

REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

Willy July

William & Close

Mike Roberts
Owner

Chuck Wood P.O. Box 3535 Lake City, Florida 32025 Phone: (386) 755 - 8699 Fex: (386) 755 - 8699

PRINTED DATE:

DRAVN BY: CHECKED BY: Chucl Wood

DESIGNED BY:

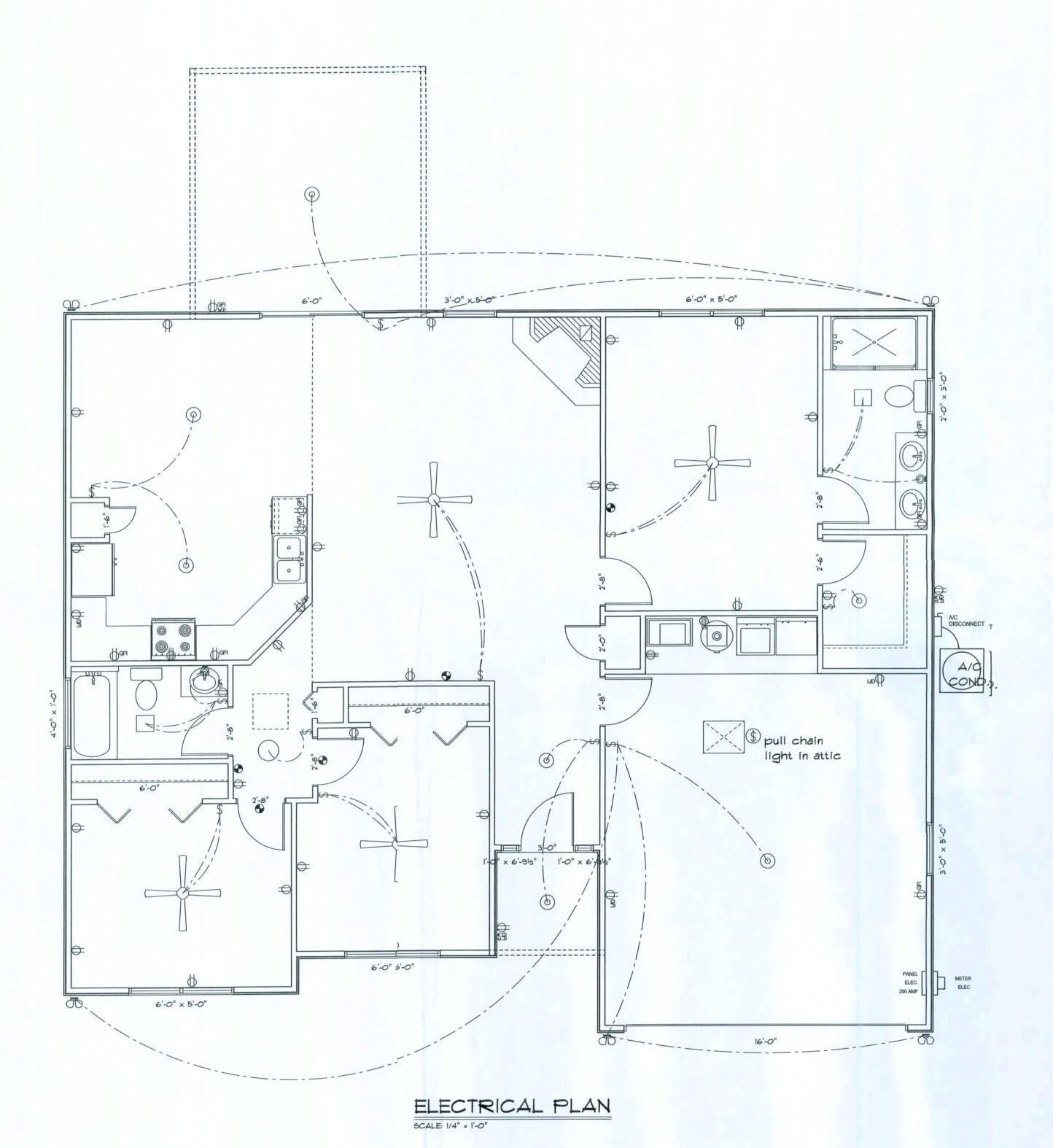
Chuck Wood

FINALIS DATE:

FINALIS DATE: 06 / 0CT / 04

JOB NUMBER:

ERAWING NUMBER





SCFTPIXN ARCHITCTURAL DESIGN SOFTWARE

ELECTRICAL	COUNT	SYMBOL
ceiling fan	4	
ceiling lamp globe	1	
ceiling light vent square	2	
ceiling pull switch	1	\$
ceiling globe light	Т	0
double spotlight	5	QP
wall mount 1	2	Q
switch	12	\$

#### Electrical Plan Notes:

- E-1 Wire all appliances, HYAC units and other equiptment per manufactures specifications.
- E-2 Consult the owner for the number or seperate telephone lines to be installed. Owner is responsible for all overages not noted on plan.
- E-3 All installations shall be per national code.
- E-4 All smoke detectors shall be 120v with battery back-up of the photoelectric type, and shall be interlocked together. Install inside and near all bedrooms.
- E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners directions and in accordance with applicable sections of the National Electric Codes latest edition. Owner is responsible for all overages not noted on plan.
- E-6 Electrical contractor shall be responssible for the design and sizing of electrical service and circuits.
- E-7 Entry of service (underground or overhead) to to be determined by contractor agreement.
- E-8 All bedroom receptacles shall be AFCI (arc fault circuit interrupter).
- E-9 All outlets to be located above base flood elevation.
- E-10 All exterior GFI outlets shall be weatherproof.
- E-II Overcurrent Protection device shall be installed on the exterior ofstructures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equiptment ground.

What

Willia 6 Ward

Mite Roberts
Owner

Cruck Wood P.0. Box 3535 Lake Ciy, Florida 32025 Phone: (386) 755 - 8699 Fax: (386) 755 - 8699

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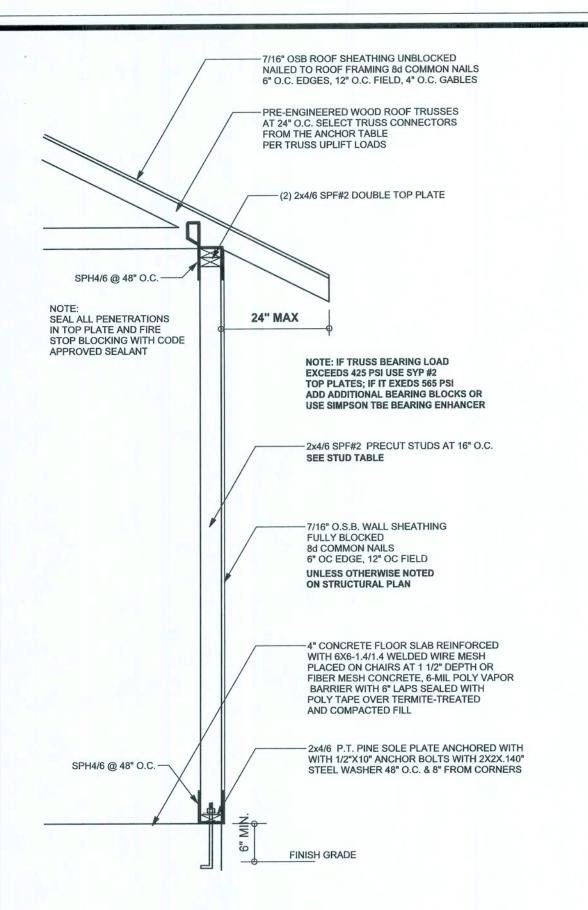
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Chuck Wool
DESIGNED BY:

Chuck Wood

FINALES DATE:
06 / OCT / 04

JOBNUMBER:

DRAWING NUMBER
3



ONE STORY WALL SECTION

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS** 

(1) 2x4 @ 16" OC TO 11'-9" STUD HEIGHT

(1) 2x4 @ 12" OC TO 13'-0" STUD HEIGHT

(1) 2x6 @ 16" OC TO 18'-10' STUD HEIGHT

(1) 2x6 @ 12" OC TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

SIMPSON H2.5A U.N.O. -SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21-

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

ESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

SCALE: 3/4" = 1'-0"

#### -(2) 2X4 SPF #2 TOP PLATE (2) SIMPSON S14 w/ (6) - 10d ----SIMPSON SPH4 @ 48" O.C. (2) SIMPSO LSTA21 w/ (8) -16d THEADER AND (8) -16FO STUD PACK -(2) 2X12 SYP #2 HEADER U.N.O SEE STRUCTURAL PLAN (2) KING STUDS-(2) JACKS STUDS w/ (2) ROWS 10d @ w/ (2) RNS 10d @ 12" O.C.ACH SIDE 12" O.C. EACH SIDE SIMPSON LT31 w/ (18) - 10d 5/8" x 10" ANHOR BOLT

TYPICA GARAGE DOOR HEADER STRAPING DETAIL

FOUNDATION SEE

SEE FOOTING DETAILS

#### **GENERAL NOTES:**

2x6 SYP #2 GARAGE DOOR BUCK K ATTACHMENT

DOOR WIDTH 3/8" x 4" LAG | 16d | (2) ROWS OF | 131 x 3 1/4" GN

5" O.C.

**GRADE & SPECIES TABLE** 

SYP #2

SYP #2

SYP #2

24F-V3 SP

MICROLAM

PARALAM

TIMBERSTRAND 1700

PRE ENGINEERED ROOF TRUSS -

DOU)UBLE 2x4 SPF TOP PLATE NAILED -

TOG)GETHER W/2-16d NAILS AT 16" O.C. 4' MIMIN. LAP w/ (12) - 16d OR 4" LAP w/

S2(320 w/ (4) - 16d &(14) - 10d

INTETERIOR CEILING AS -

TO TOP P PLATE AT

SCAL ALE: N.T.S.

IF TRUSS TO WALL STRAPS S ARE NAILED

BOTTOM)M CHORD OF TRUSS

SPEGECIFIED ON FLOOR PLAN

AI ALL STUDS TO BE 2x4
SI SPF NAILED TO TOP
AI AND BOTTOM PLATES

CCONTINUOUS FRAME TO

**CEEILING DIAPHRAGM DETAIL** 

2x10

2x12

GLB

Fb (psi) E (10<sup>6</sup> psi

1.6

1.6

1.8

1.9

1200

1050

975

2400

1600

2900

ATTACH GARAGE DOOR BUCK TO STUD P, PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"8"x4" LAG SCREWS W/ 1" WASHER LAG SCREWS MA/JAY BE

STAGGER 16d NAILS OR (2) ROWS OF .13 | 31 x 3 1/4"

COUNTERSUNK. HORIZONTAL JAMBS DO O NOT

FRANSFER LOAD. CENTER LAG SCREWS S OR

18" O.C.

GARAGE DOOR BUCK INSTALLLATION DETAIL

16' - 18' 16" O.C.

GN PER TABLE BELOW:

11' - 15'

2x6SYP #2 DOOR BUCK-

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER, IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIA

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

 $\mbox{\bf NAILS}.$  ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

	NER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE RT OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	FOUNDATION BEARING CAPACITY, GRADE AND ED AND DEBRIS ZONE, AND FLOOD ZONE.
	ONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 FATED WIND VELOCITY AND DESIGN PRESSURES.
	AD PATH FROM TRUSSES TO FOUNDATION. IF YOU CONTINUOUS LOAD PATH CONNECTION, CALL MMEDIATELY.
VERIEV THE TRUSS MANUEAU	CTURER'S SEALED ENGINEERING INCLUDES TRUSS

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

### ROOF SYSTEM DESIGN

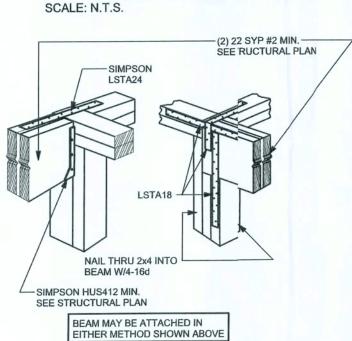
BEARING LOCATIONS.

# ----NON-SUPPORTIVE 2X4 LADDER BEAM

BEAM MID-WALL CONECTION DETAIL SCALE: N.T.S.

TOGETHEN/2-16d

MIN. (SEE RUCTURAL PLAN)



SEE STRUCTURAL PLAN

-(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

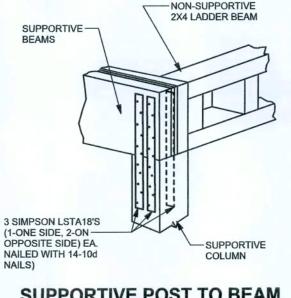
TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

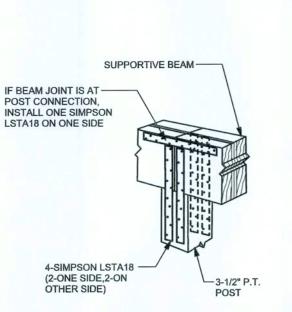
ANCHOR BOLT

SEE STRUCTURAL PLAN

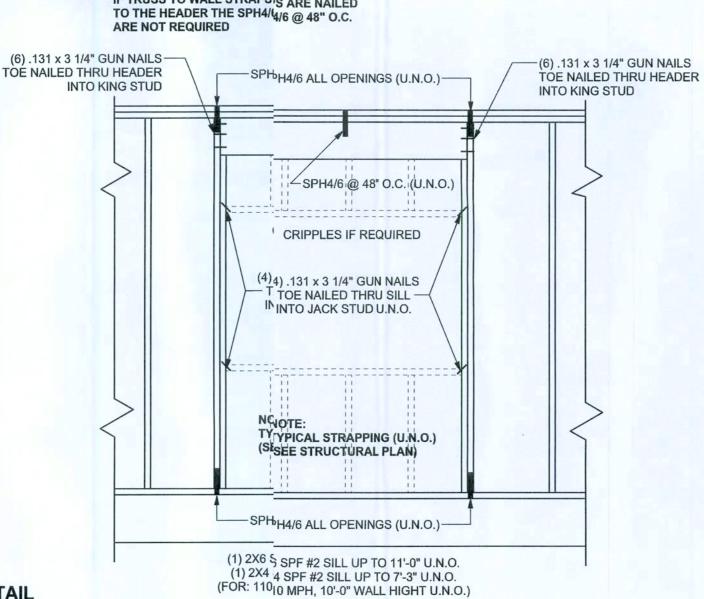
BEAM CORNER CONNECTION. DETAIL



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL



TYPICAL HIEADER STRAPING DETAIL

### **MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR IMASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Typie N, UNO		
2.2	Grout	ASTM C 476, admixtures require approva		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not		

detailed on project drawings.

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10530

< 9250

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 9035

< 9250

< 435

< 420

< 825

< 760

< 1065

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 3335

< 2200

< 2300

< 2320

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

22 -10d

16 -10d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

6-10d

2-10d

14-10d

16-10d

18-8d

TO STUDS

8-16d

18-10d, 1 1/2

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

10-10d, 1 1/2"

H2.5

H2.5A

H14-1

H10-1

H10-2

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWNS

MGT

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR\*

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

SPH4

SP6

LSTA18

LSTA21

CS20

CS16 STUD ANCHORS\*

LTT19

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU88

DSP SINGLE SILL PLAT

TO STUDS

TO FOUNDATION

5/8" THREADED ROL

2-5/8" THREADED ROD

12" EMBEDMENT

5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED RO

TO STUDS

4-10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

6-10d, 1 1/2"

12" EMBEDMENT

WIND LOADS PER FLORIDA BUILDING CODE	2004 RESIDENTIA	AL, SE	CTIO	N R30	1.2.1	
(ENCLOSED SIMPLE DIAPHRAGM BUILDING MEAN ROOF HEIGHT NOT EXCEEDING LEAS ON UPPER HALF OF HILL OR ESCARPMENT SLOPE AND UNOBSTRUCTED UPWIND FOR	ST HORIZONTAL D 60FT IN EXP. B. 3	IMEN OFT II	ISION N EXP	OR 60	FT; NO	
BUILDING IS NOT IN THE HIGH VELOCITY HU	JRRICANE ZONE					
BUILDING IS NOT IN THE WIND-BORNE DEB	RIS REGION					
1.) BASIC WIND SPEED = 110 MPH						
2.) WIND EXPOSURE = B						
3.) WIND IMPORTANCE FACTOR = 1.0						
4.) BUILDING CATEGORY = II						
5.) ROOF ANGLE = 10-45 DEGREES						
6.) MEAN ROOF HEIGHT = <30 FT						
7.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLOSED E	TIII D	ING)			
8.) COMPONENTS AND CLADDING DESIGN			,	D204	2/2//	
any commentative full objection	WINDTILLOOOK	20 (1	ADLL	11301.	2(2))	
~~	Zone	Zone Effective Wind Area (ft2)				
		_	10		100	
2 2 2	1 2	_	-21.8 -25.5	_	-18.1 -21.8	
	2 O'hg	10.0	-40.6	10.1	-40.6	
2 2 1	3	19.9	-25.5	18.1	-21.8	
2 4 5	3 O'hg		-68.3		-42.4	
Y3 4	4	21.8	-23.6	18.5	-20.4	
5 5	5	21.8	-29.1	18.5	-22.6	
The state of the s	Doors	Doors & Windows			-29.1	
2 2		Worst Case				
		(Zone 5, 10 ft2)				
2 /3		8x7 Garage Door 16x7 Garage Door		19.5	-22.9	
4 /2/ 4 5	16x7 G	arage	Door	18.5	-21.0	
155 /2-				-		

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS) 30 PSF (ATTICS WITH STORAGE)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

REVISIONS

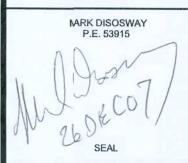
SOFTPLAN

INDLOAD EIGINEER: Mark Disosway PE No.53915, IOB 868, Lake City, FL 32056, 386-75-5419 Stated dimensins supercede scaled dimensions. Reer all questions to Mark Disosway P.E. for resolution. Do not proceecwithout clarification.

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examined this ran, and that the applicable portions of the lan, relating to wind enginee comply with setion R301.2.1, florida building code residentia 2004, to the best of my

LIMITATION: Tils design is valid for one building, at speified location.



Mke Roberts

Spec House Lot 18 Crosswinds S/D Phase 1

ADDRESS: 309SW Chesterfield Lake City, Florida 32024

MarkDisosway P.E. PO. Box 868 Lake Cty, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

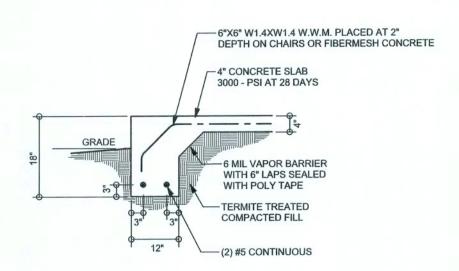
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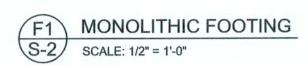
FINALS DA'E: 26 / Dec / 7

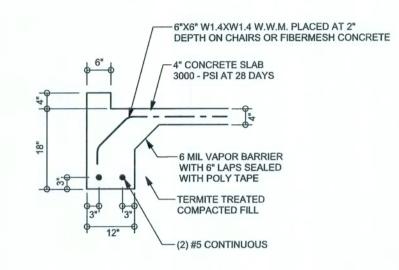
> JOB NUMBER 712261 DRAWING NUMBER

> > **S-1**

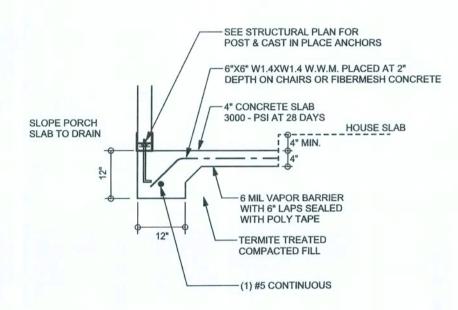
OF 3 SHEETS



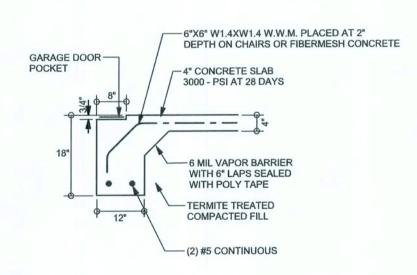




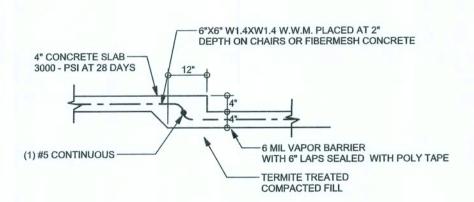
## F8 GARAGE CURB FOOTING S-2 SCALE: 1/2" = 1'-0"



# F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



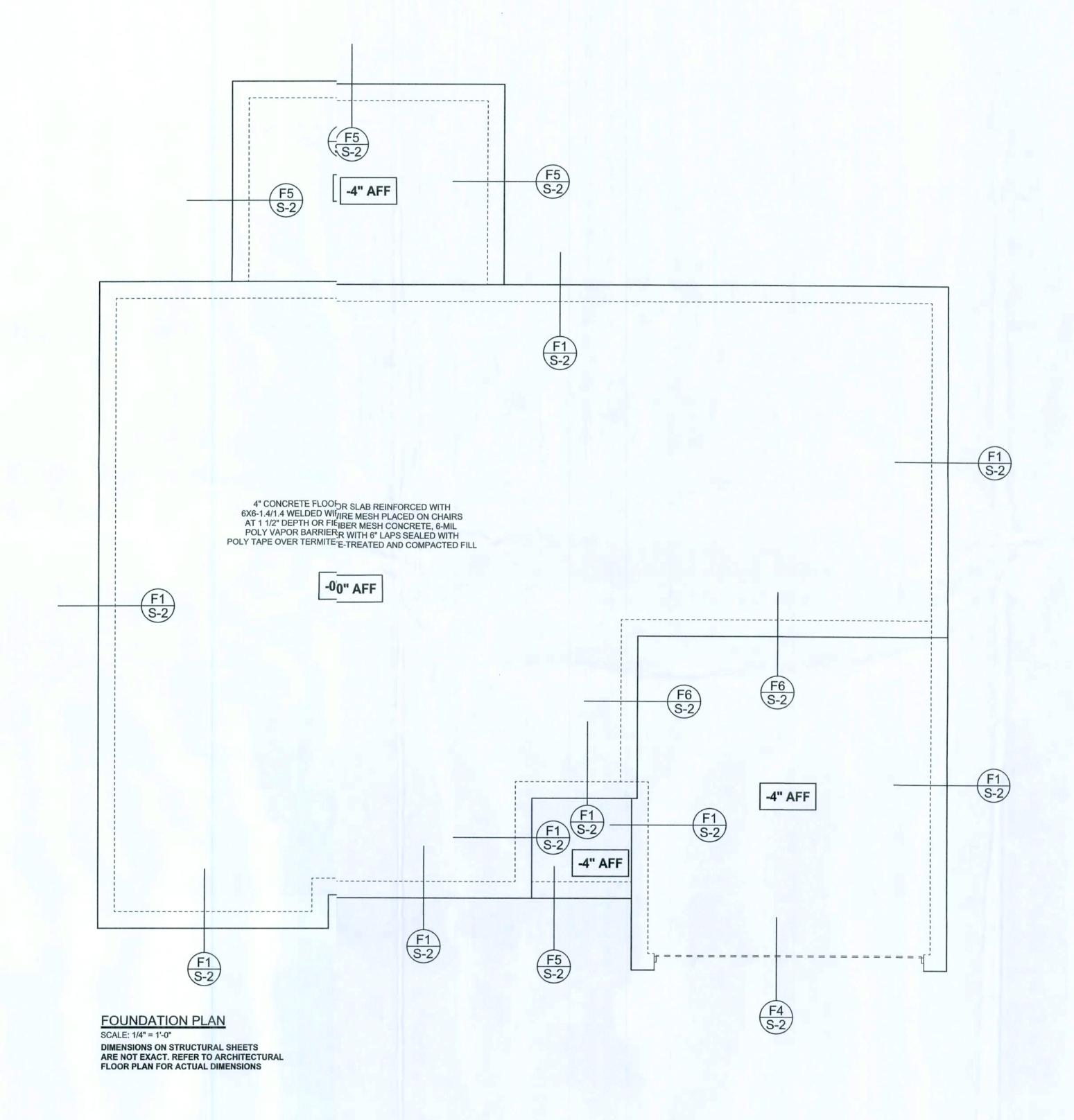
# F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



F6 TYPICAL NON - BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"

SOTPIÁN

**REVISONS** 



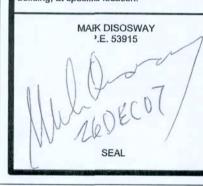
WINDLOAD ENGNEER: Mark Disosway, PE No.53915, POI 868, Lake City, FL 32056, 386-754-549

DIMENSIONS:
Stated dimensionssupercede scaled dimensions. Referall questions to Mark Disosway, PE. for resolution.
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CERTIFICATION: hereby certify that I have examined this plar and that the applicable portions of the plai, relating to wind engineering comply with sectic R301.2.1, florida building code residential 204, to the best of my knowledge.

LIMITATION: This lesign is valid for one building, at specifid location.



Mike Roberts

Spec House Lot 18 Crosswinds S/D Phase 1

ADDRESS: 309 SV Chesterfield Lake Ciy, Florida 32024

Mark Dsosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRNTED DATE:
December 26, 2007
STRUCTURAL BY:

JOB NUMBER: 712261

DRAWNG NUMBER **S-2** 

OF3 SHEETS

SEE PORCH POST DETAIL (TYP.) T18 TRUSS -539 LB —1150 LB UPLIFT SWS = 9.5' (3) 2X4 SPF # #2 STUDS
|| CENTERED | UNDER TRUSS 1778 LB — UPLIFT T17 TRUSS 774 LB — UPLIFT UPLIFT 622 LB UPLIFT ||| -(2) SPH4 TOP & BOTTOM-W AB WITHIN 3" (2) 2X12X6',2J 2K SWS 4.5' SWS = 8.0' (2(2) 2X10X16',2J 2K + 2X6 SYP #2 F'FOR 7'-0" JACK TRUSS LOAD MAX 1492 LB-UPLIFT UPLIFT SEE GARAGE DOOR HEIEADER STRAPING DETAIL (TYPYP.) SEE GARAGE DOOR HEADER STRAPING DETAIL (TYP.) STRUCTIRAL PLAN NOTES WALL LEGEND

AI LOAD BEARING FRAME WALL & PORCH HEADERS

PRMANENT TRUSS BRACING IS TO BE INSTALLED AT

LCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LÆRAL BRACING IS TO BE RESTRAINED PER BCSI1-03,

B6I-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 AF FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

SALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

AI LOAD BEARING FRAME WALL HEADERS SALL HAVE (1) JACK STUD & (1) KING STUD

DIENSIONS ON STRUCTURAL SHEETS AE NOT EXACT. REFER TO ARCHITECTURAL

FIOR PLAN FOR ACTUAL DIMENSIONS

ECH SIDE (U.N.O.)

TRSS PACKAGE

SWS = 0.0'

SWS = 0.0'

1ST FLOOR EXTERIOR WAVALL

2ND FLOOR EXTERIOR WALL

1ST FLOOR INTERIOR BEARING WALL

2ND FLOOR INTERIOR BE/EARING WALL

**HEADER LEGEND** 

(2) 2X10X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.)

-----SPAN OF HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

REQUIRED ACTUAL TRANSVERSE 33.6' 85.5'

LONGITUDINAL 28.6'

——NUMBER OF KING STUDS (FULL LENGTH)

SIZE OF HEADER MATERIAL

---NUMBER OF PLIES IN HEADER

NUMBER OF JACK STUDS (UNDER HEADER)

**REVISIONS** 

SOFTPIAN

WINDLOAD ENGIN:ER: Mark Disosway, PE No.53915, POB:68, Lake City, FL 32056, 386-754-54®

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CERTIFICATION: I lereby certify that I have examined this plan, ind that the applicable portions of the plan, elating to wind engineering comply with section 301.2.1, florida building code residential 200, to the best of my knowledge.

LIMITATION: This disign is valid for one building, at specifiedocation.

MARIFDISOSWAY P.E. 53915

#### Mike Roberts

Lot 18 Phase 1

> PRIN'ED DATE: Decemer 26, 2007

JOB NUMBER: 7.2261

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L229281

Spet House Crossvinds S/D

ADDRESS: 309 SW Chesterfield Lake City Florida 32024

Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (316) 754 - 5419 Fax: (386) 269 - 4871

STRUCTURAL BY:

FINALS DATE: 26 / Dec / 07

> DRAWING NUMBER **S-3**

> > OF (SHEETS