

73

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IUP0857-Z0127130705

Truss Fabricator: Seminole Trusses, Inc.
Job Identification: B40045a--PETERSON RESIDENCE AMERICA'S HOME PLACE -- XXX LOWER SPRINGS RD. LAKE (XXX LOW
Truss Count: 5
Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61615-31.003(5a) of the FAC
Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR857

Details: 12030EC1-GBLLETIN-GABRST10-12015EC1-

#	Ref	Description	Drawing#	Date
1	52489--A1	33' Common	12240081	08/27/12
2	52490--A2	33' Gable	12240082	08/27/12
3	52491--B1	24' Common	12240083	08/27/12
4	52492--B2	24' Gable	12240085	08/27/12
5	52493--C1	4' Common	12240084	08/27/12

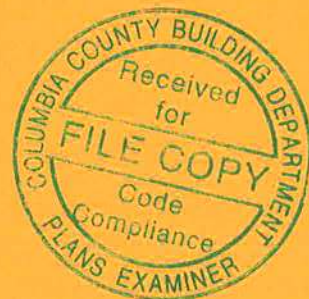
-Truss Design Engineer-

Walter P. Finn

Florida License Number: 22839

1950 Marley Drive

Haines City, FL 33844



(a) 1x4 #3SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

10.03.11.022 QTY:2 FL/-/5/-/E/-/-

Scale = .1875"/Ft.

TC LL	20.0	PSF	REF	R857- -	52490
TC DL	7.0	PSF	DATE	08/27/12	
BC DL	10.0	PSF	DRW	HCUSR857	12240082
BC LL	0.0	PSF	HC-ENG	SSB/WPF	
TOT.LD.	37.0	PSF	SEQN-	640370	
DUR.FAC.	1.25		FROM	RJL	

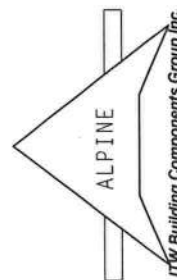


****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

*****IMPORTANT*****
FURNISH THIS DESIGN TO ALL COMPANIES INCLUDING INSTALLERS.

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, including but not limited to, the use of materials, methods of construction, or any other deviation from the design, which may result in any failure to build the truss in accordance with ABSE/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ABSE/TPI 1 Sec.2. For more information see: This job's drawings and specifications.



(B40045a--PETERSON RESIDENCE AMERICA'S HOME PLACE -- XXX LOWER SPRINGS RD. LAKE - B1 24' Common)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30 :B2 2x6 SP #1:
Webs 2x4 SP #3_12A

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=4.2 psf, wind BC DL=5.2 psf. GCPI(+/-)=0.18

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Calculated horizontal deflection is 0.09" due to live load and 0.16" due to dead load.

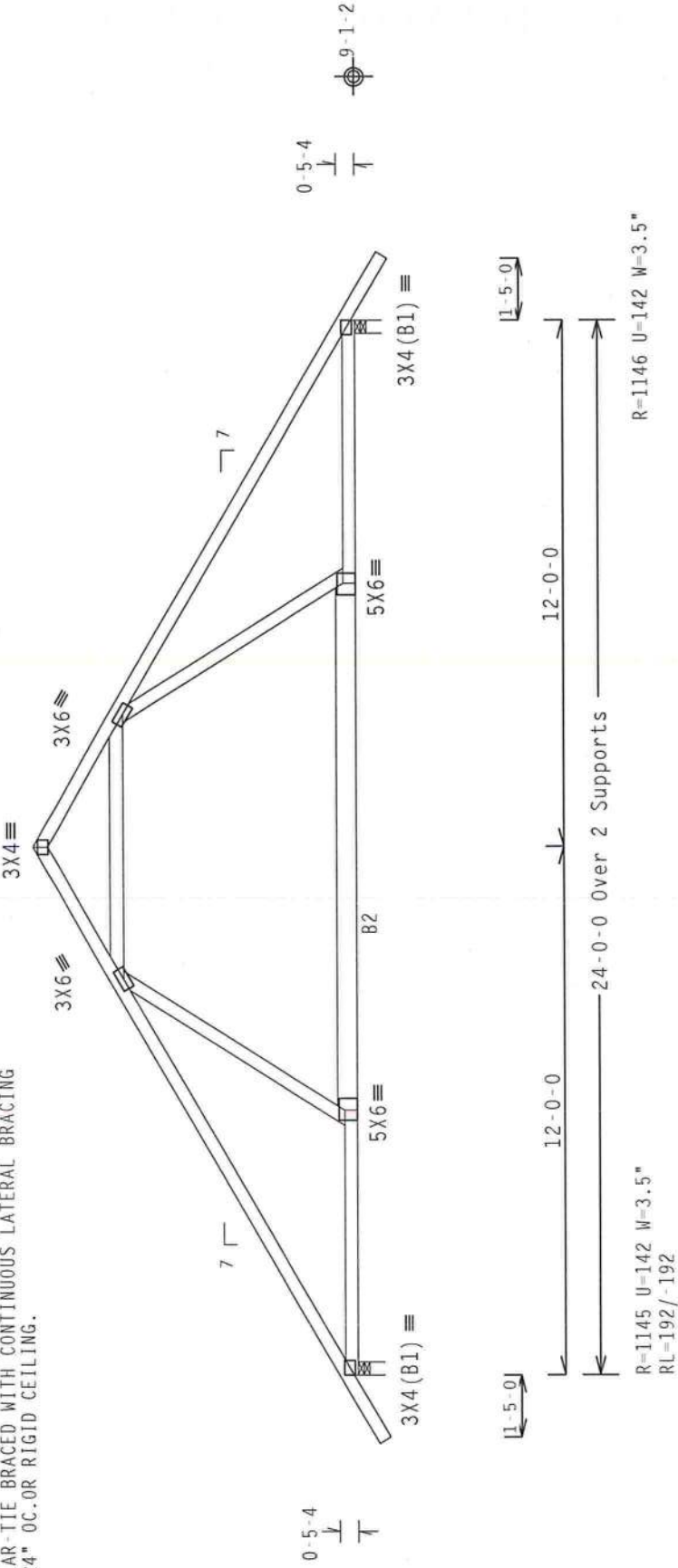
Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24" wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 56 PLF at 1.56 to 56 PLF at 25.56
BC - From 20 PLF at 0.00 to 20 PLF at 8.33
BC - From 60 PLF at 8.33 to 60 PLF at 15.71
BC - From 20 PLF at 15.71 to 20 PLF at 24.00

COLLAR-TIE BRACED WITH CONTINUOUS LATERAL BRACING AT 24" OC-OR RIGID CEILING.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .25" / Ft.

QTY: 9

10.03.11

FL / - / 5 / - / E / - / -

TC LL 20.0 PSF

TC DL 7.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 37.0 PSF

DUR.FAC. 1.25

REF R857 - - 52491

DATE 08/27/12

DRW HCUSR857 12240083

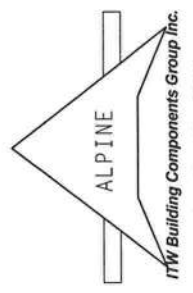
HC-ENG SSB/WPF

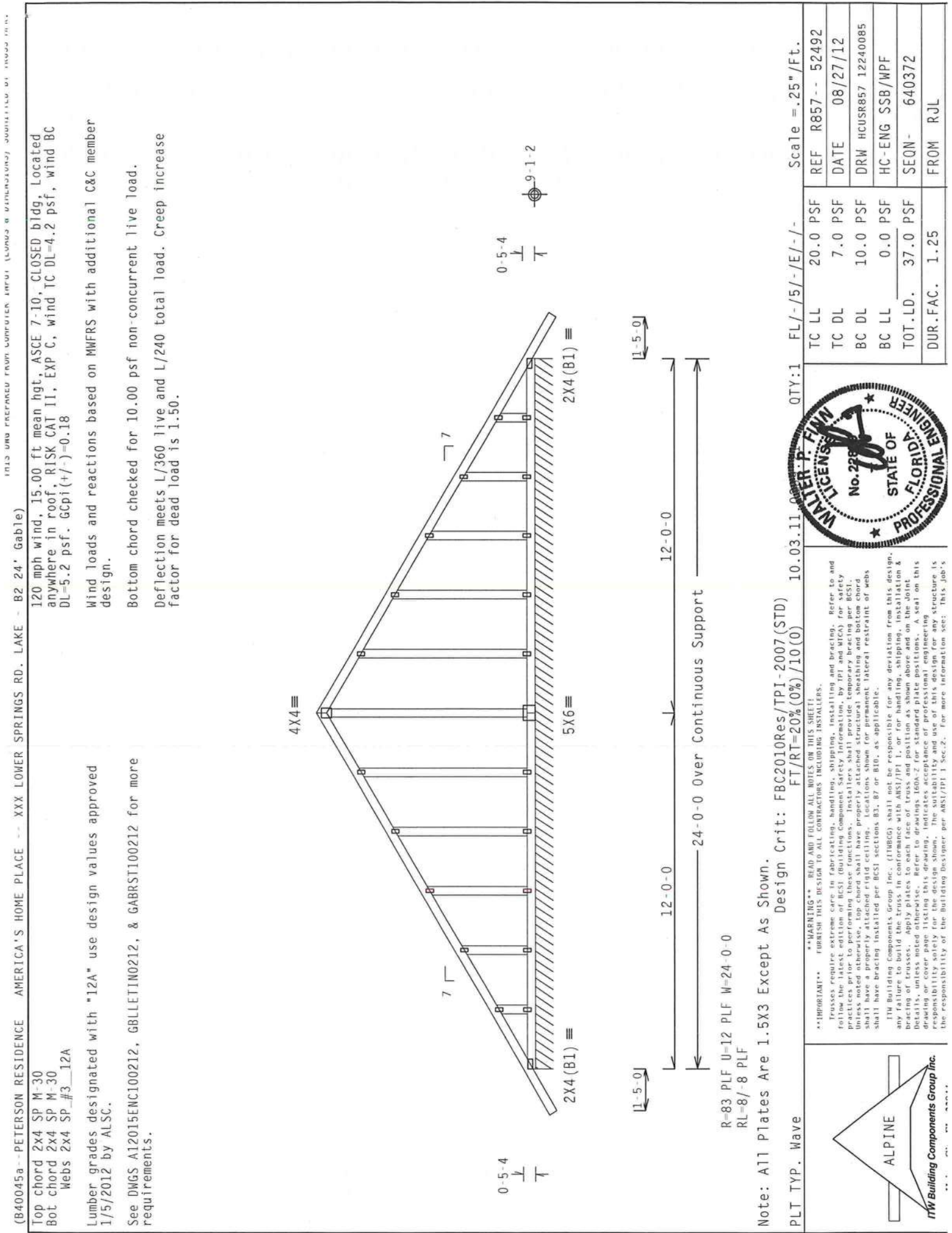
SEQN - 640371

FROM RUL



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(B40045a--PETERSON RESIDENCE AMERICA'S HOME PLACE -- XXX LOWER SPRINGS RD. LAKE - B2 24' Gable)

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP #3 12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

See DWGS A12015ENC100212, & GABRST100212 for more requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=4.2 psf, wind BC DL=5.2 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

0-5-4

11-5-0

12-0-0

24-0-0 Over Continuous Support

R-83 PLF U=12 PLF W=24-0-0
RL=8/-8 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

10.03.11

QTY:1

FL/-5/-E/-/-

Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R857 --	52492
TC DL	7.0 PSF	DATE	08/27/12	
BC DL	10.0 PSF	DRW	HCUSR857	12240085
BC LL	0.0 PSF	HC-ENG	SSB/WPF	
TOT.LD.	37.0 PSF	SEQN-	640372	
DUR.FAC.	1.25	FROM	RJL	

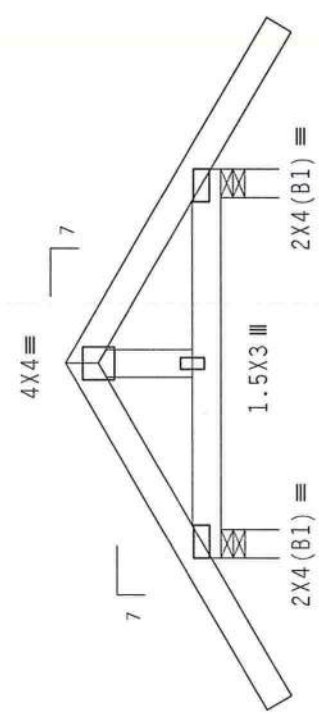
WALTER P. FINN
LICENSED PROFESSIONAL ENGINEER
No. 228
STATE OF FLORIDA

ALPINE
rtw Building Components Group Inc.

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THE Building Components Group Inc. (BCSI) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



1-5-0 2-0-0 1-5-0

Over 2 Supports

R=236 U=41 W=3.5"
RL=64/-64 R=236 U=41 W=3.5"

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20%(0%)/10(0)

10.03.11 QTY: 2 FL/-5/-E/-/- Scale = .5"/Ft.

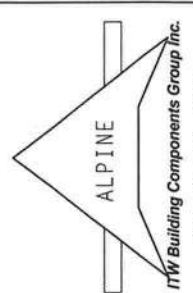


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PLT TYP. Wave



TC LL	20.0 PSF	REF	R857-- 52493
TC DL	7.0 PSF	DATE	08/27/12
BC DL	10.0 PSF	DRW	HCUSR857 12240084
BC LL	0.0 PSF	HC-ENG	SSB/WPF
TOT.LD.	37.0 PSF	SEQN	640373
DUR.FAC.	1.25	FROM	RJL

$C, K_Z t = 1.00$

Dr: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Dr: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Group A:		
Spruce-Pine-Fir		
#1 / #2	Standard	Hen-Fir #2 Stud
#3	Stud	
Douglas Fir-Larch		
#3	Stud	Southern Pine***
Standard		
Group B:		
Hen-Fir		
#1 & #2	Stud	Southern Pine***
#1		
Douglas Fir-Larch		
#1	Stud	Southern Pine***
#2		

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 95 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So, Pine Lumber design values based on the ALSC January, 2012 rule

* For (1) 'L' brace: space nails at 2" o.c.

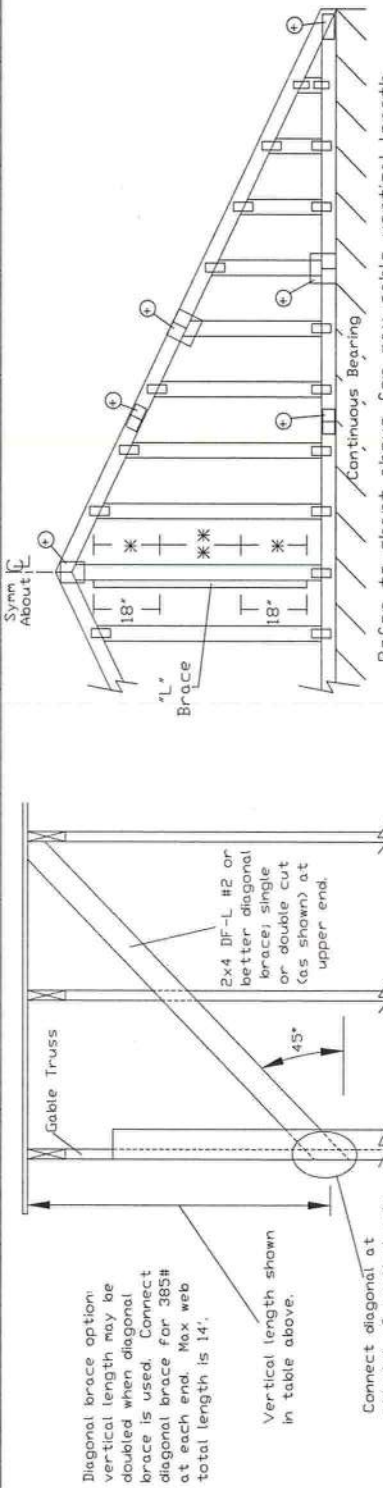
*For (2) "L" braces: space nails at 3" o.c.

L bracing must be a minimum of 80% of web member length

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

* Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



Refer to chart above for max gable vertical length.

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For more information on the general design and use of trusses, visit the website: www.vtca.com or the website: www.trustruct.com.
For more information on ITW Building Components Group Inc. visit the website: www.bcginc.com

REF ASCE7-10-GABI2030

DATE 2/14/12

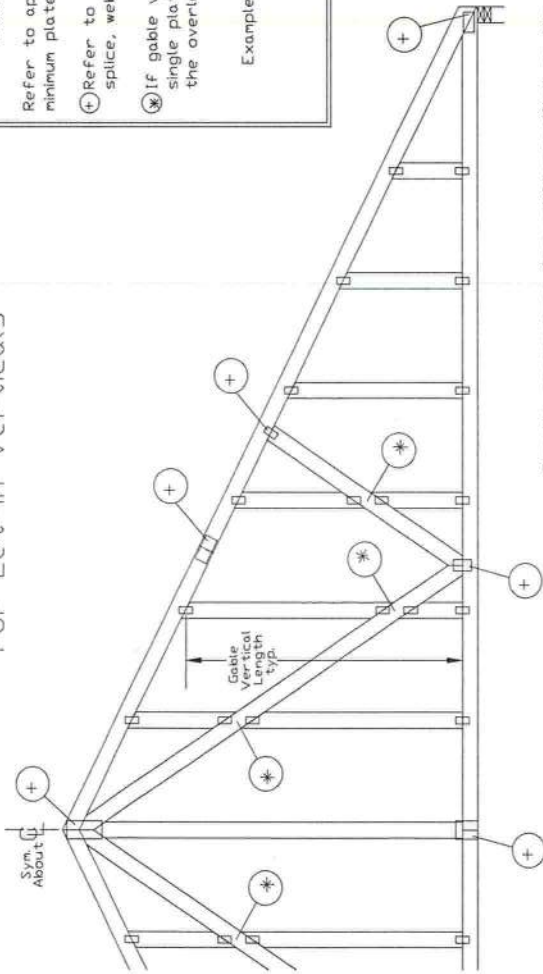
DRWG A12030ENC100212

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5	100	100	100	100
6	100	100	100	100
7	100	100	100	100
8	100	100	100	100
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75	100	100	100	100
76	100	100	100	100
77	100	100	100	100
78	100	100	100	100
79	100	100	100	100
80	100			

Earth City, MO 63045

MAX. SPACING 24.0"

Gable Detail For Let-in Verticals



Gable Truss Plate Sizes

Refer to appropriate ITV gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

- (1) 10d Common (0.148" x 3", min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

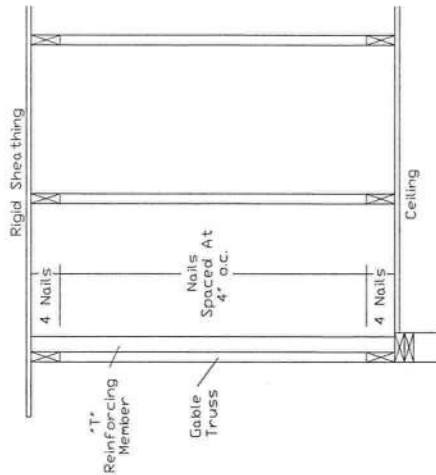
Toenailed Nails:

- (1) 10d Common (0.148" x 3", min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITV gable detail for ASCE wind load.

- ASCE 7-98 Gable Detail Drawings
 - A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109
- ASCE 7-02 Gable Detail Drawings
 - A13015020109, A12015020109, A11015020109, A10015020109, A13030020109, A12030020109, A11030020109, A10030020109
- ASCE 7-05 Gable Detail Drawings
 - A13015050109, A12015050109, A11015050109, A10015050109, A13030050109, A12030050109, A11030050109, A10030050109
- ASCE 7-10 Gable Detail Drawings
 - A11515ENC100212, A12015ENC100212, A14015ENC100212, A16015ENC100212, A18015ENC100212, A20015ENC100212, A20015END100212, A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212, A18030ENC100212, A20030ENC100212, A20030END100212

See appropriate ITV gable detail for maximum unreinforced gable vertical length.



"T" Reinforcement Attachment Detail

"T" Reinforcing Member



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITV gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length 1.30 x 8' 7" = 11' 2"

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ITVBCG www.itvbcg.com TPI www.tpinet.org VITAC www.vitacindustry.org ICC www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

REF LET-IN VERT

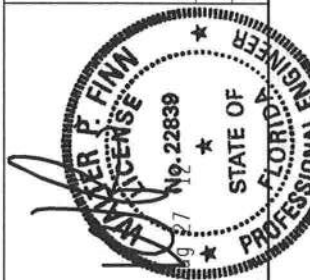
DATE 2/16/12

DRWG GBLLETIN0212

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"



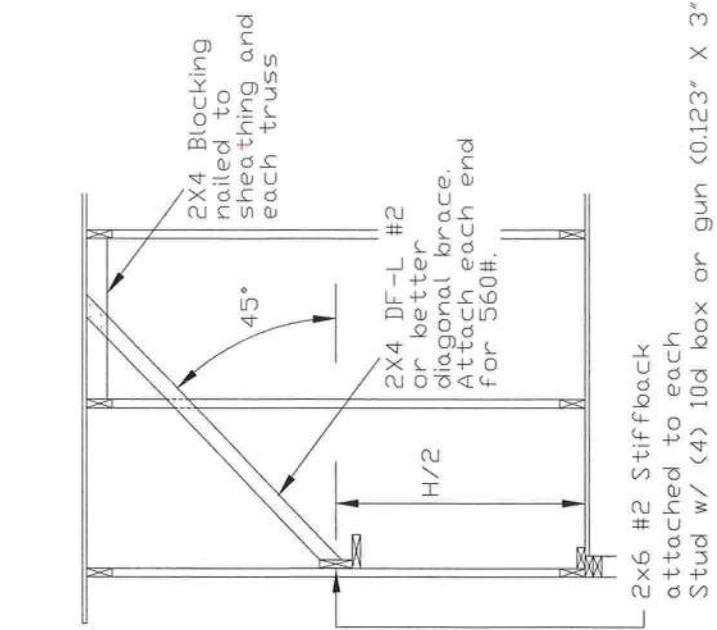
ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

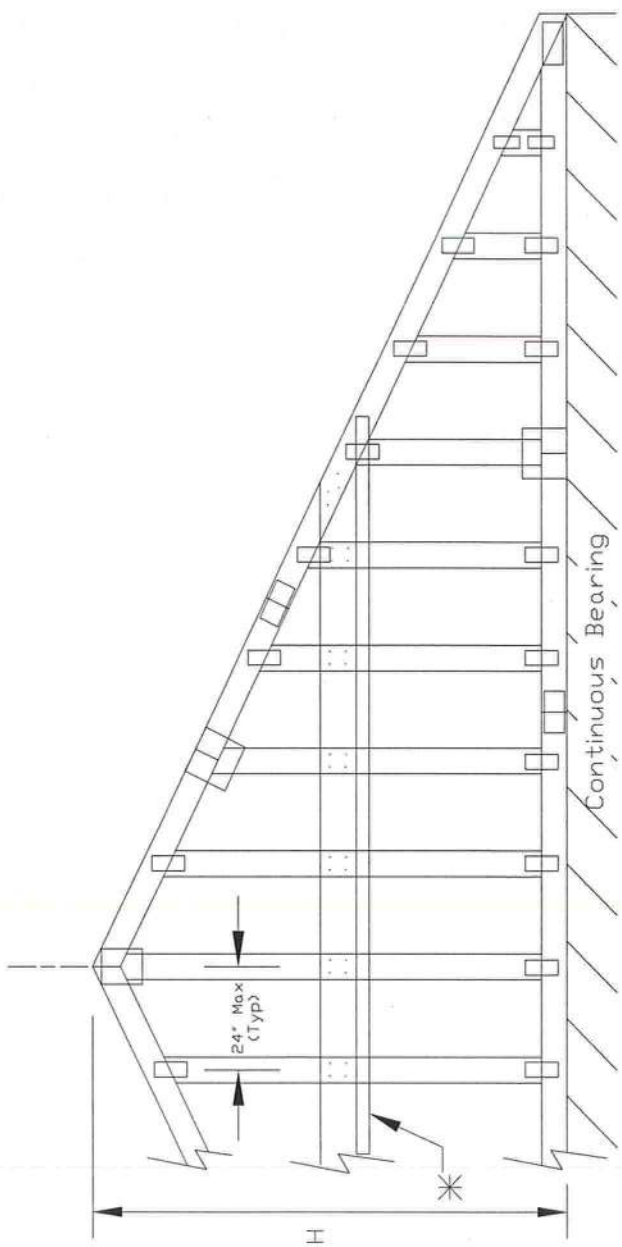
See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

Nails: 10d box or gun (0.128"x3",min) nails,



- H Less than 4'6" - no stud bracing required
- H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC10).
- H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC10).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
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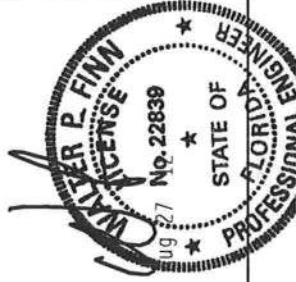
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices and details. Trusses shall be installed in accordance with the manufacturer's instructions. Unless specified otherwise, all trusses shall have a properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2. For more information see this job's general notes page and these notes. ITW BCSI www.bcsigroup.com TPI www.tpiusa.org WCA www.wcausa.org



Earth City, MO 63045

REF	GE	WHALE
DATE	2/14/12	
DRWG	GABRST100212	
MAX. TOT. LD. 60 PSF		
MAX. SPACING		



ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UP0857-Z0227130651

Truss Fabricator: Seminole Trusses, Inc.
Job Identification: B40045b--PETERSON RESIDENCE AMERICA'S HOME PLACE -- XXX LOWER SPRINGS RD. LAKE (XXX LOWE
Truss Count: 4
Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - N/A
Floor - 55.0 PSF @ 1.00 Duration
Wind - No Wind

Notes:

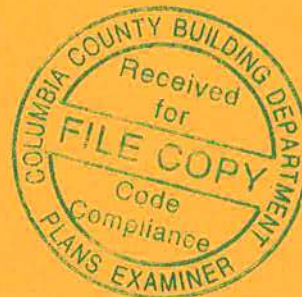
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR857

Details: STRBRIBR-

#	Ref	Description	Drawing#	Date
1	52536-F1	20'3" Floor T	12240077	08/27/12
2	52537--F2	16'5"8 Gable	12240078	08/27/12
3	52538-F3	16'7"8 Floor	12240079	08/27/12
4	52539-F4	12'11"8 Floor	12240080	08/27/12


08/27/2012

-Truss Design Engineer-
Walter P. Finn
Florida License Number: 22839
1950 Marley Drive
Haines City, FL 33844



Top chord 4x2 SP M-30
Bot chord 4x2 SP M-30
Webs 4x2 SP #3 12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

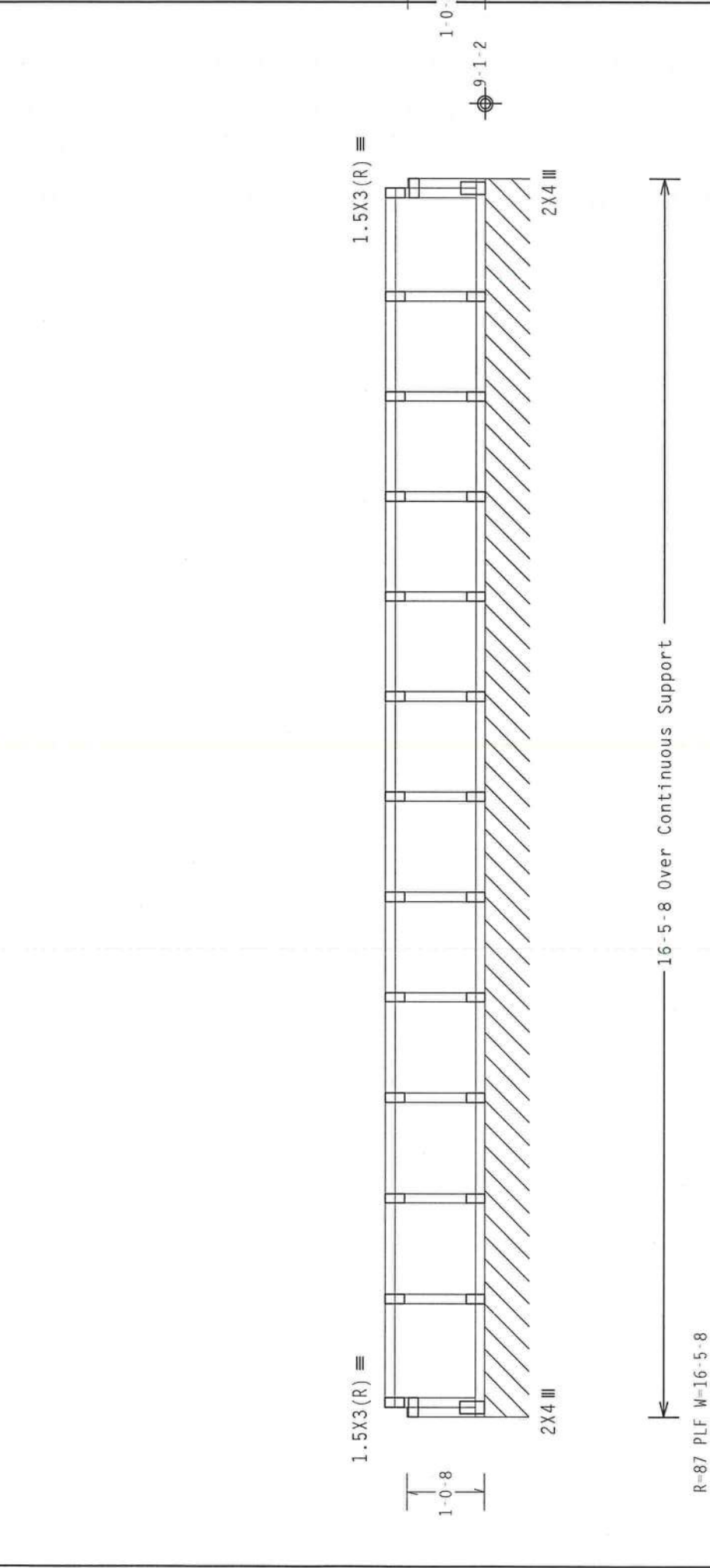
Truss must be installed as shown with top chord up.
Fasten rated sheathing to one face of this frame.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Trusses to be spaced at 19.2" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

PLT TYP. Wave

10.03.11.020000 QTY:4 FL/-/5/-/E/-/- Scale = .5" /Ft.

TC LL	40.0 PSF	REF	R857--	52537
TC DL	10.0 PSF	DATE	08/27/12	
BC DL	5.0 PSF	DRW	HCUSR857	12240078
BC LL	0.0 PSF	HC-ENG	SSB/WPF	
TOT.LD.	55.0 PSF	SEQN-	639080	
DUR.FAC.	1.00	FROM	RJL	

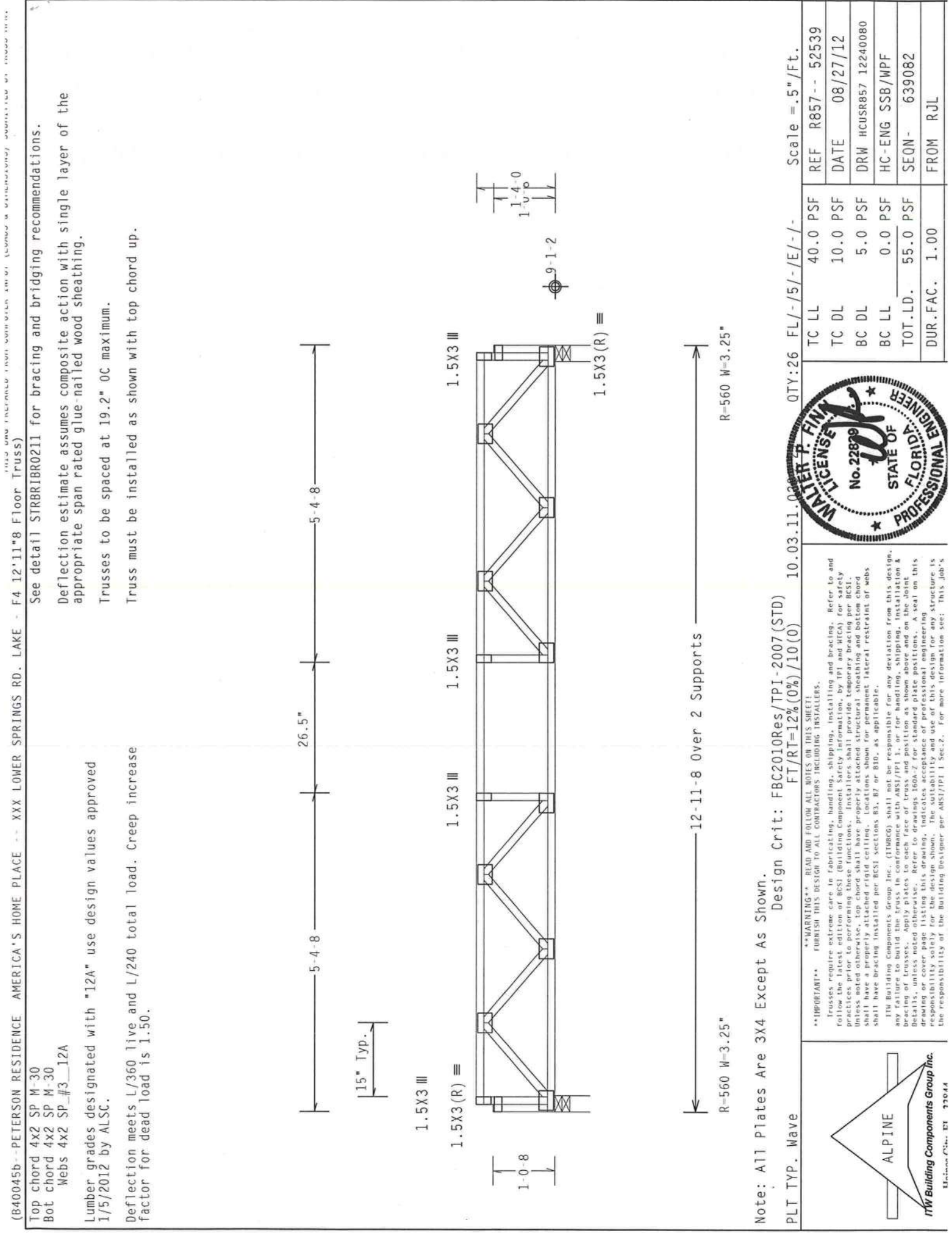
WALTER P. FINN
No. 2283
STATE OF FLORIDA
PROFESSIONAL ENGINEER

ALPINE
ITW Building Components Group Inc.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



(B40045b--PETERSON RESIDENCE AMERICA'S HOME PLACE -- XXX LOWER SPRINGS RD. LAKE - F4 12'11"8 Floor Truss)

Top chord 4x2 SP M-30
Bot chord 4x2 SP M-30
Webs 4x2 SP #3 12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

See detail STRBRIBR0211 for bracing and bridging recommendations.

Deflection estimate assumes composite action with single layer of the appropriate span rated glue-nailed wood sheathing.

Trusses to be spaced at 19.2" OC maximum.

Truss must be installed as shown with top chord up.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=12%(0%)/10(0)

QTY: 26 FL/-/5/-/E/-/-

Scale = .5" / Ft.

10.03.11.03

WALTER P. FINA

LICENSE No. 22829

STATE OF FLORIDA

PROFESSIONAL ENGINEER

1.5X3 III

1.5X3(R) III

1.5X3 III

1.5X3 III

1.5X3(R) III

1.5X3(R) III

1.5X3(R) III

1.5X3(R) III

1.5X3(R) III

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1.5X3(R) III

1.5X3(R) III

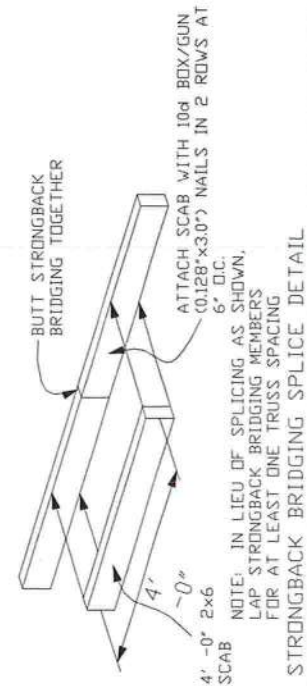
1.5X3(R) III

1.5X3(R) III

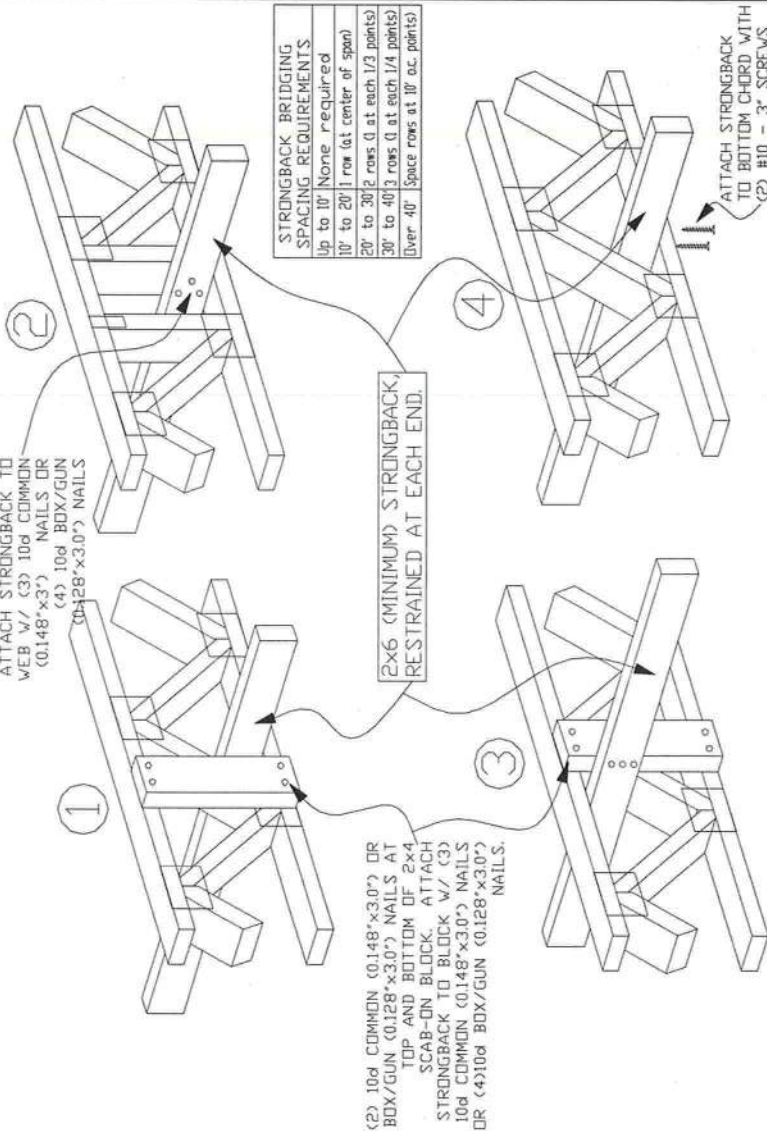
1.5X3(R) III

1.5X3(R) III

STRONGBACK BRIDGING RECOMMENDATIONS



NOTE: Details 1 and 2 are the preferred attachment methods



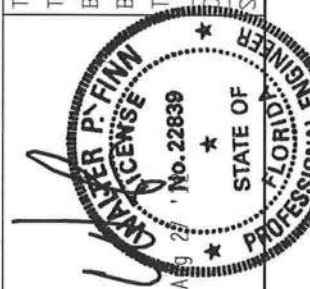
- ▶ All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
 - ▶ All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
 - ▶ The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' -0" o.c. (max.)
- The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.
- The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by ITW Building Components Group Inc.
- For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).



Earth City, MO 63045

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IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. The contractor shall be responsible for the design of the temporary bracing and the design of the permanent bracing. ITWBCG connector plates are made of 2018/16GA UH/SL/ASTM A553 grade 37/40/60 (K/A/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. ITW-BCG www.itwbcg.com TPI www.tpi.com VITA www.vitastructure.com ICD www.icdsef.org



TC LL	PSF	REF	STRONGBACK
TC DL	PSF	DATE	2/28/11
BC DL	PSF	DRWG	STRBTR0211
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			