ITW Building Components Group, Inc.

Naw One

Seal Date: 01/31/2007

-Truss Design Engineer-Arthur R. Fisher

Florida License Number: 59687

1950 Marley Drive

Haines City, FL 33844

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1T4G487-Z0131082444

Truss Fabricator: Anderson Truss Company

Job Identification: 6-395--Stanley Crawford Construc WILMOTH -- , **

Truss Count: 56

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002 (STD) /FBC

Engineering Software: Alpine Software, Versions 7.24, 7.25.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

- Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
- 2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
- 3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

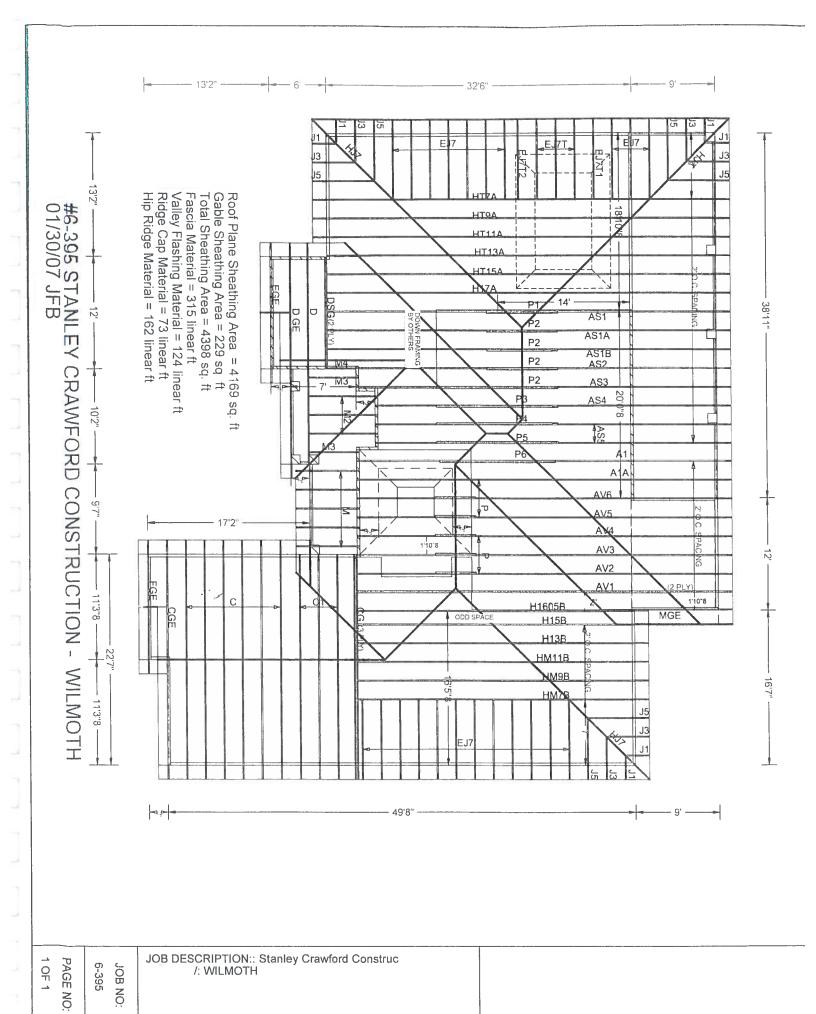
Details: BRCLBSUB-A11015EE-GBLLETIN-MAX DEAD LOAD-

#	Ref Description	Drawing#	Date
1	93369HT15A	07030001	01/30/07
2	93370H17A	07030036	01/30/07
3	93371 AS1	07030002	01/30/07
4	93372AS1A	07030003	01/30/07
5	93373AS1B	07030004	01/30/07
6	93374 AS2	07030005	01/30/07
7	93375 AS3	07030006	01/30/07
8	93376AS4	07030007	01/30/07
9	93377 AS5	07030008	01/30/07
10	93378 A1	07030009	01/30/07
11	93379 = -AV1	07030010	01/30/07
12	93380 AV2	07030011	01/30/07
13	93381AV3	07030012	01/30/07
14	93382 AV4	07030013	01/30/07
15	93383AV5	07030014	01/30/07
16	93384 AV6	07030015	01/30/07
17	93385 A1A	07030016	01/30/07
18	93386HT7A	07030030	01/30/07
19	93387 HT9A	07030008	01/30/07
20	93388HT11A	07030010	01/30/07
21	93389 HT13A	07030009	01/30/07
22	93390 HM7B	07030018	01/30/07
23	93391HM9B	07030019	01/30/07
24	93392HM11B	07030020	01/30/07
25	93393H13B	07030017	01/30/07
26	93394H15B	07030018	01/30/07
27	93395H1605B	07030019	01/30/07
28	93396 CGE	07030007	01/30/07
29	93397 C	07030013	01/30/07
30	93398C1	07030017	01/30/07
31	93399 CG	07030031	01/30/07
32	93400 DSG	07030032	01/30/07
33	93401D GE	07030033	01/30/07
34	93402D	07030034	01/30/07
35	93403EGE	07030035	01/30/07
36	93404FGE	07030001	01/30/07

	#	Ref Description	Drawing#	Date
	37	93405 HJ7	07030002	01/30/07
	38	93406EJ7	07030006	01/30/07
	39	93407 EJ7T	07030014	01/30/07
	40	93408EJ7T1	07030012	01/30/07
	41	93409 EJ7T2	07030015	01/30/07
ļ	42	93410J5	07030003	01/30/07
Ì	43	93411J3	07030004	01/30/07
	44	93412J1	07030005	01/30/07
	45	93413M4	07030011	01/30/07
	46	93414 MGE	07030016	01/30/07
ı	47	93415M3	07030020	01/30/07
	48	93416M2	07030021	01/30/07
l	49	93417 M	07030022	01/30/07
ı	50	93418P2	07030023	01/30/07
ı	51	93419P	07030024	01/30/07
ı	52	93420P1	07030025	01/30/07
ı	53	93421P3	07030026	01/30/07
	54	93422P4	07030027	01/30/07
	55	93423P5	07030028	01/30/07
1	56	93424 P6	07030029	01/30/07

25417





(6-395--Stanley Crawford Construc HICMOTH HT15A)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense :B6 Webs 2x4 SP #3 2×6 SP #1 Dense

(J) hanger connection not found in inventory file for this condition. Provide connection.

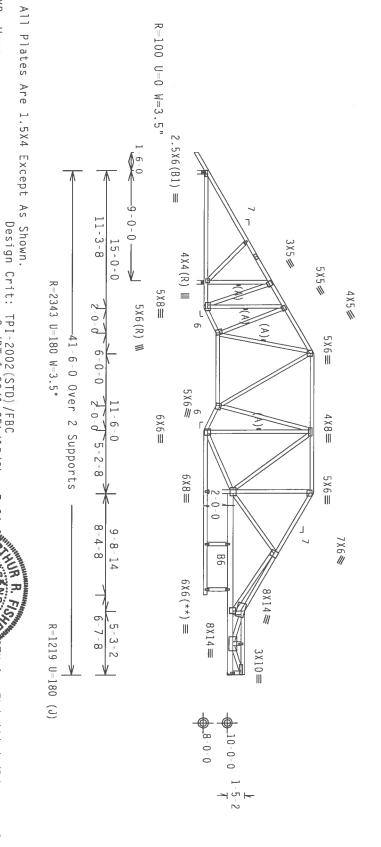
(A) Continuous lateral bracing equally spaced on member.

See detail BCFILLER1106 for bottom chord (BC) filler detail. Laterally brace BC above filler @ 24" o.c. (or as designed) Including a brace on BC directly above both ends of filler (if no rigid diaphragm exists at that point).

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



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'n

Haines City, FL 33844

ALPINE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRASS IN CONTRIBUNCE WITH APPLICABLE PROVISIONS OF HIS GRAIN FOR THIS DESIGN CONTRONS WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY AREA) AND IP.

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Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)

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PSF PSF PSF

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REF DATE

93369

14/

-/R/

Scale = .125"/Ft. R487--

SPACING DUR.FAC.

24.0" 1.25 40. 0.0

JREF-FROM SEQN HC-ENG

1T4G487_Z01

0T.LD.

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PSF

A PROPERLY ATTACHED RIGID CEILING.

F

Certificate of Authorization # 567

PLT

TYP.

Wave

Note:

ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 PLT See detail BCFILLER1106 for bottom chord (BC) filler detail Laterally brace BC above filler @ 24" o.c. (or as designed) Including a brace on BC directly above both ends of filler (if no rigid diaphragm exists at that point). l op Bot $\left(\text{J}\right)$ hanger connection not found in inventory file for this condition. Provide connection. Wind reactions based on MWFRS pressures (6-395--Stanley Crawford Construc WILMOTH p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP TYP. ALPINE Wave ||2 Dense ||2 Dense ||3 **IMPORTANT**FURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITS BUILDING COMPONENTS GROUP. JUC. SHACL NOT BE RESPONSIBLE FOR ANY DEVELVEN OF FROM THIS DESIGN, ANY FAILURE TO BUILD THE HARSS IN CONFEDENCE WITH PET OR FABRICATION, NAME HELD, SHIPPING. INSTALLING A BRACKING OF TRUSSES, IN CONFEDENCE WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY ATRA) AND PET. APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY ATRA) AND PET. CONFECTION FAILS ARE HAND OF FOLICES OF THE PROVISION OF THIS DESIGN SPEC, APPLY PLATES TO EACH FACE OF TRUSS AND, HHELDS OFFICHALS OF THE SPECIAL PROSITION OF PLATES FOLICABLE BY (1) SHALL HE PER ANNEX A OF PET 2002 SEC. J. AS SALON HIS DEATH OF PLATES OFFICIAL BY (1) SHALL HE PER ANNEX A OF PET 2002 SEC. J. AS SALON HIS DEATH OF THE PROVISION OF PLATES FOLICABLE OF PROVISIONAL ENGINEERING, RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT **WÄRNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST. (BUILDING CHMPONET! SACETY INFORMATION). PHILLINED BY THE (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 127. ALEXANDRIA, NA, 22214) AND MICA (MODO TRUSS COUNCIL OF AMERICA. 6300 CHITERRESSE LANE, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OBTHERMISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. DESIGN SHOWN. THE SUITABILITY AND BUILDING DESIGNER PER AUSI/TPI 1 SEC. 1-6-0 0-6-0 $.5 \times 6 (B1) =$ R=36U = 180Design Crit: _12 5 X 5 🕼 W = 3.5'17 USE OF THIS COMPONENT -0-0 H17A) =2301 U=258 W=3.5" 4 X 6 // 24-6-0 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) ΛUΥ 6-0 4 X 8 ≡ €X6= BUILDING Â 0ver φ ယ RESPONSIBILITY OF Supports 5 X 8≡ 5×6= $5 \times 6 =$ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED located within 6.50 ft from roof edge, CAT II, EXP DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Right end vertical not exposed to wind (A) Continuous lateral bracing equally spaced on member Ó 1.5X4 Ⅲ 1.5X4 III 9 24 10-4-8 4 X 5 // ά -14 MIHUR 5X12≢ BOENSE 1.5X4 III €X6= 0.59687 .5X4 III NO INEER 2.5X6= 7-3-2 6-7-8 4 X 5 ≡ =1216* 2.5X8= 1.5X4 III 0 = 180TC SPACING DUR.FAC \mathcal{I} TOT.LD. DL 민 رت) pressure 8-0-0 /4/-/-/R/ 0 10.0 20.0 10.0 24.0" 1.25 40 0.0 . bldg, not B, wind TC PSF PSF PSF PSF PSF 2 DATE REF FROM SEQN JREF-HC-ENG DRW HCUSR487 07030036 Scale R487--1T4G487_Z01 =.125"/Ft. MNM/AF 01/30/07 149323 10 93370 Ġ

ITW Building Components Group, Inc. Haines City, FL 3844 FI Certificate of Authorization # 567 PLT Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. condition. (J) hanger (6-395--Stanley Crawford Construc TYP. ALPINE Wave connection not found in inventory file for this Provide connection. DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/IPT 1 SEC. 2. ***IMPORTANT***URRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP. THE. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FOR HIS THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTORNANCE WITH 1971 OF FARRICALING, HANDLING, SUPPING, HISTALLING A BRACING OF HUNSES.

DESIGN CONTORNAY HITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN ESPEC, BY AFRAM) AND IPI.

CONNECTION PLATES ARE MADE OF 20/18/166A (M.1MSSY), ASTH AGS GRADE 40/60 (M. X.M.SS) SAN STEEL APPLY

PRAITS TO EACH FACE OF HUNSS AND, MULESS OTHERWISE LOCATED ON HITS DESIGN POSITION FER DRAWNINGS 160A. 2

ANY TREPECTION OF PLATES OF HOLOROD BY (1) SMALL BE FOR MURES AND FETT 2002 SEC. 3 OF HIT BRUSS COMPONENT

ROWANING INDICATES. ACCIPIANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOULLY FOR HIT BRUSS COMPONENT **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPTHG, INSTALLING AND BRACTHG. RETER TO BEST. (BUILDING COMPONENT SAFLY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 HORTH LLE STREIT, SUITE 312, ALEXANDRÍA, VA. 22314) AND MICA (MODO TRUSS COUNCIL O' AMERICA, 6300 ENTERPRISE LANE, MADISON, HI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HYSE FUNCTIONS. DWICKS OFHERWISE HOLDING FOR THE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SHALL HAVE $2.5 \times 6 (B1) =$ 6-0 MILMOTH R=438 U=180 W=3.5" 1.5X4 0 Design Crit: 12 Ö 3 X 4 (R) Ⅲ 6X61117 4 X 4 ≡ 0-0 R=1770 U=220 W=3.5" AS1) ~ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) H 0 ò -41-6 4 X 8 ≡ 5×6≡ Ö 0ver IS THE RESPONSIBILITY OF 7-6 W 0 Supports 5 X 8≡ 3 X 4 ≡ 32-6-0 4 X 6 == 110 mph wind, 15.00 ft mean hgt, ASCE 7 located within 6.50 ft from roof edge, 0 DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Right end vertical not exposed to wind (A) Continuous lateral bracing equally spaced on member 7-8-14 1.5X4 III 5X12# O WENSENS O Ng/59687 CORIOR TATE OF 3 X 5 ≡ '07 ENGINEER . 1.5X4 III 9-3-2 €X8≡ R=1344 U=180 1.5X4 III 3 X 8 ≡ BC DL BC LL TC DL DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/ (L) 7-02, CLOSED CAT II, EXP pressure 8-0-0 10 0 10.0 10.0 20.0 1.25 40 0.0 . B, wind TC PSF PSF PSF PSF PSF 9 N FROM SEQN-DATE REF HC-ENG DRW HCUSR487 07030002 Scale R487--=.125"/ft. MNM/AF 01/30/07 149308 93371

SPACING

24.0"

JREF-

Top chord 2x4 SP // Bot chord 2x4 SP // Webs 2x4 SP // (J) hänger condition. Haines City, FL 33844 PLT In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. (6-395--Stanley Crawford Construc TYP. ALPINE Wave connection not found in inventory file for this Provide connection. #2 Dense #2 Dense #3 **MAPORTANT**TURNISH A COPY OF THIS BESIGN 10 THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS BESIGN, ANY FAILURE TO BUILD THE THISS IN CONFIDENCE AT HIS PET 10 FAR FARRICALIG. AND HELG. SHIPPING. HISTALLIGA BRACING OF TRUSSES, ALPINE DESIGN CONFIDENCE AT HIS PET 10 FACH FACE OF THE PROPESSIONS OF HOS (MATIONAL DESIGN SPEC, BY ALEAN) AND PET. APPLY DIALES TO FACH FACE OF TRUSS AND, HUNESS OTHERNISE LOCATED ON HIS DESIGN, POSITION FOR BRAHMES 160A 2. ANY HISSFECTION OF PLATES OF THE NEW 15 NOT AND HIS DESIGN AND AND OF PLATES OF DIABATIC OF PROFESSIONAL ENGLINE AS OF PET 2007 SEC. 3. AS ALO IN HIS DESIGN SHOWN. HE SULFAMELLY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/PET I SEC. 2. **WARNING** RUSSES BLOURE CYREME CARE IN FABRICATION MANDEING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPORTER) SAFETY RUSPORATION, PROBLEMED BY TPT (RUSS PLATE INSTITUTE, 21B 10871 LET SIRET, SHITE 312. ALEXANDRIA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OT AMERICA, 6300 ERIESPENTS LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HUSS FUNCTIONS. UNLESS OTHERNISE HOLDER, AND SOME AND SOME HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SWALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SWALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SWALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SWALL HAVE $2.5X6(B1) \equiv$ 1-6-0 R = 436MILMOTH 1.5X4 Ⅲ 9 U = 1803X4 // -1-12 Ö Design Crit: Ö W=12" 3 X 4 ≡ 6X6៕/ 4 X 4 ≡ -0-0 2-0-0 1 R-1773 U-234 W-3.5" AS1A) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 6 4 X 8 ≡ 5 X 6 ≠ 0 0ver 7-6 W 0 Supports 5 X 8 **≡** $3 \times 4 \equiv$ 32-6-0 3 X 6 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Right end vertical not exposed to wind (A) Continuous lateral bracing equally spaced on member 5-8-14 1 8×5≡ $2.5 \times 6 \equiv$ MENSE 0.59687 4 X 5 ≡ _ 07 E OF 4 X 4 == ú Ń R=1344 U=180 1.5X4 4 X 4 ≡ BC DL TC TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-PL (L) pressure 8-0-0 10-0 40.0 10.0 20.0 10.0 1.25 0.0 PSF PSF PSF PSF PSF 2 FROM SEQN-HC-ENG DATE REF DRW HCUSR487 07030003 Scale = .125"/Ft. R487--MNM/AF 01/30/07 149312 93372

FI Certificate of Authorization # 567

SPACING

24.0"

JREF -

ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization 4 567 PLT Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. (J) hanger connection not found in inventory file for this condition. Provide connection. (6-395 - Stanley Crawford Construc WILMOTH TYP. ALPINE Wave **MARNING** TRUSKES BEODINE EXTREME CARE IN FABRICATION, INAUDING. SHIPPING, INSIALLING AND BRACING, BETTER 10 BCS1. (BUILDING COMPONENT SAFETY IN ORBATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LLE STREIT, SUITE 112, ALEXANDRIA, NA, 2214) AND HICA (MOOD TRUSS COUNCIL OF AMERICA. 6300 CHICERRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR 10 PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED OF PRIORS SHEETY PROFERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED RIGHD CLILING. **IMPORTANT** "URLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, THE. SHALL HOT BE RESPONSIBLE FOR ANY DETAILOR THAN THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPONENTS OF THE THIS PROPERTY OF THE TRUSSES.

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DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY ATRAYA AND TELL APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY ATRAYA AND TELL APPLY CONTROL OF THE SALE AND OF TAYING FOR THE TRUSSES.)

PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN POSSITION PER REMAINGS 160A 2

ANY IMPRECISION OF PARTS FORLUSION BY (1) SHALL BE FER ANDEX AS OF TELL SPEC).

A SEAL ON THIS DESIGN PORT OF THE STANDARD OF THE PROPERTY OR THE TRUSS COMPONENT OF THE PROPERTY OF THE TRUSS COMPONENT OF THE TRUSS DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/IPT 1 $2.5 \times 6 (B1) =$ **0**0 R 437 SUITABILITY AND USE OF THIS COMPONENT 1.5X4 Ⅲ 9-1-12 U-180 W-3.5" 9 3×4/ 0 Design Crit: 0 7 ex6#/ $3 \times 4 \equiv$ $4 \times 4 \equiv$ 0-0 R-1771 U-252 W-3.5" F-0-0 AS18) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 41 ANY BUILDING 0 4 X 8 ≡ 5×6= 0 0ver 3H1 S1 6 ယ RESPONSIBILITY OF THE 0 Supports 5 X 8 == 3 X 4 **=** 32-6-0 3 X 6 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED located within 6.50 ft from roof edge, CAT II, EXP DL=5.0 psf, wind BC DL=5.0 psf. 3 8 14 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Right end vertical not exposed to wind (A) Continuous lateral bracing equally spaced on member 5 X 5 🚚 WHUR R. CENSE STATE OF 1.5X4 III u 6.59687 5 X 10 ≡ * 107 w NE NEER N R=1344 U=180 1.5X4 III 5 X 4 **≡** BC LL BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-(آ pressure 8-0-0 _10-0-0 20.0 40.0 10.0 1.25 10.0 PSF 0.0 B, wind TC 6 PSF PSF PSF PSF 2 FROM SEQN DATE REF DRW HCUSR487 07030004 HC-ENG Scale = .125"/Ft. R487--JFB MNM/AF 01/30/07 149316 93373

SPACING

24.0"

JREF-

Haines City, FL 33844

FI Certification of Authorization 4 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 10 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. 8 (6-395--Stanley Crawford Construc WILMOTH Continuous lateral bracing equally spaced on member. TYP. Ġ ALPINE Wave 1-6-0 $3X4(A1) \equiv$ R=452 U=180 W=3.5' **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, LUC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BRILL THE TRUSS IN CONTRIBUTE WITH THE PERSONS OF THE CONTRIBUTE WITH THE PERSONS OF THE CONTRIBUTE WITH THE PERSONS OF THE CONTRIBUTE WITH APPLICABLE PROVISIONS OF THOS (MAIDNAL DESIGN SPEC, BY ATAPA) AND THE CONTRIBUTE AND CONTRIBUTE OF THE CONTRIBUTE AND CONTRIBUTE WITH APPLICABLE PROVISIONS OF THE CONTRIBUTE AND CONTRIBUTE AND CONTRIBUTE AND CONTRIBUTE OF THE DRAWLING SPECIAL APPLY BLATES TO EACH FACE OF TRUSS AND, MULTES OTHERNISE LOCATED ON THIS DESIGN, POSITION FOR DRAWLING SPECIAL ON THIS DESIGN AND CONTRIBUTE OF THE DRAWLING SPECIAL ON THE PERSON OF THE AND CONTRIBUTE AND THE PERSON OF THE PERSON DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMP BUILDING DESIGNER PER ANSI/IPI 1 STC. 2. A PROPERLY ATTACHED RIGID CEILING 1.5X4 9 Ö 12 ò 3 X 4 (R) Ⅲ 17 Design Crit: 11 8 X 8 3 X 4 ≡ 0-0-0 R=1629 U=201 W=3.5" \sim Ó Ö AS2) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 38-6-0 4 X 8 = 5×5≡ 0ver 3 Supports Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ 29 5 X 8 **≡** 6-0 $3 \times 4 \equiv$ 3×5≡ ۵ THUR R. 5X5# Jan 31 '07 E FAIS o. 59687 ഗ ű $\widehat{\mathbb{A}}$ 'n 4 X 8 ≡ 5 X 5 🚚 BC DL BC LL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/g 10-0 R=1222 U=180 W=3.5" 40.0 20.0 1.25 10.0 PSF 0.0 10.0 PSF 1.5X44 X 5 🥠 PSF PSF PSF 8-0-0 FROM SEQN-DATE REF HC-ENG MNM/AF DRW HCUSR487 07030005 Scale =.1875"/Ft. 10 0 R487--JFB 01/30/07 149328 93374

567

SPACING

24.0"

JREF -

1T4G487 Z01

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Haines City, FL 33844

FI Certification of Authorization 4 10 (A) 2x4 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" 0C. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (6 395 Stanley Crawford Construc WILMOTH Ġ TYP. ALPINE Wave **√**1-6-0 3X4(A1) =R=452 U=180 W=3.5" * # 567 **IMPORTANT** FURBLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP. HEC. SHALL HOT BE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN; MAY FAILURE TO BUILD THE TRANS IN CONTRIBUTED, HIS DESIGN CONTRIBUTED, AND THE PLEASEE PROVISIONS OF 1005 (MATIONAL DESIGN SPEC, BY AFAPA) AND THE DESIGN CONTRIBUTES. ALPHAY CONTRIBUTE ARE ARE OF 70/19/16/GA (M.H/SKY) ASTA AGES GRADE 40/60 (M.K/H/SS) GALV. SITEL. APPLY PLAIES TO EACH FACE OF TRUSS AND. UNILESS OTHERWISE LOCATED ON HIS DESIGN, POSITION FER DRAMINGS 160A Z AND THIS DESIGN OF PALES TOLLOWED BY 1) SHALL BE FER ANNEX AS OF THIS 2002 SEC. 3. A SEA, ON THIS DESIGN AND THE MILITARLITY AND THE PARTIES COMPONENT DESIGN SHOWN. HE SULFABLLITY AND USE OF THIS COMPONENT DESIGN SHOWN. HE SULFABLLITY AND USE OF THIS COMPONENT FOR THE RESPONSIBILITY OF THE BUILDING DESIGN PER ANSI/TPI I SEC. 2. **WARNING** RUSSAS REQUIRE EXTREME CARE IN FARRICATION HABIDEIDG, SHIPPING, INSTALLING AND BRACHIG. REFER TO RESS. (RUSTONIC COMPONING SAFELY INFORMATION), PUBLISHED BY TPT (RUSS PLATE INSTITUTE, 21 BE 187. ALEXANDRA, VA, 22314) AND WICA (ROOD TRUSS COUNCI O MERICA, 6300 ERHERS FORMATION, WILL SAFELY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. WHEELS, OTHERHYS LIGHT, ANDISON, WILL SAFELY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. WHEESS OTHERHYS LIDICATED FOR COMES SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE A PROPERTY ATTACHED REGION CHORD SHALL HAVE 1.5X4 9-1-12 9 -0-0 3 X 4 (R) 17 Design Crit: 0X6 0-0-0 3 X 4 **≡** R=1629 U=204 W=3.5" 0-0 AS3) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 38-6 5 X 5 == 4 X 8 == Ö 0ver 3 Supports 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Right end vertical not exposed to wind In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C. 12-11-2 3 X 4 ≡ 29-6-0 3 X 5 ≡ 4 X 8 ≡ 5×6# * α BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. 6 FL/-/4/-/ -14 pressure R-1222 U=180 W=3.5" 10.0 20.0 40.0 10.0 1.25 24.0" 0.0 1.5X4 4 X 5 // PSF PSF PSF PSF PSF 0 FROM DATE REF SEQN-DRW HCUSR487 07030006 JREF - 1T4G487_Z01 HC-ENG 8-0-0 Scale =.1875"/Ft 10-0-0 R487--JF B MNM/AF 01/30/07 149332 93375

ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT 10 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. (6-395--Stanley Crawford Construc WILMOTH TYP. Ġ ALPINE Wave 1-6-0 $3X4(A1) \equiv$ R=451 U=180 W=3.5" **IMPORTANT**FURHISH A COPY OF THIS DUSIGN TO THE THISTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, JUG. SHALL NOT HE RESPONSIBLE FOR MAY DEFINATION FROM HIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN COMPONENTS WITH HAPPLICABLE PROVISIONS OF THIS SECONDECTION SECOND AREA AREA THE PROVISIONS OF THIS SECONDECTION OF AREA AREA THE PROVISIONS OF THIS SECONDECTION OF AREA AREA THAT HAPPLICABLE PROVISIONS OF THIS SECONDECTION FOR AREA AND THIS COMPONENT HARDE OF THE THAT AREA THAT **MARNING** IRUSSES REQUIRE EXIRCHE CARE IN FARRICATION. IMADELING, SHIPPING, INSTALLING AND BRACING.
LETER TO BEST (BUILDING COMPONENT SAFLY INFORMATION), PUBLISHED BY TET (TRUSS PLAIT INSTITUTE, 218
HORTH LEE STREET, SUITE 317. ALEXANDRAL, VA, 22314) AND MICA (1000) TRUSS COUNCEL O MERICA, 6300
ENTERPRISE LANE, MADISON, MI 53719) FOR SAFLY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNLESS
OTHERMISE INDIVALATE FOR COMBO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGIO CELLING. DESIGN SHOWN. THE SUTTABILITY BUILDING DESIGNER PER ANSI/IPI 1 1.5X4 9 0 12 0 3X4(R) III 17 Design Crit: 6X6# 3 ¼ 4 ≡ 0-0 R=1548 U=214 W=3.5" 2 Ö 0 AS4) TPI=2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 36-6-0 0ver 3 X 4 ≡ 5×8≡ 3 Supports Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 27 12-11-2 6-0 4 X 8 **≡** .5X4 III $3 \times 5 \equiv$. 59687 5 X 8 ≡ $3 \times 4 \equiv$ 0 6 BC DL BC LL TC DL TC LL SPACING DUR.FAC. TOT.LD. 14 FL/-/4/-/-/R/ R=1138 U=180 W=3.5" 1.5X4 4 X 4 // 40.0 10.0 20.0 24.0" 1.25 10.0 0.0 PSF PSF PSF PSF PSF 8-0-0 $_{10-0-0}$ SEQN DATE REF JREF -HC-ENG DRW HCUSR487 07030007 Scale =.1875"/Ft. R487--1T4G487_Z01 MNM/AF 01/30/07 149337 93376

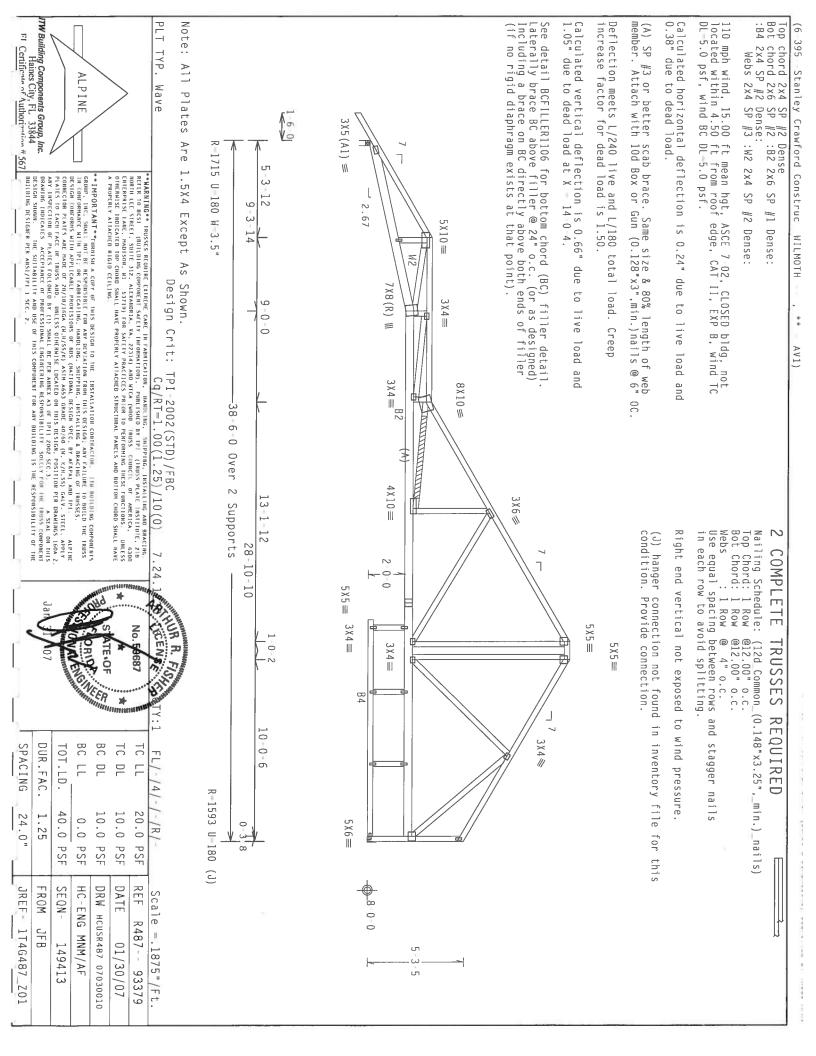
ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 PLT 10 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. (6·395--Stanley Crawford Construc WILMOTH TYP. Ġ ALPINE Wave 1-6-0 $3X4(A1) \equiv$ R=452 U=180 W=3.5" **IMPORTANT** FURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITS BUILDING COMPONENTS GROUP, MC. SMALL HOLD BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY ALLINE TO BHILD THE TRIES. IN COMPONENT WITH PER PER PROPERTY OF THE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST. (BUILDING COMPOULNE SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, ZIB WORTH LEE SREET, SUITE INST. ALEXANDRIA, NA., 22314) AND MICA (MODD TRUSS COUNCIL OF ARREAGA. 6300 CHIELRERISE LAME, MADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS OTHERWISE INDICATED OF CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE 1.5X4 9-1-12 9 ò 0 3X4(R) III 17 Design Crit: 11 8 X 8 3 X 4 ≡ ò 0 R=1546 U=213 W=3.5" 2 ò 0 AS5) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 36 - 6 - 0 0ver $3 \times 4 \equiv$ 5 X 8≡ ω Supports Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 27-6-0 4 X 8 == .5X4 III 3×5≡ THE YOR TE(OF 07 5 X 8 **≡** 3 \ 4 == 9 6 BC DL BC LL TC DL SPACING DUR.FAC. 14 TC TOT.LD. FL/-/4/-/ R=1139 U=180 W=3.5" 1.5X4 4×4// 40.0 10.0 20.0 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF PSF 10-0-SEQN DATE REF JREF - 1T4G487_Z01 HC-ENG DRW HCUSR487 07030008 Scale =.1875"/Ft. R487--MNM/AF 01/30/07 149341 93377

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / Haines City, FL 33844
FI Certificate of Authorization # 567 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. PLT TYP. 10 Shim all supports to solid bearing (B) 1×4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. (6-395--Stanley Crawford Construc WILMOTH خ ά ALPINE Wave $3X4(A1) \equiv$ #2 Dense #2 Dense #3 R=448 U=180 W=3.5" **HARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION, INAUDILING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SATELY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUHE 127. ALEXANDRIA, NA, 22214) AND MICHA (HODD TRUSS COUNCIL OF AMERICA, 6300 ERICEPRESE LAWE, MADISON, HI 53719) FOR SATELY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OBJECTION OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGIO CELLING. BUILDING DESIGNER PER 1.5X4 9 ò ò 3 X 4 (R) Ⅲ 17-3-12 Design Crit: #9X9 $3 \times 4 \equiv$ R=1635 U=203 W=3.5" 0-0-OF THIS COMPONENT A1) TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 38 φ 0 4 X 8 ≡ 5 X 5 **■** Over 3 Supports -3-0 3 X 6 ≡ A SEAL ON THES
OR THE TRUSS COMPONENT
RESPONSIBILITY OF THE 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Right end vertical not exposed to wind pressure. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (A) 2x4 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" 0C. 12 $3 \times 4 \equiv$ ယ် 24 -10 5 X 5 == (B) 2 SNSE (B) 0 5. 59687 5X12≡ Ö '07 5 X 8 == 11-8 I MAIL PAIL BLEV THE OF TENDER OF STREET 00 BC DL TC DL JC DUR.FAC. TOT.LD. 10 FL/-/4/-/-/R -10 3X4 /// 5 X 5 🗐 R=1225 U=180 W=3.5" 12-3-8 40.0 10.0 20.0 10.0 1.25 0.0 1.5X4 III PSF PSF PSF PSF PSF 4X4(R) / 8-0-0 10-0-0 DATE REF FROM SEQN-DRW HCUSR487 07030009 HC-ENG Scale = .1875"/Ft - אווו ברמאו וא מזווווממר R487--MNM/AF 01/30/07 149347 93378

SPACING

24.0"

JREF



Top chord 2x4 SP / Bot chord 2x6 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 PLT See detail BCFILLER1106 for bottom chord (BC) filler detail Laterally brace BC above filler @ 24" o.c. (or as designed) Including a brace on BC directly above both ends of filler (if no rigid diaphragm exists at that point). Calculated horizontal deflection is 0.22" due to live 0.36" due to dead load. Calculated vertical deflection is 0.42" due to live load and 0.67" due to dead load at X = $16 \cdot 0 \cdot 4$. (A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" Wind reactions based on MWFRS pressures (6-395--Stanley Crawford Construc WILMOTH TYP. ALPINE Wave 1.60 3X8(A1) =#2 Dense #2 :B4 2x4 SP #2 Dense: #3 R=1715 U=180 W=3.5 **IMPORTANT**FURBISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, MC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRIES. IN COMPONENT CHIEF TO BE FLABELFULTIC, MANDLE HOLE, SHEPPIG, HISTALLING A BRACHEO OF TRIESES. ALPINE DESIGN CONTRARS WITH APPLICABLE PROVISIONS OF HIS SHATIONAL DESIGN SPEC, BY ATRAPA AND PP.

DESIGN CONTRARS ARE AND OF POLICE FROM STATEMENT OF HIS DESIGN SPEC, BY ATRAPA AND PP.

EVALUATE TO EACH FACE OF TRUES AND. BUILESS OTHERNISE LOCATED ON HIS DESIGN, POSITION FOR BRAHINGS 160A Z.

ANY HISPECTION OF PLATES TOLLOWED BY (1) SMALL BE FER ANIREX AS OF TPIT-2002 SEC.).

ASSAL ON HISPECTION OF PLATES TOLLOWED BY (1) SMALL BE FER ANIREX AS OF TPIT-2002 SEC.).

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ASSAL ON HISPECTION OF PLATES TOLLOWED BY (1) SMALL BE FER ANIREX AS OF TPIT-2002 SEC.).

ASSAL ON HISPECTION OF PLATES TOLLOWED BY (1) SMALL BE FER ANIREX AS OF TPIT-2002 SEC.).

ASSAL ON HISPECTION OF PLATES TOLLOWED BY (1) SMALL BE FER ANIREX AS OF TRIES TO THE RESPONSIBILITY OF THE PROPERSION OF THE STATE ASSAL ON THIS DESIGN SHOWN.

BRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLINEERING RESPONSIBILITY SOLETY FOR THE TRUES COMPONENT OF THE STATE ASSAL ON THIS DESIGN SHOWN.

HE SUITABLITY AND DESCRIPTION OF THE STATE ASSAL OF THE STATE ASSAL ON THIS DESIGN SHOWN. A PROPERLY ATTACHED RIGID CEILING 3-12 1.5X4 Ⅲ 9 3X4 // ώ 14 3 X 4 ≡ 6X10 ≡ Design Crit: 8X12≡ 1.5X4 III AV2) 0 - 0 load $3 \times 4 \equiv$ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) and 38-6-0 $3 \times 4 =$ 8X14 ≡ 0ver 1.5X4 Ⅲ 7 X 1 0 ≡ 2 Supports 9 ဌာ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. ģ Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (J) hanger connection not found in inventory file for this condition. Provide connection. Right end vertical not exposed to wind pressure 5 X 8 // 28 10 - 105 X 5 ≡ 5 X 5 == 5 X 8 ≡ Jan 31 '07 CENS 4-4-7 F 04 EN SER 5 X 6 ≡ $3 \times 4 \equiv$ * BC DL SPACING TC DL C DUR FAC 8-4-3 TOT.LD. FL/-/4/-/-/R/ 4 X 5 // Е 1.5X4 III 1.5X4 III (E) R=1593 U=180 (J) 84 24.0" 40.0 10.0 20.0 10.0 1.25 0.0 1.5X4 III $3 \times 4 =$ PSF PSF PSF PSF PSF 5 X 6 == $\widehat{\geq}$ SEQN-DATE REF JREF-FROM DRW HCUSR487 07030011 HC-ENG Scale 8-0-0 R487--1T4G487_Z01 JF8 =.1875"/Ft. MNM/AF 01/30/07 149429 93380

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / Haines City, FL 33844
FI Certificate of Authorization # 567 Calculated horizontal deflection is 0.12" due to live load 0.20" due to dead load. Wind reactions based on MWFRS pressures. 10 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50 (6 395 Stanley Crawford Construc WILMOTH Continuous lateral bracing equally spaced on member TYP. 12 ALPINE Wave 1-6-0 3X8(F2) ≡ #2 Dense #2 Dense #3 R=1715 U=180 W=3.5 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BUILDING COMPONENTS GROUP. THE SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS OFSIGN, ANY FAILURE TO BUILD THE RUSS IN CONTORNANCE WHITH PET OR FABRICATION, HANDLES, SHEPPING, INSTALLING A BRACKING OF RUSSES.

DESIGN CONTORNAS WITH APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY AFAPA) AND 1P1. ALPTHE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMP BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. CONNECTOR PLAIES ARE HADE OF 20/18/16GA (W.H/SS/X)
PLAIES TO EACH FACE OF TRUSS AND, UNIESS OTHERWIST
ANY INSPECTION OF PLATES FOLLOWED BY (1) SWALL BE I
DRAWLING INDICATES ACCEPTANCE OF PROFESSIONAL ENGI **MARNING** IBUSSES REQUIRE EXTREME CARE IN FABRICATION, IMMOLIDE, SUIPPING, INSTALLING AND BRACTIG. RETER TO BEST I QUILDING COMPONENT SAFETY INFORMATION), POUR LISTED BY FFT (TRUSS FLATE INSTITUITE, ZIB HORBIT LEE SHELT, SUITE 312, ALEXANDRIA, VA, Z2314) AND WITCA (MODD TRUSS COUNCIL OF AMERICA, GOOD CRISSES CHAIL, HAVE BEFORELY ATTACHED STRUCTURAL FAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL FAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL FAMELS AND BOTTOM CHORD SHALL HAVE 9 ये.6¾ ॥ -3-14 -3-12 3×4// Design Crit: 7x6(R) 📗 5×6≡ AV3) TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 1.5X4 III 4 X 8 ≡ and 0-0 -38-6-0 Over 2 Supports THIS DESIGN, POSITION PER DRAWINGS 160A.
OF TPI1 2002 SEC.3.
A SEAL ON THI
MSIBILITY SOLELY FOR THE TRUSS COMPONEN 5X8(R) ||| 6X12= IS THE RESP DRISIBILITY OF THE 7 - 5 6X10加 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. ά 29 -2-2 $4 \times 10 (**) =$ 4 X 5 ≡ 5 X 5 ≡ GCENS 4-4-7 TATE OF o. 5968. 5 X 6 ≡ 3 X 4 ≡ * BC LL BC DL TC DL DUR.FAC. TC LL ∞ TOT.LD. 4 X 4 ≡ FL/-/4/-3 X 4 🕼 R=1593 U=180 W=3.5" /-/R/ 40.0 10.0 10.0 20.0 1.25 0.0 4 X 5 // 2X4 III PSF PSF PSF PSF PSF 8-0-0 FROM SEQN-DATE REF HC-ENG MNM/AF DRW HCUSR487 07030012 Scale =.1875"/Ft. L10-0-R487--JF8 01/30/07 149433 93381

SPACING

24.0"

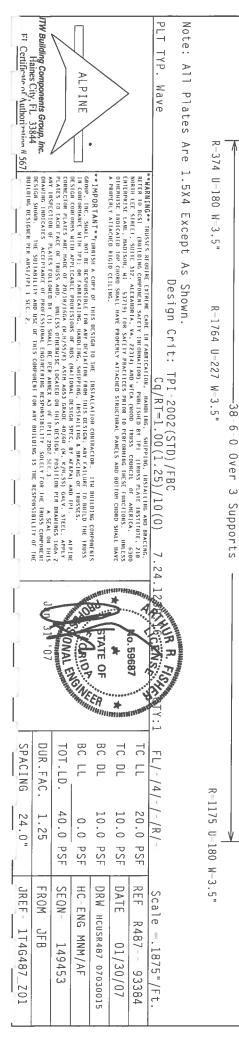
JREF -

ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization 4 567 Bot PLT Calculated horizontal deflection is 0.12" due to live load and 0.19" due to dead load. Wind reactions based on MWFRS pressures. 10 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (6-395--Stanley Crawford Construc WILMOTH p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP TYP. 12 ALPINE Wave 160 3X8 (F2) ≡ #2 Dense #2 Dense #3 R-1716 U-180 W-3.5' **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVALUENT FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IT COMPONENTS WITH APPLICABLE PROVISIONS OF THIS STATEMENT, HADDING, HISFALLING A BRACHING OF TRUSSES. ALPHIN CONTRICTOR PLATES ARE THE OF ZOLFD HOGA (M. HISFALX) AND THE CONTRICTOR PLATES ARE THOSE OF ZOLFD HOGA (M. HISFALX) ASTAIN AND THE STATEMENT OF ZOLFD HOGA (M. HISFALX) AND THE STATEMENT OF THIS DESIGN AND STATEMENT OF THIS DESIGN AND STATEMENT OF THIS DESIGN AND STATEMENT OF THE STATEMENT OF THE STATEMENT OF THIS DESIGN AND STATEMENT OF THE STATEMENT A PROPERLY ATTACHED RIGID CEILING Q 4.6¾4 Ⅲ -3-14 3×4/ 11 - 3 - 12Design Crit: 5X5≡ 3X4≡ €X6= AV4) TPI-2002(STD)/FBC Cq/RT=1.00(1.25), 38-6-0 Over 2 Supports 9-0 3 X 4 ≡ 0 16-11-2 4 X 6 ≡ 3 X 4 ≡ /10(0)€X6≡ In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Right end vertical not exposed to wind pressure 110 mph wind, 15.00 ft mean hgt, located within 4.50 ft from roof DL=5.0 psf, wind BC DL=5.0 psf. (A) Continuous lateral bracing equally spaced on member. ഗ Ġ 9 6X6**≡** 5 X 8 = Jan 31 '07 CEMSE 0-0 4-4-7 4 5 X 5 ≡ ATE OF . 59687 RIDA 5 X 8 ≡ 3 X 4 ≡ * ASCE 7-02, CLOSED bldg, not edge, CAT II, EXP B, wind TC φ BC DL TC DL SPACING TC LL DUR.FAC. ∞ TOT.LD. FL/-/4/-/-/R/ 4 5 X 5 (R) # 3 X 4 /// R-1597 U-180 W-3.5" 2-3-8 5X4(R) / 24.0" 40.0 10.0 20.0 10.0 1.25 0.0 2X4 III PSF PSF PSF PSF PSF 8-0-0 FROM SEQN-DATE REF HC-ENG JREF -DRW HCUSR487 07030013 Scale = .1875"/ft 0 R487--MNM/AF 01/30/07 149437 93382

FI Certificate of Authorization # 567 Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load. Wind reactions based on MWFRS pressures Top Bot Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. PLT TYP. 10 (6-395--Stanley Crawford Construc WILMOTH p chord 2x4 SP / t chord 2x4 SP / Webs 2x4 SP / 12 ALPINE Wave 1-6-0 3X8 (F2) ≡ ||2 Dense ||2 Dense ||3 R=1716 U=180 W=3.5 **IMPORTANT**TURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP. THE SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION ROOM THIS SIGN; ANY FAILURE TO BUILD THE TRUSS IN CONTORNANCE WITH PT; OR FARRICATION, DANDLING, SHIPPING, INSTALLING A BRACING OF RHISSES.

DESIGN CONTORNS HITH APPLICABLE PROVISIONS OF INDS (MAISTONAL DESIGN SPEC, BY ATRIA) AND TEL. ALFINE CONNECTOR TAKES ARE MORE OF ZO/TRYTONA (M. 11854), ASTA GOS GRADE 40/50 (H. X.H. %S) GALV. STEEL APPLY PLATES TO EACH TACE OF TRUSS AND, UNITES OTHERNISE LOCATED ON THIS DESIGN. POSITION PER BRACHINGS 160A-7 ANY INSPECTION OF PLATES OF LOUGHDE BY (1) SHALL BE FER AMERY AS OF TELL SPECIAL FOR THE TRUSS COMPONENT CONTRACTOR OF THE TRUSS OF PROTESSIONAL ENGINEERING RESPONSIBILITY SOLECT FOR THE TRUSS COMPONENT **WARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BRITCHIS COMPONENT SAFETY INFORMATION), PUBLISHED BY TPT (TRESS PLATE INSTITUTE, \$710 MORTH LEE STREET, SUITE 137. ALEXANDRA, VA, 22134) AND WICA (WOOD TRUSS COUNCEL OF AMERICA, 6300 ETHERMESTS LANE, HADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HESE FUNCTIONS. UNLESS OTHERMISC INDICATED FOR CONDO SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER ANSI/IP1 1 9 थे.6¾ ॥ ú 3×4/ 14 13 3×5# R ANSI/IP1 1 SEC. 2. ú 12 Design Crit: 6X12≡ .5X4 III AV5) 5 X 6≡ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 38-6-0 16-11-2 1.5X4 III 9 4 X 8 ≡ 0ver 0-0-IS THE RESPONSIBILITY OF 3 X 6 **=** 2 Supports 6X12≡ 1.5X4 III 110 mph wind, 15.00 ft mean hgt, ASCE 7 located within 4.50 ft from roof edge, DL=5.0 psf, wind BC DL=5.0 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Right end vertical not exposed to wind pressure (A) Continuous lateral bracing equally spaced on member. w 5 9 \cong TO HUR A. 5 X 6≡ 5 X 8≡ CENSE 0 - 0 5×5≡ 4 ATE OF ľ. 59687 4 5 X 8 **=** $3 \times 4 \equiv$ * ά BC LL TC DL ВС SPACING DUR.FAC C ထ TOT.LD. 믿 4 7-02, CLOSED CAT II, EXP ယ် 5 X 5 (R) # 3 X 4 /// /4/-/-/R/ R=1597 U=180 W=3.5" 5 X 4 (R) // 24.0" 10.0 10.0 20.0 40.0 ώ 0.0 2X4 III . 25 bidg, not B, wind TC PSF PSF PSF PSF PSF 10-0-8-0-0 JREF-FROM SEQN-DATE REF HC-ENG DRW HCUSR487 07030014 Scale R487--1T4G487_Z01 =.1875"/Ft. MNM/AF 01/30/07 149441 93383 ဌာ

Top Bot See detail BCFILLER1106 for bottom chord (BC) filler detail Laterally brace BC above filler @ 24" o.c. (or as designed) Including a brace on BC directly above both ends of filler (if no rigid diaphragm exists at that point). 10 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures (A) (6-395--Stanley Crawford Construc WILMOTH p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP Continuous lateral bracing equally spaced on member. 12 #2 Dense #2 Dense #3 3×5# 4 X 5 AV6) 5×5= $\widehat{\mathbb{A}}$ 3 X 4 ≡ Shim all supports to solid bearing. Right end vertical not exposed to wind pressure. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $\binom{**}{p}$ 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. 5×5= , _5x5≡ 5 X 6 ≡ 3 X 4 ≡ 1 0 3X4/// 5 X 5 #4 **→**10. ò



3X4(A1) =

2.67

5 X 5 (R)

=

4 X 8 =

3 X 6 ≡

4 X 10 ≡

5×5≡

6X8**≡** 5X5≡

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Ö Ö ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Note: All Plates 10 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Wind reactions based on MWFRS pressures PLT TYP. $\widehat{\geq}$ (6-395--Stanley Crawford Construc WILMOTH ن Continuous lateral bracing equally spaced on member ά ALPINE Wave 3X4(A1) =Are R=459 U=180 W=3.5" 3X4 Except As Shown. A PROPERLY ATTACHED RIGID CEILING 1.5X4 III 9 -1-12 Ö Ö Design Crit: 5 X 5 // 5 X 4 **≡** -3-12 R=1621 U=187 W=3.5" 0-0 A1A) 2.5X6# SE LOCATED ON THIS DESIGN, POSITION PER BRANDIGS 160A-Z PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS THEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 38 6-0 5 X 5 ≡ 4 X 8 ≡ Over 3 Supports 3 X 6 ≡ \geq 110 mph wind, 15.00 ft mean hgt, located within 4.50 ft from roof DL=5.0 psf, wind BC DL=5.0 psf. Shim all supports to solid bearing In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C. Right end vertical not exposed to wind pressure 2 ú 10 Ξ 6X6≡ CENSE No. 59687 -0 5 X 5 ≡ Ô 5 X 8 == R ≯ -11-8 ASCE 7-02, CLOSED bldg, not edge, CAT II, EXP B, wind TC ∞ BC DL TC DL TC LL DUR.FAC. -10-10TOT.LD. FL/-/4/-/-/R/-5 X 5 # R=1228 U=180 W=3.5" 2-3-8 40.0 10.0 20.0 1.25 0.0 10.0 PSF 1.5X4 **Ⅲ** 4 X 4 /// PSF PSF PSF PSF 8-0-0 to FROM SEQN-DATE REF HC-ENG DRW HCUSR487 07030016 Scale 0 R487--=. 1875"/Ft MNM/AF 01/30/07 149496 93385

BUILDING DESIGNER PER AUSI/IPI 1

SPACING

24.0"

JREF -

ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization 4 PLT Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense: Bot chord 2x6 SP #1 Dense :B1 2x8 SP SS: :B2 2x8 SP #1 Dense: Webs 2x4 SP #3 :W2, W5, W9 2x4 SP #2 Dense: Wind reactions based on MWFRS pressures. (A) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" 0C. (B) SCAB BRACE. 80% LENGTH OF WEB MEMBER. SAME SIZE, SPECIES & GRADE OR BETTER. ATTACH WITH 0.128"x3" NAILS @ 4" OC. (C) $1x4\ \#3$ or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. (6-395--Stanley Crawford Construc WILMOTH TYP. ALPINE R=200 U=0 W=3.5" Wave $5X5(A1) \equiv$. # 567 1.5X4 ₩ 0-0-0 В1 **IMPORTANT**FUNDISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, THE. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD HE HUSS IN CONTRIBUTE WHILL PLAY FOR FABRICATION, HAVE LIGHT SHALL HAD, A BRACHING OF BUSSES, IN CONTRIBUTE WHILL PLAY FARE PROVISIONS OF DRDS (HATIONAL DESIGN SPEC, BY AREA) AND IPI. CONNECTOR PAIRTS AND THE PROVISIONS OF DRDS (HATIONAL DESIGN SPEC, BY AREA) AND IPI. CONNECTOR PAIRTS AND THE SHALL HAD BY A STATEMENT OF THE BRAYHINGS IGAA. SHALL APPLY PLAYES TO LACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHINGS IGAA. ANY INSPECTION OF PLAYES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PIL 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FIL 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FIL 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FIL 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FIL 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALES ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. A SCAL ON THIS BRAWHING INDICALS ACCUPANCED BY (1) SHALL BE PER ANNEX AS OF FILE 2002 SEC.3. **WARNING** IRUSKIS BEDUIRE EXTREME CARE IN FABRICATION, DANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BESS! (BUILDING COMPONENT SAFETY REPORATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, ZIB MOBIN LEE STREE, SUITE 312. ALEXANDRIA, VA. 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, GOOD ENTERS COUNCIL OF AMERICA, GOOD ENTERPRISE LAND, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNITESS OTHERWISE, INDIVIDENTATION OF A PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER -9-0-0 11 - 35X12(R) III 6X12(R) ₩ ¥2 $12X14 \equiv$ Design Crit: 5X12(R) III -8415 U=580 W=3.5" 82 8X12(R) W 2 5 X 8 (R) III Ö HT7A) Ô 8X10(R) W 12 7 X 6 == TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 1.5X4 III $7 \times 10 \equiv 1 - 0 - 0$ 0 - 0 - 0-41-6-0 Over 2 Supports 10X10(R) /// 4 X 6 (R) III 2 6 ò -6-0 0-1 3X4 =7 X 10 ≡ 5 X 6 ≡ In lieu of structural panels or brace TC @ 24" OC, BC @ 24" OC. 110 mph wind, 15.00 ft mean hgt, ASCE Located anywhere in roof, CAT II, EXP wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. hip supports 7-0-0 jacks W/2 panel TC 3 $3 \times 4 \equiv$ THE R. F. 4 X 6 ≡ CENS . 59687 20-2-8 5 X 5 ≡ ವ * rigid 8X8*≡* 3 X 4 ≡ BC LL BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-7-02, CLOSED bldg, B, wind TC DL=5.0 ceiling use purlins and 1.5X4 / 7-0-0 no end =2296 U=222 W=3.5" 40.0 /=/R/ 10.0 20.0 24.0" 1.25 10.0 0.0 4X8(B3) = PSF PSF PSF PSF PSF 1-6-0 to SEQN-DATE REF HC-ENG DRW HCUSR487 07030030 JREF - 1T4G487_Z01 Scale R487--=.1875"/Ft. MNM/AF 01/30/07 130268 0-0-93386

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 PLT In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures. 6 395 Stanley Crawford Construc WILMOTH TYP. ALPINE Wave #2 Dense #2 Dense #3 $^{\infty}$ 124 U 180 W 12" **IMPORTANT**Gradish a copy of this design to the Installation contractor. The building components group, inc. Shall not be responsible for any devaluant from this design, any falure to build the thuss in components of the contractor and the proposition of the fall of t - MANNING** (RUSQES BEQUIRE EXTREME CARE IN FABRICATION, INAUDITIO, SHIPPING, HSTALLING AND BRACING, RELEA TO BEST (BUILDING COMPONENT SACELY HIFORNATION), PUBLISHED BY TET (FRUSS PLATE HISTITUTE, 218 WORTH (FEE SINEE), SHIFE 317, ALEXANDRIA, VA. 22314) AND HICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTREPORTS (LORIC, MAISSON, HI 53719) FOR SACELY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERWISE HOUSEAGED TO PERFORMING HIS FUNCTIONS. UNLESS OTHERWISE HOUSEAGED TO PERFORMING HIS FUNCTIONS. UNLESS A PROPERTY ATTACHED STRUCTURAL PARELS AND ROTTON CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND ROTTON CHORD SHALL HAVE BUILDING DESIGNER PER AUSI/IPI 1 SEC. 2.5X6(B1) = 1.5X4 ٥ 9 - 1 - 120-0 11 - 3 - 8Design Crit: 4 X 4 (R) Ⅲ * 5 X 8≡ 5 X 8 ≡ 4 X 4 == HT9A) r ∞ TPI=2002(STD)/FBC Cq/RT=1.00(1.25) €X6≡ 0 4 X 5 ≡ 2493 U 240 W 3.5" d 41-6=0 Over 3 Supports 6-0-0 6X12≝ .5X4 Ⅲ 2 0 23 6-0 $3 \times 4 =$ 5 X 6≡ d 3 X 6 ≡ /10(0)110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL 5.0 psf, wind BC DL 5.0 psf. (A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. $3 \times 4 \equiv$ 3 X 4 ≡ 20-2 3×5≡ 5 X 6 == $3 \times 4 \equiv$ ά CENSE . 59687 .5X4/ 9 0-0 $^{\infty}$ $2.5 \times 6 (B1) =$ 1295 * U 180 W 3.5" (6₀ BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/ 40.0 24.0" 20.0 PSF 1.25 10.0 PSF 0.0 10.0 PSF 0 PSF PSF SEQN-DATE REF HC-ENG DRW HCUSR487 07030008 JREF-Scale = .125"/Ft. R487--1T4G487_Z01 JB/AF 01/30/07 129463 93387

ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 Bot 10B In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures p chord 2x4 SP t chord 2x4 SP Webs 2x4 SP TYP. 395 Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP. THE SHALL HOT BE RESPONSIBLE FOR ANY DEVALUDE FROM HIS DESIGN: ANY TAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH 1PT; OR FARRICATION, HADRICED, SHIPPING, HISTALLING A BRACTING OF HISSES. IN THIS PROPERTY WITH APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY ALRAM, AND IPT. ALPHE CONNECTOR PLATES, ARE MADE OF ZO/BRIGAGA (M.1958/M.) ASHA 663 GRADE 40/500 M. K.M. SSPILON BERNAMMES HOAD ANY STEEL APPLY PARTES TO LACH FACE OF TRUSS AND. HULLSS OTHERMISE HOCATED ON HIS DESIGN. POSTITION DER BRAMINGS HOAD AND THE MADELY AND THE MEDICAL PROPERTY OF THE TRUSS CONDITION OF PROPERTY OF THE MEDICAL PROPERTY OF THE TRUSS CONDITION OF THE STEEL APPLY AND THE TOOS SEC. A SEAL ON THIS GRAMHING HODICALES, ACCEPTANCE OF PROFESSIONAL FURTHERING RESPONSIBILITY SOULLY FOR THE TRUSS COMPONITY. **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION, INAUDEING, SHIPPING, INSTALLING AND BRACING. RECER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TEL (SHISS PLATE INSTITUTE, ZIB UNDER LEE SHEEL, SHIFE 315, ALEXANDRIA, VA, ZZIJA) AND NECA (4000) TRUSS COUNCIL OF AMERICA, 6300 CHIERPISE LANE, HAUSSUM, NI 35719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLES OF A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER ANSI/TPI L 2.5X6(B1) R=84 U=180 9-1-12-11-0-0 3×4// 11 - 3 - 8.5×4 Ⅲ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)W=3.5" 4 X 4 (R) Ⅲ 5×5/ R=2228 U=191 W=3.5" 5 X 8≡ €X6≡ HT11A) 6X6≡ ò 4×4≡ 41-6-0 Over 3 Supports 6-0-0 AGE 45EC, BY ALREA) AND FI. ACPTHE
ANDE 40/60 (W. K.M. SS) GAA'V STEEL APPLY
THIS DESIGN, POSITION PER DRAVINGS 160A Z.
OF FPII 2002 SEC.3. A SEAL ON THIS
NSHILLITY SOLLEY FOR THE TRUSS COMPONITY
AND DUILDING IS THE RESPONSIBILLITY OF THE 1.5X4 III 6X12≝ 2-0-d 9-6-0 5 X 8≡ 3 \ 4 ≡ 3 X 6≡ 3 X 4 **=** $3 \times 4 \equiv$ Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 3 X 4 ≡ 5 X 6≡ 20-2 THUR R. 3 X 5 ≡ SENSE STATE OF 1.5X4 W Q 59687 3X4// 0 R=1352 U=180 W=3.5" $2.5 \times 6 (B1) =$ BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/ 24.0" 10.0 1.25 40.0 20.0 0.0 10.0 PSF PSF PSF PSF PSF SEQN-DATE REF JREF 1T4G487_Z01 HC-ENG DRW HCUSR487 07030010 Scale =.125"/ft. R487--JB/AF 01/30/07 129478 000 93388

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 567 PLT (A) 2x4 SP #3 or better "T" brace. 80% length of web member Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC. Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ 6 TYP. 395--Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE HUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVALUED FROM THIS DESIGN, ANY FAILURE TO BHILD THE TRUSS IN CONFORMS HITH APPLICABLE PROVISIONS OF THIS STALLING. BRANCH OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF THIS STALLING. BESIGN SPEC. BY AFRYA, AND IPI.

CONNECTOR PLATES ARE HADE OF 20/19/16GA (W.H/SS/R) ANH A653 GRANDE 40/60 (W. K/M.SS) GALV. STEEL APPLY PLATES TO LACH FACE OF TRUSS AND. BRANCH STALLING ON THIS DESIGN. POSITION PER DRAWHINGS 16GA Z. PLATES TO LACH FACE OF TRUSS AND. BRANCH STALLING HOLD THIS DESIGN. POSITION PER DRAWHINGS 16GA Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF THIS 200E YEC.3. A SEAL ON THIS DRAWHING HOLD THE SACTIONATE OF PROFESSIONAL GRADULTHING RESPONSIBILITY SOLLY OR THE TRUSS COMPONENT DRAWHING HOLD THE SULTABLETY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPI (FRUSS PLAIE INSTITUTE, 210 HORTH LEE STREEL, SUITE 317, ALEXANDRIA, VA., 22314) AND MICA (MODOL TRUSS COUNCIL OF AMERICA, 6300 ERREREPASE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE HORTHOGENERAL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUCKLED HORTHOGENERAL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE $2.5 \times 6 (B1) =$ 6-0 R=234 U=180 -1-12-3X4# .5X4 III Design Crit: 11 - 3 - 813-0 3×5/ AND USE OF THIS COMPONENT ₩=3.5° 4X4(R) Ⅲ * R=2037 U=414 W=3.5" 5 X 4 (R) 6X6≡ 4×5/ HT13A) 4×4 🥬 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) ò 3X6(**) = 1.5X4(**)5 X 8 🗏 41-6-0 Over 5X6(**) # 3 X 4 ≡ 15-6-0 1.5X4 III 0 5 X 8 **≡** 3 Supports _ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=2.8 psf, wind BC DL=2.2 psf. (**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. $3 \times 4 \equiv$ 5 X 8≡ 20 3 X 5 ≡ LENSE ά 1.5X4 Ⅲ 3X4 // WAL ENGINEERING o. 59687 TATE OF 13-0-0 '07 R=1288 U=281 W=3.5" $2.5 \times 6 (B1) =$ BC DL TC DL DUR.FAC. TOT.LD. TC FL/-/4/-8-0-0 /-/R/-10.0 20.0 1.25 40.0 10.0 PSF 0.0 PSF PSF PSF PSF DATE REF SEQN-HC-ENG DRW HCUSR487 07030009 Scale = .125"/Ft. R487--KH/AF 01/30/07 130347 93389

BUILDING DESIGNER PER ANSI/IPI

SPACING

24.0"

JREF -

Top chord 2x6 SP / Bot chord 2x6 SP / Webs 2x4 SP / From 51 rt. 1.50 to 10 PC From 5 PLF at 1.50 to 10 PC From 10 PLF at 0.00 to 10 PC 456 LB Conc. Load at 7.00 11. 19.00, 21.00, 23.00, 25.00, 27.00, 29. BC 443 LB Conc. Load at 7.00 BC 82 LB Conc. Load at 9.00, 11 9.00 11. 23.00, 25.00, 27.00, 27.00, 27.00 ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 SPECIAL LOADS 395--Stanley Crawford Construc WILMOTH TYP. -(LUMBER DUR.FAC.=1. From 31 PLF at -1 From 5 PLF at -1 From 10 PLF at 0 ALPINE Wave 1-6-0 #2 :T1 2x4 SP #2 Dense: #2 #3 :W7 2x4 SP #2 Dense: 4X8(B3) ≡ =2422 U=262 W=3.5' **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, MAY FALLURE TO BUILD THE TRUSS IN CONFORMANCE HITH PIT: OR FABRICATION, HANDLING, SHEPPLIO, HESTALLING, BRACING OF BRANCE HITH PIT: OR FABRICATION, HANDLING, SHEPPLIO, HESTALLING, A BRACING OF HUNSES, APPLICABLE PROVISIONS OF HOS SCIMALORAL DESIGN SPEC, BY AKENA, AND IDT. CONFORMS HITH APPLICABLE PROVISIONS OF HOS SCIMAL POSITION OF REALEST APPLY PLATES TO LACH FACE OF TRUSS AND. UNLESS OFHEREMSE LOCATED ON THIS DESIGN, POSITION OF REALEST FOLLOWED BY (1) SHALL BE FER ARREST AS OF THIS 2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SHALL BY THE ARREST AND SHALL BY THE ARRES **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDEING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST (QUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, ZIBE MOBIL LIE STREET, SUITE 312, ALEXANDRIA, VA, Z2314) AND NICA (MODD TRUSS COUNCIL OF AMERICA, GOOD ENTERSENT LAND. MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORHING HIESE FUNCTIONS. UNLESS OTHERWISE HOLDSLAFED FOR DOBS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER AUSI/TPI 1 PLATE 1.5X4 Ö 11.00, 29.00 E DUR.FAC.=1.25)
31 PLF at 29.50
5 PLF at 0.00
10 PLF at 29.50 11.00, 29.00 31 10 0 13.00, 15.00, 17.00 13.00, 15.00, 17.00 Design Crit: 3 X 4 ≡ 5 X 8 ≡ * HM7B) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 29-6-0 0ver $4 \times 10 =$ 2 X 4 III 7 5 X 4 ≡ Supports 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL-5.0 psf. In lieu of structural panels or rigid ceiling use brace TC @ 24" 0C, BC @ 24" 0C. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ Right end vertical not exposed to wind 4 X 4 == 25. 22-6-0 . 59687 4 X 5 == 6X8≡ R ★ BC DL TC DL SPACING TC DUR . FAC. TOT.LD. FL/-/4/-7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC pressure /-/R/-10.0 24.0" 40.0 20.0 R=2732 U=294 1.25 10.0 PSF 0.0 purlins 5 X 8≡ PSF PSF PSF 3 X 4 Ⅲ PSF SEQN-DATE REF HC-ENG JREF -DRW HCUSR487 07030018 Scale = .25"/Ft. R487--1T4G487_Z01 JB/AF 65295 01/30/07 93390 Ġ REV

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization 4 567 Wind reactions based on MWFRS pressures. PLT TYP. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. 395 Stanley Crawford Construc WILMOTH ALPINE Wave 1 - 6 - 0 $2.5 \times 6 (B1) =$ #2 Dense #2 Dense #3 R=1337 U=180 W=3.5" **IMPORTANT**FURHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR NAW DETVALION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE HITH PIP TO BE FABRICATION. HANDLING, SHIPPING, INSTALLING A BRACING OF BRUSSES, IN CONFORMANCE HITH PIP TO BE FABRICATION. AND THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF DIDS (MALIONAL DESIGN SPEC, BY ATEMA, AND THI, CONMICTION PHALES ARE AND OF 20/18/166A (M.H/SS/R) ASH A653 GRADE 40/60 (M. K/M-SS) GRALY. STEEL, APPLY PHATES TO EACH TAGE OF TRUSS AND. UNIESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAAHINGS 160A Z. ANY INSPECTION OF PHATES FOLLOWED BY (I) SHALL BE FER ANNEX AS OF THI 2002 SEC.3. A SEAL ON THIS DRAWING INSPECTANTS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE BUSS COMPONENT. **WARNING** TRUSSIS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING RETUR TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB HORRH LEE STREET, SUITE 312, ALEXANDRA, VA, 22314) AND WITEA (MOOD TRUSS COUNCIL OF ANTREAS, 6300 CHUTERRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED FOR HORSE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ATTACHED RIGID CELLING. BUILDING DESIGNER PER ANSI/TPI 9 1.5X4/ -0-0 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) $3 \times 4 =$ нмэв) 4 X 8≡ 29 0-9- $3 \times 4 \equiv$ 0ver 2 Supports 1.5X4 III 4 X 8 ≡ Right end vertical not exposed to wind pressure. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. $3 \times 4 \equiv$ 20-6-0 CEN 4 X 5 = SPACING BC DL BC LL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-24.0" 40.0 20.0 1.25 10.0 PSF 0.0 10.0 PSF R=1217 U=1801.5X4 III 4 X 5 = PSF PSF PSF JREF 1T4G487 Z01 SEQN-DATE REF HC-ENG DRW HCUSR487 07030019 Scale =.25"/Ft. R487--JB/AF 16189 01/30/07 93391 5

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / Haines City, FL 33844

Certificate of Authorical 567 In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC. Wind reactions based on MWFRS pressures. 6 -395--Stanley Crawford Construc TYP. ALPINE Wave 1.6.0 $2.5 \times 6 (B1) =$ #2 Dense #2 Dense #3 R=1337 U=180 W=3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTROHANCE HITH PIP TO REPRESENTATION, HANDLING, SHIPPING, HESTALLING A BRACTHE OF TRUSSES, IN CONTROHANCE HITH APPLICABLE PROVISIONS OF BUS (INTIONAL DESIGN SPEC, BY ATBYA) AND PIP. CONTROT OF THE PRESENTATION OF THE PROVISIONS OF BUS (INTIONAL DESIGN SPEC, BY ATBYA) AND PIP. CONTROT OF THE PRESENTATION OF THE PRESEN DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER ANSI/TPI I SEC. 2. **WARNING** TRUSSES REDURK EXTREME CARE IN FARRICATION. MANDELING, SHIPPING, INSTALLING AND BRACING, RECER TO CAST (RUTIOTHS COMPONENT SAFETY INFORMATION), PUBLISHED BY TET (FRUSS PLATE INSTITUTE, 219 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTIFERS LANE, MADISON, MI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNICESS OTHERMISE LIDITALITY TO CHORD SMALL MAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAVE MILMOTH 1.5X4 W 3X4# 0-0 Design Crit: * HM118 TPI - 2002 (STD) /FBC to Cq/RT=1.00(1.25)/10(0) 3 X 4≡ 4 X 8≡ ANY BUILDING IS THE RESPONSIBILITY OF THE 29 6 0 3 X 4≡ 0ver 2 Supports Right end vertical not exposed to wind pressure. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 4 X 8 ≡ .5X4 **Ⅲ** 8 No. 59687 CENS NAL ENGINEERING -6-0 $3 \times 4 \equiv$ $3 \times 4 \equiv$ $4 \times 4 =$ BC LL BC DL TC TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-PL 10.0 20.0 24.0" 40.0 R=1217 U=180 1.25 10.0 PSF 0.0 1.5X4 III 4 X 4 == PSF PSF PSF PSF DATE SEQN-HC-ENG REF JREF 1T4G487_Z01 DRW HCUSR487 07030020 Scale = .25"/Ft. R487-- 93392 JB/AF 16191 01/30/07 6 Ś

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / (6 395 Stanley Crawford Construc WILMOTH #2 Dense #2 Dense #3 H13B)

Wind reactions based on MWFRS pressures.

- (J) hanger connection not found in inventory file for this condition. Provide connection.
- (A) Continuous lateral bracing equally spaced on member

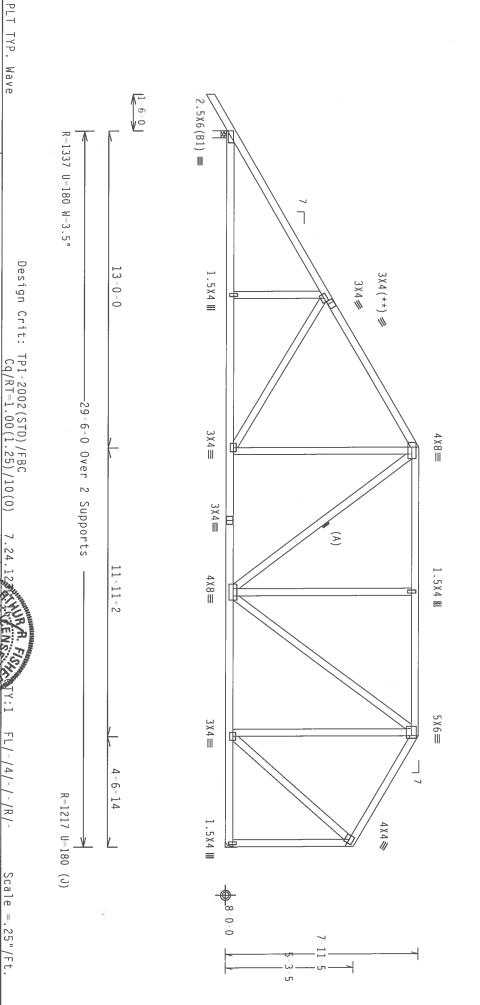
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure

In lieu of structural panels or rigid ceiling use purlins to brace TC @ $24"\ 0C$, BC @ $24"\ 0C$.



ITW Building Components Group, Inc.
Haines City, FL 33844

FI Certificate of Authorization 4 567

DESIGN SHOWN. THE SUITABILIBUILDING DESIGNER PER ANSI/IPI

IS THE RESPONSIBILITY OF

SPACING DUR.FAC. TOT.LD.

24.0"

JREF -

1T4G487_Z01

40.0

PSF PSF

1.25

FROM SEQN-HC-ENG

ALPINE

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BUILDING COMPONENTS GROUP. THE C. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FOR THIS DESIGN; ANY FATURE TO BUILD THE TRUSS IN COMPONENTS WITH THIS OF FARRICATION, ANNOTHING, SHIPPHIG, HISTALLING A BRACING OF TRUSSES.

DESIGN CONTONES WITH APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY AFRA) AND THE CONTRICTOR PLATES ARE MORE OF ZO/BJ/GRAQ (M.H/SSY, ASTH AGS GRADE 40/60 (M. X.M.SS) GARLY STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. HUMESS OTHERNISE, DOCATED ON THIS DESIGN POSITION PER BRAMINGS 160A Z ANY INSPECTION OF PRATES OF THE MISSES OTHERNISE, DOCATED ON THIS DESIGN POSITION PER BRAMINGS 160A Z ANY INSPECTION OF PRATES OF THE MISSES OTHERNISE, DOCATED ON THIS DESIGN POOR SEC. 3 A SEAL ON THIS DRAMING HUDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLCLY FOR THE TRUSS COMPONITHING

WARNING IRUSSES REQUIRE CYTECHE CARE IN FARRICATION, JAMOSLÍNG, SHIPPING, INSTALLING AND BRACING. REFER TO RESS (ROLLEING COMPONENT SAFETY INFORMATION), POULSTRED BY TPT (TRUSS PLATE INSTITUTE, 218 JOHN LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NICA (MODD TRUSS COUNCEL ON AMERICA, 6300 ENTIEREN SIGNATE, HADISON, NI 53719) FOR SAFETY PRACTICES PRIOR 10 PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE HOLDING THORS SINCL HAVE PROPERLY ATTACHED STRUCTURAL PAUELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAUELS AND BOTTOM CHORD SHALL HAVE

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10.0 20.0

PSF PSF

DATE

01/30/07

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Scale = .25"/Ft. R487--

BC DL TC DL TC LL

10.0 PSF

DRW HCUSR487 07030017

MNM/AF

149394

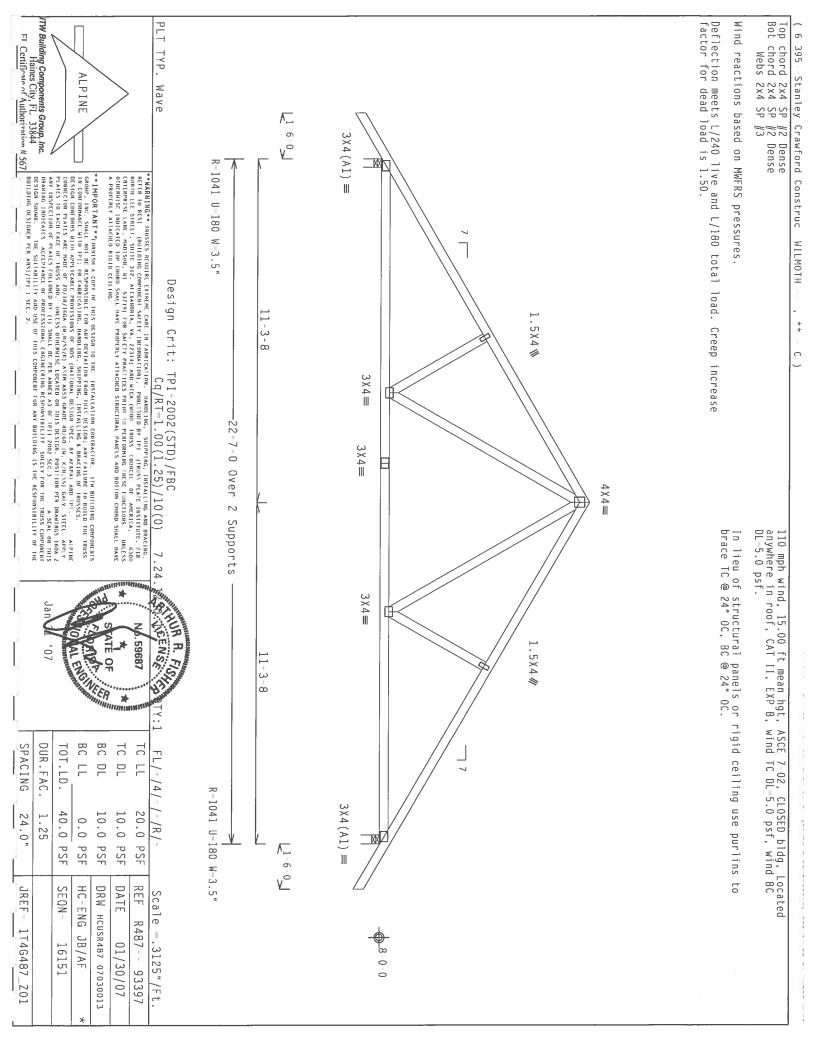
Wave

ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP PLT TYP. (J) hanger connection not found in inventory file for this condition. Provide connection. Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (6-395--Stanley Crawford Construc WILMOTH ALPINE Wave 1-6-0 $2.5 \times 6 (B1) =$ #2 Dense #2 Dense #3 R=1337 U=180 W=3.5" **IMPORTANT**TURNISH A CUPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BUILDING COMPONENTS GROUP. THE SHALL MAY BE RESPONSIBLE FOR ANY DEVIATION TRONG HIS SOCISION. ANY FALLURE TO BUILD THE RANGE OF GROUP. THE C. SHALL MAY BE RESPONSIBLE FOR ANY DESIGN CONTRACT WITH THIS OF FARRICALING, HANDLING, SHIPPING, INSTALLING A RACTING OF HINSELS. ALPHIN CONTROL OF THE APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN ESPEC, BY ATRAPA) AND THE APPLY CONTROL OF THE APPLY AND ANY SHEEL APPLY PROVIDED TO THE APPLY AND ANY SHEEL APPLY AND ANY SHEEL APPLY ANY SHEEL APPLY AND ANY SHEEL APPLY ANY SHEEL APPLY AND ANY SHEE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TP1 1 SEC. 2. A PROPERLY ATTACHED RIGID CEILING Design Crit: 3X4# 15 1.5X4 III 3X4// ò Ċ H15B) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -29 - 6 - 0 $\widehat{\mathbb{A}}$ 0ver RESPONSIBILITY OF 4 X 8 ≡ 4 X 5 ≡ \sim Supports 3 X 4 **=** 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Right end vertical not exposed to wind (A) Continuous lateral bracing equally spaced on member. 7.24 -11-2 * HUR R. CENSE STATE OF Nø. 59687 CORNOR 107 (A) 4 X 8 ≡ $3 \times 4 \equiv$ * BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-6 - 6 - 14pressure 24.0" 40.0 10.0 20.0 R=1217 U=180 1.25 10.0 PSF 0.0 1.5X4 III 3×5// PSF PSF PSF PSF DATE FROM SEQN-HC-ENG REF JREF -DRW HCUSR487 07030018 (ك Scale = .25"/ft. R487--_8-0-0 1T4G487_Z01 JF8 MNM/AF 01/30/07 149398 93394 9 S 5 Ġ

ביאטור כינוו כינה זיוו כי לרבטבים פי ביוורוביז לאום ליחחודו וכם חד ועחיים ווווע

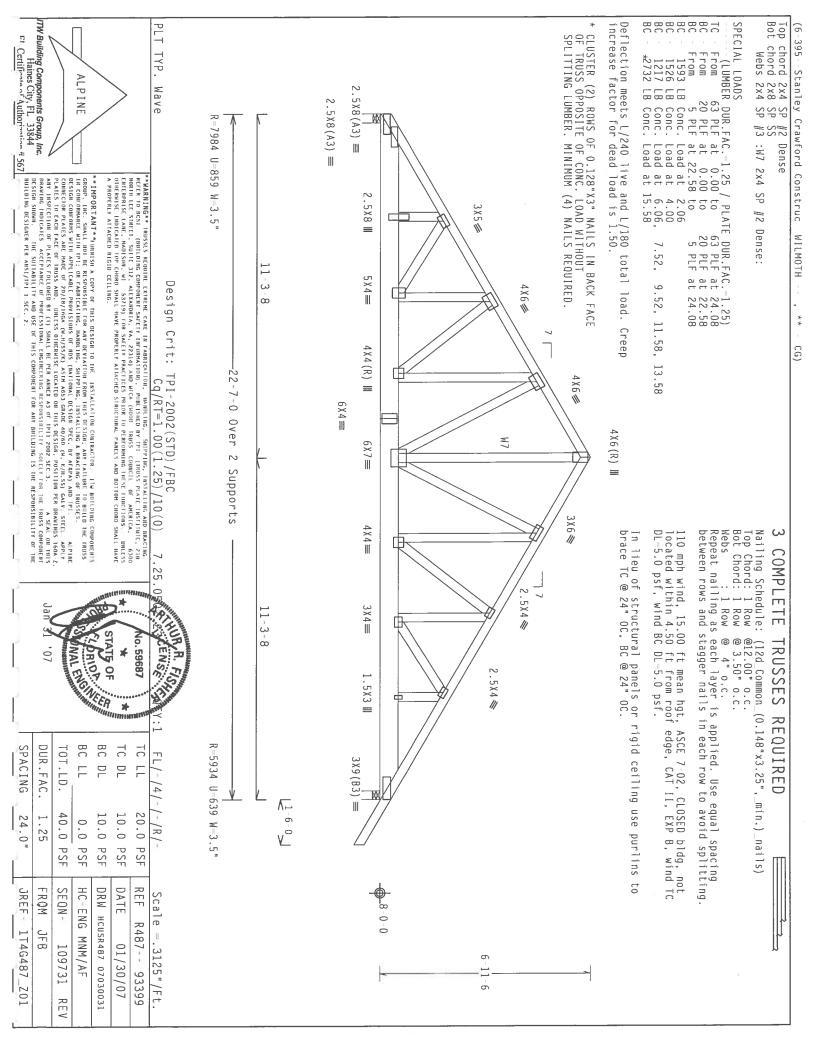
Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 (J) hanger connection not found in inventory file for this condition. Provide connection. Wind reactions based on MWFRS pressures. PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (6-395--Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT***URRISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE HUILDING COMPONENTS GROUP. THE STALL HE TO BUILD THE RUSSES IN COMPONENTS OF THE STALL HE TO BUILD THE RUSSES. IN COMPONENTS OF THIS THIS OF THIS PER PROSESS. THE COMPONENT OF THIS THIS OF THIS PER PROSESS. THE COMPONENT OF THIS PER PROSESS. THE STALL HE APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN ESPEC, BY AFRA) AND THE CONTROL OF TABLECALTE AND THIS (MAITONAL DESIGN ESPEC, BY AFRA) AND THE CONTROL OF TABLES AND THE CONTROL OF TABLES AND THE TABLES OF THE TABLES OF THE TABLES OF TABLES OF THE TABLES OF TABLES OF TABLES OF TABLES OF TABLES OF TABLES OF THE TABLES OF TABLES OF THE TABLES OF TABLES OF TABLES OF THE TABLES OF TABLES OF THE TABLES OF TABLES OF THE TABLES OF THE TABLES OF THE TABLES OF TABLES OF THE TABLES OF THE TABLES OF TABLES OF TABLES OF THE TABLES OF TABLES **WARNING** IRUSKIS BLOUIRE EXIBETE CARE IN FABRICATION, INNIDITIO, SHIPPING, HISTALLING AND BRACING, REFER TO BEST (BRUSS PLAIE (BISTIBLIE, ZIB 108 F). REFER TO BEST (BRUSS PLAIE (BISTIBLIE, ZIB 108 F). ALEXANDRIA, VA. ZEJJA) AND MICA (MODO TRUSS COUNCIE OF AMERICA, 6300 CHICERPLIS (LAME, MADISON, MI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HESE FUNCTIONS. UNICESS OFHERMISE (BRUSSLAUED FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER $2.5 \times 6 (B1) =$ 160 R=1337 U-180 W=3.5" Design Crit: 1.5X4 // H1605B) TPI -2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 9 3 X 4 ≡ 3×4 € ဌာ ά 29-6-0 3X4 // Over 2 Supports RESPONSIBILITY OF $3 \times 4 \equiv$ 4 X 8 ≡ 4 X 4 == 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Right end vertical not exposed to wind pressure (A) Continuous lateral bracing equally spaced on member. 7.24 0 * 2 $\widehat{\mathbb{A}}$ 4 X 8 ≡ $3 \times 4 \equiv$ GENSE 6.59687 ATÉ OF α 0-6 R-1217 U-180 (J) BC LL BC DL TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-1.5X4 III 4 X 4 🕼 /-/R/ 24.0" 40.0 10.0 PSF 20.0 PSF 10.0 PSF 1.25 0.0 0-0 PSF PSF G DATE REF FROM JREF-SEQN-HC-ENG DRW HCUSR487 07030019 Scale = . 1875"/Ft. Ġ R487--1T4G487_Z01 MNM/AF 01/30/07 149402 9 93395

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 Truss spaced at 24.0" OC designed to support $1\mbox{-}6\mbox{-}0$ top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Deflection meets L/240 live and L/180 total load. factor for dead load is 1.50. Wind reactions based on MWFRS pressures PLT TYP. Note: All Plates 6 MEMBER TO OTHERS. MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY 395 -- Stanley Crawford Construc WILMOTH ALPINE Wave K1-6-0) #2 Dense) #2 :B2 2x4 SP) #3 2X4(A1) =Are - MAK 980 1.5X4 Except As Shown. **IMPORTANT**TURNISH A CUPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE: SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS OUTSIGH; ANY FALLURE TO BUILD THE FRUSSES. IN COMPONENTS CHAIR CALL HE AS FARELALLING, HANDLING, SHIPPING, HISTALLING A BRACHING OF RUSSES. IN COMPONENS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFRAYA AND IPI. ALPHE CONFICENCY PLATES ARE TABLE OF TOPING FOR ANY STEEL APPLY PRACTICE OF PLATES, ARE TABLE OF TOPING FOR ANY ASSETTING PROVISIONS OF HOS CALL HOSE OF TOPING FOR BRACHINGS FOR A PROVISIONS OF THE TOPING FOR BRACHINGS FOR A PROVISIONS OF THE TOPING FOR BRACHINGS FOR A PROVISION OF PLATES FOR CHUNKED BY (1) SHALL BE OFTEN AND A TOPING FOR SEC. A SEAL ON THIS DEATH AND THE TOPING FOR THE FORSE COMPONENT HIS SECRET AND THE SOURCE OF THE TRUSS CAMP INC. **WARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, INANDETHG, SHIPPING, INSTALLING AND BRACING. RETER TO BESST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TET (TRUSS PLAIE INSTITUTE, 219 10814 LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ERRIERPISE (ANIE, MADISON, MT. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OBHERMISE INDICATED TO FORME SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL MAYE DESIGN SHOWN. THE SUTYABILE BUILDING DESIGNER PER ANSI/TPI U=180 W=3.5" #2 Dense: 9 THE SULTABILITY 3 X 4 Ⅲ Design Crit: + I I SEC. 2. Ġ 3X4参 * Creep increase OF THIS COMPONENT CGE 3 X 4 ≢ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) + ഗ 6 22 $3 \times 4 \equiv$ Ö 3×4/ -7-0 R-215 PLF U-21 PLF W-11-7-0 $3 \times 4 =$ 0ver Ī $3 \times 4 \equiv$ RESPONSIBILITY OF $4 \times 4 \equiv$ **N** Supports THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOM AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL=5.0 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. See DWGS Al1015EE0405 & GBLLETIN0405 for В2 肿 AUR R. W.S. 5. **59687** TIGINEER BC LL TC DL ВС TC SPACING DUR.FAC. TOT.LD. 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC PL 3 X 4 🦔 more /4/-/-/R/ 1-5-10-5-9 requirements. 20.0 40.0 10.0 24.0" 10.0 1.25 $3 \times 4 \equiv$ 0.0 PSF PSF PSF PSF PSF $3X4 \equiv$ DATE JREF -REF HC-ENG DRW HCUSR487 07030007 Scale R487--1T4G487_Z01 11 JB/AF 01/30/07 .3125"/Ft 129743 0-0-93396 REV



Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 567 PLT TYP. Wind reactions based on MWFRS pressures. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{cm}$ 6-395--Stanley Crawford Construc WILMOTH ALPINE Wave 3 X 4 (A1) ≡ R-935 U-180 W-3.5" #2 Dense #2 Dense #3 **IMPORTANT**FURBISH A COPY OF THIS DISIGN TO THE INSTALLATION CONTRACTOR. THE DUILDING COMPONENTS GROUP. HGC. SHALL NOT OR RESPONSIBLE FOR ANY DEVALUED FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTRIBUTED, HICL SHALL NOT OR FARBEICALTHG, HANDLIG, SHEPPING, INSTALLING A BRACHEG OF RUISSES, IN CONTRIBUTED HILL SHALL NOT OR FARBEICALTHG, HANDLIG, SHEPPING, INSTALLING A BRACHEG OF RUISSES, APPLY CONTRIBUTED AND THE ACTION OF THE PROPUSIONS OF ANDS (HATIONAL DESIGN SPEC, BY AFRA) AND THI. CONTRIBUTES ARE HANDED OF 2011BJGGAG (M. 1155K), ASHA HAS BORNE 40/60 [M. K.M.*S.) GAV STEEL APPLY PLATES TO EACH TACE OF TRUSS AND. UNICES OTHERWISE LOCATED ON THIS DESIGN. POSITION FROM THIS SEALON HILS OF THE ACCOMPONENT OF THE STATE OF THE SEALON HILS SO THE ACCOMPONENT OF THE SEALON HILS SO THE SEALON HILD SEALON HILS SO THE SEALON HILD ***MARNING** HRUSES BEQUIRE CEPTERE CARE IN FABRICATION, HANDLING, SUPPING, INSTALLING AND BRACING
REFER TO BEST (SULLING COMPONENT SILE LIY INFORMATION), PRINCIPIED BY THE LIVES FOR THE SILE INSTITUTE. ZUB
HOBERL LEE SHEET, SULLE JIZ ALEXANBRIA, W.A. 223141 AND WICA (MOOD FRUSS COUNCIL OF MARRICA.

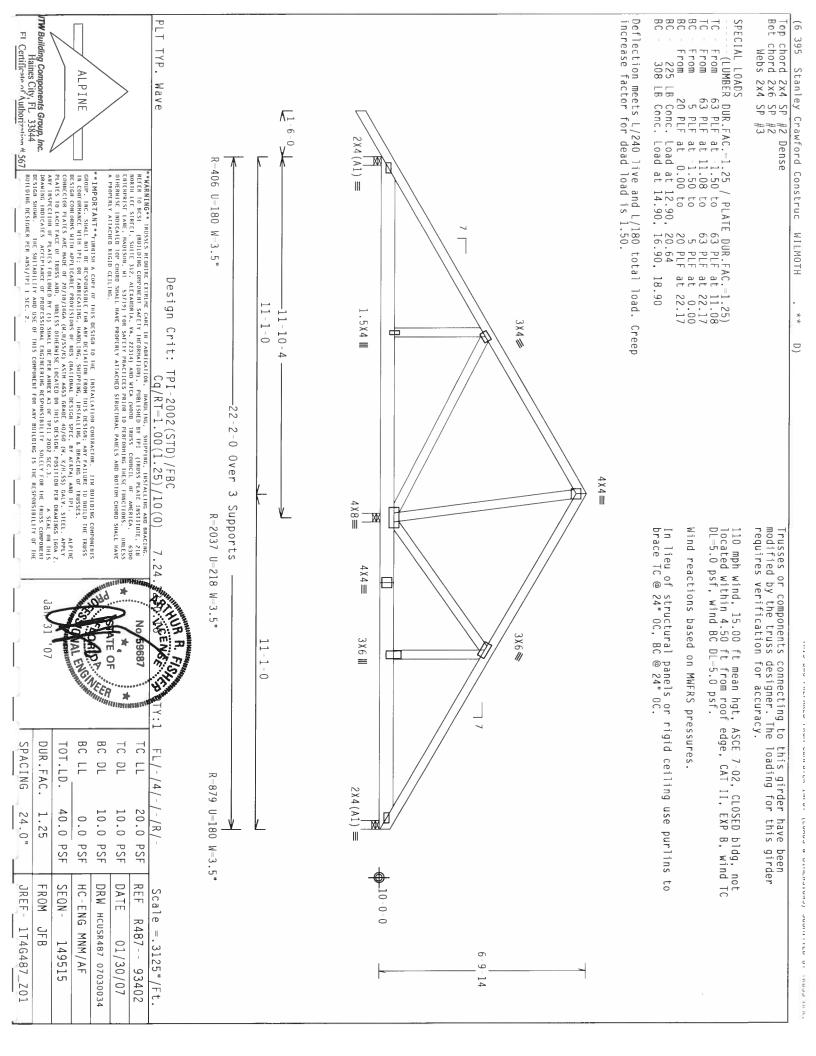
ONDERHISE LAW, MADISON, M. 14 53739 FOR SAULT PRACTICES PRIOR TO PERFORMING THESE TUNCTIONS. UNILESS
ONDERHISE LIVE FAILED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTY AND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR BUILDING DESIGNER PER AUST/TPI 1 SEC. 2. A PROPERLY ATTACHED RIGID CEILING 1.5X4 W 1-3-8 Design Crit: * 3 X 4 == C1) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 22-7-0 $3 \times 4 =$ Over 2 Supports 4 X 4 = In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. 110 mph wind, 15.00 ft mean hgt, anywhere in roof, CAT II, EXP B, DL=5.0 psf. 3 X 4 ≡ 1.5X4 // 11-3-8 E OF * ASCE 7-02, CLOSED bldg, Located wind TC DL=5.0 psf, wind BC BC DL TC DL R=1045 U=180 W=3.5" SPACING TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-3X4(A1) =W K1-6-0 20.0 24.0" 40.0 10.0 1.25 0.0 10.0 PSF PSF PSF PSF PSF SEQN-DATE REF HC-ENG DRW HCUSR487 07030017 JREF-Scale = .3125"/Ft. _8-0-0 R487--1T4G487_Z01 JB/AF 16155 01/30/07 93398



Haines City, FL 33844

Fit Certiffactor of Authoritation 4 PLT Top chord 2x4 SP #2 Dense Bot chord 2x6 SP #1 Dense Webs 2x4 SP #3 :W2 2x4 SP In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures BC BC TC SPECIAL LOADS (6 395 Stanley Crawford Construc From 63 PLF at 0.00 to From 63 PLF at 11.08 to From 20 PLF at 0.00 to 1219 LB Conc. Load at 1.90 1211 LB Conc. Load at 3.90 1344 LB Conc. Load at 5.90 TYP. LUMBER DUR.FAC.=1.25 ALPINE Wave - # 567 BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2 ***IMPORTANT***URBLISH A CUPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS SESSION, ANY FAILURE TO BUILD HE TRUSS IN CONTORNACE THIS TEST OF FARRY AND THE TRUSS OF THIS THIS THE TRUSS OF THE SHALL HOT A BRACING OF TRUSS ESTATE AND THE CONTROL OF THE TRUSS OF THIS TRUSS OF THE APPLICABLE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY ATRAY) AND THE APPLY FOATS OF THE TRUSS CONTROL OF THIS TRUSS CONTROL OF THE T **WARNING** IBUSCY: REQUIRE CTREME CARE IN FARRICATION, MANDELING, SHIPPING, INSTALLING AND BRACHING, REFER TO REST, QUILDING COMPONERS SAFLY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, ZIB ROBERT LE STREET, SUITE 312, ALEXANDRIA, VA, ZZ314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, GOOD CHIEFER STELLI, MAISSH, MI 53719) FOR SAFLY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOUSEAULD FOR COMPON SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE 2.5X8(A1) =1.90 3.90 5.90, PLATE #2 Dense: R = 3766TE DUR.FAC.=1.25)
63 PLF at 11.08
63 PLF at 12.00
20 PLF at 12.00 HILOMILM THE SUITABILITY AND USE OF THIS COMPONENT FOR 7.90, U=363 W=3.5" Design Crit: 9.90 12-0-0 DSG) 0ver 1.5X4 III TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7x6≡ 0 \sim Supports BUILDING IS THE RESPONSIBILITY OF THE 6X6(R) **Ⅲ** R=3695 U=359 W=3.5" 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Right end vertical not exposed to wind pressure Use equal spacing between rows and stagger nails in each row to avoid splitting. 3X4(R) III 0-11-0 COMPLETE Φ M 1.5X4 Ⅲ TOCKNSE TRUSSES . 59687 6 * ά REQUIRED BC DL BC LL TC DL SPACING TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-40.0 10.0 24.0" 1.25 10.0 PSF 20.0 PSF 0.0 PSF PSF PSF FROM SEQN-DATE REF JREF - 1T4G487_Z01 DRW HCUSR487 07030032 HC-ENG Scale =.3125"/Ft R487--JFB MNM/AF 01/30/07 149388 9 93400 -14

Haines City, FL 33844
FI Certificate of Authorization # 567 THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER. CONNECTIONS SHALL BE FROM THE ROOF DIAPHRAGM TO THE CEILING DIAPHRAGM. THIS TRUSS IS NOT DESIGNED FOR LATERAL WIND PRESSURE APPLIED TO THE FACE. ANY LATERAL LOAD FROM WIND MUST BE TRANSFERRED TO THE BUILDING DIAPHRAGMS. LATERAL BRACING FOR WIND TO BE DESIGNED AND FURNISHED BY OTHERS. Top Bot In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures + MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS. PLT TYP. Note: All Plates 6 chord 2x4 SP L chord 2x4 SP Webs 2x4 SP 395 Stanley Crawford Construc ALPINE Wave 1 P #2 Dense P #2 Dense P #3 :W2 2x4 SP 6 $2X4(A1) \equiv$ Are R=711 U-180 W-3.5" 3X4 / 1.5X4 Except As Shown. BUILDING DESIGNER PER ANSI/IPI **WARNING** (BUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST, (BUILDING COMPONIENT SAFETY IMPORMATION), PUBLISHED BY FPT (TRUSS PLAIE INSTITUTE, 218 MORTH LEE STREET, SUITE 317, ALEXANDRIA, VA, 2231) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6306 CHIERPRISE LAKE, HADISON, HI 5375) FOR SAFETY PRACTICES PRIOR TO PERFORMING INCSE FUNCTIONS. UNLESS OTHERPRISE LAKE, MAD BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGID CEILING. #2 4-9 Dense MILMOTH THE SUTTABILITY AND USE OF THIS COMPONENT FOR 13 Design Crit: 1 SEC 3X4 / 11-10-4 3X4 // 0 GE) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 9 22 ω BUILDING 2 0 0ver 4 X 4 = RESPONSIBILITY OF W Supports 4 X 8 ≒ R-2137 U-258 W-3.5" ΔX Truss spaced at 24.0" OC designed to support 1-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. ++MEMBER TO BE BRACED BY PROPERLY ATTACHED SHEATHING OR PURLINS @ 24" OC. CHORD ENDS TO BE LATERALLY BRACED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. See DWGS A11015EE1106 & GBLLETIN1106 for 3 X 4 ≡ * WATHUR . 9-1-9 NSENS 0.59687 3 X 4 // 07 * TY:1BC DL BC LL TC DL J SPACING DUR.FAC TOT.LD 3 X 4 // -552 U-180 W-12" more 1-5-10-5-11 /4/-2X4(A1) =3X4 /// /-/R/ 20.0 requirements 40.0 10.0 10.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF REF DATE SEQN JREF -FROM DRW HCUSR487 07030033 HC-ENG Scale = .3125"/Ft. R487--1T4G487_Z01 JFB MNM/AF 01/30/07 149370 6 93401 REV



Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Haines City, FL 33844
FI Certificate of Authorization # 567 PLT In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. See DWGS Al1015EE1106 & GBLLETIN1106 for more requirements. Wind reactions based on MWFRS pressures. (6-395--Stanley Crawford Construc TYP. ALPINE Wave #2 Dense #2 Dense #3 R=316 U=195 W=3.5" R=126 PLF U=26 PLF W=11-8 $2X4(A1) \equiv$ **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE UNSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE. SHALL HOT BE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORRANCE HITH PIE OR FARRE FRONTISIONS OF THIS PROVIDED BEACH OF TRUSSES.

ALPINE CONNECTOR PLAILS ARE HADE OF ZO/INB/IGA, NH. MISS/N. ASTH A653 GRADU 40/60 (N. X/M.SS) GALV. STEEL, APPLY PLAIES TO EACH FACE OF TRUSS AND. BUILESS OTHERWISE LOCALIE ON THIS DESIGN, POSITION FER BRANCHING SIGA Z. ANY INSPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER BRANCH CONFORMATICES ACCEPTOREM HIS DESIGN SHOWN. THE SUITABILLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILLITY OF THE DESIGN SHOWN. THE SUITABILLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILLITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. A PROPERLY ATTACHED RIGID CEILING. 3X4 // W 1-5-13 HIOMIH 3X4 // Design Crit: * EGE) .5 8 4 1 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) Ö Ġ 12 0-0 0ver 4 X 4 = 2 Supports Truss spaced at 24.0" OC designed to support 1–6 0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$.5X4 III 0 中 o CEN 3X4/// 1-5-13 * 3 X 4 // $2X4(A1) \equiv$ 0-5-11 BC LL BC DL TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-<u>1</u> 1 · 6 · 0 − ✓ /-/R/-10.0 20.0 24.0" 40.0 10.0 1.25 0.0 PSF PSF PSF PSF PSF DATE REF JREF-FROM SEQN-HC-ENG DRW HCUSR487 07030035 10-0-0 Scale =.5"/Ft. R487--1T4G487_Z01 MNM/AF 149359 01/30/07 93403 REV

Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc. Haines City, FL 33844 BC BC PLT In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C. BC @ 24" 0C. See SPECIAL LOADS 6 DWGS All015EE0405 & GBLLETIN0405 for more requirements. TYP. 395 Stanley Crawford Construc From From -- (LUMBER DUR.FAC.=1.25 / From 85 PLF at -1.50 From 5 PLF at -1.50 From 20 PLF at 0.00 From 5 PLF at 11.29 ALPINE Wave 3 X 4 == 1-6-0-> #2 Dense #2 Dense #3 R=150 PLF U=43 -1.50 -1.50 0.00 11.29 **IMPORTANT**FURBISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVALION FROM HIST DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMACE AND THE PROPERTY OF THE PROPERTY OF THE CONTRACT AND THE PROPERTY OF THE PROPERTY OF THE CONTRACT OF THE PROPERTY OF THE CONTRACT OF THE CONTRACT OF TRUSS ARE AND OF THE CONTRACT OF TRUSS ARE AND. UNLESS OTHER SECONDS OF THIS DESIGN, POSITION PER DRAMINGS IGAZ ANY INSPECTION OF PLATES OLD THE SULFACE OF THE SECOND OF THIS DESIGN, POSITION PER DRAMINGS IGAZ ANY INSPECTION OF PLATES OLD ONE PROPERTY OF THE RESPONSIBILITY OF THE DRAMING INDIAN. THE SULFACE OF THE TRUSS COMPONENT OF THE TRUSS COMPONENT OF THE TRUSS COMPONENT OF THE TRUSP COMPONEN $3 \times 4 \equiv$ **WARNING** RUSSES BEUNEK CYBENE CARE IN FARRICATION. MANDEING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST. (BUSICALE AND BRACING.) RETER TO BEST. (BUSICALE AND BRACING.) PUBLISHED BY FFT (FRUSS PLATE INSTITUTE, ZIO MOBINE LEE SIREEL, SUITE 312 ALEXANDRIA, VA. ZZZIA) AND NICA. (MODD TRUSS COUNCIL OF AMERICA. 6200 ELICEPRISE LANE, MADISON, NI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED TO ROUBD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS, AND BOTTOM CHORD SMALL HAVE to PLATE PLF E DUR.FAC.=1.25) 85 PLF at 12.79 5 PLF at 0.00 20 PLF at 11.29 5 PLF at 12.79 85 20 5 MILMOTH -10 - 8R ANSI/IPI I SEC. 2. ₩=11-3 X 4 ∅ Design Crit: * . 5 X 4 FGE \Box . ω ώ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ Over Continuous Support 4 X 4 ≡ , 5 X 4 Ф)/10(0)110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL=5.0 psf. Truss spaced at 24.0" OC designed to support 1–6–0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Dead loads are stated on projected horizontal area basis. Wind reactions based on MWFRS pressures ώ Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. .5X4 Ⅲ .5X4 III 卣 3×4// 1-10-8 07 0-5-BC DL BC LL $\frac{1}{2}$ TC LL DUR.FAC. TOT.LD. FL/-/4/-7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC P $3 \times 4 \equiv$ / - /R / -40.0 10.0 10.0 20.0 1.25 0.0 PSF PSF PSF PSF PSF DATE REF SEQN-HC-ENG DRW HCUSR487 07030001 Scale =.5" R487--JB/AF 16152 01/30/07 /ft. 93404 REV

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Certificate of Authorization # 567

BUILDING DESIGNER PER ANSI/IPI

SPACING

24.0"

JREF 1T4G487_Z01

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Provide Provide ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 Wind Hipjack supports 7-0-0 setback jacks with no webs. PLT 6 395 Stanley Crawford Construc WILMOTH TYP. reactions based on MWFRS pressures (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord. ALPINE Wave #2 Dense #2 Dense #3 ***IMPORTANT***URBHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THIS. SHALL HOT BE RESPONSABLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALLURE TO BUILD THE FRUSSES. IN COMPONENTS HITH THIS OF FARRICATION, HANDLING, SHEPPING, INSTALLING A BRACTING OF RUSSES. BY CHECKEN HITH THE OFFARRICATION, HANDLING, SHEPPING, INSTALLING A BRACTING OF RUSSES. BY AFRAYA AND THE COMMICTION PLATES, ARE FARDE OF 70/18/16/04 (H.18/SS), ASTH ASSI GRADE 40/50 (H. X./H.SS), GALV. STEEL. APPLY PLATES TO FACH FACE OF RUSSES, AND, HULLSS OTHERWISE, LOCATED ON HITS DESIGN. POSITION PER BRAKHINGS 160A Z ANY INSPECTION OF PLATES OF COMPONENT HITS COMPONENT HITS SOURCE OF THE RUSSES. AND CONTROLLED ON HITS PLATES CONCERNING HOUSEAST AND THE RUSSES. ASSISTED FACE OF THE RUSSES. ASSISTED FACE OF THE RUSSES ASSISTED FACE OF THE RUSSES ASSISTED FACE OF THE RUSSES. ASSISTED FACE OF THE RUSSES ASSISTED FACE OF THE RUSSES ASSISTED FACE OF THE RUSSES. ASSISTED FACE OF THE RUSSES CANDED FOR THE RUSSES COMPONENT. ****MARNING** HRUSSIS REQUIRE CIRELIE CARE IN FABRICATION, HANDLING, SHIPPIM, INSTALLING AND BRACING REFER TO REST (BUILDING GOMPONENT SKETY) HIGHWALION). PUBLISHED BY TPI (BRUSS PLATE HISTITUE, 218 HORRIL HE STRIET, SHITI 312, ALEXANDRIA, WA, 72314) AND NICA (MODO TRUSS COMECIL DE AMERICA, BOD GUIERNES LANCE, HAISON, MILL SIZ, DE SAFLY PRACIFICES PRIOR TO PERFORMING THISE UNICIONS. UNILESS OFMERNISE INDICATED FOR CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE *WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION.
REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). BUILDING DESIGNER PER A PROPERLY ATTACHED RIGID CEILING. 2-1-7-SUITABILITY AND USE OF 2X4(A1) =Design Crit: \overline{x} $M_{\rm M}$ =466 U=180 W=4.95" * 4.95 HJ7) TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 9-10-13 Over 1.5X4₩ w RESPONSIBILITY OF Supports In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Jan 31 '07 3 X 4 ≡ CENSE 0.59687 P OF R=361 U=180 R=266 U=180 BC LL BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/ 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0 0 L8-0-0 10.0 40.0 20.0 24.0" 10.0 PSF 1.25 0.0 PSF PSF PSF PSF JREF 1T4G487_Z01 DATE REF HC-ENG DRW HCUSR487 07030002 Scale = .375"/ft. R487--JB/AF 01/30/07 16234 93405

ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 Wind reactions based on MWFRS pressures. Top Bot PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ chord 2x4 SP chord 2x4 SP 395--Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense **IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE THISTALLIANT ON CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE. SHALL HOLD HE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN. ANY FAILURE TO BUILD THE RRISES IN COMPONENCE WITH HELD ON FLARE CHARLET ON FRANCISCHIC. HANDLING, SUPPING, HISTALLING & BRACHER OF RRISES. A REPORT OF THIS DESIGN FOR THE PROPERTY OF THE CONNECTION PLATES ARE HADE OF ZO/JRJ JORGA (M. 1875/Y) ASTH AGES GRADE 40/GO (M. KM. 85) GAV. SHEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. HINLESS OHERWISE LOCATED ON HIS DESIGN, POSITION PER BRAHHIGS 160A-Z. ANY HISPECTION OF PLATES COLUMNED BY (1) SHALL BE PER ANNER AS OF IPIL-2002 SEC.3. AS SEA, ON THIS DESIGN SHOWN AND THE SULFABLE OF PROPERSSIONAL CHIGHTENING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN AND THE SULFABLE OF PROPERSSIONAL CHIGHTENING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN AND THE SULFABLE OF PROPERSSIONAL CHIGHTENING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN AND THE SULFABLE OF PROPERSSIONAL CHIGHTENING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN AND THE SULFABLE OF THE SOURCE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** ISUSSES REGUIRE LYRENE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BESS. (RULLOING COMPONENT SAFET FROMENSION), PUBLISHED BY IPT (FRUSS PLATE INSTITUTE, 218 URBH MEE STREE, SUITE 312, ALEXANDRIA, VA, Z2314) AND WICA (ROOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPLS, LANE, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO ROODS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE K1 6 0 V $2X4(A1) \equiv$ R-412 Design Crit: U-180 W=3.5" -7-0-0 Over 3 Supports EJ7) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 wind BC DL=5.0 psf. Provide (Provide In lieu of structural panels or rigid ceiling @ 24" OC, BC @ 24" OC. R-82 U-180 R-190 U-180 22 *) 16d common nails(0.162"x3.5"),) 16d common nails(0.162"x3.5"), TOGENSE 5. 59687 5 12-1-4 8-0-0 * BC LL BC DL TC DL TC LL SPACING DUR.FAC TOT.LD. FL/-/4/-/-/R/ toe nailed at Top chord. toe nailed at Bot chord. use 40.0 10.0 10.0 20.0 24.0" 1.25 0.0 purlins PSF PSF PSF PSF PSF to SEQN-DATE REF JREF-DRW HCUSR487 07030006 HC-ENG Scale = .375"/Ft. psf, brace TC R487--1T4G487_Z01 JB/AF 16202 01/30/07 93406

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Haines City, FL 33844
F1 Certificate of Authorization # 567 Wind reactions based on MWFRS pressures PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 6 395 -- Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**Gurish a copy of this design to the installation contractor. The building composities group, fac, shall not be responsible for any deviation from this design, any failure to build the thuss in composities that the form far far failure. The failure for building the thin pris of failure for this hand, incl., shipping, installing a backing of frustes, a library conforms with applicable provisions of this (pational design spec, by Afray and Delegons of the prison failure failure for the failure forms of this failure fai **WARNING** IRUSSES REQUIRE EXTREME CARE IN FARRICATION, INABOLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING CHROMORIN SAFELY INFORMATION), PUBLISHED BY FPI (TRUSS PLAIL INSTITUTE, 210 HORTH LEE STREET, SUITE 137, ALEXANDRAIA, VA, 22131) AND NICA (NOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPASE LAIL, ANDISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMENT HIESE FUNCTIONS. UNLESS OTHERWISE HOUSEARED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE **1** 6 0 € 2X4(A1) =R = 415Design Crit: N U=180 W=3.5" w -7-0-0 Over 3 Supports ά 1.5X4 Ⅲ 4 X 4 == EJ7T) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 2-0-0 6 1.5X4 III 5×5= 2 ά ω Provide Provide In lieu of structural panels or rigid ceiling @ 24" 0C, BC @ 24" 0C. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. R-90 U-180 R=184 U=180 2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord * w . 59687 Ġ 12-1-4 9-0-0 8 0 0 BC LL BC DL TC DL DUR.FAC TC TOT.LD. FL/-/4/-/-/R/ esu 20.0 40.0 10.0 10.0 PSF 1.25 0.0 purlins bldg, not located
TC DL=5.0 psf, PSF PSF PSF PSF to brace DATE REF SEQN-DRW HCUSR487 07030014 HC-ENG Scale = .375"/Ft. R487--JB/AF TC 01/30/07 129748 93407

BUILDING DESIGNER PER ANSI/IPT

1 550

SPACING

24.0"

JREF - 1T4G487 Z01

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP 6 395 Stanley Crawford Construc WILMOTH #2 Dense #2 Dense #3 EJ7T1)

110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind wind BC DL=5.0 psf. bldg, not located TC DL=5.0 psf,

purlins

In lieu of structural panels or rigid ceiling use @ 24" OC, BC @ 24" OC. to brace TC

Provide Provide

2) 16d common nails (0.162"x3.5"), toe nailed at Top chord. 2) 16d common nails (0.162"x3.5"), toe nailed at Bot chord.

Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2X4(A1) =1.5X4 III 4 X 4 == 6 5 X 5 = 1.5X4 Ⅲ R 90 U 180 R 183 U-180 ىب 0 J 8-0-0 8-11-0

2

ά

1-10-0

R=415 ယ်၊ U=180 W-3.5" -7-0-0 Over 3 Supports 2-10-8

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

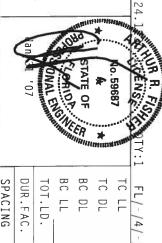
A PROPERLY ATTACHED RIGID CEILING.

Haines City, FL 33844
FI Certificate of Authorization # 567 ***IMPORTANT***TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP. THIS SHALL NOT BE EXPONESTBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONTROPANCE WITH TELE OF FARRICATION, HANDLING, SHEPPING, INSTALLING A BRACTHO OF TRUSSES.

APPLIES DESIGN CONTORES WITH APPLICABLE PROVISSIONS OF HOS (MATIONAL DESIGN SPEC, BY ATRAY) AND DELABLE PROVISSIONS OF HOS (MATIONAL DESIGN SPEC, BY ATRAY) AND DELABLE CONTROL APPLICABLE PROVISSIONS OF HOS (MATIONAL DESIGN SPEC, BY ATRAY) AND THE APPLY PRACTES TO FACH FACE OF TRUSS AND. HURLES OTHERWISE DOCATED ON HITS DESIGN POSITION FER DRAWLINGS 160A A APPLY PRACTES TO FACH FACE OF TRUSS AND. HURLES OTHERWISE DOCATED ON HITS DESIGN POOR AS A SEA ON HITS DRAWLING HUDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLICLY FOR HIT RUSS COMPONENT

ALPINE

BUILDING DESIGNER PER SULTABILITY AND 1 SEC. USE OF THIS COMPONENT BUILDING RESPONSIBILITY OF



/-/R/ 20.0

Scale =.375"/Ft. R487--

PSF

REF

93408

] T			STATE OF THE PARTY	ONE PROPERTY	ER Vuun	**************************************
	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL
	24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF
	JREF - 1T4G487 Z01		SEQN- 16238	HC-ENG JB/AF *	DRW HCUSR487 07030012	DATE 01/30/07

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844

C1 Certiffer of Authorization # 567 Provide Provide Wind reactions based on MWFRS pressures PLT TYP. 6 395 -- Stanley Crawford Construc WILMOTH ALPINE 2) 16d common nails(0.162"x3.5"),
2) 16d common nails(0.162"x3.5"), Wave #2 Dense #2 Dense #2 Dense **IMPORTANT**FIRMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, HGC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, WAY FAILURE TO BUILD THE TRUSS IN CONFORMANCE HITH PETS OR FLARE FRONYISHOUS OF HDS (MAIDOAL DESIGN SOR AN ARTHUR FOR FRONYISHOUS OF HDS (MAIDOAL DESIGN SOR ARTAN) AND IPT. DESIGN COMPORES WITH APPLICABLE PROVISIONS OF HDS (MAIDOAL DESIGN SEC. BACKLY, STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. BULES OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHIGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX AS OF IPTI 200Z SEC. 3. A SEAL ON THIS DESIGN SEC. 3. A SEAL ON THIS DESIGN SEC. 3. THE MAIN SHOWN. THE SULFAME FACE OFFORT THE HUSS COMPONENT DESIGN SHOWN. THE SULFAME HOLD SHOWN. DESIGN SHOWN. THE SUITABILI BUILDING DESIGNER PER ANSI/IPI **WARNING** TRUSSIS REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO REST. (RULDING COMPONENT SAFELY INFORMATION), PUBLISHED BY IPI (TRUSS PLATE INSTITULE, ZIB URBILLE, ZIB, ALEXANDRIA, VA, ZZJIA) AND NICA (ROOD DRUSS COUNCIL O' AMERICA, 6300 ENTERPRISE LANE, MAISSON, NI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING INFSETURETIONS. UNLESS OTHERNISE INACTION OF A SAFELY PRACTICES PRIOR TO PERFORMING INFSETURETIONS. UNLESS OTHERNISE INACTION FOR SAFELY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE **1** 6 0 **y** $2X4(A1) \equiv$ R = 413M Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 2 toe nailed at Top chord. toe nailed at Bot chord. 3-8 U-180 W-3.5" -7-0-0 Over 3 $3 \times 4 \equiv$ EJ7T2) 3×4 / 9 1.5X4 III 1.5X4 Ⅲ 3×4€ Supports 3-15 -6-8-110 mph wind, 15.00 ft mean hgt, ASCE 7-within 4.50 ft from roof edge, CAT II, E wind BC DL=5.0 psf. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. R-152 U-180 R=120 U=180 * WHUR. Jan WENSE' 0.59687 9 0 02 BC LL BC DL TC DL TC SPACING DUR.FAC. TOT.LD. 7-02, CLOSED EXP B, wind /4/= /-/R/ 10.0 20.0 40.0 10.0 24.0" 1.25 0.0 bldg, not located TC DL=5.0 psf, PSF PSF PSF PSF PSF DATE REF JREF 1T4G487 Z01 HC-ENG DRW HCUSR487 07030015 Scale = .375"/Ft. R487--JB/AF 16250 01/30/07 93409

TW Building Components Group, Inc. Haines City, FL 33844 PLT Wind reactions based on MWFRS pressures Top chord 2x4 SP Bot chord 2x4 SP Deflection meets L/240 live and L/180 total load. Greep increase factor for dead load is 1.50. FL Certificate of Authorization # 567 σ TYP. 395 Stanley Crawford Construc ALPINE Wave #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT OR RESPONSENCE FOR MAY DEPLATED FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFIDENCE HITH PIP TO RE FABRICATION. HADDLING, SHEPPING, URSTALLING & BRACING OF TRUSSES.

DESIGN CONFIDENCE HITH PIP THE FABRICATIONS OF DIDS (MAIDONA, DESIGN SPEC, BY MARA) AND TRISCORSION OF THE PROPERTY DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER AUSI/TPI 1 ***MARNING** RINSES REQUIRE EXERTHE CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING.
RETER 10 RES. (MULLDING COMPONENT SAFETY INFORMATION). PUBLISHED BY FPI (TRBUS PLATE INSTITUTE, 210)
ENTER 15 STATE, SUITE 312. ALEXANDRIA, VA. AZZIA) AND WICA (WOOD TRBUS COUNCIL OF AMERICA. 6300
ENTERRESE LANT. HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE THICTIONS. HULLSS
OTHERRISE LIDICATED TO FUNDE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING. 1 6-0-MILMOTH $2X4(A1) \equiv$ Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) =335 U=180 W=3.5" -5-0-0 Over 5ل Supports SOLELY FOR THE TRUSS COMPONENT Provide 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Provide In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. R=54 U=180 R-129 U=180 渺 22) 16d common nails(0.162"x3.5"), 16d common nails(0.162"x3.5"), STATE OF ENSE lo. 59687 יייז טאים נערגעטפה נעהה רחבהחודא זמגחו (רחשה? Thensions) אחפשווודה פל ואחסט שדא Ġ 10-11-4 BC DL SPACING TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/ toe nailed toe nailed 24.0" 40.0 20.0 1.25 0.0 10.0 PSF 10.0 PSF PSF PSF PSF at Top chord. at Bot chord. JRFF-DATE REF SEQN-HC-ENG DRW HCUSR487 07030003 Scale =.5"/Ft. R487--1T4G487_Z01 JB/AF 16206 01/30/07 93410

Top Bot ITW Building Components Group, Inc.
Haines City, FL 33844

Certifiant of Authorianian 4 567 Wind reactions based on MWFRS pressures PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ chord 395 Stanley Crawford Construc WILMOTH TYP. ALPINE 2x4 SP 2x4 SP Wave #2 Dense #2 Dense **IMPORTANT** Turnish a copy of this design to the installation contraction. The milling components group. The, shall not be recipionally from this design; any fallure to build the third state that price with price of that periods, design from this design; and fallure the build the third state of the state of the periods of the state of the **WARNING** IRUSSES REDUIRE EXTRINI CARE IN FABRICATION, HARDLING, SHIPPING, INVIALLING AND BRACING. RECER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TET (ERRES PLATE INSTITUTE, 220 MORTH LEE SIRCEI, SUITE 312, ALEXANDRIA, VA, 22314) AND NICA (MODD TRUSS COUNCIL OR AMERICA, 6300 EULERPH SE LANE, HARISKON, NI 53718) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOUSEAULED FOR COMED SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED REGION CHORD SHALL HAVE <u>1-6-0-</u>√ $2X4(A1) \equiv$ Design Crit: M 265 U=180 W=3.5" 0 0 Over 3 Supports J3 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) R 63 U 180 R-24 U-180 Provide (Provide (In lieu of structural panels or rigid ceiling use purlins to brace TC @ $24\mbox{"}$ OC, BC @ $24\mbox{"}$ OC. 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind 5 994 8-0-0 22 THUR R.) 16d common nails (0.162"x3.5"), toe nailed at Top chord.) 16d common nails (0.162"x3.5"), toe nailed at Bot chord. CENSE TATE OF 6.59687 * BC DL BC LL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/-7-02, CLOSED bldg, LTC DL=5.0 psf, wind 40.0 20.0 1.25 10.0 PSF 0.0 10.0 PSF PSF PSF PSF Located | BC DL=5.0 SEQN-DATE REF HC-ENG DRW HCUSR487 07030004 Scale R487--=.5"/Ft. JB/AF 16211 01/30/07 93411

Certifirm

SPACING

24.0"

JREF-

ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization 4 567 Wind reactions based on MWFRS pressures. Top chord 2x4 SP Bot chord 2x4 SP PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 395 - Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS. IN CONFORMACE HITH MET OR TARRESCALID. HANDLING, SHIPPING. HISTALLING & BRACHER OF RRUSSES, IN CONFORMS WITH APPLICABLE PROVISIONS OF MISS (MAI DOMA DESIGN SPEC, BY AFRA) AND DPI. CONFORMS WITH APPLICABLE PROVISIONS OF MISS (MAI DOMA DESIGN SPEC, BY AFRA) AND APPLICABLE OF TARRESCALID. HOWEVER, AND ARE ASSOCIATED ON THIS SPECIAL PROVISION OF TO TO TARRESCALID. WHISTSKY, ASHA ASSOCIATED ON THIS DESIGN. POSITION PER DRAWHIRS 160A Z. ANY INSPECTION OF MAILS FOLLOWED BY (I) SHALL BE PER ANNER AS OF THI 2002 SEC.3. A SEAL ON THIS DRAWHIRS INDIVIDUAL SECONDOLUTION OF MAILS FOLLOWED BY (I) SHALL BE PER ANNER AS OF THI 2002 SEC.3. A SEAL ON THIS DRAWHIRS INDIVIDUAL SECONDOLUTION OF MAILS SCHOOL THE THIS CORPORATION OF THE MISS COMPONENT OF THE MISS CORPORATION **HARNING** IRUSSIS NEOUIRE EXTREME CARE IN FARRICATION, INABELING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (FRUSS PLATE INSTITUTE, 2016 MORH LEE SHEET, SHIE 312, ALEXANDRIA, VA, 22314) AND MICA (MODO) TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LANE, MAINSON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNLESS OTHERWISE INDICALID FOR FORDS SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE **1** 6 0 **1 1** 1 0 0 Over 3 Supports 2X4(A1) =Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) M 257 U-180 W-3.5" J1) æ R 57 U 180 15 U-180 8-0-0 • Provide Provide In lieu of structural panels or rigid ceiling @ $24\ ^{\circ}$ OC, BC @ $24\ ^{\circ}$ OC. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 7.24 HUR R. FI. 22) 16d common nails (0.162"x3.5"), toe nailed at Top chord.) 16d common nails (0.162"x3.5"), toe nailed at Bot chord. CENSE No.59687 * BC LL BC DL TC DL TC DUR.FAC. TOT.LD. FL/-/4/-/-/R/-use 40.0 10.0 PSF 20.0 PSF 10.0 PSF 1.25 0.0 purlins to brace TC PSF PSF DATE REF SEQN-DRW HCUSR487 07030005 HC-ENG Scale = .5"/Ft. R487--JB/AF 01/30/07 16214 93412

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

SPACING

24.0"

JREF 1T4G487_Z01

6 395 -- Stanley Crawford Construc WILMOTH * M4)

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / #2 Dense #2 Dense #3

Wind reactions based on MWFRS pressures

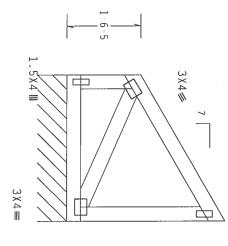
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.

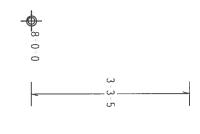
110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind DL=5.0 psf. 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC

Right end vertical not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

1.5X4 III





0 Over Continuous Support

R 83 PLF U-60 PLF W-3-0-0

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

WARNING IRUSSES REQUIRE EXPREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (DUILDING COMPORENT SAFETY INFORMATION), PORLISING BY PET (TRUSS PLATE INSTITUTE, ZIQ DORIN LEE STREET, SHIFE 312, ALEXANDRIA, VA, Z2314) AND NICA (MODO TRUSS COUNCIL OF OMERICA, 6300 ENTERENSE LAME, MAISSON, MAI

ITW Building Components Group, Inc. Haines City, FL 33844 F1 Certificate of Authorization # 567 **IMPORTANT***URRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. HC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS OFSIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTROPRINGE HITE PIE: OR FARRICALING, HANDLING, SHIPPIRE, HUSTALLING A BRACHING OF TRUSSES. ADDITION OF THE PROPERTY OF THE PROP BUILDING DESIGNER PER ANSI/TPI R ANSI/TPI 1 SEC IS THE RESPONSIBILITY OF

ALPINE

TUR R. OENSE lo. 59687 TE OF GINEER TC DL TC LL B.C. ВС FL/-/4/-/-/R/ DL

10.0 PSF 10.0 PSF

DRW HCUSR487 07030011

JB/AF

20.0

PSF

Scale = .5"/Ft. R487--

DATE REF

01/30/07

93413

Jan 31 '07	ON AL CONTRACTOR	Contraction of the second
DUR.FAC.	TOT.LD.	BC LL
1.25	40.0	0.0
	PSF	PSF

SEQN-HC-ENG

129672

SPACING

24.0"

JREF-

Top chord 2x4 SP p Bot chord 2x4 SP p Webs 2x4 SP p #2 Dense #2 Dense #3

Wind reactions based on MWFRS pressures

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

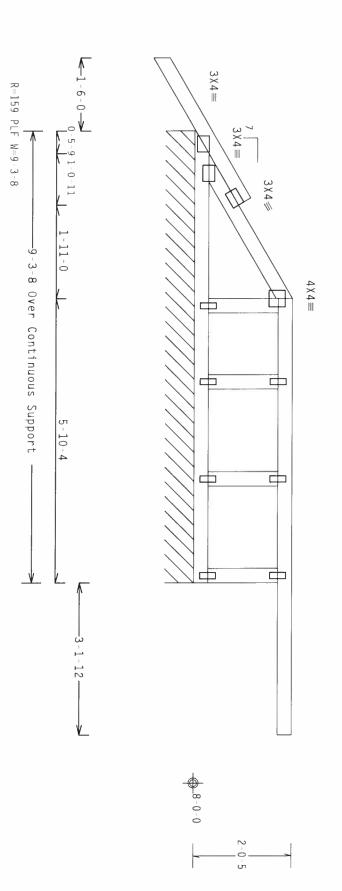
In lieu of structural panels or rigid ceiling use brace TC @ 24" 0C, BC @ 24" 0C. purlins

Right end vertical not exposed to wind pressure

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Top chord overhangs have been checked only for loads as indicates. Overhangs not checked for man loads or long-term deflection.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP.

Wave Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

VENSE

/4/-

/-/R/

Scale

=.5"

/Ft. 93414

WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION. INABELING, SHIPPING, INSTALLING AND BRACING RECER TO BEST. (BUILDING COMPORENT SAFETY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 21B 1087). ALEXANDRIA, VA, 22314) AND NEGA (HODD) TRUSS COUNCEL OF AMERICA, 6300 ENTERGRAFICA, AND SOME, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERIORHING INFSE FUNCTIONS. UNLESS OTHERMISC INDICATED TO REGION SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

Haines City, FL 33844
FI Certificate of Authorization # 567 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTROPRINCE HIM THE TO BE FABRICATION. THE THIS, HANDLING, SHIPPING, HISTALLING A BRACTHE OF TRUSSES. IN CONTROPRINCE HIM THE APPLICABLE PROVISIONS OF THIS (MAIDONAL DESIGN SPEC, NY AFRA) AND THE CONTROL OF THIS ARE THAD OF TO THE THIS APPLY BLAIT OF THE TRUSS AND AND THE CONTROL OF THE TRUSS AND AND THE TRUSS OFFERNISH LOCATED ON THIS DESIGN, POSITION FOR BRAHINGS LOCA Z. ANY HISFOLDING OF PACES FOLLOWED BY CL) SHALL BE FER ANDREX AS OF THIS TORS SEC. A SEAL ON THIS DESIGN AND THE TRUSS OF THE TRUSS OFFERNISH THE TRUSS AND THE TRU

ALPINE

DRAHING INDICATES ACCEPTANCE OF DESIGN SHOWN THE SUITABILITY BUILDING DESIGNER PER ANSI/IPI 1 THE SHITABILITY RESPONSIBILITY OF

TATE 0.59687 * BC LL BC DL TC DL SPACING DUR.FAC. JC TOT.LD. 10.0 20.0 40.0 10.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF DATE REF JREF -SEQN-DRW HCUSR487 07030016 HC-ENG R487--

JB/AF

129737

REV

01/30/07

395 Stanley Crawford Construc MILMOTH

3

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / #2 Dense #2 Dense #3

Wind reactions based on MWFRS pressures.

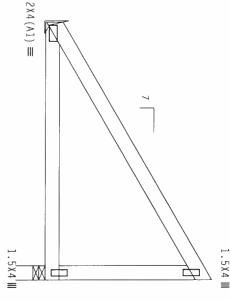
 $\left(\text{J}\right)$ hanger connection not found in inventory file for this condition. Provide connection.

Deflection meets $\lfloor /240 \rfloor$ live and $\lfloor /180 \rfloor$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C.





R=225 U=180 (J) 3-8 Over \sim Supports R = 215U=180 W=3.5"

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /10(0)

PLT TYP.

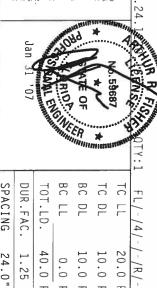
Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPHIG, INSTALLING AND BRACING. RELER UD BCSI. (BULICING COMPONINT SAFETY INFORMATION), PUBLISHED BY TET (FRUSS PLATE HISTITUTE, 210 MORTH LEE STREET, SUIT 173, ALEXANDRA, VA, 22314) AND NECA (400D TRUSS COUNCEL OF AMERICA, 6300 ENTREMENTS, FUNDESS, ALEXANDRA, VA, 22314) AND NECA (400D TRUSS COUNCEL OF AMERICA, 6300 CHIERPISE LANE, HAUSSON, NI 52719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING HIESE FUNCTIONS. UNLESS OTHERNIST HUNDLAND FOR DOBBOOK HALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

Haines City, FL 33844
FI Certificate of Authorization # 567 **IMPORTANT**URRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP. THE SHALL NOT BE RESPONSIBLE FOR ANY DEPLATION FROM THIS SESSION; ANY FALLURE TO BULLD THE TRUSS IN COMPONENTS HITCH THIS DESIGN. THE PROPERTY OF TARETCAL THIS, INSTALLING A BRACING OF RUSSES.

BY COMPONENTS HITH APPLICABLE PROPRISTONS OF HDS (HALTONAL DESIGN SPEC, BY ATRA) AND THE CONTROL OF THE APPLICABLE PROPRISTONS OF HDS (HALTONAL DESIGN SPEC, BY ATRA) AND THE APPLY PRACTICE OF ALTERS ARE TABLE OF ZO/BOTROM (H.1958/N.) ASTH AGES GRADE (40/50 HR. X.H.) SSS GRAVE STEEL APPLY PRACTICE OF AREA OF THE SESSION OF PROPRISTON OF THE ROBALHOS SIGNAL APPLY PRACTICE OF PROPRISTON O DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPRUILDING DESIGNER PER ANSI/TPI 1 SEC. 2 3111 \$ 1 RESPONSIBILITY OF

ALPINE



		31 '07	RAM E	BIONNIN	OF FRANCE	*	59687
	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
	24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
	JREF - 1T4G487_Z01	FROM JFB	SEQN- 149373	HC-ENG MNM/AF	DRW HCUSR487 07030020	DATE 01/30/07	REF R487 93415

Scale =.5"/Ft

(6-395--Stanley Crawford Construc WILMOTH M2)

האוזי בשיע ואינוטאיני ואטון פטוח טובא נואיטו עביסטים מינובתיסנטים] סטטוווון בע טון ואטטט וווא.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

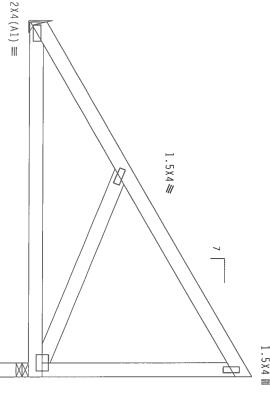
(J) hanger connection not found in inventory file for this condition. Provide connection.

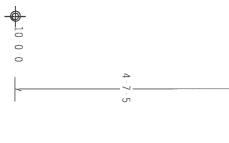
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure

In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC.





R=308 U=180 (L) ω Over 2 Supports R=298 U=180 W=3.5"

3 X 4 ==

Design Crit: TPI=2002(STD)/FBC Cg/RT=1.00(1.25)

PLT TYP.

Wave

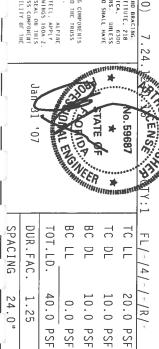
***MARNING** RUSSKS REGURET YREFE CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RETER TO BEST (BRISS PLATE INSTALLING AND BRACING, BELLEVED BY FFI (FRISS PLATE INSTITUTE, 210 UNDER LOS LATE INSTITUTE, 210 UNDER LOS LATE INSTITUTE, 210 UNDER LOS COUNCIL OF AMERICA, 6300 CHILERPLSE LATE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HILSE FUNCTIONS. UNLESS OTHERWISE, HOLICATED TOP CORDS SHALL HAVE PROPERTY ATTACHED STRUCTURAL PARELS, AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED REGION OF THE CONTROL OF THE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE TUSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, JUG. SHALL NOT BE RESPONSIBLE FOR ANY DEFEATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONTROLLER HITH PIP OR FABRICATHO. HANDLING, SHEPPHO, HISTALLING A BRACING OF BRUSSES, OESIGN CONTROLLER HITH PIP OR FABRICATHO. HANDLING, SHEPPHO, HISTALLING A BRACING OF BRUSSES, APPHORE CONNECTOR PALATES. ARE HADE OF 2071BJAGAGA (M. HISSEY), ASHA ASS GRADE 4076G (M. K/MISS) GALV. STEEL. APPLY PLAIES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON HIS DESIGN, POSITION PER DRAWHINGS 160A. Z
ANY HISPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PPIL-2002 SEC. 3
A SEAL ON HIS
BRANING INDICALES ACCEPTANCE OF PROFESSIONAL FROM HIS AND ANY FOR THE RUSS COMPONENT
DESIGN SHOWN. HIS SHITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group, Inc.
Haines City, FL 33844
F1 Certificate of Authorization # 567

BUILDING DESIGNER PER ANSI/IPI 1 SEC.

ALPINE



PSF PSF

> DATE REF

01/30/07 93416

Scale = .5"/Ft. R487--

DRW HCUSR487 07030021

MNM/AF 149378

JREF 1T4G487_Z01

FROM SEQN-HC-ENG Haines City, FL 3844
FI Certificate of Authorization # 567 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures. 395 Stanley Crawford Construc TYP. ALPINE Wave **IMPORTANT**NURBLESH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BUILDING COMPONENTS GROUP. THE SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION HOW THIS DESIGN: ANY FALLURE TO BUILD THE TRUSS IN CONTORNACE HITH FIT OR FARRICATION, HANDLING, SHIPPING, INSTALLING A BRACHING OF TRUSSES.

APPENDENCE OF THE SHIP OF THE PROVISIONS OF HOS (MAITONAL DESIGN SPEC, BY ATRAY) AND IPI.

APPENDENCE ARE MORE OF TOP/HET/GROUP (M.M.) SEYN, ASTH AGS GRADE GARGE GARGE MAY STEEL APPLY PLAITS TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE, DOCATED ON HITS DESIGN, POOR THE DRAWINGS HOME AND HITS GRADE GARGE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. A PROPERLY ATTACHED RIGID CEILING **★**1-6-0-**↓** MILMOTH 2 X 4 (A1) Design Crit: MD =346 U=180 W=3.5" * -5-3-8 Over 3 TPI-2002 (STD) /FBC Cq/RT=1.00 (1.25) /10 (0) \sim ANY BUILDING IS THE RESPONSIBILITY OF THE Supports R = 1961.5X4 III U=180 W=3.5" 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $\mathbb{M} \oplus$ Right end vertical not exposed to wind Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 7.24 _10-0-0 CENSE No. 59887 HIOP TE OF * SPACING BC DL TC JC DUR.FAC. TOT.LD. FL/-/4/-/-/R/-PL _ pressure 24.0" 10.0 20.0 40.0 1.25 0.0 10.0 PSF PSF PSF PSF PSF JREF -DATE REF FROM SEQN-HC-ENG DRW HCUSR487 07030022 Scale =.5"/Ft. R487-- 93417 1T4G487_Z01 MNM/AF 01/30/07 149382

(6 395 Stanley Crawford Construc WILMOTH , ** P2)

THE PROPERTY OF STATE AND A CAMPA OF THE PARTY OF THE PAR

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

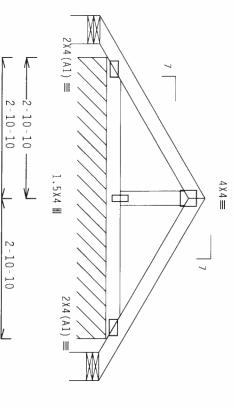
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0405 or PIGBACKB0405 for piggyback details. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 19.37 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C.





U=180 W=6.946" R=83 PLF U=31 PLF W=5-9-4 R=-6 U=180 W=6.946"

6

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

WARNING IRUSSES REDUIRE EXIREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SACETY INCORALION), PUBLISHED BY IP! (TRUSS PLATE INSTITUTE, 218 MORTH LEE SIREI, SUITE 132. ALEXANDRA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300 CHIERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORHING HIESE FINICIONS. HULLSS OTHERWISE HOLDSCALED FOR CORDS SHALL HAVE A PROPERTY ATTACHED STRUCTURAL PARIELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CELLING.

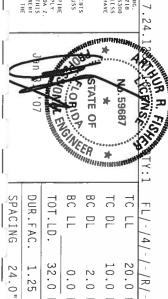
IMPORTANT*URBLISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BHILDING COMPONENTS CROUP. THE C. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN. ANY FAILURE TO BUILD THE TRASS IN CONTRIBUTED WHILE PROPERTIES.

IN CONTRIBUTED WHILE PROPERTIES OF THE CONTRIBUTED WHILE AND THE CONTRIBUTED WHITE APPLICABLE PROPERTIES OF THE CONTRIBUTED WHITE ARE THAT OF 20/18/16/06 (H.)/15/54) ASTH AGE OF THE APPLY DEALES TO EACH FACE OF TRUSS AND. WHILES OTHERWISE LOCALED ON THIS DESIGN. POSITION FOR DRAWINGS 160A 2. ANY INSTRUMENT OF PLACES TO EACH FACE OF TRUSS AND. WHILES OTHERWISE LOCALED ON THIS DESIGN. POSITION FOR DRAWINGS 160A 2. ANY INSTRUMENT OF PLACES OF THE CONTRIBUTED OF THE STORM OF PLACES FOLOMED BY 11 SHALL BE FOR ANY SHELL THE SHORM. THE SHIFT AND USES OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER FOR ANY SHELL THE SHOWN.

Haines City, FL 33844

Fit Certificate of Authorization # 567

ALPINE



) PSF

DRW HCUSR487 07030023

MNM/AF

149476

REF DATE

01/30/07

Scale =.5"/Ft.

R487 - -

93418

PSF

HC-ENG SEON-FROM C

Haines City, FL 33844
FI Certificate of Authorization 4 PLT Wind reactions based on MWFRS pressures l op Bot Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (6 395 Stanley Crawford Construc WILMOTH chord 2x4 SP chord 2x4 SP TYP. ALPINE Wave Authorization # 567 #2 Dense #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BUILDING COMPONENTS GROUP. THE: SHALL HOT BE RESPONSIBLE FOR ANY DETAILOR FOR HIS THEN HIS DESIGN: ANY FALLERE TO BUILD THE TRUSS IN COMPONENTS HIT HIS PROPERTY OF TABLECT HIGH. HANDLING, SHIPPING. INSTALLING & BRACTING OF TRUSSES. ALPHE CONTROL OF THE PROPERTY OF THE PR **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING, REFER TO BEST (BUILDING COMPONENT SATELY INFORMATION), PUBLISHED BY THE (TRUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SUITE 127. ALEXANDRIA, NA, 227.214) AND MICHA (MODO TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAME, MADISON, H 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNILESS OBTHERMISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED ATTACHED RIGHD CELLING. DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/IPI 1 R=19 U=180 W=6.946* R=82 PLF U=68 PLF W=2-7-11 ← 4-4-7 Over 3 Supports $2X4(A1) \equiv$ 1 3 14 1 - 3 - 14Design Crit: AND USE OF THIS COMPONENT SEC. 2. $3 \times 4 \equiv$ V 1-3-14 $2X4(A1) \equiv$ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) R=19 U=180 W-6.946* OZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT NG IS THE RESPONSIBILITY OF THE 000 3-82 Refer to details. BRACED @ 110 mph wind, 18.78 ft mean hgt, ASCE Located anywhere in roof, CAT II, EXP wind BC DL=1.2 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. brace DNG PIGBACKAO405 or PIGBACKBO405 for piggyback TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO 24" OC, UNLESS OTHERWISE SPECIFIED. CENS. IE,Ok * BC LL BC DL TC DL SPACING DUR.FAC. TC TOT.LD. 7:02, CLOSED bldg, B, wind TC DL=5.0 psf, 14/-24.0" 32.0 20.0 /-/R/ 1.25 10.0 PSF 0.0 2.0 PSF PSF PSF PSF DATE JREF -FROM SEQN HC-ENG DRW HCUSR487 07030024 REF Scale R487--1T4G487_Z01 =.5"/Ft. MNM/AF 01/30/07 149499 93419

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Haines City, FL 33844
FI Certification of Authoritation # 567 PLT Refer to DWG PIGBACKA0405 or PIGBACKB0405 for piggyback details. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (6-395 -Stanley Crawford Construc ,WILMOTH TYP. ALPINE Wave #2 Dense #2 Dense #3 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. THE BUILDING COMPONENTS CAROUR, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE RITH PT: OR FARRE TAITION, HANDLING, SHIPPLING, HISTALLING, A BRACING OF TRUSSES, IN CONFORMANCE RITH APPLICABLE PROVISIONS OF ANDS (MALIDNAL DESIGN SECC. M. AFRAYA, AND IPI. CONFECTOR PLATES ARE HADE OF 70/18/16/AGA (M.11/55/K), ASTH A653 GRADE 40/50 (M. K/M.5S) GALV. STEEL. APPLY PLATES TO LACH FACE OF TRUSS AND. UNLESS OHIGHNIS LOCALED ON HIS DESIGN, POSITION PER DRAHINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWING OF OF THE STEEL AND THIS DESIGN. A SEA, ON THIS DESIGN SHOWN.

BRANING INDICATES ACCEPTANCE OF TRUSS SHOWN OF THIS COMPONENT OR A SEA, ON THIS DESIGN SHOWN.

BRANING INDICATES ACCEPTANCE OF TRUSS SHOWN OF THIS COMPONENT OR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

BRANING INDICATES ACCEPTANCE OF TRUSS SHOWN OF THIS COMPONENT OR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** RUSSES BEOURE CITEME CARE IN FABRICATION. MANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BEST. (BULLDING COMPONENT SAFETY REGERATION), PUBLISHED BY FFT (FRUSS PLATE INSTITUTE, ZED MORTH LEE STREEL, SHITE 312. ALEXANDRIA, VA. Z2314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 CHILEBRISE LAHE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING HISSE FUNCTIONS. UNLESS OTHERWISE HOLDSLAFED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGERATION. DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/TPI 1 R-16 $2X4(A1) \equiv$ U-180 W-6.946" R-75 PLF 55 PLF W-5-9-4 $\frac{1}{1}$ $\frac{1}{1}$ $\frac{1}{10}$ $\frac{2}{10}$ $\frac{10}{10}$ Design Crit: 3 X 4 ≡ AND USE OF THIS COMPONENT SEC. 2. * -6-0 P1) 0ver TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 1.5X4 III BΕ .5X4 III ω 6-0 474 Supports 3X4₩ 1-10 $2X4(A1) \equiv$ \mathbb{R} =16110 mph wind, 18.78 ft mean hgt, ASCE Located anywhere in roof, CAT II, EXP wind BC DL=1.2 psf. U-180 W-6.946" In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC. . 24 HUR R. CENSE Ng/.59687 0000 * BC LL BC DL TC DL DUR.FAC. TC TOT.LD. FL/-В, -02, CLOSED bldg, wind TC DL-5.0 [4/-/-/R/ 32.0 10.0 20.0 1.25 0.0 2.0 PSF PSF PSF PSF PSF a principational application of those psf, DATE REF FROM SEQN HC-ENG DRW HCUSR487 07030025 Scale = .5"/Ft. R487---MNM/AF 01/30/07 149473 93420

SPACING

24.0"

JREF -

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / Refer to details. BRACED @ ITW Building Components Group, Inc.
Haines City, FL 33844

Ft Certificate of Authorization # 567 PLT Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$ (6-395--Stanley Crawford Construc WILMOTH TYP. DWG PIGBACKAO405 or PIGBACKBO405 for piggyback TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO 24" OC, UNLESS OTHERWISE SPECIFIED. ALPINE Wave #2 Dense #2 Dense #3 26 2X4 (A1) U=180 W=6.946"
R=69 PLF U=54 PLF W=11-2-**IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN FOR FAILURE TO BUILD THE TRUSS IN CONFORMANCE HITH PIP. OR FABRICATHO, HADDLING, SHAPLING, HISTALLING & BRACTING OF REUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF THIS CHAILONAL DESIGN SPEC, BY ATAPA) AND IPI.

CONNECTOR PLAIES ARE HADE OF 20/18/166A (H.H.955/K) ASTH A653 GRAVE ASTEMS 4060 (M. K/H.553) GAUV. SIECL. APPLY PLAIES TO EACH FACE OF TRUSS AND. BUILESS OTHERNISE (DOCATED ON THIS DESIGN, POSITION PER DRAHRINGS 160A Z. ANY INSPECTION OF PLAIES FOLLOWED BY (I) SHALL BE PER BHIREY AS OF THIS 200EY SEC.3. A SEAL ON THIS BRANKING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOFTLY OR THE RESPONSIBILITY OF THE BRANKING INDICATES ACCEPTANCE OF THE CONFORMANCE OF PROFESSIONAL CHAILERS AND THE SOURCE AS COMPONENT OF THE STANDARD CHAILES ACCEPTANCE OF THE SCOPPONENT OF THE STANDARD CHAILES ACCEPTANCE OF THE SCOPPONENT OF THE STANDARD CHAILES ACCEPTANCE OF THE STANDARD CHAILES ACCEPTANCE OF THE SCOPPONENT OF THE SCOPPONENT OF THE STANDARD CHAILES ACCEPTANCE OF THE SCOPPONENT OF THE STANDARD CHAILES ACCEPTANCE OF THE SCOPPONENT BUILDING DESIGNER PER ANSI/IPI I A PROPERLY ATTACHED RIGID CETLING 1.5X4 III 1.5X4 III 10 10 က် 3 X 4 ≡ Design Crit: 7 AND USE OF THIS COMPONENT SEC. 2. ώ 9 * σı 14 2 P3) 3 X 4 **■** ļ TPI-2002(STD)/FBC ___Cq/RT=1.00(1.25)/10(0) BΕ Ò 0ver 1.5X4 Ⅲ 1.5X4 III 45 ф ω Supports ĊΠ. ဟ် Ń (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. 110 mph wind, 18.78 ft mean hgt, ASCE Located anywhere in roof, CAT II, EXP wind BC DL=1.2 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ $24\,^{\circ}$ OC, BC @ $24\,^{\circ}$ OC. 3X4(**)24 /// 1.5X4(**) W 1.5X4 III n B b 中 CENSO **10/59687** ATE OF 1-4-12 NO PRO 107 ENGINEER . 2X4(A1) =R=31U=180 W=6.946 * BC LL BC DL TC DL SPACING DUR.FAC T TOT.LD. FL/-/4/-/-/R/ 7-02, CLOSED bldg, B, wind TC DL=5.0 psf, 0000 32.0 24.0" 10.0 20.0 0.0 2.0 طبل . 25 نہٰ PSF PSF PSF PSF PSF JREF-FROM SEQN DATE REF DRW HCUSR487 07030026 HC-ENG Scale R487--1T4G487_Z01 .5 MNM/AF 01/30/07 149479 /Ft. 93421

Bot chord 2x4 SP / Bot chord 2x4 SP / ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 567 Refer to DWG PIGBACKA0405 or PIGBACKB0405 for piggyback details. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. PLT TYP. (6 395 Stanley Crawford Construc WILMOTH ALPINE Wave #2 Dense #2 Dense #3 =29 2X4 (A1) U=180 W=6.946" R=69 PLF U=21 PLF W=11-2-6 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, THE SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS OSSIGN, ANY FAILURE TO BHILD THE TRUSS IN CONTRIBUTE HIM THE TOR FARRITATHO, HANDLING, SHEPTH, HISTALLING A BRACTHO OF TRUSSES. IN CONTRIBUTE HIM THE LOCABLE PROVISIONS OF THOS (MATIONAL DESIGN SPEC, BY AREA) AND PH. DESIGN CONTROLS HIM APPLICABLE PROVISIONS OF THIS OSSIGN SPEC, BY AREA) AND PH. CONTROL OR PHAIRS ARE HADE OF 70/10/10/AM (MATINGS) AND ADDRESS OF THE TRUSS OF THE PHAIRS SHOW AND THE DRAWLING SHOW AND THE PHAIRS SHOW AND THE PHAIRS SHOW AND THE DRAWLING SHOW AND THE PHAIRS SHOW AND THE TRUSS COMPONENTS. ***WARNING** IRUSSES REQUIRE EXTREMÉ CARE IN FABRICATION, HANDEING, SHIPPING, INSTALLING AND BRACING REFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE STRETT, SUITE INSTALLANG, AGOO CHICERRISE TANT, HADISON, HI SATU) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERWISE INDICATED OF PUBLISHERS IN AND SON, HI SATU) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERWISE INDICATED OF PUBLISHED SHOULD HERE WILLIAMS. WHEN STREAMS AND SOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARTY AND SON THE CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARTY SIND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILE BUILDING DESIGNER PER ANSI/TPI Ш 1.5X4 Ⅲ 1.5X4 III W THE SUITABILITY AND USE OF THIS COME R PER ANSI/TPI 1 SEC. 2 -12 ப் Design Crit: 3 X 4 ≡ 2-11-2 P4) TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 0ver 1.5X4 W 1.5X4 Ⅲ 4-4-14 \Box 3 Supports 3 X 4 # In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" 0C, BC @ 24" 0C. 110 mph wind, 19.52 ft mean hgt, located within 4.50 ft from roof DL=5.0 psf, wind BC DL=1.2 psf. 1.5X4 III 4 OCENSE'S ф \Box STATE OF (o. 59687 $2X4(A1) \equiv$ =29 U=180 W=6.946" * ASCE /-edge, (BC DL SPACING DUR.FAC. 8 C TC DL TOT.LD. C Ξ 7-02, CLOSED bldg, not CAT II, EXP B, wind TC -/4/-/ 8 20.0 32.0 24.0" 10.0 1.25 ' - /R/ <u>ئ</u>ن س 0.0 2.0 PSF PSF PSF PSF PSF DATE JREF -FROM REF SEQN-HC-ENG DRW HCUSR487 07030027 Scale R487--1T4G487_Z01 =.5"/Ft. MNM/AF 01/30/07 149482 93422

PLT Refer to DWG PIGBACKA0405 or PIGBACKB0405 for piggyback details. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Haines City, FL 33844 (6-395--Stanley Crawford Construc WILMOTH FI Certificate of Authorization # 567 TYP. ALPINE Wave =29 U=180 W=6.946" 2X4 (A1) R=69 PLF U= 54 PLF W=11-2 **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE MULLDING COMPONENTS GROUP, JUZ. SHALL HOT BE RESPONSIBLE FOR ANY DEVALUEN FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPONENT WITH PEPTLEARLE PROVISIONS OF HOS (IMPING, INSTALLIGA & BRACHE OF REMSEES, ALPING CONTROL WITH PEPTLEARLE PROVISIONS OF HOS (IMITIONAL DESIGN SPEC, BY ATAPA) AND FPI. CONTROL OF THIS PROVISION OF HOS (IMITIONAL DESIGN SPEC, BY ATAPA) AND FPI. CONTROL OF THIS PROVISION OF PAULES OF THE PROVISION OF HOS OF THIS DESIGN AS AND THE SECOND OF PAULES OF THE PROVISION AND AND THE SECOND OF PAULES OF THE PROVISION OF THE PROPERSON OF THE PROP **WARNING** TRUSSES REDUIRE EXTREME CARE IN TARRICATION, INAULTIC, SHIPPING, INSTALLING AND BRACING. RETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PHULS,TICLE BY TET (TRUSS PLATE INSTITUTE, 210 HORTH LEE STREET, SUITE 3172, ALEXANDRIA, VA, 22314) AND HICA (1000) TRUSS COUNCIL OF AMERICA, 6300 ENTERPAISE LANE, MADISON, HI 53/19) FOR SAFETY PRACTICES PRIOR TO PEFFORNING THESE FUNCTIONS. UNLESS OTHERNISE HOLDSCALED TO PEODRE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Ш 1.5X4 III 1.5X4 Ⅲ 4 φ ĠΊ Design Crit: 7 ù 3×4≡ 9 2 P5) 1 TPI-2002(STD)/FBC ___Cq/RT=1.00(1.25)/10(0) BΕ 2 0ver .5X4 III .5X4 **Ⅲ** Ġ Б 3 Supports -10 3X4# 110 mph wind, 18.78 ft mean hgt, ASCE Located anywhere in roof, CAT II, EXP wind BC DL=1.2 psf. In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC. Deflection meets L/240 live and L/180 increase factor for dead load is $1.50\,\mathrm{.}$ W 1.5X4 III . 5 X 4 Ⅲ -4-6 TE OF 2X4(A1) =29 U=180 W=6.946" * BC DL TC DL ВС SPACING DUR.FAC. TC TOT.LD. FL/-/4/-/ 7-02, CLOSED bldg, B, wind TC DL=5.0 psf, total load. Creep <u></u> 32.0 10.0 20.0 24.0" 1.25 -/R/ 0.0 2.0 طیا PSF PSF PSF PSF PSF FROM SEQN DATE REF HC-ENG DRW HCUSR487 07030028 JREF -Scale R487--1T4G487_Z01 JF B =.5"/Ft. MNM/AF 01/30/07 149485 93423

Top chord Bot chord ITW Building Components Group, Inc.
Haines City, FL 33844

FI Certificate of Authorization # 567 PLT Refer to DWG PIGBACKA0405 or PIGBACKB0405 for piggyback details. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. (6-395=:Stanley Crawford Construc WILMOTH TYP. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE Wave R=232X4 (A1) U=180 W=6.946" R=70 PLF U=21 PLF -1-10 **IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRIES. IN CONFORMACE ATHE PET: OR FARRICATHO, AMOUNTED, SHIPPING, HISTALLING & BRACING OF HUSSES, ALPINE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HUS (MATIONAL DESIGN SPEC, BY ATRA) AND PET. APPLY DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HUS (MATIONAL DESIGN SPEC), BY ATRAY, AND PET. APPLY DELAYES TO EACH FACE OF TRUESS. AND. HUNC'S OTHERWISE LOCATED ON HIS DESIGN, POSITION PER DRAWHIGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERY AS OF PET. 2002 SEC. 3. A SEA, ON THIS DESIGN FOR THE STREAM OF PLATES OF PROVISIONS OF PET. 2002 SEC. 3. A SEA, ON THIS DESIGN SHOWN IN SUPPLIANCE OF PROFESSIONAL INGUITE RESPONSIBILITY AND USE OF THIS COMPONENT OF SHORT SHOWN IN SUPPLIANCE OF PROFESSIONAL INGUITE RESPONSIBILITY AND USE OF THIS COMPONENT OF SHORT SHOWN IN SUPPLIANCE OF PROFESSIONAL INGUITE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMERICAN **WARNING** IRUSSES REQUIRE EXIBHE CARE IN FABRICATION. INABELING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST. (BUILDING COMPONENT SAFELY INFORMATION). PHBLISHED BY TEI (TRUSS EVALE INSTITUTE, 218 HORTH LEE STREET, SUITE 317, ALEXANDRA, VA. 22313) AND MICA (MODD TRUSS COUNCIL OF AMERICA. 6300 ENTERFORMER, ALEXANDRA, VA. 2313) AND MICA (MODD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAIM, MADISON, MI 53719) FOR SAFELY PRACTICES PRIOR TO PERFORMING HESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO COROBO SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SMALL HAVE 111 4 X 4 == 4 1.5X4 III W=10-6-14 ú Design Crit: 2 Ψ P6) 10 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) Over 3 Supports <u>8</u>E ∞ 1.5X4 III .5X4 III -3-10 400 110 mph wind, 19.04 ft mean hgt, ASCE 7-02, CLOSED located within 4.50 ft from roof edge, CAT II, EXP DL=5.0 psf, wind BC DL=1.2 psf. In lieu of structural panels or rigid ceiling use purlins brace TC @ 24" 0C, BC @ 24" 0C. Wind reactions based on MWFRS pressures 24 1.5X4 中山 4 X 4 == 1-1-10 $2X4(A1) \equiv$ STATE OF OF MS lo. 59687 مح =23 U=180 W=6.946" * BC DL TC DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/ 2 \vdash <u>~~</u> 5 32.0 10.0 20.0 1.25 0.0 2.0 B, wind TC PSF PSF PSF PSF PSF DATE FROM SEQN-HC-ENG REF DRW HCUSR487 07030029 Scale = .5"/Ft. R487--JF B MNM/AF 01/30/07 149488 93424

Proups a princustantal spanitives of twose illu-

SPACING

24.0"

JREF-

CLB WEB BRACE SUBSTITUTION

BRACING METHOD IS DESIRED THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB

NOTES

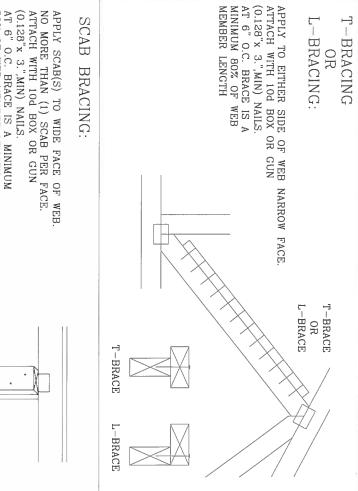
BRACING. THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

BRACING ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE

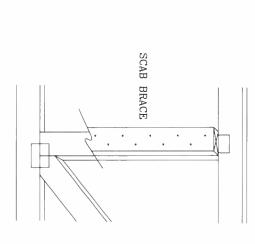
1-2X8	2X6	1 ROW	2X8
2-2X6(*)	2X6	2 ROWS	2X8
1-2X6	2X4	1 ROW	2X6
2-2X4(*)	2X6	2 ROWS	2X6
1-2X4	2X4	1 ROW	2X3 OR 2X4
2-2X4	2X6	2 ROWS	2X3 OR 2X4
/E BRACING	ALTERNATIVE BRACING T OR L-BRACE SCAB BR	SPECIFIED CLB	WEB MEMBER
SCAB BRACE		BRACING	SIZE

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

* FACE OF WEB CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH



(0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SIHT
DRAWING
REPLACES
DRAWING
579,640

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	A MANAGEMENT TO LAND	Jan LORIONAL ENGINE	STATE OF SERVING	**************************************	No.59687	A PICENSE TO	UR R. Flo
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL	THIS DE
		PSF	PSF	PSF	PSF	PSF REF	THIS DRAWING REPLACES DRAWING
			-ENG	DRWG	DATE	REF	PLACES
			-ENG MLH/KA	DRWG BRCLBS	11/1/06	CLB SU	DRAWING



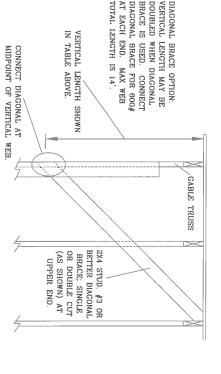
*****VARNING*** TRUSSES REDUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CTRUSS PLATE INSTITUTE, 218 NURTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA ("VDDD TRUSS COUNCIL C AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESS FUNCTIONS. UNICESS DITERVISE NOTACIED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

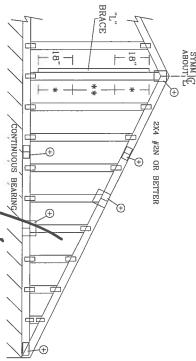
WEMPERFANIEM CURNISH CORY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FOON THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPIJ OR FABRICATING, HANDLING, SHPPING, INSTALLING BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS CNATIONAL DESIGN SPEC BY AFRICA ON THE CONNECTION PLATES ARE HADDE OF 20/18/1664 CWL/MSS/SY ASTH AGS GRADIN 40/60 (WL/MSS/SY) ASTH AGS GRADIN 40/60 SUITABILITY AND USE OF DESIGNER, PER ANSI/TPI 1

R BST. UB1106

ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 1.00, EXPOSURE

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		1	2	"		0	. (ζ.			1	6	,,		0	. (7.			2	4	,,		0	. (\mathbb{C}		SPACING	CARI
		<u> </u>) 	U.)	TIT	口 叮	770	Ω]			j 1	<u>V.</u>)	TIT	工 丁	J J C	C J			j <u>j</u>	<u>V.</u>)	TIT	I I	UTT	SI	SPECIES	CARIE VERTICAL
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 11"	5' 0"	5' 0"	5 <u>1</u>	5 4	4. 9.	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4' 6"	4' 9"	4' 10"	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4' 0"	4' 0"	4' 2"	4' 3"	3' 9"	3' 9"	3' 9"	3' 10"	BRACES	S C
	7' 5"	8' 5"	8' 5"	B, 5,	8,5	7' 3"	B, 5,	B, 5,	8 5	ص ق	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	5, 3,	6' 1"	6' 2"	6' 8"	6'8"	5 ¹	6' 0"	6' 0"	6' 8"	GROUP A	(1) 1X4 "L"
	7' 5"	8' 7"	8 5"	9' 1"	9' 1"	7' 3"	80,	හ _. ග්.	8' 8"	6, 5,	7' 6"	7' 7"	8' 3"	B, 3,	6' 4"	7' 4"	7' 4"	7' 10"	5. 3.	6' 1"	6' 2"	7' 2"	7' 2"	5 2	6' 0"	6'0"	6' 10"	GROUP B	" BRACE *
	9' 10"	10' 0"	10' 0"	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7' 11"	7' 11"	7' 11"	6'9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
MAS	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8'0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	BRACE *
SAMM IS	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9'5"	9' 5"	9' 5"	9′ 5″	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10' 10"	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	BRACE **
	14' 0"	- 1	14' 0"			14' 0"		14′0″				14' 0"			12' 11"		14'0"	14'0"		12' 5"					12' 3"			GROUP A	(1) 2X6 "L"
	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"			14'0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"		- 1	12' 8"	-1	13' 5"	- 1	12' 3"	12' 4"	12' 9"	GROUP B	BRACE *
	14' 0"	- 1	14' 0"	14' 0"	14′0″	14' 0"	14' 0"	14' 0"		- 1	14' 0"		14, 0,	14' 0"	14' 0"	14' 0"	- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L"
	14' 0"	1	14' 0"		14' 0"	14'0"	14'0"	14'0"	- 1	14' 0"		- 1	14'0"			14'0"		14'0"	14' 0"		14'0"	14' 0"	- 1			14' 0"	14' 0"	GROUP B	BRACE **





A: HEM-FIR #2 STUNDARD SOUTHERN PINE #3 STUNDARD SOUTHERN PINE STUNDARD STANDARD B: B: DUGLAS FIR-LARCH #1 #1		#1 #2 DOUGLAS FIR—LARCH	#1 & BTR #1 #1	GROUP B:		RD	#3 #3 STUD STUD	JGLAS FIR-LARCH SOUTHERN PINE	STUD #3	HEM-FI	GROUP A:	
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BR,

GABLE TRUSS DETAIL NOTES:

GABLE END SUPPORTS LOAD FROM 4' 0" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. CONTINUOUS BEARING (5 PSF TC DEAD LOAD). OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

ATTACH EACH "L" BRACE WITH 10d NAILS.

*FOR (1) "L" BRACE: SPACE NAILS TO 2" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

**FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

PLYWOOD OVERHANG.

GREATER THAN 4' 0", BUT	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES	
2X4	1X4 OR 2X3	NO SPLICE	TE SIZES	
	ХЗ	ř		

MEMBER LENGTH.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB

LENGTH GREATER THAN 11' 6 REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. DATE REF DRWG ENG A11015EE1106 ASCE7-02-GAB11015 11/1/06 2.5X4

REFER TO CHART ABOVE FOR MAX GABI

AVARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST GALLENG COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CTRUSS PLATE INSTITUTE. 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VICA VOOD TRUSS COLNCIL OF ARRICA. 6300 ENTERPRISE LIN, MADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TO FORD SHALL HAVE PROBERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

***IMPRITANT** FURNISH CORP OF THIS DESIGN TO INSTALLATION CONTRACTOR ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSSS IN CORPORANCE WITH FIJ OR FABRICATING, HANDLING, SUPPING, INSTALLING BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFBEA) AND TPI. ALPINE CONNECTIOR PLATES OF LOCATED ON THIS DESIGN, POSITION FER DRAWING SECOND TRUSS AND, UNLESS DIFFERVISE LOCATED ON THIS DESIGN, POSITION FER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES OF THIS DESIGN, POSITION FER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANKEY AS OF THIS -2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL BEGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING

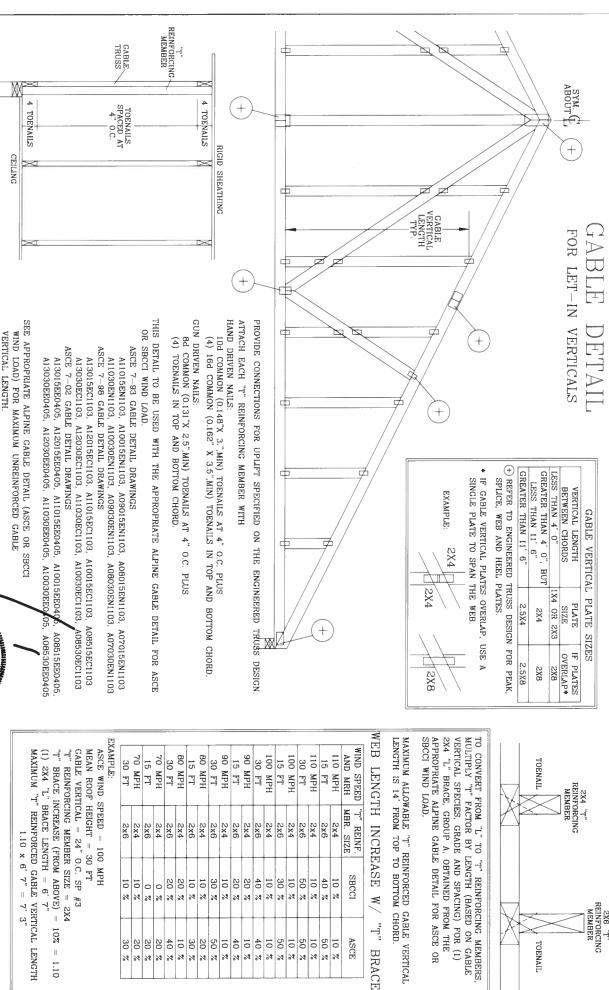
ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

ALPINE

CORIOR ING. CONSE **5.59687** ्रा गामस्याः MAX. MAX. TOT. LD. 60

PSF

SPACING 24.0"



TOENAIL 2X4 "T" REINFORCING MEMBER 2X6 "T" REINFORCING MEMBER TOENAIL

2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD. VERTICAL SPECIES, GRADE AND SPACING) FOR (1) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WIND SPEED	"T" REINF.	CDCCI	ACCE
AND MRH	MBR. SIZE	SBCC1	ADCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	2 01
30 FT	2x6	50 %	2 09
100 MPH	2x4	10 %	2 01
15 FT	2x6	30 %	20 %
100 MPH	2x4	10 %	2.01
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	7 01
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	2,0	20 %
15 FT	2x6	2 0	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE: MAXIMUM (1) 2X4 "L" BRACE LENGTH = 6' 7" "T" REINFORCING MEMBER SIZE = 2X4 GABLE VERTICAL = 24" O.C. SP #3 MEAN ROOF HEIGHT = 30 FT ASCE WIND SPEED = 100 MPH "T" BRACE INCREASE (FROM ABOVE) = 10% "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

= 1.10

REPLACES DRAWINGS GAB98117 876,719 & HC26294035

	AND STREET, SALES	MAX MAX	STATE OF SE	*	0.59687	Sense Co
MAX	DUR.	MAX	Willian)	mu	MULL	Man
MAX SPACING 24.0"	DUR. FAC.	TOT. LD. 60 PSF				
NG :	ANY	LD. 60				
24.0"	Y	0 PSF				
			-ENG	DRWG	DATE	REF
			-ENG DLJ/KAR	GBLLETIN1106	11/1/06	LET-IN VERT

ALPINE ENGINEERED PRODUCTS, INC. POMPANO BEACH, FLORIDA

SUITABILITY AND USE OF DESIGNER, PER ANSI/TPI

WAMPRITANIEW FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSS IN COMPORANCE WITH FIJ OR FABRICATING, HANDLING, SHPPING, INSTALLING BRACING OF TRUSSSS. DESIGN COMPORANCE WITH FIJ OR FABRICATING, HANDLING, SHPPING HANDLING, STEPLANDLING, STEPLANDLIN

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ALPINE