

**Project Information for:** 

L263148

Builder:

Hugo Escalante

Lot:

4 and 5

Subdivision:

3 Rivers Estates

County:

Columbia

Truss Count:

Design Program: MiTek 20/20 6.3 Building Code:

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Roof (psf): 42.0

Wind:

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions. Contractor of Record, responsible for structural engineering:

Hugo Escalante Florida License No. CRC1326967

Address: P.O. Box 280 Fort White, Florida 32038

Drwg. #

J1919410

J1919411

J1919412

J1919413

J1919414

J1919415

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T07

T08

T09

T10

T10G

T09G

Date

12/20/07

12/20/07

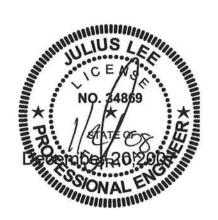
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12/20/07

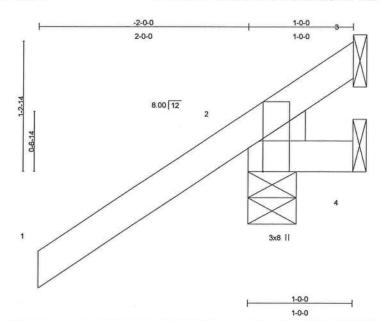
No.	Drwg. #	Truss ID	Date	No
1	J1919382	CJ1	12/20/07	29
2	J1919383	CJ11	12/20/07	30
3	J1919384	CJ11A	12/20/07	31
4	J1919385	CJ3	12/20/07	32
5	J1919386	CJ5	12/20/07	33
6	J1919387	CJ5A	12/20/07	34
7	J1919388	CJ7	12/20/07	
8	J1919389	CJ7A	12/20/07	
9	J1919390	CJ9	12/20/07	
10	J1919391	CJ9A	12/20/07	
11	J1919392	EJ13	12/20/07	
12	J1919393	EJ13A	12/20/07	
13	J1919394	EJ13B	12/20/07	
14	J1919395	EJ13C	12/20/07	
15	J1919396	EJ13D	12/20/07	
16	J1919397	HJ18	12/20/07	
17	J1919398	HJ18A	12/20/07	
18	J1919399	PB1	12/20/07	
19	J1919400	PB1A	12/20/07	
20	J1919401	PB1B	12/20/07	
21	J1919402	PB1C	12/20/07	
22	J1919403	PB1D	12/20/07	
23	J1919404	T01	12/20/07	
24	J1919405	T02	12/20/07	
25	J1919406	T03	12/20/07	
26	J1919407	T04	12/20/07	
27	J1919408	T05	12/20/07	
28	J1919409	T06	12/20/07	



Ply Job Truss Truss Type Qty 00 J1919382 CJ1 L263148 **JACK** 8 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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Scale = 1:10.4

Plate Of	ffsets (X,)	(): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mati	rix)						Weight: 8 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=261/0-5-8, 4=5/Mechanical, 3=-95/Mechanical

Max Horz 2=112(load case 6)

Max Uplift 2=-286(load case 6), 4=-11(load case 4), 3=-95(load case 1) Max Grav 2=261(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/52, 2-3=-101/103

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.12 and 2 = 0.00

#### **NOTES**

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

& All hearing page assumed to be SYP No.2 crushing capacity of 565.00 psi

December 20,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek con Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0.0	
L263148	CJ1	JACK	8	1		J1919382
					Job Reference (optional)	

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# NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 11 lb uplift at joint 4 and 95 lb uplift at joint 3.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ11	MONO TRUSS	6	1	J <sup>1</sup>	1919383
		merre mese			Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 19 16:13:42 2007 Page 1

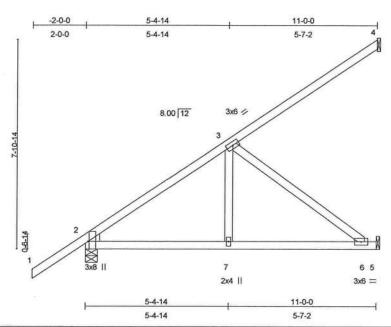


Plate Of	fsets (X,Y	):	[2:0-3-8,Edge]										
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.29	Vert(LL)	0.15	6-7	>884	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	ВС	0.41	Vert(TL)	-0.07	6-7	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.25	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 55 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

oc purlins.

BOT CHORD Rigid ceiling

Rigid ceiling directly applied or 7-9-8 oc bracing.

REACTIONS (lb/size) 4=120/Mechanical, 2=476/0-5-8, 5=210/Mechanical

Max Horz 2=297(load case 6)

Max Uplift 4=-84(load case 6), 2=-250(load case 6), 5=-217(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/53, 2-3=-417/317, 3-4=-99/50

BOT CHORD

2-7=-510/272, 6-7=-510/272, 5-6=0/0

WEBS

3-7=-377/206, 3-6=-340/637

# JOINT STRESS INDEX

2 = 0.36, 2 = 0.00, 3 = 0.35, 6 = 0.21 and 7 = 0.15

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

December 20,2007

Scale = 1:41.0

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center,
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ11	MONO TRUSS	6	1		J1919383
					Job Reference (optional)	

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#### NOTES

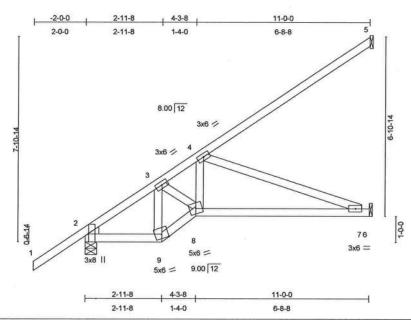
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 4, 250 lb uplift at joint 2 and 217 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ11A	SPECIAL	2	1	J19	919384
2200110	001111	OI LOWE	-		Job Reference (optional)	

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Scale = 1:41.9

Plate Of	ffsets (X, Y	/): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.07	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.12	7-8	>999	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.01	6	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 59 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 9-9-0 oc

bracing.

REACTIONS (lb/size) 5=141/Mechanical, 2=476/0-5-8, 6=189/Mechanical

Max Horz 2=297(load case 6)

Max Uplift 5=-98(load case 6), 2=-116(load case 6), 6=-69(load case 6) Max Grav 5=141(load case 1), 2=476(load case 1), 6=195(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-391/0, 3-4=-521/132, 4-5=-117/59 BOT CHORD 2-9=-216/243, 8-9=-254/319, 7-8=-419/484, 6-7=0/0 WEBS 3-9=-222/189, 3-8=-267/326, 4-8=0/257, 4-7=-513/444

#### JOINT STRESS INDEX

2 = 0.30, 2 = 0.00, 3 = 0.21, 4 = 0.19, 7 = 0.14, 8 = 0.79 and 9 = 0.13

# **NOTES**

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 
B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This 
truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Collyn page 2

Truse Design Engineer Florida PE No. 24869 1100 Ccastal Bay Blvd Boynton Beach, FL 33435

December 20,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors, Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ11A	SPECIAL	2	1	0.00 (17) 0	J1919384
2200110		OI LOW L	-		Job Reference (optional)	

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#### NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 5, 116 lb uplift at joint 2 and 69 lb uplift at joint 6.

LOAD CASE(S) Standard



 Job
 Truss
 Truss Type
 Qty
 Ply
 0 0
 J1919385

 L263148
 CJ3
 JACK
 8
 1
 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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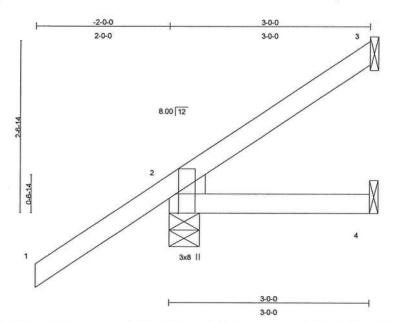


Plate Of	fsets (X,Y	(): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	2-4	>999	240	((01/03/03/03/03))	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 14 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or

3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=24/Mechanical, 2=257/0-5-8, 4=14/Mechanical

Max Horz 2=172(load case 6)

Max Uplift 3=-40(load case 7), 2=-226(load case 6), 4=-32(load case 4) Max Grav 3=33(load case 4), 2=257(load case 1), 4=41(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-78/19

BOT CHORD 2-4=0/0

# JOINT STRESS INDEX

2 = 0.11 and 2 = 0.00

# **NOTES**

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All hearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Design Engineer Florida PE No. 34869 1109 Coestal Bay Blyd Boynton Beach, FL 33435

December 20,2007

Scale = 1:16.2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building doe. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	СЈЗ	JACK	8	1	300,000	J1919385
				1000	Job Reference (optional)	

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#### NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3, 226 lb uplift at joint 2 and 32 lb uplift at joint 4.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ5	JACK	6	1		J1919386
		or tort	ľ		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:01 2007 Page 1

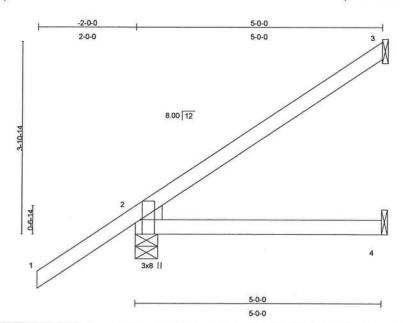


Plate Of	fsets (X, Y	'): [2:0-3-8,Edge]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.08	2-4	>699	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.05	2-4	>999	240	(10000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)	**************************************					Weight: 21 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=99/Mechanical, 2=300/0-5-8, 4=24/Mechanical

Max Horz 2=233(load case 6)

Max Uplift 3=-113(load case 6), 2=-235(load case 6), 4=-55(load case 4)

Max Grav 3=99(load case 1), 2=300(load case 1), 4=71(load case 2)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-99/44

BOT CHORD 2-4=0/0

# JOINT STRESS INDEX

2 = 0.13 and 2 = 0.00

# NOTES

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

\*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All hearings age assumed to be SYP No.2 crushing capacity of 565.00 psi

Truse Cesian Engineer Florida PE No. 34889 1 100 Ceastal Bay Blvd. Boynton Beach, FL 35435

December 20,2007

Scale = 1:22.1

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ5	JACK	6	1		J1919386
			157.9		Job Reference (optional)	

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#### NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 3, 235 lb uplift at joint 2 and 55 lb uplift at joint 4.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ5A	SPECIAL	2	1		J1919387
	00071				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:01 2007 Page 1

Scale = 1:22.1

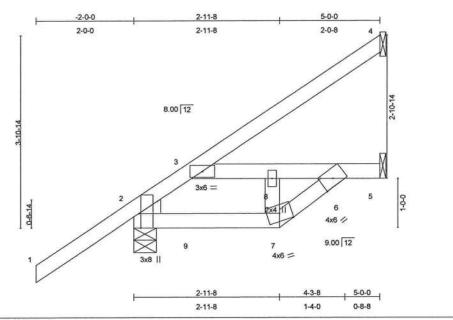


Plate Of	fsets (X,Y	'): [2:0-3-8,Edge]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.09	6-8	>604	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.03	6-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 27 lb	

1	11	NA	B	_	
	"	w	_	_	к

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3

WEBS WEDGE

Left: 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 8-5-9 oc

bracing.

REACTIONS (lb/size) 4=86/Mechanical, 2=314/0-5-8, 5=52/Mechanical

Max Horz 2=283(load case 6)

Max Uplift 4=-92(load case 6), 2=-331(load case 6), 5=-191(load case 7) Max Grav 4=86(load case 1), 2=314(load case 1), 5=113(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-180/399, 3-4=-71/37

2-9=-509/92, 7-9=-509/92, 6-7=-541/124, 3-8=-92/374, 6-8=-91/361, 5-6=0/0 **BOT CHORD** 

7-8=-30/88 **WEBS** 

#### JOINT STRESS INDEX

Continued on page 2

2 = 0.21, 2 = 0.00, 3 = 0.15, 6 = 0.10, 7 = 0.13 and 8 = 0.05

# NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

December 20,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00	200000000000000000000000000000000000000
L263148	CJ5A	SPECIAL	2	1		J1919387
1000 Maria (1000 M					Job Reference (optional)	

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# **NOTES**

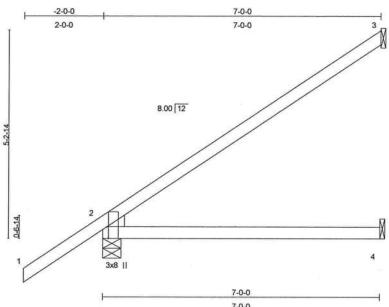
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 4, 331 lb uplift at joint 2 and 191 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	NOW ARRANGED THE OWNERS AND A
L263148	CJ7	MONO TRUSS	6	1		J1919388
Service Co. 1	3-20			1	Job Reference (optional)	

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7-0-0 Plate Offsets (X,Y): [2:0-3-8,Edge] LOADING (psf) SPACING 2-0-0 CSI DEFL I/defl **PLATES** GRIP (loc) TCLL 20.0 1.25 TC 0.44 Plates Increase Vert(LL) 0.30 2-4 >264 360 MT20 244/190 TCDL 1.25 BC Lumber Increase 0.44 Vert(TL) -0.162-4 >513 240 BCLL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 3=151/Mechanical, 2=355/0-5-8, 4=44/Mechanical REACTIONS

Max Horz 2=211(load case 6)

Max Uplift 3=-116(load case 6), 2=-201(load case 6), 4=-67(load case 5) Max Grav 3=151(load case 1), 2=355(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/53, 2-3=-151/67

**BOT CHORD** 2-4=0/0

### JOINT STRESS INDEX

2 = 0.52 and 2 = 0.00

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 3, 201 lb uplift at joint 2 and 67 lb uplift at joint 4.

# LOAD CASE(S) Standard

December 20,2007

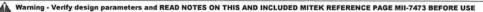
Scale = 1:27.4

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.



Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occe. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

December 20,2007



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Julius Les Trues Design Engineer Flonds PE No. 34868 1 199 Constal Bay Blvd 1 199 Constal Bay Blvd 1 199 Constal Bay Blvd

December 20,2007

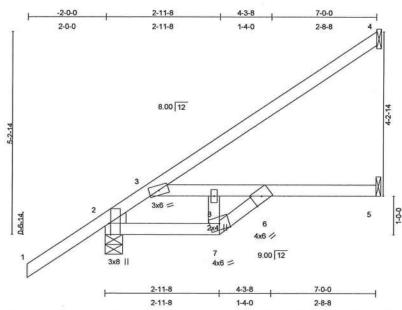


Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building component into the overall building component into the coverall building component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building component into the overall building and bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ7A	SPECIAL	2	1		J1919389
Control State of the	DT-T100A				Job Reference (optional)	

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Scale = 1:28.0

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.07	6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.14	6	>587	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 34 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=129/Mechanical, 2=374/0-5-8, 5=77/Mechanical

Max Horz 2=211(load case 6)

Max Uplift 4=-87(load case 6), 2=-105(load case 6), 5=-3(load case 6)

Max Grav 4=129(load case 1), 2=374(load case 1), 5=134(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-291/0, 3-4=-108/54

BOT CHORD 2-7=-174/221, 6-7=-193/226, 3-8=-221/175, 6-8=-216/149, 5-6=0/0

WEBS 7-8=-128/172

# JOINT STRESS INDEX

2 = 0.50, 2 = 0.00, 3 = 0.69, 6 = 0.42, 7 = 0.16 and 8 = 0.09

# NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 
B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This 
truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive leads a page 2

Julius Lare Truse Design Engineer Florida PE No. 24869 1100 Caastel Bay Blvd Boynton Basch, Et. 33436

December 20,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ7A	SPECIAL	2	1		J1919389
					Job Reference (optional)	

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# **NOTES**

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 4, 105 lb uplift at joint 2 and 3 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ9	MONO TRUSS	6	1		J1919390
					Job Reference (optional)	

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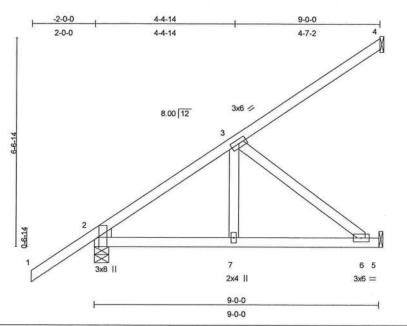


Plate Of	fsets (X,Y	):	[2:0-3-8,Edge]								1		
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.28	Vert(LL)	0.08	6-7	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.04	6-7	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	3CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)	, ,					Weight: 46 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

**WEBS** 

2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0

**BOT CHORD** 

Rigid ceiling directly applied or 9-0-8 oc bracing.

REACTIONS (lb/size) 4=100/Mechanical, 2=414/0-5-8, 5=163/Mechanical

Max Horz 2=254(load case 6)

Max Uplift 4=-71(load case 6), 2=-225(load case 6), 5=-171(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/53, 2-3=-327/242, 3-4=-83/42

**BOT CHORD** 

2-7=-404/206, 6-7=-404/206, 5-6=0/0

**WEBS** 

3-6=-260/510, 3-7=-303/167

# JOINT STRESS INDEX

2 = 0.39, 2 = 0.00, 3 = 0.28, 6 = 0.17 and 7 = 0.12

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

December 20,2007

Scale = 1:34 3

Continued on page 2 👠 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





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Job	Truss	Truss Type	Qty	Ply	00	222
L263148	CJ9	MONO TRUSS	6	1		J1919390
					Job Reference (optional)	

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# NOTES

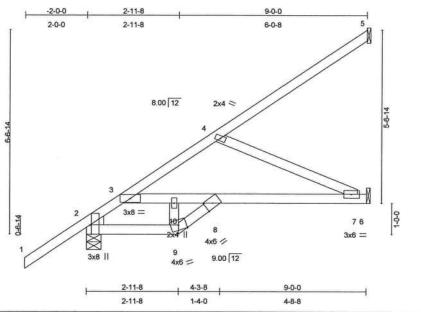
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 4, 225 lb uplift at joint 2 and 171 lb uplift at joint 5.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ9A	SPECIAL	2	1		J1919391
					Job Reference (optional)	

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		1	2-11-8		4-3-8	9-0-	00				
		1.5	2-11-8		1-4-0	4-8-	8				
fsets (X,Y	): [2:0-3-8,Edge]										
G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.15	7-8	>714	360	MT20	244/190
7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.27	7-8	>384	240	Short-surfaction on	
10.0	* Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.06	6	n/a	n/a		
5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 47 lb	
	IG (psf) 20.0 7.0 10.0	G (psf) 20.0 7.0 10.0 Plates Increase Lumber Increase * Rep Stress Incr	SPACING   2-0-0   Plates Increase   1.25   10.0   * Rep Stress Incr	2-11-8	2-11-8	2-11-8   1-4-0	2-11-8   1-4-0   4-8-	SPACING   2-0-0   CSI   DEFL   In (loc)	SPACING   2-0-0   CSI   DEFL   In (loc)   I/defl	SPACING   2-0-0   CSI   DEFL   in (loc)   l/defl   L/d	SPACING   2-0-0   CSI   DEFL   In (loc)   I/defl   L/d   PLATES

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=103/Mechanical, 2=414/0-5-8, 6=160/Mechanical

Max Horz 2=254(load case 6)

Max Uplift 5=-75(load case 6), 2=-115(load case 6), 6=-58(load case 6) Max Grav 5=103(load case 1), 2=414(load case 1), 6=166(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-240/0, 3-4=-339/78, 4-5=-88/43

BOT CHORD 2-

2-9=-88/67, 8-9=-98/66, 3-10=-195/214, 8-10=-208/216, 7-8=-283/281, 6-7=0/0

WEBS

9-10=-11/91, 4-7=-307/309

#### JOINT STRESS INDEX

2 = 0.27, 2 = 0.00, 3 = 0.60, 4 = 0.16, 7 = 0.08, 8 = 0.32, 9 = 0.09 and 10 = 0.06

#### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Colive leads page 2

Trues Cesian Engineer Florida PE No. 34869 1100 Coastal Bay Blvd Boynton Beach, FL 33435

December 20,2007

Scale = 1:35.0

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTok connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ocd. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	CJ9A	SPECIAL	2	1		J1919391
			_		Job Reference (optional)	

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# NOTES

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 5, 115 lb uplift at joint 2 and 58 lb uplift at joint 6.

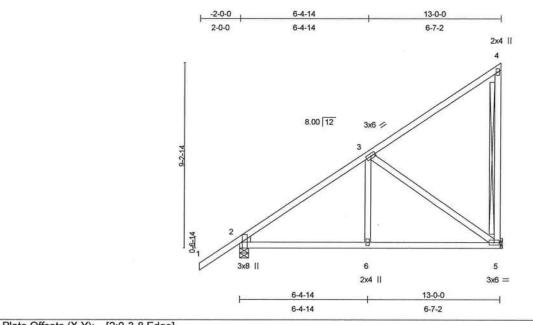
LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Planda PE No. 34889 1 103 Gressel Bay Blvd Boynton Besch Et 33435



Job	Truss	Truss Type	Qty	Ply	0 0	
L263148	EJ13	MONO TRUSS	3	1		J1919392
					Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.15	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.08	5-6	>999	240	DOMES STANDARD	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.43	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		PI2002	(Mati	rix)						Weight: 77 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 **WEBS** WEDGE

Left: 2 X 4 SYP No.3

#### BRACING TOP CHORD

**BOT CHORD WEBS** 

Structural wood sheathing directly applied or 6-0-0

oc purlins.

Rigid ceiling directly applied or 7-5-2 oc bracing. 2 X 4 SYP No.3 - 4-5

T-Brace:

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 2=535/0-5-8, 5=393/Mechanical

Max Horz 2=338(load case 6)

Max Uplift 2=-276(load case 6), 5=-358(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD** 

1-2=0/53, 2-3=-495/372, 3-4=-117/57

**WEBS** 

2-6=-597/328, 5-6=-597/328 3-6=-405/227, 3-5=-401/729, 4-5=-138/144

#### JOINT STRESS INDEX

2 = 0.36, 2 = 0.00, 3 = 0.39, 4 = 0.07, 5 = 0.34 and 6 = 0.17

# NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live Confinded on page 2

December 20,2007

Scale = 1:54.2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	LOTTE SHAPE TO A SECOND
L263148	EJ13	MONO TRUSS	3	1		J1919392
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:48:02 2007 Page 2

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 2 and 358 lb uplift at joint

LOAD CASE(S) Standard



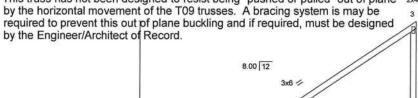
Job	Truss	Truss Type	Qty	Ply	00	
L263148	EJ13A	MONO TRUSS	2	2		J1919393
					Job Reference (optional)	

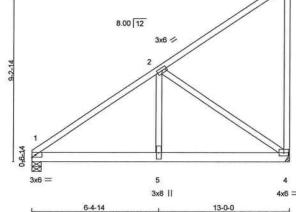
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WARNING: This truss has not been designed to resist being "pushed or pulled" out of plane by the horizontal movement of the T09 trusses. A bracing system is may be

Scale = 1:54.7





Simpson HGUS28-2

				0-4-14			0-1-2					
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.06	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.11	4-5	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.01	4	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 167 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=2374/0-5-8, 4=2374/Mechanical

Max Horz 1=280(load case 5)

Max Uplift 1=-595(load case 5), 4=-789(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2587/596, 2-3=-149/44, 3-4=-135/99

BOT CHORD 1-5=-689/2056, 4-5=-689/2056

WEBS 2-5=-665/2450, 2-4=-2452/817

# **JOINT STRESS INDEX**

1 = 0.70, 2 = 0.91, 3 = 0.65, 4 = 0.33 and 5 = 0.39

#### **NOTES**

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60. Continued on page 2

Julius Les Truss Design Engineer Flonda PE No. 34869 1100 Ceastal Bay Blvd Bovnton Basch, Ft. 33434

December 20,2007

🛦 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	5W7 - 5 C C C C C C C C C C C C C C C C C C
L263148	EJ13A	MONO TRUSS	2	_		J1919393
	333333333			2	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:04 2007 Page 2

#### NOTES

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 1 and 789 lb uplift at joint 4.

# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 1-4=-322(F=-312)

Julius Lee Truse Cesian Engineer Flonda PE No. 34869 1100 Ceasial Ray Blvd Bovoton Beach, Fl. 33431



Job	Truss	Truss Type	Qty	Ply	00	
L263148	EJ13B	MONO TRUSS	3	1		J1919394
0					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 19 16:21:18 2007 Page 1

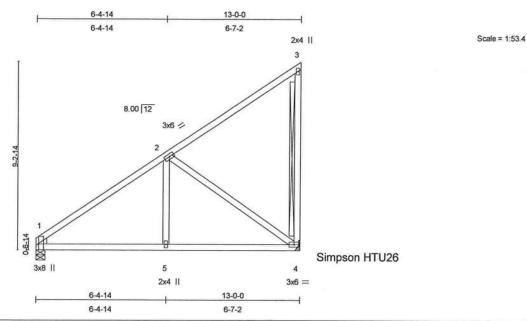


Plate Of	fsets (X,Y)	[1:0-3-8,Edge]										
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.05	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.08	4-5	>999	240	WW. 25	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.01	4	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)	,					Weight: 74 lb		

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

# BRACING

TOP CHORD

**BOT CHORD WEBS** 

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2 X 4 SYP No.3 - 3-4

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 4=404/Mechanical, 1=404/0-5-8

Max Horz 1=283(load case 6)

Max Uplift 4=-211(load case 6), 1=-13(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-515/3, 2-3=-118/56

**BOT CHORD** 1-5=-300/350, 4-5=-300/350

WEBS 2-5=0/232, 2-4=-428/367, 3-4=-135/143

#### JOINT STRESS INDEX

1 = 0.48, 1 = 0.00, 2 = 0.20, 3 = 0.07, 4 = 0.17 and 5 = 0.17

# **NOTES**

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

December 20,2007

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	40-25
L263148	EJ13B	MONO TRUSS	3	1		J1919394
ELGGTAG	20,00	IMONO TROCC			Job Reference (optional)	

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#### NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 4 and 13 lb uplift at joint

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L263148	EJ13C	SPECIAL	1	1		J1919395
					Job Reference (optional)	

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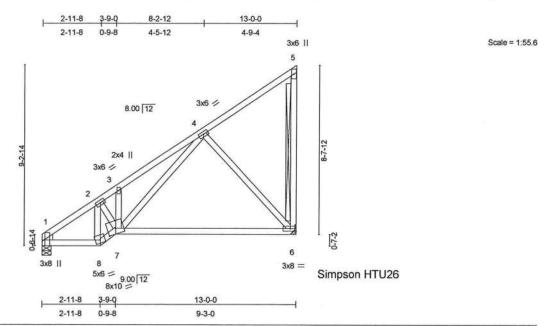


Plate Of	fsets (X,Y	):	[1:0-3-8,Edge], [7:0-	-5-0,0-3-5]									
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.41	Vert(LL)	-0.13	6-7	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.23	6-7	>659	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	212002	(Mat	rix)	30.3					Weight: 83 lb	i i

#### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

#### BRACING

TOP CHORD

**BOT CHORD WEBS** 

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 9-8-5 oc bracing.

T-Brace:

2 X 4 SYP No.3 - 5-6 Fasten T and I braces to narrow edge of web with

10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 6=404/Mechanical, 1=404/0-5-8

Max Horz 1=283(load case 6)

Max Uplift 6=-211(load case 6), 1=-13(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-564/60, 2-3=-616/146, 3-4=-666/255, 4-5=-91/47, 5-6=-103/110

**BOT CHORD** 1-8=-391/402, 7-8=-408/482, 6-7=-225/243

**WEBS** 2-8=-336/148, 2-7=-62/294, 3-7=-148/159, 4-7=-299/431, 4-6=-346/331

#### JOINT STRESS INDEX

1 = 0.32, 1 = 0.00, 2 = 0.23, 3 = 0.08, 4 = 0.29, 5 = 0.15, 6 = 0.34, 7 = 0.47 and 8 = 0.14

# NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live

a) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

December 20,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0 0	720 20 30 30 30 30
L263148	EJ13C	SPECIAL	1	1		J1919395
					Job Reference (optional)	

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#### **NOTES**

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 6 and 13 lb uplift at joint 1.

LOAD CASE(S) Standard

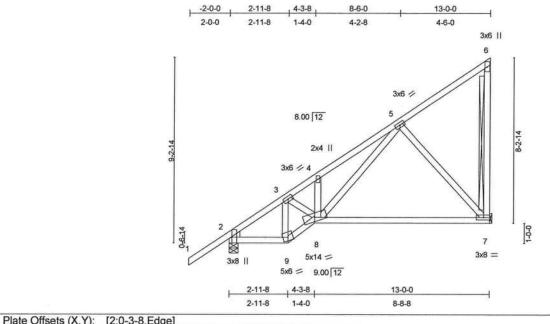
Julius Les Trues Design Engineer Flonds PE No. 34868 I 109 Ceastel Bay Blvd Brown Basen El 20448



Job Truss Truss Type Qty Ply 00 J1919396 L263148 EJ13D SPECIAL 1 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.11	7-8	>999	360	MT20	244/190
TCDL	7.0	1	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.19	7-8	>788	240	200,20000	
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	212002	(Mate	rix)	25/2012/3/2014 <b>9</b> /3/2014 <b>8</b> /3/					Weight: 86 lb	)

#### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

## BRACING TOP CHORD

**BOT CHORD** 

**WEBS** 

Structural wood sheathing directly applied or 6-0-0

oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.3 - 6-7

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum

end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 7=393/Mechanical, 2=535/0-5-8

Max Horz 2=338(load case 6)

Max Uplift 7=-200(load case 6), 2=-117(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/53, 2-3=-494/0, 3-4=-619/132, 4-5=-662/247, 5-6=-86/44, 6-7=-97/104

**BOT CHORD** 2-9=-296/333, 8-9=-329/408, 7-8=-213/237

**WEBS** 3-9=-263/210, 3-8=-142/235, 4-8=-157/174, 5-8=-292/444, 5-7=-341/317

# JOINT STRESS INDEX

2 = 0.40, 2 = 0.00, 3 = 0.16, 4 = 0.09, 5 = 0.30, 6 = 0.14, 7 = 0.31, 8 = 0.91 and 9 = 0.18

#### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

\*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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Scale = 1:54.1

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0 0	
L263148	EJ13D	SPECIAL	1	1		J1919396
					Job Reference (optional)	

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#### NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 200 lb uplift at joint 7 and 117 lb uplift at joint

LOAD CASE(S) Standard

ullus Les russ Cesian Endineer londa PE No. 34889 169 Ceastel Bay Blvd loynton Besch, FL 33436



Job Truss Truss Type 00 Qty Ply J1919397 L263148 **HJ18** MONO TRUSS 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:22:49 2007 Page 1 -2-9-15 4-6-2 8-11-15 13-5-13 18-4-10 4-6-2 4-5-13 2-9-15 4-5-13 4-10-13 Scale = 1:53.3 3x6 II 5.66 12 4x6 -5x6 = 3x6 = 5x6 = 10 9 15 16 8 17 2x4 II 3x6 = 3x6 =8x10 = Simpson HTU28X SKL/R45 w/ (26) 16d common wire nails in the supporting member w/ (20) 10d1.5 common wire nails into the supported member Supporting member: (2) ply and (3) ply 2x8 SYP 4-6-2 8-11-15 13-5-13 18-4-10 4-6-2 4-5-13 4-5-13 4-10-13 Plate Offsets (X,Y): [4:0-3-0,0-3-0], [7:Edge,0-3-8] LOADING (psf) SPACING 2-0-0 CSI DEFL (loc) I/defl L/d **PLATES GRIP** TCLL 20.0 1.25 Plates Increase TC 0.64 Vert(LL) 0.17 7-8 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.90 Vert(TL) -0.187-8 >999 240 BCLL 10.0 NO WB 0.85 Rep Stress Incr -0.04Horz(TL) n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 122 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 4-5-12 BOT CHORD 2 X 4 SYP No.2 oc purlins. **WEBS** 2 X 4 SYP No.3 BOT CHORD Rigid ceiling directly applied or 4-10-9 oc bracing. WEDGE 2 X 4 SYP No.3 - 6-7, 5-7 **WEBS** T-Brace: Left: 2 X 4 SYP No.3 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. REACTIONS (lb/size) 7=1512/Mechanical, 2=1127/0-8-8, 6=167/Mechanical Brace must cover 90% of web length. Max Horz 2=478(load case 5) Max Uplift 7=-1438(load case 5), 2=-930(load case 5), 6=-148(load case 5) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-11=0/50, 2-11=0/50, 2-3=-1774/1345, 3-4=-1654/1253, 4-12=-1206/911, 12-13=-1075/848

5-13=-950/781, 5-14=-141/18, 6-14=-73/60

2-10=-1496/1516, 9-10=-1496/1516, 9-15=-1374/1432, 15-16=-1374/1432,

8-16=-1374/1432, 8-17=-934/990, 7-17=-934/990

**WEBS** 6-7=0/0, 3-10=-80/120, 3-9=-105/148, 4-9=-197/211, 4-8=-632/630, 5-8=-1118/1144,

5-7=-1692/1596

#### JOINT STRESS INDEX

**BOT CHORD** 

2 = 0.73, 2 = 0.00, 3 = 0.40, 4 = 0.59, 5 = 0.71, 6 = 0.15, 7 = 0.25, 8 = 0.75, 9 = 0.35 and 10 = 0.34

December 20,2007

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L263148	НЈ18	MONO TRUSS	3	1		J1919397
					Job Reference (optional)	

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#### **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1438 lb uplift at joint 7, 930 lb uplift at joint 2 and 148 lb uplift at joint 6.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-11=-54, 6-12=-54, 7-15=-10

Concentrated Loads (lb)

Vert: 13=-200(F=-100, B=-100) 14=-240(F=-120, B=-120) 16=-326(F=-163, B=-163) 17=-420(F=-210, B=-210)

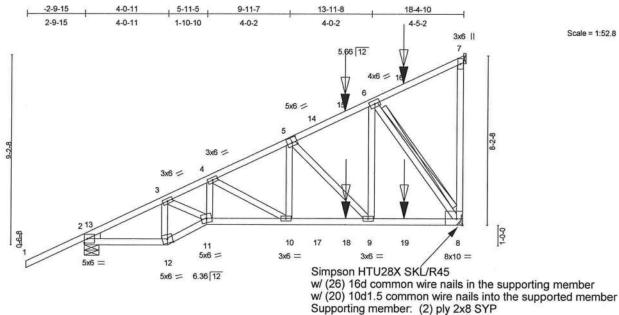
Trapezoidal Loads (plf)

Vert: 11=0(F=27, B=27)-to-12=-153(F=-49, B=-49), 2=-1(F=5, B=5)-to-15=-28(F=-9, B=-9)

Julius Les fruse Cosign Engineer Flonds PE No. 34888 I 109 Coastal Bay Blvd Boynton Usach, Ft. 93495



Job	Tru	ss		Truss Ty	pe		Qty	Ply	00	Two the way of the delivery
L263148	HJ1	8A		SPECIAL	Ē		1	1		J191939
Builders FirstSo			32055			6 300 a Apr	10 2006 1	AiTok Inc	Job Reference (optional)	
Duliders Filston	ource, Lake	City, 11	32033			0.300 S Apr	19 2000 1	vii i ek iiic	dustries, Inc. Thu Dec 20	12.30.32 2007 Page 1
	Ĺ	-2-9-15		4-0-11	5-11-5	9-11-7	1	3-11-8	18-4-10	
	1.0	2-0-15		4.0.11	1 10 10	402		102	4.50	



	H	4-0-11	0-11-0	9-11-7	13-11-8	18-4-10	4
	•	4-0-11	1-10-10	4-0-2	4-0-2	4-5-2	2
DI	 						_

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.15	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.15	10-11	>999	240	53/3/2556	177.27
BCLL	10.0	* Rep Stress Incr	NO	WB	0.72	Horz(TL)	-0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 122 lb	

BRACING TOP CHORD

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
WEDGE	

Left: 2 X 4 SYP No.3

SYP No.3 BOT CHORD WEBS

Structural wood sheathing directly applied or 3-10-5 oc purlins, except end verticals.
Rigid ceiling directly applied or 4-2-8 oc bracing.

T-Brace: 2 X 4 SYP No.3 - 6-8
Fasten T and I braces to narrow edge of web with
10d Common wire nails, 9in o.c., with 4in minimum
end distance.

nical, 2=1131/0-8-8, 7=173/Mechanical Brace must cover 90% of web length.

REACTIONS (lb/size) 8=1525/Mechanical, 2=1131/0-8-8, 7=173/Mechanical Max Horz 2=503(load case 5)

Max Uplift 8=-1450(load case 5), 2=-933(load case 5), 7=-154(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-13=0/50, 2-13=0/50, 2-3=-1719/1295, 3-4=-2560/2162, 4-5=-1963/1581, 5-14=-1264/988,

14-15=-1162/953, 6-15=-1046/881, 6-16=-168/53, 7-16=-54/51, 7-8=0/0

2-12=-1461/1448, 11-12=-1587/1586, 10-11=-2238/2294, 10-17=-1622/1696, 17-18=-1622/1696, 9-18=-1622/1696, 9-19=-991/1051, 8-19=-991/1051

3-12=-765/751, 3-11=-993/1040, 4-11=-349/305, 4-10=-670/690, 5-10=-506/472,

5-9=-917/897, 6-9=-1218/1255, 6-8=-1731/1632

Julius Lee Truss Design Engineer Flonds PE No. 34869 1400 Caastal Bay Blvd Goynton Beach, FL 33435

# JOINT STRESS INDEX

**BOT CHORD** 

**WEBS** 

2 = 0.67, 2 = 0.00, 3 = 0.60, 4 = 0.40, 5 = 0.53, 6 = 0.72, 7 = 0.42, 8 = 0.30, 9 = 0.82, 10 = 0.35, 11 = 0.73 and 12 = 0.42



Job	Truss	Truss Type	Qty	Ply	00	
L263148	HJ18A	SPECIAL	. 1	1		J1919398
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:30:32 2007 Page 2

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1450 lb uplift at joint 8, 933 lb uplift at joint 2 and 154 lb uplift at joint 7.
- 5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-13=-54, 7-14=-54, 8-17=-10

Concentrated Loads (lb)

Vert: 15=-206(F=-103, B=-103) 16=-282(F=-141, B=-141) 18=-332(F=-166, B=-166) 19=-390(F=-195, B=-195)

Trapezoidal Loads (plf)

Vert: 13=-0(F=27, B=27)-to-14=-153(F=-49, B=-49), 2=-1(F=5, B=5)-to-12=-10(F=-0, B=-0), 12=-10(F=-0, B=-0)-to-11=-15(F=-2, B=-2), 11=-15(F=-2, B=-2)-to-17=-28(F=-9, B=-9)

Julius Lee Truse Design Engineer Flonda FE No. 24868 1169 Coastal Bay Blvd Bovnton Besch, FL 33435



Job	Truss	Truss Type		Qty	Ply	0.0				
L263148	PB1	GABLE		3	1	lah F		- /N	n.	J1919399
Builders FirstSe	ource, Lake City, FI 320	055	6.300	s Feb 15 2006	MiTek Ir	JOD F	s Inc.	e (optiona Tue Dec 1	8 12:16:09 2007	Page 1
			0.00	701 05 10 2000		iodotiio	, 1110.	100 000 1	0 12.10.00 2001	r age r
	<u> </u>	10-0-0					20-0-0			
		10-0-0		4x6 =		1	10-0-0			Scale = 1:40.3
				4,00 —						
C.S. &	12	0 12	4	5	6		N N N N N N N N N N N N N N N N N N N	7	8g 01:1-0	
	4-0-0		13	12 11	16-0-0		10	20	D-0-0	
	4-0-0		3-0-0		6-0-0			4	-0-0	
LOADING (ps: TCLL 20.0		2-0-0 1.25	CSI TC 0.21	Vert(LL)	in -0.03 10	(loc)	l/defl >999	L/d 360	PLATES MT20	<b>GRIP</b> 244/190
TCDL 7.			BC 0.26		-0.03 10		>999	240	IVITZU	244/190
BCLL 10.0		r YES	WB 0.15	Horz(TL)	0.00	9	n/a	n/a		
BCDL 5.	Code FBC2004	4/TPI2002	(Matrix)						Weight: 92 lb	)
	2 X 4 SYP No.2 2 X 4 SYP No.2 2 X 4 SYP No.3 2 X 4 SYP No.3			BRACING TOP CHOR BOT CHOR	6 D R	-0-0 oc	purlins		g directly applie	
FORCES (lb)	- Maximum Compress	d case 4) case 4), 12=-29 ad case 7) case 5), 9=74(lo 0=366(load case)	9(load case 6), pad case 1), 12 se 11) Tension	14=-175(load 2=422(load cas	case 6), e 1), 14	=366(ld				
TOP CHORD	1-2=-199/192, 2-3= 7-8=-74/85, 8-9=-37		70/118, 4-5=-1	3/164, 5-6=0/1	64, 6-7=	-64/91	1			

### JOINT STRESS INDEX

**BOT CHORD** 

2 = 0.24, 3 = 0.33, 4 = 0.33, 5 = 0.39, 6 = 0.33, 7 = 0.33, 8 = 0.24, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

2-14=-21/110, 13-14=-21/110, 12-13=-21/110, 11-12=-21/110, 10-11=-21/110,

5-12=-219/1, 3-14=-261/237, 4-13=-92/112, 6-11=-92/112, 7-10=-261/237

### NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

8-10=-21/110

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1	GABLE	3	1		J1919399
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:09 2007 Page 2

### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 1, 29 lb uplift at joint 12, 175 lb uplift at joint 14 and 170 lb uplift at joint 10.
- 10) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Fruse Design Engineer Flonds PE No. 34889 I 109 Coasial Bay Blyd Swynton Desch E. Maass



Job	Truss	Truss Type	Qty	Ply	0.0	50000000000000000000000000000000000000
L263148	PB1A	GABLE	2	1		J1919400
					Job Reference (optional)	

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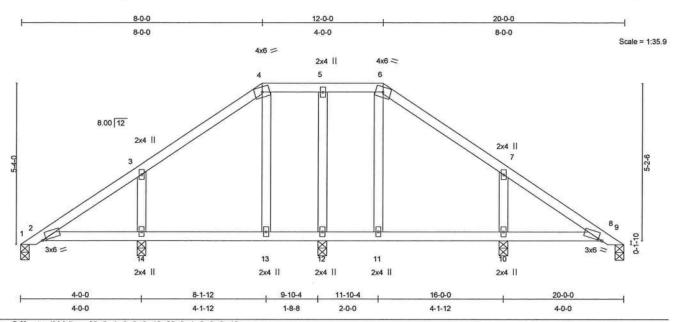


Plate Of	fsets (X,Y	<u>(): [2:0-1-8,0-0-1], [8:</u>	0-1-8,0-0-	-1]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.02	10-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.03	10-11	>999	240	1 11007 =3	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	9	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	200 0000 FOR \$22.00 M.					Weight: 89 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=148/0-3-8, 9=148/0-3-8, 14=342/0-3-8, 10=342/0-3-8, 12=283/0-3-8

Max Horz 1=-146(load case 4)

Max Uplift 1=-41(load case 4), 9=-7(load case 6), 14=-154(load case 6), 10=-145(load

case 7), 12=-72(load case 5)

Max Grav 1=148(load case 1), 9=148(load case 1), 14=354(load case 10), 10=354(load case 11), 12=283(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-2=-151/150, 2-3=-152/103, 3-4=-202/146, 4-5=-108/168, 5-6=-108/168,

6-7=-202/146, 7-8=-152/45, 8-9=-76/24

**BOT CHORD** 2-14=-31/109, 13-14=-31/109, 12-13=-30/108, 11-12=-30/108, 10-11=-30/109,

8-10=-30/109

4-13=-57/57, 6-11=-57/49, 3-14=-255/224, 7-10=-255/224, 5-12=-147/62 **WEBS** 

### JOINT STRESS INDEX

2 = 0.44, 3 = 0.33, 4 = 0.28, 5 = 0.33, 6 = 0.28, 7 = 0.33, 8 = 0.44, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1A	GABLE	2	1		J1919400
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:10 2007 Page 2

### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 1, 7 lb uplift at joint 9, 154 lb uplift at joint 14, 145 lb uplift at joint 10 and 72 lb uplift at joint 12.
- 10) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

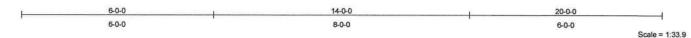
LOAD CASE(S) Standard

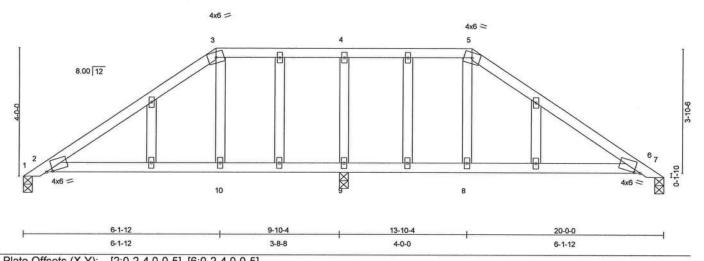
Julius Lee Truse Design Engineer Flonda PE No. 24869 1109 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	12-12-12-12-12-12-12-12-12-12-12-12-12-1
L263148	PB1B	GABLE	2	1		J1919401
LEGGTIG	1.010	O' IDEE	-		Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.22	2-10	>547	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.31	2-10	>387	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.10	7	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	2002	(Mat	rix)	12.00.000.000					Weight: 91 lb	

LUMBERTOP CHORD<br/>BOT CHORD2 X 4 SYP No.2TOP CHORD<br/>2 X 4 SYP No.2Structural wood sheathing directly applied or<br/>6-0-0 oc purlins.WEBS<br/>OTHERS2 X 4 SYP No.3BOT CHORDRigid ceiling directly applied or 10-0-0 oc<br/>bracing.

REACTIONS (lb/size) 1=488/0-3-8, 9=288/0-3-8, 7=488/0-3-8

Max Horz 1=109(load case 5)

Max Uplift 1=-105(load case 6), 9=-114(load case 5), 7=-107(load case 7) Max Grav 1=488(load case 1), 9=296(load case 10), 7=488(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-254/129, 2-3=-684/341, 3-4=-534/357, 4-5=-534/357, 5-6=-684/341,

6-7=-254/129

BOT CHORD 2-10=-187/530, 9-10=-186/533, 8-9=-186/533, 6-8=-187/530

WEBS 3-10=0/160, 5-8=-1/160, 4-9=-233/131

### JOINT STRESS INDEX

2 = 0.91, 3 = 0.47, 4 = 0.33, 5 = 0.47, 6 = 0.91, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33 and 18 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This Coffession designates for C-C for members and forces, and for MWFRS for reactions specified.

Julius Lee Truse Design Engineer Florida PE, No., 34868 1109 Geastal Bay Blvd Boynton Besch, FL 33435

December 20,2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1B	GABLE	2	1		J1919401
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:11 2007 Page 2

### **NOTES**

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 1, 114 lb uplift at joint 9 and 107 lb uplift at joint 7.
- 11) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

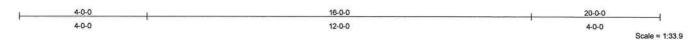
LOAD CASE(S) Standard

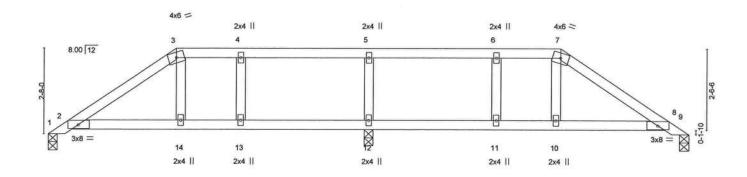
Julius Lee Truse Design Engineer Florida PE No. 34869 1100 Caestal Bay Blvd Coverage Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1C	GABLE	2			J1919402
L203146	PBIC	GABLE	2	1	Job Reference (optional)	

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-		-12 -12	10- 5-1	0-0 0-4		1		10-4		-1	20-0-0 4-1-12	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.11	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.14	13-14	>821	240	2575 June 15	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.06	9	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 74 lb	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=418/0-3-8, 9=418/0-3-8, 12=428/0-3-8

Max Horz 1=72(load case 5)

Max Uplift 1=-92(load case 5), 9=-93(load case 4), 12=-158(load case 5) Max Grav 1=418(load case 1), 9=418(load case 1), 12=430(load case 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-217/110, 2-3=-686/335, 3-4=-566/333, 4-5=-565/333, 5-6=-565/333,

6-7=-566/333, 7-8=-686/335, 8-9=-217/110

**BOT CHORD** 2-14=-217/556, 13-14=-220/565, 12-13=-220/565, 11-12=-220/565, 10-11=-220/565

, 8-10=-217/556

WEBS 3-14=-46/158, 7-10=-48/158, 5-12=-309/205, 4-13=-80/67, 6-11=-80/67

### JOINT STRESS INDEX

2 = 0.67, 3 = 0.31, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.31, 8 = 0.67, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp Figure 2: B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

December 20,2007

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1C	GABLE	2	1		J1919402
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:12 2007 Page 2

### NOTES

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 1, 93 lb uplift at joint 9 and 158 lb uplift at joint 12.
- 10) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

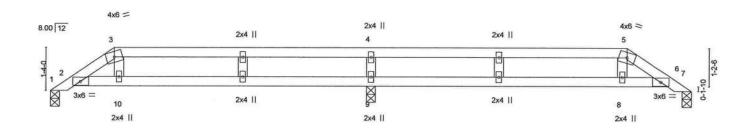
dulius Lee Truse Design Engineer Planda PE No. 24569 1 109 Cassiel Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	Set of Respond Considerations (
L263148	PB1D	GABLE	2	1		J1919403
					Job Reference (optional)	

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<u> </u>	2-1-12 2-1-12				1		17-10 8-0-		20-0-0			
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.06	9-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.12	9-10	>951	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 63 lb	
LUMBE		V 4 CVD N= 2				BRACING			<i>2</i> 1		- r - a - r	8

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2		6-0-0 oc purlins.
WEBS	2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
OTHERS	2 X 4 SYP No.3		bracing.

REACTIONS (lb/size) 1=341/0-3-8, 7=341/0-3-8, 9=582/0-3-8

Max Horz 1=35(load case 5)

Max Uplift 1=-99(load case 5), 7=-100(load case 4), 9=-212(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-177/92, 2-3=-800/377, 3-4=-766/400, 4-5=-766/400, 5-6=-800/377,

6-7=-177/92

BOT CHORD 2-10=-358/757, 9-10=-346/765, 8-9=-346/765, 6-8=-358/757

WEBS 3-10=0/241, 5-8=0/241, 4-9=-472/334

### JOINT STRESS INDEX

2 = 0.55, 3 = 0.62, 4 = 0.33, 5 = 0.62, 6 = 0.55, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.33 and 14 = 0.33

### NOTES

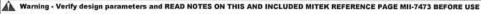
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"

 Provide adequate drainage to prevent water ponding. Continued on page 2 Julius Lass Truss Cesign Engineer Floride PE No. 34868 1 100 Crestel Bey Blyd Boynton Besch. FL 33431

December 20,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L263148	PB1D	GABLE	2	1		J1919403
	130, -100, 131, 100, 11				Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:13 2007 Page 2

### **NOTES**

- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 1, 100 lb uplift at joint 7 and 212 lb uplift at joint 9.
- 10) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 24868 1 100 Coestel Bay Blvd Occupanto Design



Job Truss Ply 0.0 Truss Type Qty J1919404 L263148 T01 SPECIAL Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:35:49 2007 Page 1 -2-0-0, 2-11-8 4-3-8 8-7-12 13-0-0 15-9-8 22-10-15 29-10-9 37-0-0 43-5-6 50-0-0 52-0-0 2-0-0 2-11-8 1-4-0 2-9-8 4-4-4 7-1-7 6-11-11 7-1-7 6-5-6 6-6-10 2-0-0 Scale = 1:90.0 5x6 = 5x8 = 2x4 II 8x10 = 8 9 7 4x6 / 8.00 12 4x6 < 4x6 / 21 24 20 5x6 = 5x8 23 25 18 17 26 16 15 14 4x6 =19 4x10 II 10x14.00 12 5x14 II 10x14 = 5x8 = 2x4 II 6x8 = 8x10 = 3x6 || 8x10 = 8x10 > 2-11-8 4-3-8 15-9-8 8-7-12 22-10-15 29-10-9 37-0-0 14-5-8 43-5-6 50-0-0 2-11-8 1-4-0 4-4-4 5-9-12 1-4-0 7-1-7 6-11-11 7-1-7 6-5-6 6-6-10 Plate Offsets (X,Y): [2:0-2-8,0-0-14], [2:0-0-8,0-9-6], [9:0-5-0,0-4-8], [12:0-9-4,0-1-0] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) I/defl L/d **PLATES** GRIP

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 6 SYP No.1D

**BOT CHORD** 2 X 8 SYP No.1D \*Except\*

2-23 2 X 6 SYP No.1D, 16-19 2 X 6 SYP No.1D

1.25

1.25

NO

TC

BC

WB

(Matrix)

0.20

0.73

0.84

Code FBC2004/TPI2002

Plates Increase

Lumber Increase

Rep Stress Incr

**WEBS** 2 X 4 SYP No.3

20.0

7.0

10.0

5.0

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

Vert(LL)

Vert(TL)

Horz(TL)

TOP CHORD Structural wood sheathing directly applied or 5-7-12

360

240

n/a

MT20

Weight: 901 lb

244/190

12

>999

>999

n/a

**BOT CHORD** 

0.23 20-21

-0.40 20-21

0.21

Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=4315/0-5-8, 12=4294/0-5-8

Max Horz 2=-335(load case 3)

Max Uplift 2=-1763(load case 4), 12=-1751(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/59, 2-3=-6974/2934, 3-4=-9806/4204, 4-5=-8532/3740, 5-6=-7077/3155,

6-7=-6503/2955, 7-8=-6244/2723, 8-9=-6244/2723, 9-10=-5649/2587, 10-11=-6812/3022,

11-12=-6988/2953, 12-13=0/62

**BOT CHORD** 2-23=-2400/5455, 22-23=-2627/6002, 21-22=-3539/8102, 21-24=-3115/7069,

> 20-24=-3115/7069, 19-20=-3125/7040, 19-25=-2712/6067, 18-25=-2713/6063, 17-18=-2650/6203, 17-26=-2650/6198, 16-26=-2650/6198, 15-16=-2649/6212,

14-15=-2332/5665, 12-14=-2332/5665

**WEBS** 3-23=-2863/1250, 3-22=-1456/3388, 4-22=-386/1002, 4-21=-1142/469, 5-21=-709/1569,

5-20=-1570/732, 6-20=-1582/3470, 7-20=-1368/2595, 7-19=-2916/1438, 7-18=-188/384,

8-18=-424/225, 9-18=-100/144, 9-17=-60/326, 9-15=-992/423, 10-15=-1424/3135,

11-15=-236/148, 11-14=-87/135

### JOINT STRESS INDEX

2 = 0.90, 2 = 0.50, 3 = 0.61, 4 = 0.32, 5 = 0.44, 6 = 0.92, 7 = 0.75, 8 = 0.34, 9 = 0.26, 10 = 0.79, 11 = 0.32, 12 = 0.69, 12 = 0.25; enaber 20,2007 Continued by = 0.29 = 0.63, =

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, et and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	40-20-70-00-00
L263148	T01	SPECIAL	1			J1919404
				2	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:35:49 2007 Page 2

### NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 8 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

6) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1763 lb uplift at joint 2 and 1751 lb uplift at joint 12.

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-6=-54, 6-10=-54, 10-13=-54, 2-23=-10, 22-23=-10, 22-24=-10, 20-24=-179(F=-169), 19-20=-179(F=-169),

19-25=-179(F=-169), 25-26=-10, 15-26=-179(F=-169), 12-15=-10

Concentrated Loads (lb)

Vert: 6=-173(F) 10=-173(F) 15=-1595(F) 24=-1595(F)

Julius Lee Fuse Design Engineer Florida PE No. 24869 I 109 Coastel Bay Blvd. 20vnton Beach, FL 23435



				0.0	Ply	Qty			/ре	Truss T	ss	Т	Job
J1919405						1			ı	SPECIA	,	Т	_263148
	)	e (optional	Referenc	Job					-	OI LOW	-		
007 Page 1	8 12:16:15 20				MiTek	15 2006	0 s Feb	6.30			e City, FI 32055	tSource, L	Builders I
52-0-0	50-0-0		42-3-9		-0	35-0		00	25-0-	0	9-7-12 15-0-0	11-8 4-3-8	-2-0-0
2-0-0 Scale = 1:93.	7-8-7		7-3-9	,	-0	10-0	140	0	10-0-		5-4-4 5-4-4	11-8 1-4-0	2-0-0
										8x10 =			
				5x8 = 8			5x8 =			6	8.00 12		
1				The same of the sa						A	8x10 //		
		8x10 🔷									. //		
		9	/							11/1	5	2x4	
		A				111						4x6 //	
												3	
10 4,0													4 2
1112/15		- W		AR.	IEI		4			17		_ 18	1 2 2 P
5x6 = \(\)	.5	12		13	14		15			16		19	4
		2x4		3x8 =	3x6 =		2x4			6x8 > 8x10 >	12	9.0 8x10	
-1	50-0-0 7-8-7		42-3-9 7-3-9	1	-	35-0 10-0	+		25-0 9-2-	15-9-8 1-4-0	14-5-8 10-2-0	11-8 4-3-8 11-8 1-4-0	1
	7-8-7			:0-5-0,			8], [10:0				:0-0-7,0-0-2], [5:		late Off
GRIP	PLATES	L/d	l/defl	(loc)			DE		CSI	2-0-0	ACING		OADING
244/1	MT20	360	>999		-0.26	(LL)	100-07-07	0.23	TC	1.25	ates Increase		CLL
A 130 15		240	>999		-0.56	(TL)	0.0740	0.69	BC	1.25	mber Increase		CDL
				10	0.25	(TĹ)		0.80	WB	YES	p Stress Incr		CLL
		n/a	n/a	10	0.23		1	0.00			de FBC2004/TI		CDL

LUMBER TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 4 SYP No.2

VEBS 2 X 4 SYP No.3 \*Except\*

7-16 2 X 4 SYP No.2, 7-13 2 X 4 SYP No.2

### BRACING

TOP CHORD Structural wood sheathing directly applied or

4-4-12 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-1-13 oc

bracing.

WEBS T-Brace:

2 X 4 SYP No.3 -6-16, 7-16, 7-13, 9-13

Fasten T and I braces to narrow edge of web

with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1706/0-5-8, 10=1706/0-5-8

Max Horz 2=-281(load case 4)

Max Uplift 2=-378(load case 6), 10=-378(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/56, 2-3=-2385/1079, 3-4=-3406/1488, 4-5=-3438/1628, 5-6=-2247/1189,

6-7=-1689/1030, 7-8=-1650/1019, 8-9=-2072/1105, 9-10=-2465/1145, 10-11=0/56

BOT CHORD 2-19=-678/1803, 18-19=-760/2098, 17-18=-777/2139, 16-17=-624/2125,

15-16=-657/1967, 14-15=-656/1966, 13-14=-656/1966, 12-13=-726/1921,

10-12=-726/1921

WEBS 3-19=-1188/414, 3-18=-494/1311, 4-18=-167/210, 5-17=-483/344, 6-17=-559/1609,

6-16=-830/317, 7-16=-537/299, 7-15=0/274, 7-13=-590/309, 8-13=-281/630,

9-13=-351/287, 9-12=0/214, 5-18=-425/1011

Julius Las Truse Design Engineer Flonda PE No. 24865 1109 Coestal Bay Blyd Boynton Beach, FL 2343:

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L263148	T02	SPECIAL	1	1		J1919405
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:15 2007 Page 2

### JOINT STRESS INDEX

2 = 0.79, 3 = 0.54, 4 = 0.33, 5 = 0.26, 6 = 0.38, 7 = 0.27, 8 = 0.63, 9 = 0.28, 10 = 0.74, 12 = 0.33, 13 = 0.56, 14 = 0.70, 15 = 0.33, 16 = 0.72, 17 = 0.67, 18 = 0.61 and 19 = 0.56

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 378 lb uplift at joint 2 and 378 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truse Cesian Engineer Florida PE No. 34869 I 109 Gresial Bay Blvd Sovinton Besch, FL 33436



Job Truss Truss Type Qty Ply 00 J1919406 L263148 T03 HIP 8 1 Job Reference (optional) Builders FirstSource, Lake City, FI 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 19 16:26:00 2007 Page 1 -2-0-0 7-6-10 15-0-0 25-0-0 35-0-0 42-5-6 50-0-0 52-0-0 2-0-0 7-6-10 7-5-6 10-0-0 10-0-0 7-5-6 7-6-10 2-0-0 Scale = 1:88.5 5x8 = 5x8 = 5x8 = 8.00 12 5 6 8x10 / 8x10 > 5x6 = 5x6 = 16 15 14 13 12 11 10 2x4 II 5x8 = 5x6 = 2x4 II 5x6 = 5x8 =2x4 || 15-0-0 7-6-10 25-0-0 35-0-0 42-5-6 50-0-0 7-6-10 7-5-6 10-0-0 10-0-0 7-5-6 7-6-10 Plate Offsets (X,Y): [2:0-2-12,0-0-14], [3:0-5-0,0-4-8], [7:0-5-0,0-4-8], [8:0-2-12,0-0-14] LOADING (psf) SPACING 2-0-0 CSI DEFL in I/defl L/d **PLATES GRIP** (loc) TCLL 20.0 Plates Increase 1.25 TC 0.30 0.25 11-13 Vert(LL) >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.66 Vert(TL) -0.45 11-13 >999 240 **BCLL** 10.0 Rep Stress Incr NO WB 0.59 Horz(TL) 0.14 8 n/a n/a **BCDL** Code FBC2004/TPI2002 (Matrix) Weight: 384 lb LUMBER BRACING Structural wood sheathing directly applied or 4-4-10 TOP CHORD 2 X 6 SYP No.1D TOP CHORD **BOT CHORD** 2 X 6 SYP No.1D oc purlins, except **WEBS** 2 X 4 SYP No.3 \*Except\* 2-0-0 oc purlins (5-0-12 max.): 4-6. 5-15 2 X 4 SYP No.2, 5-11 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 7-3-15 oc bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 3-15, 4-15, 6-11, 7-11 2 X 6 SYP No.1D - 5-15, 5-11 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. REACTIONS (lb/size) 2=2297/0-5-8, 8=2297/0-5-8 Max Horz 2=-278(load case 4)

Max Uplift 2=-561(load case 5), 8=-561(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/59, 2-3=-3502/1673, 3-4=-3164/1659, 4-5=-2567/1487, 5-6=-2567/1487,

6-7=-3164/1659, 7-8=-3502/1673, 8-9=0/59

BOT CHORD 2-16=-1162/2781, 15-16=-1162/2781, 14-15=-1281/3194, 13-14=-1281/3194,

12-13=-1281/3194, 11-12=-1281/3194, 10-11=-1162/2781, 8-10=-1162/2781

WEBS 3-16=0/179, 3-15=-286/251, 4-15=-577/1207, 5-15=-1036/473, 5-13=-271/753,

5-11=-1036/472, 6-11=-577/1207, 7-11=-286/251, 7-10=0/179

Trues Design Engineer Florida PE No. 34869 1100 Cassial Bay Blvd Boynton Beach, FL 33436

### JOINT STRESS INDEX

2 = 0.94, 3 = 0.35, 4 = 0.72, 5 = 0.28, 6 = 0.72, 7 = 0.35, 8 = 0.94, 10 = 0.34, 11 = 0.36, 12 = 0.91, 13 = 0.55, 14 = 0.91,  $15 \Rightarrow 0.91$ ,  $15 \Rightarrow 0.91$ 

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. 
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the 
responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection 
and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 
6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	Manual Bridge (Manual Bridge)
L263148	T03	HIP	8	1		J1919406
					Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Dec 19 16:26:00 2007 Page 2

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 561 lb uplift at joint 2 and 561 lb uplift at joint 8.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

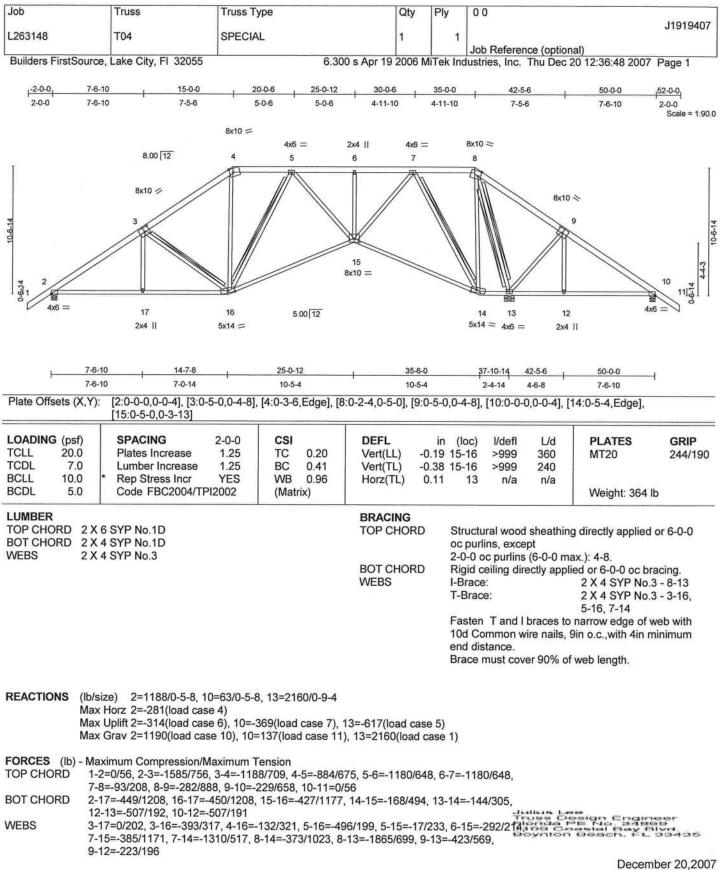
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

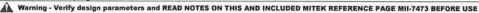
Vert: 1-4=-54, 4-6=-54, 6-9=-54, 2-15=-10, 11-15=-70(F=-60), 8-11=-10

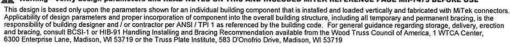
Julius Lee Truse Design Engineer Flonda FE No. 34866 1 109 Grestel Bay Blvd Bovnton Beach, FL 23435





Continued on page 2







Job	Truss	Truss Type	Qty	Ply	00	HIMACHINA SURVEY ON COURS
L263148	T04	SPECIAL	1	1		J1919407
Professional Parist	1,12,00048				Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:36:48 2007 Page 2

### JOINT STRESS INDEX

2 = 0.68, 3 = 0.25, 4 = 0.27, 5 = 0.34, 6 = 0.34, 7 = 0.46, 8 = 0.74, 9 = 0.36, 10 = 0.57, 12 = 0.34, 13 = 0.64, 14 = 0.58, 15 = 0.63, 16 = 0.35 and 17 = 0.34

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

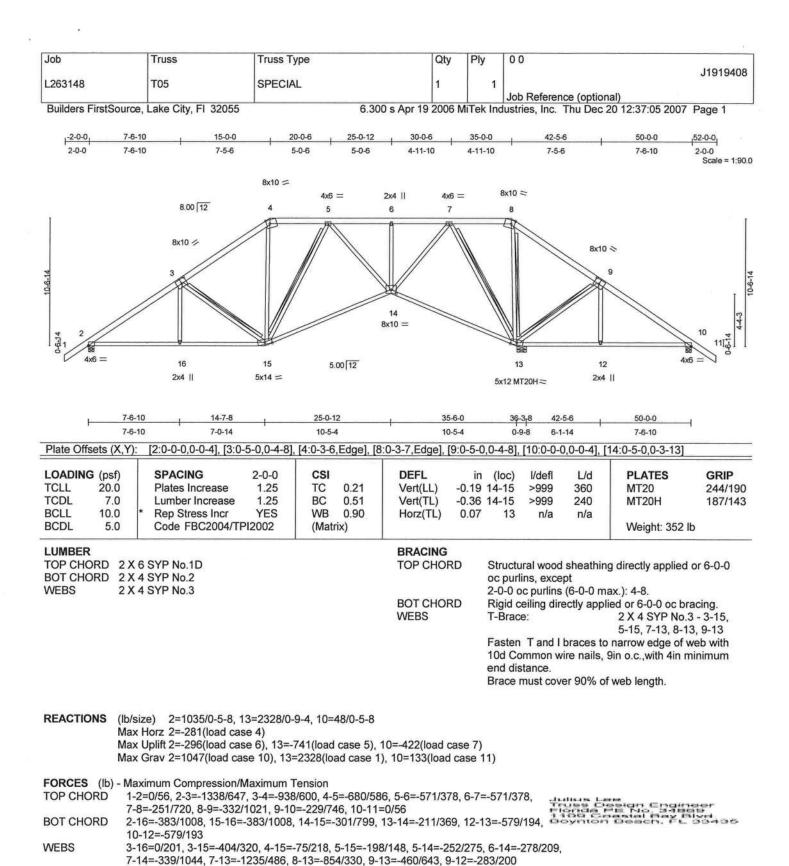
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2, 369 lb uplift at joint 10 and 617 lb uplift at joint 13.

LOAD CASE(S) Standard

Julius Lee Fruse Design Engineer Florida FIE No. 34988 I 109 Coestal Bay Blvd 20ynton Beach, FL 33435





### JOINT STRESS INDEX

2 = 0.68, 3 = 0.24, 4 = 0.25, 5 = 0.34, 6 = 0.34, 7 = 0.41, 8 = 0.39, 9 = 0.37, 10 = 0.58, 12 = 0.34, 13 = 0.89, 14 = 0.63, 15 = 0.31 and 16 = 0.34

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	Accessor 5 and 6 to 10 to
L263148	T05	SPECIAL	1	1		J1919408
				- OT:	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:37:05 2007 Page 2

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

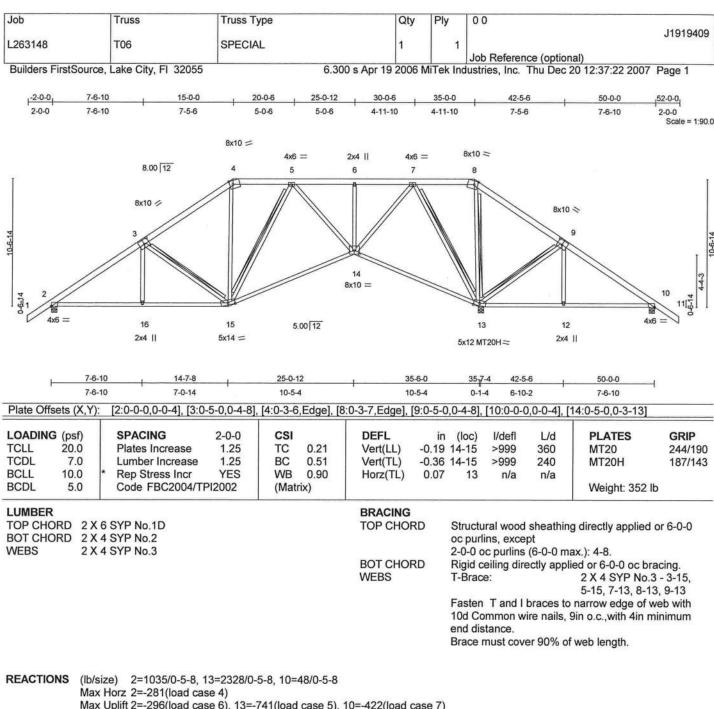
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 741 lb uplift at joint 13 and 422 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Honda PE No. 34888 I 100 Coastal Bay Blyd





Max Uplift 2=-296(load case 6), 13=-741(load case 5), 10=-422(load case 7)
Max Grav 2=1047(load case 10), 13=2328(load case 1), 10=133(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/56, 2-3=-1338/647, 3-4=-938/600, 4-5=-680/586, 5-6=-571/378, 6-7=-571/378,

7-8=-251/720, 8-9=-332/1021, 9-10=-229/746, 10-11=0/56

BOT CHORD 2-16=-383/1008, 15-16=-383/1008, 14-15=-301/799, 13-14=-211/369, 12-13=-579/194,

10-12=-579/193

3-16=0/201, 3-15=-404/320, 4-15=-75/218, 5-15=-198/148, 5-14=-252/275, 6-14=-278/209,

7-14=-339/1044, 7-13=-1235/486, 8-13=-854/330, 9-13=-460/643, 9-12=-283/200

### JOINT STRESS INDEX

2 = 0.68, 3 = 0.24, 4 = 0.25, 5 = 0.34, 6 = 0.34, 7 = 0.41, 8 = 0.39, 9 = 0.37, 10 = 0.58, 12 = 0.34, 13 = 0.89, 14 = 0.63, 15 = 0.31 and 16 = 0.34

Continued on page 2

**WEBS** 

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
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Engine

Job	Truss	Truss Type	Qty	Ply	00	44,000,000
L263148	T06	SPECIAL	1	1		J1919409
				1	Job Reference (optional)	

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:37:22 2007 Page 2

### **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

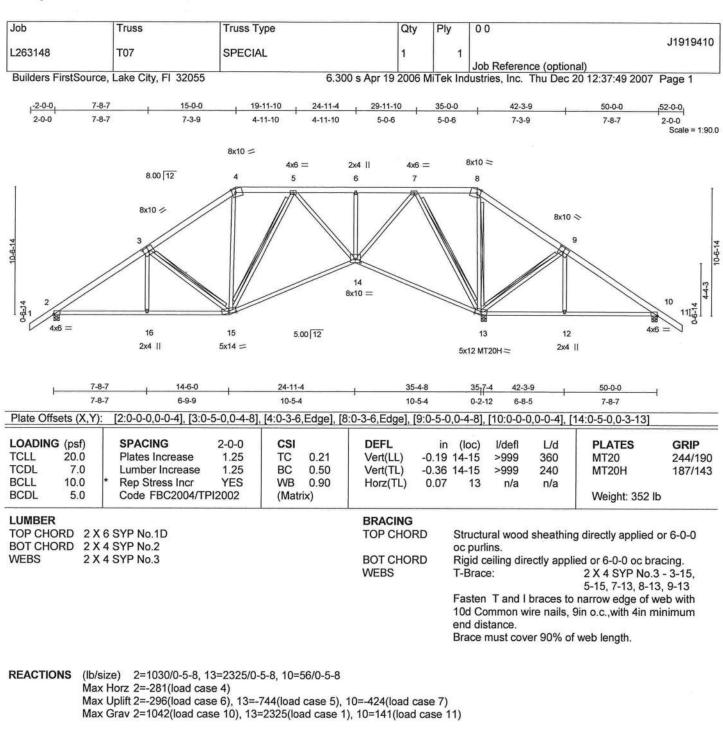
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 741 lb uplift at joint 13 and 422 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34869 Florida PE No. 34869 1400 Gewstell Bay Blvd Boynton Beach, FL 33436





FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/56, 2-3=-1326/641, 3-4=-932/601, 4-5=-674/583, 5-6=-566/374, 6-7=-566/374,

7-8=-251/724, 8-9=-329/1009, 9-10=-226/739, 10-11=0/56

BOT CHORD 2-16=-379/996, 15-16=-379/996, 14-15=-299/793, 13-14=-218/372, 12-13=-573/191,

0-12=-573/190

3-16=0/198, 3-15=-403/321, 4-15=-84/220, 5-15=-194/145, 5-14=-253/276, 6-14=-276/207, 7-14=-338/1042, 7-13=-1323/485, 8-13=-841/320, 0-13=-460/645, 0-13=-386/203

7-14=-338/1042, 7-13=-1232/485, 8-13=-841/320, 9-13=-460/645, 9-12=-286/202

### JOINT STRESS INDEX

**WEBS** 

2 = 0.71, 3 = 0.24, 4 = 0.24, 5 = 0.34, 6 = 0.34, 7 = 0.40, 8 = 0.38, 9 = 0.37, 10 = 0.61, 12 = 0.34, 13 = 0.89, 14 = 0.63, 15 = 0.31 and 16 = 0.34

December 20,2007

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	00	National #646400 #664000
L263148	T07	SPECIAL	1	1		J1919410
	100000				Job Reference (optional)	

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### **NOTES**

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are MT20 plates unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 744 lb uplift at joint 13 and 424 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee Truss Design Engineer Honda PE No. 34869 I 100 Coesial Bay Blvd



Job Truss Truss Type Qty Ply 00 J1919411 SPECIAL L263148 T08 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Dec 20 12:44:47 2007 Page 1 Builders FirstSource, Lake City, FI 32055 -2-0-0 6-9-15 13-0-0 14-6-0 19-8-10 24-11-4 30-1-14 35-4-8 37-0-0 43-2-1 50-0-0 52-0-0 2-0-0 6-9-15 6-2-1 1-6-0 1-7-8 5-2-10 5-2-10 5-2-10 5-2-10 6-2-1 6-9-15 2-0-0 Scale = 1:90.0 6x8 = 6x8 = 4x6 = 4x6 = 8x10 = 4x6 = 4x6 6 9 5 7 8 8.00 12 2x4 || 2x4 || 8x10 = 4x6 = 4x6 > 5x8 = 5x8 = 20 19 21 15 5.00 12 4x6 =8x10 = 8x10 = 4x6 = 6-9-15 19-8-10 30-1-14 35,7-4 43-2-1 24-11-4 50-0-0 6-9-15 5-2-10 5-2-10 5-2-10 0-2-12 7-6-13 6-9-15 Plate Offsets (X,Y): [7:0-5-0,0-4-8] LOADING (psf) 2-0-0 SPACING CSI DEFL I/defl GRIP in (loc) L/d **PLATES** TCLL 20.0 1.25 Plates Increase TC 0.15 Vert(LL) 0.10 19-20 >999 360 MT20 244/190 TCDL BC 7.0 Lumber Increase 1.25 0.31 Vert(TL) -0.14 19-20 >999 240 BCLL 10.0 Rep Stress Incr NO WB 0.43 0.05 Horz(TL) 15 n/a n/a BCDL 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 1329 lb LUMBER BRACING TOP CHORD 2 X 6 SYP No.1D TOP CHORD Structural wood sheathing directly applied or 6-0-0 **BOT CHORD** 2 X 6 SYP No.1D \*Except\* 2-19 2 X 8 SYP No.1D, 12-15 2 X 8 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. **WEBS** 2 X 4 SYP No.3 REACTIONS (lb/size) 2=3209/0-5-8, 15=9989/0-5-8, 12=-970/0-5-8 Max Horz 2=240(load case 4) Max Uplift 2=-1799(load case 4), 15=-6389(load case 4), 12=-1335(load case 9) Max Grav 2=3215(load case 9), 15=9989(load case 1), 12=794(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-5268/3119, 3-4=-5164/3208, 4-5=-4039/2698, 5-6=-3382/2205,

6-7=-1856/1173, 7-8=-1860/1175, 8-9=-748/1235, 9-10=-1681/2801, 10-11=-1442/2389,

11-12=-1438/2405, 12-13=0/62

2-20=-2703/4239, 20-21=-2495/3776, 19-21=-2490/3767, 18-19=-2864/4302, **BOT CHORD** 

17-18=-2389/3690, 16-17=-1330/905, 15-16=-3090/1873, 15-22=-2401/1472,

14-22=-2390/1467, 12-14=-1964/1202

**WEBS** 3-20=-194/161, 4-20=-418/802, 4-19=-1306/1684, 5-19=-780/975, 5-18=-1061/793,

Cesion 6-18=-677/953, 6-17=-2018/1352, 7-17=-256/137, 8-17=-2519/4014, 8-16=-2847/1763,

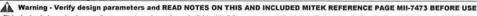
9-16=-1493/2523, 9-15=-2337/1401, 10-15=-2367/1348, 10-14=-799/1048, 11-14=-373/247

### JOINT STRESS INDEX

 $2 = 0.35, \ 3 = 0.34, \ 4 = 0.35, \ 5 = 0.28, \ 6 = 0.25, \ 7 = 0.11, \ 8 = 0.56, \ 9 = 0.41, \ 10 = 0.34, \ 11 = 0.34, \ 12 = 0.21, \ 14 = 0.28, \ 15 = 0.78, \ 16 = 0.28, \ 10 = 0.28,$ 0.46, 17 = 0.28, 18 = 0.29, 19 = 0.28 and 20 = 0.28

December 20,2007

Continued on page 2



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Job	Truss	Truss Type	Qty	Ply	00	
L263148	T08	SPECIAL	1	_		J1919411
5 5				3	Job Reference (optional)	

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### NOTES

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.

6) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1799 lb uplift at joint 2, 6389 lb uplift at joint 15 and 1335 lb uplift at joint 12.

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-10=-54, 10-13=-54, 2-21=-10, 19-21=-179(F=-169), 17-19=-10, 15-17=-10, 15-22=-179(F=-169), 12-22=-10

Concentrated Loads (lb)

Vert: 4=-173(F) 19=-2374(F) 15=-2374(F) 10=-173(F) 21=-1595(F) 22=-1595(F)

Julius Las Tues Design Engineer Florida PE No. 34888 1 100 Coastal Bay Blyd Boynton Besch, FL 35435



Job	Truss	Truss Typ	e	Qty	Ply	0.0		4		J1919412
L263148	Т09	SCISSOR	ł	6		1 Joh	Referen	ce (optiona	al)	
Builders FirstSo	ource, Lake City, FI 3205	5	6.30	0 s Feb 15 200	6 MiTek	Industri	es, Inc.	Tue Dec	18 12:16:23	2007 Page 1
	5-7-14		10-10-12	16-	1-10			21-9-8		
	5-7-14		5-2-14	4x6 =	2-14			5-7-14		Scale = 1:45.6
7.72.10	8.00 12	3x6 = 2 8 2x4    5.00    12		7 5x8 =		3x6 \( 4 \)				4-4-3
Simpson HTU26	3x6 = 5-7-14		10-10-12	. 16-	1-10			21-9-8	3x6 ≈	Simpson HTU2 or
	5-7-14	1	5-2-14	-1	-14			5-7-14		0-3-8 bearing
Plate Offsets (2	X,Y): [1:0-0-15,Edge],	[5:0-0-15,Ed	ge]							
LOADING (psi TCLL 20.0 TCDL 7.0 BCLL 10.0	Plates Increase Lumber Increase * Rep Stress Incr	YES	CSI TC 0.48 BC 0.45 WB 0.45	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.17 -0.33 0.35	(loc) 7 6-7 5	l/defl >999 >787 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 5.0	Code FBC2004/1	PI2002	(Matrix)						Weight:	98 lb
	2 X 4 SYP No.2 2 X 4 SYP No.2 2 X 4 SYP No.3			BRACING TOP CHOI BOT CHOI	RD RD	4-0-0 o	c purlins eiling di	3.	ng directly a	*1*00 (COC)

REACTIONS (lb/size) 1=683/Mechanical, 5=683/Simpson HTU26 or 0-3-8 bearing

Max Horz 1=-205(load case 4)

Max Uplift 1=-142(load case 6), 5=-142(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2225/892, 2-3=-1617/541, 3-4=-1617/541, 4-5=-2225/892

**BOT CHORD** 1-8=-695/1898, 7-8=-691/1898, 6-7=-691/1898, 5-6=-695/1898

WEBS 2-8=0/164, 2-7=-527/441, 3-7=-406/1417, 4-7=-527/441, 4-6=0/164

### **JOINT STRESS INDEX**

1 = 0.68, 2 = 0.41, 3 = 0.73, 4 = 0.41, 5 = 0.68, 6 = 0.33, 7 = 0.76 and 8 = 0.33

### NOTES

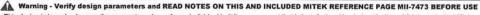
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

December 20,2007



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Job	Truss	Truss Type	Qty	Ply	00	
L263148	T09	SCISSOR	6	1	J19194 <sup>2</sup>	12
L203140	100	COISSON			Job Reference (optional)	

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### **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 1 and 142 lb uplift at joint 5.

LOAD CASE(S) Standard



Julius Les Truse Design Engineer Florida PE No. 34866 1 100 Cessial Bay Blvd Boynton Beach, FL 33425

December 20,2007



Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L263148	T09G	GABLE	1	1		J1919413
					Job Reference (optional)	
<b>Builders FirstS</b>	ource, Lake City, FI 3	2055 6.3	300 s Feb 15 2006 I	MiTek In	dustries, Inc. Tue Dec 18 12:16:24 2007	Page 1

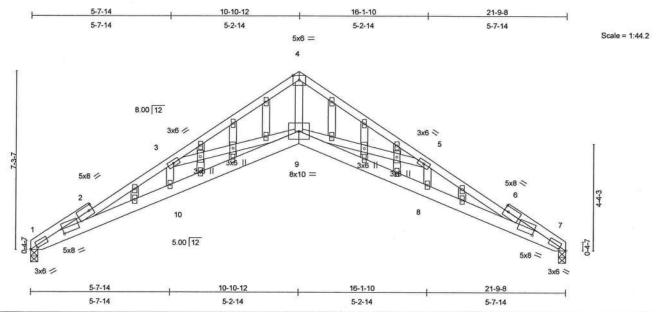


Plate Offsets (X,Y): [1:0-3-6,0-0-15], [1:1-6-2,0-0-10], [7:0-3-6,0-0-15], [7:1-6-2,0-0-10]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.68	8-9	>378	360	MT20	244/19
<b>TCDL</b>	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.56	8-9	>462	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.78	Horz(TL)	-0.65	7	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TI	212002	(Mat	rix)	,				in moreos	Weight: 134 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.1D \*Except\*

1-2 2 X 4 SYP No.2, 6-7 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

**WEBS** 2 X 4 SYP No.3

**OTHERS** 2 X 4 SYP No.3 BRACING

TOP CHORD Structural wood sheathing directly applied or

2-10-5 oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 3-9-0 oc

bracing.

REACTIONS (lb/size) 1=957/0-3-8, 7=957/0-3-8

Max Horz 1=-243(load case 4)

Max Uplift 1=-658(load case 6), 7=-658(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-3872/4735, 2-3=-3776/4737, 3-4=-2687/2872, 4-5=-2687/2872,

5-6=-3776/4737, 6-7=-3872/4734

**BOT CHORD** 1-10=-4152/3439, 9-10=-3988/3464, 8-9=-3988/3464, 7-8=-4153/3439

**WEBS** 3-10=-395/127, 3-9=-1076/1788, 4-9=-2955/2438, 5-9=-1076/1788, 5-8=-395/127

### JOINT STRESS INDEX

1 = 0.87, 1 = 0.89, 2 = 0.00, 2 = 0.87, 3 = 0.63, 4 = 0.81, 5 = 0.63, 6 = 0.00, 6 = 0.87, 7 = 0.87, 7 = 0.89, 8 = 0.33, 9 = 0.56, 10= 0.33, 11 = 0.33, 12 = 0.33, 13 = 0.42, 14 = 0.33, 15 = 0.33, 16 = 0.42, 17 = 0.33, 18 = 0.33, 19 = 0.33, 20 = 0.33, 21 = 0.33, 21 = 0.33, 22 = 0.33, 23 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.33, 222 = 0.33, 23 = 0.33, 24 = 0.33, 25 = 0.42, 26 = 0.33, 27 = 0.33, 28 = 0.42, 29 = 0.33 and 30 = 0.33

### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L263148	T09G	GABLE	1	1		J1919413
			1.7		Job Reference (optional)	

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### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 658 lb uplift at joint 1 and 658 lb uplift at joint 7.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

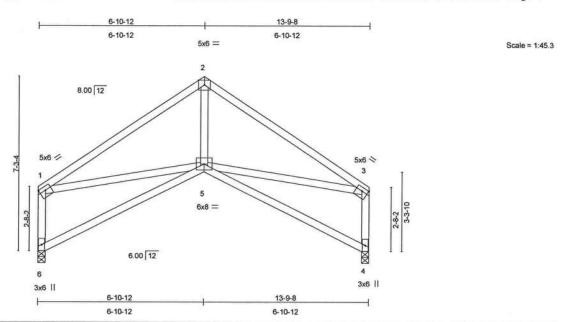
Vert: 1-4=-79(F=-25), 4-7=-79(F=-25), 1-9=-10, 7-9=-10

Julius Les Truse Design Engineer Florida PE No. 34868 1100 Coassa Bay Blyd Bountes Bases Bay Blyd



Job	Truss	Truss Type	Qty	Ply	00	
L263148	T10	SCISSOR	6	1		J1919414
		5.50.5.5.13	1		Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.04	4-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.07	4-5	>999	240	All the second s	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 80 lb	

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 6=432/0-3-8, 4=432/0-3-8 Max Horz 6=-125(load case 4)

Max Uplift 6=-81(load case 6), 4=-81(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-659/277, 2-3=-659/277, 1-6=-434/250, 3-4=-434/250

BOT CHORD

5-6=-192/187, 4-5=-74/120

**WEBS** 

2-5=-24/337, 1-5=-60/390, 3-5=-112/390

### JOINT STRESS INDEX

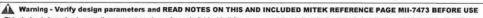
1 = 0.83, 2 = 0.67, 3 = 0.83, 4 = 0.26, 5 = 0.72 and 6 = 0.26

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truss Design Engineer Flonda PE No. 24868 1 100 Ceastal Bay Blvd Boynton Besch, Ft, 23425

December 20,2007



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	and the second control of
L263148	T10	SCISSOR	6	1		J1919414
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:25 2007 Page 2

### NOTES

- 5) Bearing at joint(s) 6, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 6 and 81 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Les Truss Cesian Engineer Florida PE No. 34868 1189 Ceastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00	
L263148	T10G	GABLE	1	1		J1919415
		140			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:26 2007 Page 1

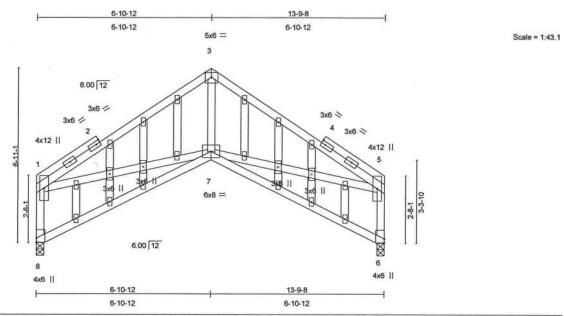


Plate Offsets	(X,Y	):	[20:0-0-0,0-0-0],	[21:0-0-0	,0-0-0]

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	0.18	6-7	>884	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.07	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.40	Horz(TL)	-0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)					10177477011	Weight: 113 lb	

11	III	ΛB	E	D
_	JIN			~

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WERS

2 X 4 SYP No.3 \*Except\*

1-8 2 X 4 SYP No.2, 5-6 2 X 4 SYP No.2

**OTHERS** 

2 X 4 SYP No.3

### BRACING

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 8=602/0-3-8, 6=602/0-3-8

Max Horz 8=151(load case 5)

Max Uplift 8=-404(load case 6), 6=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-989/995, 2-3=-865/1003, 3-4=-865/1003, 4-5=-989/995, 1-8=-636/620,

5-6=-636/620

BOT CHORD 7-8=-277/211, 6-7=-159/207

WEBS 3-7=-803/380, 1-7=-481/563, 5-7=-481/563

### JOINT STRESS INDEX

1 = 0.94, 2 = 0.00, 2 = 0.42, 2 = 0.43, 3 = 0.78, 4 = 0.00, 4 = 0.43, 4 = 0.42, 5 = 0.94, 6 = 0.69, 7 = 0.86, 8 = 0.69, 9 = 0.00, 10 = 0.00, 11 = 0.00, 12 = 0.00, 13 = 0.00, 14 = 0.00, 15 = 0.00, 16 = 0.00, 17 = 0.00, 18 = 0.00, 19 = 0.00, 20 = 0.00, 21 = 0.00, 22 = 0.00, 23 = 0.00, 24 = 0.00, 25 = 0.00, 26 = 0.00, 27 = 0.00 and 28 = 0.00

### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

Trues Design Engineer Florida PE No. 34865 1 100 Chastal Bay Blyd Boynton Besch, FL 95436



Job	Truss	Truss Type	Qty	Ply	00	
L263148	T10G	GABLE	1	1		J1919415
					Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Tue Dec 18 12:16:27 2007 Page 2

### **NOTES**

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads,
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Bearing at joint(s) 8, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 8 and 404 lb uplift at joint 6.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-79(F=-25), 3-5=-79(F=-25), 7-8=-10, 6-7=-10

dullus Les Truse Design Engineer Flonda FE No. 24969 1109 Coesial Rey Blvd Bovnton Besch, FL 23435

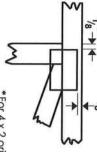


# Symbols

# PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless securely seat. plates to both sides of truss and Dimensions are in inches. Apply dimensions indicate otherwise



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates

## PLATE SIZE

4 × 4

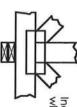
dimension is the length parallel perpendicular to slots. Second The first dimension is the width

# LATERAL BRACING



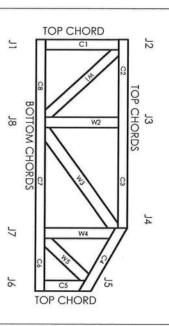
continuous lateral bracing. Indicates location of required

## BEARING



which bearings (supports) occur. Indicates location of joints at

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

# CONNECTOR PLATE CODE APPROVALS

ICBO 3907, 4922

BOCA

96-31, 96-67

9667, 9432A

SBCCI

WISC/DILHR

960022-W, 970036-N

561

易



MiTek Engineering Reference Sheet: MII-7473

# General Safety Notes

# Damage or Personal Injury Failure to Follow Could Cause Property

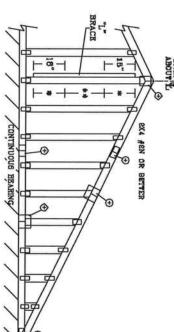
- 1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- is Cut members to bear tightly against each
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- 4 Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.
- lumber shall not exceed 19% at time of fabrication. Unless otherwise noted, moisture content of

S

- 6 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
- 7 Camber is a non-structural consideration and practice is to camber for dead load deflection. is the responsibility of truss fabricator. General
- 00 shown indicate minimum plating requirements Plate type, size and location dimensions
- 9 Lumber shall be of the species and size, and grade specified. in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling. erection and installation of trusses.
- 1993 MiTek® Holdings, Inc.

### ASCE 7-02: 130 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, Н 11 1.00, EXPOSURE C

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			1	υ. Τ	2	TII	I I	STI	なりず	CHANGE OF SOCIAL		1	<i>V.</i>	2	TIT	I I	OF I	CDT			1	V.	)	TTT.	I I	OFF	CIT	SPACING SPECIES	GABLE VERTICAL
	STANDARD	STUD	<b>‡</b> 3	#2	<b>*</b> 13	STANDARD	STUD	£3	\$1 / #2	STANDARD	STUD	<b>†</b> 3	#2	<b>\$</b> 3	STANDARD	STUD	#3	\$1 / #2	STANDARD	STUD	<b>4</b> 3	#2	<b>£</b> 1	STANDARD	STUD	<b>\$</b> 3	\$1 / #2	GRADE	BRACE
	4' 3"	4 4	4' 4"	4. 7	4 8	4' 2"	4' 2"	4' 2"	4. 3,	3, 10,	4 0"	4. 0.	4. 2.	4' 3"	3. 8,	3' 9,	3' 8"	3. 10.	3' 4"		3. 6.			3. 3.	3' 3'	3' 3	3' 4"	BRACES	Š
	6' 1"	7 1"	7' 2"	7. 4*	7' 4"	6' 11"	6' 11"	6' 11"	7' 4'	5' 3"	6 1"	6. 27	8' 8	8	5. 23	8, 0,	6 0	6. 8,	4' 3"		5 0	6' 10"	5' 10"	4' 2"	4' 11"	4' 11"	6' 10"	GROUP A	(1) 1X4 °
	6" 1"	7' 1"	7' 2"	7' 11"	7' 11"	6' 11°	6 11	6' 11"	7. 7.	5. 3.	6' 1"	6. 5.	7' 2"	7, 2	6. 5.			6, 10.		5, 0,	6.0.	6' 3"	6' 3"	4. 5.	4' 11"	4' 11"	6° 0°	GROUP B	"L" BRACE .
	8, 0,	8. 9.	8, 8,	B, 8 <sub>o</sub>	8, 8,	7' 10"	8' 9"	8, 9,	B. 8.	6" 11"	7' 11"	7' 11"	7' 11"	7' 11"	6, 10,	7' 11"	7' 11"	7' 11"	5' 8"	8, 2,	6. 8,	6' 11"	6' 11"	5. 6,	6' 5"	6 6	6' 11"	GROUP	(1) 2X4 "L" BRACE
KKS	8' 0"	9. 25.	9, 5,	9' 5"	8, 2,	7' 10"	8' 9"	B' 8"	8' 11"	6' 11"	B' 1*	8. 2.	8' 8"	8 6	6, 10,	7' 11"	7' 11"	8 1"	5. 8.	6' 7"	6. 8.	7' 5"	7' 5"	5. 6.	6, 9,	6' 6"	7' 1"	A GROUP B	L" BRACE .
3 MKKS		10' 6"	10' 5"	10' 6"			10' 5"	10' 6"	10. 6.	9' 4"	8, 2,	9. 6.	8, 9,	8, 5,	8. 5.	9' 6"	9' 5"	9. 6.	7. 8.	8 3	B' 3°	8' 3*	8 3	7' 5'	8' 3"	8' 3"	8. 3.	GROUP A	(2) 2X4 "L"
	10' 8"	10' 11"	10' 11"	11, 5,					10. 8.	9' 4"	9' 11"	8. 11	10' 2"	10 2	9, 5,		8, 2,	9 8"		8' 8"	8.8.	8' 11"	B' 11"	7. 5.	8, 3,	8' 3"	- 1	GROUP B	BRACE **
	. 1	13' 8"	13' B"	13. 8.			13. 8.		13' 8"	10' 10"	12' 5"	12. 6.	12' 6"	12' 5"		12' 4"	12' 4"	12. 6.		10' 3"	10. 4		10' 10"	8,	10' 0°		10' 10"	GROUP	
	12' 6"	14. 0.	14, 0,	14' 0"	14' 0"	12' 3"	13' 6"	13' 6"	14 0"	10' 10'	12, 6,	12' 8"	13' 5"	13' 5"	10. 2.	12' 4"	12' 4"	12' 9"	8 10	10' 3"	10' 4'	11' 8'	11' 8"	8.	10' 0"	10, 1,	11. 2.	A GROUP B	(1) 2X6 "L" BRACE .
	14' 0"	14. 0.	14' 0"	14' 0"	14. 0.	14' 0"	14' 0"	14 0	14. 0.		14 0	14. 0.	14. 0	14, 0,	14. 0.	14. 0.	14' 0"	14. 0.	12' 0"	12' 11"	12, 11,	12' 11"	12' 11"	11. B.	12' 11"	12' 11"	12. 11.	B GROUP A GROUP B	(2) ZXB "L" BRACE
	14' 0"	14 0	14' 0"		14 0	14' 0"	14' 0"	14 0	14. 0.	14. 0"	14.00	14. 0.	14' 0"	14' D"	14. 0.	14' D"	14' O"	14. 0.	12' 0"	13' 7"	13' 7'	13' 11"	13' 11"		12' 11"	12' 11"	13' 3"	CROUP B	BRACE **
CABLE END SUPPORTS LOAD FROM	CONTINUOUS BEARING (O FSF IC	COMMUNICIPATION /S DESCRIPTIONS FOR		LIVE LOAD DEPLECTION CONTROL IS	CABLE IRUSS DETAIL				12	_	SDUTHERN PINE DOUGLA		#1 & BIR	HEM-MIR	GROUP B:			CIAITUANU	TOTA	#3	DOUGLAS FIR-LARCH SOUT	2000	24	HAI-JAIL	GROUP A:		BRACING GROUP SPECIES		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WINN DIAGONAL
BRACE IS USSED. CONNECT
INACONAL BRACE FOR SAGE
AT EACH YED. MAY WEB
TOTAL LENGTH IS 14\*.

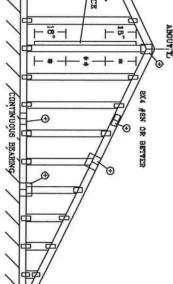
GABLE TRUSS

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

SPF #1/#Z, DF-L #Z,
SPF #1/#Z, DR BETTER
DIAGONAL BRACE;
SINGLE OR DOUBLE
CUT (AS SHOWN) AT
UPPER RND.

CONNECT DIAGONAL AT YES.

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH



CABLE TRUSS DETAIL NOTES:

DOUGLAS FIR-LARCH

AND GRADES:

HEM-PIR
AZ STUD

#3 STANDARD

SOUTHERN PINE

STANDARD

ABLE END SUPPORTS LOAD FROM 4' 0"
DUTLODWERS WITH E' 0" GVERHANG, DR 12"
PLYWOOD OVERHANG. IDE UPLATT CONNECTIONS FOR 136 PLF OVER NTINUOUS BEARING (6 PSF TC DEAD LOAD). LOAD DEPLECTION CRITERIA IS L/240.

ATTACH EACH 'L' BRACE WITH 10d NAILS.

# FOR (1) 'L' BRACE: SPACE NAILS AT 2" O.C.

# FOR (2) 'L' BRACE: SPACE NAILS AT 3" O.C.

## FOR (2) 'L' BRACES: SPACE NAILS AT 3" O.C.

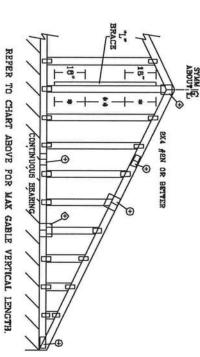
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

FOR	DESIGN PLATES.	PEAK, SPLICE, AND HEEL I
	2.5X4	GREATER THAN 11' 6"
	200	GREATER THAN 4 D', BUT LESS THAN 11 B'
ex3	1X4 DR	LESS THAN 4' 0"
ଥି	NO SPL	VERTICAL LENGTH
S	IE SIZES	GABLE VERTICAL PLAT

				THESE FUNCTIONS. UNITES SHEET SHALL HAVE A PROPERTY ATTROCE PRICE TO PERFORM STRUCTURAL PARETS ATTROCED SHALL HAVE PROPERTY ATTROCED SHALL HAVE PROPERTY ATTROCED SHALL HAVE A PROPERTY ATTROCED REGISTROCTURAL PARETS AND EDITION CHERG SHALL HAVE A PROPERTY ATTROCED REGISTROCTURAL PARETS AND EDITION CHERG SHALL HAVE A PROPERTY ATTROCED REGISTROCTURAL PARETS AND EDITION CHERG SHALL HAVE A PROPERTY ATTROCED REGISTROCTURAL PARETS AND EDITION CHERG.	_	***WARWING** TRUSSES REQUIRE EXTREM CARE IN FARRICATING, HAND ING. SHIPPING, INSTALLING AND
STATE OF FLORIDA	No: 34869			DELRAY BEACH, PL 33444-2161	CONS. ENGINEERS P.A.	ULIUS LEE'S
MAX. SPACING 24.0"		MAX. TOT. LD. 60 PSF				
			-ENG	DRWG MITEN SID GABLE 15 E HI	DATE 11/26/03	REF ASCEY-02-GAB13015

### ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, 11 1.00, EXPOSURE 0

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	STANDARD	STUD	<b>\$</b> 3	#2	<b>#</b> 3	STANDARD	STUD	<b>\$</b> 3	\$1 / #2	STANDARD	STUD	<b>#3</b>	#23	<b>£</b> 3	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	<b>£</b> 3	Z#	<b>#</b> 1	STANDARD	STUD	<b>#</b> 3	\$1 / #2	GRADE	BRACE
	4' 0"	4.	4. 2,	4' 4	4 5"	3' 11'	3' 11"	3' 11"	4. 0.	-	3' 8	3. 8,		4. 0.	3. 7.	3' 7"	3' 7"	3.8	3' 0,		3' 3'	3' 5"	3' 6"		3' 1"	3' 1"	3. 23.	BRACES	Š
	5' 6"	6 4	6,	6' 11"	5' 11°	5' 4"	6.3	6 3	6' 11"	4. 9.	5' 6"	5. 7.	8' 4"	B. 4.	4. 8.	5,	U.		3' 10"	-	4. 6.	5' 6"	5' 6"	3' 9"	4' 6"	4. 5	5. 6.	GROUP A	(1) 1X4 °
	5" 6"	6' 4"	6' 5"	7' 6"	7' 6"	5' 4"	6' 3"	6' 3"	7. 5.	4. 9.	5' 6"	6. 7.	8' 10"	6 10	4' B'	6.	5. 5.	6' 6"	3' 10"		4. 6.	5' 11"	5' 11"	3. 9.	4' 5"	- 0.4	6. B.	GROUP B	"L" BRACE
	7' 3"	8.3	8' 3"	B' 3°	8, 3,	7' 1"	B' 3"	8' 3"	8. 3.	6" 3"	7' 3"	7' 4'	7' 8"	7' B"	6. 5.	7. 22.	7' 2"	7' 6"	6, 1,,	5' 11"		6, 6,	6' 8"	6.0.	5' 10"	34.00	6. 6.	GROUP A	(1) 2X4
3	7' 3"	8. 6.	8' 6"	8' 11"	B' 11.	1.77	8° 3°	8' 3"	8' 6'	6. 3.	7' 3"		8' 1"			7' 2"	7' 2"		5. 1.	5' 11"	6. 0.	7' 0"		5. 0.			6. 8.	GROUP B	(1) 2X4 "L" BRACE
STATE OF THE STATE	8. 8.	9. 10.	9' 10"	9, 10,	B, 10 <sub>e</sub>	9' 6"			9' 10"		8, 11,"	8. 11	8' 11"		8. 3.	8' 11"	8' 11"	8. 11	8' 11"	7' 10"	7' 10"	7' 10"		6. 9.	7' 10"	7' 10"	7' 10"	GROUP A	• (2) 2X4 "
	8, 8,	10' 4"	10' 4"	10' 7"	10' 7"		9' 10"	8' 10"	10. 1.			8. 6.	9' 7"	8, 2,	6' 3'		8' 11"		6, 11,	8, 0,	00	œ	8	6. 9.	7' 10"	_	8.0.	GROUP	"L" BRACE
	11. 4.	12' 11"	12, 11,	12. 11.	12' 11"	11' 1"	12, 10	12' 11"	12' 11"		11. 4.	11. 5.	11, 9,	11, 8,	9. 7"		11' 2"	11. 9.	8. 0.	B, 3,		10′ 3°			9' 1"		10. 3	B GROUP	** (1) 2X6
	11' 4"	13' 1'	13' 3'	13' 11°	13' 11"		12' 10"	12, 11,	13' 4"	9, 9,	11, 4,	11' 6'	12' B"	12' B"	8. 4.	11. 1.	11' 2"	12' 1°	8.0.	8 3	9 4	11, 1,,		7. 10.	9′ 1"	9' 1"	10. 7.	A GROUP	"L" BRACE *
	14' 0"	14. 0.	14. 0,	14	14 0	14	14' 0"	14. 0.		13' 3"	14. 0,	14. 0.	14' 0"	14. 0.	12. 11.	14' 0"	14' 0"	$\dashv$	10,	12 3	12. 3,	12' 3"	+	+	12' 3"	12. 3,	12. 3.	B GROUP A	(2) ZXB "L" BRACE
	14' 0"	14. 0	14' 0"		14 0	14. 0*	14. 0.	14 0	14 0	13' 3"	14 0	14. 0	14' D"	14' 0"	12. 11.	14" D"	14' 0"	14' 0'			12' 8'		13' 2"	10. 4.	12′ 3°	12' 3"	12. 7.	GROUP B	L' BRACE .
CABUL END SUPPORTS LOAD FROM 4 0	CONTROLOG DEALING (O FOR IC DEAL LIAD).	CONTRIBUTE CONNECTIONS FOR 180 PLF OVER	The state of the s	LIVE LOAD DEPLECTION CONTROL IS 1 /240	CABLE INUSS DETAIL NOTES:				#2	ш	SOUTHERN PINE DOUGLAS FIR-LARCH		#1 & BIR	HEM-FIR	יייים אייים	Cacin		STATISAND	I T	***	DOUGLAS FIR-LARCH SOUTHERN PINE	Po STON STONE	STIM 43	IA-WIN	OUP A:		BRACING GROUP SPECIES AND GRADES:		



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
MIACONAL BRACE FOR 980f
AT EACH END. MAX WEB

GABLE TRUSS

TOTAL LENGTH IS 14".

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

ZX4 SP OR DF-L #2 OH BETTER DIAGONAL BRACE, SINGLE OR DOUBLE

AT UPPER END

MIDPOINT OF VERTICAL WEB.

ABLE END SUPPORTS LOAD FROM 4' 0" DUTLODKERS WITH 2' 0" DVERHANG, DR 12" PLYWOOD OVERHANG.

ATTACH EACH 'L' BRACE WITH 10d NAILS AF 8' O.C.

\$ FOR (1) 'L' BRACE: SPACE NAILS AF 8' O.C.

\$ FUR (2) 'L' BRACE: SFACE NAILS AT 3' O.C.

IN 18' END ZONES AND 6' O.C. BETWEEN ZONES. MEMBER LENGTH. T. BRACING MUST BE A MINIMUM OF 80% OF WEB

	2.5X4	2.5X		AN 11' 6"	MAHE	REATER
	27.		TUB	0 0	THAN 1	ATER ESS T
23	界	1X4	L	٠,	N 4.	S THA
Q	SPLI	NO		HLDN	CAL LA	VERT
33	SIZES	10	PLAT	TICAL	VERT	ABLE

MAYARONGEM TRASSIS REDURE EXTREME CARE IN FARRICATING, HANDLING, SADPING, INSTALLING AND BROCING. ROTER TO BEST 1-43 KNULLING COMPOSET SAFETY HATDANTIDA, PUBLISHED BY TPI CTRUSS PLATE INSTITUTE, 383 THORPORTO DR. SULTE 200, MINISON, H. SAFETY PARCITIES PRIBE TO PERFORMING THE AIRCRA, 6300 ENTERPRISC LW, MAUSON, UT SATIP) AND LYCEA, KAUDD TRUSS CLONGLING THE SAFETY PARCITIES PRIBE TO PERFORMING THESE FAULTH AND PERFORM THE APPROPRIES UNICESS OTHERWISE INDICATED, 1019 CHORD SHALL HAVE PROPERTY ATTACHED RIGID CELLING.

CONS. DELRAY I

No: 34869			BEACH, PL 33444-2151	ENGINEERS P.A.	S, HHI SI
	MAX.				
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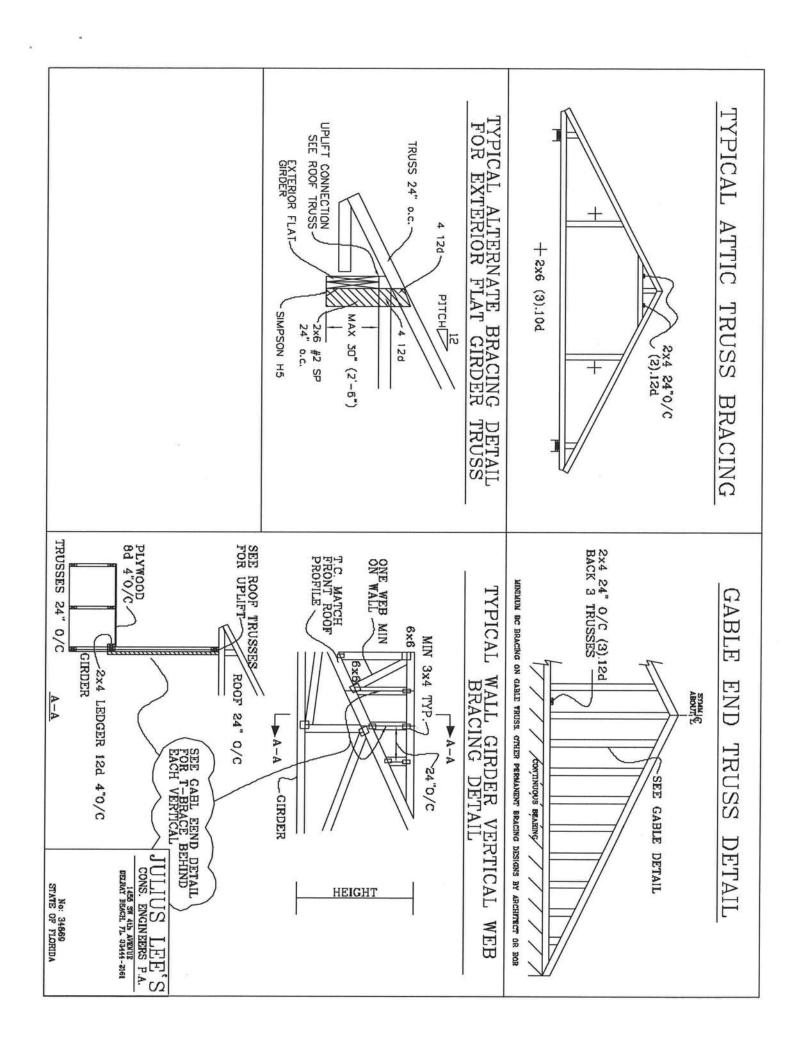
11/26/03

ASCE7-02-CAB13030

STATE OF FLORIDA

MAX. SPACING

24.0"



BOT CHORD 2X4 2X4 ಡೆದರು 公公司 BETTER BETTER BETTER

# PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTON CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS: 130 MPH WIND, 30' MEAN HCT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND EC DL=6 PSF

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL-5 PSF, WIND BC DL-5 PSF 110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 PSF, WIND BC DL-5 PSF

FRONT FACE (B,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

#2 OR BETTER

EITHBR PLATE LOCATION IS ACCEPTABLE

SPLICE

B

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D-SPLICE No.

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n C 20' FLAT TOP CHORD WAX SPAN

TOINT M D m > C 4X8 OR 3X6 TRULOX AT 4'
ROTATED VERTICALLY 584 5X3 4X8 2X4 30 2.5X4 1.5X4 SPANS 5X5 5X6 34 뒫 2.6X4 .5X4 5X5 **6X8** 88 5 1.5X4 **5**X6 3X6 52 C,

ATTACH TRULOX PLATES WITH (8) 0.120 X 1.375 NAILS, C EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION. 3°

2x4 T BRAC MEMBER OR MEMBER AT	7'9" TO 10' MEMBER OR MEMBER. AT	"TO 7'9" NO BRACING	DE CONCLU
CE. SAME GRADE, SPECIES AS WEB BETTER, AND 80% LENGTH OF WEB TACH WITH 16d NAILS AT 4° OC.	E. SAME GRADE, SPECIES AS WEB BETTER, AND 80% LENGTH OF WEB TACH WITH 8d NAILS AT 4" OC.		REGULARD BRACING

* PIG	
GYBACK	
PIGGYBACK SPECIAL PLATE	
PLATE	

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

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DRAWING	S. Andrews and Market Street
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834,017 &	
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			MING	CIRUSS CNG AND		
No: 34889 STATE OF FLORIDA			152	CONS. ENGINEERS P.A.	VILLI SILLIII	IMANU CITI
SPACING 24.0"	47 PSF AT 1.15 DUR. FAC.	1.25 DUR. FAC.	1.33 DUR. FAC.	55 PSF AT	MAX LOADING	NG KEPLACES DRAWINGS
		-ENG JL	DRWGMITEK STD PIGGY	DATE 09/12/07	REF PIGGYBACK	INID DRAMING REPLACES DRAWINGS 634,016 634,017 & 847,045

MANARHIGEM TRACSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLIN BACING REFER TO EXEL 1-03 GUILLING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (PARLING INFORMATION), PUBLISHED BY TPI (PARLING INFORMATION), PUBLISHED BY TPI (PARLING INFORMATION), PUBLISHED BY TRACHICA SATIO TRACE, AND ENTERPRISE LA MADISON, MI 337199 FOR SAFETY PRACTICES, PRIOR TO PERFORE THE TRACE AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BIBLIO CEILING.

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VC-TYP.

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\*ATTACH

PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

## VALLEY TRUSS DETAIL

TOP BOT CHORD CHORD WEBS 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. ZX4 SP #3 OR BETTER.

2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

\*\* ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (CASCE 7-02 130 MPH WIND. 15" MEAN HEIGHT, ENCL BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. OR (3) 16d ENCLOSED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

ENGINEERS' SEALED DESIGN.

\*\*\* NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

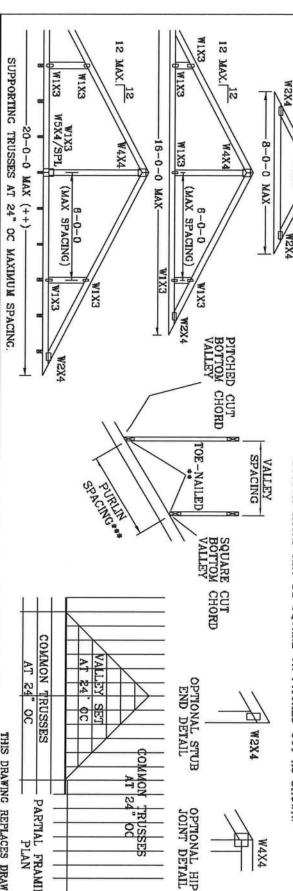
4-0-0 MAX

12 NAX.

W2X4

++ LARGER SPANS NAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



PARTIAL FRAMING

THIS DRAWING REPLACES DRAWING A105

MYWARHINGAM TRUSSES REGULTRE EXTREME EARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING REFOR TO BEST 1-02 GRUILLING COPPINENT SAFETY INFORMATION, PUBLISHED BY THE (F BACING REFOR TO BEST 1-02 GRUILLING COPPINENT SAFETY PROPERTY AND WICE ACCORDING THE STRUCTURE, SED CONDITION BY, SHITTE 201, MAISSIN, VJ. 53799 AND WICE ACCORDING AND FOR FAREICA, GOOD SHALL HAVE PROPERLY AITACHED THESE FUNCTIONS, UNLESS OTHERWISES, MORIGATED, TOP CHOOD SHALL HAVE PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY AITACHED STRUCTURAL PARELS AND STRUCTURAL PARE

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No: 34869 STATE OF FLORIDA				DELRAY BEACH, FL 35444-2161	ENGINE	JULIUS LEE'S		
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			JL	VALTRUSS1103	11/26/03	VALLEY DETAIL	THE PERSON NAME AND PARTY OF P	

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE. END DISTANCE. SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

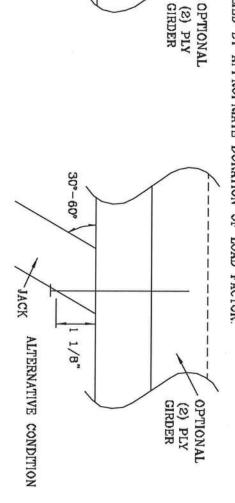
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

# MAXIMUN VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

5 493#	4 394#	3 296#	2 197#	TOE-NAILS 1 PLY	NUMBER OF SOUTHERN PINE	
639#	511#	383#	256#	2 PLIES		
452#	361#	271#	181#	1 PLY	DOUGLAS	
5 493# 639# 452# 585# 390# 507# 384	468#	351#	234#	2 PLIES	DOUGLAS FIR-LARCH	
390#	312#	234#	156#	1 PLY		
507#	406#	304#	203#	2 PLIES	HEM-FIR	
384#	307#	230#	154#	1 PLY	SPRUCE	
496#	397#	298#	199#	2 PLIES	SPRUCE PINE FIR	

LOAD

GIRDER (2) PLY



1/B"

JACK

THIS DRAWING REPLACES DRAWING 784040

			STRUCTURAL PANE	PLATE JUSTI Dr America, THESE TUNC	BRACING. RI			
	WARRING. RETER TO BEST 1-33 CHULLING COMPORNET SAFELY (MFCHANZLING, SHIPPING, INSTALLING AND SACING. RETER TO BEST 1-33 CHULLING COMPORNET SAFELY (MFCHANTING), PUBLISHED BY TOP CRUSS LATE INSTITUTE, 983 PROCERID SA, SUITE 260, MADISON, VI. 33719) AND VICA (MODI PHUSS CONCEL) AND METER FACE LANGE (MADISON, VI. 33719) TOR SAFELY PRACTICES FROM TO PEREPRENCE FROM THE PROPERTY ATTACHED SHALL HAVE A PROPERTY ATTACHED RIGHD CILING.							
STATE OF FLORIDA	No: 34869			DELEAY BEACH, FL S3444-2161	CONS. ENGINEERS P.A.	JULIUS LEE'S		
SPACING	DUR. FAC.	TOT. LD.	BC LL	BC DL	TC DL	TC LL		
	1.00	PSF	PSF	PSF	PSF	PSF		
			-ENG JL	DRWG	DATE	REF		
			IL	CNTONAIL1103	09/12/07	TOE-NAIL		