

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

### Wind

Wind loads based on MWFRS with additional C&C

## **Additional Notes**

The overall height of this truss excluding overhang is 6-11-10



B-N 1693 - 284 1942 -361 L-K

1941 1941 N - M -362 K-J -387 M-L 1693 -277 J-H 1942 -387 Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Tens. Comp. L-F D-M 386 -54 409 - 109 D-L 409 - 109 F-K 386 -54 E-L 175 - 443



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SEQN: 325014 HIPS Job Number: 20-4381rep Ply: Cust R 215 JRef: 1Y2K2150006 LOT 88 ROLLING MEADOWS FROM: CDM Qty: 1 DrwNo: 233.24.1654.21183 Truss Label: C02 AK / WHK 08/20/2024 4'6"13 13'5"6 17'9" 22'0"10 26'6" 30'11"4 35'10" 4'5"3 2'2"13 4'5"6 4'3"10 4'3"10 4'5"6 4'5"3 4'10"12 =5X5 =5X6 5'9"10 **6X12** ₩3X10 TSR =7X6 =4X4(B2)=4X6 B2 **B3** =7X8 =5X5 M III2X4 ■7X8 |||2X4 III2.5X6 |||2X4 |||2X4 35'10" 0"10 4'5"3 4'5"6 4'1"4 4'5"6 4'5"6 4'5"3 4'10"12 13'6" 4'6"13 13'5"6 17'7"4 22'0"10 26'6 30'11"4 35'10" 11,13

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.58 ft Loc. from endwall: not in 9.00 ft GCoi: 0.18	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	PP Deflection in loc L/defl L/# VERT(LL): 0.224 G 999 24 VERT(CL): 0.458 G 933 18 HORZ(LL): 0.119 K - HORZ(TL): 0.243 K - Creep Factor: 2.0 Max TC CSI: 0.651 Max BC CSI: 0.744 Max Web CSI: 0.880
	Wind Duration: 1.60	WAVE	VIEW Ver: 19.02.02B.0122.15

CSI Criteria						A	Maxim
eflection	on i	n k	oc L	/defl	L/#		G
T(LL):	0.3	224	G	999	240	Lo	c R+
T(CL):	0.4	458	G	933	180	В	1589
Z(LL):	0.	119	K			K	1572
Z(TL):	0.3	243	K			Wi	nd read
p Facto	or: 2	2.0					Brg V
TC CS	1:	0.6	351				Brg V
BC CS	1:	0.7	744				arings
Web C							mbers
							ximun ords
						-	IV-SEC-11

B 1	589	-	1-	/926	/285	/185
K 1	572	1-	1-	/914	/279	1-
Wind	reacti	ons ba	sed or	MWFRS		
BE	Brg Wi	d = 4.0	Mi	n Req = 1.9	)	
KE	Brg Wi	d = 4.0	Mi	n Reg = 1.9	)	
Beari	ngs B	& Kare	e a rig	id surface.		
Memb	pers no	ot listed	d have	forces less	than 3	375#
Maxir	num 1	Top Ch	ord F	orces Per	Ply (lb	s)
				Chords		
B-C		478 - 19	987	G-H	810	-3059
C-D		355 -3	746	H-I	651	-2270
D-E		674 - 26	680	1-J	593	-2209
E-F		745 -2	770	J-K	603	- 2508

/Rh

Non-Gravity

/RL

/Rw /U

Maximum Reactions (lbs)

Gravity

F-G

/R

# **Plating Notes**

Lumber

All plates are 3X4 except as noted.

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; B2,B3 2x6 SP 2400f-2.0E; Webs: 2x4 SP #3;

Wind loads based on MWFRS with additional C&C member design.

### **Additional Notes**

The overall height of this truss excluding overhang is

Laterally brace BC above filler @ 24" O.C. (or as designed) Including a brace on BC directly above both ends of filler (if no rigid diaphragm exists at that point)

Maximu	m Bot Chord	Forces Per	Ply (lbs)
	Tens Comp	Chords	

812 - 3071

Chords	rens.c	Jomp.	Choras	rens.	Comp.
B-W	1497	- 306	T-R	. 2807	- 559
C-X	3295	- 658	R-P	2774	- 551
X-V	3234	-645	0-N	1849	- 348
V-U	3209	-642	N-M	2093	- 444
U-T	2251	-411	M-K	2094	- 443

Maximum Web Forces Per Ply (lbs)

Webs	Tens.0	Comp.	Webs	Tens.	Comp.
C-W	469	-2299	T-F	176	-557
X-W	1514	-299	F-P	381	-82
V-D	629	-92	P-H	1092	-223
D-U	266	-1042	P-O	2310	- 456
E-U	502	-92	H-O	271	- 1065
E-T	745	- 196	0-1	669	- 177



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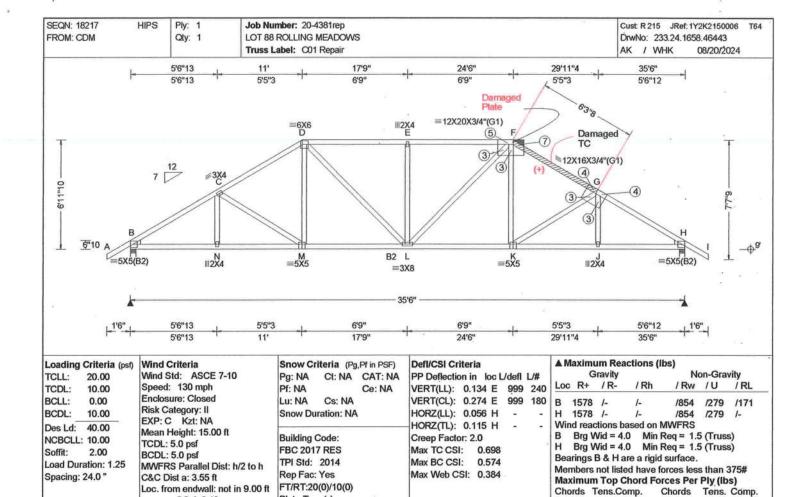
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Truss repaired for Damaged plate at joint F and damaged partial TC member between F and G

GCpi: 0.18

Wind Duration: 1.60

Refer to drawings 233.24.1654.09980 for lumber, plates, and other data not listed here.

Note: Prior to and during the repair operation, this truss and any supported spans must be temporarily braced and shored. The design and positioning of this bracing and shoring to be designed by Building Designer.

Remove and replace TC member: Field applied 2x4 SP#1 (or better), cut to fit tight member.

VIEW Ver: 24.01.02.0529.20

Plate Type(s):

WAVE

550 - 2038 D-E 588 - 1984 G-H

C-D

554 - 2360

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. 1693 - 285 1942 -362 L-K

E-F

F-G

588 - 1984

550 -2037

553 -2359

B-N 1941 1940 -363 - 387 N-M K-J M-L 1693 -278 J-H 1941 -387 Maximum Web Forces Per Ply (lbs)

Tens.Comp. Webs Tens. Comp. D-M 386 -55 L-F 409 - 109 F-K D-L 409 - 109 386 -54 E-L 175 -443

### Additional Notes

(G1) Gusset Plates are 3/4" APA STRUCTURAL I RATED SHEATHING, 48/24, EXP 1. Apply gusset to each face of truss and attach with evenly distributed 0.113"x2.0" Nails specified in circles. Hatched lines indicate portions on gussets protruding outside of the perimeter of the truss that may be trimmed flush with the truss profile.

Minimum Nail/Screw Spacing Requirements Based on

Minimum Nail/Screw Spacing Requirements based on ANSI/AF&PA NDS-2001:
End Distance 1-3/4" Edge Distance 5/8" Spacing Between Rows 5/8" Spacing in a Row 1-3/4" Maximum Number of Rows for Member Size: 2x4 4 Rows



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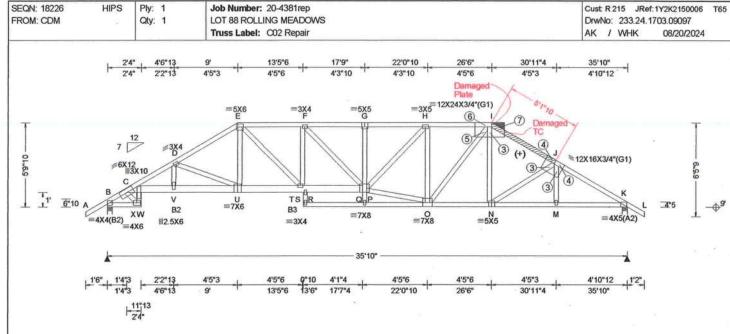
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North Building, 4th I Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria
TCLL: 20.00	Wind Std: ASCE 7-10
TCDL: 10.00	Speed: 130 mph
BCLL: 0.00	Enclosure: Closed
BCDL: 10.00	Risk Category: II
	EXP: C Kzt: NA
Des Ld: 40.00	Mean Height: 15.00 ft
NCBCLL: 10.00	TCDL: 5.0 psf
Soffit: 2.00	BCDL: 5.0 psf
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h
Spacing: 24.0 "	C&C Dist a: 3.58 ft
72	Loc. from endwall: not in 9.00 ft
	GCpi: 0.18
	Wind Duration: 1 60

Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA

**Building Code: FBC 2017 RES** TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE

Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.225 G 999 240 VERT(CL): 0.461 G 927 180 HORZ(LL): 0.119 K HORZ(TL): 0.243 K

Creep Factor: 2.0 Max TC CSI: 0.652 Max BC CSI: 0.689 Max Web CSI: 0.880

VIEW Ver: 24.01.02.0529.20

A			ctions (I			
	G	ravity		N	on-Grav	vity
Loc	R+	/ R-	/Rh	/ Rw	/U	/RL
В	1589	1-	/-	/847	/284	/145
K	1572	1-	1-	/853	/279	1-

Min Req = 1.9 (Truss) Min Req = 1.9 (Truss) Brg Wid = 4.0Brg Wid = 4.0 Bearings B & K are a rigid surface.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

B-C	479 - 1987	G-H	810	-3059
C-D	857 - 3746	H-1	651	-2270
D-E	674 - 2680	1-J	594	-2208
E-F	746 - 2770	J-K	603	-2507
F-G	813 - 3071			

Refer to drawings 233.24.1654.21183 for lumber, plates, and other data not listed here.

Truss repaired for Damaged plate at joint I and damaged partial TC member righ of Joint I

Note: Prior to and during the repair operation, this truss and any supported spans must be temporarily braced and shored. The design and positioning of this bracing and shoring to be designed by Building Designer.

(+) Remove and replace TC member: Field applied 2x4 SP#1 (or better), cut to fit tight member.

### **Additional Notes**

(G1) Gusset Plates are (G1) Gusset Plates are
3/4" APA STRUCTURAL I RATED SHEATHING, 48/24, EXP 1.
Apply gusset to each face of truss and attach with evenly
distributed 0.113"x2.0" Nails specified in
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protruding outside of the perimeter of the truss that may
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Minimum Nail/Screw Spacing Requirements Based on
ANSI/AF&PA NDS-2001:
End Distance 1-3/4" Edge Distance 5/8"

End Distance 1-3/4" Edge Distance 5/8" Spacing Between Rows 5/8" Spacing in a Row 1-3/4" Maximum Number of Rows for Member Size: 2x4 4 Rows



B-W 1497 -307 T-	R 2807 - 560
C-X 3295 -660 R-	P 2774 -552
X - V 3234 - 647 O -	N 1849 - 348
V-U 3209 -644 N-	M 2092 -444
U-T 2251 -412 M	· K 2093 - 443

Maximum Bot Chord Forces Per Ply (lbs)

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens.	Comp.
C-W	471 - 2299	T-F	176	- 558
X-W	1514 - 301	F-P	518	-82
V-D	629 -92	P-H	1092	-224
D-U	267 - 1042	P-O	2310	- 457
E-U	502 -93	H-O	271	-1064
E-T	768 - 196	0-1	669	- 177

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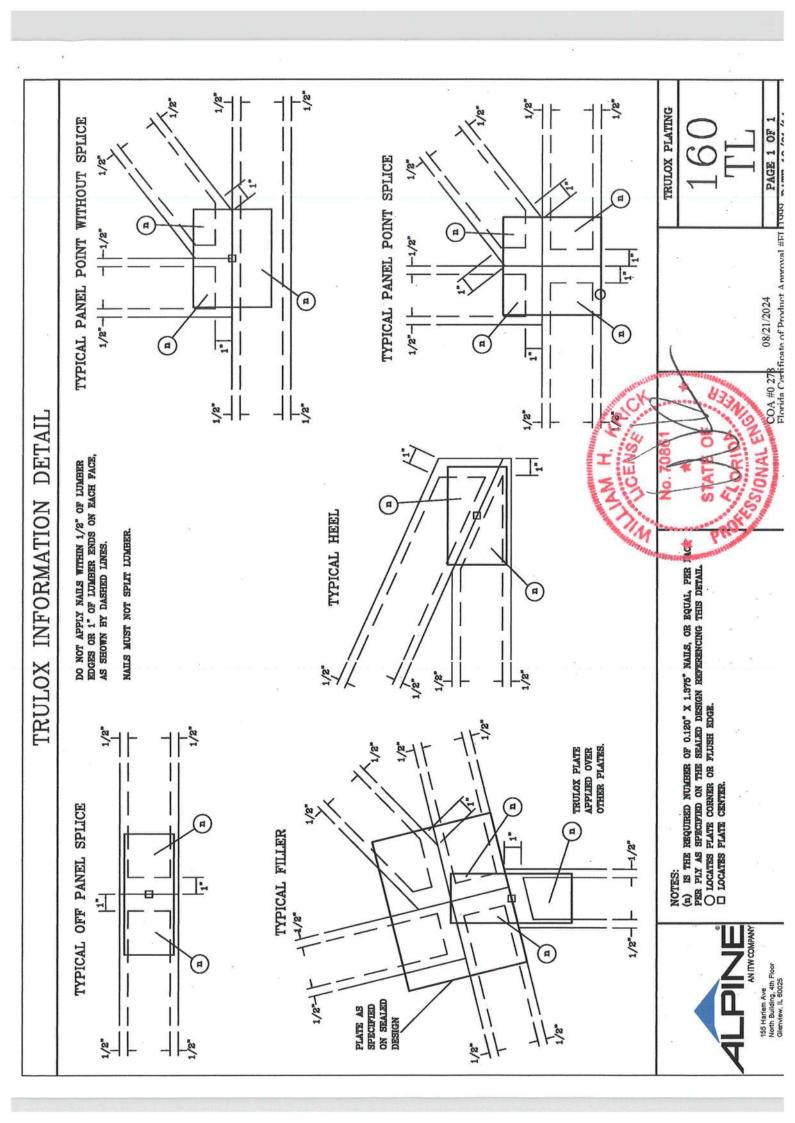
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North Building, 4th I Glenview, IL 60025 4th Floor





Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com



This item has been digitally signed by William H. Krick on the date adjacent to the seal.

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COA #0 278

Florida Certificate of Product Approval #FL 1999

08/21/2024

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 20-4381rep
Job Description: LOT 88 ROLLING MEADOWS	
Address: 322 SW Timberland Ct, Lake City, FL	

Job Engineering Criteria:				
Design Code: FBC 2017 RES	IntelliVIEW Version: 19.02.02B through 24.01.02  JRef #: 1Y2K2150006			
Wind Standard: ASCE 7-10 Wind Speed (mph): 130  Building Type: Closed	Design Loading (psf): 40.00			

This package contains general notes pages, 4 truss drawing(s) and 1 detail(s).

Item	Drawing Number	Truss	
1	233.24.1654.09980	C01	
3	233.24.1658.46443	C01 Repair	
5	160TL	*	

Item	Drawing Number	Truss
2	233.24.1654.21183	C02
4	233.24.1703.09097	C02 Repair

## **General Notes**

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teethr shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

# **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

## Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

## **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

## Bearing Information:

The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

# General Notes (continued)

## Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

## Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

**Key to Terms:** 

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Page 2 of 3

# **General Notes** (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

## References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com