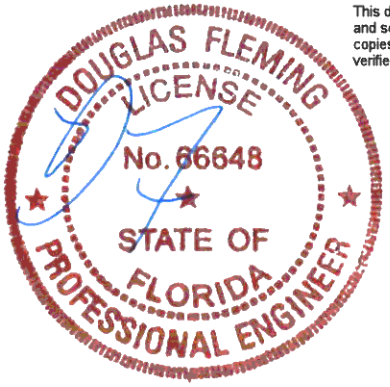

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Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

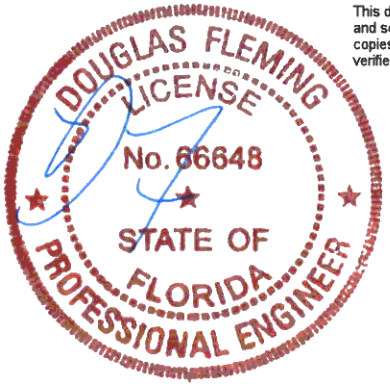
COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

Site Information:	**Page 1:**
Customer: W. B. Howland Company, Inc.	*Job Number:* 21-5560
Job Description: Miller Res	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 21.02.00 *JRef #:* 1XI72150016
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 120 *Building Type:* Closed	*Design Loading (psf):* 40.00, 55.00

This package contains general notes pages, 47 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	231.22.1320.30640	A01	2	231.22.1320.52367	A02
3	231.22.1320.54607	A03	4	231.22.1320.56817	A04
5	231.22.1321.10837	A05	6	231.22.1321.13670	A06
7	231.22.1321.16240	A07	8	231.22.1321.54427	A08
9	231.22.1321.56260	A09	10	231.22.1322.00107	A10
11	231.22.1322.15097	A11	12	231.22.1322.21807	A12
13	231.22.1322.24473	A13	14	231.22.1322.26287	A14
15	231.22.1322.29387	A15	16	231.22.1322.31243	B01
17	231.22.1322.32520	B02	18	231.22.1324.10600	B03
19	231.22.1324.13013	B04	20	231.22.1324.19303	B05
21	231.22.1326.05270	B06	22	231.22.1326.29323	B07
23	231.22.1326.30887	B08	24	231.22.1326.32440	B09
25	231.22.1326.43033	B10	26	231.22.1326.44767	C01
27	231.22.1326.46440	C02	28	231.22.1326.58870	C03
29	231.22.1327.35063	D01	30	231.22.1328.05960	D02
31	231.22.1328.17657	E01	32	231.22.1328.18903	E02
33	231.22.1328.26237	F01	34	231.22.1328.28177	F02
35	231.22.1328.31323	PB01	36	231.22.1328.33447	PB02
37	231.22.1328.38877	PB03	38	231.22.1328.59590	PB04
39	231.22.1329.03483	V01	40	231.22.1329.04660	V02
41	231.22.1329.05777	V03	42	231.22.1329.07077	V04
43	231.22.1329.09333	V05	44	231.22.1329.10680	V06
45	231.22.1329.11980	V07	46	231.22.1329.13857	V08
47	231.22.1329.16847	V09	48	A12030ENC160118	
49	BRCLBSUB0119		50	GABRST160118	



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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	*Job Number:* 21-5560
Job Description: Miller Res	
Address:	

Item	Drawing Number	Truss
51	GBLLETIN0118	
53	CNNAILSP1014	
55	VALTN160118	

Item	Drawing Number	Truss
52	A12015ENC160118	
54	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

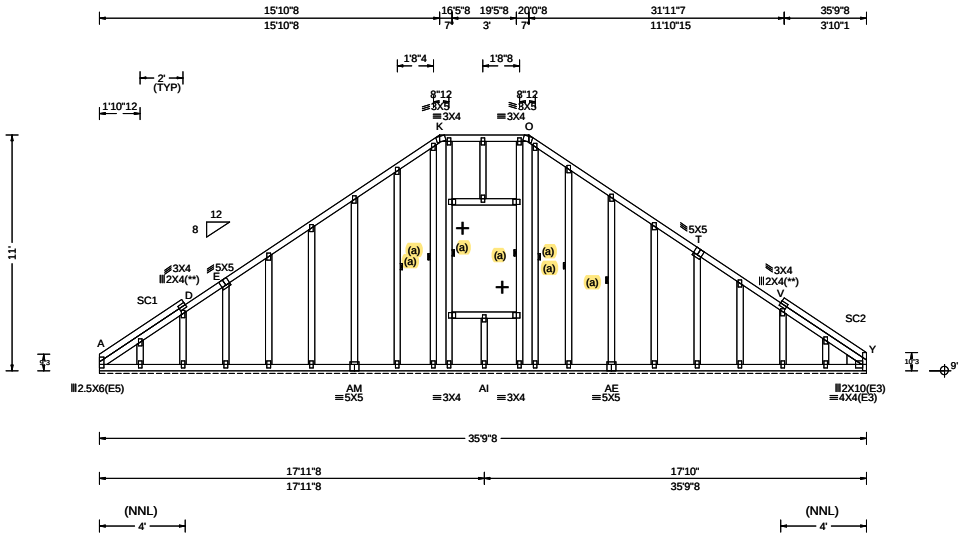
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 100071 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A01		Cust: R 215 JRef:1XI72150016 T2 DrwNo: 231.22.1320.30640 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.58 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.005	M	999	240
VERT(CL): 0.011	M	999	180
HORZ(LL): 0.005	V	-	-
HORZ(TL): 0.006	V	-	-

Creep Factor: 2.0
Max TC CSI: 0.338
Max BC CSI: 0.039
Max Web CSI: 0.165

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
Y*	84	/-	/-	/55	/-	/7

Wind reactions based on MWFRS
Y Brg Wid = 429 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

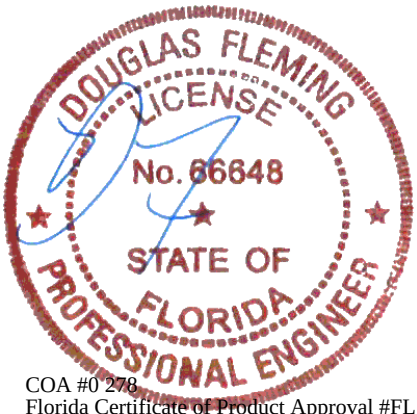
Wind loading based on both gable and hip roof types.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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08/19/2022

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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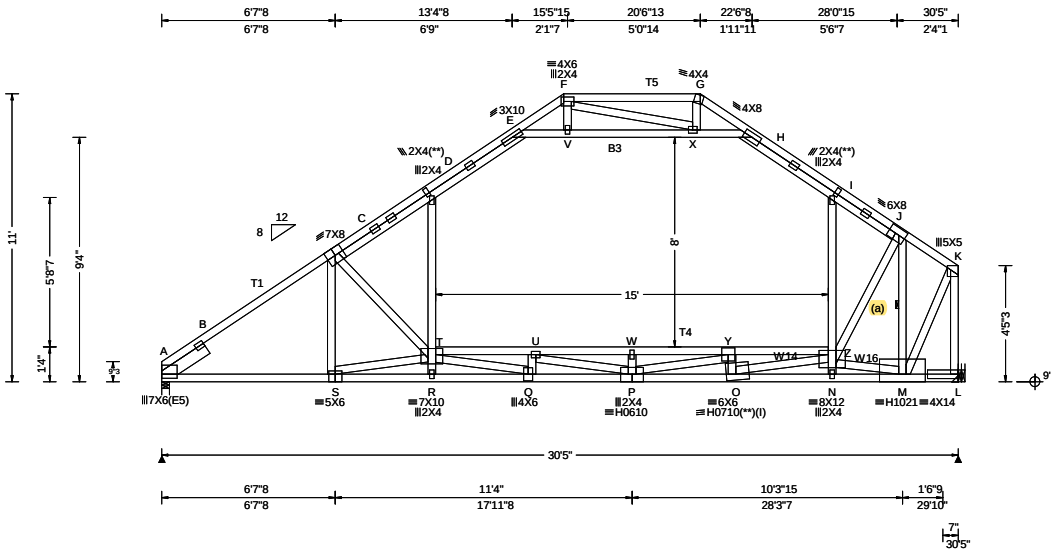
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100058 SPEC FROM:	Ply: 1 Qty: 9	**Job Number:** 21-5560 Miller Res **Truss Label:** A02		Cust: R 215 JRef:1XI72150016 T43 DrwNo: 231.22.1320.52367 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.04 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.462	Q	789	240
VERT(CL): 0.905	Q	403	180
HORZ(LL): 0.257	T	-	-
HORZ(TL): 0.516	T	-	-

Creep Factor: 2.0
Max TC CSI: 0.792
Max BC CSI: 0.562
Max Web CSI: 0.896

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	2069	/-	/-	/732	/-	/161
L	2410	/-	/-	/687	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.7 (Truss)
L Brg Wid = - Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	3713	- 82	A - B	198	- 3171
D - E	1591	- 3098	B - C	14	- 3109
H - I	2260	- 72	E - F	0	- 436
I - J	872	- 3096	J - K	14	- 1101

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - S	2498	- 38	P - O	2696	0
S - R	4309	- 147	O - N	189	- 3058
R - Q	4394	- 151	N - M	187	- 2944
Q - P	5031	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
S - T	192	- 1990	W - Y	0	- 2812
C - S	545	0	X - H	156	- 1708
C - T	129	- 1161	Y - O	13	- 1055
T - Q	1507	- 369	Y - Z	277	- 999
T - U	0	- 3460	O - Z	5122	0
D - T	1159	0	Z - N	400	0
E - V	151	- 1445	Z - I	1184	0
U - P	132	- 674	Z - J	1676	0
U - W	0	- 2812	Z - M	3890	- 97
F - X	53	- 384	J - M	0	- 2741
V - X	151	- 1437	M - K	1936	0
P - Y	2215	0	K - L	0	- 2367

Lumber

Top chord: 2x4 SP M-31; T1,T4,T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP M-31; W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.972'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

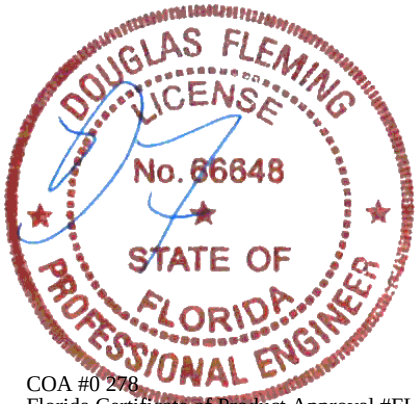
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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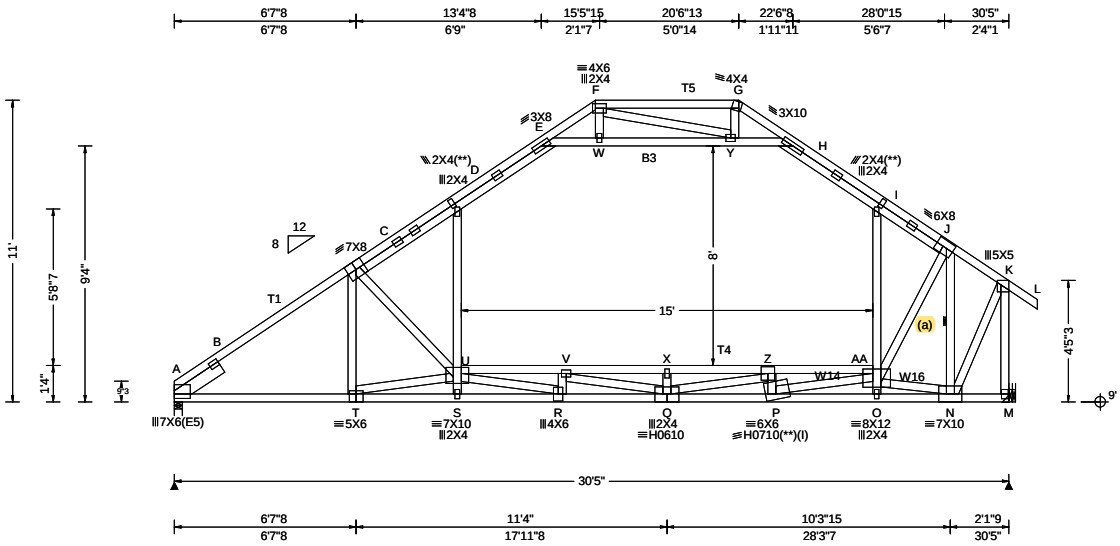
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100046 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A03		Cust: R 215 JRef:1XI72150016 T19 DrwNo: 231.22.1320.54607 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.04 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.462	R	789	240
VERT(CL): 0.905	R	403	180
HORZ(LL): 0.257	U	-	-
HORZ(TL): 0.516	U	-	-

Creep Factor: 2.0
Max TC CSI: 0.792
Max BC CSI: 0.562
Max Web CSI: 0.896

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
A	2069	/-	/-	/732	/-	/161
M	2410	/-	/-	/687	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.7 (Truss)
M Brg Wid = - Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	3713	- 82	A - B	198	- 3171
D - E	1591	- 3098	B - C	14	- 3109
H - I	2260	- 72	E - F	0	- 436
I - J	872	- 3096	J - K	14	- 1101

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - T	2498	- 38	Q - P	2696	0
T - S	4309	- 147	P - O	189	- 3058
S - R	4394	- 151	O - N	187	- 2944
R - Q	5031	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
T - U	192	- 1990	X - Z	0	- 2812
C - T	545	0	Y - H	156	- 1708
C - U	129	- 1161	Z - P	13	- 1055
U - R	1507	- 369	Z -AA	277	- 999
U - V	0	- 3460	P -AA	5122	0
D - U	1159	0	AA- O	400	0
E - W	151	- 1445	AA- I	1184	0
V - Q	132	- 674	AA- J	1676	0
V - X	0	- 2812	AA- N	3890	- 97
F - Y	53	- 384	J - N	0	- 2741
W - Y	151	- 1437	N - K	1936	0
Q - Z	2215	0	K - M	0	- 2367

Lumber

Top chord: 2x4 SP M-31; T1,T4,T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP M-31; W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.972'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

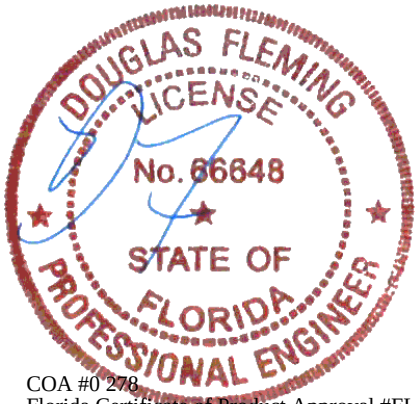
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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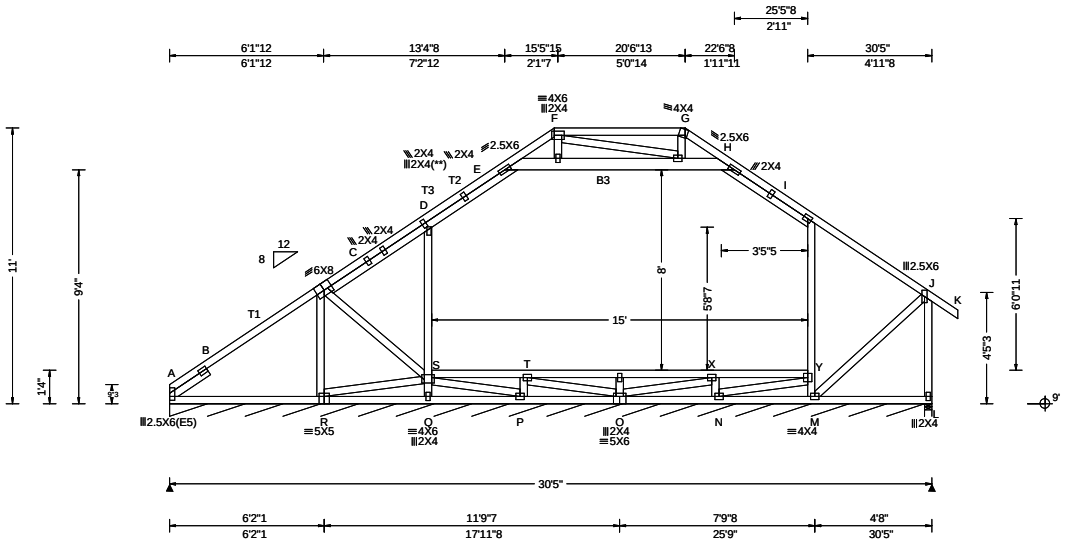
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100134 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A04		Cust: R 215 JRef:1XI72150016 T49 DrwNo: 231.22.1320.56817 KD / DF 08/19/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.059 H 999 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.136 H 999 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.029 B - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.060 B - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.594
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.340
Spacing:	24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.474
		C&C Dist a: 3.04 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
		Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A*	149	/-	/-	/5	/-	/-
L	1288	/-	/-	/-	/135	/-

Wind reactions based on MWFRS
A Brg Wid = 361 Min Req = -
L Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
C - D	90	-534	F - G	65	-543
A - B	153	-839	G - H	78	-623
B - C	85	-716	H - I	307	-992
D - E	74	-573	I - J	106	-895
E - F	83	-646			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - R	549	-62	O - N	569	-56
P - O	458	-44	N - M	674	-77

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - R	78	-574	X - N	3	-472
D - S	53	-375	Y - I	56	-482
S - Q	42	-492	Y - M	50	-564
S - T	389	-38	M - J	921	-105
P - T	7	-506	J - L	154	-1253

Lumber

Top chord: 2x4 SP #2; T1,T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B3 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.836'

Plating Notes

All plates are 3X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

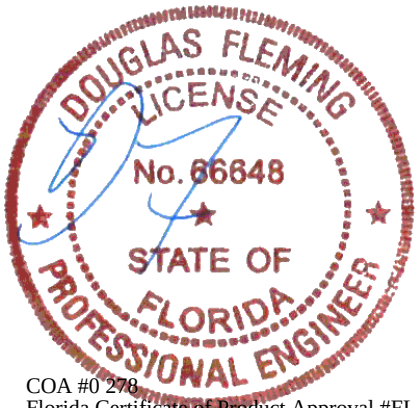
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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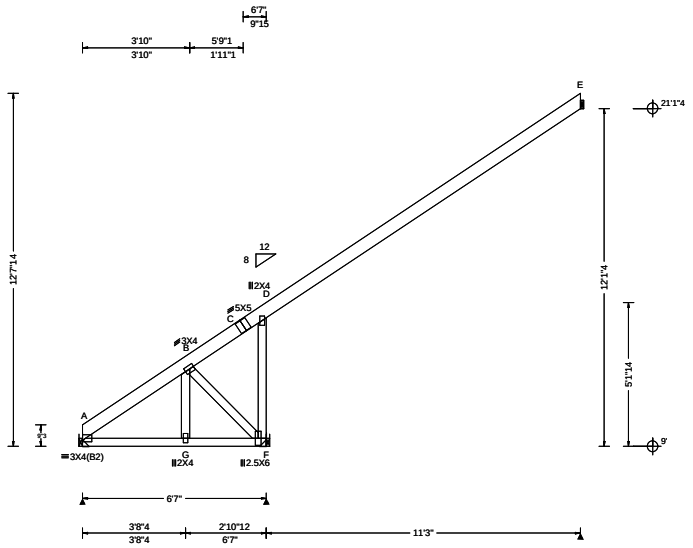
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100105 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A05	Cust: R 215 JRef:1XI72150016 T38 DrwNo: 231.22.1321.10837 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.71 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	D	999	240
VERT(CL): 0.006	D	999	180
HORZ(LL): 0.004	D	-	-
HORZ(TL): 0.007	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.379
Max BC CSI: 0.118
Max Web CSI: 0.345

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	233	/-	/-	/148	/7	/178
F	824	/-	/-	/508	/90	/-
E	291	/-	/-	/168	/45	/-

Wind reactions based on MWFRS
A Brg Wid = - Min Req = -
F Brg Wid = - Min Req = -
E Brg Wid = 1.5 Min Req = -
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
D - F	228	-804

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

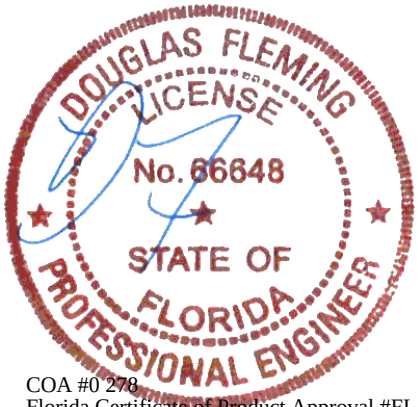
Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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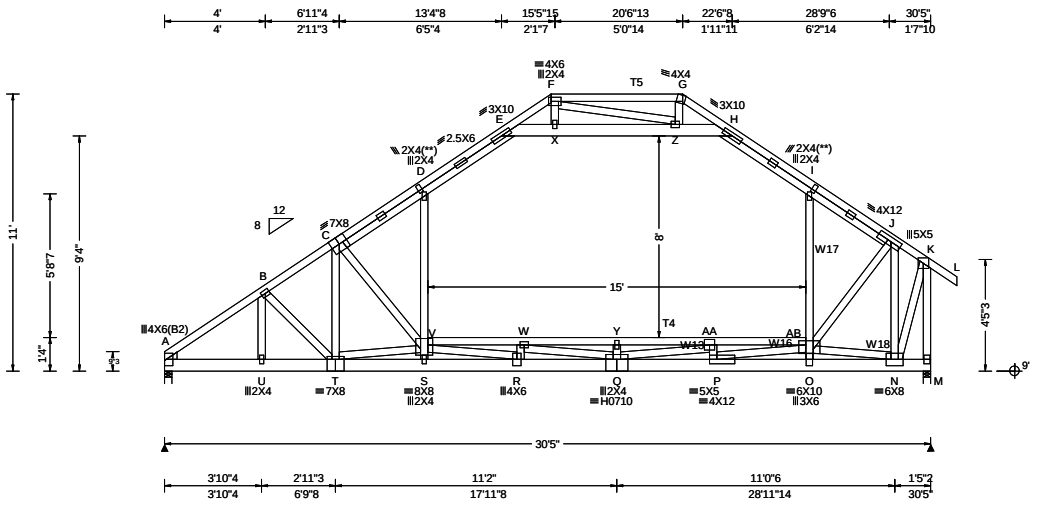
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100136 SPEC FROM:	Ply: 2 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A06	Cust: R 215 JRef:1XI72150016 T24 DrwNo: 231.22.1321.13670 KD / DF 08/19/2022		

2 Complete Trusses Required



Loading Criteria (psf)

TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria

Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.04 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.399	Y	913	240
VERT(CL): 0.779	Y	467	180
HORZ(LL): 0.185	D	-	-
HORZ(TL): 0.360	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.807
Max BC CSI: 0.448
Max Web CSI: 0.879

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	3336	/-	/-	/16	/85	/-
M	4252	/-	/-	/6	/85	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
M Brg Wid = 3.5 Min Req = 1.8 (Truss)
Bearings A & M are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	1201	- 2503	C - D	1228	- 3351
I - J	999	- 3058	H - I	437	- 1813
A - B	69	- 2436	J - K	14	- 662
B - C	72	- 2539			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - U	1955	- 52	R - Q	4424	- 42
U - T	1960	- 52	Q - P	2993	0
T - S	2739	- 97	P - O	65	- 1698
S - R	2780	- 98	O - N	62	- 1611

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
T - V	41	- 1011	Z - H	6	- 1780
C - V	27	- 983	AA- P	24	- 770
D - V	1314	0	AA-AB	0	- 1265
V - R	1593	0	P -AB	4552	- 29
V - W	4	- 2842	AB- O	831	0
E - X	0	- 1652	AB- I	1162	0
W - Q	788	- 389	AB- J	1456	- 48
W - Y	0	- 3675	AB- N	2275	- 73
X - Z	0	- 1653	J - N	58	- 2435
Q -AA	2266	- 69	N - K	1658	- 26
Y -AA	0	- 3675	K - M	42	- 1959

Lumber

Top chord: 2x4 SP M-31; T4,T5 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W13 2x4 SP #2; W16,W17, W18 2x4 SP M-31;
Lt Wedge: 2x4 SP #3;

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Plating Notes

All plates are 3X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

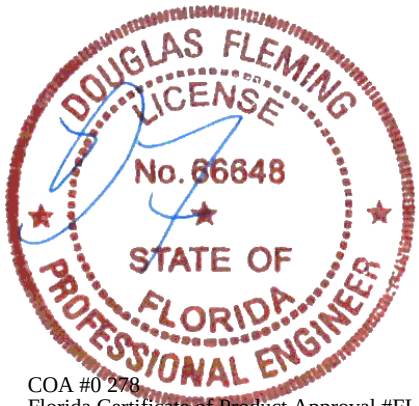
THE LOADING SHOWN MUST BE APPROVED BY THE BUILDING DESIGNER.

Wind

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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08/19/2022

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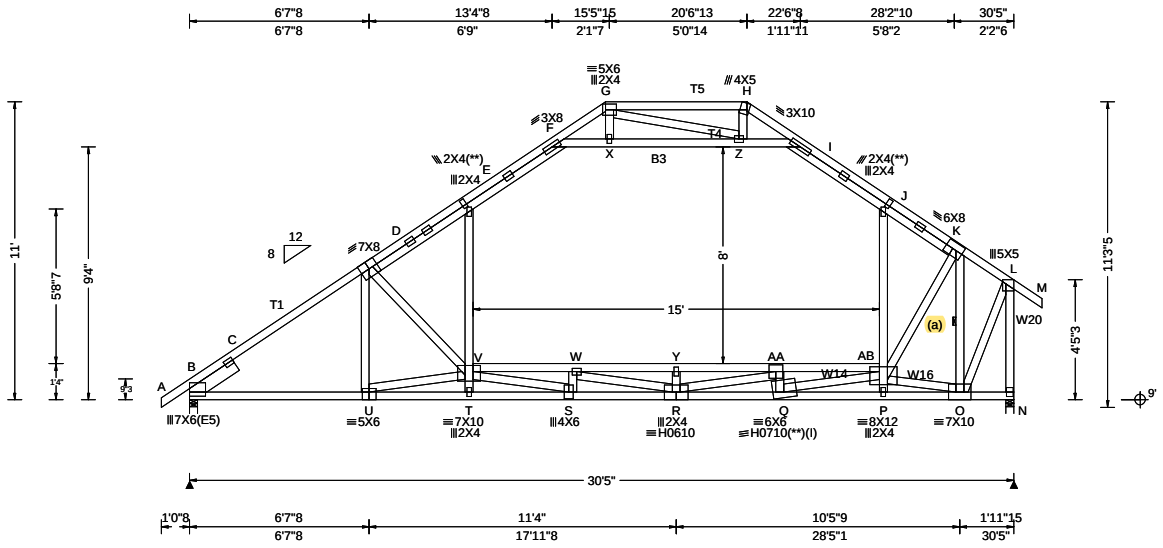
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100086 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 21-5560 Miller Res **Truss Label:** A07		Cust: R 215 JRef:1XI72150016 T47 DrwNo: 231.22.1321.16240 KD / DF 08/19/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.460 S 792 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.896 S 407 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.255 V - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL): 0.510 V - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit:	0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.792
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.559
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.858
		C&C Dist a: 3.04 ft	FT/RT:20(0)/10(0)	
		Loc. from endwall: not in 9.00 ft	Plate Type(s):	
		GCpi: 0.18	WAVE, HS	VIEW Ver: 21.02.00.1005.17
		Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	2136	/-	/-	/771	/-	/181
N	2478	/-	/-	/722	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.8 (Truss)
N Brg Wid = 3.5 Min Req = 2.1 (Truss)
Bearings B & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	3711	0	B - C	191	-3162
E - F	1607	-3031	C - D	86	-3100
I - J	2271	0	F - G	202	-434
J - K	841	-3054	K - L	65	-1050

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	2488	-70	R - Q	2668	0
U - T	4263	-193	Q - P	214	-3006
T - S	4347	-197	P - O	211	-2897
S - R	4993	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
U - V	206	-1952	Y -AA	0	-2772
D - U	533	-1	Z - I	10	-1711
D - V	65	-1144	AA- Q	19	-1059
V - S	1508	-362	AA-AB	285	-967
V - W	0	-3418	Q -AB	5039	0
E - V	1158	0	AB- P	387	0
F - X	2	-1450	AB- J	1157	0
W - R	146	-673	AB- K	1683	-27
W - Y	0	-2772	AB- O	3796	-144
G - Z	55	-381	K - O	45	-2707
X - Z	2	-1442	O - L	1932	-28
R -AA	2211	0	L - N	84	-2435

Lumber

Top chord: 2x4 SP M-31; T1,T4,T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14,W20 2x4 SP M-31;
W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.972'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 3X4 except as noted.

(I) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

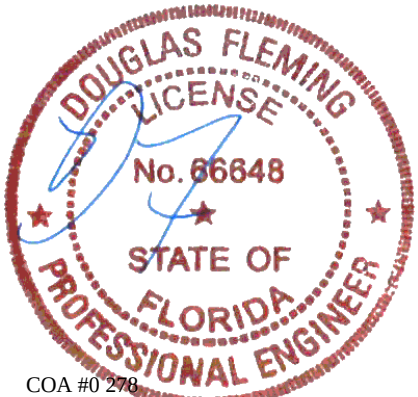
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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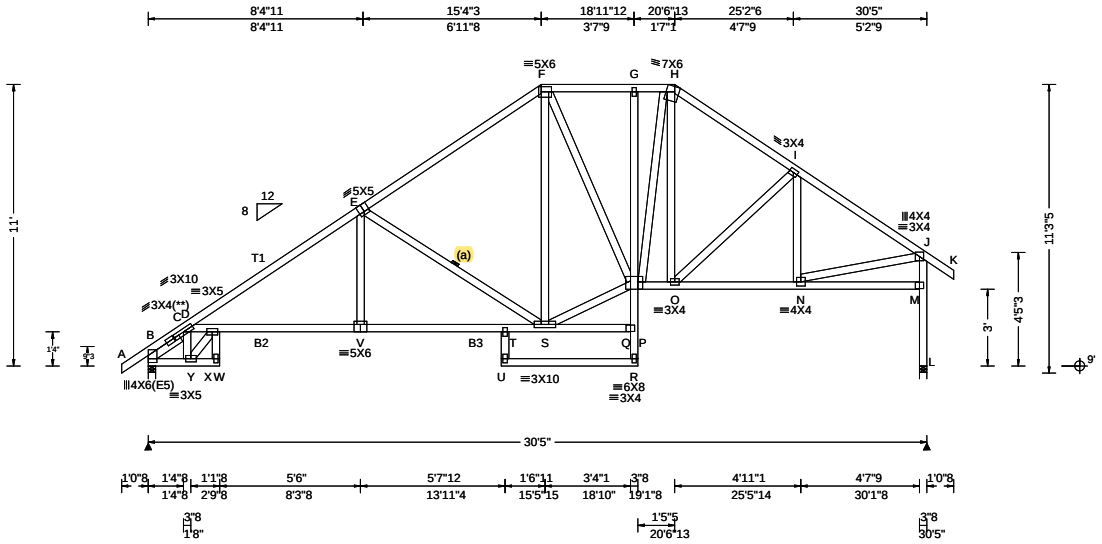
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100072 SPEC FROM:	Ply: 1 Qty: 3	**Job Number:** 21-5560 Miller Res **Truss Label:** A08		Cust: R 215 JRef:1XI72150016 T41 DrwNo: 231.22.1321.54427 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.04 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.434	W	837	240
VERT(CL): 0.895	W	405	180
HORZ(LL): 0.208	R	-	-
HORZ(TL): 0.429	R	-	-

Creep Factor: 2.0
Max TC CSI: 0.800
Max BC CSI: 0.902
Max Web CSI: 0.842

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

Loc	Gravity R+	/ R-	/ Rh	Non-Gravity / Rw	/ U	/ RL
B	1338	/-	/-	/768	/-	/181
L	1345	/-	/-	/720	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
L Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings B & L are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	62	- 1433	F - G	166	- 1188
C - D	55	- 1383	G - H	166	- 1189
D - E	117	- 2238	H - I	166	- 1415
E - F	165	- 1376	I - J	105	- 1509

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - Y	888	- 97	T - S	1798	- 76
D - X	1189	- 15	P - O	1103	- 3
X - V	1810	- 78	O - N	1198	- 29
V - T	1804	- 79			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - Y	125	- 1021	S - P	1145	- 17
Y - X	1092	- 111	P - H	437	- 56
V - E	523	0	N - J	1360	- 33
E - S	82	- 934	M - L	79	- 1345
F - P	378	- 14	M - J	97	- 1308

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2,B3 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

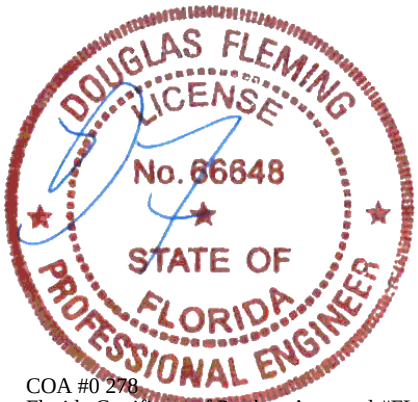
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



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Florida Certificate of Product Approval #FL1999
08/19/2022

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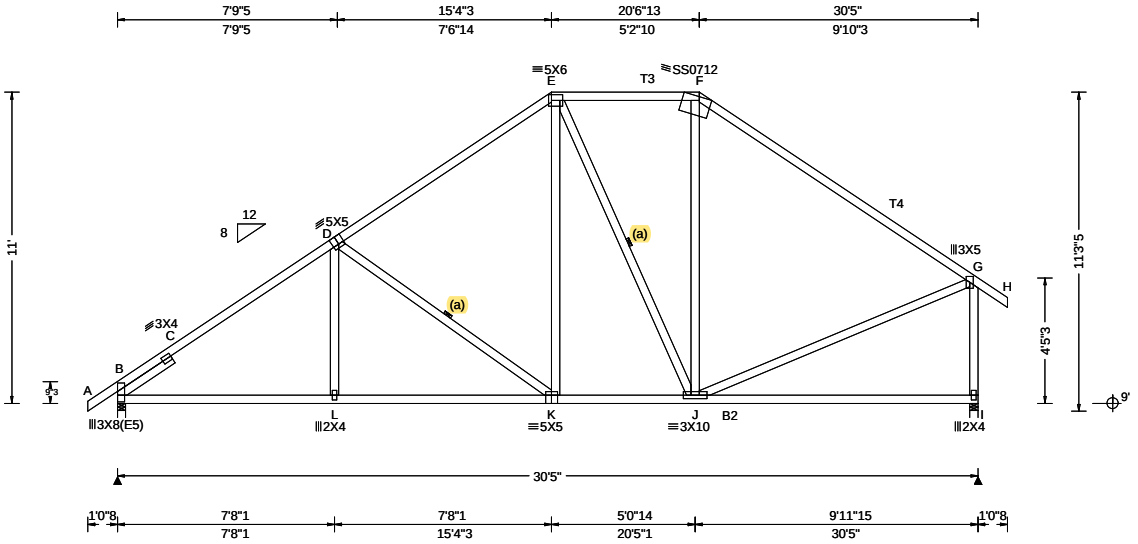
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99855 SPEC FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** A09	Cust: R 215 JRef:1XI72150016 T12 DrwNo: 231.22.1321.56260 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.04 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, 18SS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.100	C	999	240
VERT(CL): 0.187	C	999	180
HORZ(LL): 0.070	C	-	-
HORZ(TL): 0.131	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.801
Max BC CSI: 0.864
Max Web CSI: 0.522

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1540	/-	/-	/771	/-	/180
I	1458	/-	/-	/721	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.8 (Truss)
I Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & I are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	210	- 2172	E - F	160	- 967
C - D	85	- 2074	F - G	135	- 1311
D - E	159	- 1436			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - L	1655	- 61	K - J	1070	- 10
L - K	1650	- 61			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - K	67	- 720	J - G	1045	- 7
E - K	608	0	G - I	112	- 1378

Lumber

Top chord: 2x4 SP #2; T3,T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.331'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

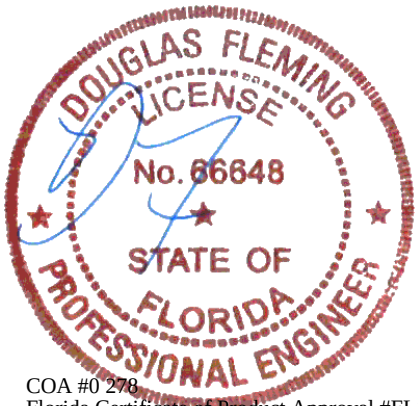
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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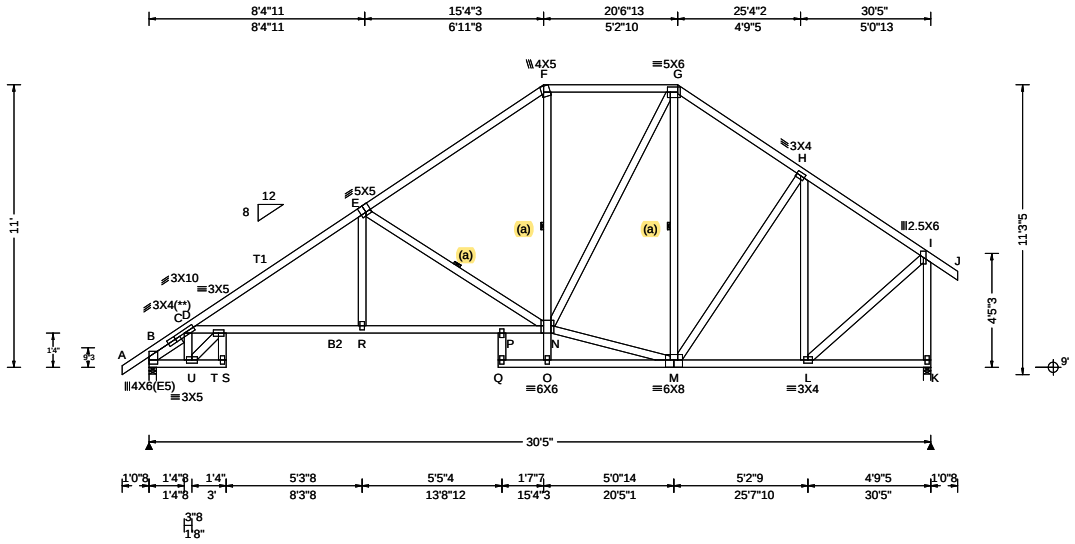
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100073 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A10		Cust: R 215 JRef:1XI72150016 T18 DrwNo: 231.22.1322.00107 KD / DF 08/19/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL):	0.449 S 813 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.929 S 393 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.218 L - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.451 L - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	
Soffit:	0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.777
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI:	0.899
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI:	0.560
		C&C Dist a: 3.04 ft	FT/RT:20(0)/10(0)		
		Loc. from endwall: not in 9.00 ft	Plate Type(s):		
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17	
		Wind Duration: 1.60			

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1345	/-	/-	/771	/-	/181
K	1345	/-	/-	/722	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
K Brg Wid = 3.5 Min Req = 1.6 (Truss)
Bearings B & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	64	-1457	F - G	165	-1050
C - D	58	-1406	G - H	167	-1067
D - E	118	-2242	H - I	96	-943
E - F	163	-1403			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	915	-101	R - P	1805	-78
D - T	1135	-9	P - N	1769	-71
T - R	1811	-78	M - L	737	-18

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - U	115	-901	N - M	798	0
U - T	1032	-105	N - G	503	-21
R - E	486	0	H - L	72	-519
E - N	81	-899	L - I	953	-21
F - N	408	0	I - K	94	-1305

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

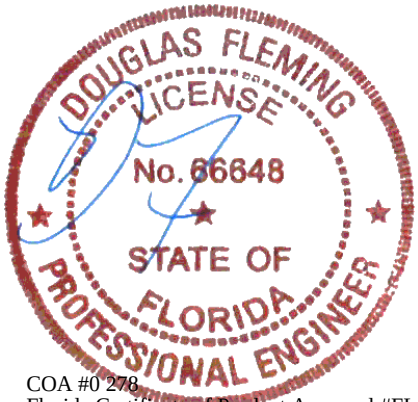
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



COA #0 278
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08/19/2022

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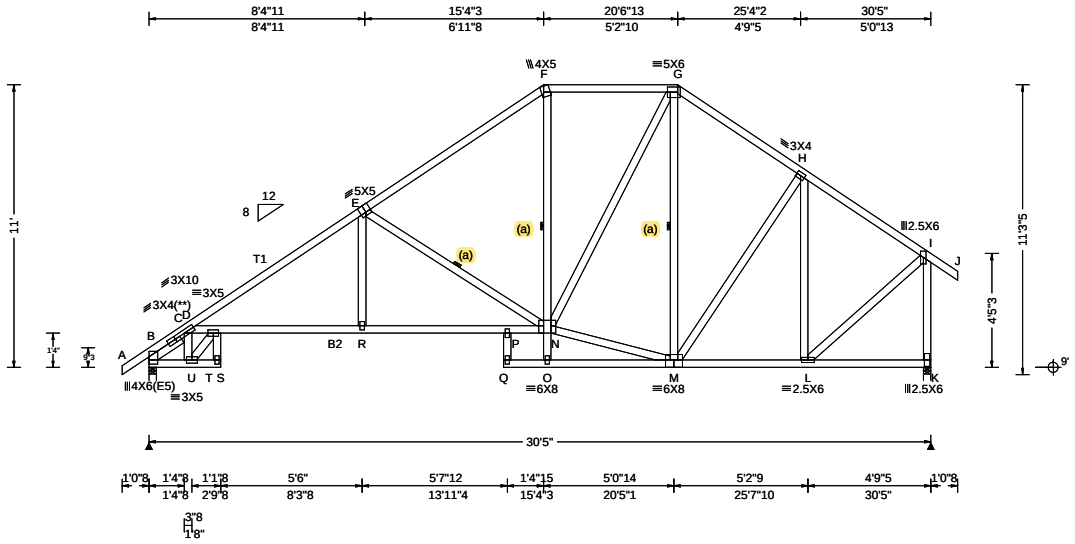
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100074 SPEC FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** A11		Cust: R 215 JRef:1XI72150016 T40 DrwNo: 231.22.1322.15097 KD / DF 08/19/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL):	0.464 S 786 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.928 S 393 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.241 L - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.482 L - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	
Soffit:	0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.795
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI:	0.919
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI:	0.528
		C&C Dist a: 3.04 ft	FT/RT:20(0)/10(0)		
		Loc. from endwall: not in 9.00 ft	Plate Type(s):		
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17	
		Wind Duration: 1.60			

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1363	/-	/-	/771	/-	/181
K	1417	/-	/-	/722	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
K Brg Wid = 3.5 Min Req = 1.7 (Truss)
Bearings B & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	63	-1457	F - G	165	-1080
C - D	56	-1406	G - H	167	-1111
D - E	117	-2280	H - I	96	-1005
E - F	163	-1440			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	899	-98	R - P	1840	-79
D - T	1217	-14	P - N	1805	-72
T - R	1846	-78	M - L	788	-18

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - U	126	-1033	N - M	838	0
U - T	1105	-113	N - G	487	-21
R - E	482	0	H - L	72	-489
E - N	81	-906	L - I	1021	-21
F - N	409	0	I - K	94	-1384

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

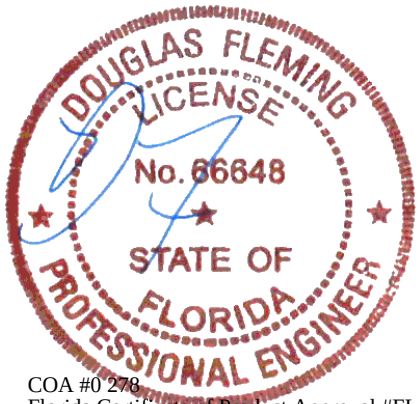
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



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08/19/2022

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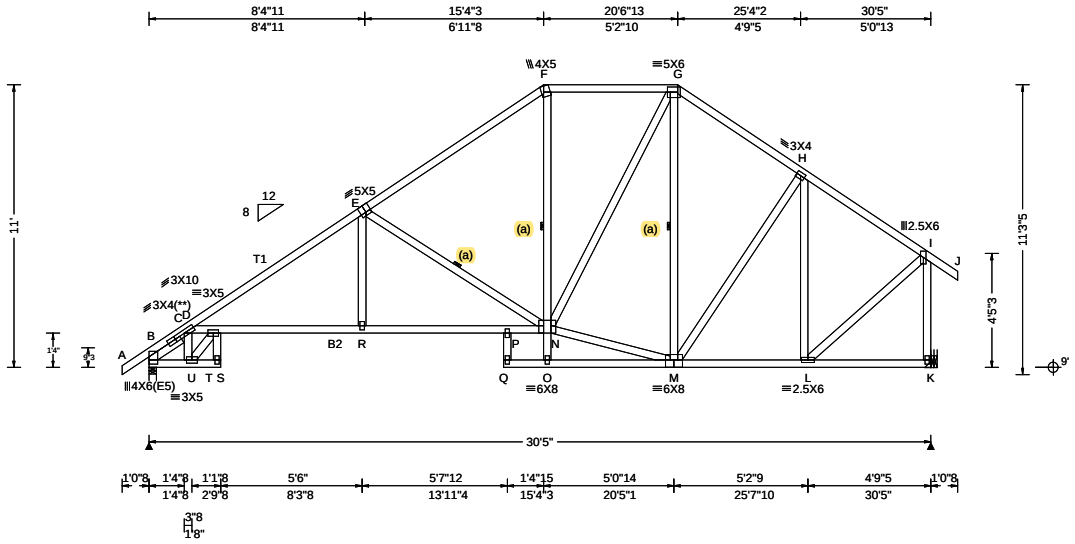
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100075 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** A12		Cust: R 215 JRef:1XI72150016 T17 DrwNo: 231.22.1322.21807 KD / DF 08/19/2022	



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	
TCLL:	20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc L/defl L/#
TCDL:	10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL):	0.464 S 786 240
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.928 S 393 180
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.241 L - -
Des Ld:	40.00	EXP: B Kzt: NA		HORZ(TL):	0.482 L - -
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	
Soffit:	0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI:	0.795
Load Duration:	1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI:	0.919
Spacing:	24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI:	0.528
		C&C Dist a: 3.04 ft	FT/RT:20(0)/10(0)		
		Loc. from endwall: not in 9.00 ft	Plate Type(s):		
		GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17	
		Wind Duration: 1.60			

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1363	/-	/-	/771	/-	/181
K	1417	/-	/-	/722	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.6 (Truss)
K Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	63	-1457	F - G	165	-1080
C - D	56	-1406	G - H	167	-1111
D - E	117	-2280	H - I	96	-1005
E - F	163	-1440			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	899	-98	R - P	1840	-79
D - T	1217	-14	P - N	1805	-72
T - R	1846	-78	M - L	788	-18

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - U	126	-1033	N - M	838	0
U - T	1105	-113	N - G	487	-21
R - E	482	0	H - L	72	-489
E - N	81	-906	L - I	1021	-21
F - N	409	0	I - K	94	-1384

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

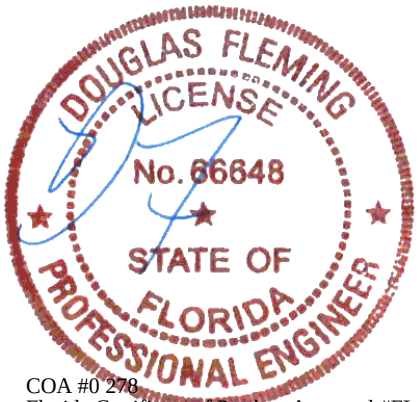
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 2'0" O.C.Max. including a lateral brace at chord ends.



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08/19/2022

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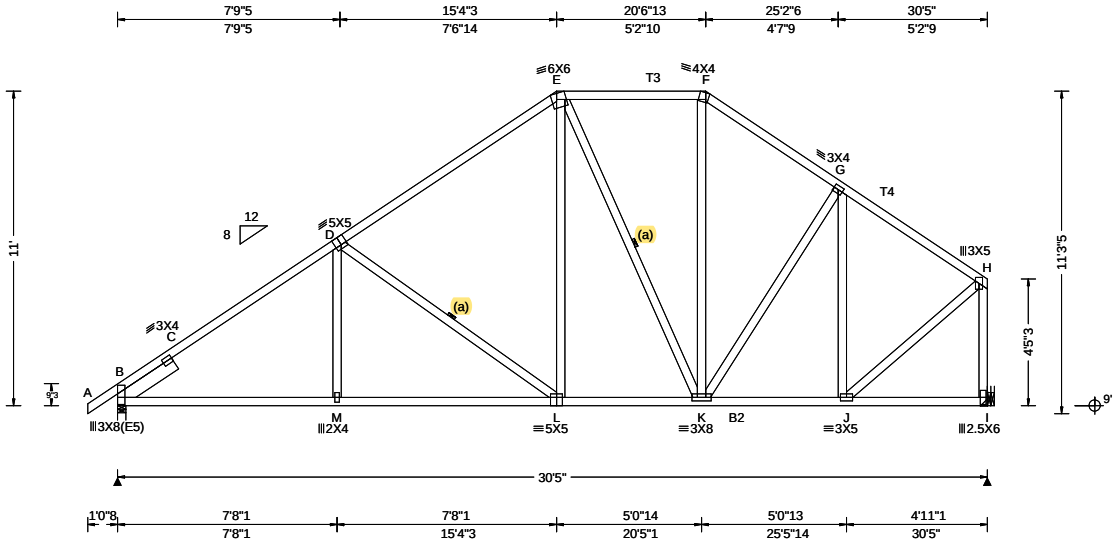
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100076 COMN FROM:	Ply: 1 Qty: 9	**Job Number:** 21-5560 Miller Res **Truss Label:** A13	Cust: R 215 JRef:1XI72150016 T9 DrwNo: 231.22.1322.24473 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.04 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.075	C	999	240
VERT(CL): 0.138	C	999	180
HORZ(LL): 0.053	C	-	-
HORZ(TL): 0.097	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.427
Max BC CSI: 0.426
Max Web CSI: 0.604

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1560	/-	/-	/772	/-	/167
I	1461	/-	/-	/678	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
I Brg Wid = - Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	297	- 2230	E - F	204	- 958
C - D	157	- 2106	F - G	215	- 1239
D - E	213	- 1472	G - H	125	- 1109

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - M	1679	- 143	L - K	1104	- 50
M - L	1674	- 143	K - J	877	- 56

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
D - L	116	- 709	J - H	1122	- 69
E - L	629	- 2	H - I	128	- 1426
G - J	101	- 545			

Lumber

Top chord: 2x4 SP M-31; T3,T4 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 2.331'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

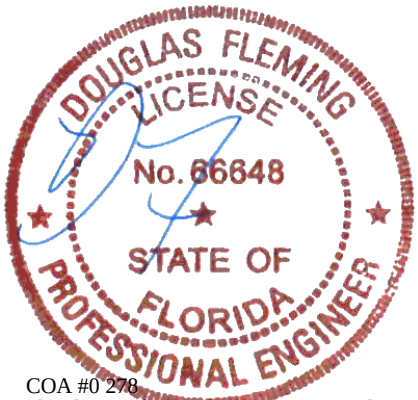
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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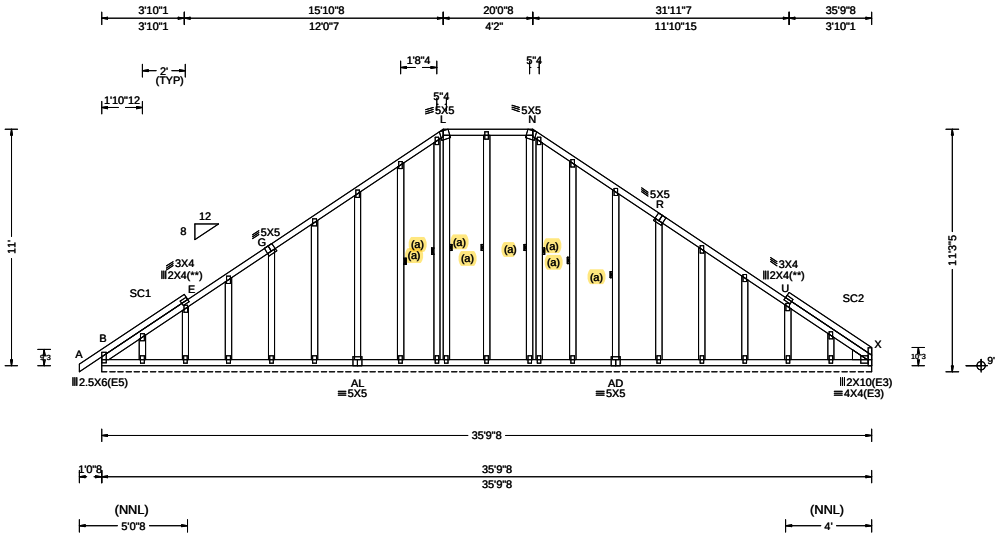
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100077 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A14		Cust: R 215 JRef:1XI72150016 T6 DrwNo: 231.22.1322.26287 KD / DF 08/19/2022	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.001 M 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 M 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.005 U - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.007 U - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.338
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.037
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.163
	C&C Dist a: 3.58 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
X*	86	/-	/-	/54	/1	/8

Wind reactions based on MWFRS
X Brg Wid = 429 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

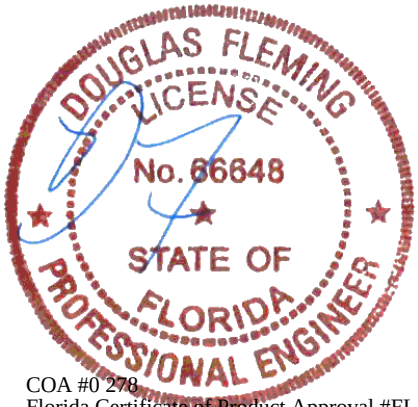
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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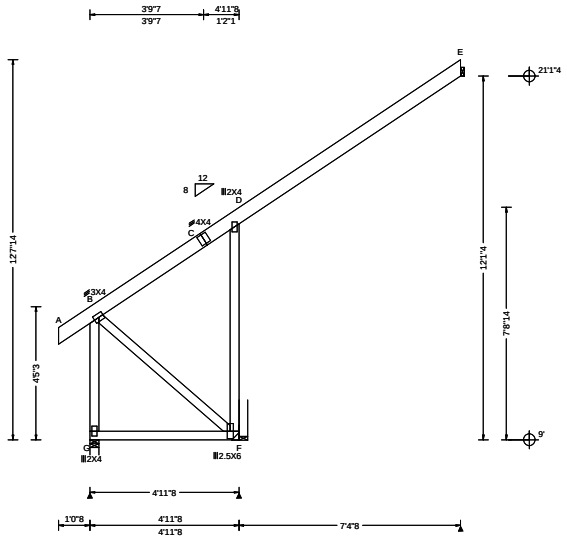
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100052 MONO FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** A15	Cust: R 215 JRef:1XI72150016 T10 DrwNo: 231.22.1322.29387 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 17.20 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	D	999	240
VERT(CL): 0.006	D	999	180
HORZ(LL): -0.006	D	-	-
HORZ(TL): 0.007	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.142
Max BC CSI: 0.292
Max Web CSI: 0.495

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	249	/-	/-	/254	/22	/180
F	544	/-	/-	/406	/259	/-
E	199	/-	/-	/112	/54	/-

Wind reactions based on MWFRS
G Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
E Brg Wid = 1.5 Min Req = -
Bearing G is a rigid surface.
Members not listed have forces less than 375#

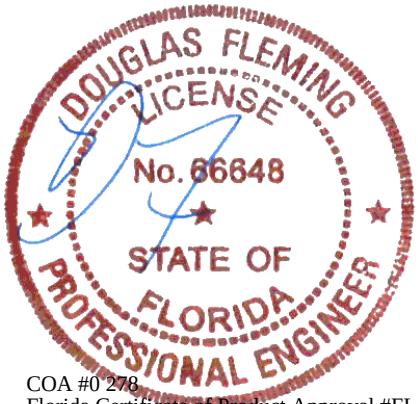
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
D - F	227 - 494

Lumber
Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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08/19/2022

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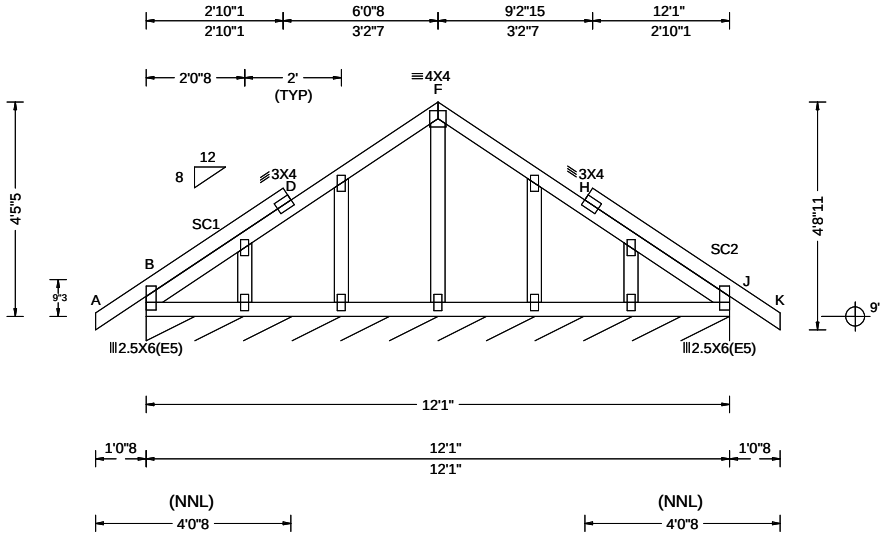
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100000 GABL FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** B01	Cust: R 215 JRef:1XI72150016 T8 DrwNo: 231.22.1322.31243 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	H	999	240
VERT(CL): 0.004	H	999	180
HORZ(LL): -0.001	H	-	-
HORZ(TL): 0.002	H	-	-

Creep Factor: 2.0
Max TC CSI: 0.140
Max BC CSI: 0.032
Max Web CSI: 0.041

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
J*	95	/-	/-	/49	/-	/7

Wind reactions based on MWFRS
J Brg Wid = 145 Min Req = -
Bearing B is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

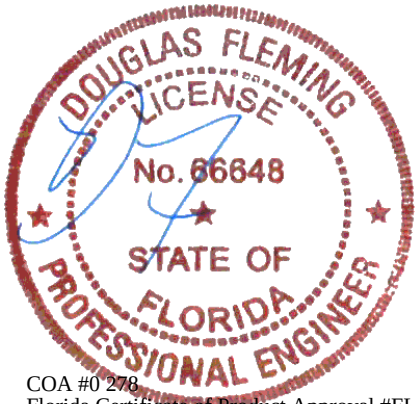
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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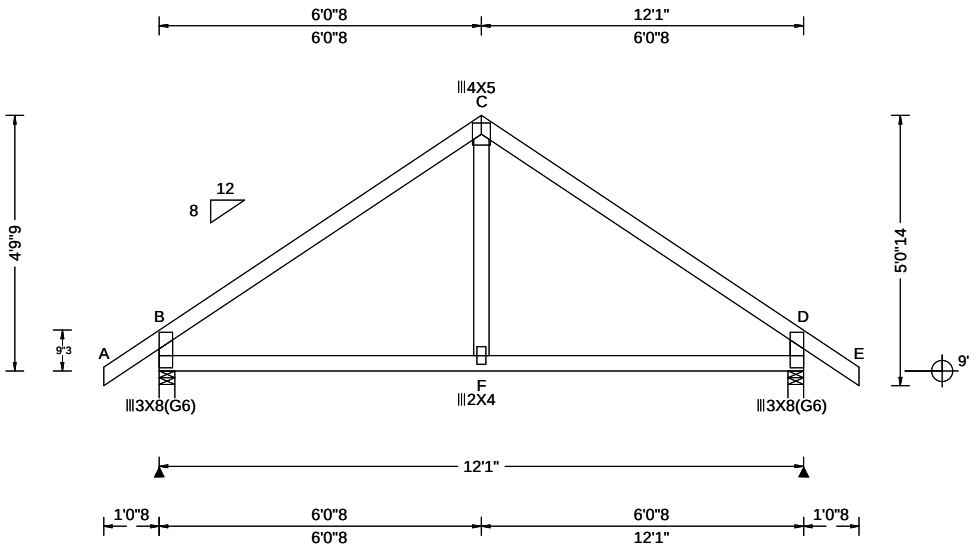
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99787 COMN FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** B02	Cust: R 215 JRef:1XI72150016 T22 DrwNo: 231.22.1322.32520 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	F	999	240
VERT(CL): 0.008	F	999	180
HORZ(LL): 0.004	D	-	-
HORZ(TL): 0.008	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.365
Max BC CSI: 0.334
Max Web CSI: 0.099

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	574	/-	/-	/329	/-	/85
D	574	/-	/-	/291	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & D are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	139	-578	C - D	139	-578

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	393	0	F - D	393	0

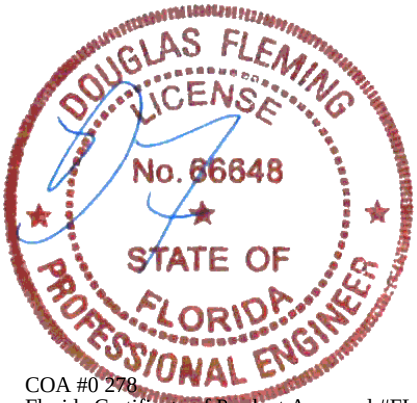
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;Rt Stub Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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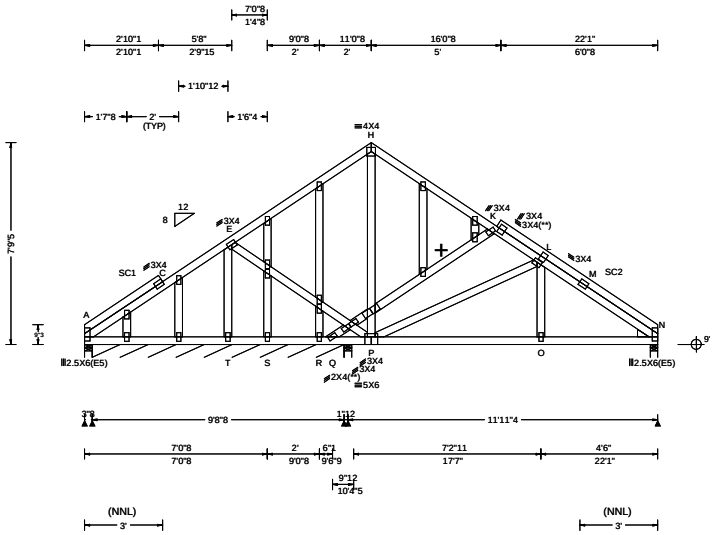
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100009 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B03		Cust: R 215 JRef:1XI72150016 T7 DrwNo: 231.22.1324.10600 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.029	M	999	240
VERT(CL): 0.060	M	999	180
HORZ(LL): -0.013	M	-	-
HORZ(TL): 0.027	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.329
Max BC CSI: 0.373
Max Web CSI: 0.315

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	69	/-30	/-	/69	/28	/226
A*	110	/-	/-	/106	/11	/-
Q	188	/-	/-	/160	/-	/-
N	563	/-	/-	/505	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
A Brg Wid = 116 Min Req = -
Q Brg Wid = 3.5 Min Req = 1.5 (Truss)
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A, A, Q, & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - L	24	-386	M - N	0	-769
L - M	0	-666			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	582	0	O - N	586	0

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
T - E	0	-404

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

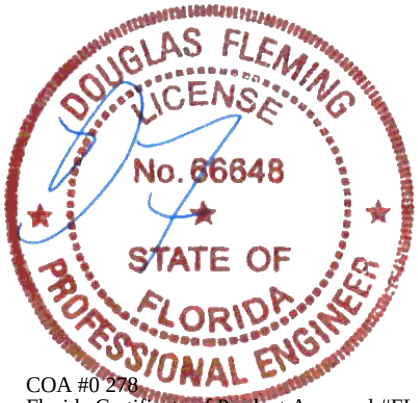
Wind loading based on both gable and hip roof types.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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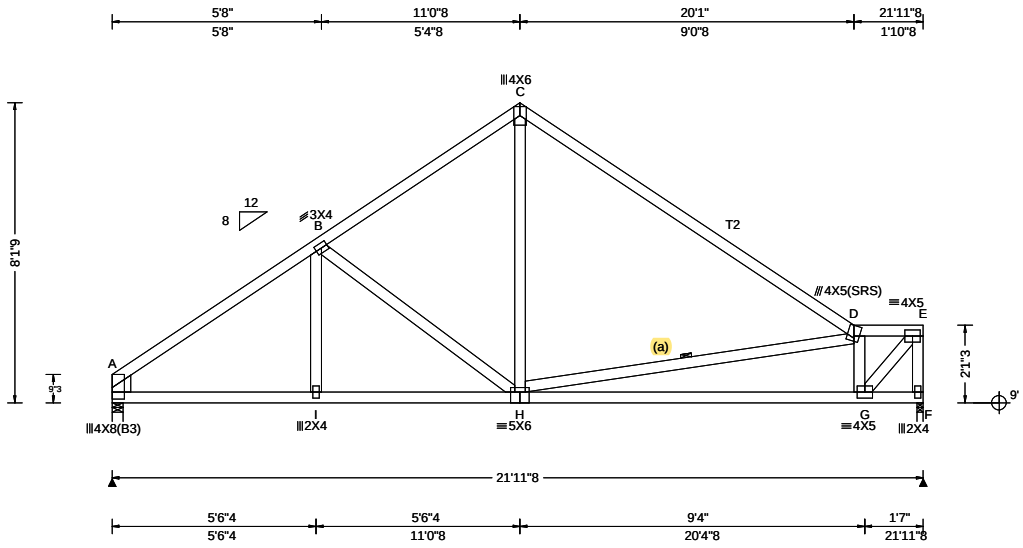
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99798 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B04	Cust: R 215 JRef:1XI72150016 T1 DrwNo: 231.22.1324.13013 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.033	H	999	240
VERT(CL): 0.068	H	999	180
HORZ(LL): 0.016	G	-	-
HORZ(TL): 0.034	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.631
Max BC CSI: 0.731
Max Web CSI: 0.551

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	923	/-	/-	/516	/-	/116
F	923	/-	/-	/493	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 2.0 Min Req = 1.5 (Truss)
Bearings A & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	114	-1250	C - D	131	-1007
B - C	150	-922	D - E	93	-954

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - I	945	-73	H - G	1101	-126
I - H	943	-73			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	524	0	G - E	1447	-140
H - D	133	-403	E - F	62	-973
D - G	211	-928			

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

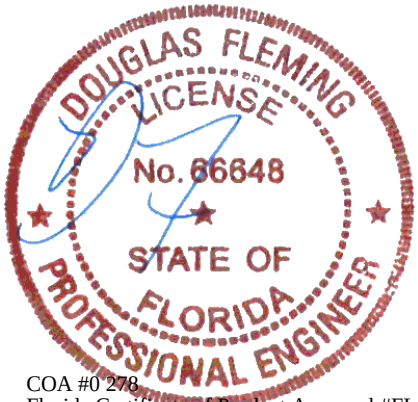
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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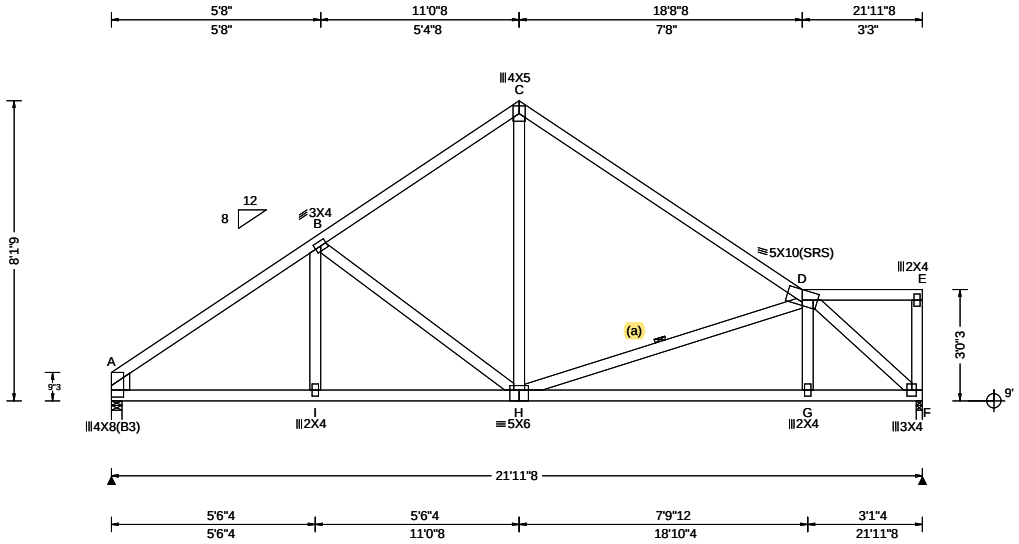
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99801 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B05		Cust: R 215 JRef:1XI72150016 T11 DrwNo: 231.22.1324.19303 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.035	H	999	240
VERT(CL): 0.073	H	999	180
HORZ(LL): 0.017	F	-	-
HORZ(TL): 0.035	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.670
Max BC CSI: 0.555
Max Web CSI: 0.447

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	923	/-	/-	/517	/-	/115
F	923	/-	/-	/482	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 2.0 Min Req = 1.5 (Truss)
Bearings A & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	24	-1255	C - D	67	-966
B - C	74	-916			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - I	950	-19	H - G	1044	-52
I - H	948	-20	G - F	1050	-48

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	531	0	D - F	61	-1367

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

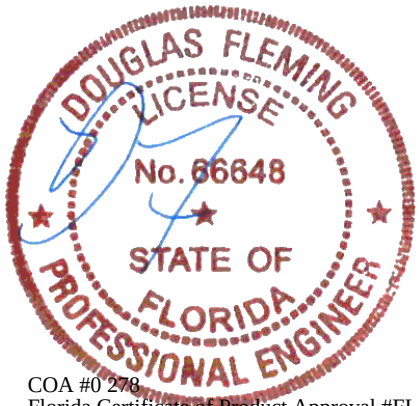
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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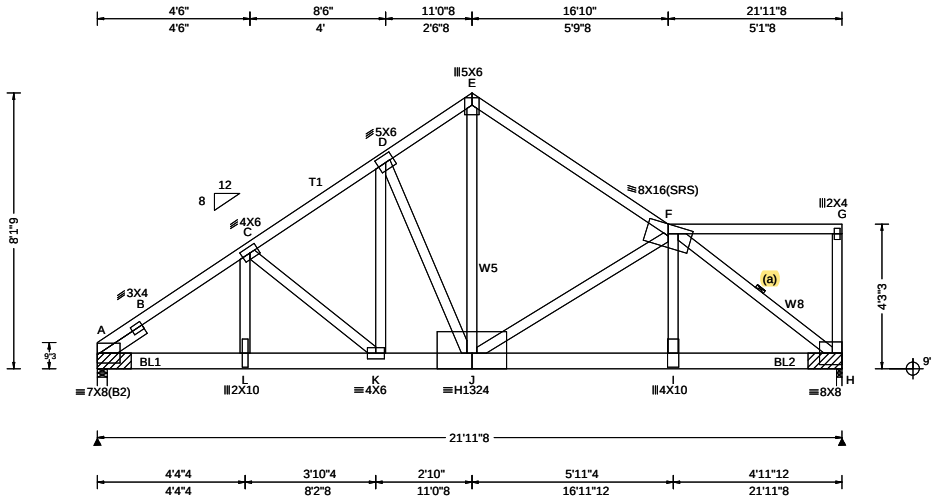
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100040 SPEC FROM: Page 1 of 2	Ply: 2 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B06	Cust: R 215 JRef:1XI72150016 T26 DrwNo: 231.22.1326.05270 KD / DF 08/19/2022

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	0.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.116	K	999	240
VERT(CL): 0.230	K	999	180
HORZ(LL): 0.041	C	-	-
HORZ(TL): 0.081	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.565
Max BC CSI: 0.651
Max Web CSI: 0.868

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	9178	/-	/-	/-	/574	/-
H	8295	/-	/-	/-	/1104	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = -
H Brg Wid = 2.0 Min Req = -
Bearings A & H are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	632	-5704	D - E	501	-3869
B - C	627	-5673	E - F	506	-3910
C - D	588	-4645			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - L	4670	-509	J - I	4685	-623
L - K	4592	-506	I - H	4771	-635
K - J	3782	-480			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
L - C	1241	-43	E - J	4096	-530
C - K	26	-999	J - F	248	-1764
K - D	1623	-181	F - I	2279	-319
D - J	158	-1399	F - H	794	-5967

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5,W8 2x4 SP M-31;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at 0.00 to 64 plf at 21.96
BC: From 10 plf at 0.00 to 10 plf at 21.96
BC: 1348 lb Conc. Load at 0.35, 2.35
BC: 1461 lb Conc. Load at 4.35, 6.35, 8.35,10.35 12.35,14.35,16.35,18.35,20.02

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

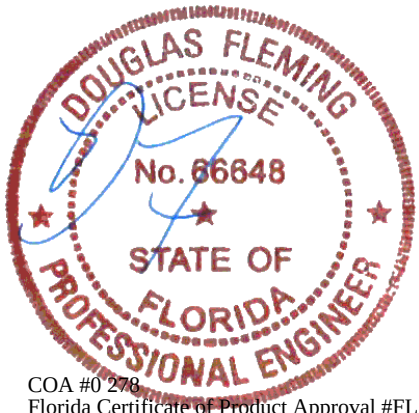
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	1	12"	6	Rigid Surface
2	21.792'	2	12"	14	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.



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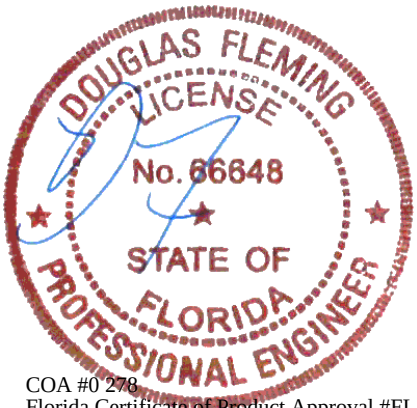
Glenview, IL 60025

Blocking

Apply additional nailing over the following bearings
with fasteners at 4" oc both perpendicular and
parallel to grain. In lieu of additional nailing,
apply blocking reinforcement to
prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)



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08/19/2022

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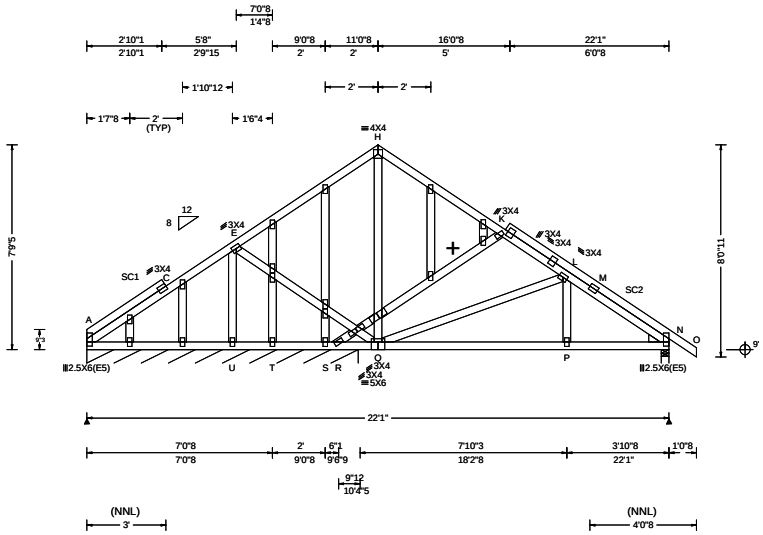
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99998 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B07		Cust: R 215 JRef:1XI72150016 T5 DrwNo: 231.22.1326.29323 KD / DF 08/19/2022	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.023 I 999 240 VERT(CL): 0.047 I 999 180 HORZ(LL): -0.006 I - - HORZ(TL): 0.012 I - - Creep Factor: 2.0 Max TC CSI: 0.209 Max BC CSI: 0.427 Max Web CSI: 0.337 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A*	124	/-	/-	/98	/-	/15
N	647	/-	/-	/387	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 123 Min Req = -
N Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
K - L	25	- 445	M - N	0	- 812
L - M	0	- 709			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
Q - P	621	0	P - N	625	0

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
U - E	0	- 436

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

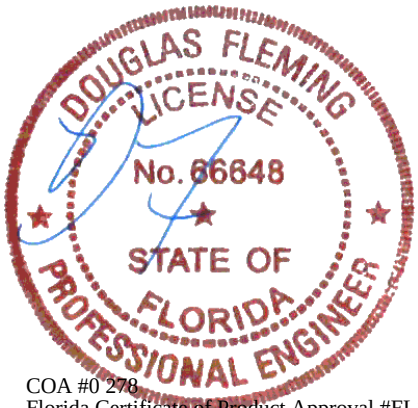
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

+ Member to be laterally braced for horizontal wind loads. bracing system to be desiged and furnished by others.



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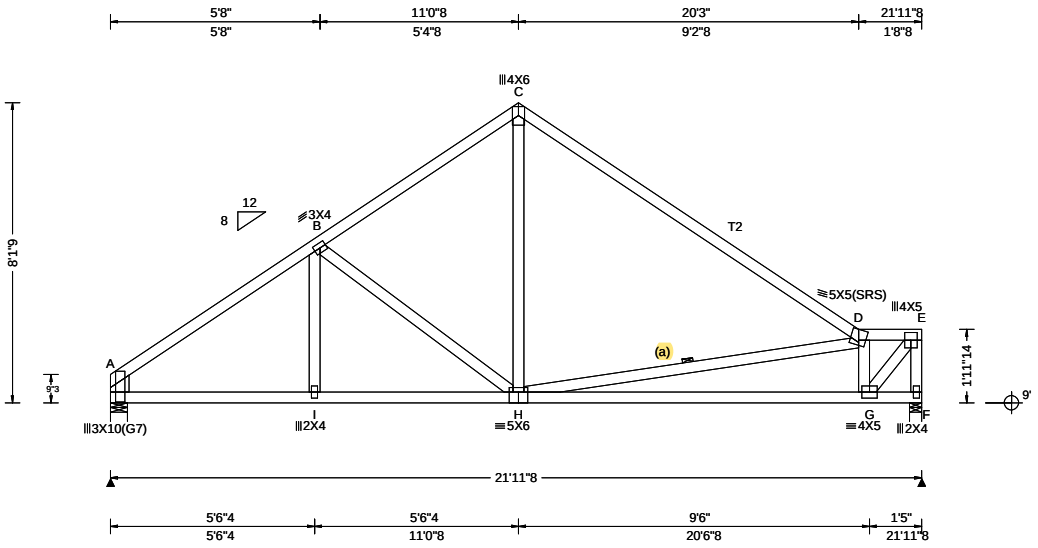
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99776 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B08	Cust: R 215 JRef:1XI72150016 T13 DrwNo: 231.22.1326.30887 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.032	H	999	240
VERT(CL): 0.067	H	999	180
HORZ(LL): 0.016	G	-	-
HORZ(TL): 0.034	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.663
Max BC CSI: 0.756
Max Web CSI: 0.563

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	923	/-	/-	/516	/-	/116
F	923	/-	/-	/494	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.5 Min Req = 1.5 (Truss)
F Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings A & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	114	-1250	C - D	130	-1013
B - C	150	-923	D - E	91	-948

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - I	945	-70	H - G	1110	-128
I - H	943	-70			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	523	0	G - E	1477	-141
H - D	137	-409	E - F	50	-986
D - G	222	-964			

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

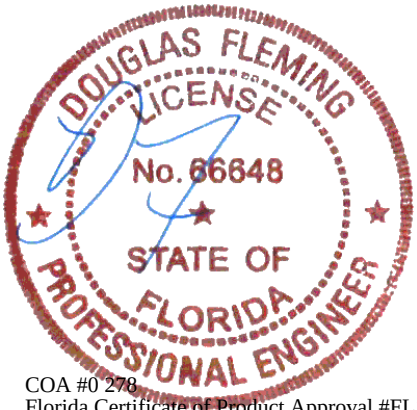
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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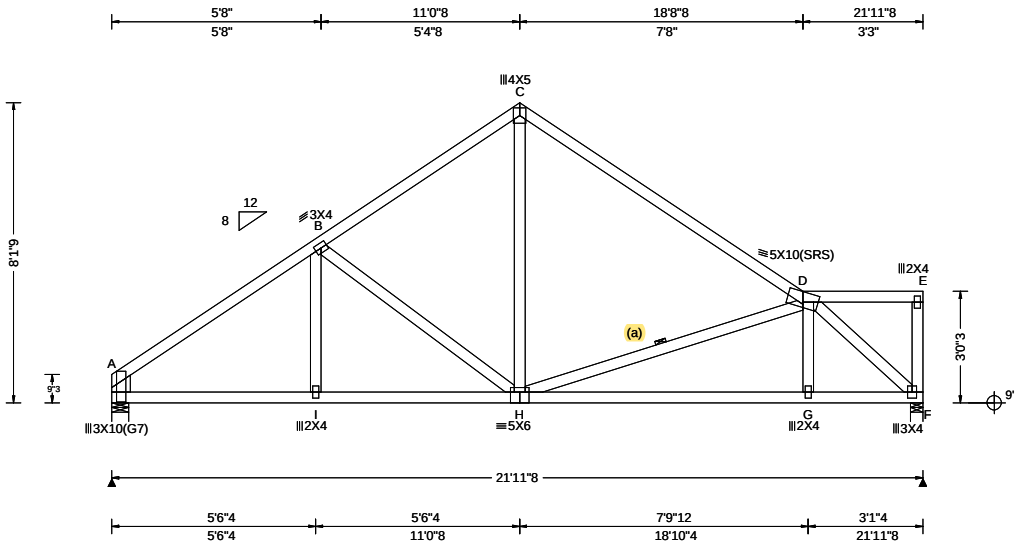
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99781 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B09		Cust: R 215 JRef:1XI72150016 T14 DrwNo: 231.22.1326.32440 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.035	H	999	240
VERT(CL): 0.073	H	999	180
HORZ(LL): 0.017	F	-	-
HORZ(TL): 0.035	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.670
Max BC CSI: 0.555
Max Web CSI: 0.447

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	923	/-	/-	/517	/-	/115
F	923	/-	/-	/482	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.5 Min Req = 1.5 (Truss)
F Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings A & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	24	-1255	C - D	67	-966
B - C	74	-916			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - I	950	-19	H - G	1044	-52
I - H	949	-20	G - F	1050	-48

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - H	531	0	D - F	61	-1367

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

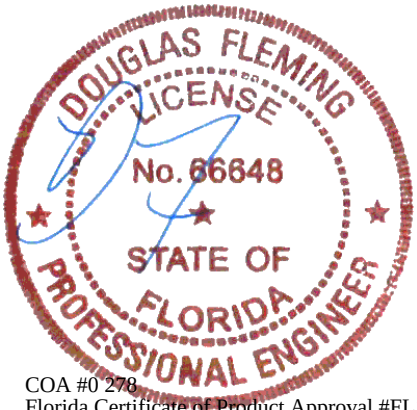
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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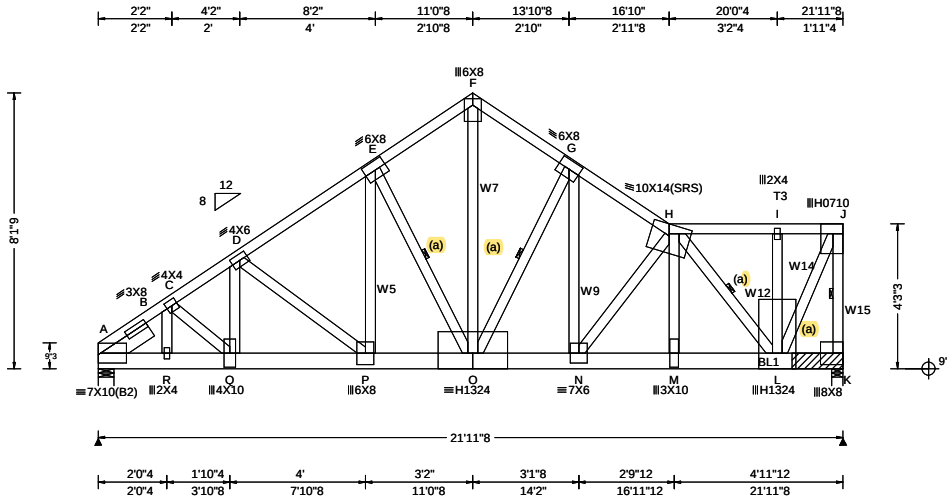
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100048 SPEC FROM: Page 1 of 2	Ply: 2 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** B10	Cust: R 215 JRef:1XI72150016 T31 DrwNo: 231.22.1326.43033 KD / DF 08/19/2022	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.181	N	999	240
VERT(CL): 0.360	N	724	180
HORZ(LL): 0.062	E	-	-
HORZ(TL): 0.124	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.608
Max BC CSI: 0.802
Max Web CSI: 0.870

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	12954	/-	/-	/345	/-	/-
K	12775	/-	/-	/341	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.5 Min Req = 5.4 (Truss)
K Brg Wid = 4.0 Min Req = -
Bearings A & K are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	0	-8394	F - G	0	-5961
B - C	0	-8321	G - H	0	-7420
C - D	0	-8660	H - I	0	-3159
D - E	0	-7489	I - J	0	-3158
E - F	0	-5962			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - R	6502	0	O - N	6060	0
R - Q	6431	0	N - M	6946	0
Q - P	7171	0	M - L	7008	0
P - O	6083	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - Q	1154	0	G - N	2910	0
Q - D	1251	0	N - H	0	-1377
D - P	0	-1263	H - M	1647	0
P - E	2960	0	H - L	0	-6062
E - O	0	-2437	L - J	7070	0
F - O	6400	0	J - K	0	-6355
O - G	0	-2424			

Lumber

Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5,W9,W12,W15 2x4 SP #2; W7, W14 2x4 SP M-31;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.605'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 3.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at 0.00 to 64 plf at 21.96
BC: From 10 plf at 0.00 to 10 plf at 21.96
BC: 2410 lb Conc. Load at 2.02, 4.02, 6.02, 8.02 10.02,12.02,14.02,16.02,18.02,20.02

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

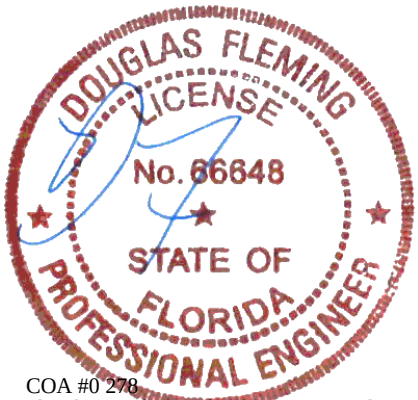
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks:0.131"x3", min. nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	21.625'	1	18"	24	Rigid Surface

Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.



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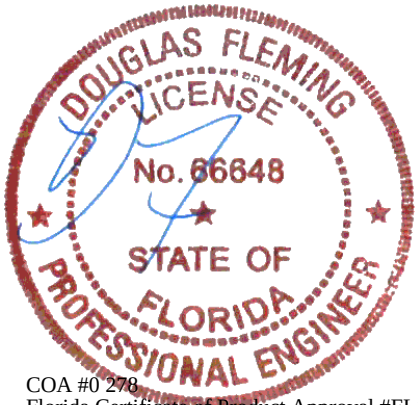
Blocking

Apply additional nailing over the following bearings
with fasteners at 4" oc both perpendicular and
parallel to grain. In lieu of additional nailing,
apply blocking reinforcement to
prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)



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08/19/2022

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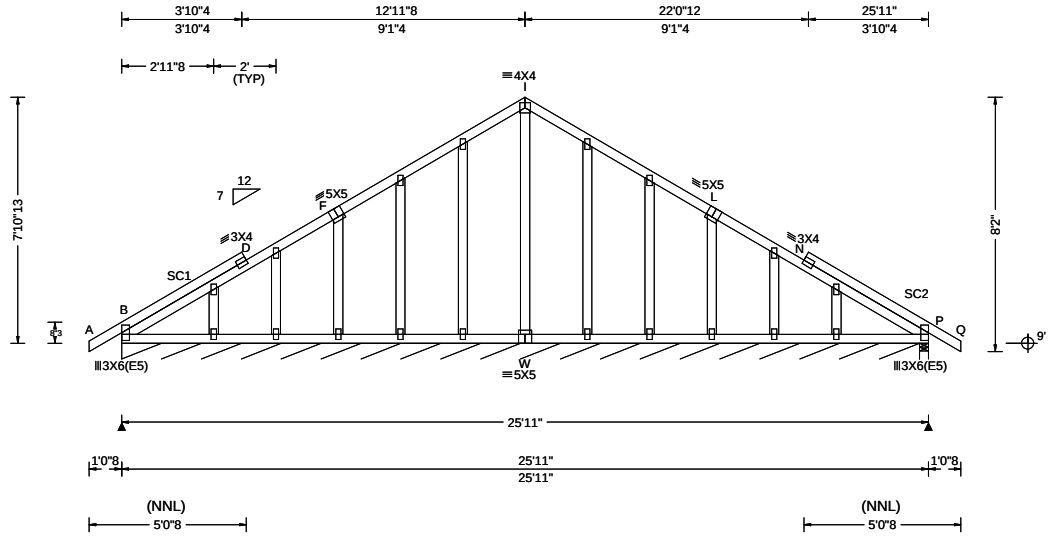
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99768 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** C01	Cust: R 215 JRef:1XI72150016 T15 DrwNo: 231.22.1326.44767 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.003	N	999	240
VERT(CL): 0.006	N	999	180
HORZ(LL): 0.002	N	-	-
HORZ(TL): 0.003	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.293
Max BC CSI: 0.056
Max Web CSI: 0.116

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	80	/-	/-	/42	/-	/5
P	231	/-	/-	/121	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 307 Min Req = -
P Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & P are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

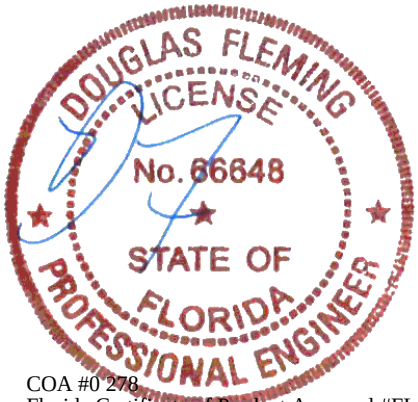
Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



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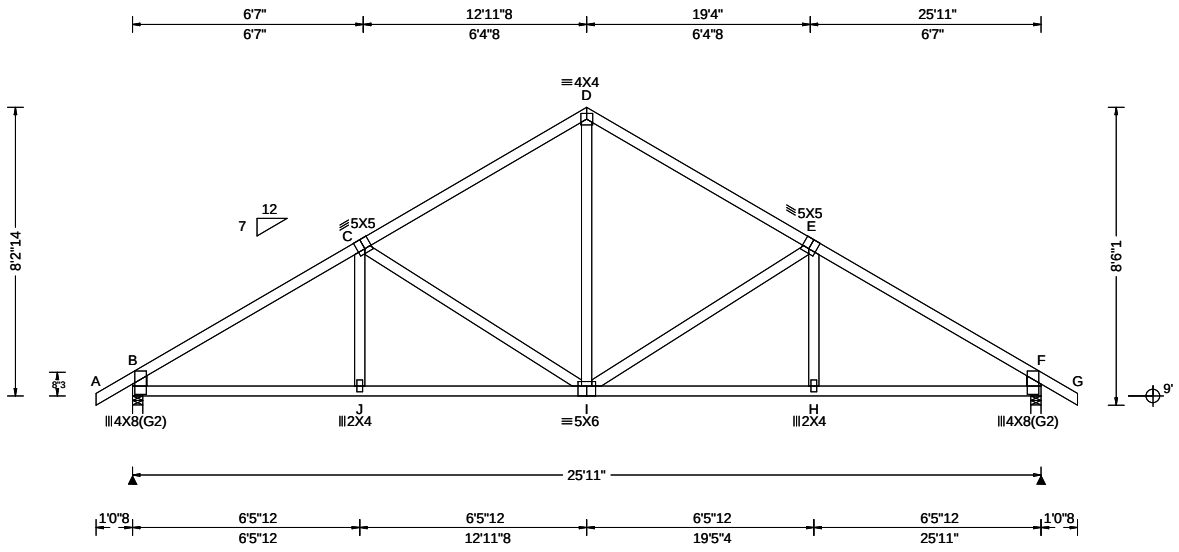
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99764 COMN FROM:	Ply: 1 Qty: 6	**Job Number:** 21-5560 Miller Res **Truss Label:** C02	Cust: R 215 JRef:1XI72150016 T28 DrwNo: 231.22.1326.46440 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.053 I	999	240
VERT(CL):	0.108 I	999	180
HORZ(LL):	0.026 F	-	-
HORZ(TL):	0.053 F	-	-

Creep Factor: 2.0
Max TC CSI: 0.438
Max BC CSI: 0.479
Max Web CSI: 0.546

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1143	/-	/-	/636	/-	/130
F	1143	/-	/-	/636	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Truss)
F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	111	-1641	D - E	143	-1157
C - D	142	-1157	E - F	111	-1641

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	1319	-13	I - H	1316	-18
J - I	1316	-14	H - F	1319	-18

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - I	91	-486	I - E	91	-486
D - I	664	-32			

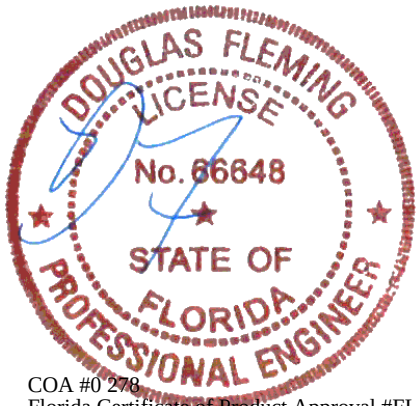
Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;Rt Stub Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



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08/19/2022

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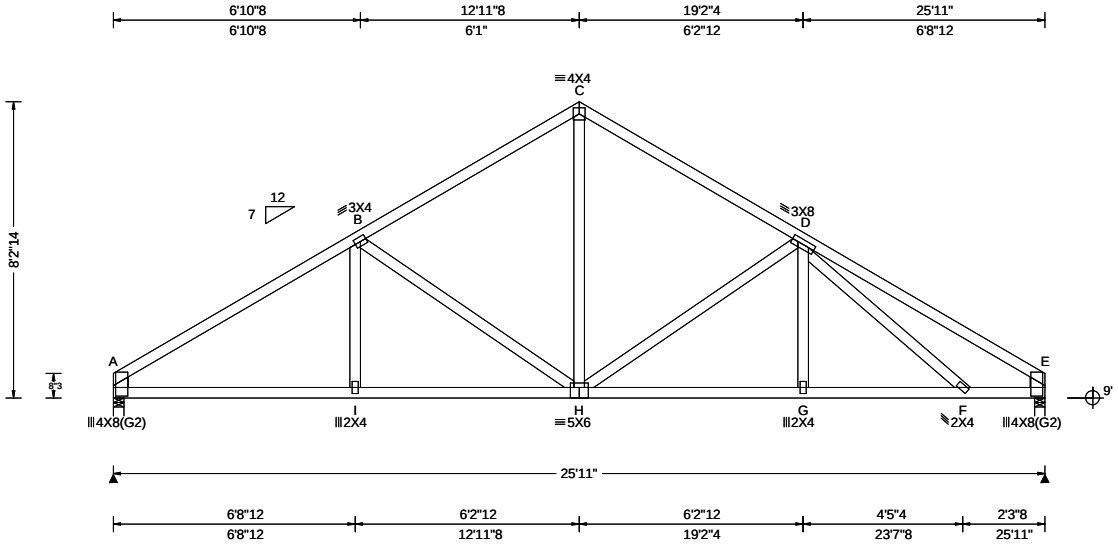
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100111 COMN FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** C03	Cust: R 215 JRef:1XI72150016 T42 DrwNo: 231.22.1326.58870 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.049	H	999	240
VERT(CL): 0.102	H	999	180
HORZ(LL): 0.019	E	-	-
HORZ(TL): 0.039	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.552
Max BC CSI: 0.214
Max Web CSI: 0.616

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	1098	/-	/-	/14	/-	/-
E	1290	/-	/-	/9	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Truss)
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings A & E are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	0	-1679	C - D	1	-1196
B - C	0	-1194	D - E	0	-1942

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - I	1350	0	G - F	1411	0
I - H	1347	0	F - E	1580	0
H - G	1413	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	0	-494	H - D	0	-567
C - H	725	0			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;Rt Stub Wedge: 2x4 SP #3;

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

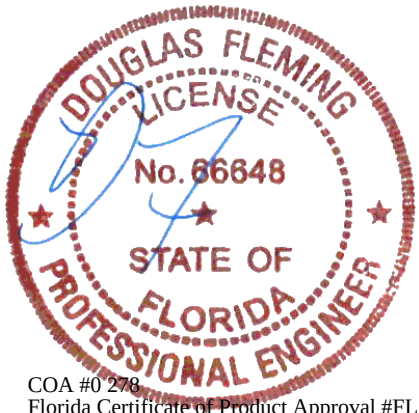
TC: From	63 plf at	0.00 to	63 plf at	25.92	
BC: From	20 plf at	0.00 to	20 plf at	25.92	
BC:	233 lb Conc. Load at 23.69				

Wind

Wind loads and reactions based on MWFRS.

Wind loading based on both gable and hip roof types.

THIS TRUSS MUST BE INSTALLED AS SHOWN
AND NOT END FOR END.



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08/19/2022

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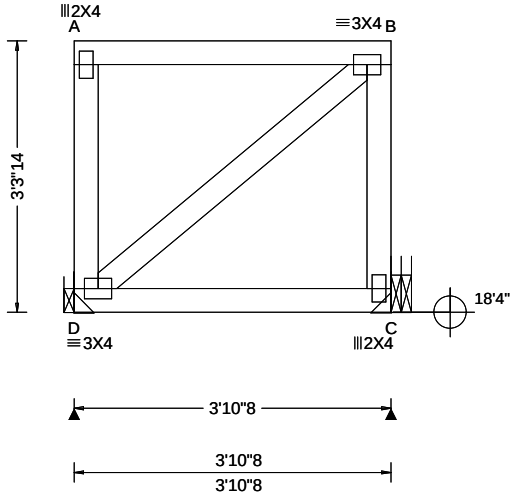
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100132 FLAT FROM:	Ply: 2 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** D01	Cust: R 215 JRef:1XI72150016 T39 DrwNo: 231.22.1327.35063 KD / DF 08/19/2022	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 20.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 40.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 21.66 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	240
VERT(CL): 0.003	B	999	180
HORZ(LL): 0.001	A	-	-
HORZ(TL): 0.002	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.488
Max BC CSI: 0.043
Max Web CSI: 0.205

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	1062	/-	/-	/79	/83	/-
C	1123	/-	/-	/79	/88	/-

Wind reactions based on MWFRS
D Brg Wid = - Min Req = -
C Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - D	69	-512	B - C	69	-542

Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Nailnote
Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @ 5.75" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

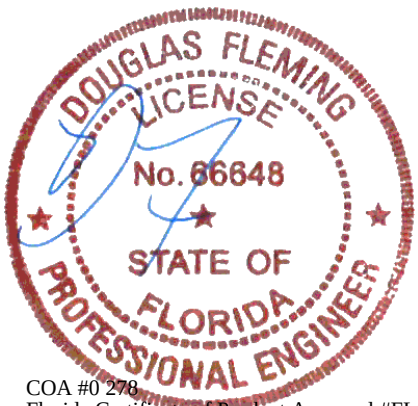
TC: From	60 plf at	0.13 to	60 plf at	0.25
TC: From	560 plf at	0.25 to	560 plf at	4.00
BC: From	20 plf at	0.13 to	20 plf at	4.00

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind
Wind loads based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes
Truss must be installed as shown with top chord up.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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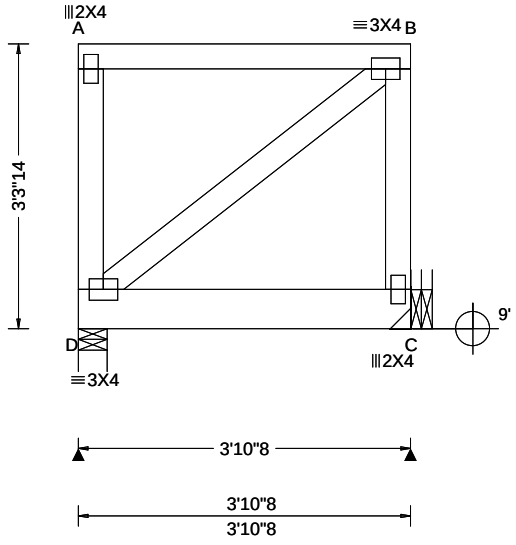
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100126 FLAT FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** D02	Cust: R 215 JRef:1XI72150016 T23 DrwNo: 231.22.1328.05960 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.000 A 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 A 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.000 A - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 A - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.263
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.372
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.063
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 10.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	567	/-	/-	/-	/46	/-
C	567	/-	/-	/-	/46	/-

Wind reactions based on MWFRS
D Brg Wid = 4.0 Min Req = 1.5 (Truss)
C Brg Wid = - Min Req = -
Bearing D is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads

------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 3.88
BC: From 20 plf at 0.00 to 20 plf at 3.88
BC: 824 lb Conc. Load at 1.94

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

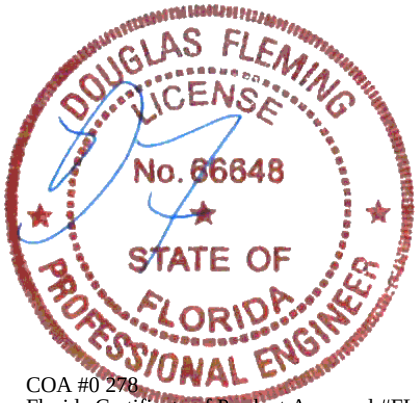
Wind

Wind loads and reactions based on MWFRS.

End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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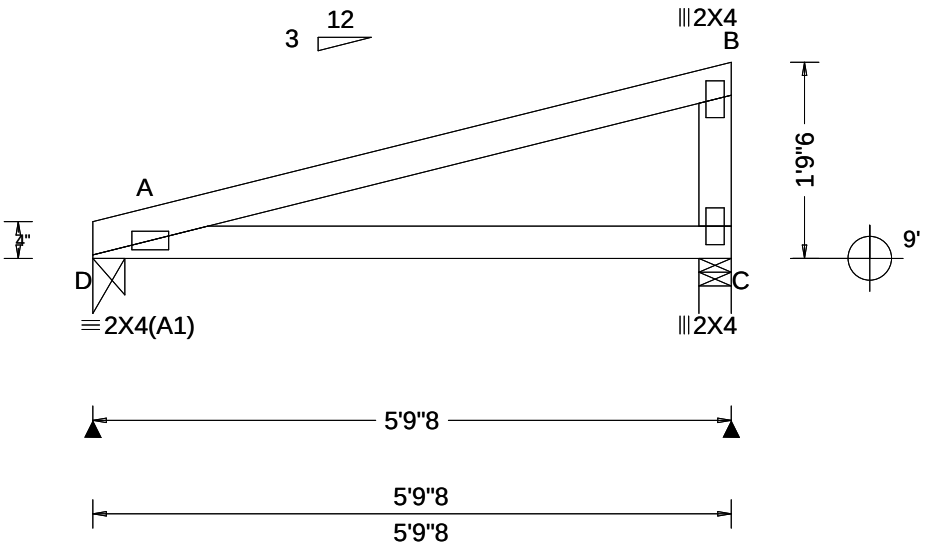
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99785 MONO FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** E01	Cust: R 215 JRef:1XI72150016 T20 DrwNo: 231.22.1328.17657 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	NA		
VERT(CL):	NA		
HORZ(LL):	0.009 A	-	-
HORZ(TL):	0.017 A	-	-

Creep Factor: 2.0
Max TC CSI: 0.415
Max BC CSI: 0.331
Max Web CSI: 0.125

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
D	239	/-	/-	/122	/-	/25
C	227	/-	/-	/116	/-	/-

Wind reactions based on MWFRS
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
C Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings D & C are a rigid surface.
Members not listed have forces less than 375#

Lumber

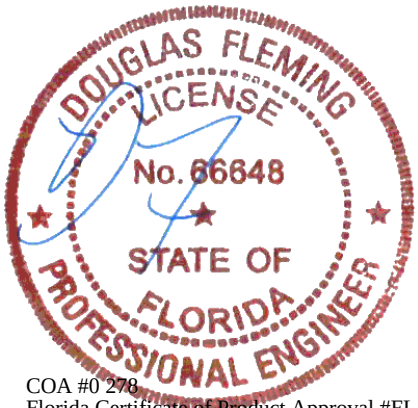
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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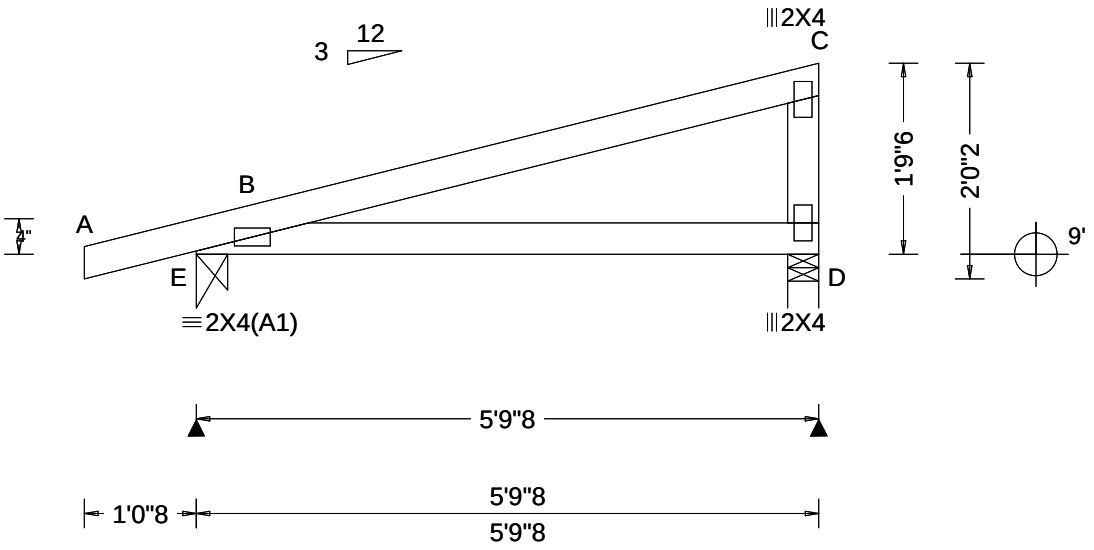
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99783 MONO FROM:	Ply: 1 Qty: 13	**Job Number:** 21-5560 Miller Res **Truss Label:** E02	Cust: R 215 JRef:1XI72150016 T16 DrwNo: 231.22.1328.18903 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): NA
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): NA
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.008 B - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.015 B - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.394
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.297
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.118
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	310	/-	/-	/160	/8	/32
D	220	/-	/-	/114	/-	/-

Wind reactions based on MWFRS
E Brg Wid = 3.5 Min Req = 1.5 (Truss)
D Brg Wid = 3.5 Min Req = 1.5 (Truss)
Bearings E & D are a rigid surface.
Members not listed have forces less than 375#

Lumber

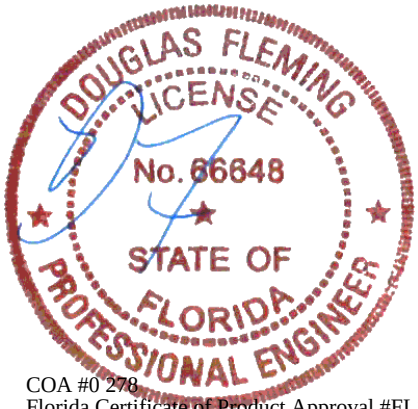
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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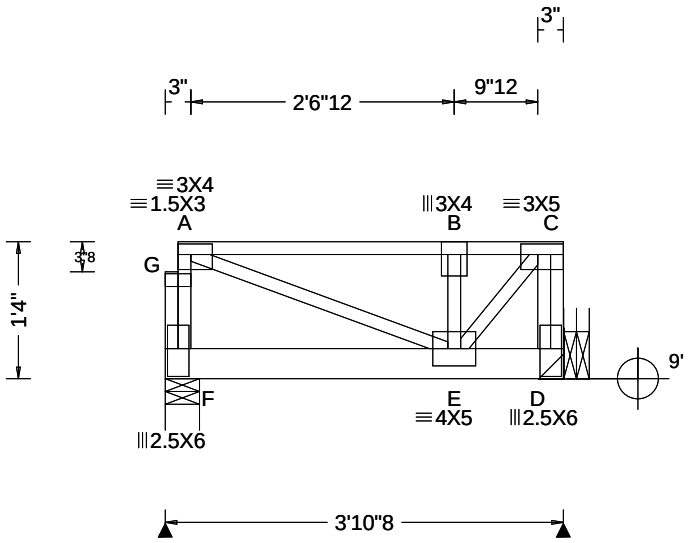
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100130 SY42 FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** F01	Cust: R 215 JRef:1XI72150016 T3 DrwNo: 231.22.1328.26237 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	40.00
TCDL:	10.00
BCLL:	0.00
BCDL:	5.00
Des Ld:	55.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.00
Spacing: 24.0 "

Wind Criteria
Wind Std: NA
Speed: NA mph
Enclosure: NA
Category: NA
EXP: NA Kzt: NA
Mean Height: NA ft
TCDL: NA psf
BCDL: NA psf
MWFRS Parallel Dist: NA
C&C Dist a: NA ft
Loc. from endwall: NA
I: NA GCpi: NA
Wind Duration: NA

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:12(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.015	B	999	480
VERT(CL): 0.021	B	999	360
HORZ(LL): -0.004	C	-	-
HORZ(TL): 0.006	C	-	-

Creep Factor: 2.0
Max TC CSI: 0.697
Max BC CSI: 0.160
Max Web CSI: 0.296

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
F	483 /-	/-	/-	/-	/-
D	475 /-	/0	/-	/-	/-

F Brg Wid = 4.0 Min Req = 1.5 (Truss)
D Brg Wid = - Min Req = -
Bearing F is a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
G - F	0	-413	B - E	0	-894
A - G	0	-402	E - C	621	0
A - E	393	0			

Lumber

Top chord: 4x2 SP M-31;
Bot chord: 4x4 SP #2;
Webs: 4x2 SP #3;

Special Loads

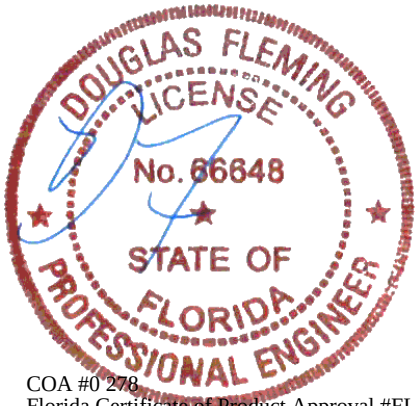
------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 100 plf at 0.12 to 100 plf at 3.88
BC: From 10 plf at 0.00 to 10 plf at 3.88
TC: 544 lb Conc. Load at 1.81

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
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08/19/2022

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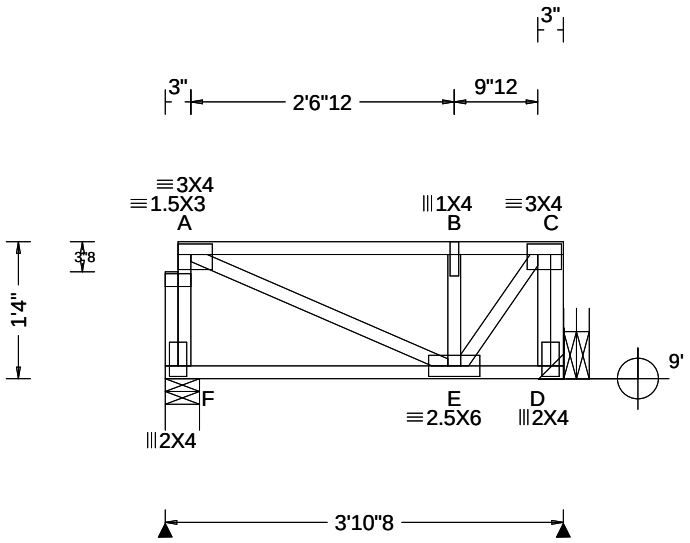
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100128 SY42 FROM:	Ply: 1 Qty: 4	**Job Number:** 21-5560 Miller Res **Truss Label:** F02		Cust: R 215 JRef:1XI72150016 T27 DrwNo: 231.22.1328.28177 KD / DF 08/19/2022	



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 B 999 480 VERT(CL): 0.009 B 999 360 HORZ(LL): -0.002 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.095 Max Web CSI: 0.124 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	196	/-	/-	/-	/-	/-
D	217	/-	/-	/-	/-	/-

F Brg Wid = 4.0 Min Req = 1.5 (Truss)
D Brg Wid = - Min Req = -
Bearing F is a rigid surface.
Members not listed have forces less than 375#

Lumber

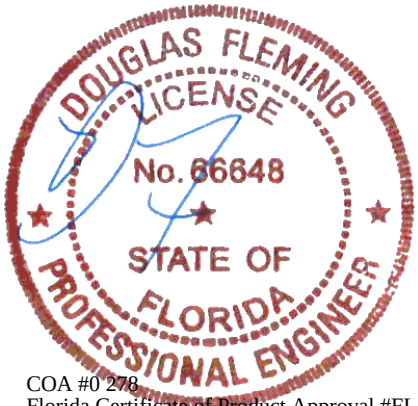
Top chord: 4x2 SP #2;
Bot chord: 4x2 SP #2;
Webs: 4x2 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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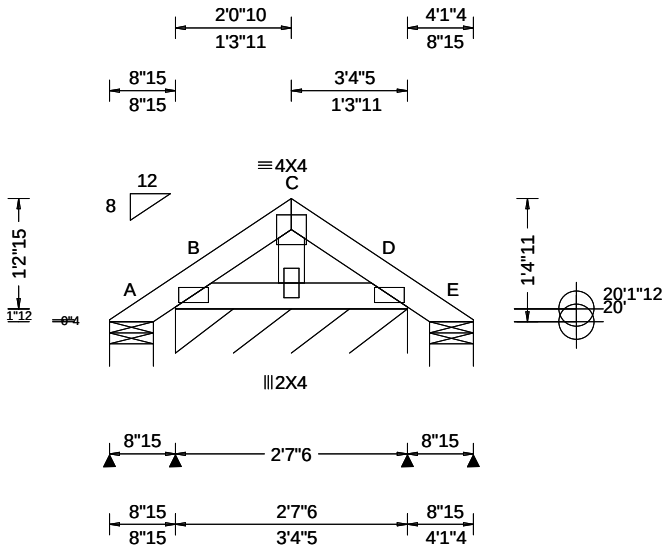
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100078 GABL FROM:	Ply: 1 Qty: 2	**Job Number:** 21-5560 Miller Res **Truss Label:** PB01	Cust: R 215 JRef:1XI72150016 T21 DrwNo: 231.22.1328.31323 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.70 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	D	999	240
VERT(CL): 0.000	D	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.013
Max BC CSI: 0.009
Max Web CSI: 0.008

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	17	/-	/-	/21	/7	/20
B*	82	/-	/-	/58	/5	/-
E	17	/-	/-	/14	/2	/-

Wind reactions based on MWFRS
A Brg Wid = 5.9 Min Req = 1.5 (Truss)
B Brg Wid = 31.4 Min Req = -
E Brg Wid = 5.9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

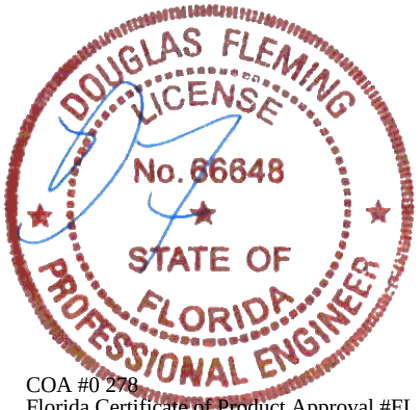
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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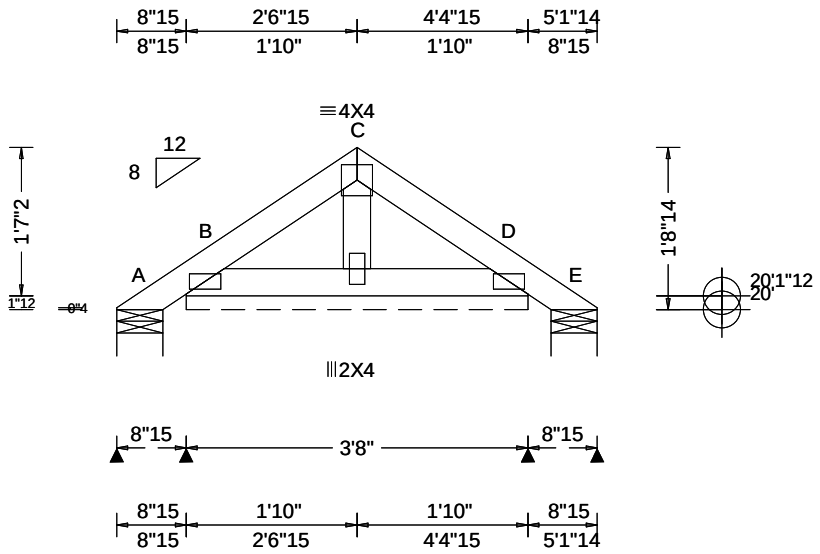
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100079 SPEC FROM:	Ply: 1 Qty: 32	**Job Number:** 21-5560 Miller Res **Truss Label:** PB02	Cust: R 215 JRef:1XI72150016 T32 DrwNo: 231.22.1328.33447 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.88 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.030
Max BC CSI: 0.016
Max Web CSI: 0.011

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	9	/-	/-	/22	/12	/25
B*	82	/-	/-	/57	/6	/-
E	9	/-	/-	/8	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 5.9 Min Req = 1.5 (Truss)
B Brg Wid = 44.0 Min Req = -
E Brg Wid = 5.9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

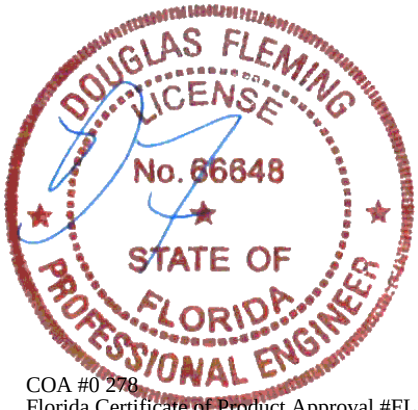
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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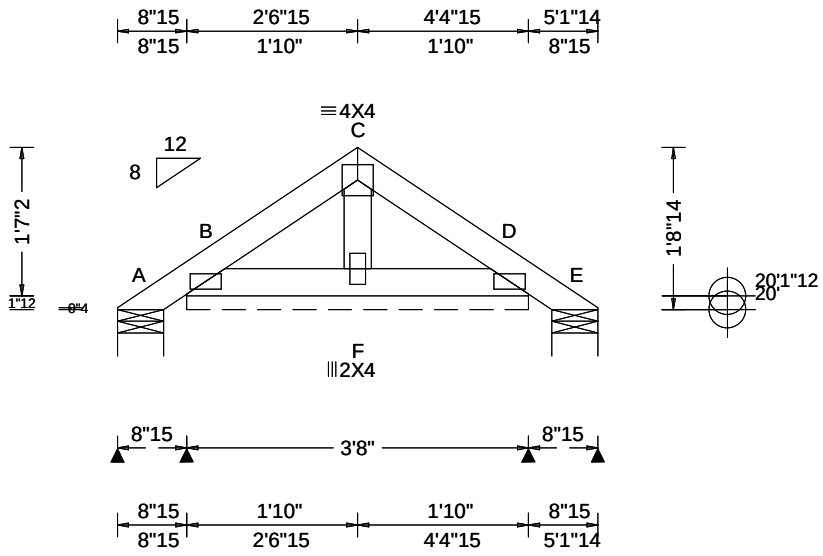
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100028 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** PB03	Cust: R 215 JRef:1XI72150016 T4 DrwNo: 231.22.1328.38877 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	0.00
Soffit:	0.00
Load Duration:	1.25
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 20.88 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.030
Max BC CSI: 0.016
Max Web CSI: 0.011

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	8	/-	/-	/17	/14	/27
B*	82	/-	/-	/55	/1	/-
E	8	/-	/-	/2	/0	/-

Wind reactions based on MWFRS
A Brg Wid = 5.9 Min Req = 1.5 (Truss)
B Brg Wid = 44.0 Min Req = -
E Brg Wid = 5.9 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

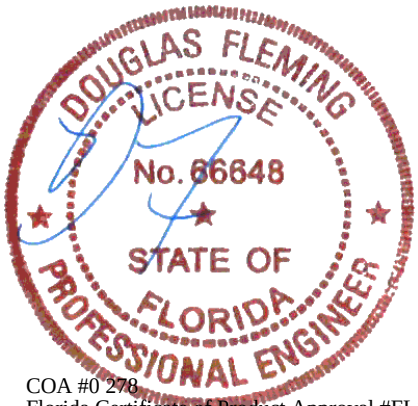
All plates are 2X4(A1) except as noted.

Wind

Wind loads based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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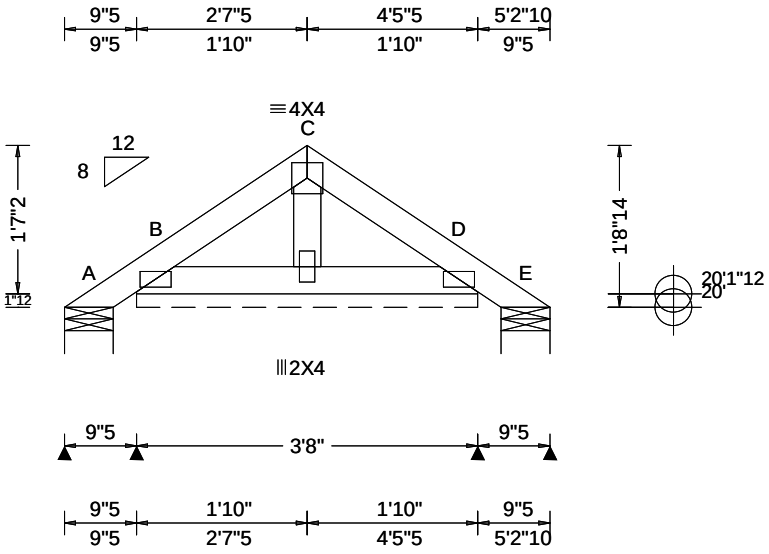
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100080 SPEC FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** PB04	Cust: R 215 JRef:1XI72150016 T44 DrwNo: 231.22.1328.59590 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 0.00 ft
TCDL: 5.0 psf
BCDL: 2.0 psf
MWFRS Parallel Dist: > 2h
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): 0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.030
Max BC CSI: 0.016
Max Web CSI: 0.011

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	9	/-	/-	/22	/12	/25
B*	82	/-	/-	/57	/6	/-
E	9	/-	/-	/8	/-	/-

Wind reactions based on MWFRS
A Brg Wid = 6.3 Min Req = 1.5 (Truss)
B Brg Wid = 44.0 Min Req = -
E Brg Wid = 6.3 Min Req = 1.5 (Truss)
Bearings A, B, & E are a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

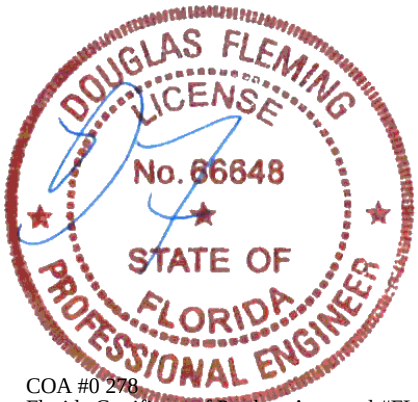
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



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08/19/2022

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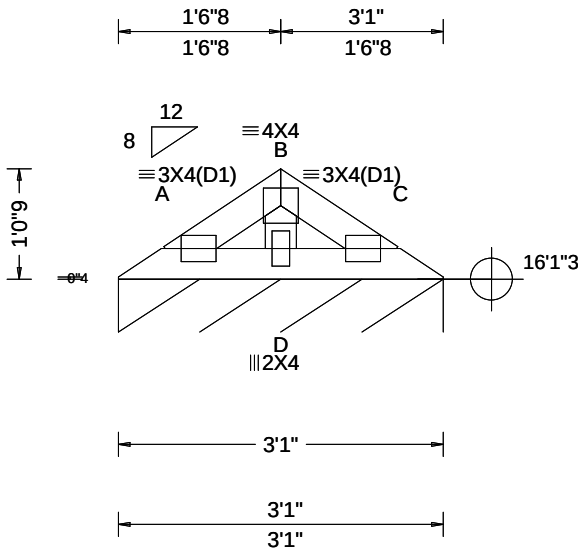
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99970 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V01	Cust: R 215 JRef:1XI72150016 T45 DrwNo: 231.22.1329.03483 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.78 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.019 Max BC CSI: 0.016 Max Web CSI: 0.015 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/38	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 37.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

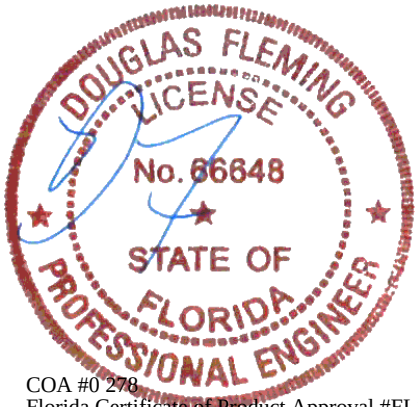
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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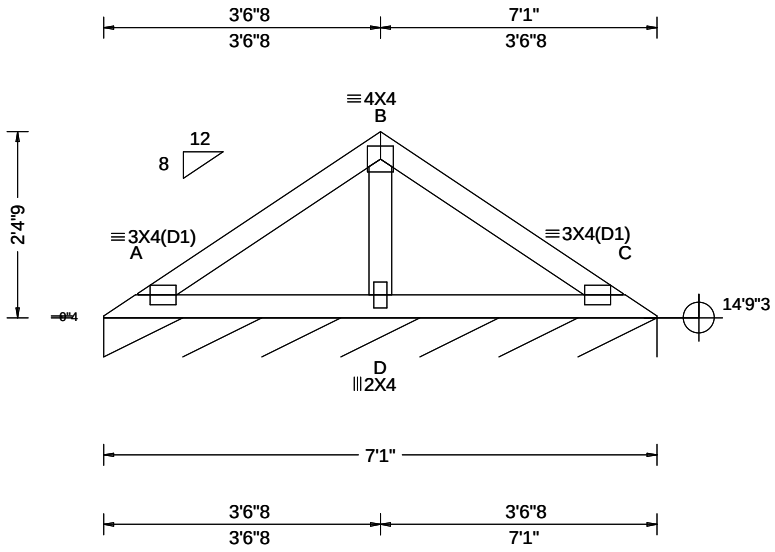
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99972 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V02	Cust: R 215 JRef:1XI72150016 T46 DrwNo: 231.22.1329.04660 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.11 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.009 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.154 Max BC CSI: 0.133 Max Web CSI: 0.062 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	84	/-	/-	/42	/-	/5

Wind reactions based on MWFRS
C Brg Wid = 85.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

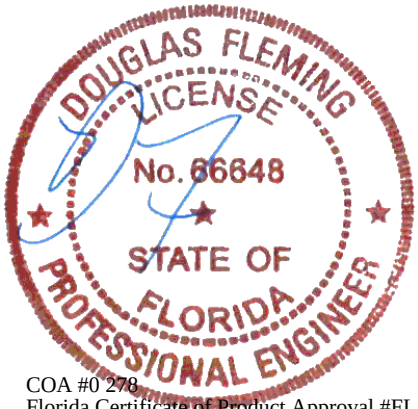
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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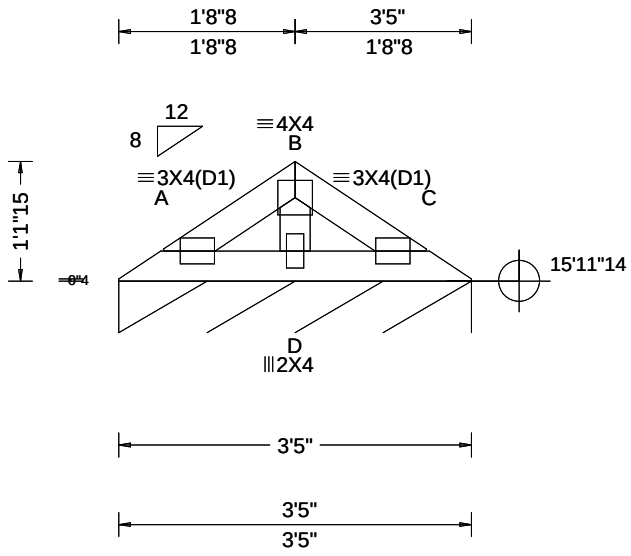
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99974 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V03	Cust: R 215 JRef:1XI72150016 T33 DrwNo: 231.22.1329.05777 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.72 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.026 Max BC CSI: 0.021 Max Web CSI: 0.018 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/39	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 41.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

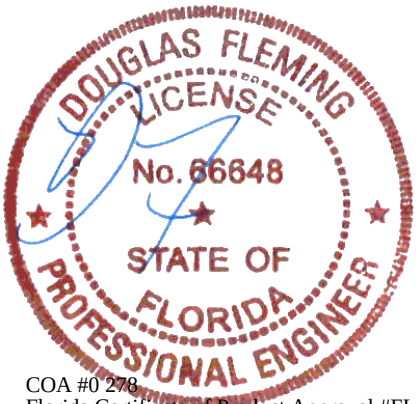
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
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08/19/2022

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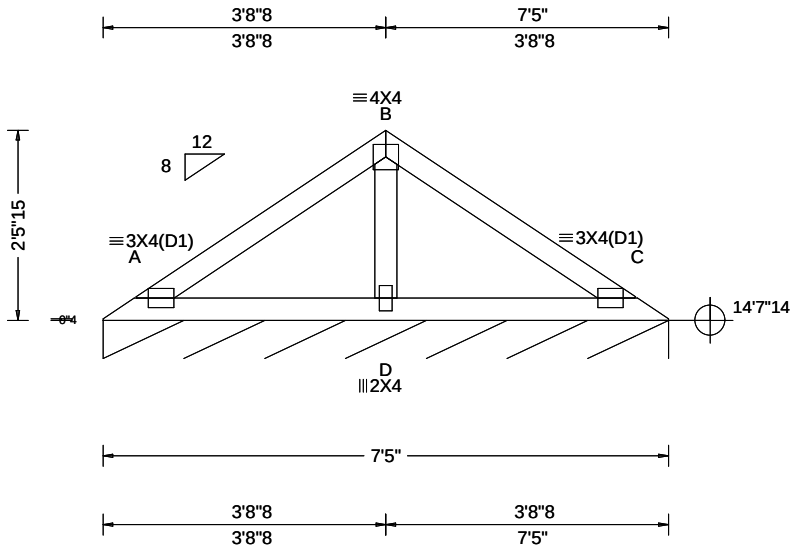
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99976 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V04	Cust: R 215 JRef:1XI72150016 T25 DrwNo: 231.22.1329.07077 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 16.06 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.005 A	999	240
VERT(CL):	0.010 A	999	180
HORZ(LL):	-0.003 C	-	-
HORZ(TL):	0.005 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.172
Max BC CSI: 0.148
Max Web CSI: 0.067

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	84	/-	/-	/42	/-	/5

Wind reactions based on MWFRS
C Brg Wid = 89.0 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.

COA #0 278
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08/19/2022

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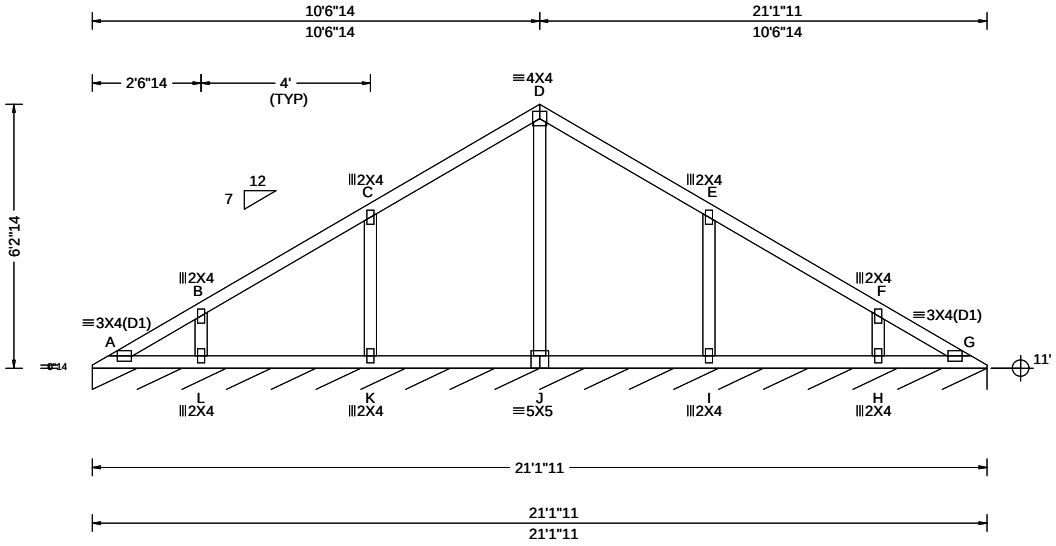
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99978 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V05		Cust: R 215 JRef:1XI72150016 T36 DrwNo: 231.22.1329.09333 KD / DF 08/19/2022	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	D	999	240
VERT(CL): 0.002	D	999	180
HORZ(LL): 0.001	E	-	-
HORZ(TL): 0.001	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.214
Max BC CSI: 0.109
Max Web CSI: 0.129

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	83	/-	/-	/43	/-	/4

Wind reactions based on MWFRS
G Brg Wid = 253 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

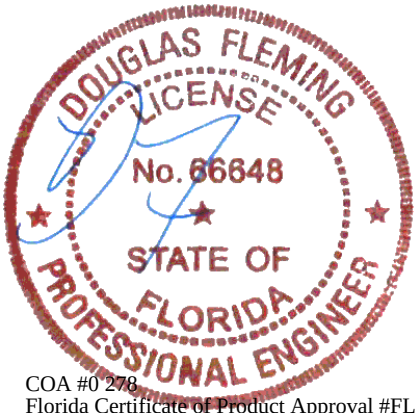
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
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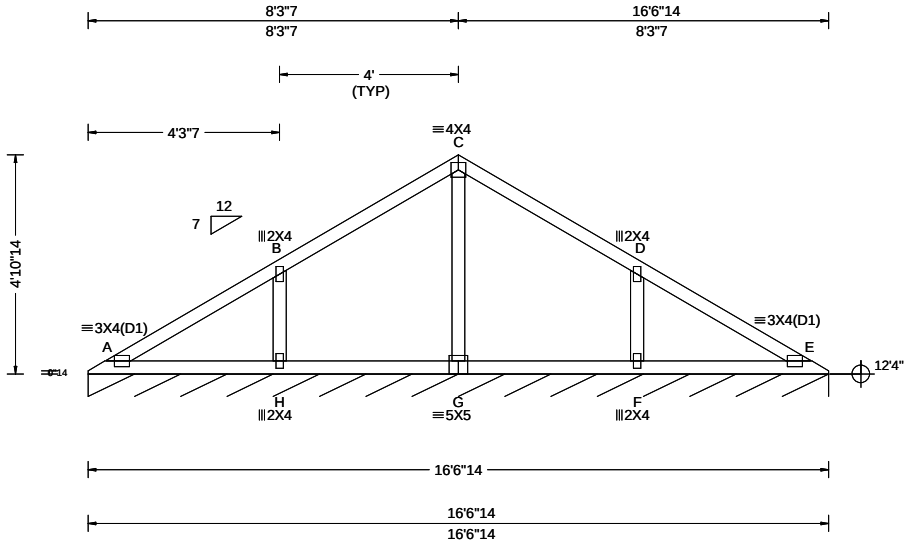
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99980 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V06	Cust: R 215 JRef:1XI72150016 T34 DrwNo: 231.22.1329.10680 KD / DF 08/19/2022



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 120 mph
Enclosure: Closed
Risk Category: II
EXP: B Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.006	E	999	240
VERT(CL): 0.012	E	999	180
HORZ(LL): -0.002	E	-	-
HORZ(TL): 0.005	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.305
Max BC CSI: 0.160
Max Web CSI: 0.106

VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	83	/-	/-	/43	/-	/4

Wind reactions based on MWFRS
E Brg Wid = 198 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

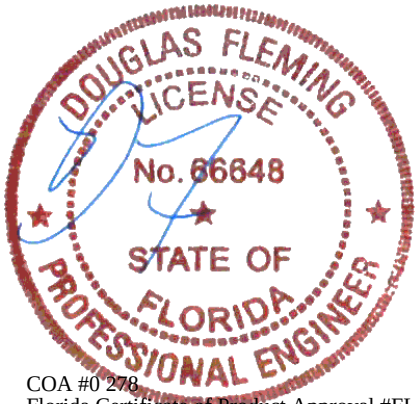
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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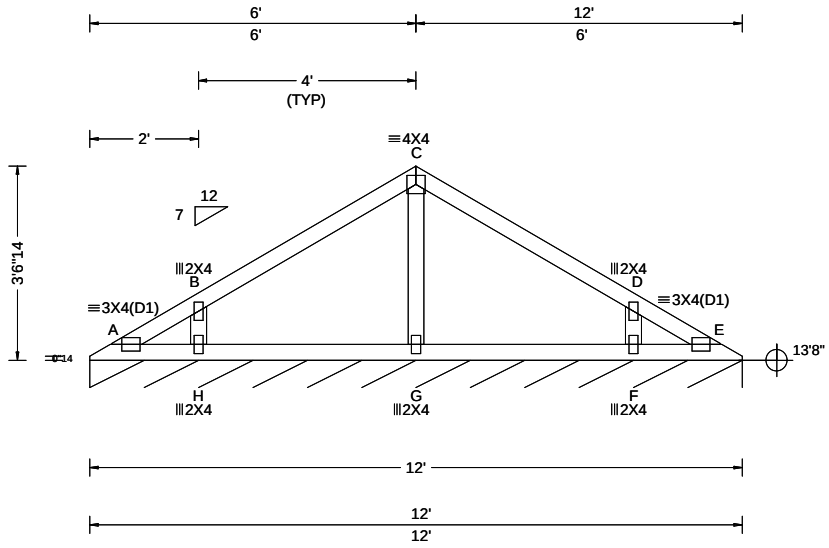
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99984 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V07	Cust: R 215 JRef:1XI72150016 T37 DrwNo: 231.22.1329.11980 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.001 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.000 A - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.001 A - -
NCBCLL: 10.00	Mean Height: 15.61 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.204
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.117
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.048
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	83	/-	/-	/42	/-	/4

Wind reactions based on MWFRS
E Brg Wid = 144 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

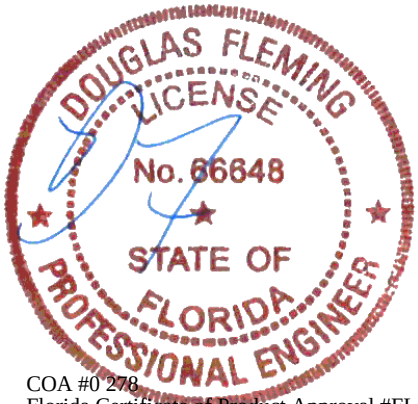
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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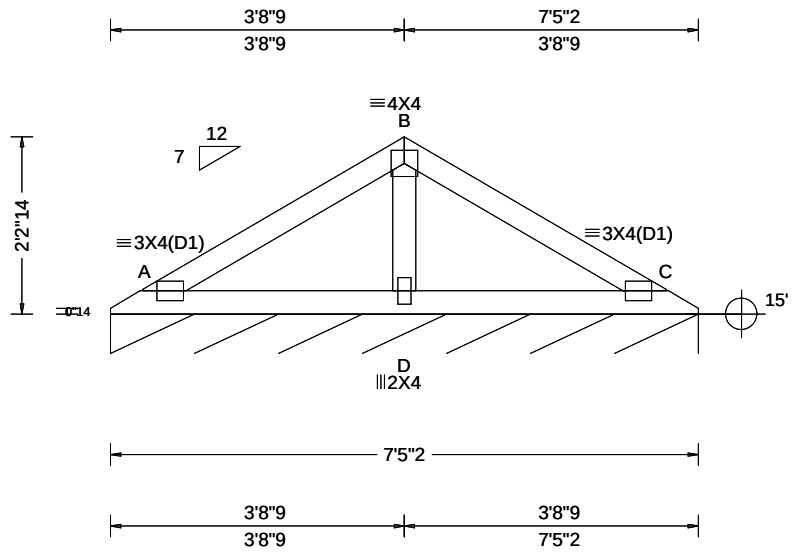
Glenview, IL 60025

SEQN: 99986 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V08	Cust: R 215 JRef:1XI72150016 T35 DrwNo: 231.22.1329.13857 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 120 mph	Pf: NA Ce: NA	VERT(LL): 0.005 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.011 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 C - -
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.005 C - -
NCBCLL: 10.00	Mean Height: 16.27 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.168
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.151
Spacing: 24.0 "	MWFRS Parallel Dist: h to 2h	Rep Fac: Yes	Max Web CSI: 0.059
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.00.1005.17
	Wind Duration: 1.60		

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	83	/-	/-	/41	/-	/4

Wind reactions based on MWFRS
C Brg Wid = 89.1 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

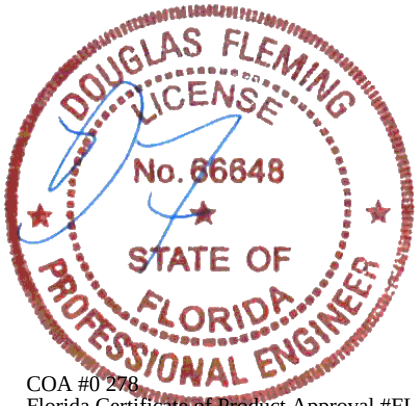
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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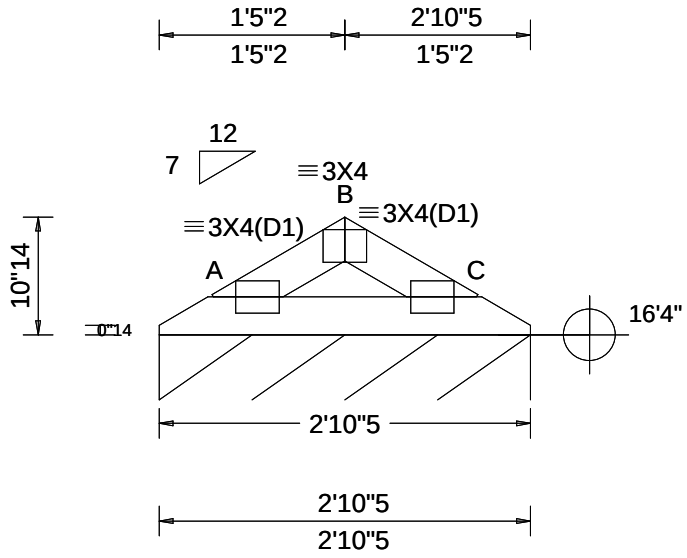
Glenview, IL 60025

SEQN: 99988 VAL FROM:	Ply: 1 Qty: 1	**Job Number:** 21-5560 Miller Res **Truss Label:** V09	Cust: R 215 JRef:1XI72150016 T30 DrwNo: 231.22.1329.16847 KD / DF 08/19/2022



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.94 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.002 A 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.033 Max BC CSI: 0.052 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
C*	82	/-	/-	/37	/-	/3

Wind reactions based on MWFRS
C Brg Wid = 34.3 Min Req = -
Bearing A is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

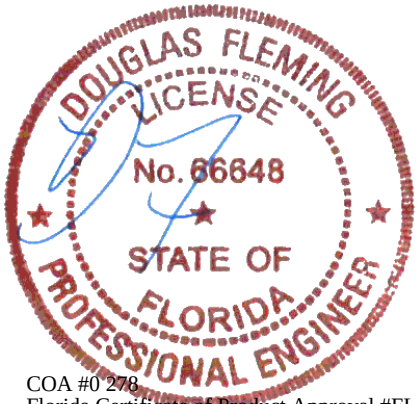
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	6' 2"	6' 7"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	11' 11"	12' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	7' 0"	7' 5"	9' 4"	10' 0"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

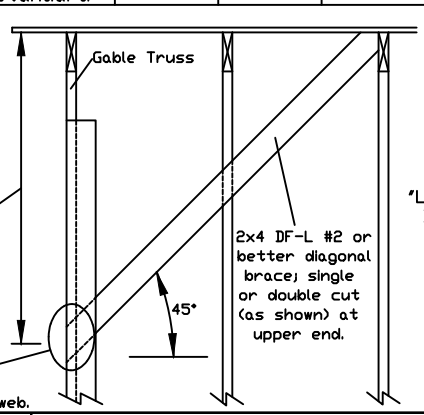
Attach 'L' braces with 10d (0.128"x3.0" min) nails.

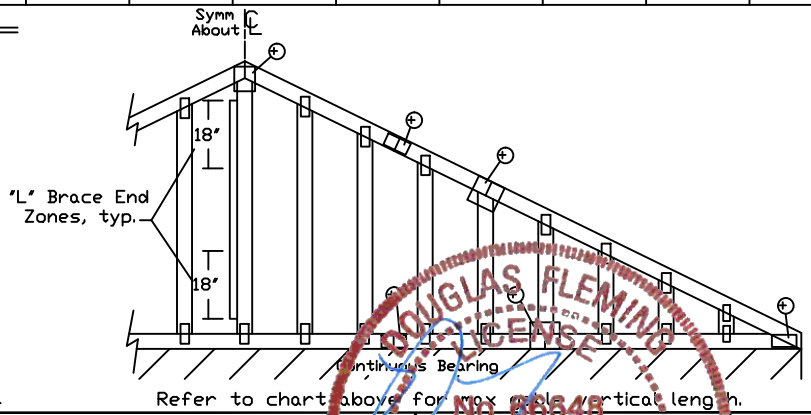
* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 385# at each end. Max web total length is 14'.





Refer to chart above for max gable vertical length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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LICENSE
No. 66548
STATE OF FLORIDA
PROFESSIONAL ENGINEER
08/19/2022
COA #0.278

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB12030
DATE	01/26/2018
DRWG	A12030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

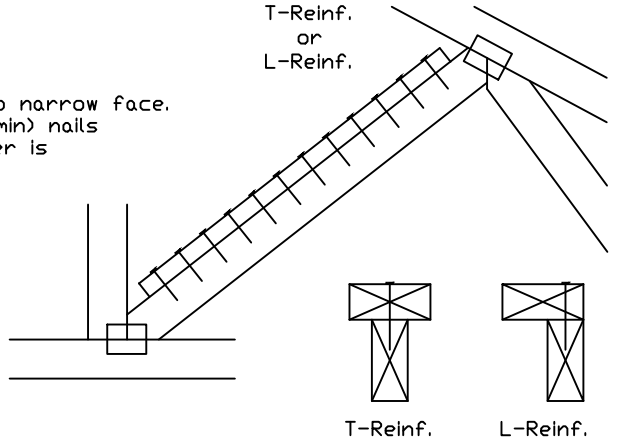
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

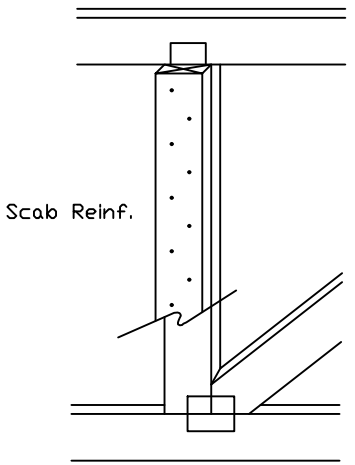
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

TC LL		PSF
TC DL		PSF
BC DL		PSF
BC LL		PSF
TOT. LD.		PSF
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

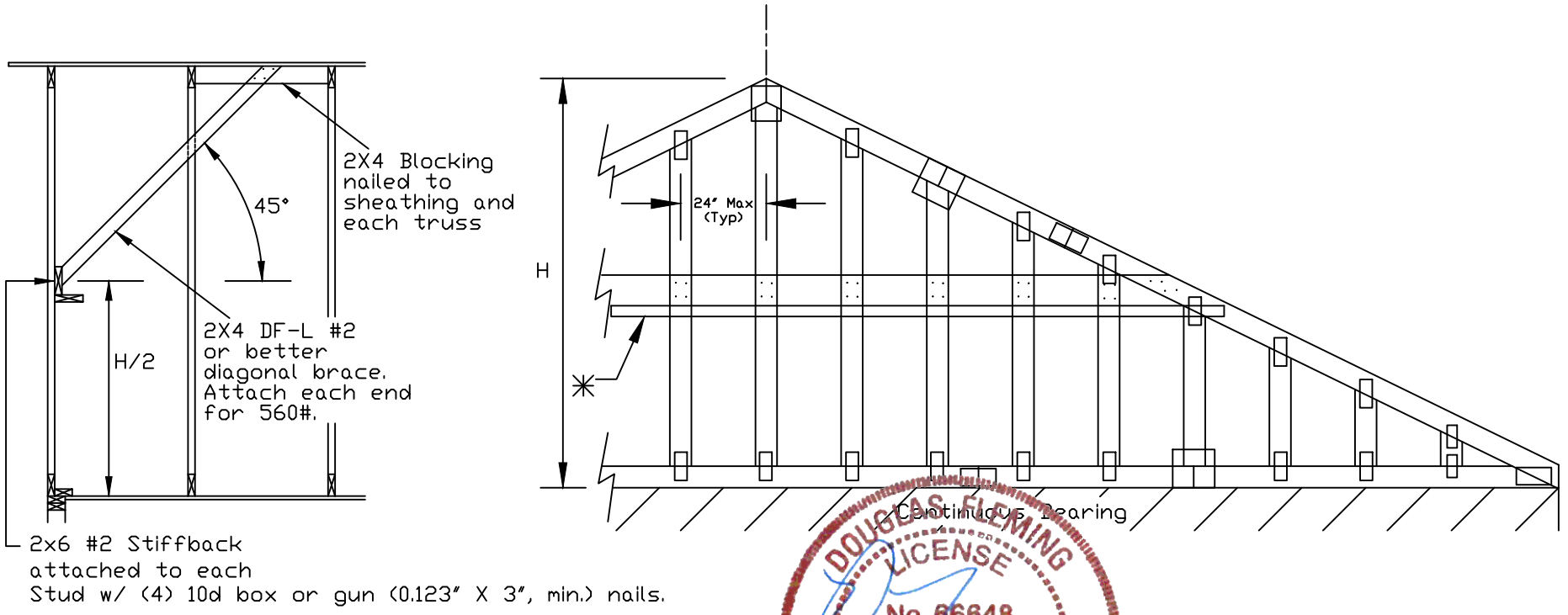
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08/19/2022

COA #0.278

ALPINE AN ITW COMPANY

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North Building, 4th Floor
Glenview, IL 60025

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviations from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

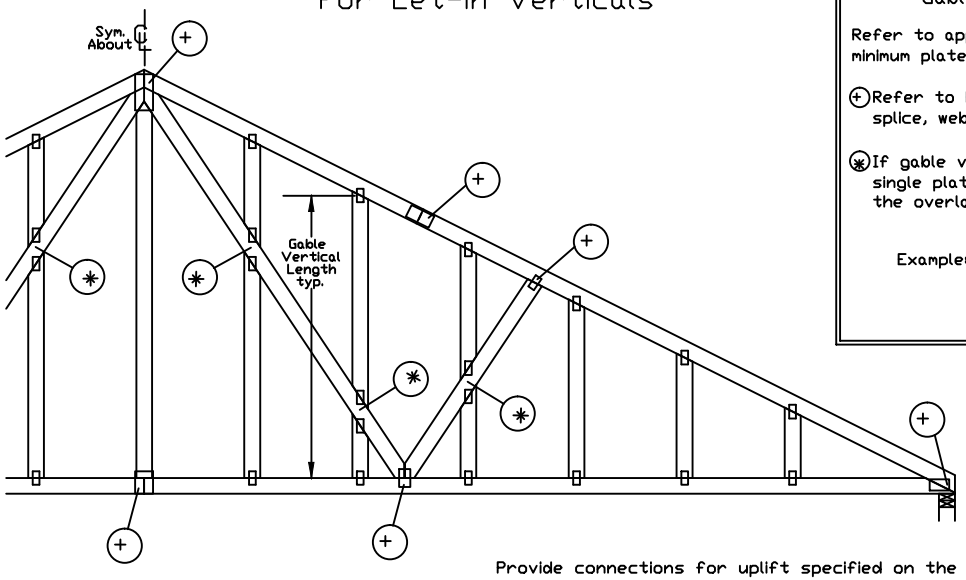
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	01/02/2018
DRWG	GABRST160118

Gable Detail
For Let-in Verticals



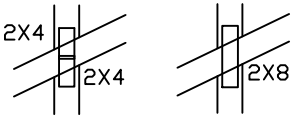
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

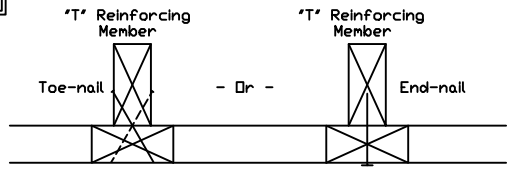
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

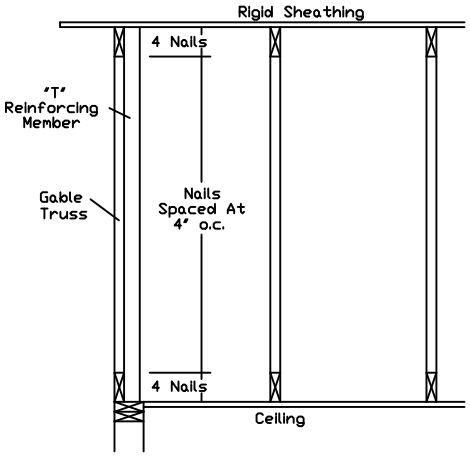
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
08/19/2022
COA #0.278

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	7' 8"	8' 2"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 7"	7' 0"	8' 10"	9' 5"	11' 5"	11' 10"	13' 10"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 0"	8' 4"	8' 7"	9' 10"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	8' 1"	8' 7"	10' 10"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 9"	9' 6"	9' 10"	11' 3"	11' 8"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	9' 4"	9' 11"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

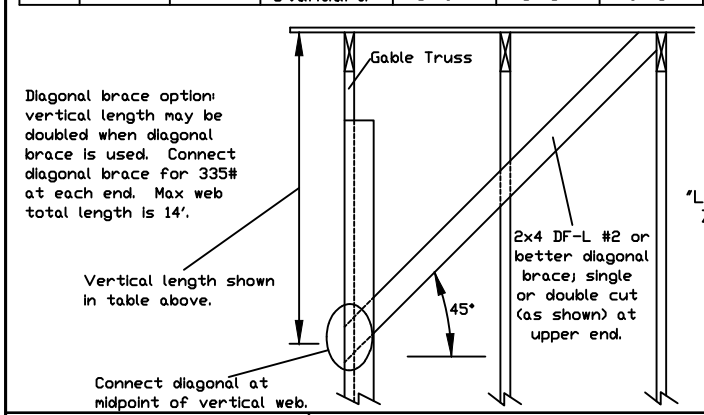
Attach 'L' braces with 10d (0.128"x3.0" min) nails.

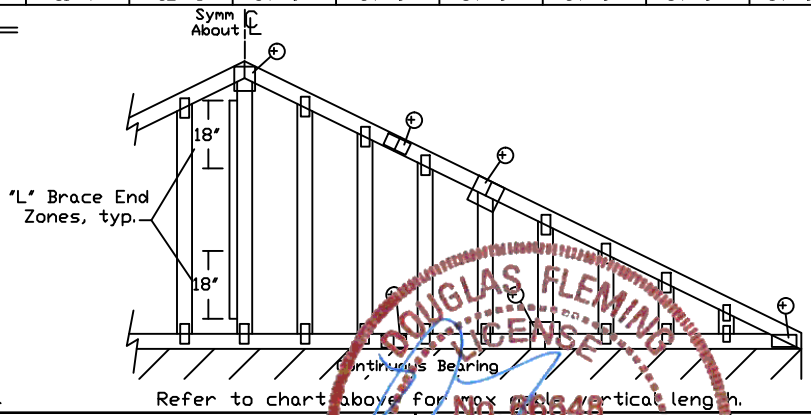
* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web total length is 14'.





Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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DOUGLAS FLEMING LICENSE NO. 66548 STATE OF FLORIDA PROFESSIONAL ENGINEER
08/10/2022
COA #0 278

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB12015
DATE	01/26/2018
DRWG	A12015ENC160118

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

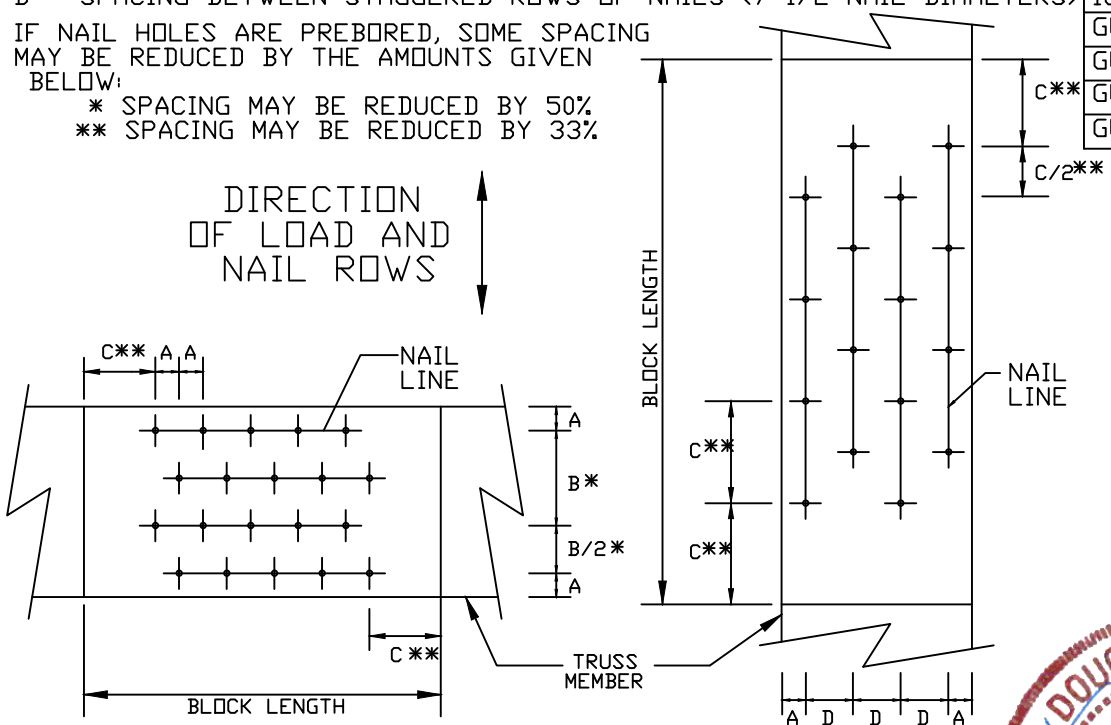
LOAD PERPENDICULAR TO GRAIN
A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
A - EDGE DISTANCE (6 NAIL DIAMETERS)
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
* SPACING MAY BE REDUCED BY 50%
** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08/19/2022

C.O.A #0 278

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

REF	NAIL SPACE
DATE	10/01/14
DRWG	CNNAILSP1014

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

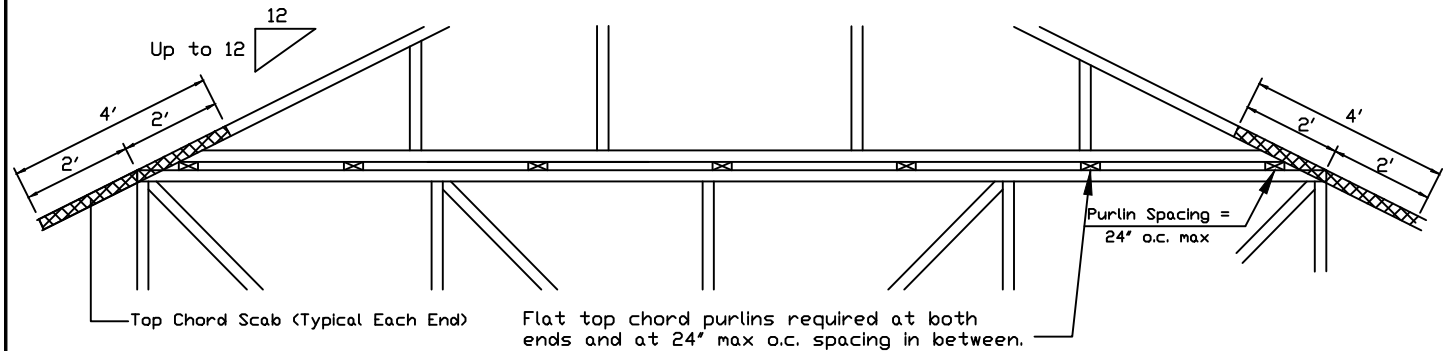
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

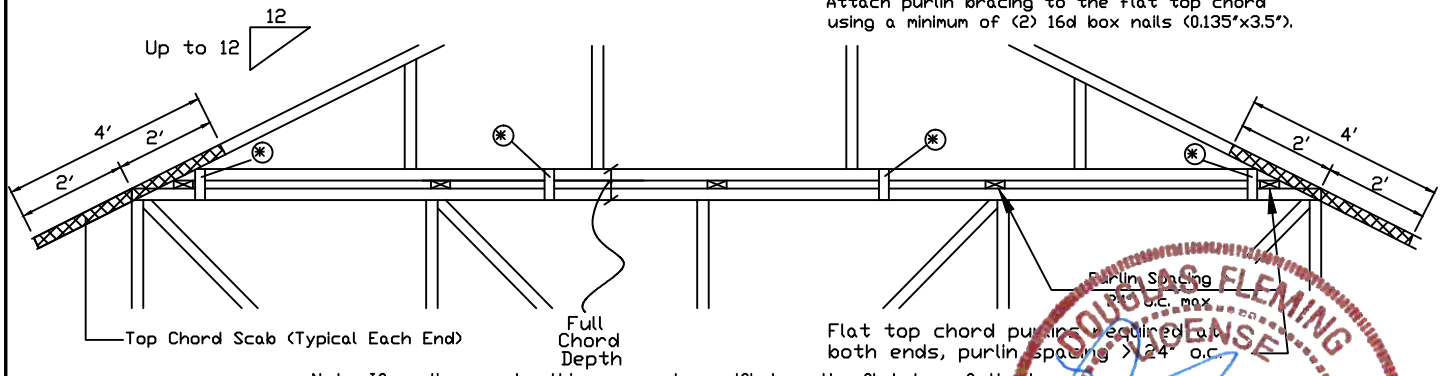
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER!**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
08/19/2022
COA#0 278

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
(2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location:
170 mph for SP (G = 0.55, min.),
155 mph for DF-L (G = 0.50, min.), or
120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

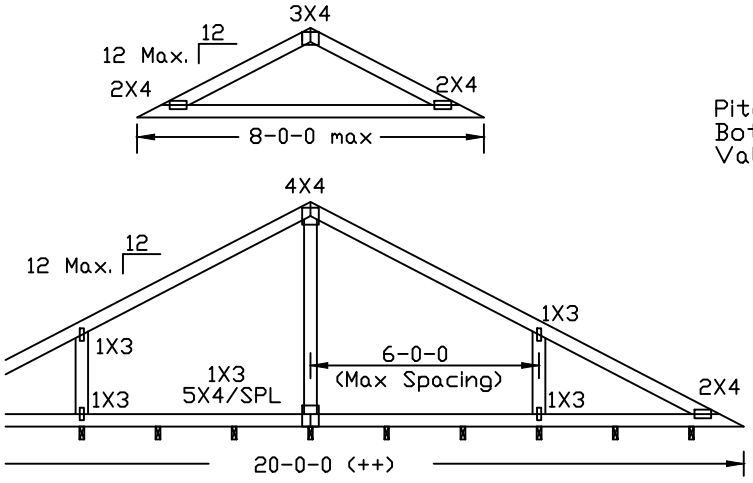
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

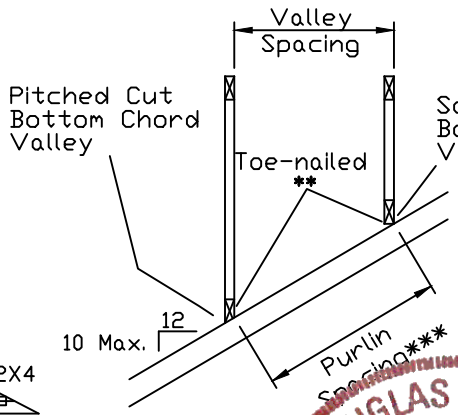
Top chord of truss beneath valley set must be braced with:
properly attached, rated sheathing applied prior to valley truss installation.
Or
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
Or
By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

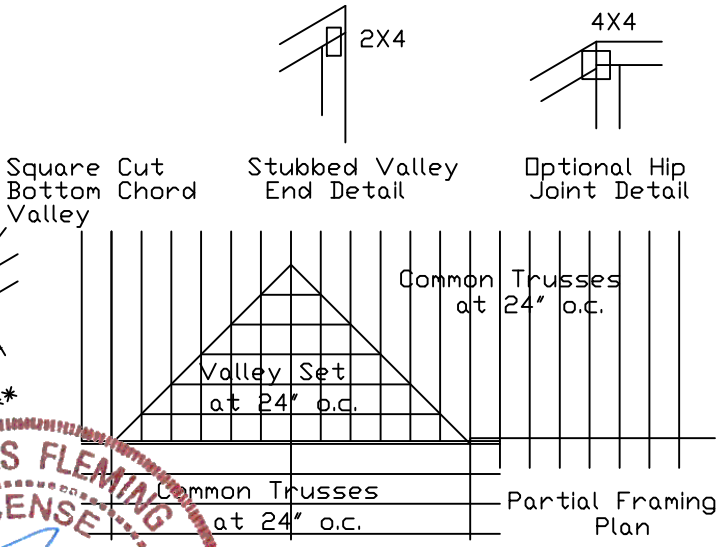
*** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



Supporting trusses at 24" o.c. maximum spacing.





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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08/19/2022
COA #0.278

TC LL	30	30	40	PSF
TC DL	20	15	7	PSF
BC DL	10	10	10	PSF
BC LL	0	0	0	PSF
TOT. LD.	60	55	57	PSF
DUR.FAC.	1.25	1.33	1.15	1.15
SPACING			24.0"	

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VALTN160118