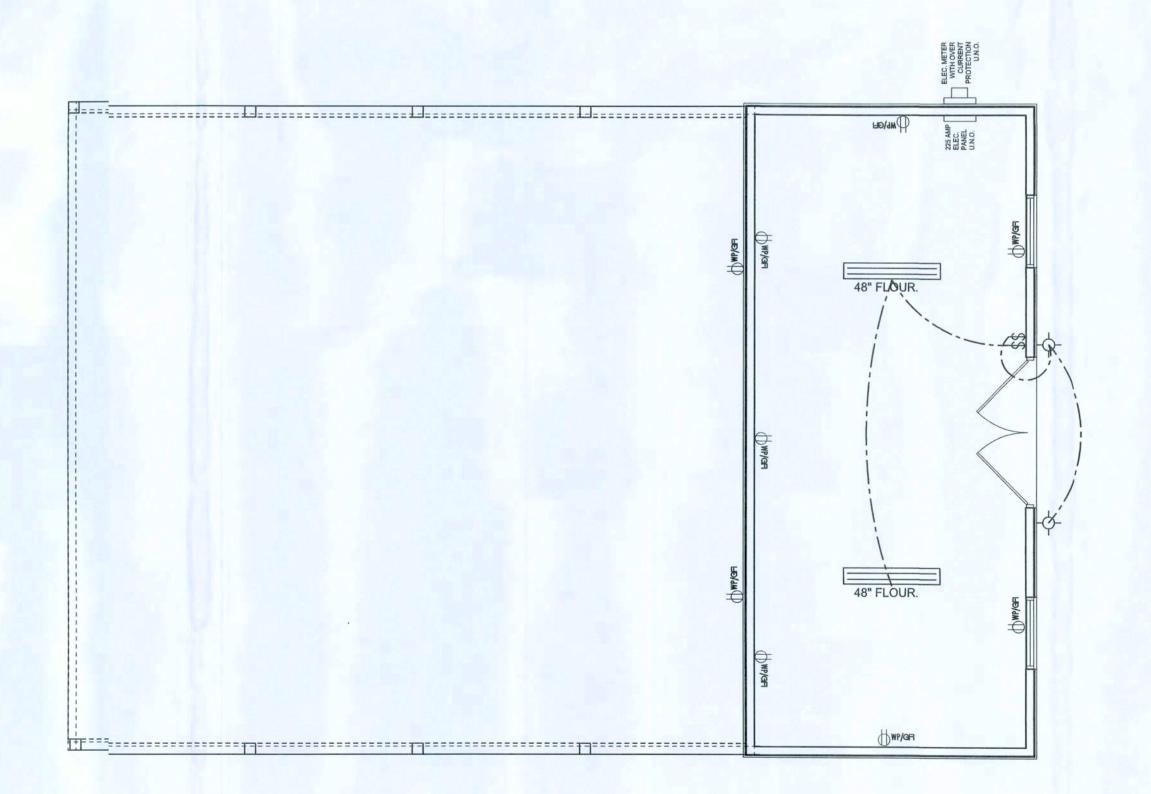


ALL CEIEILINGS TO BE 8' UNLESS NOTED OTHERWISE



ELEECTRICAL PLAN
SCALE E: 1/4" = 1'-0"

REVISIONS 4/08/2108

PERMIT# 08-0414

SOTPIAN

AREA SUMMARY

WORK-SHOP AREA	323	S.F
POLE BARN AREA	754	S.F.
TOTAL AREA	1077	S.F.

ELECTRICAL LEGEND

	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QP	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
→	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
#	220v OUTLET
⊕an	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
•	GARAGE DOOR OPENER
	WALL HEATER

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
 NEAR ALL BEDROOMS.
- E -5
 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
 DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
 SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- = (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE
- E -10

 LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC
 CONDUCTORS ENTER THE BUILDING.
 SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED
 INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL
 APPROVAL OF THE BUILDING OFFICIAL

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PCB 868, Lake City, FL 32056, 386-754-419
DIMENSIONS: Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution. Do not proceed without clarification.
COPYRIGHTS AID PROPERTY RIGHTS: Mark Disosway, F.E. hereby expressly reserves its common law opyrights and property right in these instruments of service. This document is not to be reprodued, altered or copied in any form or manner whout first the express written permission and cnsent of Mark Disosway. CERTIFICATIONI hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering
comply with sectin R301.2.1, florida building code residential 204, to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location.
building, at specific totation.
MARK DISOSWAY P.E. 53915
1 Du
Mostbegg
SEAL

Jonathan Perry
Construction

Rene & Debie Dibut Pole Barn

> ADDRESS: 259 NW Mershon St. LakeCity FL. 32055

Mark Disosway P.E. P.0. Box 868 Lake City, Florida 32056 Phone: β86) 754 - 5419 Fax: (336) 269 - 4871

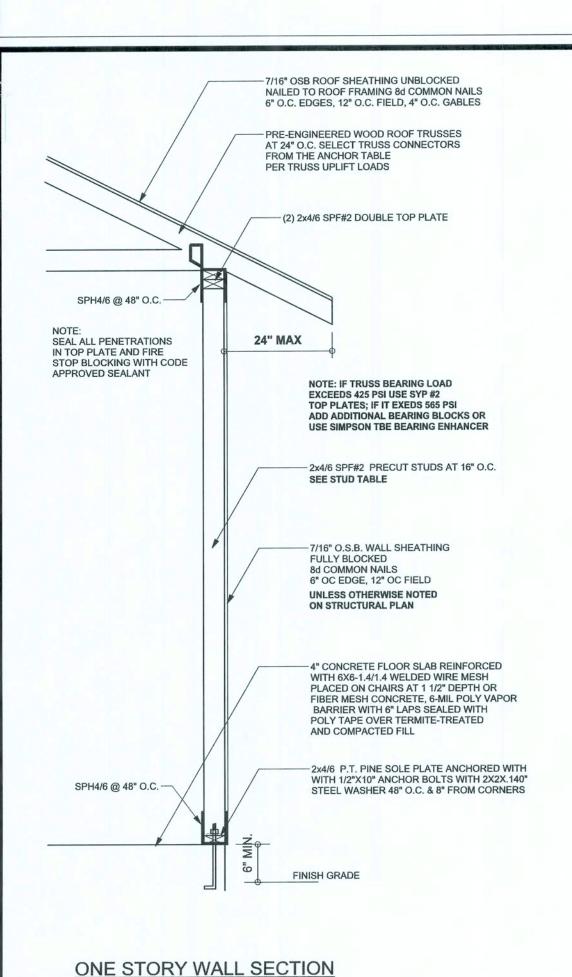
PRINTED DATE:
April08, 2008

DRAWN BY: CHECKED BY:
Ben Sparks

FINALS DATI: 1 / Apr / 08

> JOBNUMBER: 303312 DRAVING NUMBER

> > 1 Of 3 SHEETS

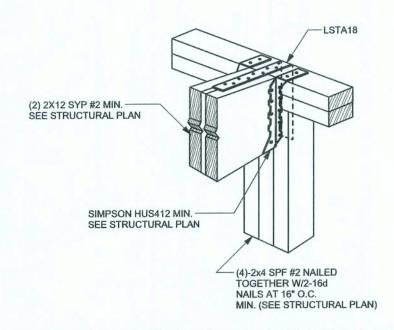


EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

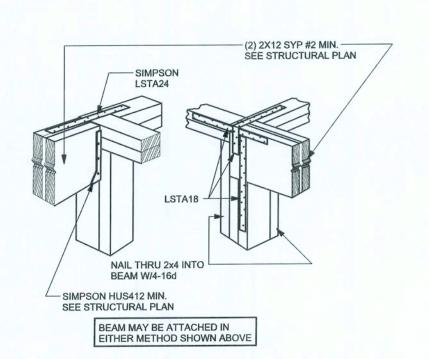
SCALE: 3/4" = 1'-0"

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

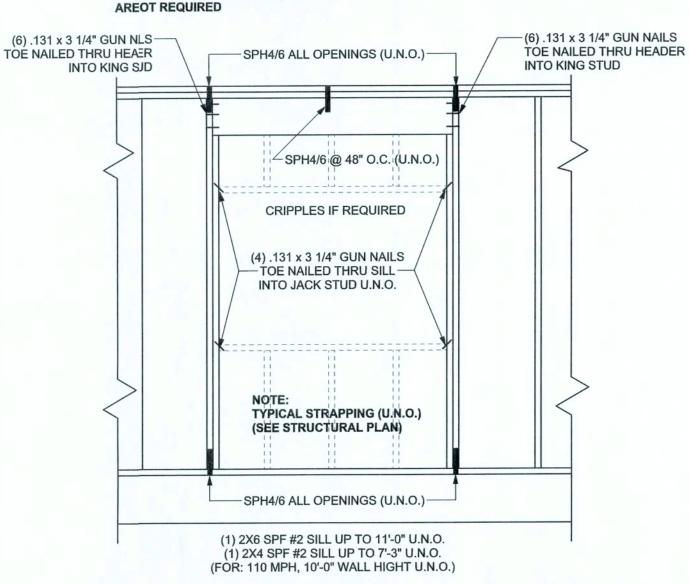


BEAM MID-WALL CONNECTION DETAIL



BEAM CORNER CONNECTION. DETAIL SCALE: N.T.S.

IF TISS TO WALL STRAPS ARE NAILED TO TE HEADER THE SPH4/6 @ 48" O.C.



TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2 2	1200	1.6
2x10	SYP #2 2	1050	1.6
2x12	SYP#2 ₂	975	1.6
GLB	24F-V3 SF _{SP}	2400	1.8
LSL	TIMBERSTR/RAND	1700	1.7
LVL	MICROLAIAM	1600	1.9
PSL	PARALAN	2900	2.0

-2X4 OUTRIGGER @ 24" O.C. -7/16" OSB ROOF SHEATHING 8d 6" O.C. EDGE, 12" O.C. FIELD, & 4" O.C. GABLES H3 EACH -- BLOCKING REQUIRED BETWEEN OUT RIGGIGERS INSTALL 2X4 SPF #2 DIAGONAL BRACE -AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND RAT RUN @ 6' O.C. DIAGONAL BRACE MUST T BE NAILED

TO TRUSS WEBS FOR LETENGTH

OVER 12' IT MAY BE "T" BF BRACED UP
TO 12' AND UNBRACED UTUP TO 7' ATTACH RAT RUN TO-BLOCKING w/ (4) 12dS /- (4) 12dS --TOF NAIL TRUSS -TO TOP PLATE w/ (8) -16d TO TRUSS - 2X4X8' RAT RUN NAIL EASACH CONNECTION w/ (4) 12dSdS AND (8) -16d TO WALL @ 48" O.C. U.N.O. - (8) 12dS - 2X4 SPF #2 BLOCKING - H3 INSTALLED HORIZONTALLY 12dS = 12dd SINKER ODR .135" X 3.125" OF)R .131 X 3.25"

> SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED GABLE BRACING DETAIL

> > PRE ENGINEERED ROOF TRUSS -

OOOUBLE 2x4 SPF TOP PLATE NAILED -

TO TOGETHER W/2-16d NAILS AT 16" O.C. 4' N MIN. LAP W/ (12) - 16d OR 4" LAP W/

ALL STUDS TO BE 2x4

CONTINUOUS FRAME TO

CELLING DIAPHRAGM DETAIL

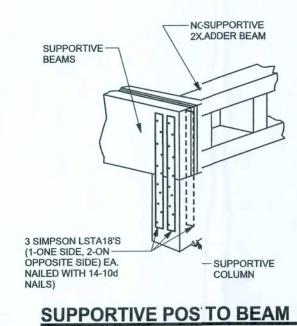
SPF NAILED TO TOP AND BOTTOM PLATES

CSCS20 w/ (4) - 16d &(14) - 10d

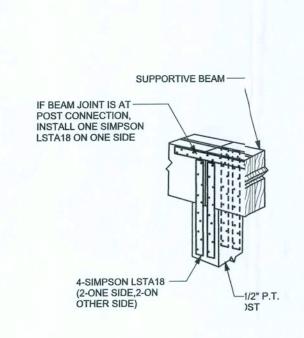
INTINTERIOR CEILING AS SPECIFIED ON FLOOR PLAN

CONTINUOUS FRAME -

TO TOFOP PLATE AT
BOTTO OM CHORD OF TRUSS

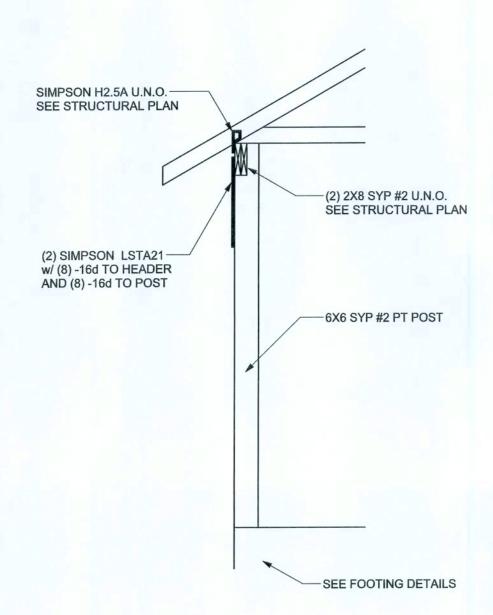


DETAIL FOR SINGE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POT TO BEAM DETAIL

SCALE: N.T.S.



4X4 / 6X6 PORCH POST DETAIL SCALE: 1/2" = 1'-0"

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2 2	1200	1.6
2x10	SYP#2 2	1050	1.6
2x12	SYP #2 2	975	1.6
GLB	24F-V3 SF _{SP}	2400	1.8
LSL	TIMBERSTR/RAND	1700	1.7
LVL	MICROLAI	1600	1.9
PSL	PARALAN	2900	2.0

BUILDER'S RESPONSIBILITY

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE

PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR

RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR

INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD

PER THE MANUFACTURER'S RECOMMENDATIONS, FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT

CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO

OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED.

STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT

CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER

FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND

AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S

	ILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR CALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 MENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE .	A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL LOAD ENGINEER IMMEDIATELY.
DESIGN, I	HE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, D-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet met ties not completely embedded in mortar grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7 Movement joints Contractor assumes responsibility		Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*				
		HEAVY GIRDER HEDOWNS			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
	< 2200	ABU44	12-16d		1/2" AB	
< 2200						
< 2200 < 2300	< 2300	ABU66	12-16d		1/2" AB	

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 20					
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS V MEAN ROOF HEIGHT NOT EXCEEDING LEAST I ON UPPER HALF OF HILL OR ESCARPMENT 60 SLOPE AND UNOBSTRUCTED UPWIND FOR 50	HORIZONTAL D FT IN EXP. B. 3	OFT IN	SION	OR 60	FT; NC
BUILDING IS NOT IN THE HIGH VELOCITY HURF	RICANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	REGION				
1.) BASIC WIND SPEED = 110 MPH					
2.) WIND EXPOSURE = B					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED B	UILDI	NG)		
8.) COMPONENTS AND CLADDING DESIGN W	IND PRESSUR	ES (T	ABLE	R301.	2(2))
	Zone	Effect	tive Wi	ind Are	ea (ft2)
***		1	0		100
	1	19.9	-21.8	18.1	-18.1
2 2	2	19.9	-25.5	18.1	-21.8
5	2 O'hg		-40.6		-40.6
2 2 2 5	3	_	-25.5	18.1	-21.8
4	3 O'hg	-	-68.3		-42.4
	4		-23.6		-20.4
55	5	21.8	-29.1	18.5	-22.6
THE WAY		0 145	dans.	21.8	20.4
/3	Doors	& vvino	lows	21.0	-29.1

DESIGN LOADS FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF NOT IN FLOOD ZONE (BUILDER TO VERIFY)

8x7 Garage Door 19.5 -22.9

6x7 Garage Door 18.5 -21.0

REVISIONS 4/08/2018 PERMIT # 08-0414

SOFPLAN

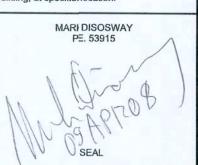
PE No.53915, POB368, Lake City, FL 32056, 386-754-549 DIMENSIONS: stated dimensions spercede scale dimensions. Refer d questions to

Mark Disosway, P.E for resolution

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CERTIFICATION: I rereby certify that I have camined this plan, and that the applicable portions of the plan, elating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This esign is valid for one building, at specific location.



Jonatian Perry Construction

Rene & Debie Dibut Pob Barn

> AIDRESS: 259 NV Mershon St.

> Lake Cty FL. 32055

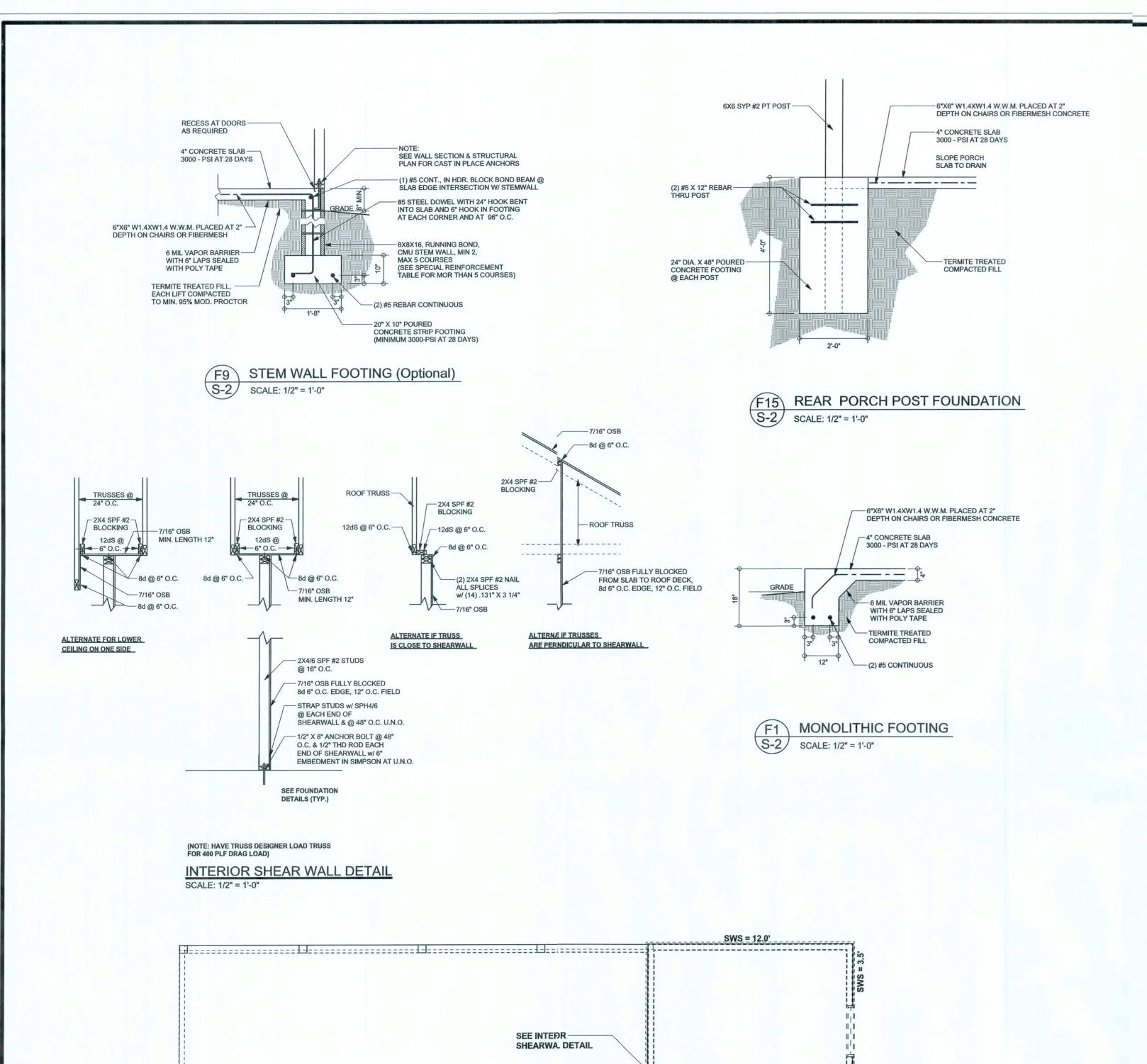
Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (336) 754 - 5419 Fax: (383) 269 - 4871

PRINTED DATE: April 03, 2008 CHECKED BY Ben Sparks

FINALS DATE: 1 / Apr / 08

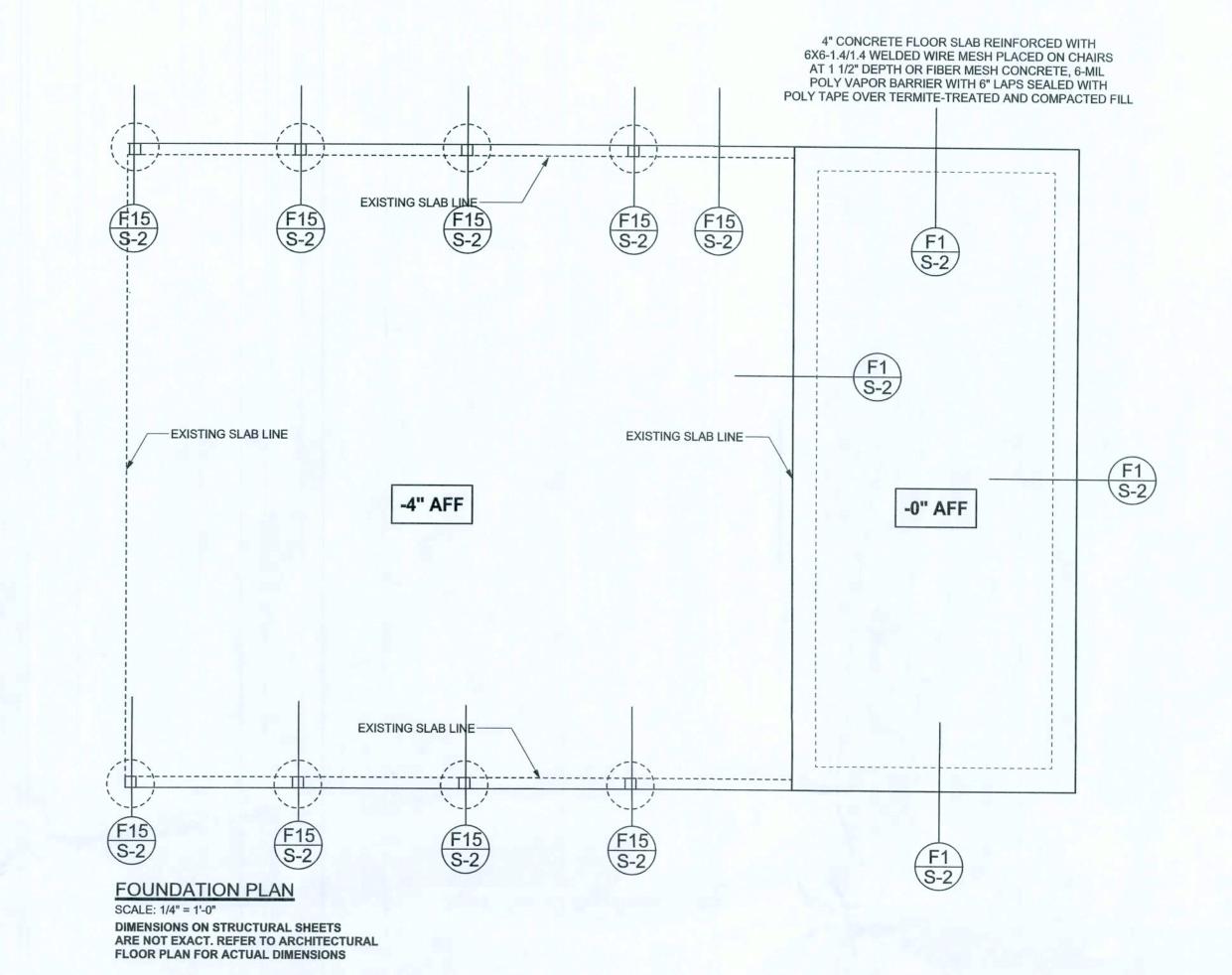
> JOB NUMBER: 8)3312 DRAWNG NUMBER

> > **S-1** OF : SHEETS



USE H2.5A (480lb) FOR ALL TRUSS TO WALL FRAME AND DRCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

STRUCTURAL PLAN SCALE: 1/4" = 1'-0"



PE No.53915, POB 86, Lake City, FL 32056, 386-754-5419 Stated dimensions suprcede scaled dimensions. Refer all uestions to Mark Disosway, P.E. fr resolution. Do not proceed withou clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. breby expressly reserves its common law copyrints and property right in these instruments of srvice. This document is not to be reproduced, Itered or copied in any form or manner withou first the express written permission and conset of Mark Disosway. CERTIFICATION: I heeby certify that I have examined this plan, and that the applicable portions of the plan, reating to wind engineerin comply with section R.01.2.1, florida building code residential 2004, o the best of my

WINDLOAD ENGINER: Mark Disosway,

REVISIONS

3/19/2008

4/08/2008

PERMIT #08-0414

SOFTPLAN

LIMITATION: This desgn is valid for one building, at specified loation.

MARK DSOSWAY P.E.53915

> Jonathin Perry Construction

Rene & Debie Dibut PoleBarn

> ADIRESS: 259 NW Mershon St. Lake Cit FL. 32055

Mark Disesway P.E. P.O. Fox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTID DATE: April 08,2008 DRAWN BY: CHECKED BY: Ben Sparks

FINALS DATE: 1 / Apr / 08

JOB NUMBER: 803312 DRAWING NUMBER **S-2**

OF 3 SHEETS

TOTAL SHEAR VWALL SEGMENTS

F = = = = F

SWS = 12.0'

SWS = 0.0' INDICATES S SHEAR WALL SEGMENTS REQUIRED ACTUAL TRANSVERSE 5.5.5' 14.0'

STRUCTURAL PLAN NOTES

LONGITUDINAL 8.8.0

ALL LOAD B BEARING FRAME WALL & PORCH HEADERS SHALL BE A A MINIMUM OF (2) 2X8 SYP #2 (U.N.O.)

24.0'

- ALL LOAD B BEARING FRAME WALL HEADERS SHALL HAVE/E (1) JACK STUD & (1) KING STUD EACH SIDE & (U.N.O.)
- SN-3

 DIMENSION NS ON STRUCTURAL SHEETS
 ARE NOT EXCACT. REFER TO ARCHITECTURAL
 FLOOR PLAIAN FOR ACTUAL DIMENSIONS

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

PERMANEN NT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS S AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BGCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACICKAGE

HEADER LEGEND (2) 2X8X0',1J 1K ← HEADER/BEAM CALL-OUT (U.N.O.) -NUMBER OF KING STUDS (FULL LENGTH) -NUMBER OF JACK STUDS (UNDER HEADER) -SPAN OF HEADER SIZE OF HEADER MATERIAL -NUMBER OF PLIES IN HEADER

> CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER ?? JOB #??