



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: 1125-003 - Johnny Mathis

MiTek, Inc.

16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Customer Info: Johnny Mathis Project Name: . Model: .
Lot/Block: . Subdivision: .
Address: ., .
City: Lake City State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

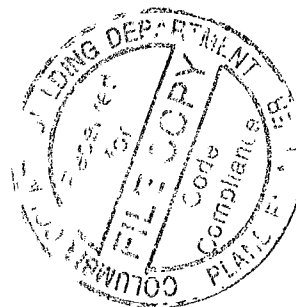
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Design Program: MiTek 20/20 8.8
Wind Code: ASCE 7-22 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 9 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T39093311	A01	11/5/25
2	T39093312	A02	11/5/25
3	T39093313	A03	11/5/25
4	T39093314	A04	11/5/25
5	T39093315	A05	11/5/25
6	T39093316	A06	11/5/25
7	T39093317	A07	11/5/25
8	T39093318	B01	11/5/25
9	T39093319	B02	11/5/25



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc.
under my direct supervision based on the parameters
provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2027.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Julius Lee PE No. 34869
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date

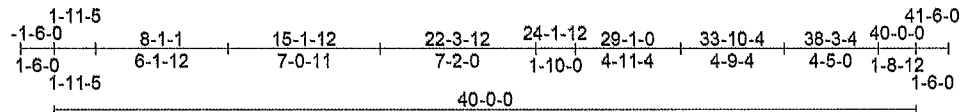
November 6, 2025

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093311
1125-003	A01	Roof Special Structural Gable	1	1	Job Reference (optional)	

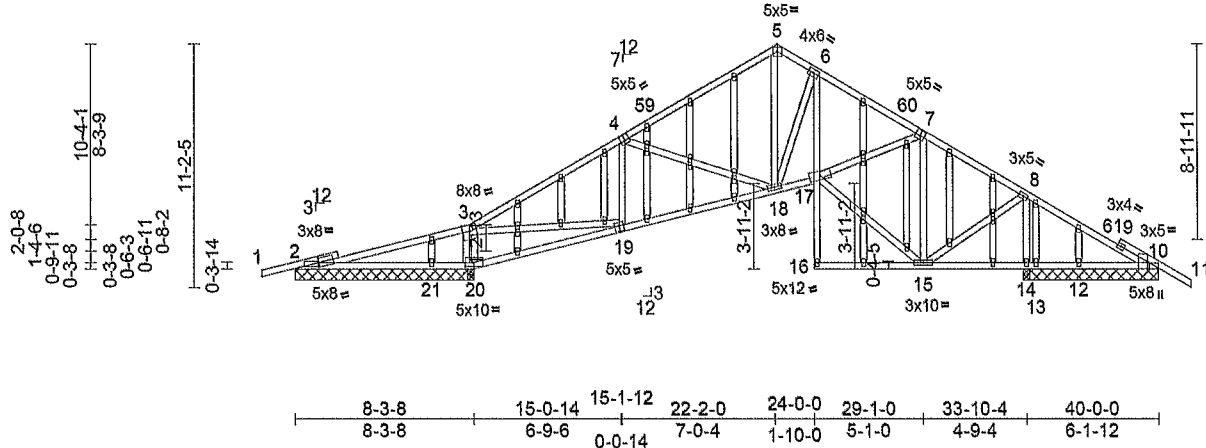
Mayo Truss Company, Inc., Mayo, FL - 32066,

Run 8 83 S Sep 3 2025 Print: 8 830 S Sep 3 2025 MITek Industries, Inc. Wed Nov 05 09:37:25
ID:76hBudwlBesBO5h8zvacxlyMHbS-RfC?PsB70Hq3NSgPqnL6w3uiTXbGKWRCDoI7J4zJC?I

Page. 1



LATERALLY BRACE TOP CHORD WITH PURLINS AT 2-0-0 o/c
IF STRUCTURAL SHEATHING IS NOT DIRECTLY APPLIED



Scale = 1 106.5

Plate Offsets (X, Y) [2 0-0-4,0-2-6], [2 Edge,0-2-4], [4-0-2-8,0-3-4] [7 0-2-8,0-3-0], [10 0-3-8,Edge], [10:0-0-9,Edge], [19-0-2-8,0-3-0], [20 0-5-0,0-1-4], [24 0-1-11,0-1-0], [25 0-1-11,0-1-0], [28 0-1-11,0-1-0], [35 0-1-8,0-1-0], [41-0-1-12,0-1-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 58	0 07	21-55	>999	240	MT20	244/190
TCDL	10 0	Lumber DOL	1 25	BC	0 55	-0 16	18-19	>999	180		
BCLL	0 0*	Rep Stress Incr	YES	WB	0 56	0 08	14	n/a	n/a		
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS						Weight 306 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 2
OTHERS 2x4 SP No 2

BRACING

TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied

REACTIONS

(size) 2=8-3-8, 10=6-3-8, 12=6-3-8,
13=6-3-8, 14=0-3-8, 20=8-3-8,
21=8-3-8
Max Horiz 2=195 (LC 11)
Max Uplift 2=-76 (LC 8), 10=-135 (LC 23),
13=-193 (LC 1), 20=-28 (LC 12)
Max Grav 2=433 (LC 23), 10=4 (LC 24),
12=132 (LC 3), 13=-19 (LC 12),
14=1830 (LC 1), 20=1201 (LC 1),
21=231 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum
Tension
TOP CHORD 1-2=0/22, 2-3=-74/276, 3-5=-1323/95,
5-6=-887/110, 6-8=-1143/130, 8-10=-51/849,
10-11=0/45
BOT CHORD 2-21=-188/27, 20-21=-188/27,
18-20=-315/1139, 17-18=0/937, 16-17=0/91,
6-17=0/324, 15-16=0/20, 14-15=-666/83,
13-14=-666/83, 12-13=-666/83,
10-12=-666/83
WEBS 6-18=-418/41, 3-20=-1114/291, 4-19=-204/82,
3-19=0/1339, 4-18=-339/148, 5-18=-19/602,
7-17=0/626, 8-14=-1581/104, 7-15=-876/18,
15-17=0/431, 8-15=-1/1217

NOTES

1) Unbalanced roof live loads have been considered for
this design

- 2) Wind ASCE 7-22, Vult=130mph (3-second gust)
Vasd=101mph, TCDL=6.0psf, BCDL=6 0psf; h=15ft,
B=45ft, L=40ft, eave=5ft; Cat. II, Exp B, Enclosed,
MWFRS (directional) and C-C Zone3 -1-6-0 to 2-3-7,
Zone1 2-3-7 to 22-3-12, Zone2 22-3-12 to 27-11-10,
Zone1 27-11-10 to 41-6-0 zone, cantilever left and right
exposed, end vertical left and right exposed, C-C for
members and forces & MWFRS for reactions shown,
Lumber DOL=1 60 plate grip DOL=1 60
- 3) Truss designed for wind loads in the plane of the truss
only For studs exposed to wind (normal to the face),
see Standard Industry Gable End Details as applicable,
or consult qualified building designer as per ANSI/TPI 1
- 4) Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- 5) All plates are 2x4 (I) MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10 0 psf bottom
chord live load nonconcurrent with any other live loads
- 8) * This truss has been designed for a live load of 20 0psf
on the bottom chord in all areas where a rectangle
3-06-00 tall by 2-00-00 wide will fit between the bottom
chord and any other members
- 9) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 76 lb uplift at joint
2, 135 lb uplift at joint 10, 28 lb uplift at joint 20, 193 lb
uplift at joint 13, 76 lb uplift at joint 2 and 135 lb uplift at
joint 10
- 10) This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord

LOAD CASE(S) Standard



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16023 Swingley Ridge Rd Chesterfield, MO 63017
Date:

November 6, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 1/2/2023 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

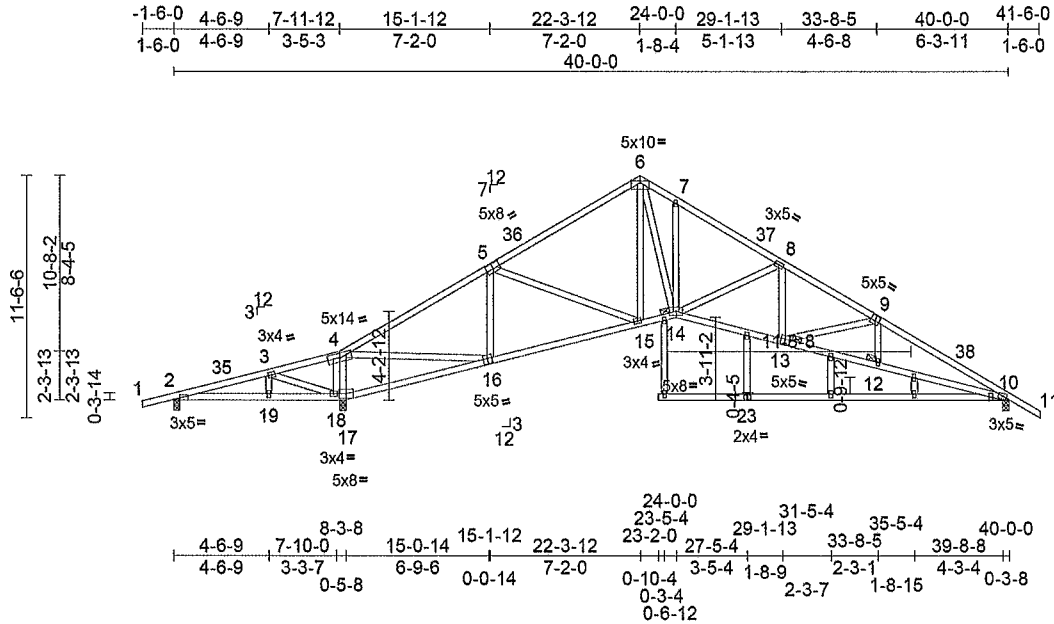
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Chesterfield, MO 63017
314.434.1200 / MITek-US.com

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093312
1125-003	A02	Roof Special	4	1	Job Reference (optional)	

Mayo Truss Company, Inc. Mayo, FL - 32066,

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Page: 1



Scale = 1 110 1

Plate Offsets (X, Y) [2 0-3-4,Edge], [5 0-4-0,0-3-0], [9 0-2-8,0-3-0], [13 0-2-8,0-3-0], [16 0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0.48	Vert(LL)	-0 18	13-14	>999	240	MT20	244/190
TCDL	10 0	Lumber DOL	1.25	BC	0 73	Vert(CT)	-0.38	13-14	>999	180		
BCLL	0 0*	Rep Stress Incr	YES	WB	0 54	Horz(CT)	0 23	10	n/a	n/a		
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							Weight. 260 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No 2

BOT CHORD 2x4 SP No 2

WEBS 2x4 SP No 2

BRACING

TOP CHORD Structural wood sheathing directly applied

BOT CHORD Rigid ceiling directly applied

JOINTS 1 Brace at Jt(s) 13, 14, 12

REACTIONS

(size) 2=0-3-8, 10=0-3-8, 17=0-3-8
Max Horiz 2=193 (LC 11)
Max Uplift 2=-120 (LC 8), 10=-36 (LC 12), 17=-57 (LC 12)
Max Grav 2=207 (LC 23), 10=1283 (LC 1), 17=1964 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=0/625, 3-4=-13/1109, 4-6=-1595/115, 6-7=-1882/165, 7-8=-1963/85, 8-10=-3194/97, 10-11=0/45
BOT CHORD 2-19=-569/0, 18-19=-569/0, 17-18=-1042/113, 15-17=-1300/1249, 14-15=0/1305, 12-14=0/2771, 10-12=0/2770
WEBS 6-14=-98/1369, 5-16=-586/102, 5-15=-63/232, 6-15=-8/189, 7-14=-206/138, 8-13=0/384, 8-14=-677/105, 9-13=-464/85, 9-12=0/188, 4-17=-1320/223, 4-16=-29/2400, 4-18=-134/0, 3-18=-601/201, 3-19=-60/189

NOTES

1) Unbalanced roof live loads have been considered for this design

- Wind ASCE 7-22, Vult=130mph (3-second gust) Vasd=101mph, TCDL=6 0psf; BCDL=6 0psf; h=15ft, B=45ft, L=40ft; eave=5ft, Cat. II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 -1-6-0 to 2-6-0, Zone1 2-6-0 to 22-3-12, Zone2 22-3-12 to 27-11-10, Zone1 27-11-10 to 41-6-0 zone, cantilever left and right exposed, end vertical left and right exposed, porch left exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 (I) MT20 unless otherwise indicated
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 120 lb uplift at joint 2, 36 lb uplift at joint 10 and 57 lb uplift at joint 17
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard



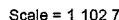
Julius Lee PE No 34869
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd Chesterfield, MO 63017
Date:

November 6, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 rev 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpiinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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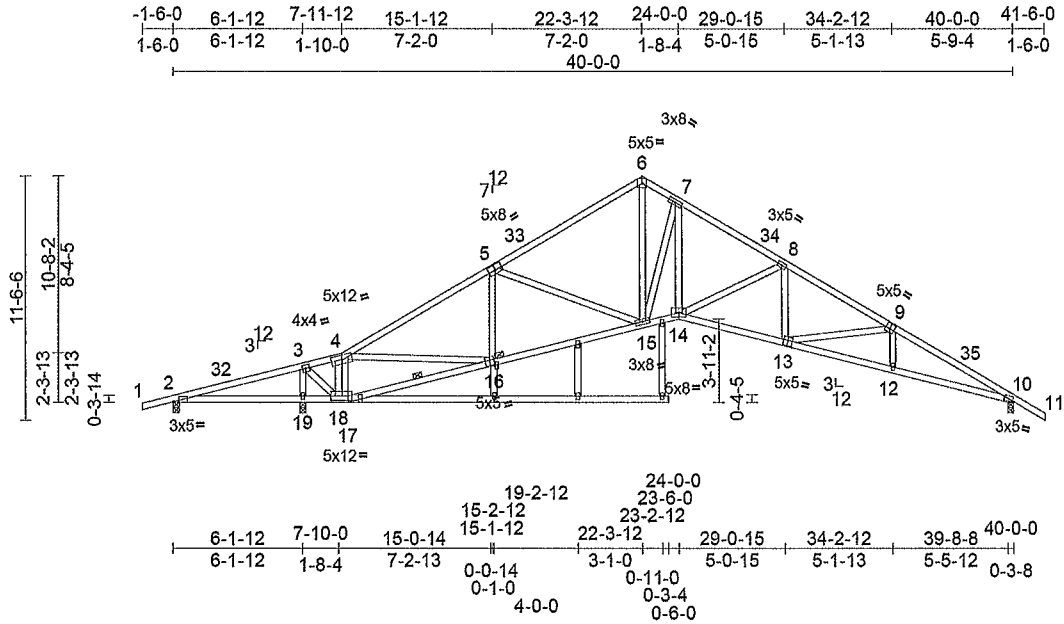
Page 1

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093314
1125-003	A04	Roof Special	4	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066

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Page 1



Scale = 1/109.4

Plate Offsets (X, Y) [2 0-3-4,Edge], [5.0-4-0,0-3-0], [9 0-2-8,0-3-0], [10 0-0-14,0-0-7], [13-0-2-8,0-3-0], [16 0-2-8,0-3-0], [16 0-2-0,0-0-8], [17 0-9-12,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 62	Vert(LL)	-0 23	13-14	>999	240	MT20	244/190
TCDL	10 0	Lumber DOL	1.25	BC	0 71	Vert(CT)	-0.47	13-14	>865	180		
BCLL	0 0*	Rep Stress Incr	YES	WB	0 85	Horz(CT)	0 28	10	n/a	n/a		
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							Weight. 254 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied
JOINTS 1 Brace at Jt(s) 16

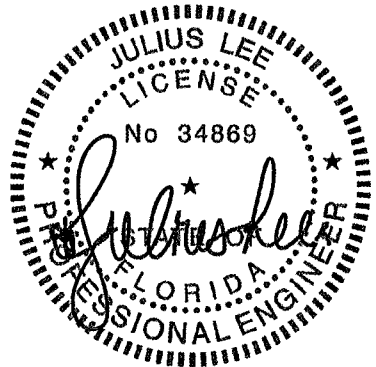
REACTIONS (size) 2=0-3-8, 10=0-3-8, 19=0-3-8
Max Horiz 2=193 (LC 11)
Max Uplift 2=-163 (LC 24), 10=-38 (LC 12), 19=-33 (LC 12)
Max Grav 2=20 (LC 23), 10=1368 (LC 1), 19=2095 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=0/1549, 3-4=-46/222, 4-6=-1989/133, 6-7=-1719/154, 7-8=-2247/108, 8-10=-3500/114, 10-11=0/45
BOT CHORD 2-19=-1457/48, 18-19=-1457/48, 17-18=-145/115, 15-17=-127/1718, 14-15=0/1911, 12-14=-22/3052, 10-12=-21/3043
WEBS 3-19=-1858/65, 7-15=-1327/99, 5-16=-340/84, 5-15=-291/149, 6-15=-78/1434, 7-14=0/1380, 8-14=-688/105, 8-13=0/381, 9-13=-483/91, 9-12=0/179, 4-17=0/384, 4-18=-1505/40, 3-18=0/1822, 4-16=0/1611

NOTES
1) Unbalanced roof live loads have been considered for this design

- Wind ASCE 7-22, Vult=130mph (3-second gust) Vasd=101mph, TCDL=6 0psf, BCDL=6 0psf, h=15ft, B=45ft, L=40ft, eave=5ft, Cat II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 -1-6-0 to 2-6-0, Zone1 2-6-0 to 22-3-12, Zone2 22-3-12 to 27-11-10, Zone1 27-11-10 to 41-6-0 zone, cantilever left and right exposed, end vertical left and right exposed, porch left exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component
- All plates are 2x4 (||) MT20 unless otherwise indicated
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 2, 33 lb uplift at joint 19 and 38 lb uplift at joint 10
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard



Julius Lee PE No. 34869
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

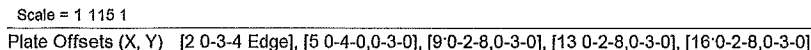
November 6, 2025

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Mayo Truss Company, Inc. Mayo FL - 32066, Run 8 83 S Sep 3 2025 Print: 8 830 S Sep 3 2025 MiTek Industries, Inc. Wed Nov 05 09:37:28 Page 1
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LUMBER
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No 2

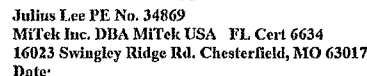
BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied
JOINTS 1 Brace at Jt(s). 16

REACTIONS
(size) 2=0-3-8, 10=0-3-8, 17=0-3-8
Max Horiz 2=193 (LC 11)
Max Uplift 2=-120 (LC 8), 10=-36 (LC 12),
17=-57 (LC 12)
Max Grav 2=207 (LC 23), 10=1283 (LC 1),
17=1964 (LC 1)

FORCES
(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=0/626, 3-4=-13/1106,
4-6=-1595/115, 6-7=-1873/165,
7-8=-1962/85, 8-10=-3194/97, 10-11=0/45
BOT CHORD 2-19=-569/0, 18-19=-569/0,
17-18=-1038/113, 15-17=-1296/1249,
14-15=0/1306, 12-14=0/2771, 10-12=0/2769
6-14=-98/1363, 5-16=-585/102,
5-15=-63/232, 6-15=-8/189, 7-14=-206/138,
8-13=0/384, 8-14=-677/105, 9-13=-464/85,
9-12=0/188, 4-17=-1321/223, 4-16=-29/2394,
4-18=-134/0, 3-18=-593/201, 3-19=-60/189

- 2) Wind ASCE 7-22, Vult=130mph (3-second gust)
Vasd=101mph, TCDL=6 0psf; BCDL=6 0psf; h=15ft,
B=45ft, L=40ft; eave=5ft, Cat. II, Exp B, Enclosed,
MWFRS (directional) and C-C Zone3 -1-6-0 to 2-6-0,
Zone1 2-6-0 to 22-3-12, Zone2 22-3-12 to 27-11-10,
Zone1 27-11-10 to 41-6-0 zone, cantilever left and right
exposed , end vertical left and right exposed, porch left
exposed,C-C for members and forces & MWFRS for
reactions shown, Lumber DOL=1 60 plate grip
DOL=1 60
- 3) Building Designer / Project engineer responsible for
verifying applied roof live load shown covers rain loading
requirements specific to the use of this truss component.
- 4) All plates are 2x4 (||) MT20 unless otherwise indicated
- 5) This truss has been designed for a 10 0 psf bottom
chord live load nonconcurrent with any other live loads
- 6) * This truss has been designed for a live load of 20 0psf
on the bottom chord in all areas where a rectangle
3-06 00 tall by 2-00-00 wide will fit between the bottom
chord and any other members
- 7) Bearing at joint(s) 10 considers parallel to grain value
using ANSI/TPI 1 angle to grain formula Building
designer should verify capacity of bearing surface
- 8) Provide mechanical connection (by others) of truss to
bearing plate capable of withstanding 120 lb uplift at joint
2, 36 lb uplift at joint 10 and 57 lb uplift at joint 17
- 9) This truss design requires that a minimum of 7/16"
structural wood sheathing be applied directly to the top
chord and 1/2" gypsum sheetrock be applied directly to
the bottom chord

LOAD CASE(S) Standard



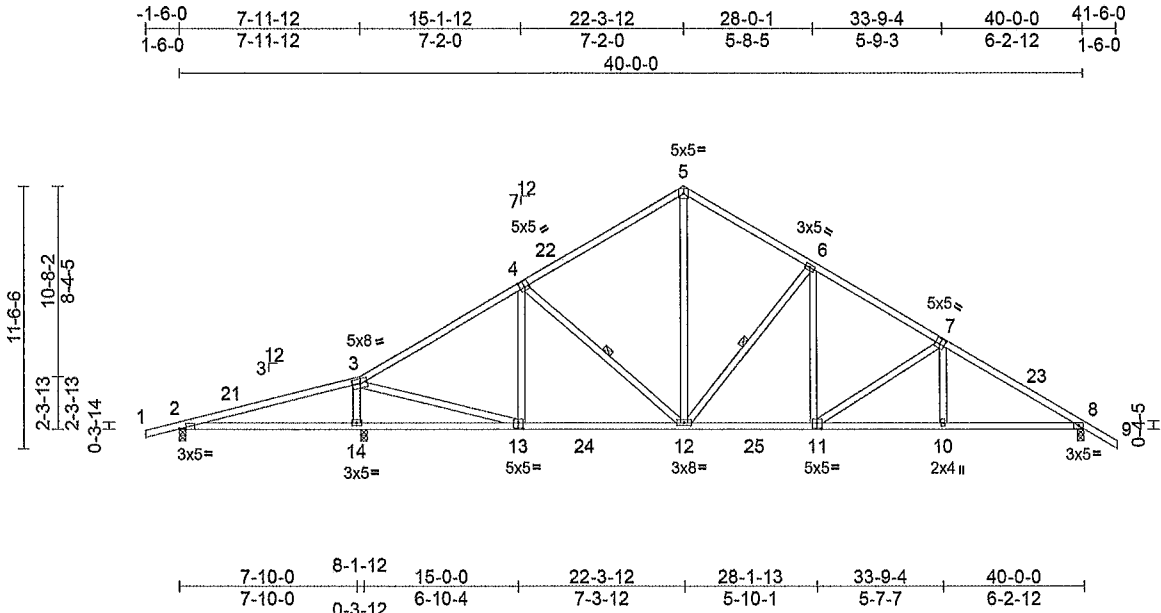
November 6, 2025

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI-1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpiinst.org) and BCS Building Component Safety Information available from the Structural Building Component Association (www.sbcsc.com/components.com)

MiTek
16023 Swingley Ridge Rd
Chesterfield MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093316
1125-003	A06	Roof Special	5	1	Job Reference (optional)	



Scale = 1 101 6

Plate Offsets (X, Y). [2 0-3-4,Edge], [4 0-2-8,0-3-4], [7 0-2-8,0-3-0], [11 0-2-8,0-3-0], [13:0-2-8,0-3-0]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 58	Vert(LL)	0 12	14-17	>768	240	MT20	244/190
TCDL	10 0	Lumber DOL	1 25	BC	0 60	Vert(CT)	-0 24	12-13	>999	180		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 46	Horz(CT)	0 06	8	n/a	n/a		
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							Weight. 221 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 2

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied
WEBS 1 Row at midpt 4-12, 6-12

REACTIONS (size) 2=0-3-8, 8=0-3-8, 14=0-3-8
Max Horiz 2=193 (LC 11)
Max Uplift 2=-108 (LC 12), 8=-36 (LC 12), 14=-60 (LC 12)
Max Grav 2=324 (LC 23), 8=1515 (LC 18), 14=1909 (LC 17)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/22, 2-3=-97/438, 3-5=-1548/197, 5-6=-1325/204, 6-8=-2374/166, 8-9=0/45
BOT CHORD 2-14=-357/80, 12-14=-246/1357, 10-12=-3/1975, 8-10=-2/1979
WEBS 3-14=-1654/139, 3-13=0/1531, 4-13=-234/96, 4-12=-291/104, 5-12=-58/968, 6-12=-760/121, 6-11=0/534, 7-11=-571/70, 7-10=0/247

NOTES
1) Unbalanced roof live loads have been considered for this design
2) Wind ASCE 7-22, Vult=130mph (3-second gust) Vasd=101mph, TCDL=6 0psf; BCDL=6 0psf; h=15ft, B=45ft, L=40ft; eave=5ft, Cat. II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 -1-6-0 to 2-6-0, Zone1 2-6-0 to 22-3-12, Zone2 22-3-12 to 28-0-1, Zone1 28-0-1 to 41-6-0 zone, cantilever left and right exposed , end vertical left and right exposed, porch left exposed,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component
- 4) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10 0psf
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 2, 60 lb uplift at joint 14 and 36 lb uplift at joint 8
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard



Julius Lee PE No 34869
MiTek Inc. DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd Chesterfield, MO 63017
Date:

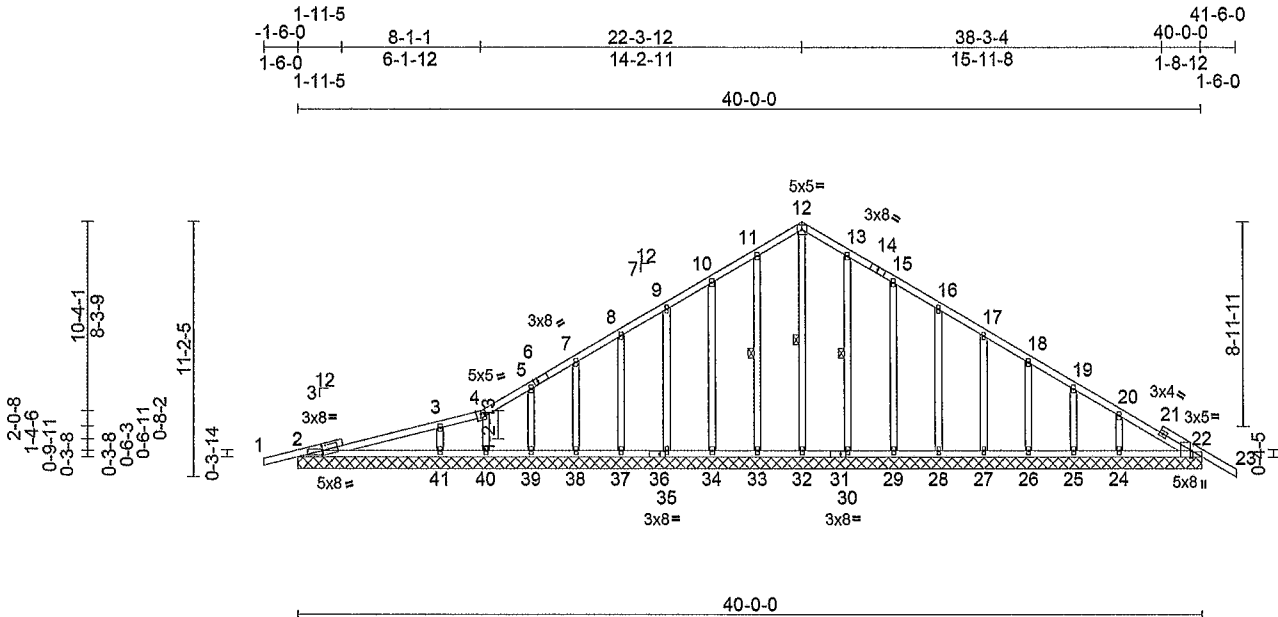
November 6,2025

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093317
1125-003	A07	Roof Special Supported Gable	1	1	Job Reference (optional)	

Mayo Truss Company, Inc. Mayo, FL - 32066, Run 8 83 S Sep 3 2025 Print. 8 830 S Sep 3 2025 MiTek Industries, Inc. Wed Nov 05 09:37:28 Page 1


LATERALLY BRACE TOP CHORD WITH PURLINS AT 2-0-0 o/c
IF STRUCTURAL SHEATHING IS NOT DIRECTLY APPLIED

ID sOr5mcYoLPA7U3f_NyMHeW-RIC7PsB70Hq3NSgPqnL8w3uITXbGKWCDol7J4zJC7f



Scale = 1 101 6												
Plate Offsets (X, Y) [2 0-0-4,0-2-6], [2:Edge,0-2-4], [6 0-2-2,0-1-8], [22 0-3-8,Edge], [22'0-0-9,Edge], [31'0-2-12,0-1-8], [36.0-2-12,0-1-8]												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 38	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10 0	Lumber DOL	1 25	BC	0 38	Vert(CT)	n/a	-	n/a	999		
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 12	Horz(CT)	0 01	45	n/a	n/a		
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							Weight. 259 lb	FT = 20%

LUMBER		TOP CHORD		1-2=0/22, 2-3=-137/129, 3-4=-127/106, 4-5=-125/130, 5-7=-118/109, 7-8=-105/105, 8-9=-96/173, 9-10=-88/240, 10-11=-109/312, 11-12=-128/369, 12-13=-128/369, 13-15=-109/312, 15-16=-85/240, 16-17=-64/173, 17-18=-52/105, 18-19=-59/42, 19-20=-76/49, 20-22=-129/81, 22-23=0/45		3) Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1	
TOP CHORD		2x4 SP No 2				4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.	
BOT CHORD		2x4 SP No.2				5) All plates are 2x4 (II) MT20 unless otherwise indicated	
OTHERS		2x4 SP No 2				6) Gable requires continuous bottom chord bearing	
BRACING		Structural wood sheathing directly applied				7) Gable studs spaced at 2-0-0 oc	
TOP CHORD		Rigid ceiling directly applied				8) This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads	
BOT CHORD		Rigid ceiling directly applied				9) * This truss has been designed for a live load of 20 Opsf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members	
WEBS		1 Row at midpt 12-32, 11-33, 13-30		BOT CHORD			
REACTIONS (size)		2=40-0-0, 22=40-0-0, 24=40-0-0, 25=40-0-0, 26=40-0-0, 27=40-0-0, 28=40-0-0, 29=40-0-0, 30=40-0-0, 32=40-0-0, 33=40-0-0, 34=40-0-0, 35=40-0-0, 37=40-0-0, 38=40-0-0, 39=40-0-0, 40=40-0-0, 41=40-0-0		2-41=-64/207, 40-41=-64/207, 39-40=-66/207, 38-39=-66/207, 37-38=-66/207, 35-37=-66/207, 34-35=-66/207, 33-34=-66/207, 32-33=-66/207, 30-32=-66/207, 29-30=-66/207, 28-29=-66/207, 27-28=-66/207, 26-27=-66/207, 25-26=-66/207, 24-25=-66/207, 22-24=-66/207			
Max Horiz		2=195 (LC 11)		WEBS			
Max Uplift		2=-55 (LC 8), 22=-24 (LC 12), 25=-24 (LC 12), 26=-14 (LC 12), 27=-17 (LC 12), 28=-15 (LC 12), 29=-20 (LC 12), 30=-6 (LC 12), 33=-6 (LC 12), 34=-20 (LC 12), 35=-15 (LC 12), 37=-16 (LC 12), 38=-16 (LC 12), 39=-18 (LC 12), 40=-81 (LC 17), 41=-3 (LC 12)		12-32=-270/61, 11-33=-126/90, 10-34=-120/119, 9-35=-120/108, 8-37=-122/112, 7-38=-112/105, 5-39=-153/132, 4-40=-19/34, 3-41=-377/225, 13-30=-126/90, 15-29=-120/119, 16-28=-120/109, 17-27=-119/110, 18-26=-125/112, 19-25=-101/106, 20-24=-180/130			
Max Grav		2=416 (LC 1), 22=242 (LC 1), 24=258 (LC 1), 25=126 (LC 18), 26=169 (LC 1), 27=158 (LC 18), 28=161 (LC 18), 29=160 (LC 18), 30=166 (LC 24), 32=165 (LC 12), 33=166 (LC 17), 34=160 (LC 1), 35=160 (LC 17), 37=164 (LC 23), 38=146 (LC 17), 39=220 (LC 1), 40=15 (LC 9), 41=588 (LC 1)		NOTES			
FORCES		(lb) - Maximum Compression/Maximum Tension		1) Unbalanced roof live loads have been considered for this design			
				2) Wind ASCE 7-22, Vult=130mph (3-second gust) Vasd=101mph, TC DL=6 Opsf; BCDL=6 Opsf; h=15ft, B=45ft, L=40ft, eave=2ft; Cat II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 zone, cantilever left and right exposed , end vertical left and right exposed,C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60			



Julius Lee PE No. 34869

Job	Truss	Truss Type	Qty	Ply	Johnny Mathis
1125-003	A07	Roof Special Supported Gable	1	1	T39093317
					Job Reference (optional)

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 2, 24 lb uplift at joint 22, 6 lb uplift at joint 33, 20 lb uplift at joint 34, 15 lb uplift at joint 35, 16 lb uplift at joint 37, 16 lb uplift at joint 38, 18 lb uplift at joint 39, 81 lb uplift at joint 40, 3 lb uplift at joint 41, 6 lb uplift at joint 30, 20 lb uplift at joint 29, 15 lb uplift at joint 28, 17 lb uplift at joint 27, 14 lb uplift at joint 26, 24 lb uplift at joint 25, 55 lb uplift at joint 2 and 24 lb uplift at joint 22.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

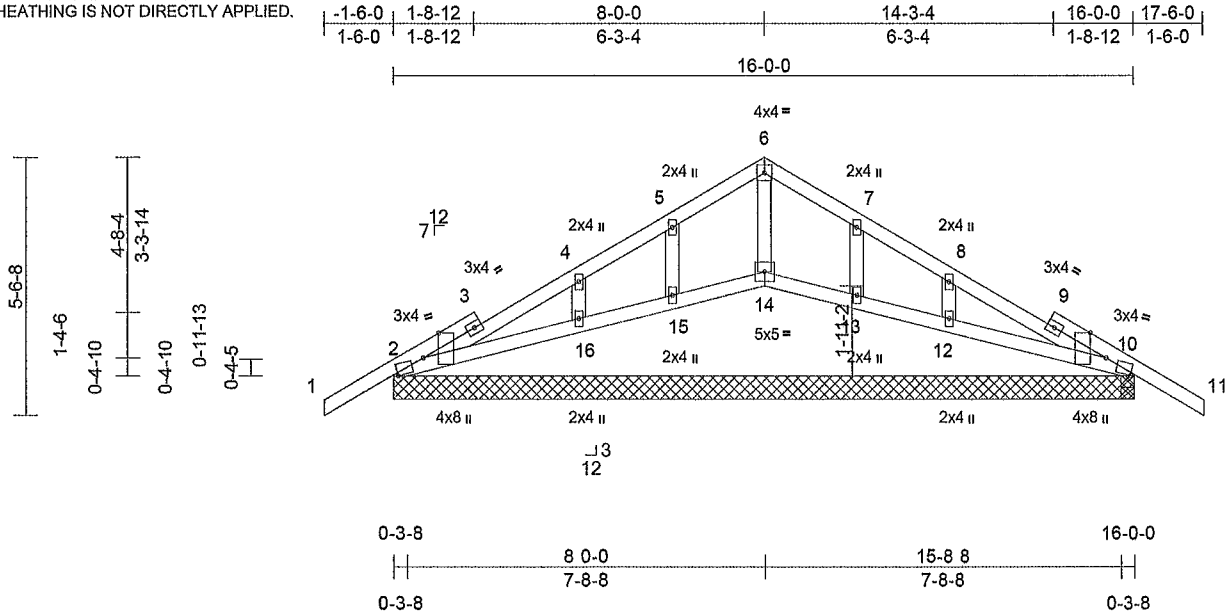
Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093318
1125-003	B01	Scissor	1	1	Job Reference (optional)	

Mayo Truss Company, Inc. Mayo, FL - 32066,

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Page 1

LATERALLY BRACE TOP CHORD WITH PURLINS AT 2-0-0 o/c
IF STRUCTURAL SHEATHING IS NOT DIRECTLY APPLIED.



Scale = 1.49.7											
Plate Offsets (X, Y) [2 0-6-6,Edge], [2-0-7-4,Edge], [10 0-6-6,Edge], [10 0-7-4,Edge]											
Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 17	Vert(LL)	0 00	16-19	>999	240	MT20
TCDL	10 0	Lumber DOL	1.25	BC	0 11	Vert(CT)	-0 01	16-19	>999	180	244/190
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 02	Horz(CT)	0 00	20	n/a	n/a	
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							
										Weight: 72 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
OTHERS 2x4 SP No 2

BRACING
TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied

REACTIONS (size) 2=16-0-0, 10=16-0-0, 12=16-0-0, 13=16-0-0, 14=16-0-0, 15=16-0-0, 16=16-0-0
Max Horiz 2=-90 (LC 10)
Max Uplift 2=-59 (LC 12), 10=-66 (LC 12), 12=-6 (LC 12), 13=-20 (LC 12), 15=-19 (LC 12), 16=-10 (LC 12)
Max Grav 2=242 (LC 23), 10=251 (LC 24), 12=266 (LC 1), 13=127 (LC 24), 14=184 (LC 17), 15=118 (LC 23), 16=285 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-4=-68/95, 4-5=-50/75, 5-6=-56/126, 6-7=-56/126, 7-8=-38/73, 8-10=-86/64, 10-11=0/50
BOT CHORD 2-16=-46/103, 16-16=-59/107, 14-15=-52/105, 13-14=-52/105, 12-13=-57/106, 10-12=-56/136
WEBS 6-14=-135/0, 5-15=-103/80, 4-16=-184/109, 7-13=-106/81, 8-12=-176/105

NOTES
1) Unbalanced roof live loads have been considered for this design

- Wind ASCE 7-22, Vult=130mph (3-second gust)
Vasd=101mph, TCDL=6 0psf; BCDL=6 0psf; h=15ft, B=45ft, L=24ft, eave=2ft, Cat II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 zone, cantilever left and right exposed, end vertical left and right exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP1 1
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 59 lb uplift at joint 2, 66 lb uplift at joint 10, 19 lb uplift at joint 15, 10 lb uplift at joint 16, 20 lb uplift at joint 13, 6 lb uplift at joint 12, 59 lb uplift at joint 2 and 66 lb uplift at joint 10
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard



Julius Lee PE No. 34869
MITek Inc DBA MITek USA FL Cert 6634
16023 Swingley Ridge Rd. Chesterfield, MO 63017
Date:

November 6, 2025

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Design valid for use only with MITek® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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Chesterfield MO 63017
314.434 1200 / MITek-US.com

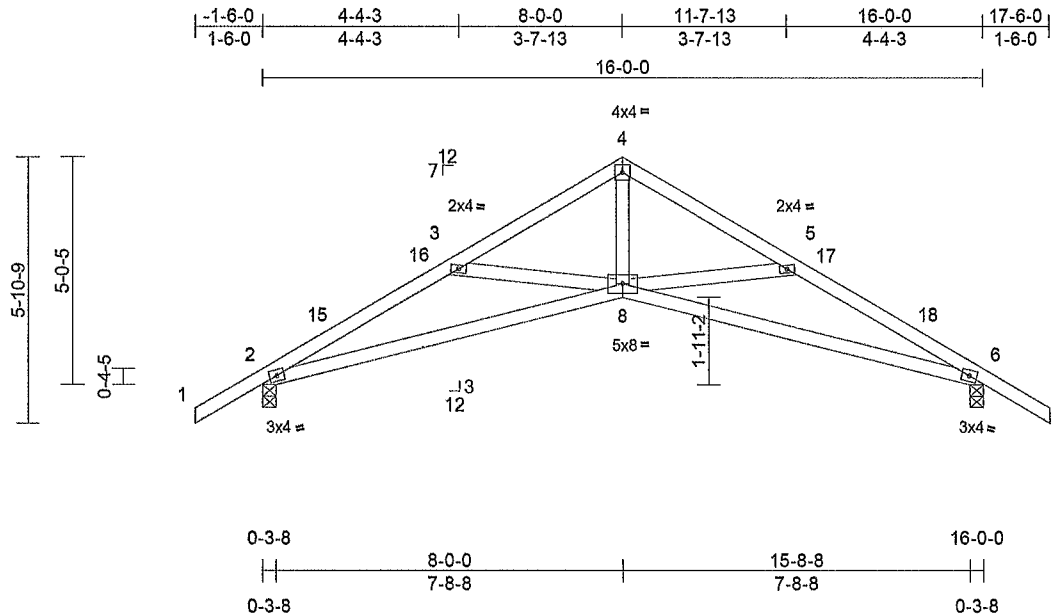
Job	Truss	Truss Type	Qty	Ply	Johnny Mathis	T39093319
1125-003	B02	Scissor	5	1	Job Reference (optional)	

Mayo Truss Company, Inc. Mayo, FL - 32066,

Run 8.83 S Sep 3 2025 Print. 8 830 S Sep 3 2025 MiTek Industries, Inc Wed Nov 06 08:37 29

Page: 1

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Scale = 1/51

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20 0	Plate Grip DOL	1 25	TC	0 23	Vert(LL)	-0 09	8-11	>999	240	MT20
TCDL	10 0	Lumber DOL	1 25	BC	0.53	Vert(CT)	-0 19	8-11	>998	180	244/190
BCLL	0 0 *	Rep Stress Incr	YES	WB	0 19	Horz(CT)	0 08	6	n/a	n/a	
BCDL	10 0	Code	FBC2023/TPI2014	Matrix-AS							Weight: 73 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No 2
BOT CHORD 2x4 SP No 2
WEBS 2x4 SP No 2

BRACING

TOP CHORD Structural wood sheathing directly applied
BOT CHORD Rigid ceiling directly applied

REACTIONS

(size) 2=0-3-8, 6=0-3-8
Max Horiz 2=-96 (LC 10)
Max Uplift 2=-36 (LC 12), 6=-36 (LC 12)
Max Grav 2=730 (LC 1), 6=730 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-1507/118, 3-4=-1147/48,
4-5=-1147/36, 5-6=-1507/131, 6-7=0/45
BOT CHORD 2-8=-26/1317, 6-8=-56/1317
WEBS 4-8=0/858, 3-8=-347/150, 5-8=-347/149

NOTES

- Unbalanced roof live loads have been considered for this design
- Wind ASCE 7-22, Vult=130mph (3-second gust)
Vasd=101mph, TCDL=6 0psf, BCDL=6 0psf, h=15ft, B=45ft, L=24ft; eave=4ft, Cat. II, Exp B, Enclosed, MWFRS (directional) and C-C Zone3 -1-6-0 to 1-6-0, Zone1 1-6-0 to 8-0-0, Zone2 8-0-0 to 12-2-15, Zone1 12-2-15 to 17-6-0 zone, cantilever left and right exposed, end vertical left and right exposed, C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1 60 plate grip DOL=1 60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component
- This truss has been designed for a 10 0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20 0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members

- Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula Building designer should verify capacity of bearing surface
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 2 and 36 lb uplift at joint 6
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

LOAD CASE(S) Standard



Julius Lee PE No. 34869
MiTek Inc DBA MiTek USA FL Cert 6634
16023 Swingley Ridge Rd Chesterfield, MO 63017
Date

November 6, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev 1/2/2023 BEFORE USE.

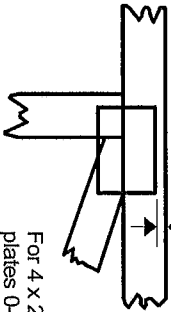
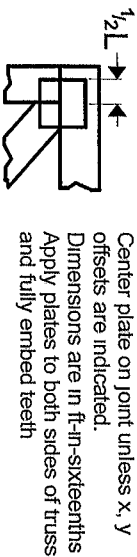
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcsccomponents.com)

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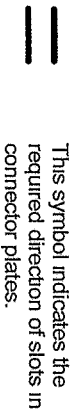
16023 Swingley Ridge Rd
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

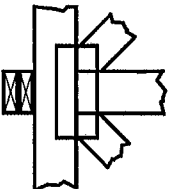
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or L bracing if indicated.

BEARING

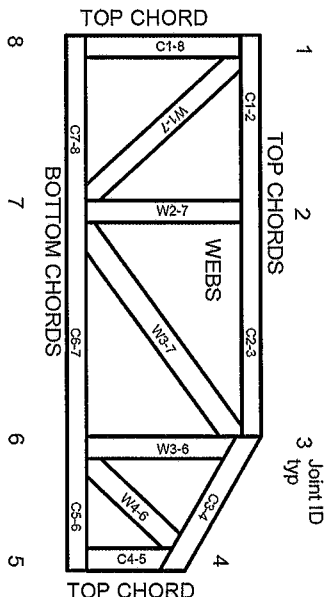


Indicates location where bearing (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only

Industry Standards:

ANSI/TP11* National Design Specification for Metal Plate Connected Wood Truss Construction
DSB-22 Design Standard for Bracing
BCSI Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraint & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports.

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

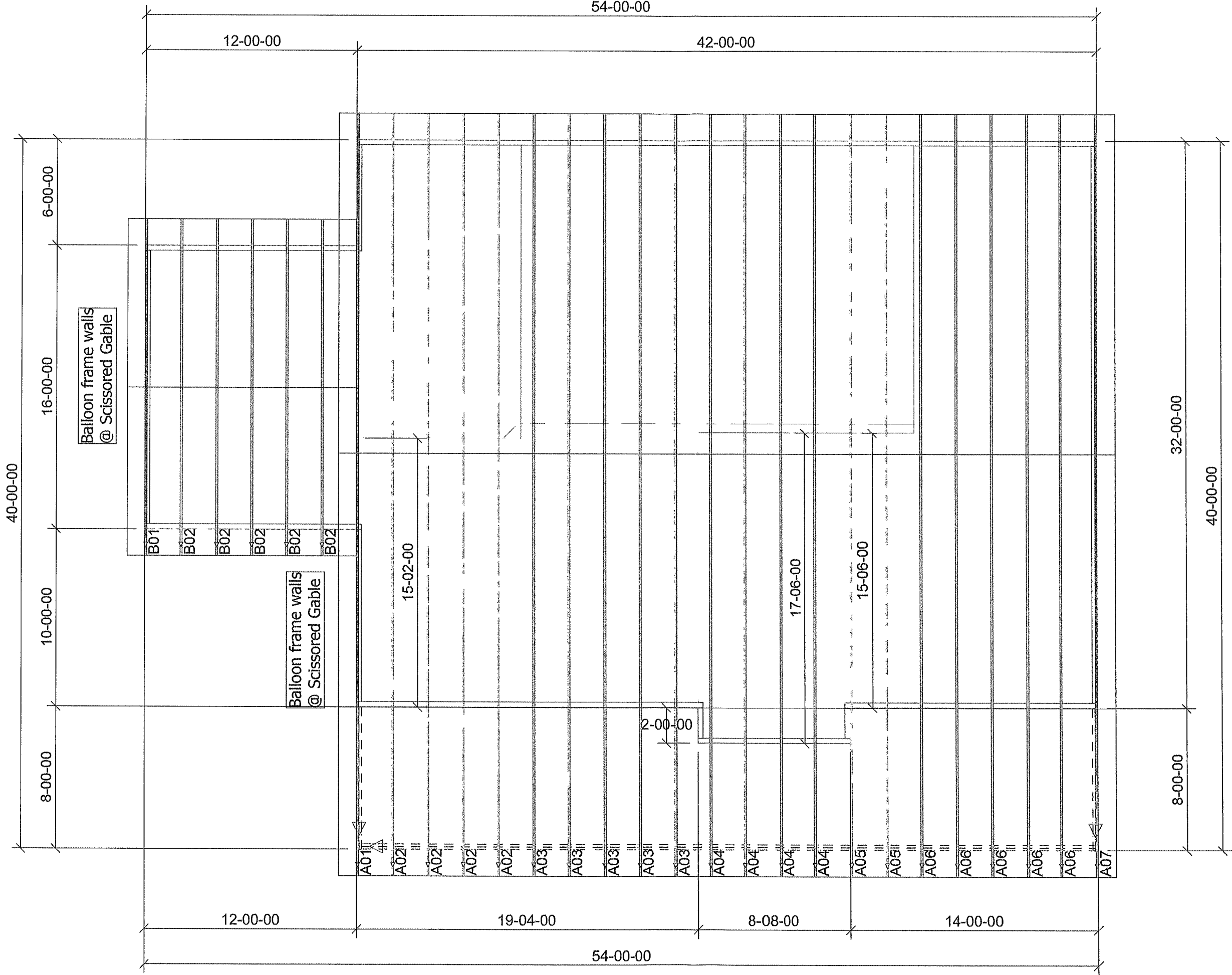
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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative for 1 bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified
13. Top chords must be sheathed or purlins provided at spacing indicated on design
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated

MITek®



Mathis Johnny

Client: IND-RES
Date: 11/5/2025
Quote Date: / /
Seal Date: / /
Designer:
Job Number: 1125-003