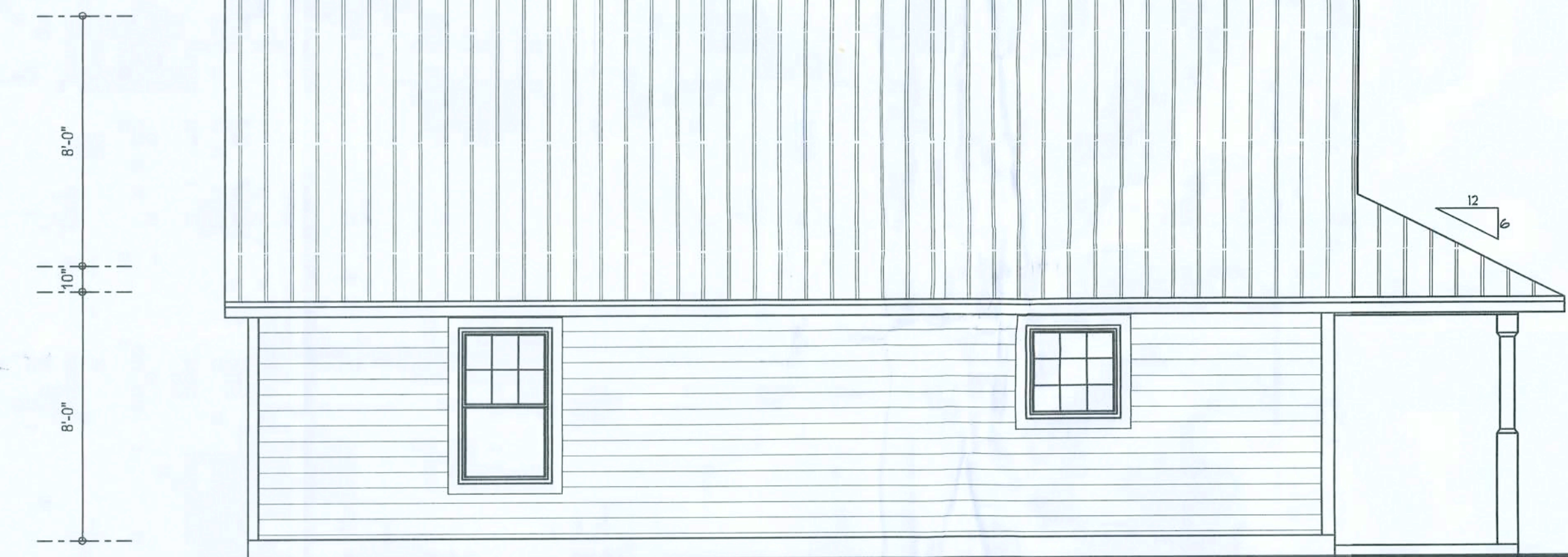




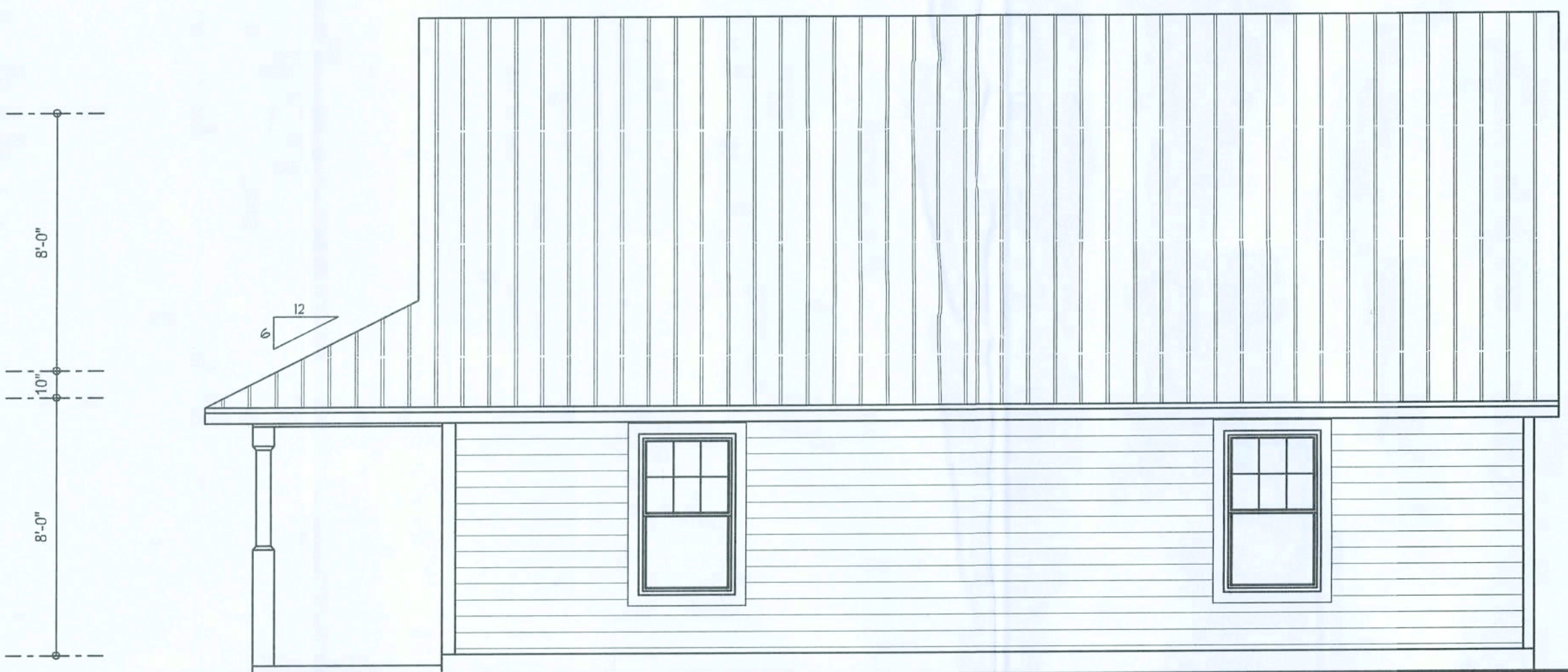
REAR ELEVATION
SCALE: 1/4" = 1'-0"



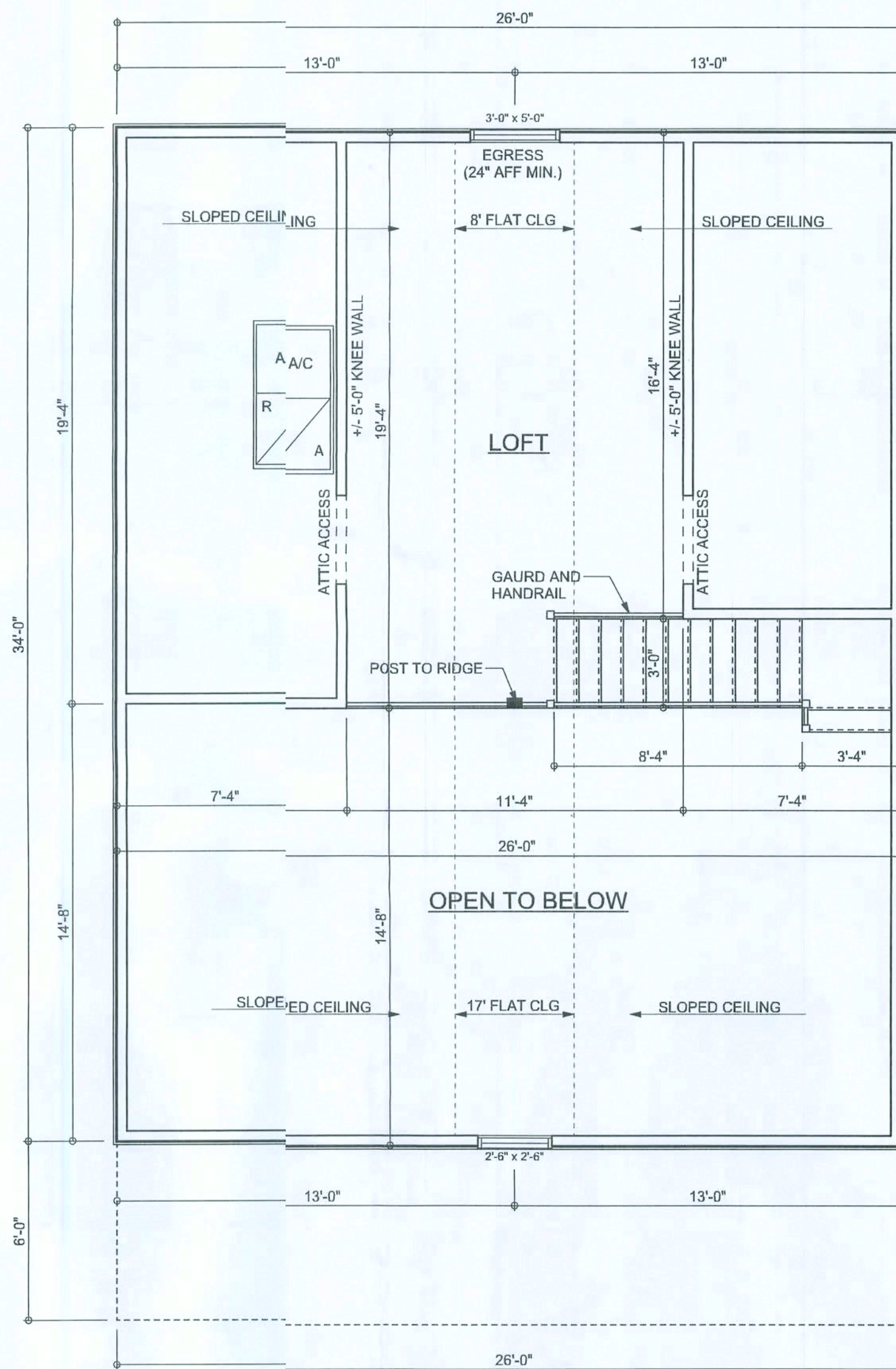
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



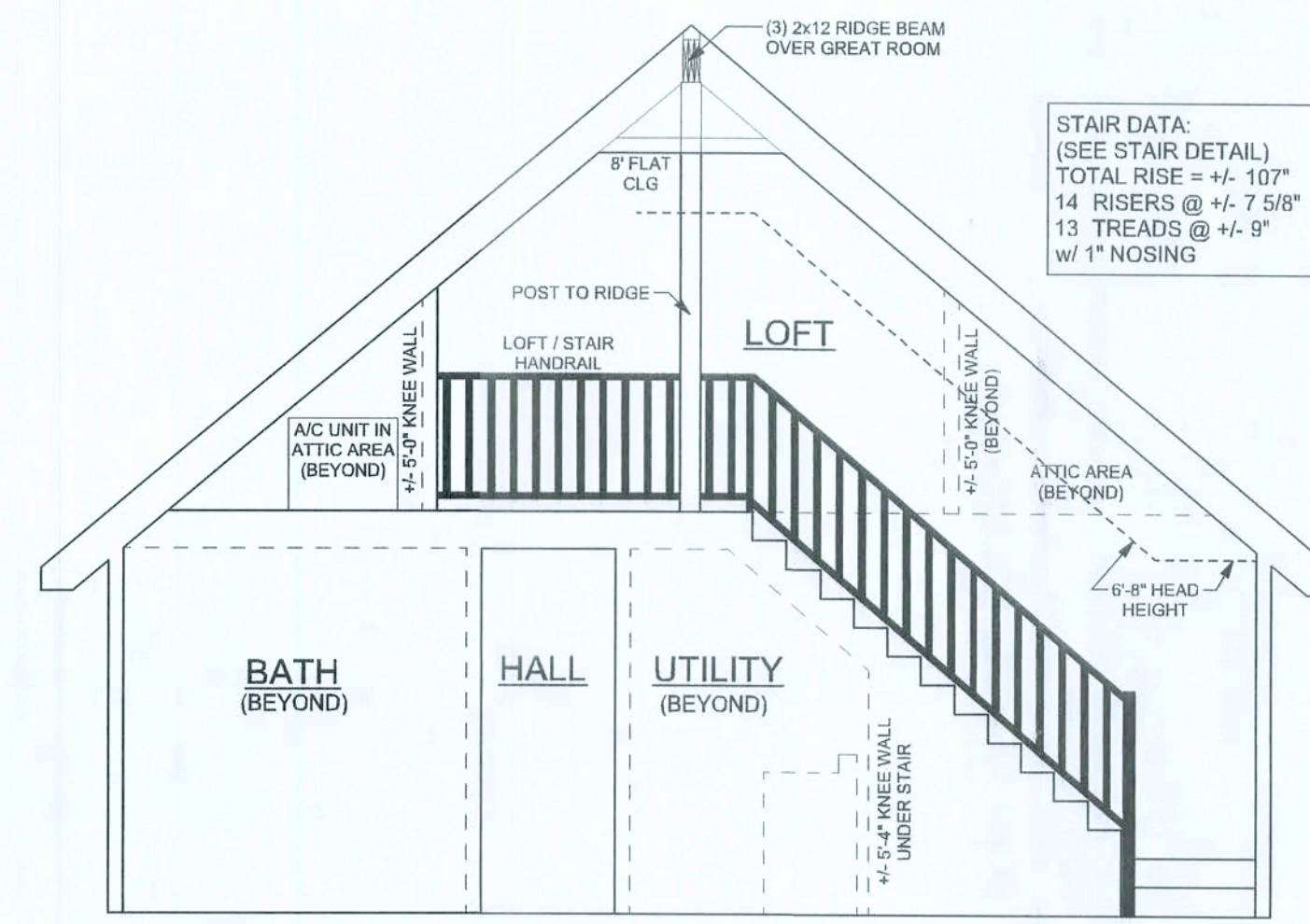
RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



2ND FLOOR LAYOUT
SCALE: 1/4" = 1'-0"

AREA SUMMARY

1st FLOOR LIVING AREA	884	S. F.
LOFT AREA	220	S. F.
PORCH AREA	156	S. F.
TOTAL AREA	1260	S. F.



SECTION @ STAIRWAY
SCALE: 1/4" = 1'-0"

STAIR DATA:
(SEE STAIR DETAIL)
TOTAL RISE = +/- 10'7"
14 RISERS @ +/- 7 5/8"
13 TREADS @ +/- 8"
w/ 1" NOSING

REQUIRED ROOF VENTILATION:

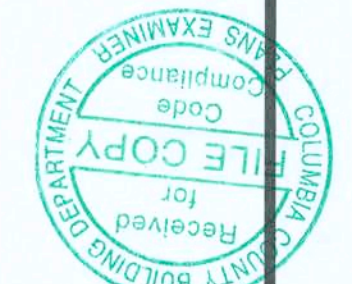
AS PER FLORIDA BUILDING CODE 2309.7

RISE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
884 S.F. / 300 x 50% = 1.5 S.F. RIDGE VENT AREA REQUIRED
17 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
884 S.F. / 300 x 50% = 1.5 S.F. SOFFIT VENT AREA REQUIRED
50 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 18 IN²/FT (1.1 FT²/FT)
2. OFF-RIDGE VENTS = 70 FT² PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN²/FT (0.3 FT²/FT)



REVISIONS



WINDLOAD ENGINEER:
Mark Disway, P.E.
No. 53915, POB 866, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions surround scaled
dimensions. Refer all questions to
Mark Disway, P.E. or resolution.
Do not proceed without clarification.

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the express written permission and consent
of Mark Disway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to
wind engineering comply with section
R301.2.1, Florida building code
residential 2007,
to the best of my knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DISWAY
P.E. 53915
Mark Disway
14 Aug 09
SAL

Crawford Stanley
Construction
Jordan Residence

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
August 4, 2009

DRAWN BY: STRUCTURAL BY:
Evan Beamley Evan Beamley

FINALS DATE:
14 Aug 09

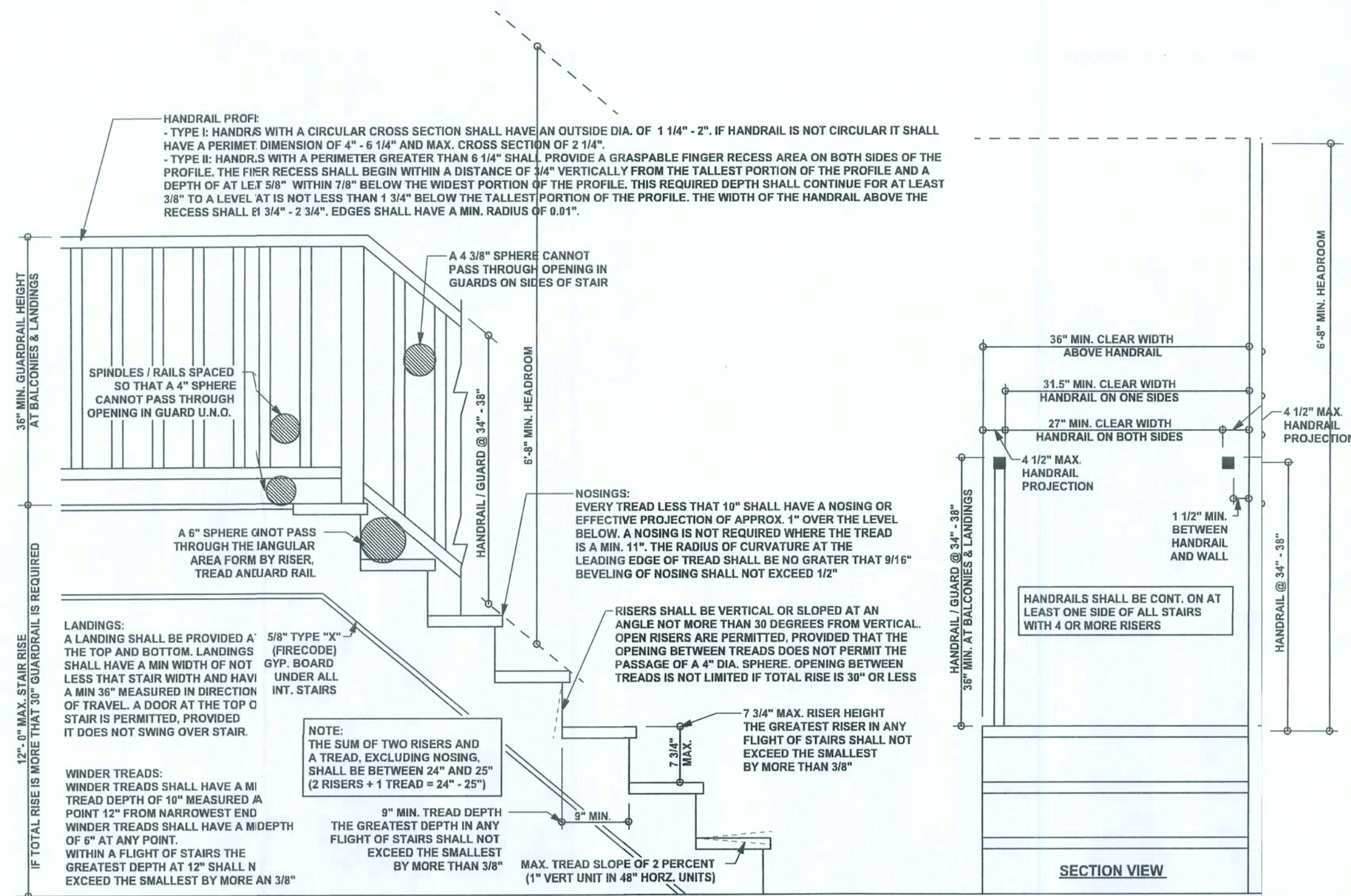
JOB NUMBER:
90154

DRAWING NUMBER
#1
OF 4 SHEETS



STAIR DESIGN LOAD REQUIREMENTS:

- GUARDRAILS AND HANDRAILS:**
- 250 LB SINGLE CONCENTRATED LIVE LOAD APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP
- GUARDRAILS IN-FILL COMPONENTS:**
- 50 LB LIVE LOAD APPLIED HORIZONTALLY ON AN AREA EQUAL TO 1 FT² (THIS LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH ANY OTHER LIVE LOAD REQUIREMENT)
- GLAZING USED IN HANDRAIL ASSEMBLIES AND GUARDS SHALL BE DESIGNED WITH A SAFETY FACTOR OF 4. THE SAFETY FACTOR SHALL BE APPLIED TO EACH OF THE CONCENTRATED LOADS APPLIED TO THE TOP OF THE RAIL, AND TO THE LOAD ON THE IN-FILL COMPONENTS. THESE LOADS SHALL BE DETERMINED INDEPENDENT OF ONE ANOTHER, AND LOADS ARE ASSUMED NOT TO OCCUR WITH ANY OTHER LIVE LOAD.**
- STAIRS:**
- 40 PSF LIVE LOAD, OR 300 LB CONCENTRATED LOAD OVER AN AREA OF 4 IN² (WHICHEVER PRODUCES THE GREATER STRESSES)

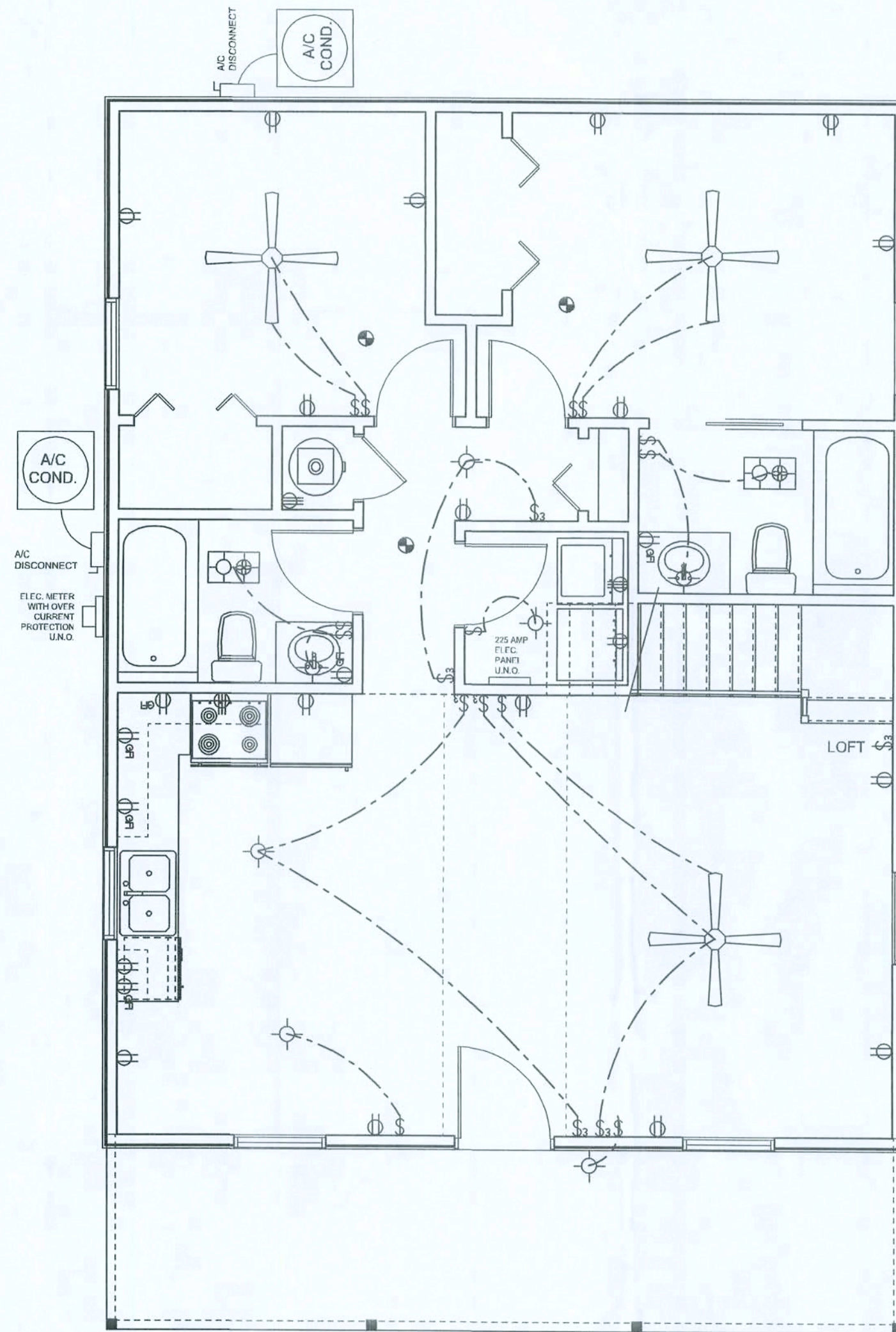
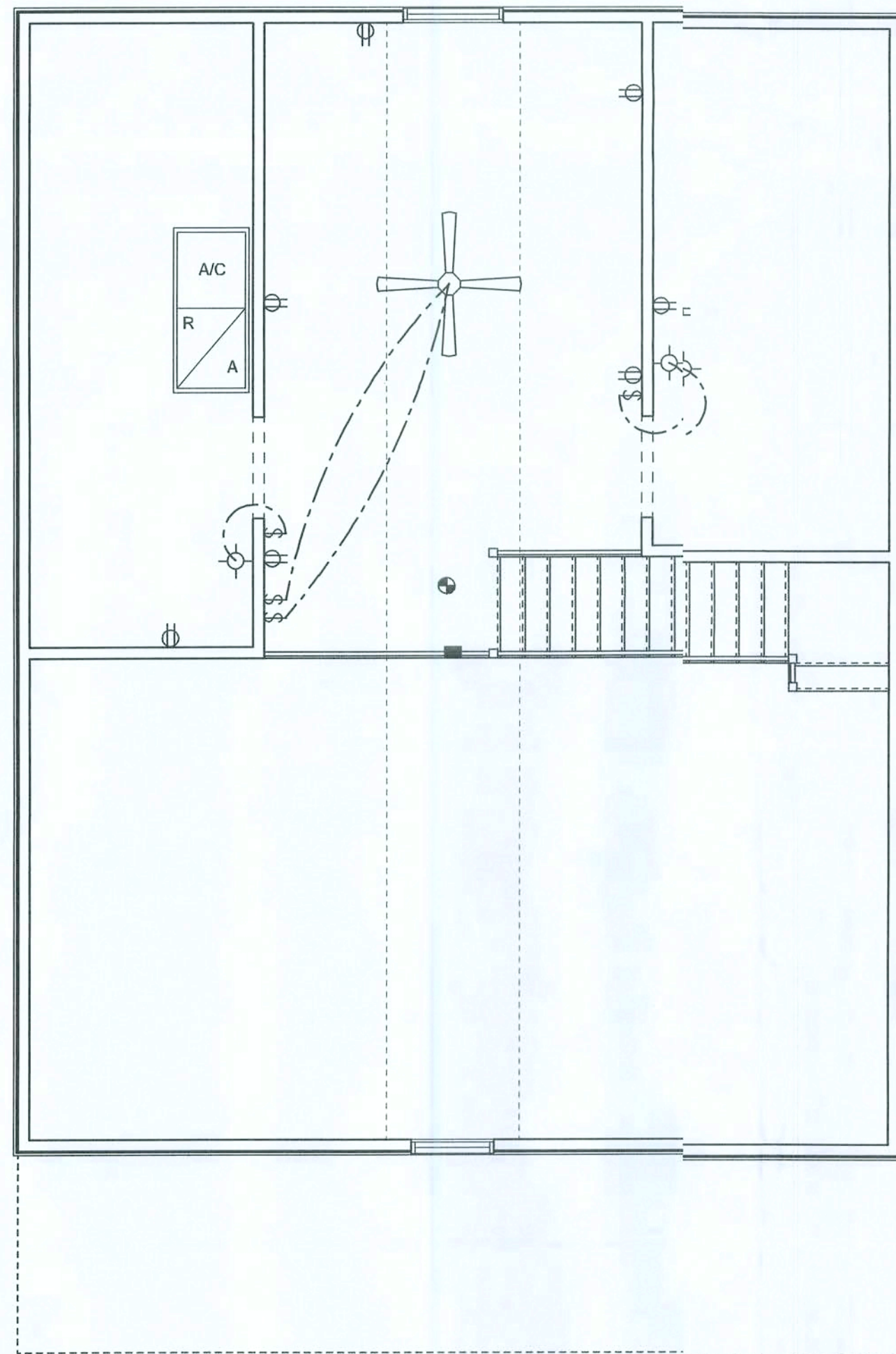


TYPICAL STAIR AND GUARDRAIL REQUIREMENTS
SCALE: 3/4" = 1'-0"

ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR'L SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
- E -11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED IN LIGHT
	BATH EXHAUSTFAN WITH LIGHT
	BATH EXHAUSTFAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX JTLET
	SMOKE DETECTOR
	WALL SWTCH
	3 WAY WALLSWITCH
	4 WAY WALLSWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION CK
	GARAGE DOOR OPENER
	CARBON MOXIDE ALARM



REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Discosway, P.E.
No. 53915, PCB 868, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and list the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOSWAY
P.E. 53915

Mark Discosway
1-AUG-09
SEAL

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Construction

Jorden Residence

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August 14, 2009

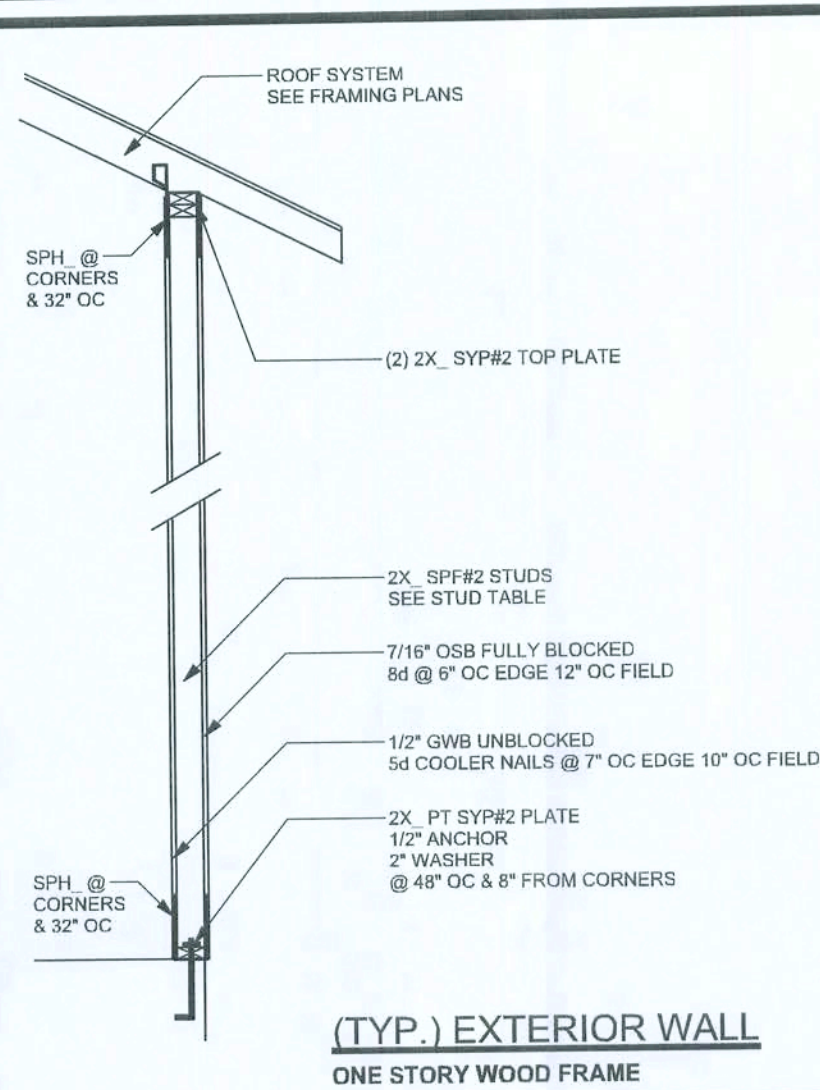
DRAWN BY: Evan Beamley
STRUCTURAL BY: Evan Beamley

FINALS DATE:
14 Aug 09

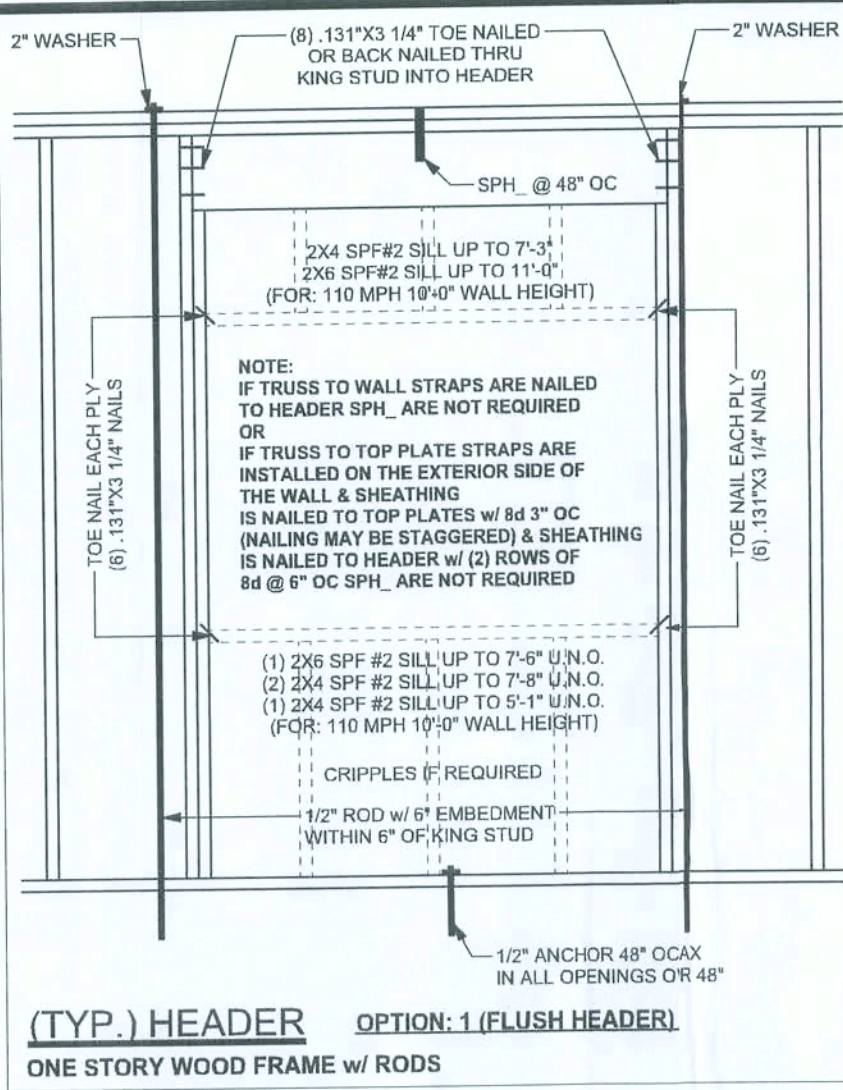
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DRAWING NUMBER

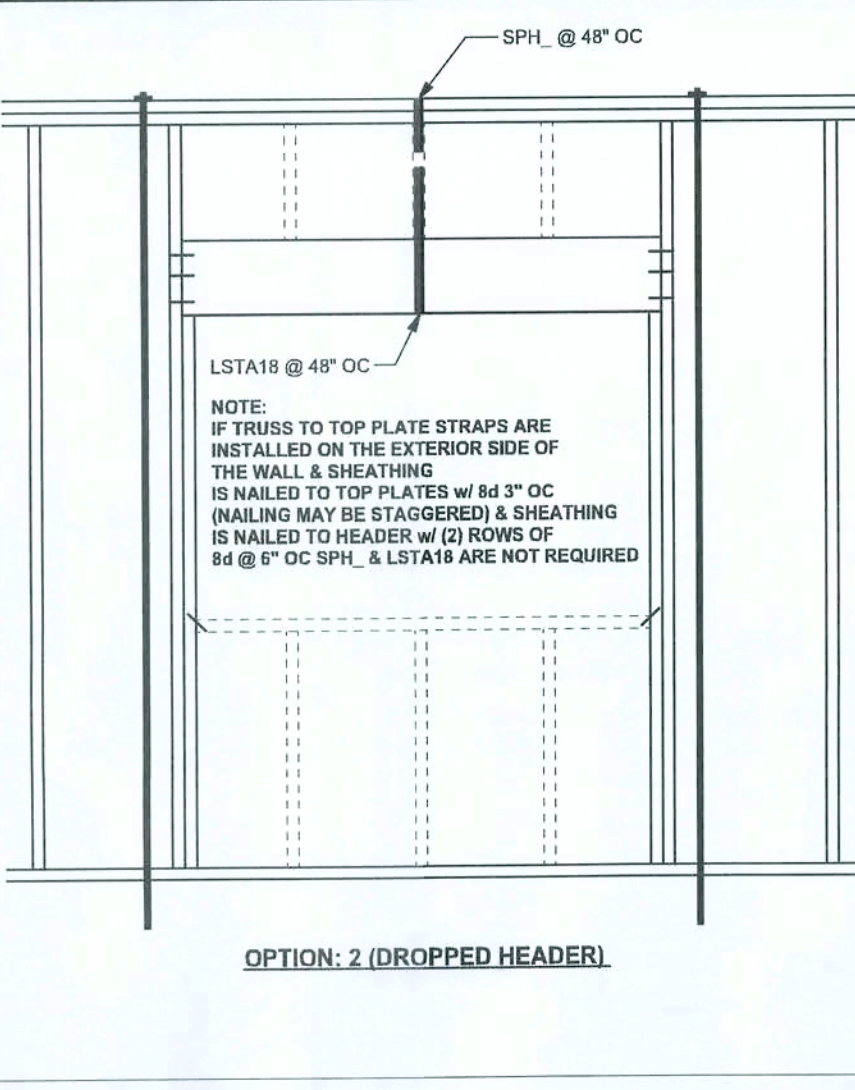
#2
OF 4 SHEETS



(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME



(TYP.) HEADER OPTION: 1 (FLUSH HEADER)
ONE STORY WOOD FRAME w/ RODS



OPTION: 2 (DROPPED HEADER)

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	285	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	980	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	855	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LT512 - LT520	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MT512 - MT530	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HT516 - HT530	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								
LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LG14-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MGT	3965	3330					22 -10d	5/8" ANCHOR
HGT-2	10980	6485					16 -10d	2-5/8" ANCHOR
HGT-3	10530	9035					16 -10d	2-5/8" ANCHOR
HGT-4	9250	9250					16 -10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
SP14	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
SP6	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
SP16	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
LSTA18	1235	1110					14-10d	14-10d
LSTA21	1235	1235					16-10d	16-10d
CS20	1030	1030					14-10d	14-10d
CS16	1705	1705					22-10d	22-10d
STUD ANCHORS								
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT31	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HDS4	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR
TO FOUNDATION								
TO STUDS								
TO RAFTERS								
TO PLATES								

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-8" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING A NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS: 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL'S TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W14 x W14, FB = 89KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONCERNING TO ASTM A185 LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C1116. SUPPLIER TO PROVIDE ASTM C1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPLICES 40" DB (20" FOR 6 BARS) UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING. OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131) 4"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 2" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTOR, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON EACH TRUSS SHEET.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HYPERBOLIC, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE; AND UNOBSTRUCTED UPWIND FOR 50' HEIGHT OR 1 MILE WHICHEVER IS LESS.)

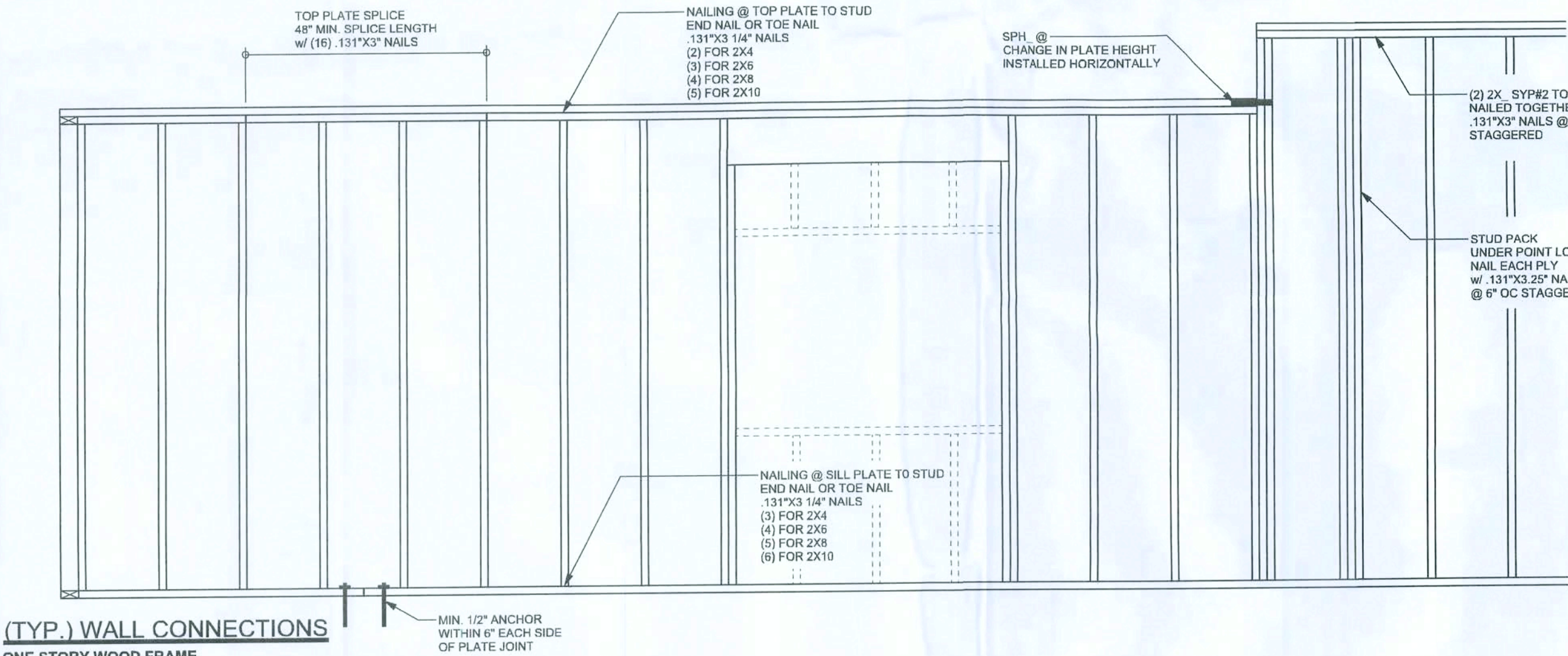
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

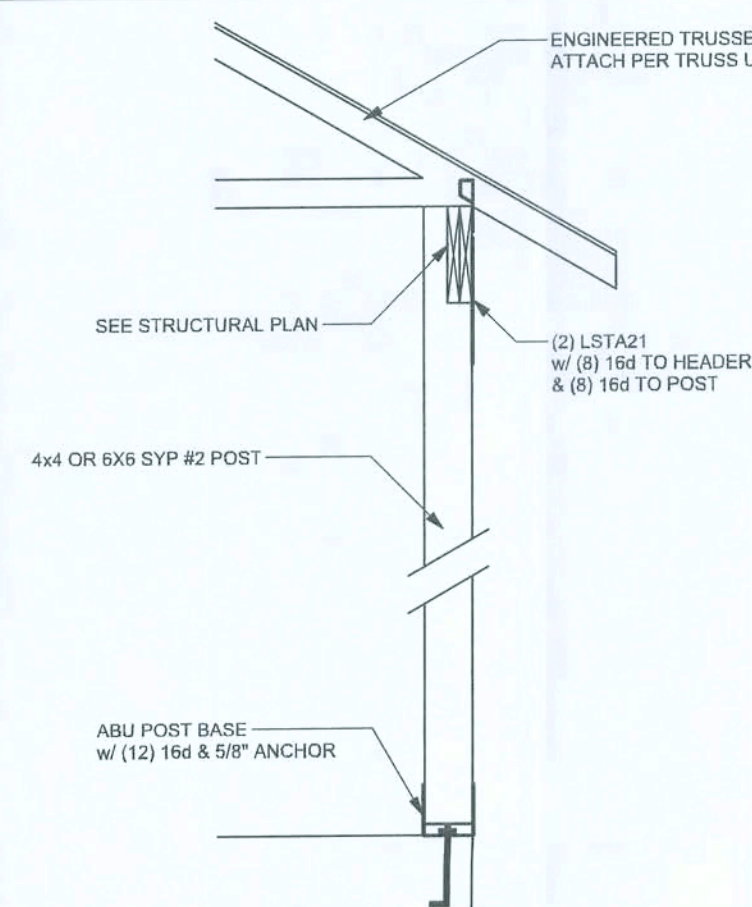
- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = C
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = -30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	(R2)
	10	100
1	27.8 - 30.5	25.3 - 25.3
2	27.8 - 35.7	25.3 - 30.5
2 On g	25.8	25.8
3	27.8 - 35.7	25.3 - 30.5
3 On g	25.8	25.8
4	30.5 - 33.0	25.9 - 28.5
5	30.5 - 40.7	25.9 - 31.6
Doors & Windows	30.5	40.7
Wind Case (Zone 5, 10 R2)		
8x7 Garage Door	27.3	32.0
16x7 Garage Door	25.9	29.4

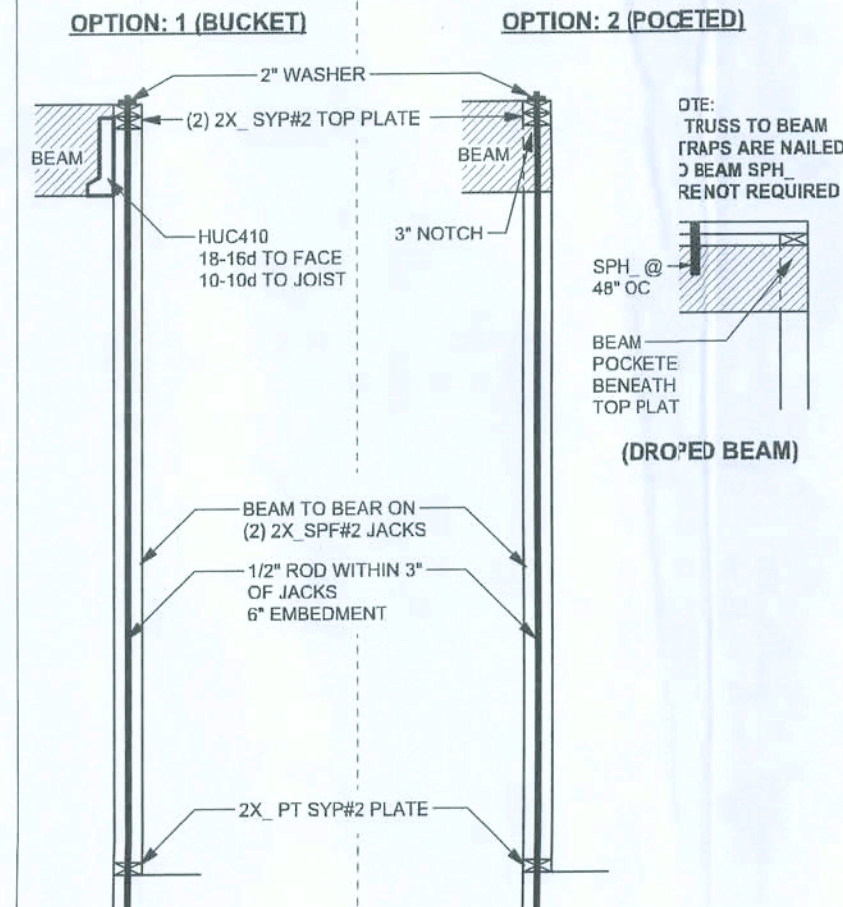
DESIGN LOADS	
FLOOR:	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3-12)
ROOF:	20 PSF (FLAT OR <4-12)
	16 PSF (4-12 TO <12-12)
	12 PSF (12-12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

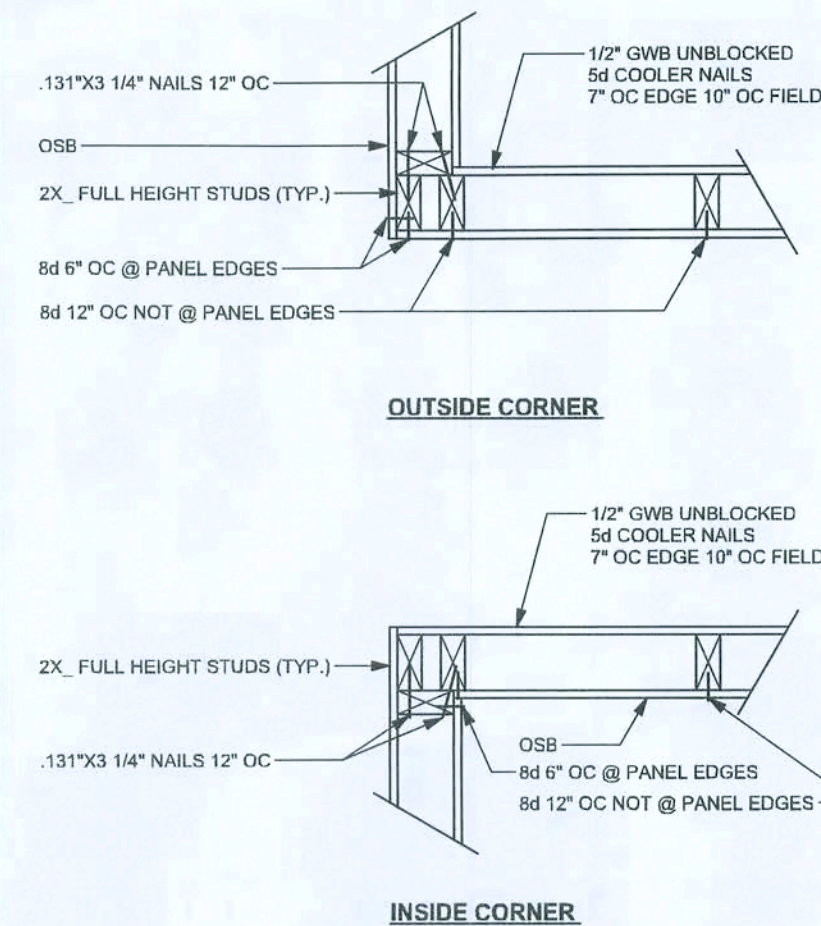


(TYP.) PORCH POST
ONE STORY WOOD

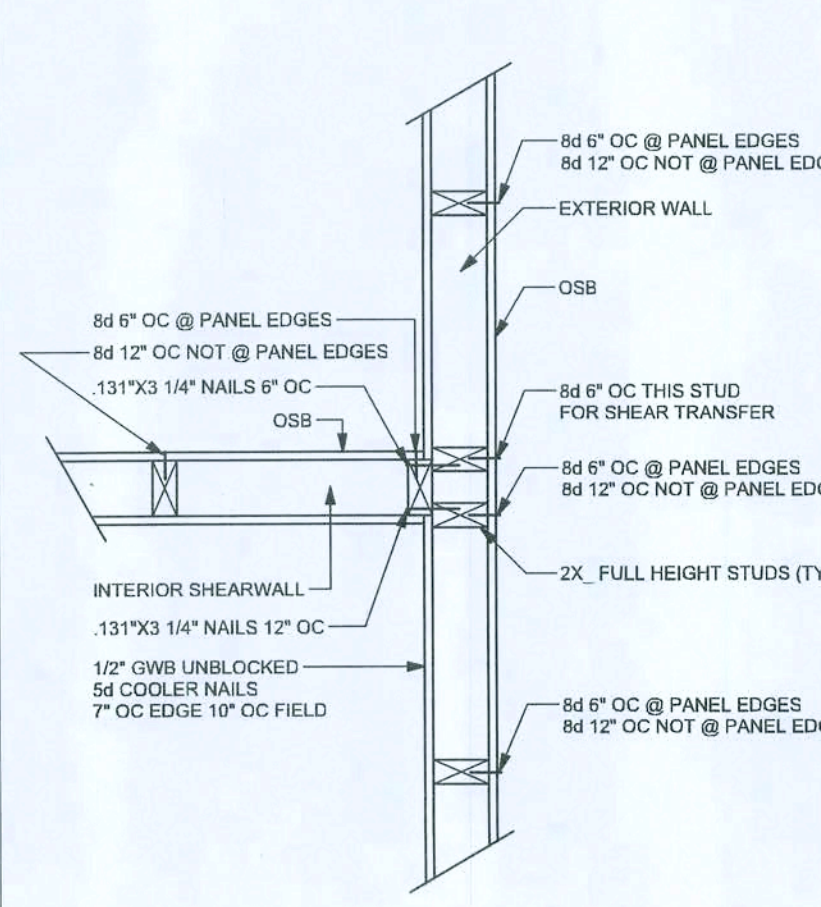


(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS

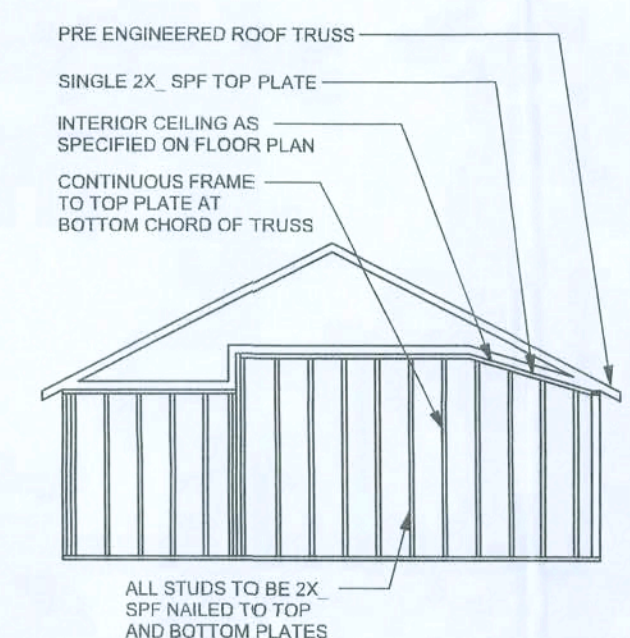
ALLOWABLE UPLIFT:
1779 LB



(TYP.) CORNER FRAMING
WOOD FRAME



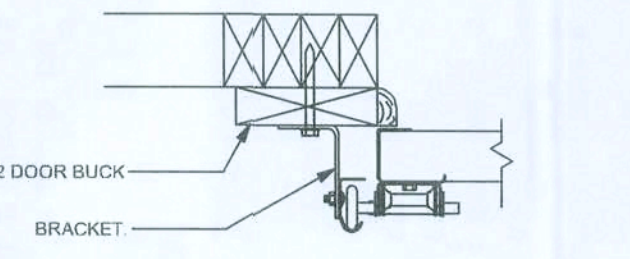
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



**CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL**
WOOD FRAME

2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\"/>

DOOR WIDTH	3/8\"/>
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(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

REVISIONS	

SOFTPLAN
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER:
Mark Disway, P.E.
No. 53915, PCB 666, Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions suprcede scaled dimensions. Refer all questions to Mark Disway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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P.E. 53915

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Fax: (386) 269 - 4871

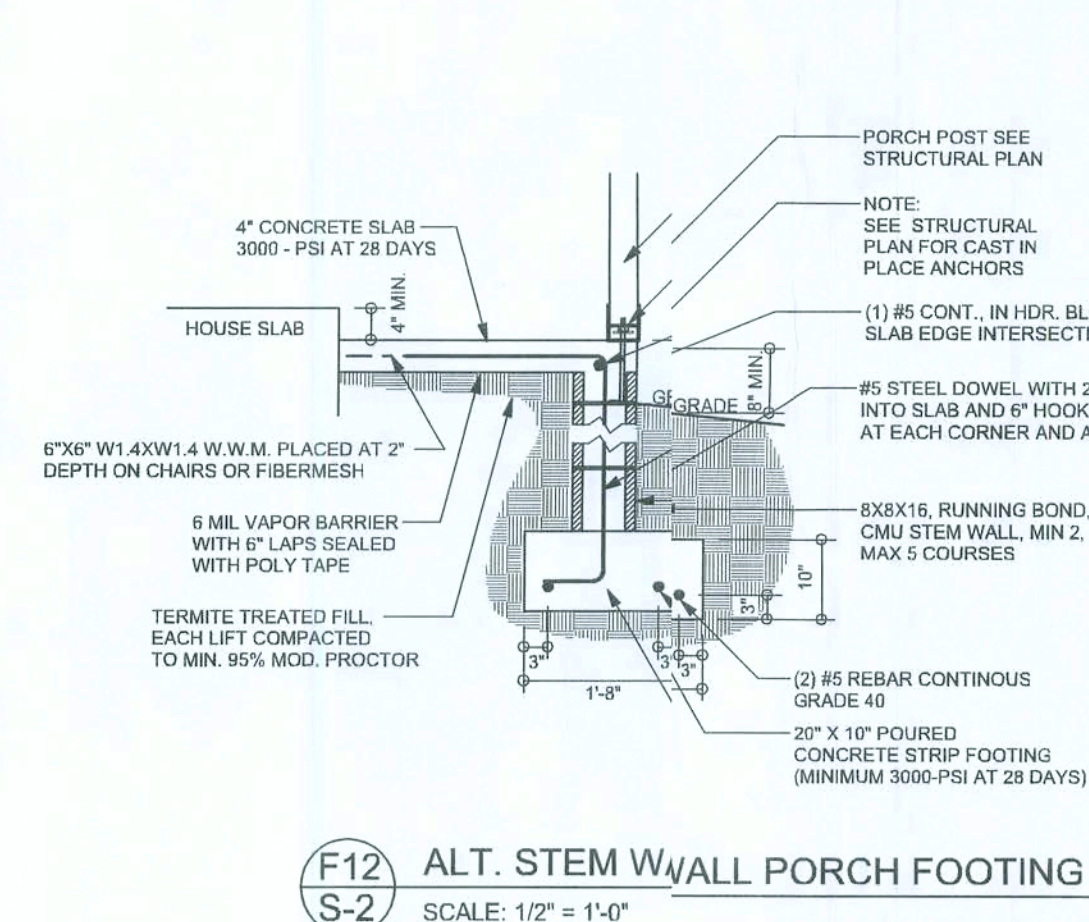
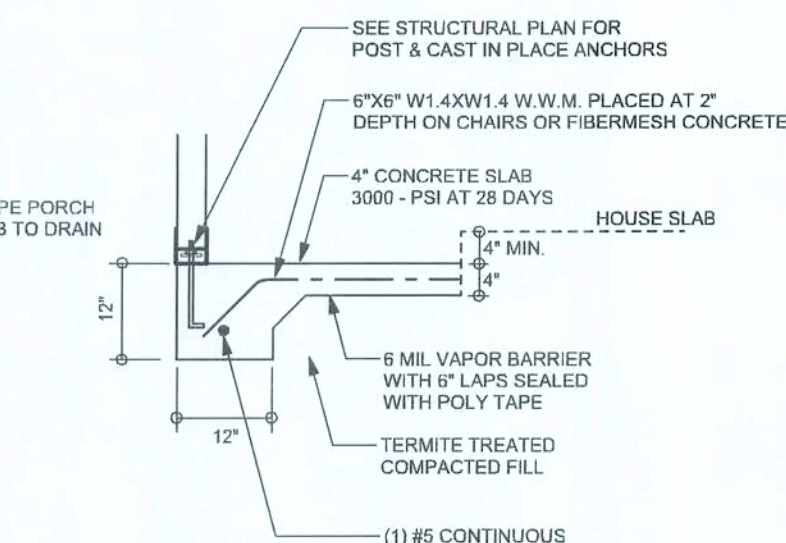
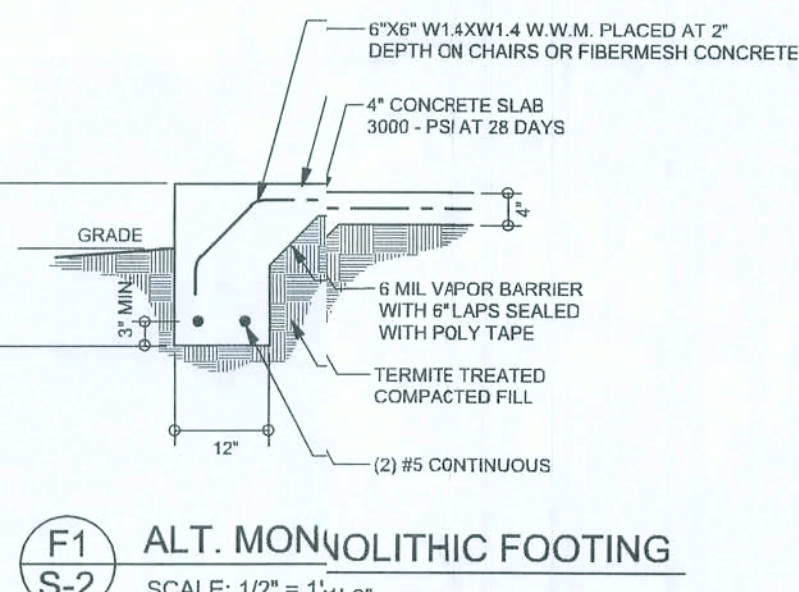
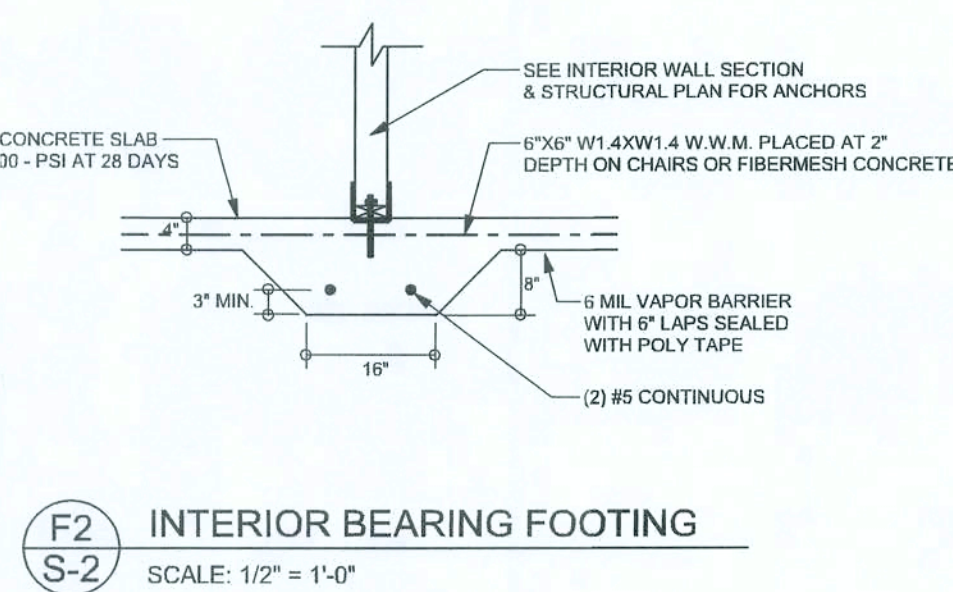
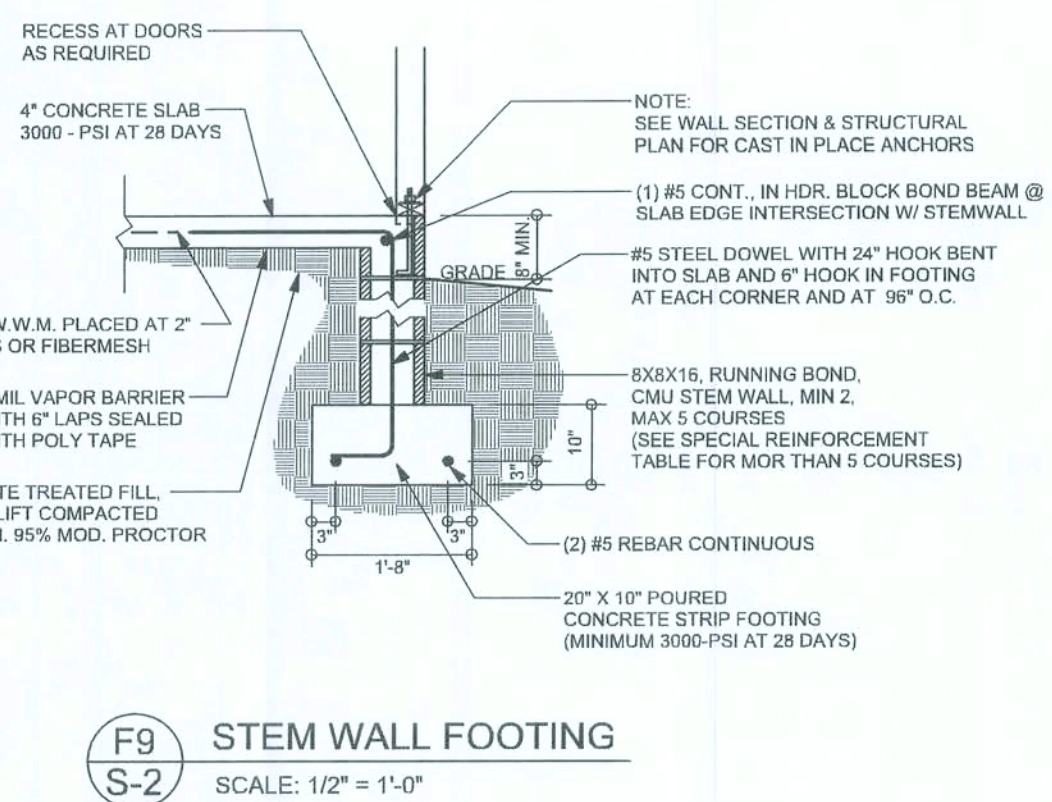
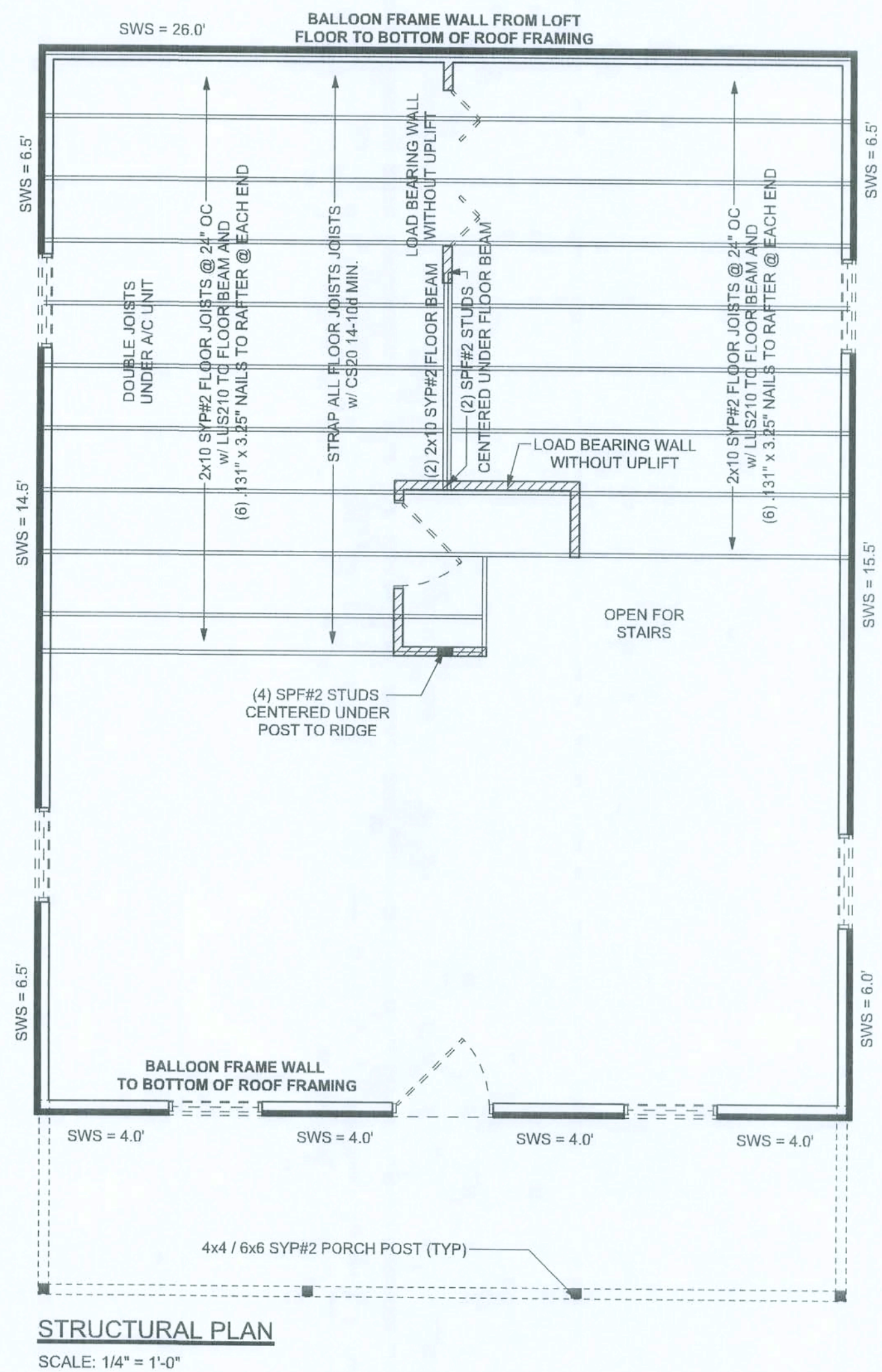
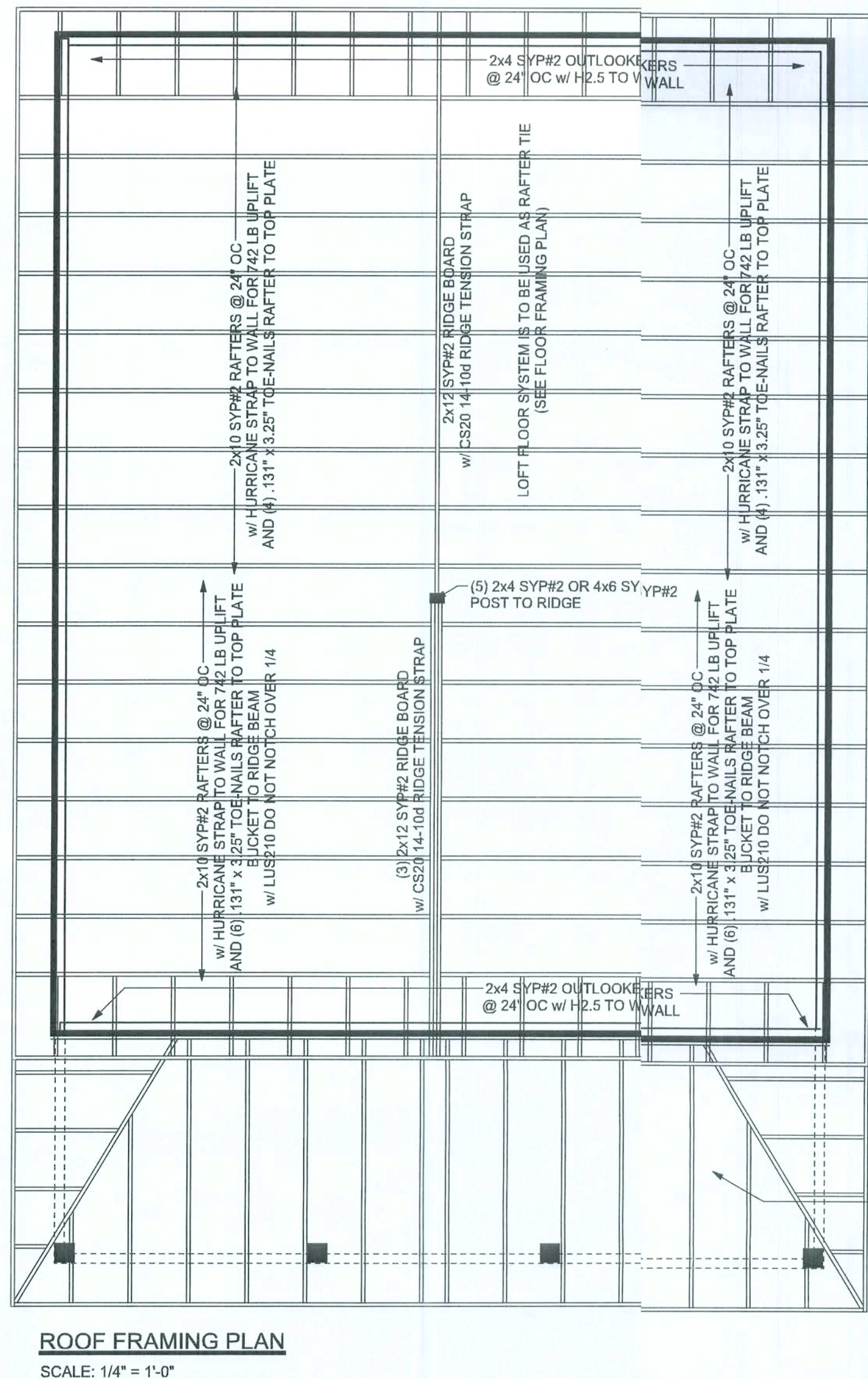
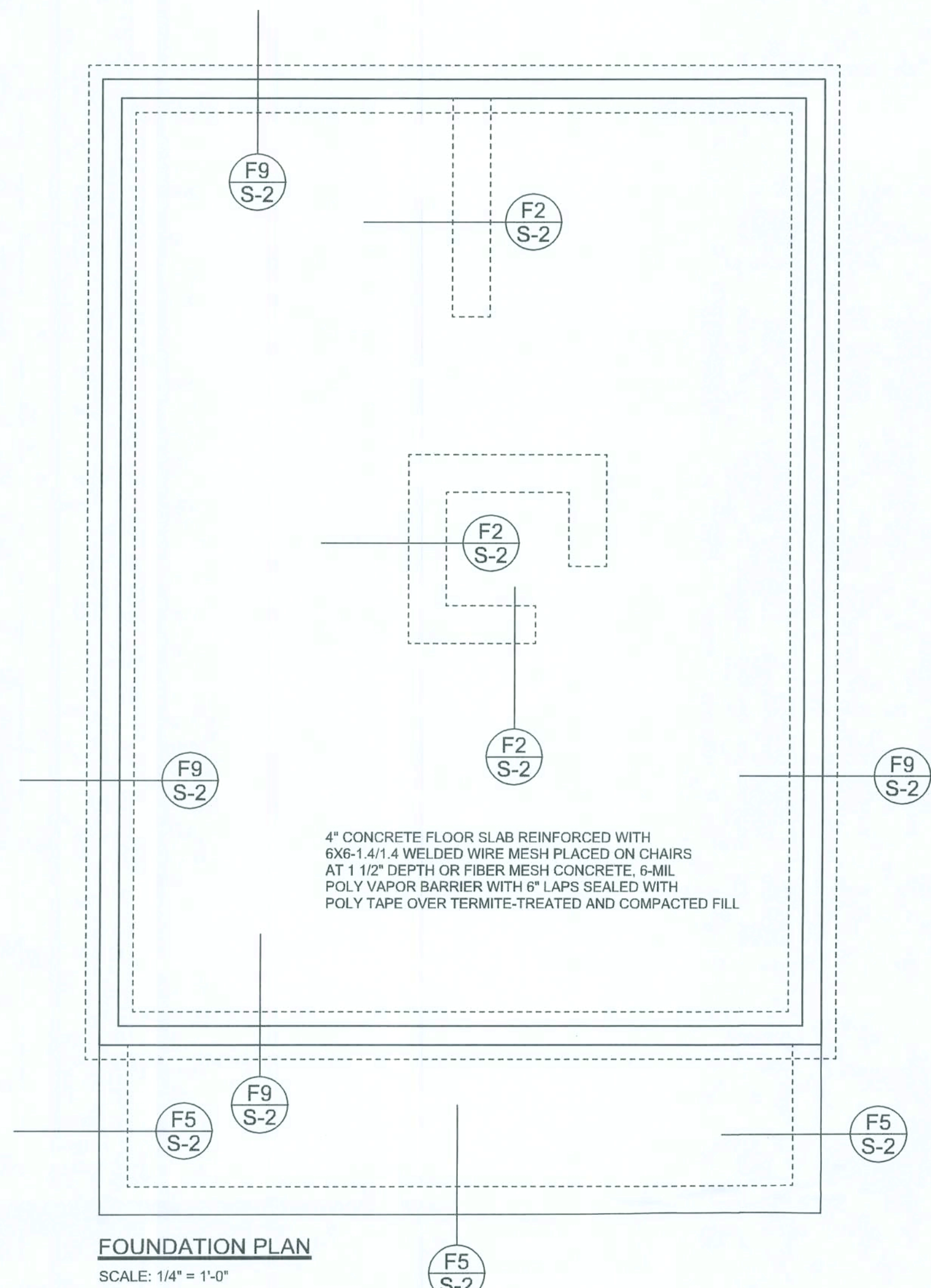
PRINTED DATE:
August 4, 2009

DRAWN BY: STRUTURAL BY:
Evan Beamsley Evan Beamsley

FINALS DATE:
14 Aug 09

JOB NUMBER:
906154
DRAWING NUMBER

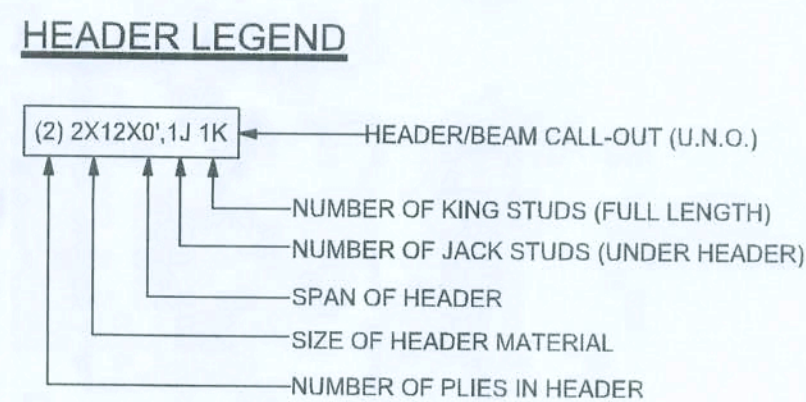
S-1
OF 43 SHEETS



TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 8" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall away from the soil pressure, within 2" of the exterior side of the wall. If the wall is over 8' high, add Diagonal ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



TOTAL SHEAR WALL TABLE

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	TRANSVERSE	LONGITUDINAL
ACTUAL	42.0'	55.5'
REQUIRED	35.3'	18.0'

- STRUCTURAL PLAN NOTES**
- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTPLAN

ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:

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No. 53915, POB 868, Lak City, FL 32056.

386-754-5419

DIMENSIONS:

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without certification.

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CERTIFICATION:

I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F301.2.1, Florida Building Code residential 2007, to the best of my knowledge.

LIMITATION:

This design is valid for one building, at specified location.

MARK DISOSWAY

P.E. 53915

SEAL

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PRINTED DATE:

August 14, 2009

DRAWN BY:

Evan Beasley

STRUCTURAL BY:

Evan Beasley

FINALS DATE:

14 Aug 09

JOB NUMBER:

906134

DRAWING NUMBER

S-2

OF 4 SHEETS

