



RE: 3250262 - GIEGEIG CONST. - LOT 29 CW

MiTek USA, Inc.

16023 Swingley Ridge Rd

Chesterfield, MO 63017

Site Information:

Customer Info: GIEBEIG CONST. Project Name: Spec Hse Model: St. Johns Modified Subdivision: Crosswinds

Lot/Block: 29 Address: TBD SW Chesterfield Circle, TBD

City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Design Program: MiTek 20/20 8.5

Wind Code: ASCE 7-16

Wind Speed: 130 mph

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 25 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index she conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No. 1 2 3 4 5 6	Seal# T28368025 T28368026 T28368027 T28368028 T28368029 T28368030	Truss Name CJ01 CJ03 CJ05 EJ01 HJ10	Date 7/27/22 7/27/22 7/27/22 7/27/22 7/27/22	No. 15 16 17 18 19	Seal# T28368039 T28368040 T28368041 T28368042 T28368043 T28368043	Truss Name T09 T10 T11 T12 T13 T14	Date 7/27/22 7/27/22 7/27/22 7/27/22 7/27/22
6	T28368030	T01	7/27/22	20	T28368044	T14	7/27/22
1	T28368031 T28368032 T28368033	T01G T02 T03	7/27/22 7/27/22 7/27/22	21 22 23	T28368045 T28368046 T28368047	T15 T16 T17	7/27/22
8 9 10 11	T28368034 T28368035	T04 T05	7/27/22	24 25	T28368048 T28368049	T17G T18	7/27/22 7/27/22 7/27/22
12 13	T28368036 T28368037	T06 T07	7/27/22 7/27/22		. 200000 10		1121122
14	T28368038	T08	7/27/22				



This item has been electronically signed and sealed by Magid, Michael, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: Magid, Michael

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 27,2022



Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW	
			3300		CONSTRUCTION CONTROL SECTION S	T28368025
3250262	CJ01	Jack-Open	10	1		4
			15.75		Job Reference (optional)	

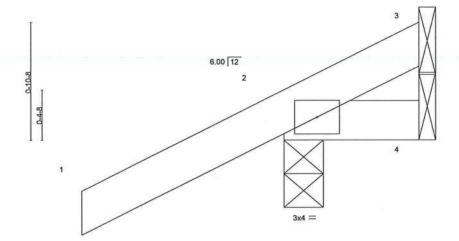
Builders FirstSource (Lake City,FL),

ID:fRijugoliQj9qlqT_5CiYdzq7NP-cN19ccQmlV6ktB8Q8yBHMTjaFYQcLAmkZ64oh5ytxfT

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:8.2



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.14	Vert(LL)	0.00	7	>999	240	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.03	Vert(CT)	0.00	7	>999	180	LIOUXNA CONTRACTOR		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a	100		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP						Weight: 6 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=39(LC 12)

Max Uplift 3=-6(LC 1), 2=-67(LC 12), 4=-19(LC 1) Max Grav 3=7(LC 16), 2=179(LC 1), 4=18(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

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Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chosterfield, MO 63017

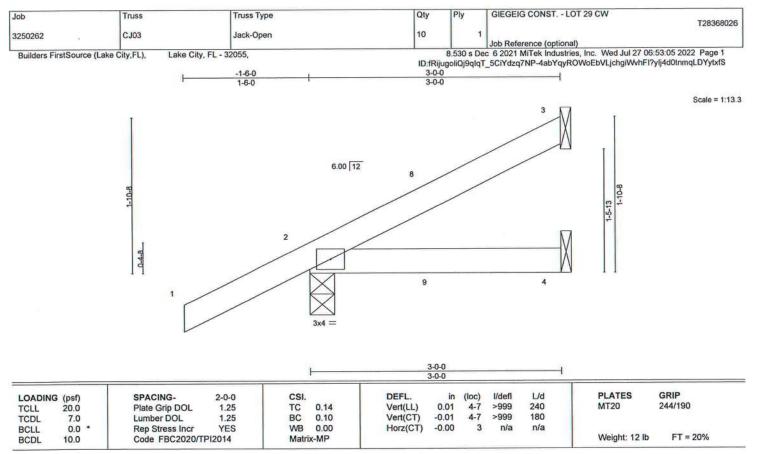
July 27,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TP/1 Quality Criteria, DSB-99 and BCSI Building Composafety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=73(LC 12)

Max Uplift 3=-35(LC 12), 2=-58(LC 12), 4=-16(LC 9) Max Grav 3=60(LC 1), 2=210(LC 1), 4=50(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

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July 27,2022

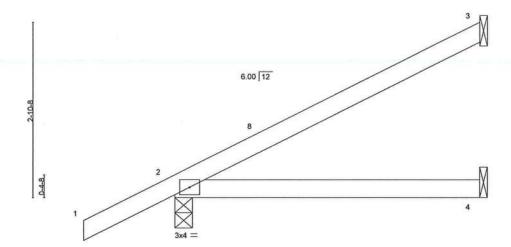
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Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW	
					T283	368027
3250262	CJ05	Jack-Open	10			
1.000.000.0000		Company (Class (Class))			Job Reference (optional)	
Builders FirstSource	ce (Lake City,FL), Lake	City, FL - 32055,		8.530 s D	ec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:05 2022 Pag	e 1
		400	ID:fRijug	goliQj9qlqT	_5CiYdzq7NP-4abYqyROWoEbVLjchgiWhFjmyjZ4d0tnmqLDYyt	xfS

Scale = 1:18.2



				-			-0-0 -0-0						
LOADING ((psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 2	20.0	Plate Grip DOL	1.25	TC	0.28	Vert(LL)	0.03	4-7	>999	240	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.24	Vert(CT)	-0.05	4-7	>999	180	SANSTOPATO S		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a			
	10.0	Code FBC2020/T	PI2014	Matri	x-MP						Weight: 18 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=107(LC 12)

Max Uplift 3=-67(LC 12), 2=-65(LC 12)

Max Grav 3=113(LC 1), 2=276(LC 1), 4=88(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-11-4 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.

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Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 27,2022

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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

GIEGEIG CONST. - LOT 29 CW Truss Type Qty Job Truss T28368028 3250262 EJ01 Jack-Partial 23 Job Reference (optional)
8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:06 2022 Page 1 Builders FirstSource (Lake City,FL), Lake City, FL - 32055, ID:fRijugoliQj9qlqT_5CiYdzq7NP-Ym9w1IS0H6MS7VlpFNDIRuooyMykp3G10QZvl_ytxfR Scale = 1:23.2 6.00 12 0-4-B 11 12 7-0-0 Plate Offsets (X,Y)-[2:0-1-13,0-1-8] GRIP PLATES LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d 244/190 240 TCLL 20.0 Plate Grip DOL 1.25 TC 0.70 Vert(LL) 0.30 4-7 >274 MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.69 Vert(CT) 0.26 4-7 >323 180 BCLL 0.0 * Rep Stress Incr YES WB 0.00 Horz(CT) -0.01 3 n/a n/a Weight: 25 lb FT = 20% Code FBC2020/TPI2014 BCDL 10.0 Matrix-MS BRACING-LUMBER-TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD 2x4 SP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD **BOT CHORD** 2x4 SP No.2

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=137(LC 12)

Max Uplift 3=-86(LC 12), 2=-76(LC 12), 4=-40(LC 9)

Max Grav 3=164(LC 1), 2=346(LC 1), 4=126(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
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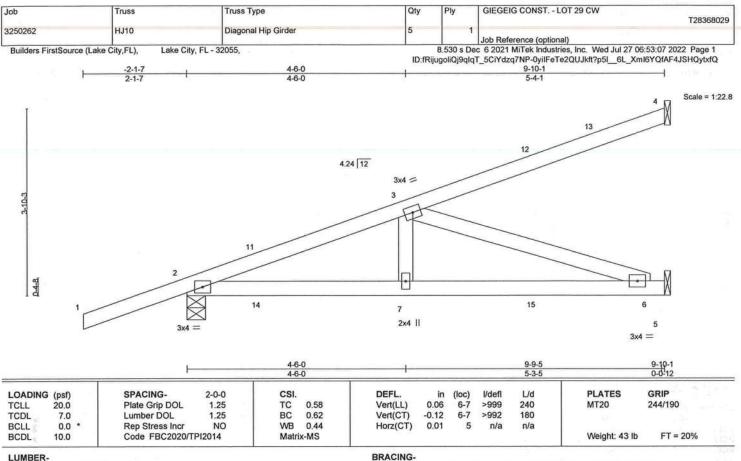
Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

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TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

2x4 SP No.2 TOP CHORD **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 WEBS

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical

Max Horz 2=149(LC 22)

Max Uplift 4=-77(LC 4), 2=-298(LC 4), 5=-142(LC 4) Max Grav 4=149(LC 1), 2=527(LC 1), 5=299(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-799/340

BOT CHORD 2-7=-395/729, 6-7=-395/729 3-7=-60/281, 3-6=-768/416 WEBS

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb)
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 59 lb down and 73 lb up at 1-6-1, 59 lb down and 73 lb up at 1-6-1, 22 lb down and 38 lb up at 4-4-0, 22 lb down and 38 lb up at 4-4-0, and 43 lb down and 78 lb up at 7-1-15, and 43 lb down and 78 lb up at 7-1-15 on top chord, and 41 lb down and 43 lb up at 1-6-1, 41 lb down and 43 lb up at 1-6-1, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, and 64 lb down at 7-1-15, and 64 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-6(F=-3, B=-3) 12=-73(F=-36, B=-36) 15=-59(F=-29, B=-29)

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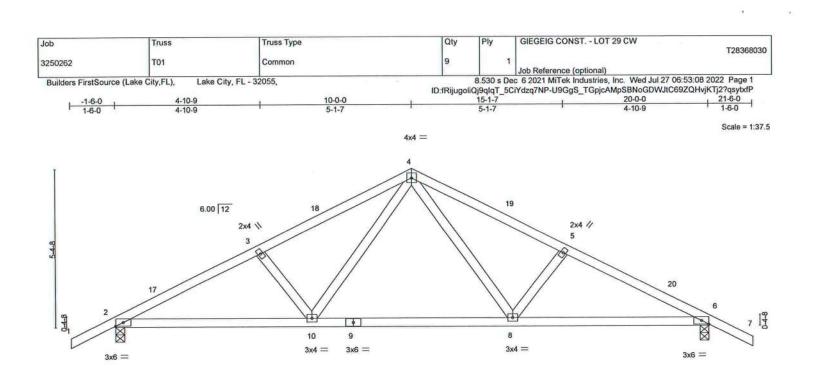
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Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-2-14 oc bracing.



		\vdash	6-7-7 6-7-7		-+-		13-4-9 6-9-1			_		6-7-7	——I
Plate Offs	ets (X	Y)-	[6:0-2-15,Edge]										
LOADING	(psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
CLL	20.0		Plate Grip DOL	1.25	TC	0.40	Vert(LL)	-0.17	8-10	>999	240	MT20	244/190
CDL	7.0		Lumber DOL	1.25	BC	0.93	Vert(CT)	-0.33	8-10	>729	180		
BCLL BCDL	10.0		Rep Stress Incr Code FBC2020/T	NO PI2014	WB Matri	0.26 x-MS	Horz(CT)	0.04	6	n/a	n/a	Weight: 94 lb	FT = 20%

LUMBER-

2x4 SP No.2 TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-2-10 oc purlins.

Rigid ceiling directly applied or 9-3-8 oc bracing.

REACTIONS.

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=85(LC 12)

Max Uplift 2=-241(LC 12), 6=-241(LC 13) Max Grav 2=1024(LC 1), 6=1024(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1764/517, 3-4=-1619/510, 4-5=-1619/510, 5-6=-1764/517

2-10=-378/1532, 8-10=-193/1019, 6-8=-392/1532 BOT CHORD

4-8=-194/680, 4-10=-194/680 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 10-0-0, Exterior(2R) 10-0-0 to 13-0-0, Interior(1) 13-0-0 to 21-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
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- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20

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Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

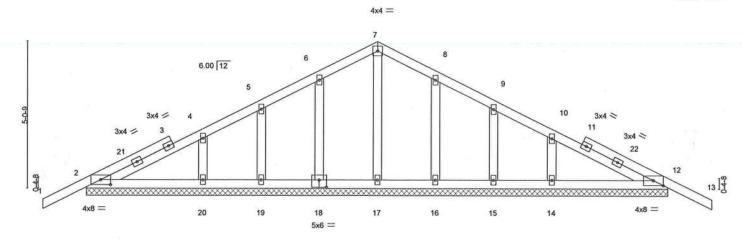
July 27,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obligase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW	T28368031
3250262	T01G	Common Supported Gable	1	1	Job Reference (optional)	120300031
Builders FirstSource	ce (Lake City,FL), Lake	City, FL - 32055,	ID:fRijug	8.530 s De	c 6 2021 MiTek Industries, Inc. Wed Jul 27 06: CiYdzq7NP-yLq2fKUua1k1 y10wVnS3XQS7Z6	
1-6-		10-0-0		25 30 25 77	20-0-0	21-6-0
1-6-	0	10-0-0			10-0-0	1-6-0

1-6-0 Scale = 1:38.1



				20-0-0							
2:0-4-0,0-2-1], [12:0-4-0,	0-2-1], [18:0-3-	-0,0-3-0]									
SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
Plate Grip DOL	1.25	TC	0.13	Vert(LL)	-0.00	13	n/r	120	MT20	244/190	
Lumber DOL	1.25	BC	0.10	Vert(CT)	-0.00	13	n/r	120	DOMOTORS -		
Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	12	n/a	n/a			
Code FBC2020/TI	PI2014	Matri	x-S						Weight: 105 lb	FT = 20%	
	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25	Plate Grip DOL 1.25 TC Lumber DOL 1.25 BC Rep Stress Incr YES WB	SPACING- 2-0-0 CSI. Plate Grip DOL 1.25 TC 0.13 Lumber DOL 1.25 BC 0.10 Rep Stress Incr YES WB 0.05	SPACING- 2-0-0 CSI. DEFL. Plate Grip DOL 1.25 TC 0.13 Vert(LL) Lumber DOL 1.25 BC 0.10 Vert(CT) Rep Stress Incr YES WB 0.05 Horz(CT)	20-0-0 20-0-0	20-0-0 20-0-0 20-0-0	20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 2	20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0 20-0-0	2:0-4-0,0-2-1], [12:0-4-0,0-2-1], [18:0-3-0,0-3-0] SPACING- Plate Grip DOL 1.25 TC 0.13 Vert(LL) -0.00 13 n/r 120 MT20 Lumber DOL 1.25 BC 0.10 Vert(CT) -0.00 13 n/r 120 Rep Stress Incr YES WB 0.05 Horz(CT) 0.00 12 n/a n/a	20-0-0 20-0-0 20-0-0 20-0-0 SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES GRIP Plate Grip DOL 1.25 TC 0.13 Vert(LL) -0.00 13 n/r 120 MT20 244/190 Lumber DOL 1.25 BC 0.10 Vert(CT) -0.00 13 n/r 120 Rep Stress Incr YES WB 0.05 Horz(CT) 0.00 12 n/a n/a

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **OTHERS** 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=-81(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 18, 19, 20, 16, 15, 14

Max Grav All reactions 250 lb or less at joint(s) 2, 12, 17, 18, 19, 20, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 10-0-0, Corner(3R) 10-0-0 to 13-0-0, Exterior(2N) 13-0-0 to 21-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 18, 19, 20, 16, 15, 14,
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 12.

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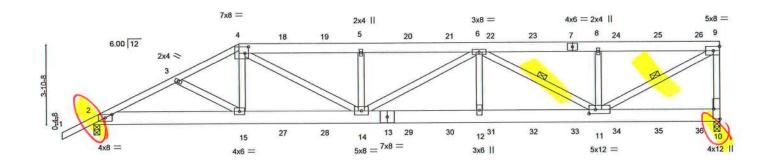
Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 27,2022



GIEGEIG CONST. - LOT 29 CW Truss Type Qty Ply Job Truss T28368032 T02 Half Hip Girder 3250262 Job Reference (optional) 8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:11 2022 Page 1 Builders FirstSource (Lake City,FL), Lake City FL - 32055 ID:fRijugoliQj9qlqT_5CiYdzq7NP-vkyp4?W96e_kDGAm2wpx8yVbPNlLU7vmAhHfQBytxfM 24-2-14 4-0-11

Scale = 1:53.4



		7-0-0 7-0-0	-	12-10-2 5-10-2	-1-	18-6-8 5-8-6	-+		24-2- 5-8-6		30-1-0 5-10-2	
Plate Offsets (X,Y)-		[2:0-4-0,0-1-15], [4:0-2-0,0-3-12], [10:Ec							000			
LOADING TCLL TCDL	(psf) 20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.85 0.28	DEFL. Vert(LL) Vert(CT)	0.26 1 -0.40 1		I/defl >999 >886	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 *	Rep Stress Incr Code FBC2020/Ti	NO PI2014	WB Matrix	0.87 c-MS	Horz(CT)	0.06	10	n/a	n/a	Weight: 223 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

2x6 SP No.2 *Except* TOP CHORD

1-4: 2x4 SP No.2

BOT CHORD 2x8 SP 2400F 2.0E 2x4 SP No.3 *Except* WEBS

4-14,6-14,6-11,9-11: 2x4 SP No.2

(size) 10=0-3-8, 2=0-3-8 REACTIONS. Max Horz 2=141(LC 8)

Max Uplift 10=-1166(LC 5), 2=-888(LC 8)

Max Grav 10=2560(LC 1), 2=2234(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-4390/1854, 3-4=-4271/1862, 4-5=-5311/2384, 5-6=-5307/2381, 6-8=-3375/1520,

TOP CHORD 8-9=-3375/1520, 9-10=-2376/1097

BOT CHORD 2-15=-1688/3878, 14-15=-1693/3837, 12-14=-2322/5165, 11-12=-2322/5165

4-15=-263/714, 4-14=-819/1753, 5-14=-714/365, 6-12=-123/496, 6-11=-2075/930,

8-11=-659/340, 9-11=-1736/3857

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 10=1166, 2=888.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 128 lb down and 90 lb up at 7-0-0, 110 lb down and 90 lb up at 9-0-12, 110 lb down and 90 lb up at 11-0-12, 110 lb down and 90 lb up at 13-0-12, 110 lb down and 90 lb up at 15-0-12, 110 lb down and 90 lb up at 17-0-12, 110 lb down and 90 lb up at 19-0-12, 110 lb down and 90 lb up at 21-0-12, 110 lb down and 90 lb up at 23-0-12, 110 lb down and 90 lb up at 25-0-12, 110 lb down and 90 lb up at 27-0-12, and 113 lb down and 90 lb up at 29-0-12, and 139 lb down and 88 lb up at 29-11-4 on top chord, and 335 lb down and 236 lb up at 7-0-0, 86 lb down and 60 lb up at 9-0-12, 86 lb down and 60 lb up at 11-0-12, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 15-0-12, 86 lb down and 60 lb up at 17-0-12, 86 lb down and 60 lb up at 19-0-12, 86 lb down and 60 lb up at 21-0-12, 86 lb down and 60 lb up at 23-0-12, 86 lb down and 60 lb up at 25-0-12, and 86 lb down and 60 lb up at 27-0-12, and 88 lb down and 59 Ib up at 29-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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July 27,2022

LOAD CARES VISIANDED HE arameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 2-7-7 oc purlins,

6-11, 9-11

Rigid ceiling directly applied or 7-10-3 oc bracing.

except end verticals.

1 Row at midpt

Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW T283680
3250262	T02	Half Hip Girder	1	1	Job Reference (optional)

Builders FirstSource (Lake City,FL),

Lake City, FL - 32055,

8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:11 2022 Page 2 ID:fRijugoliQj9qlqT_5CiYdzq7NP-vkyp4?W96e_kDGAm2wpx8yVbPNlLU7vmAhHfQBytxfM

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

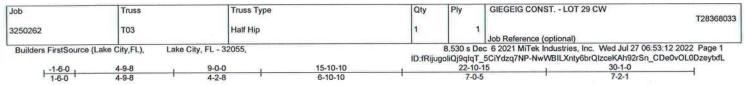
Uniform Loads (plf)

Vert: 1-4=-54, 4-9=-54, 2-10=-20

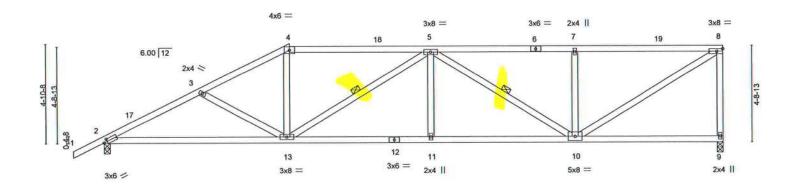
Concentrated Loads (lb)

Vert: 4=-110(F) 7=-110(F) 9=-139(F) 15=-335(F) 14=-64(F) 5=-110(F) 18=-110(F) 19=-110(F) 20=-110(F) 21=-110(F) 22=-110(F) 23=-110(F) 24=-110(F) 25=-110(F) 25=-110(F)





Scale = 1:54.1



	9-0-0			15-10-10				22-10-			30-1-0	
		9-0-0		6-10-10			7-0-5				7-2-1	
Plate Offs	ets (X,Y)-	[2:0-1-15,0-1-8]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.57	Vert(LL)	-0.13	13-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.75	Vert(CT)	-0.29	13-16	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.07	9	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS						Weight: 161 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (size) 9=0-3-8, 2=0-3-8

Max Horz 2=174(LC 12)

Max Uplift 9=-288(LC 9), 2=-212(LC 12)

Max Grav 9=1106(LC 1), 2=1191(LC 1)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-3=-2041/407, 3-4=-1818/390, 4-5=-1577/366, 5-7=-1412/364, 7-8=-1412/364,

8-9=-1042/304

BOT CHORD 2-13=-443/1796, 11-13=-487/1918, 10-11=-487/1918

WEBS 4-13=-57/519, 5-13=-506/180, 5-11=0/260, 5-10=-599/145, 7-10=-405/196,

8-10=-421/1638

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 9-0-0, Exterior(2R) 9-0-0 to 13-3-1, Interior(1) 13-3-1 to 29-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=288, 2=212.

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Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 27,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

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Structural wood sheathing directly applied or 3-10-11 oc purlins,

5-13, 5-10

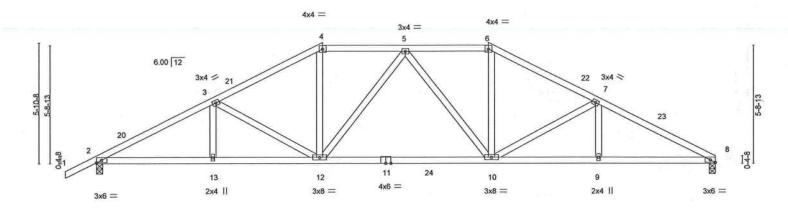
Rigid ceiling directly applied or 8-5-7 oc bracing.

except end verticals.

1 Row at midpt

Job	Truss	Truss Type		Qty	Ply	GIEGEIG CONST LO	T 29 CW	T28368034
3250262	T04	Híp		1	1	Job Reference (optional)		12000004
Builders FirstSource	(Lake City,FL), Lake	City, FL - 32055,	200000			c 6 2021 MiTek Industries /dzq7NP-r64ZVhXPeGFS	, Inc. Wed Jul 27 06:53:1	
, -1-6-0 ,	5-7-15	11-0-0	15-0-8	19-1-0	- 1	24-5-1	30-1-0	0.60)
1-6-0	5-7-15	5-4-1	4-0-8	4-0-8		5-4-1	5-7-15	

Scale = 1:53 9



	1	5-7-15 5-7-15	11-0-0 5-4-1			19-1-0 8-1-0		-		24-5-1 5-4-1	30-1-0 5-7-15		
Plate Offse	ets (X,Y)-	[8:0-2-15,Edge]										AV. Court	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.31	Vert(LL)	-0.23	10-12	>999	240	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.41	10-12	>877	180	7 PH M 3		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.30	Horz(CT)	0.09	8	n/a	n/a	75-12500 1.11		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	75 10					Weight: 156 lb	FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 8=0-3-8, 2=0-3-8 Max Horz 2=102(LC 12)

Max Uplift 8=-238(LC 13), 2=-270(LC 12) Max Grav 8=1202(LC 2), 2=1271(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-2228/431, 3-4=-1821/373, 4-5=-1572/363, 5-6=-1575/359, 6-7=-1825/375,

7-8=-2233/442

BOT CHORD WEBS

2-13=-407/1953, 12-13=-407/1953, 10-12=-242/1657, 9-10=-338/1969, 8-9=-338/1969 3-12=-428/188, 4-12=-80/599, 5-12=-273/116, 5-10=-271/114, 6-10=-80/602,

7-10=-442/198

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 11-0-0, Exterior(2R) 11-0-0 to 15-0-8, Interior(1) 15-0-8 to 19-1-0, Exterior(2R) 19-1-0 to 23-4-1, Interior(1) 23-4-1 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=238, 2=270,

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Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd, Chesterfield, MO 63017

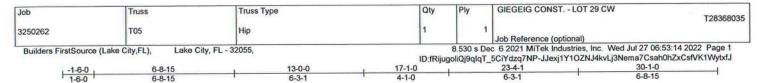
July 27,2022

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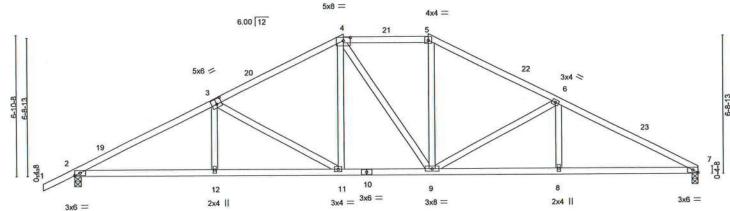


Structural wood sheathing directly applied or 3-8-7 oc purlins.

Rigid ceiling directly applied or 9-2-11 oc bracing.







	6-8-15	10	13-0-0	17-1-0	1	23-4-1		30-1-0	
	6-8-15		6-3-1	4-1-0	- 1	6-3-1		6-8-15	
Plate Offsets (X,Y)-	[3:0-3-0,0-3-0], [4:0-4-0,0	-1-15], [7:0-2-1	5,Edge]						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.45 BC 0.60 WB 0.53 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.09 11 -0.19 11-12 0.08 7	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 156 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-8-1 oc purlins. Rigid ceiling directly applied or 9-3-4 oc bracing.

20 1 0

REACTIONS.

(size) 7=0-3-8, 2=0-3-8

Max Horz 2=118(LC 12)

Max Uplift 7=-234(LC 13), 2=-267(LC 12) Max Grav 7=1111(LC 1), 2=1196(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2035/413, 3-4=-1513/362, 4-5=-1278/354, 5-6=-1516/362, 6-7=-2051/421

BOT CHORD 2-12=-394/1760, 11-12=-395/1758, 9-11=-197/1276, 8-9=-315/1777, 7-8=-315/1777

WEBS 3-12=0/273, 3-11=-553/226, 4-11=-80/396, 5-9=-71/396, 6-9=-571/235, 6-8=0/274

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 13-0-0, Exterior(2E) 13-0-0 to 17-1-0, Exterior(2R) 17-1-0 to 21-4-1, Interior(1) 21-4-1 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=234, 2=267.

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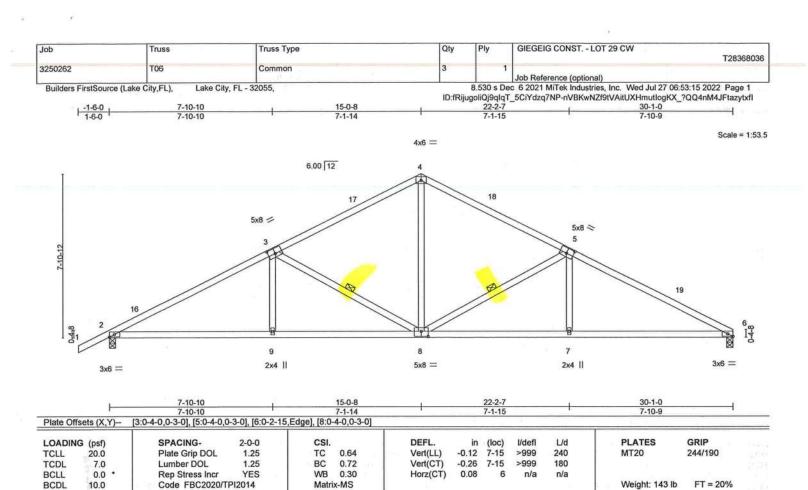
Michael S. Magid PE No.53681 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 27,2022

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LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

BRACING-TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied or 3-3-5 oc purlins. Rigid ceiling directly applied or 9-3-6 oc bracing.

5-8.3-8 1 Row at midpt

REACTIONS.

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=134(LC 12)

Max Uplift 2=-264(LC 12), 6=-231(LC 13) Max Grav 2=1196(LC 1), 6=1111(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1984/390, 3-4=-1352/330, 4-5=-1353/336, 5-6=-1994/401 2-9=-381/1704, 8-9=-382/1701, 7-8=-286/1712, 6-7=-286/1715 TOP CHORD **BOT CHORD**

WEBS

4-8=-142/792, 5-8=-692/281, 5-7=0/320, 3-8=-679/274, 3-9=0/318

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 15-0-8, Exterior(2R) 15-0-8 to 18-0-10, Interior(1) 18-0-10 to 30-1-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arin DOI =1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=264, 6=231.

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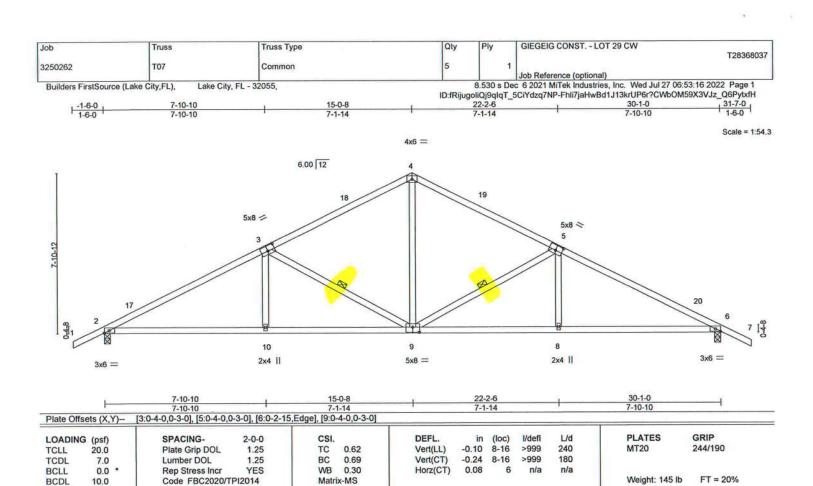
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BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 6=0-3-8

Max Horz 2=123(LC 16) Max Uplift 2=-263(LC 12), 6=-263(LC 13)

Max Grav 2=1194(LC 1), 6=1194(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1979/390, 3-4=-1348/329, 4-5=-1348/329, 5-6=-1979/390 BOT CHORD 2-10=-370/1700, 9-10=-370/1697, 8-9=-258/1697, 6-8=-257/1700 WEBS 4-9=-135/787, 5-9=-678/274, 5-8=0/318, 3-9=-678/274, 3-10=0/318

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 15-0-8, Exterior(2R) 15-0-8 to 18-0-10, Interior(1) 18-0-10 to 31-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=263, 6=263.

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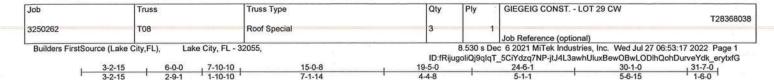


Structural wood sheathing directly applied or 3-4-12 oc purlins.

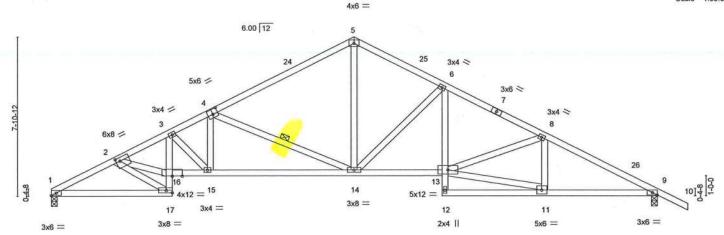
5-9, 3-9

Rigid ceiling directly applied or 9-5-1 oc bracing.

1 Row at midpt







	1:	0-0-0	10-10	15-0-8	1 19-0-0	24-0-1	30-1-0	
		6-0-0 1-	10-10	7-1-14	4-4-8	5-1-1	5-6-15	58.
Plate Offs	sets (X,Y)	[2:0-2-4,0-2-0], [4:0-3-0,	0-3-0], [9:0-2-1	5,Edge]				
LOADING	G (psf)	SPACING-	2-0-0	CSI.	DEFL. in (loc	c) I/defl L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC 0.62	Vert(LL) -0.17 1	5 >999 240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC 0.76	Vert(CT) -0.35 14-19	5 >999 180	A Machinary	
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.84	Horz(CT) 0.17	9 n/a n/a		
BCDL	10.0	Code FBC2020/7	PI2014	Matrix-MS			Weight: 172 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

6-12: 2x4 SP No.3

2x4 SP No.3

WEBS

REACTIONS. (size) 1=0-3-8, 9=0-3-8 Max Horz 1=-134(LC 17)

Max Uplift 1=-231(LC 12), 9=-264(LC 13)

Max Grav 1=1111(LC 1), 9=1196(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2148/458, 2-3=-2778/593, 3-4=-2467/525, 4-5=-1541/351, 5-6=-1501/353, 6-8=-2033/414, 8-9=-2076/411

1-17=-472/1886, 16-17=-235/979, 3-16=-112/399, 15-16=-553/2519, 14-15=-480/2225, BOT CHORD

13-14=-240/1773, 6-13=-75/445, 9-11=-287/1804

2-17=-1822/489, 2-16=-510/2198, 3-15=-396/101, 4-15=-45/521, 4-14=-1012/352, WEBS

5-14=-180/993, 6-14=-661/232, 11-13=-281/1727

NOTES.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B: Encl.. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-1-1, Interior(1) 3-1-1 to 15-0-8, Exterior(2R) 15-0-8 to 18-0-10, Interior(1) 18-0-10 to 31-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=231, 9=264.

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801

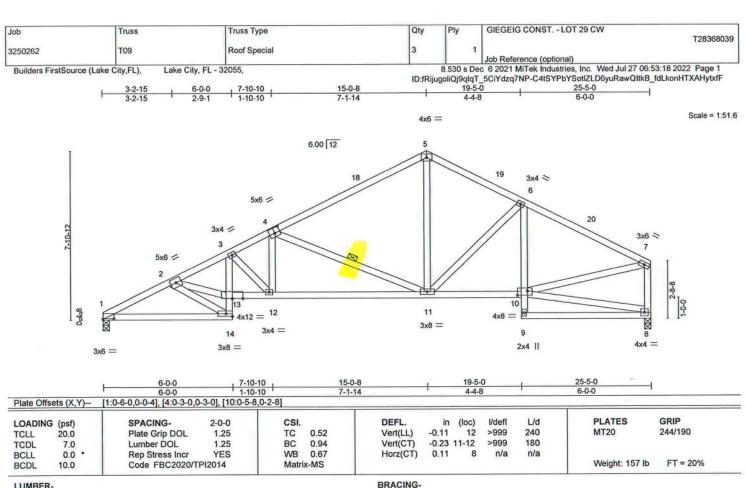


Structural wood sheathing directly applied or 3-2-10 oc purlins.

4-14

Rigid ceiling directly applied or 7-11-0 oc bracing.

1 Row at midpt



TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

3-14,6-9: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

(size) 1=0-3-8, 8=0-3-8 Max Horz 1=159(LC 12)

Max Uplift 1=-203(LC 12), 8=-174(LC 13)

Max Grav 1=935(LC 1), 8=935(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-1773/411, 2-3=-2241/536, 3-4=-1954/468, 4-5=-1090/300, 5-6=-1053/315,

6-7=-1176/296, 7-8=-877/238 1-14=-477/1556, 13-14=-239/821, 3-13=-114/349, 12-13=-559/2017, 11-12=-485/1768, **BOT CHORD**

10-11=-208/991

2-14=-1510/497, 2-13=-519/1766, 3-12=-334/102, 4-12=-46/482, 4-11=-954/354,

5-11=-133/600, 7-10=-208/984

NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-1-1, Interior(1) 3-1-1 to 15-0-8, Exterior(2R) 15-0-8 to 18-0-8, Interior(1) 18-0-8 to 25-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=203, 8=174.

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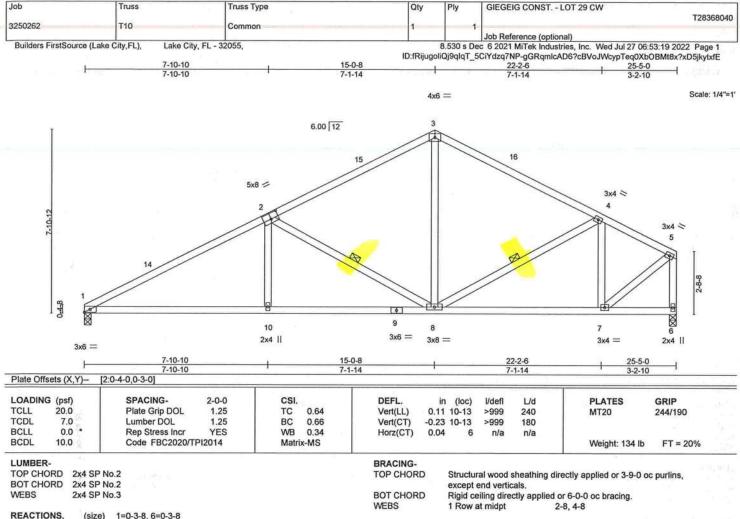
Structural wood sheathing directly applied or 3-7-9 oc purlins,

4-11

Rigid ceiling directly applied or 2-2-0 oc bracing.

except end verticals.

1 Row at midpt



(size) 1=0-3-8, 6=0-3-8 Max Horz 1=159(LC 12)

Max Uplift 1=-203(LC 12), 6=-174(LC 13)

Max Grav 1=935(LC 1), 6=935(LC 1)

TOP CHORD

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 1-2=-1608/361, 2-3=-966/281, 3-4=-961/278, 4-5=-781/195, 5-6=-924/220

BOT CHORD

1-10=-381/1370, 8-10=-381/1367, 7-8=-159/698

WEBS

2-10=0/318, 2-8=-693/281, 3-8=-79/487, 4-7=-444/166, 5-7=-207/900

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 15-0-8, Exterior(2R) 15-0-8 to 18-0-8, Interior(1) 18-0-8 to 25-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=203, 6=174.

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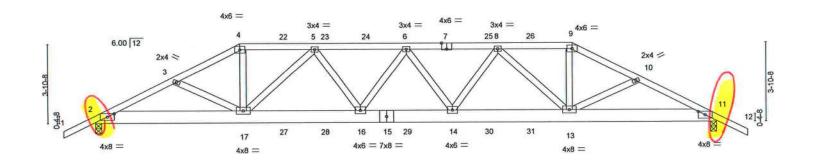
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Job	Truss		Truss Type		Qty	Ply	GIEGEIG CO	ONST LOT 29 CW		T28368041
3250262	T11		Hip Girder		1	1	Job Reference	e (optional)		,
Builders FirstSou	urce (Lake City,FL),	Lake City, F	L - 32055,				c 6 2021 MiTe	k Industries, Inc. W		
-1-6-0	3-10-15	7-0-0	10-7-2	15-0-8	19-5-15		23-1-0	26-2-1	30-1-0	31-7-0
1-6-0	3-10-15	3-1-1	3-7-2	4-5-7	4-5-7		3-7-2	3-1-1	3-10-15	1-6-0

Scale = 1:53.9



	4	7-0-0	1	12-9-13	ř.	17-3-3		23-	1-0	T	30-1-0	
		7-0-0	7	5-9-13		4-5-6		5-9	-13		7-0-0	
Plate Offse	ts (X,Y)-	[2:0-4-0,0-1-15], [7:0-3-0,	Edge], [11:0-4-	0,0-1-15]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.25	TC	0.75	Vert(LL)	0.28	14-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.27	Vert(CT)	-0.44	14-16	>805	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.65	Horz(CT)	0.08	11	n/a	n/a	Vertical control of the control of t	
BCDL	10.0	Code FBC2020/T	PI2014	Matrix	K-MS						Weight: 200 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x8 SP 2400F 2.0E

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 11=0-3-8

Max Horz 2=63(LC 12)

Max Uplift 2=-944(LC 8), 11=-961(LC 9) Max Grav 2=2261(LC 1), 11=2301(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-4424/1925, 3-4=-4275/1885, 4-5=-3863/1737, 5-6=-5194/2304, 6-8=-5216/2303,

8-9=-3938/1769, 9-10=-4361/1921, 10-11=-4509/1961

BOT CHORD 2-17=-1710/3913, 16-17=-2096/4849, 14-16=-2328/5369, 13-14=-2104/4892,

11-13=-1679/3989

WEBS 4-17=-681/1584, 5-17=-1429/676, 5-16=-299/660, 6-16=-350/187, 6-14=-301/159,

8-14=-273/615, 8-13=-1375/643, 9-13=-657/1549

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=944, 11=961.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 128 lb down and 90 lb up at 7-0-0, 110 lb down and 90 lb up at 9-0-12, 110 lb down and 90 lb up at 11-0-12, 110 lb down and 90 lb up at 13-0-12, 110 lb down and 83 lb up at 15-0-8, 110 lb down and 90 lb up at 17-0-4, 110 lb down and 90 lb up at 19-0-4, and 110 lb down and 90 lb up at 21-0-4, and 230 lb down and 173 lb up at 23-1-0 on top chord, and 335 lb down and 236 lb up at 7-0-0, 86 lb down and 60 lb up at 9-0-12, 86 lb down and 60 lb up at 11-0-12, 86 lb down and 60 lb up at 15-0-4, 86 lb down and 60 lb up at 15-0-4, and 335 lb down and 60 lb up at 15-0-4, and 335 lb down and 60 lb up at 17-0-4. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

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Michael S. Magid PE No.53681 MiTek Inc, DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 27,2022

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-0-9 oc purlins.

Rigid ceiling directly applied or 7-10-6 oc bracing.

Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW	
3250262		III 6: I	F		TO SERVICE HER CONTRACT TO CONTRACT TO SERVICE STATE OF THE SERVICE STAT	T28368041
3250262	T11	Hip Girder	1	1	Job Reference (optional)	

Builders FirstSource (Lake City,FL),

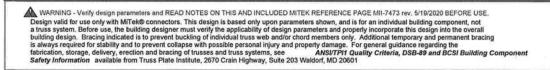
Lake City, FL - 32055,

8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:21 2022 Page 2 ID:fRijugoliQj9qlqT_5CiYdzq7NP-cfZbBQeQljFKQoxhd1?HY3wLOP9oqiuETFiBncytxfC

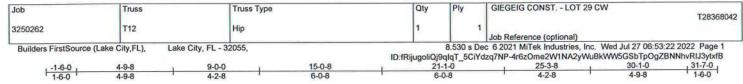
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-4=-54, 4-9=-54, 9-12=-54, 2-11=-20

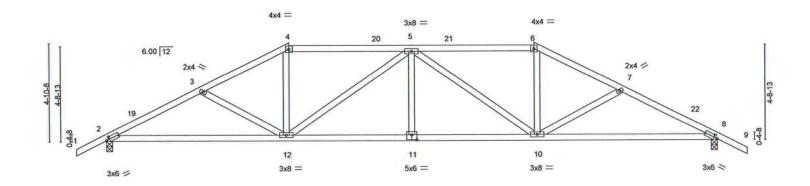
Vert: 1-4-54, 4-5-64, 5-12-54, 2-1-25 Concentrated Loads (lb) Vert: 4=-110(B) 7=-110(B) 9=-182(B) 17=-335(B) 16=-64(B) 6=-110(B) 14=-64(B) 13=-335(B) 22=-110(B) 23=-110(B) 24=-110(B) 25=-110(B) 26=-110(B) 27=-64(B) 28=-64(B) 29=-64(B) 30=-64(B) 31=-64(B)







Scale = 1:54.8



		9-0-0			15-0-8	1	21-1-0				30-1-0	
		9-0-0			6-0-8	1	6-0-8		1		9-0-0	- A:
Plate Offse	ets (X,Y)-	[2:0-1-15,0-1-8], [8:0-1-1	5,0-1-8], [11:0-	3-0,0-3-0]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.41	Vert(LL)	-0.14 1	2-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.74	Vert(CT)	-0.30 1	2-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.57	Horz(CT)	0.09	8	n/a	n/a	91028 Co AF-1/74906-6207/65	realist parameter
BCDL	10.0	Code FBC2020/T	PI2014	Matrix	-MS	TANGE AND LINES					Weight: 152 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

(size) 2=0-3-8, 8=0-3-8

Max Horz 2=76(LC 12)

Max Uplift 2=-273(LC 12), 8=-273(LC 13) Max Grav 2=1194(LC 1), 8=1194(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2049/455, 3-4=-1822/386, 4-5=-1578/374, 5-6=-1578/374, 6-7=-1822/386,

7-8=-2049/455

BOT CHORD 2-12=-409/1804, 11-12=-329/1923, 10-11=-329/1923, 8-10=-333/1804

WEBS 4

4-12=-74/530, 5-12=-519/167, 5-10=-519/167, 6-10=-74/530

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 9-0-0, Exterior(2R) 9-0-0 to 13-3-1, Interior(1) 13-3-1 to 21-1-0, Exterior(2R) 21-1-0 to 25-5-6, Interior(1) 25-5-6 to 31-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=273, 8=273.

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July 27,2022

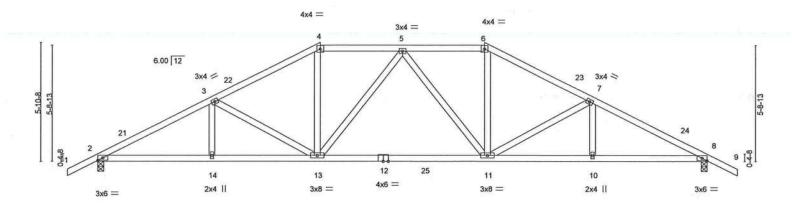


Structural wood sheathing directly applied or 3-11-5 oc purlins.

Rigid ceiling directly applied or 9-2-9 oc bracing.

Job	Truss		Truss Type			Qty	Ply	GIEGEIG CONST L	OT 29 CW	
112										T28368043
3250262	T13		Hip			1		1		
								Job Reference (options	al)	
Builders FirstSource (Lake City,FL),	Lake City, Fl	32055,				8.530 s D	ec 6 2021 MiTek Industri	ies, Inc. Wed Jul 27 06:53:	23 2022 Page 1
						ID:	fRijugoliQj9	qqqT_5CiYdzq7NP-Y1gLl	b6fgHKV1f654IS1IdU?o1Cj	tli?XwZBIsVytxfA
, -1-6-0 ,	5-7-15	1	11-0-0	1 .	15-0-8	19-1-0	1	24-5-1	30-1-0	, 31-7-0
1-6-0	5-7-15		5-4-1		4-0-8	4-0-8		5-4-1	5-7-15	1-6-0

Scale = 1:54.8



1		5-7-15	11-0-0	-		19-1-0		-1	24-		30-1-0		
Plate Offsets (X	,Y)	5-7-15 [8:0-2-15,Edge]	5-4-1			8-1-0			5-4	1-1	5-7-15		
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	(e) i
TCLL 20.0		Plate Grip DOL	1.25	TC	0.29	Vert(LL)	-0.23	11-13	>999	240	MT20	244/190	
TCDL 7.0	6	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.41	11-13	>880	180	0		
BCLL 0.0	•	Rep Stress Incr	YES	WB	0.29	Horz(CT)	0.09	8	n/a	n/a			
BCDL 10.0	1	Code FBC2020/T	PI2014	Matri	x-MS						Weight: 158 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-9-8 oc purlins.

Rigid ceiling directly applied or 9-4-4 oc bracing.

REACTIONS.

(size) 2=0-3-8, 8=0-3-8

Max Horz 2=-91(LC 17)

Max Uplift 2=-270(LC 12), 8=-270(LC 13) Max Grav 2=1270(LC 2), 8=1270(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-2225/430, 3-4=-1818/366, 4-5=-1569/357, 5-6=-1569/357, 6-7=-1818/366, 7-8=-2225/431

BOT CHORD 2-14=-396/1950, 13-14=-396/1950, 11-13=-224/1653, 10-11=-307/1950, 8-10=-307/1950 WEBS 3-13=-429/188, 4-13=-79/598, 5-13=-271/115, 5-11=-271/114, 6-11=-79/598,

7-11=-429/189

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 11-0-0, Exterior(2R) 11-0-0 to 15-0-8, Interior(1) 15-0-8 to 19-1-0, Exterior(2R) 19-1-0 to 23-4-1, Interior(1) 23-4-1 to 31-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=270. 8=270.

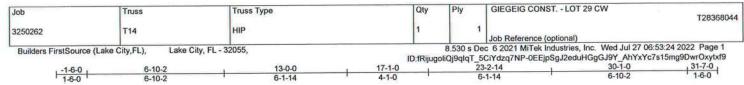
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Michael S. Magid PE No.53681 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

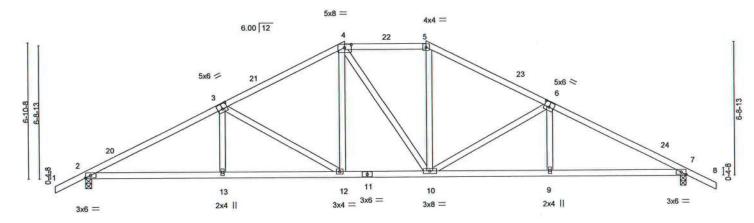
July 27,2022

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITE® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obligase with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Scale = 1:55.7



	—	6-10-2 6-10-2		13-0-0 6-1-14		17-1-0 4-1-0	1		-2-14 -1-14		30-1-0 6-10-2	
Plate Offse	ets (X,Y)-		-1-15], [6:0-3-0	The second liverage and the se	2-15,Edge]							
LOADING TCLL TCDL	(psf) 20.0 7.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.43 0.57	DEFL. Vert(LL) Vert(CT)	in -0.09 -0.19	(loc) 12 12-13	I/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0 *	Rep Stress Incr Code FBC2020/T	YES PI2014	WB Matrix	0.51 c-MS	Horz(CT)	0.08	7	n/a	n/a	Weight: 158 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-9-8 oc purlins. Rigid ceiling directly applied or 9-4-12 oc bracing.

REACTIONS.

(size) 2=0-3-8, 7=0-3-8

Max Horz 2=107(LC 12)

Max Uplift 2=-267(LC 12), 7=-267(LC 13) Max Grav 2=1194(LC 1), 7=1194(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2029/411, 3-4=-1508/355, 4-5=-1272/352, 5-6=-1509/355, 6-7=-2028/411 2-13=-383/1754, 12-13=-383/1756, 10-12=-185/1272, 9-10=-291/1756, 7-9=-291/1754 WEBS 3-13=0/273, 3-12=-556/226, 4-12=-80/395, 5-10=-71/396, 6-10=-555/226, 6-9=0/272

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-2, Interior(1) 1-6-2 to 13-0-0, Exterior(2E) 13-0-0 to 17-1-0, Exterior(2R) 17-1-0 to 21-4-1, Interior(1) 21-4-1 to 31-7-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=267, 7=267.

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July 27,2022

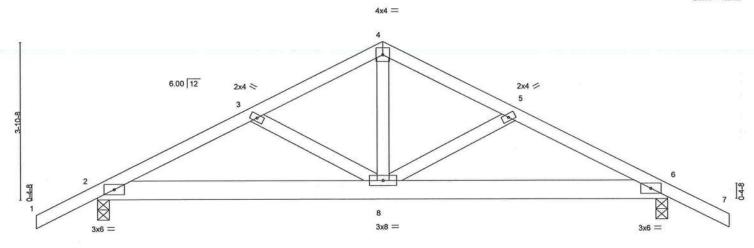
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see __ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW	70000004
		OUEFNEOOF				T28368045
3250262	T15	QUEENPOST	11		Job Reference (optional)	
Builders FirstSource	e (Lake City,FL), Lake City, F	L - 32055,	ID:fRiju		Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06 gt_5CiYdzq7NP-UQo50ohxpyllvQFSst3Div4Ao0V	
-1-6-0	3-10-15	7-0-0	, 1	0-1-1	14-0-0	15-6-0
160	2 10 15	3.1.1		3-1-1	3,10,15	1-6-0

Scale = 1:27.3



1	7-0-0 7-0-0		14-0-0						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2020/TPI2014	CSI. TC 0.15 BC 0.34 WB 0.35 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) 0.04 8-10 >999 240 Vert(CT) -0.07 8-10 >999 180 Horz(CT) 0.02 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 76 lb FT = 20%					

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS

2x4 SP No.3

(size) 2=0-3-8, 6=0-3-8 REACTIONS.

Max Horz 2=-63(LC 13)

Max Uplift 2=-322(LC 8), 6=-322(LC 9) Max Grav 2=906(LC 1), 6=906(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1499/574, 3-4=-1326/546, 4-5=-1326/546, 5-6=-1499/574 TOP CHORD

2-8=-511/1321, 6-8=-468/1321 BOT CHORD

4-8=-389/906 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=322, 6=322.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 101 lb down and 89 lb up at 7-0-0 on top chord, and 541 lb down and 351 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 2-6=-20, 1-4=-54, 4-7=-54

Concentrated Loads (lb)

Vert: 4=-73(B) 8=-541(B)

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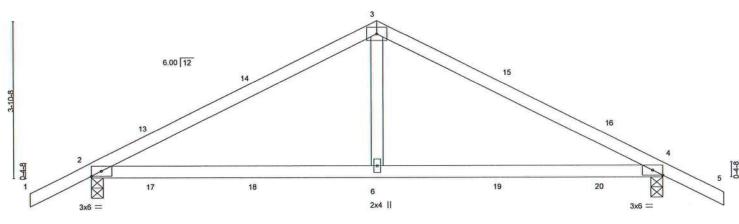
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Structural wood sheathing directly applied or 4-8-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

GIEGEIG CONST. - LOT 29 CW Qty Job Truss Truss Type Ply T28368046 2 3250262 T16 Common Job Reference (optional) 8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:25 2022 Page 1 Lake City, FL - 32055, Builders FirstSource (Lake City,FL), ID:fRijugoliQj9qlqT_5CiYdzq7NP-UQo50ohxpyllvQFSst3Div44o0U4me3qOtgPwNytxf8 -1-6-0 1-6-0 7-0-0 Scale = 1:27.3 4x6 =



	7-0-0						14-0-0					
	1	7-0-0					7-0-0					
Plate Offsets (X,Y) [4:0-2-15,Edge]												
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.53	Vert(LL)	0.14	6-12	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.13	6-9	>999	180	2000000111	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.01	4	n/a	n/a	A000087 (2008)	
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS	CARCORDAN 44783					Weight: 54 lb	FT = 20%

BRACING-TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

(size) 2=0-3-8, 4=0-3-8

Max Horz 2=63(LC 12) Max Uplift 2=-148(LC 9), 4=-148(LC 8)

Max Grav 2=599(LC 1), 4=599(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-719/787, 3-4=-719/788 BOT CHORD 2-6=-588/575, 4-6=-588/575

3-6=-440/320 WEBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 7-0-0, Exterior(2R) 7-0-0 to 10-0-0, Interior(1) 10-0-0 to 15-6-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=148, 4=148.

This item has been electronically signed and sealed by Magid, Michael, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Michael S. Magid PE. No. 53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 27,2022

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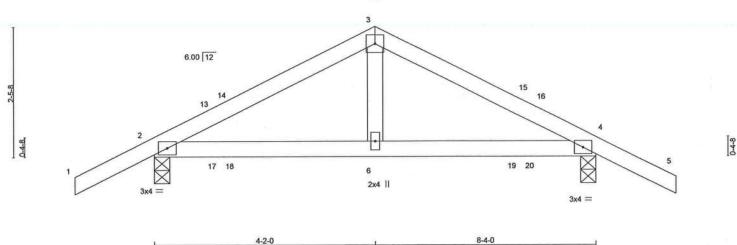
Structural wood sheathing directly applied or 5-11-14 oc purlins.

Rigid ceiling directly applied or 6-11-15 oc bracing.

Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 C	.w	T28368047
3250262	T17	Common	1	1	Job Reference (optional)		
Builders FirstSou	rce (Lake City,FL),	Lake City, FL - 32055,	ID:fRijugo		ec 6 2021 MiTek Industries, Inc. iCiYdzq7NP-zcMUE8hZaFtcWaq		
	-1-6-0	4-2-0	i		8-4-0	9-10-0	1
. 3	1-6-0 4-2-0		4-2-0 1-6-0			1-6-0	1

4x4 =

Scale = 1:20.9



	H		4-2-0 4-2-0					8-4-0 4-2-0			
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.24	Vert(LL)	0.03	6-9	>999	240	MT20	244/190
CDL 7.0	Lumber DOL	1.25	BC	0.18	Vert(CT)	-0.02	6-9	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(CT)	-0.00	4	n/a	n/a		
BCDL 10.0	Code FBC2020/T	PI2014	Matri	x-MS	100.00					Weight: 34 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.2 TOP CHORD **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 4=0-3-8

Max Horz 2=42(LC 12)

Max Uplift 2=-98(LC 12), 4=-98(LC 13) Max Grav 2=389(LC 1), 4=389(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-390/544, 3-4=-390/544 2-6=-378/310, 4-6=-378/310

BOT CHORD WEBS 3-6=-286/180

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-2-0, Exterior(2R) 4-2-0 to 7-2-0, Interior(1) 7-2-0 to 9-10-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.

This item has been electronically signed and sealed by Magid, Michael, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Michael S. Magid PE. No.53681 MiTek Inc. DBA MiTek USA FL. Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

July 27,2022

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeNS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucollapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

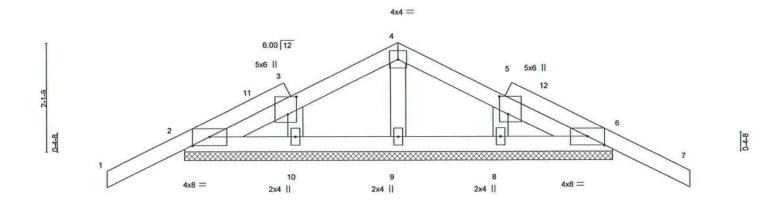


Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-2-14 oc bracing.

Job	Truss	Truss Type	Qty	Ply	GIEGEIG CONST LOT 29 CW				
2000000				550		T28368048			
3250262	T17G	GABLE	1	1	SERVICE CONTROL MANAGEMENT OF THE CONTROL OF THE CO				
					Job Reference (optional)				
Builders FirstSour	Builders FirstSource (Lake City, FL), Lake City, FL - 32055,			8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Jul 27 06:53:27 2022 Page 1					
		CONTRACTOR OF STREET, CANADA CONTRACTOR OF	ID:fRijugoli	Qj9qlqT_50	CiYdzq7NP-RpwsRUiBLZ?T8jPr_H6i	hoKAV3qGuEZw7rB9W?Gytxf6			
	-1-6-0	4-2-0			8-4-0	9-10-0			
	1-6-0	4-2-0			4-2-0	1-6-0			

Scale = 1:21.6



8-4-0 Plate Offsets (X,Y)— [2:0-4-0,0-2-1], [3:0-4-2,0-2-0], [5:0-4-2,0-2-0], [6:0-4-0,0-2-1]												
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.16	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.03	Vert(CT)	-0.01	7	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	6	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-S						Weight: 40 lb	FT = 20%

8-4-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-4-0.

(lb) - Max Horz 2=37(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 2, 6, 9, 10, 8

Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

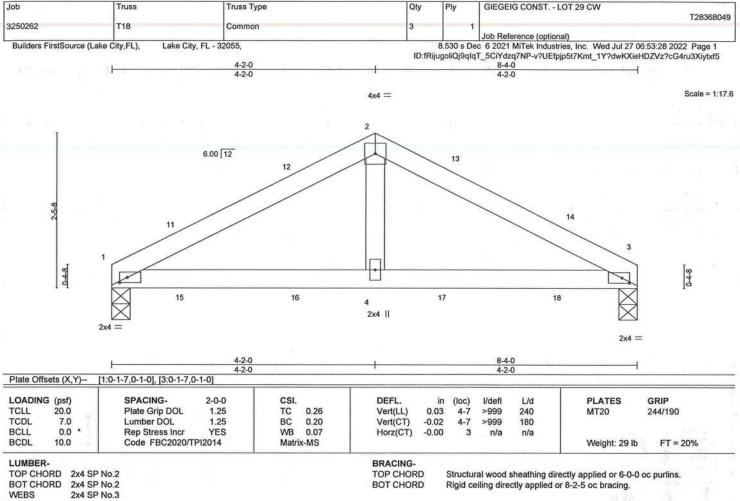
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 4-2-0, Corner(3R) 4-2-0 to 7-2-0, Exterior(2N) 7-2-0 to 9-10-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6, 9, 10, 8.

This item has been electronically signed and sealed by Magid, Michael, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Michael S. Magid PE No.53681 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Dates

July 27,2022





REACTIONS.

(size) 1=0-3-8, 3=0-3-8

Max Horz 1=-31(LC 13)

Max Uplift 1=-82(LC 9), 3=-82(LC 8)

Max Grav 1=308(LC 1), 3=308(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-430/601, 2-3=-430/601

BOT CHORD

1-4=-479/349, 3-4=-479/349

WEBS

2-4=-321/186

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-2-0, Exterior(2R) 4-2-0 to 7-2-0, Interior(1) 7-2-0 to 8-4-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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July 27,2022



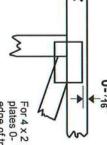
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹18" from outside edge of truss.

||

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

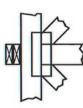
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

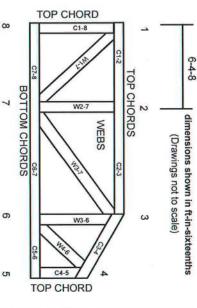
Industry Standards:

ANSI/TPI1: National Design Specification for Metal
Plate Connected Wood Truss Construction.

DSB-89

Design Standard for Bracing.
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

ω

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

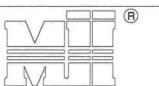
5.

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

T-Brace size

Nails

Web

Nails

2x4 or 2x6 or 2x8

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nail Spacing

6" o.c.

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

10d (0.131" X 3")

Nailing Pattern

Nail Size

	11	Nails	
		SPACING	
WEB	+	+ - +	
		T-BRA	ICE
Nails	Section Detail		
	T-Brace		
	Web		•

I-Brace

	Brace Size for One-Ply Truss Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

	Brace Size for Two-Ply Truss				
	Specified Rows of La	Continuous iteral Bracing			
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

T-Brace / I-Brace must be same species and grade (or better) as web member.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

February 12, 2018

SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

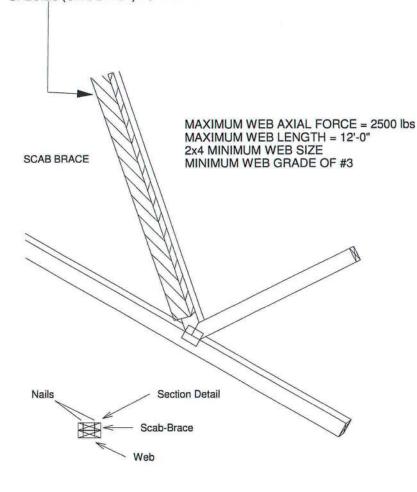


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

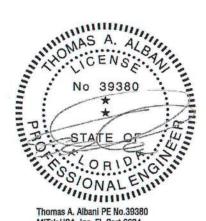
Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



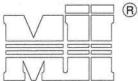
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

February 12, 2018

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

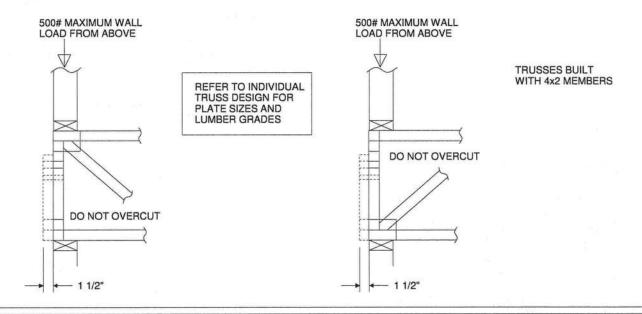
MiTek USA, Inc. Page 1 of 1

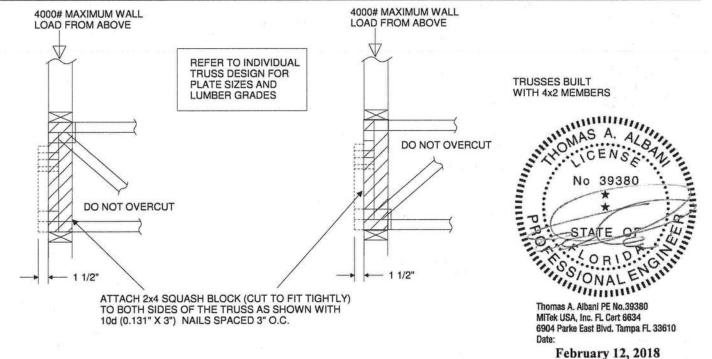


MiTek USA, Inc. ENGINEERED BY

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

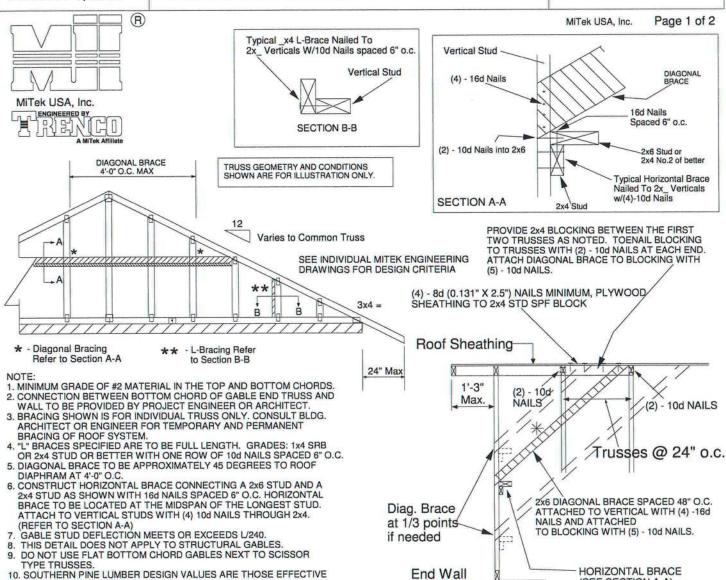
 3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.
 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

MII-GE130-D-SP



11. NAILS DESIG	NATED 10d)				
Minimum Stud Size	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade		Maximum Stud Length						

2x4 SP No. 3 / Stud 12" O.C. 7-1-3 11-5-7 3-9-13 4-1-1 5-9-6 2x4 SP No. 3 / Stud 16" O.C. 6-10-8 10-3-13 3-5-4 3-6-8 5-0-2 2x4 SP No. 3 / Stud 24" O.C. 2-9-11 2-10-11 4-1-1 5-7-6 8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE : 1.60

06-01-13 BY SPIB/ALSC.

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



(SEE SECTION A-A)

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

February 12, 2018

Standard Gable End Detail

MII-GE130-SP

Page 1 of 2

- 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

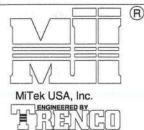
ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

MiTek USA, Inc.



DIAGONAL BRACE 4'-0" O.C. MAX

Typical x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 = B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

- 10d

NAILS

Roof Sheathing

1'-3"

Max.

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

 Diagonal Bracing Refer to Section A-A

L-Bracing Refer to Section B-B

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
- ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A) 7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSE 10. SOUTHERN PI 06-01-13 BY SI 11. NAILS DESIGN NAILS DESIGN	S. INE LUMBE PIB/ALSC. IATED 10d	R DESIGN	I VALUES A	RE THOSE				
Minimum Stud Size	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6		
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1		

4-5-6

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE : 1.60

2x4 SP No. 3 / Stud 24" O.C. 3-0-10 3-1-12

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

9-1-15



Thomas A. Albani PE No.39380 MITek USA, Inc. FL. Cert 6634 6904 Parke East Blvd. Tampa FL 33610

February 12, 2018

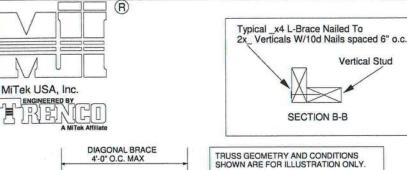


Standard Gable End Detail

MII-GE140-001

Page 1 of 2

MiTek USA, Inc.



Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 =L-Bracing Refer - Diagonal Bracing Refer to Section A-A to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK

Roof Sheathing

24" Max

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB

OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

1'-3" Max.	(2) - 10d NAILS
1	Trusses @ 24" o.c.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

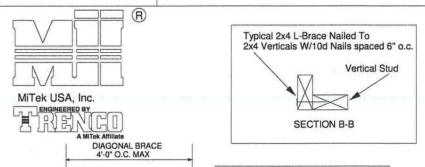
January 19, 2018

Standard Gable End Detail

MII-GE170-D-SP

Page 1 of 2

MiTek USA, Inc.



Vertical Stud 2X6 SP OR SPF No. 2 DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A 2X4 SP OR SPF No. 2

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING

24" Max

DRAWINGS FOR DESIGN CRITERIA

3x4 =

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Diagonal Bracing Refer to Section A-A

►A

- L-Bracing Refer to Section B-B

В

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

1'-0"	(2) - 10d
Max.	NAILS
Diag. Brace	2x6
at 1/3 points	48"
if needed	(4)-
End Wall	

Roof Sheathing

1'-0"

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species	Stud Spacing	Without Brace	DO.		2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4		
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3		
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13		
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7		
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5		
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET ASCE 7-10 170 MPH

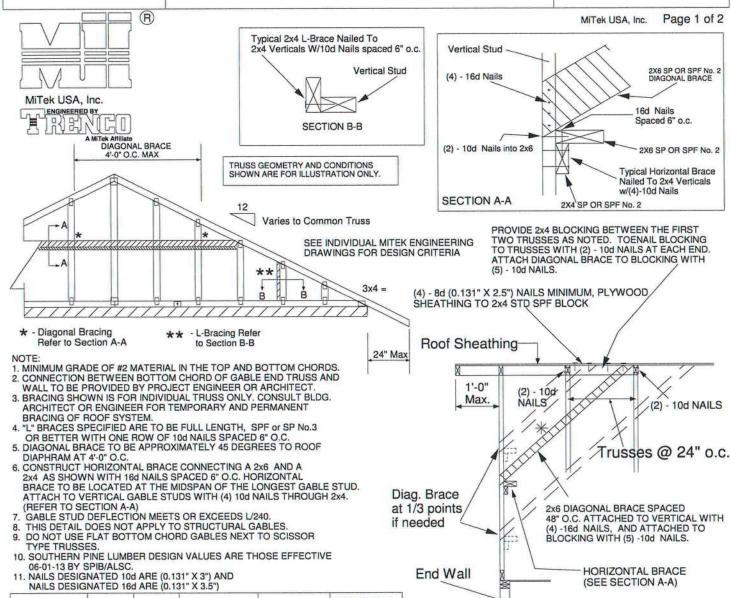
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. **DURATION OF LOAD INCREASE: 1.60** CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

MII-GE180-D-SP



Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-10

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C **ASCE 7-10 DURATION OF LOAD INCREASE: 1.60**

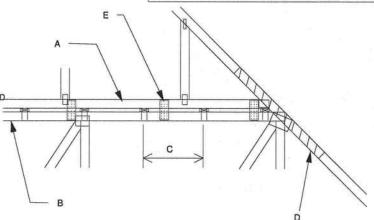
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

- PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131" X 3.5") TOE-NAILED.
- BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
- 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: DIRECTIONS AND:

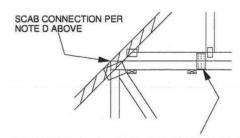
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft. FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH

MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72° O.C. W/ (4) (0.131° X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

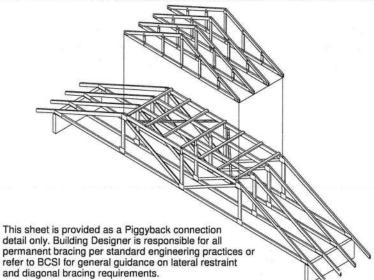


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

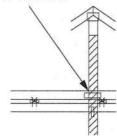
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

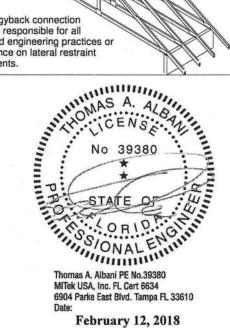
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DETAIL.

ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

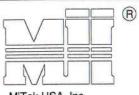
MII-PIGGY-ALT 7 - 10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

DURATION OF LOAD INCREASE: 1.60

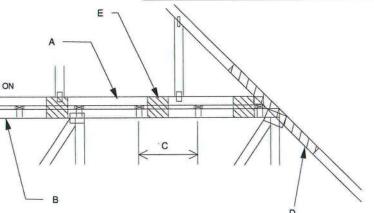
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



MiTek USA, Inc.

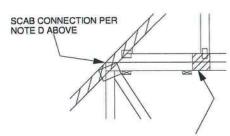
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) 0(0.131" X 3.5") TOE-NAILED.
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
 D 2 X _ X 4-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
 INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
 IS CONTINUIOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
 DIRECTIONS AND:

 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
 PIGGYBACK SPAN OF 12 ft.
- E FOR WIND SPEED IN THE RANGE 126 MPH 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS ATTACH WITH 3 6d (0.113" x 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL 12 NAILS)

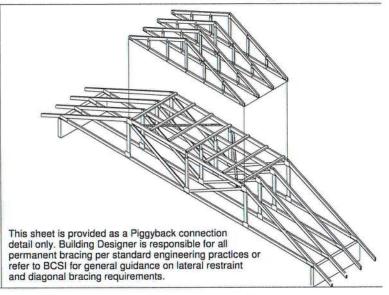


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

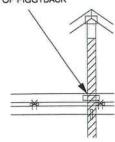
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
- ATTACH 2 x x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
 (MINIMUM 2X4)
 THIS CONSIST
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
- GREATER THAN 4000 LBS.
 FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
 NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



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STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

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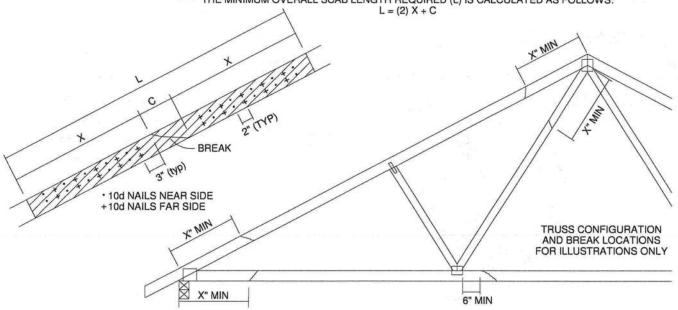


TOTAL NUMBER OF			MAXIMUM FORCE (Ibs) 15% LOAD DURATION							
OF BREAK *		X	SP		DF		SPF		HF	
2x4	2x6	er fer for	2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES
 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS
 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID

- LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

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R

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NOTES:

- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

> VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

> > SIDE VIEW (2x3) 2 NAILS

> > > NEAR SIDE NEAR SIDE

	DIAM.	SP	DF	HF	SPF	SPF-S
O	.131	88.0	80.6	69.9	68.4	59.7
LONG	.135	93.5	85.6	74.2	72.6	63.4
3.5" L	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

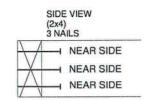
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

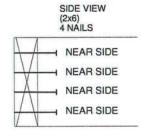
EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

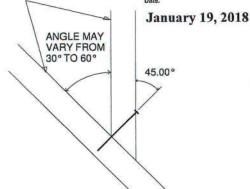
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

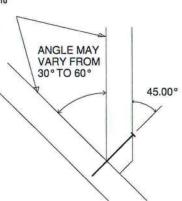


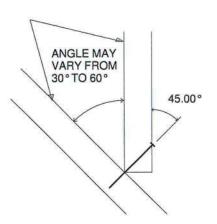




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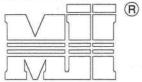






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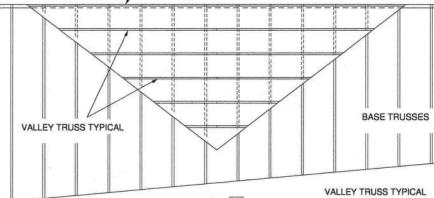
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ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS.

> DETAIL A (NO SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

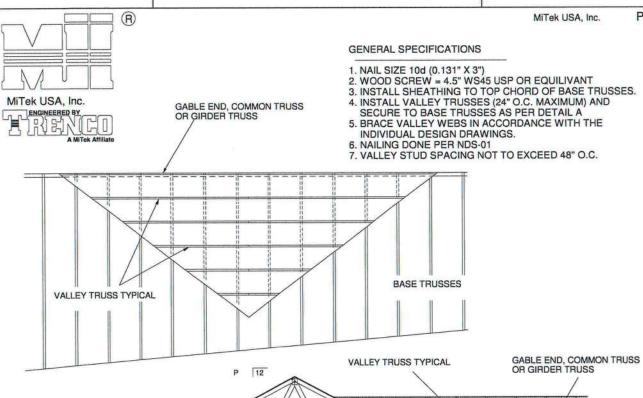


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TRUSSED VALLEY SET DETAIL

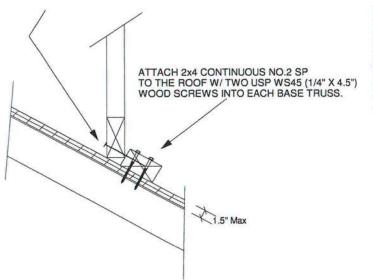
MII-VALLEY HIGH WIND2

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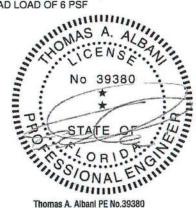
SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

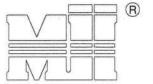
ON THE TRUSSES



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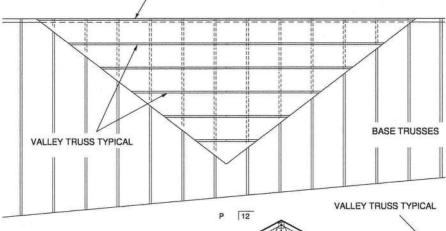
MiTek USA, Inc.

ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

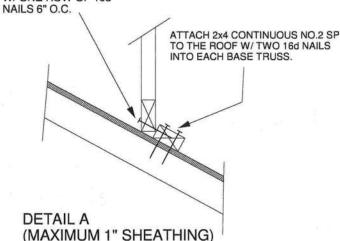
- NAIL SIZE 16d (0.131" X 3.5")
 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d

N.T.S.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING
EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 60 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 4.2 PSF
ON THE TRUSSES ON THE TRUSSES

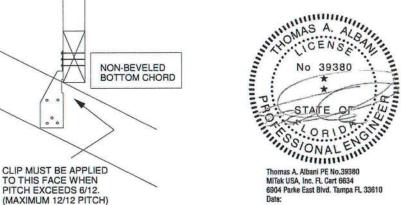


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TRUSSED VALLEY SET DETAIL MII-VALLEY **AUGUST 1, 2016** (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT Page 1 of 1 R MiTek USA, Inc. TO EXCEED 48" O.C. SPACING MiTek USA, Inc. THE RELEASE BY FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED **EXPOSURE B or C BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.



Standard Gable End Detail

MII-GE146-001

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(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

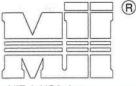
ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

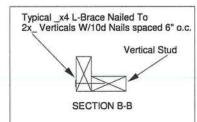
TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED



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3x4 =

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Roof Sheathing

1'-3"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

- 10d

NAILS

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA - Diagonal Bracing L-Bracing Refer

DIAGONAL BRACE

4'-0" O.C. MAX

NOTE

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:

2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

to Section B-B

DIAPHRAM AT 4'-0" O.C.

CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maxin			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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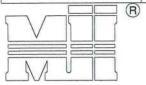
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

A MITCH Affiliate

TRUSS CRITERIA:

LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

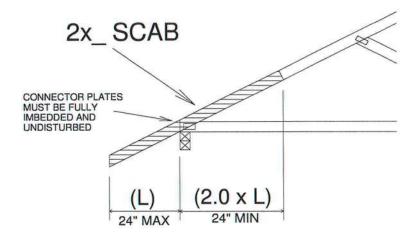
NOTES:

1. ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF

TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

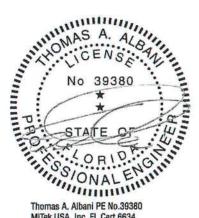


IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Gert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

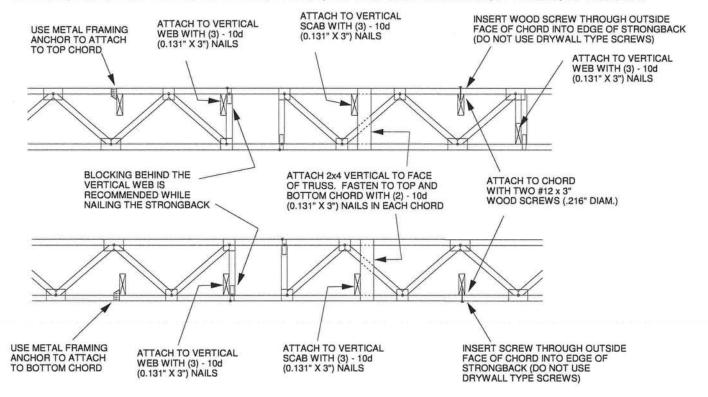
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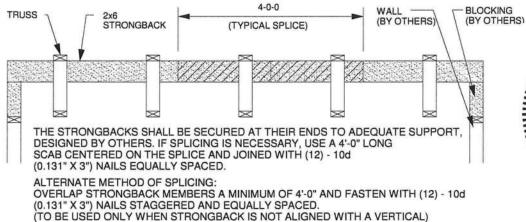


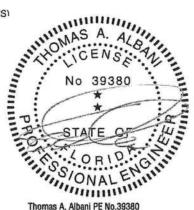
TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.







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