	3079	3966	4853	740	6627
4'-6"	(54")	PRECAST	1207	1940	2724	3508	4292	077	5861
+-0	(54)	TRECAGI	1207	1940	2724	3508	4292	077	5861
-1 48	(0.48)	DDECAGE	1016	1632	2290	2949	3607	265	4924
5'-4"	(64")	PRECAST	1016	1632	2290	2949	3607	265	4924
EL 401	/70"\	DDECAST	909	1492	2093	2694	3295	397	4498
5'-10"	(70")	PRECAST	929	1492	2093	2694	3295	397	4498
CI CII	/70"\	DDECAST	835 (12)	1340	1880	2419	2959	498	4038
6'-6"	(78")	PRECAST	835	1340	1880	2419	2959	498	4038
71.00	(0.00)	DDECART	727 (23)	1021	1634 (12)	2102 (11)	2571(10)	039 (10)	3508 (9
7'-6"	(90")	PRECAST	727	1166	1634	2102	2571	039	3508
01.48	(44011)	DDECAGE	591	680	1133 (15)	1471 (15)	1811 (15)	152 (16)	2494 (15
9'-4"	(112")	PRECAST	591	851	1326	1705	2084	463	2842
	// 0.00)		530	552	914 (15)	1185 (15)	1458 (15)	732 (15)	2007 (15
10'-6"	(126")	PRECAST	530	686	1183	1526	1865	204	2544
			474	485	798 (15)	1034 (15)	1272 (15)	510 (15)	1749 (15
11'-4"	(136")	PRECAST	494	599	1028	1422	1738	053	2369
			470 (9)	441	723 (14)	936 (14)	1151(15)	366 (15)	1582 (15
12'-0"	(144")	PRECAST	470	543	928	1349	1649	948	2247
			418 (15	373	606 (14)	783 (14)	962 (14)	141 (14)	1321 (14
13'-4"	(160")	PRECAST	428	455	770	1145	1444	718	1993
			384 (15)		559 (14)	723 (14	887 (14)	052 (14)	1218 (14
14'-0"	(168")	PRECAST	410	420	709	1050	1434 (8)		
			230	323	519 (13)			376 (14)	
14'-8"	(176")	PRESTRESSE	246	390	655	968	1324 (8)		
			224	302	485 (13)			309 (13)	
15'-4"	(184")	PRESTRESSE	230	364	609	897	1224 (8)		
0,000 0000	Contraction (S)		187	255	404 (12	,555.5		754 (12	
17'-4"	(208")	PRESTRESSE	192	303	500	732		268 (14	
			162	222	347 (11)	100000		346 (12	
19'-4"	(232")	PRESTRESSE	166	261	424	616		057 (14	
			142	198	306 (11	393 (11)	_		
21'-4"	(256")	PRESTRESSE)	-		531		303 (13)	
			142	230	369	_		545 (11)	
22'-0"	(264")	PRESTRESSE	137	192	295 (10)				
			137	175	354 267 (10)	508 341 (10)	_	861 (13) 491 (10)	
								154-25 1 /101	

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DDR U-LINTELS

			SAFE LOAD - POUNDS PERNEAR FOOT							
<u> </u>		TYPE		8RF6-0B	8RF10-0B	8RF14-0B	8RF18-0B	-22-0B	8RF26-0B	8RF30-0B
LENG	TH		8RU6	8RF6-1B	8RF10-1B	8RF14-1B	8RF18-1B	F22-1B	8RF26-1B	8RF30-1B
	Transfer to the State of the St			1749	3355	3280	4349	5421	6493	7567
4'-4"	(52")	PRECAST	1635	1891	3699	5206	6639	060	9479	10893
				1596	3063	2992	3968	946	5924	6904
4'-6"	(54")	PRECAST	1494	1756	3699	5206	6639	060	9479	10893
EL 011	(0.08)	DDECAGE		920	1770	1716	2277	1839	3402	3966
5'-8"	(68")	PRECAST	866	1167	2481	4567	6389	060 (34)	7917 (19)	9311 (19
EL 401	/70II)	DDECAST		859	1653	1600	2124	2649	3174	3700
5'-10"	(70")	PRECAST	810	1113	2342	4242	6639 (10)	060 (38)	7402 (19)	8706 (19
	(====)		707	901	1825	3120	5048	747	9448	7360
6'-8"	(80")	PRECAST	797	901	1825	3120	5048	'915	9479	10893 (32
71.01	(0.011)	DDECAST		755	1490	2459	3776	743	7239	5623
7'-6"	(90")	PRECAST	669	755	1490	2459	3776	743	8998 (19)	10893 (48
OL OF	/4.4C#\		444	466	999	1568	2253	3129	4091	3146
9'-8"	(116")	PRECAST	411	526	999	1568	2253	3129	4150	5891 (47

SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOFJ-LINTELS

		SAFE LOAD - POUNDS PER LINER FOOT							
	_	TYPE	8RF6-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	=26-1T	8RF30-1T
LENGTH		8RF6-2T	8RF10-2T	8RF14-2T	8RF18-2T	8RF22-2T	F26-2T	8RF30-2T	
	400-113		905	1748	2635	3522	4409	296	6183
4'-4"	(52")	PRECAST	905	1748	2635	3522	4409	296	6183
A		") PRECAST	867	1675	2525	3374	4224	074	5924
4'-6"	(54")		867	1675	2525	3374	4224	074	5924
51.0E	(0011)	DDECAGE	675	1301	1960	2618	3277	935	4594
5'-8"	(68")	PRECAST	675	1301	1960	2618	3277	935	4594
EL 401	/70W	DDECAST	655	1262	1900	2538	3176	815	4453
5'-10"	(70")	PRECAST	655	1262	1900	2538	3176	815	4453
			570	1012	1651	2204	2758	3312	3865
6'-8"	(80")	PRECAST	570	1097	1651	2204	2758	3312	3865
	(506	797	1462 (8)	1952 (7)	2442 (5)	2931 (6)	3257
7'-6"	(90")	PRECAST	506	967	1462	1952	2442	2931	3421
01.01	/4.4.OII)	DDECAST	395	491	931 (12	1301 (15	1640 (15	980 (15)	2322 (15
9'-8"	(116")	PRECAST	395	589	1135	1514	1893	272	2652

GRADE & SPECCIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP#2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP#2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDDS

		TO 0 10'-6" STUD HEIGHT
(1)	2x4 @ 12" OC	TOO 11'-7" STUD HEIGHT
(1)	2x6 @ 16" OC	TO 0 16'-10" STUD HEIGHT
(1)	2x6 @ 2" OC	TOO 18'-7" STUD HEIGHT

WOOD ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

PLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d	1	
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d	PITTON	
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	L L L L L L L L L L L L L L L L L L L	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS	1974	TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
< 2200	< 2200	ABU44	12-16d		1/2" AB	
< 2300	< 2300	ABU66	12-16d		1/2" AB	
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB	

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15* EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3 CMU standard		ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F Coating for corrosion protection		Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet meta ties not completely embedded in mortar o grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W 1.4 x W 1.4 , FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTMIC 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

BEARING LOCATIONS.

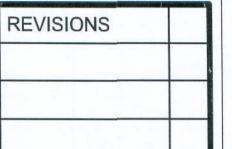
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;

DESIGN DATA

BUILDING IS NOT IN THE HIGH VELOCITY HURRI	CANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	REGION				
1.) BASIC WIND SPEED = 110 MPH			-		
2.) WIND EXPOSURE = C		-		115	
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES	Contra-				
6.) MEAN ROOF HEIGHT = <30 FT				-	
7.) INTERNAL PRESSURE COEFFICIENT = N/A	ENCLOSED	BUII D	ING)		
8.) COMPONENTS AND CLADDING DESIGN WII				2301 7/	211
U. COMI ONLIVIO AND CLABBING DESIGN WI	AD I INCOSO	INEO (I	ADLL	1301.2(-))
*	Zone Effective W		ind Area (ft2		
			0		100
	1		-30.5	-	-25.3
	2	27.8		25.3	-30.5
2 2 2	2 O'hg	27.8	-56.8 -35.7	25.3	-56.8
2 4 2 5	3 O'hg	21.0	-35.7 -95.6	25.3	-59.3
4	4	30.5	-33.0	25.9	-28.
555	5	30.5	-40.7	25.9	-31.6
NAT NAT	Doors	& Wind	dows	30.5	-40.7
[3]	Wor	st Cas	е		
	(Zone	5, 10	ft2)		
19 2 19	8x7 Gar			27.3	-32.0
4 /2 4	16x7 Ga	arage [Door	25.9	-29.4
	-				-
515					1
			- >		
DESIGN LOADS					
FLOOR 40 PSF (ALL OTHER DWELLING ROOM	S)	-			
30 PSF (SLEEPING ROOMS)					
30 PSF (ATTICS WITH STORAGE)					
10 PSF (ATTICS WITHOUT STORAGE,	<3:12)				
ROOF 20 PSF (FLAT OR <4:12)					
16 PSF (4:12 TO <12:12)					
12 PSF (12:12 AND GREATER)					
STAIRS 40 PSF (ONE & TWO FAMILY DWELLING	S)				=====
SOIL BEARING CAPACITY 1000PSF		-			





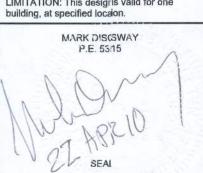
No.53915, POB 868, Lak City, FL 32056, 386-754-5419

Stated dimensions superede scaled

dimensions. Refer all quetions to Mark Disosway, P.E. for esolution. Do not proceed without carification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. herey expressly reserves its common law:opyrights and property right in these insuments of service. This document is not to b reproduced, altered or copied in any form or nanner without first the express written permision and consent of Mark Disosway.

portions of the plan, relating to wind engineering comply vith section R301.2.1, florida buildingcode residential 2007 and 200! Supplements, to the best of my knowlege. LIMITATION: This designis valid for one

CERTIFICATION: I hereb certify that I have examined this plan, and tat the applicable



Chris & Dora **Marin**

ADDRESS: Columbia Couty, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386)754 - 5419 Fax: (386) 269 - 4871

PRINTECDATE: April 20, 2(10 DRAWN BY: STRUCTURAL BY: David Disosway David Disosway

FINALS DATE: Dec. 30, 2009

> JOB NUMBER: 811211 DRAWING NUMBER

> > OF 11 SHEETS

2ND FLOOR ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPIAN ARCHITECTURA DESIGN SOFTWARE

Chris & Dora Martin

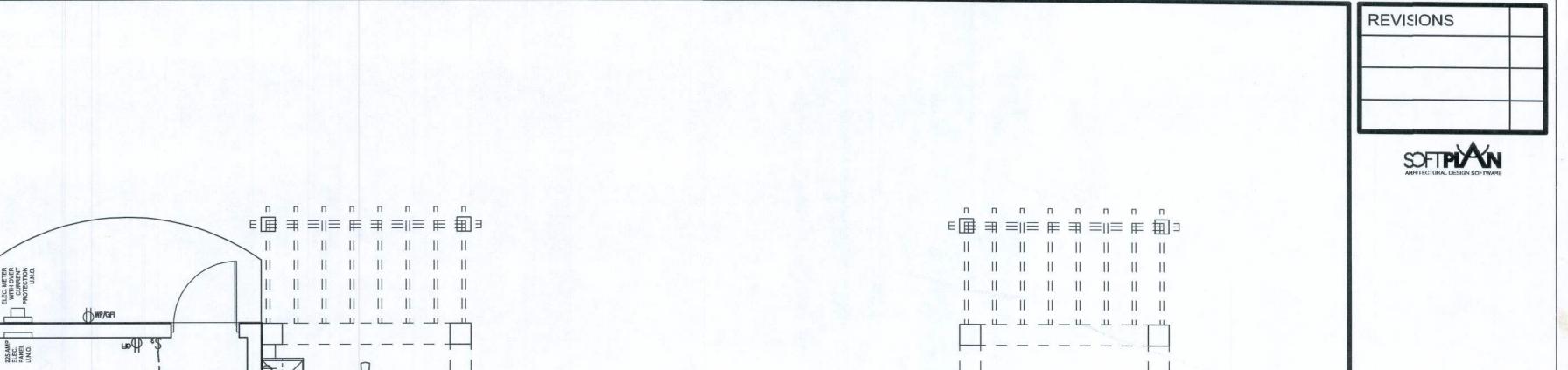
ADDRES: Columbia Couny, Florida

PRINTED (ATE: April 20, 201)

FINALS DATE: Dec. 30, 2009

JOB NUMBER: 811211 DRAWING NUMBER

> #6 OF 11 SHIETS



ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMIT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODI

E -4

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
NEAR ALL BEDROOMS.

E -5
TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI

E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

A SERVICE DISCONNECT WITH OVER CURRENT PROTECTN SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC

E -10 CONDUCTORS ENTER THE BUILDING.
SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATE
INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL
APPROVAL OF THE BUILDING OFFICIAL

CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHII 0'
OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS IVING
A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIRERCE,
OR ATTACHED GARAGE.

0	ELECTRICAL LEGEN
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦ -	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
	LIGHT FIXTURE
Ф	DUPLEX OUTLET
Ф	220v OUTLET
Д ая	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
9	GARAGE DOOR OPENER
⊕ cm	CARBON MONOXIDE ALARM

Tamper Proof Rec.

UFer ground Required

AFEI Where NEC

2008 NEC

The state of the s	
	₩r/cri
48° F/OUR.	
	AC DISCONNECT AAC COND BOTH
1ST FLOOR ELECTR RICAL PLAN SCALE: 1/4" = 1'-0"	LED/AM D

Chris & Dora Martin

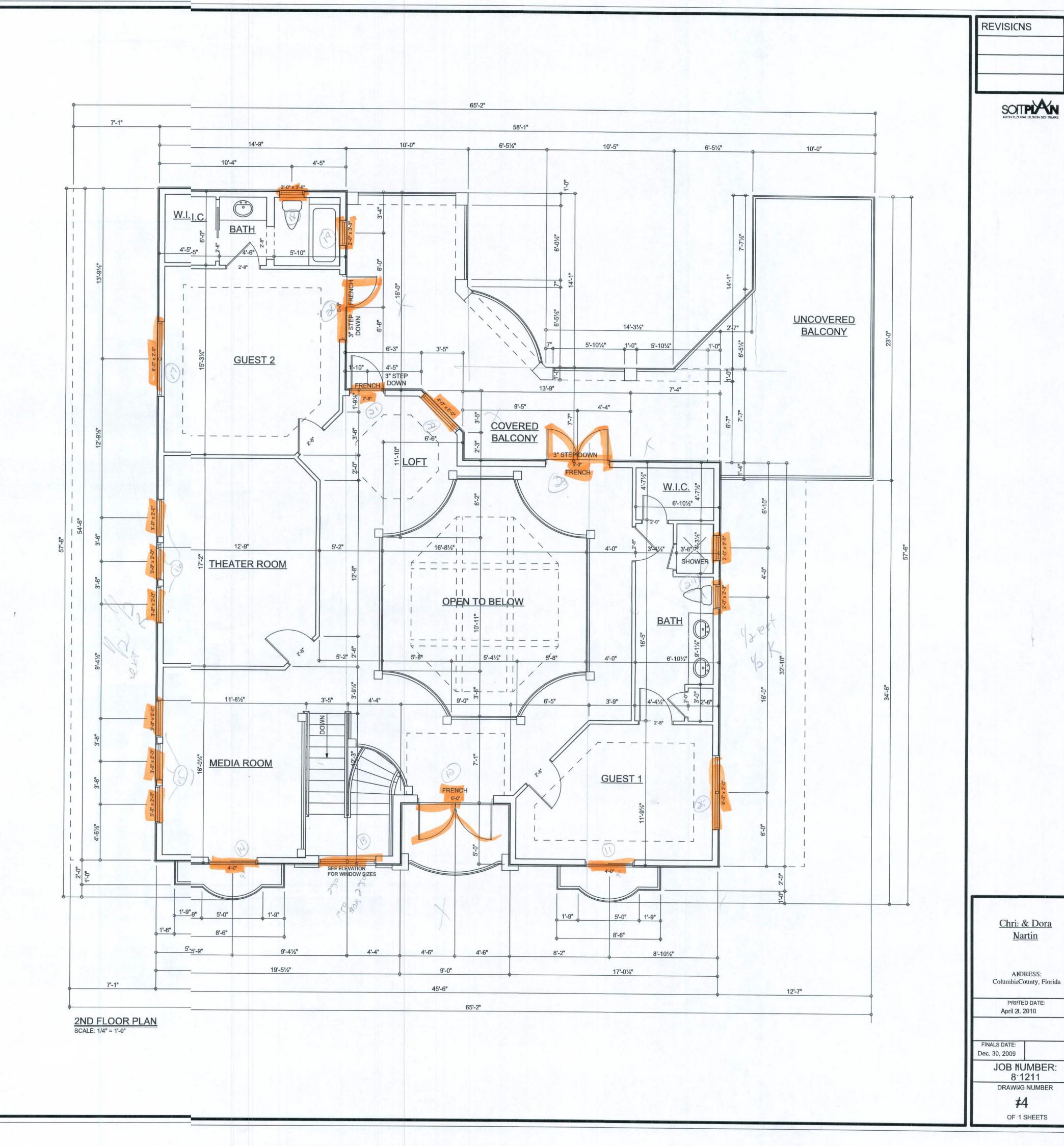
ADDRESS: Coumbia County, Florida

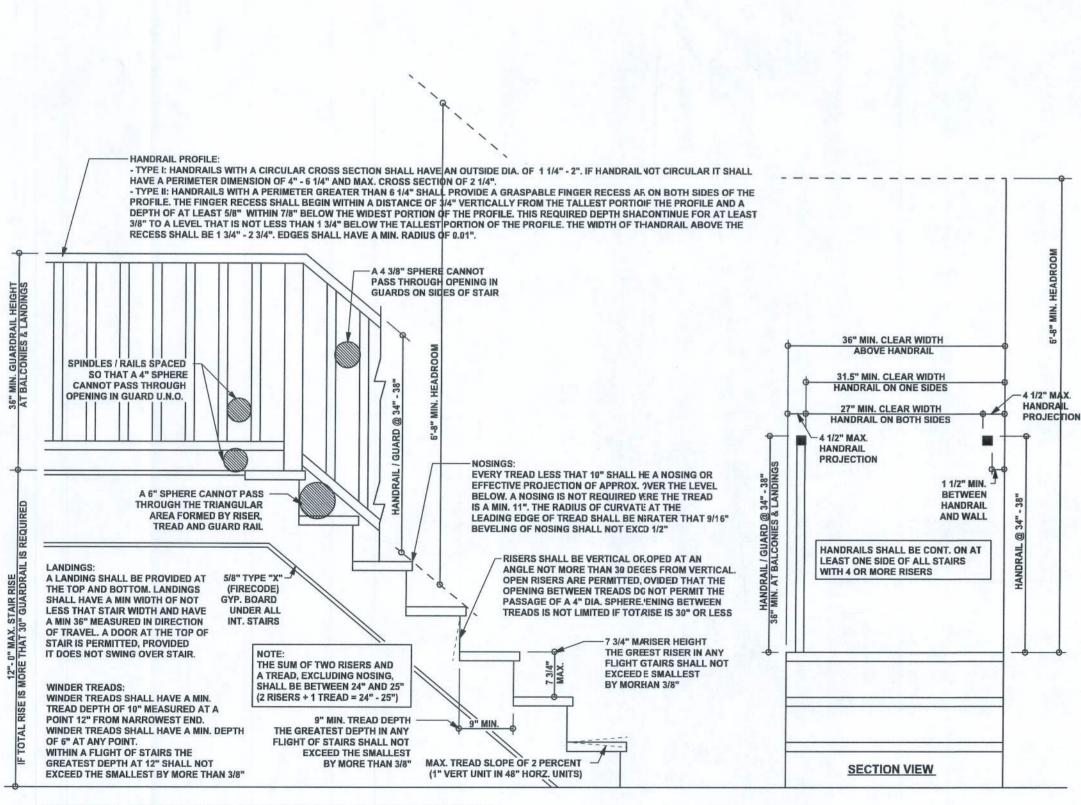
PRINTED DATE: April 20, 2010

FINALSDATE: Dec. 30,2009

JOB NUMBER: 811211 IRAWING NUMBER

#5
OF 11 SHEETS





TYPICAL STAIR AND GUARDRAIL REQUIRMENTS
SCALE: 3/4" = 1'-0"

