Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1STF487-Z0530165354

Truss Fabricator: Anderson Truss Company
Job Identification: 5-547-MIKE BENNETT
Truss Count: 32
Model Code: Florida Building Code 2004
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software,Version 7.22.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 12/30/2005

-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB

#	Ref	Description	Drawing#	Date
1	27197	A1-GE	05363181	12/29/05
2	27198	A2	05363152	12/29/05
3	27199	A3	05363153	12/29/05
4	27200	A4	05363154	12/29/05
5	27201	A5G	05363155	12/29/05
6	27202	A6	05363156	12/29/05
7	27203	A7	05363157	12/29/05
8	27204	A8	05363158	12/29/05
9	27205	A9-GE	05363182	12/29/05
10	27206	B1	05363159	12/29/05
11	27207	B2	05363160	12/29/05
12	27208	B3	05363151	12/29/05
13	27209	B4G	05363161	12/29/05
14	27210	C1	05363162	12/29/05
15	27211	C2	05363163	12/29/05
16	27212	C3	05363164	12/29/05
17	27213	C4	05363165	12/29/05
18	27214	C5	05363166	12/29/05
19	27215	C6G	05363167	12/29/05
20	27216	FGA	05363168	12/29/05
21	27217	D1	05363169	12/29/05
22	27218	D2	05363170	12/29/05
23	27219	D3	05363180	12/29/05
24	27220	D4G	05363171	12/29/05
25	27221	HJ7	05363172	12/29/05
26	27222	EJ7	05363173	12/29/05
27	27223	J7	05363174	12/29/05
28	27224	J5	05363175	12/29/05
29	27225	J3	05363176	12/29/05
30	27226	J1	05363177	12/29/05
31	27227	HJD	05363178	12/29/05
32	27228	AP1	05363179	12/29/05

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
 Document ID:1STF487-Z0530165354

Truss Fabricator:	Anderson Truss Company
Job Identification:	5-547-MIKE BENNETT (5-547|-MIKE BENNETT)
Truss Count:	3
Model Code:	Florida Building Code 2004
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.22.
Structural Engineer of Record:	
Address:	
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration Floor - N/A Wind - 110 MPH ASCE 7-02 -Closed

Notes:

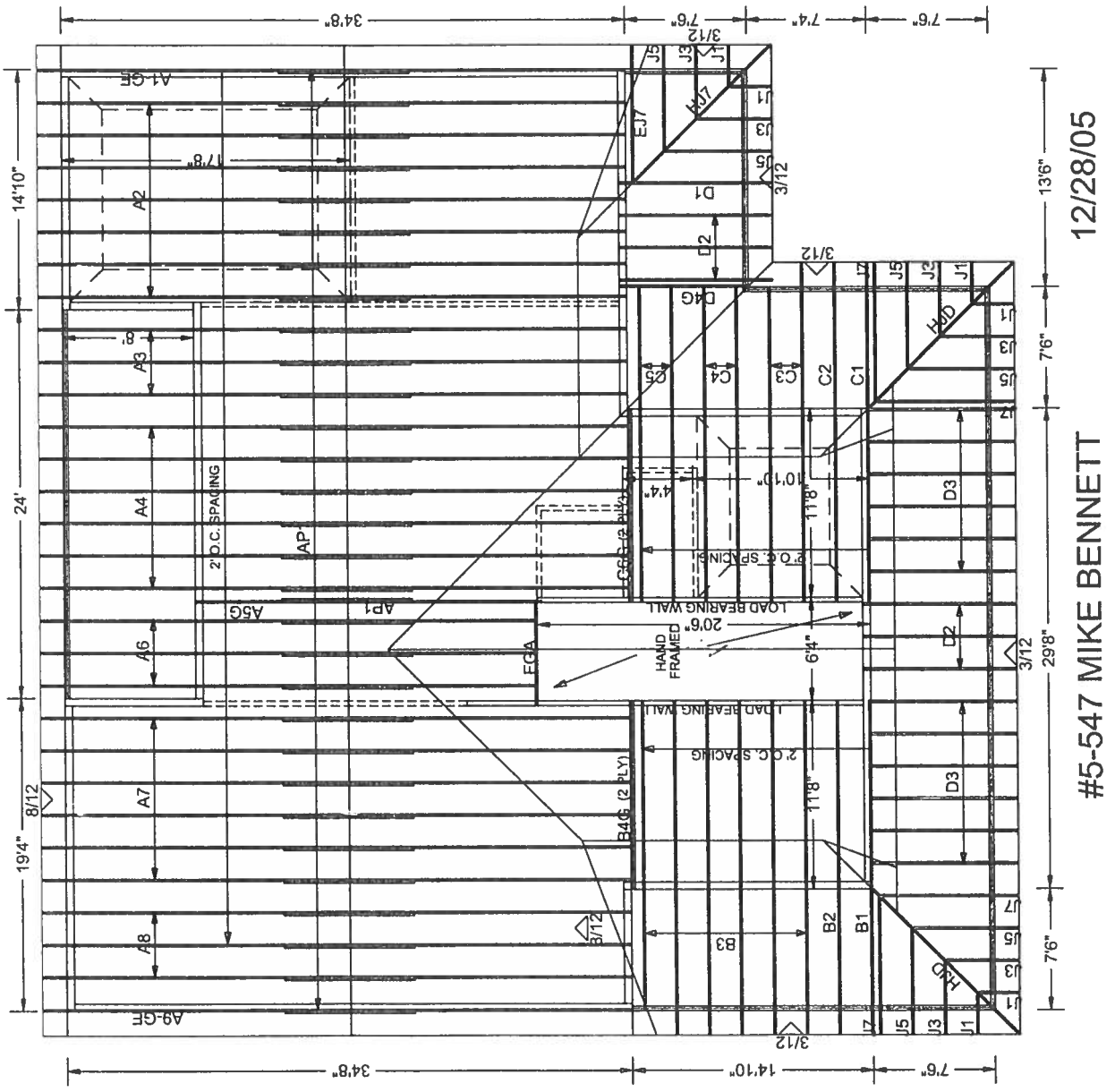
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Seal Date: 12/30/2005

-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

Revised Trusses

#	Ref	Description	Drawing#	Date
1	27197--A1-GE		05363181	12/29/05
2	27205--A9-GE		05363182	12/29/05
3	27215--C6G		05363167	12/29/05



#5-547 MIKE BENNETT

12/28/05

Scale: 3/32" = 1'

352-494-7838

(5-547 MIKE BENNETT - A1-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense :B1 2x6 SP #2:
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

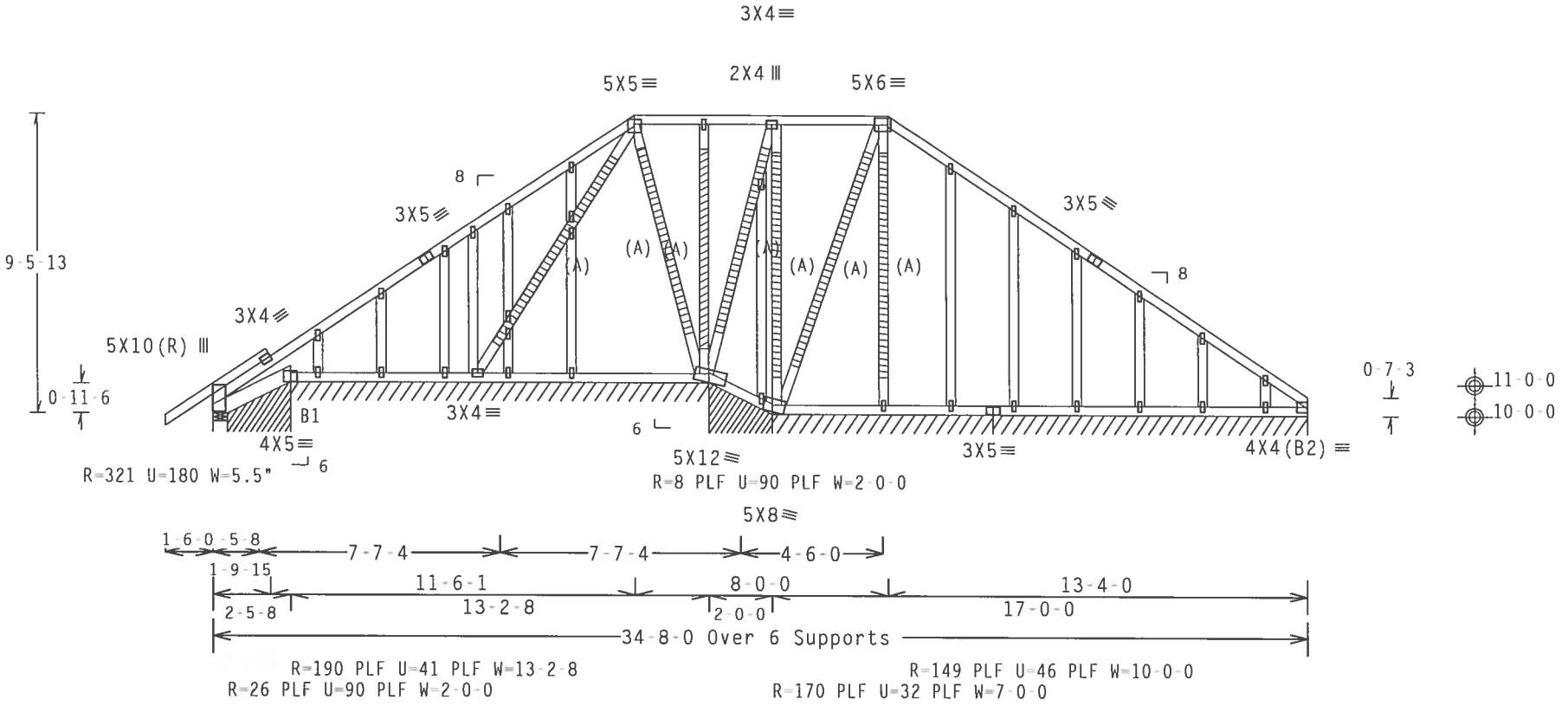
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Shim all supports to solid bearing.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

(A) SP #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

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WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27197
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363181
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 71284 REV
DUR.FAC.	1.25	

(5 547 MIKE BENNETT A2)

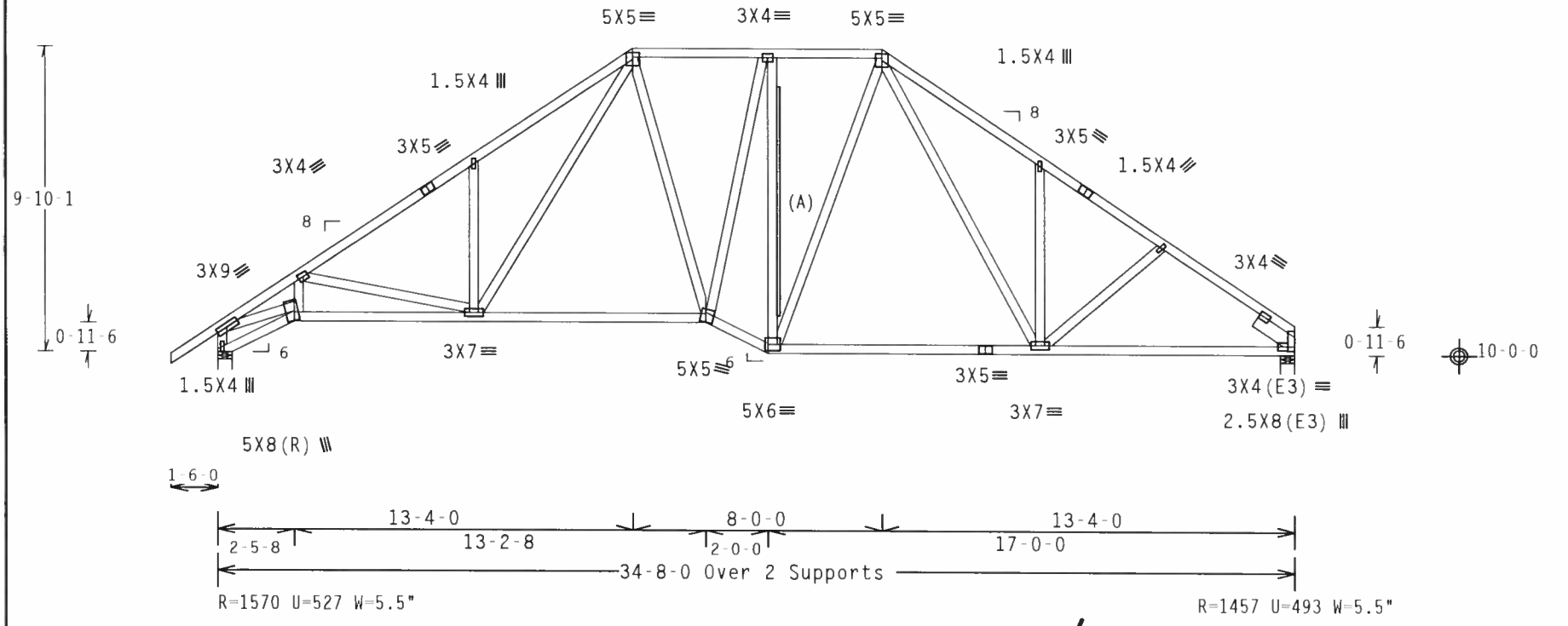
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

(A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 [illegible] TY:7 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 55687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27198
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363152
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21443
DUR.FAC.	1.25	

(5 547 MIKE BENNETT A3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

(C) 2x4 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

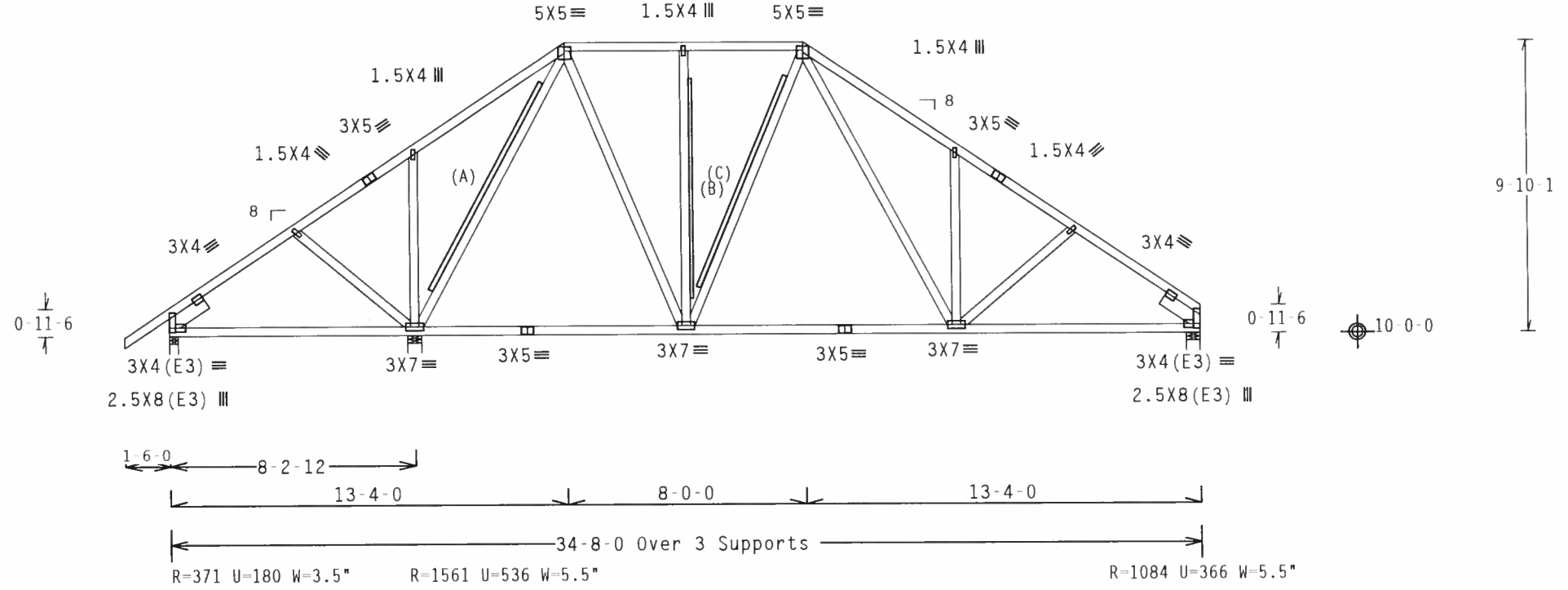
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

(B) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(A) 2x6 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:3 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27199
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363153
BC LL	0.0 PSF		HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21363
DUR.FAC.	1.25		

(5-547-MIKE BENNETT - A4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

(B) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(A) Continuous lateral bracing equally spaced on member. Or 2x6 SP #3 or better "T" brace. 80% length of web member. Attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

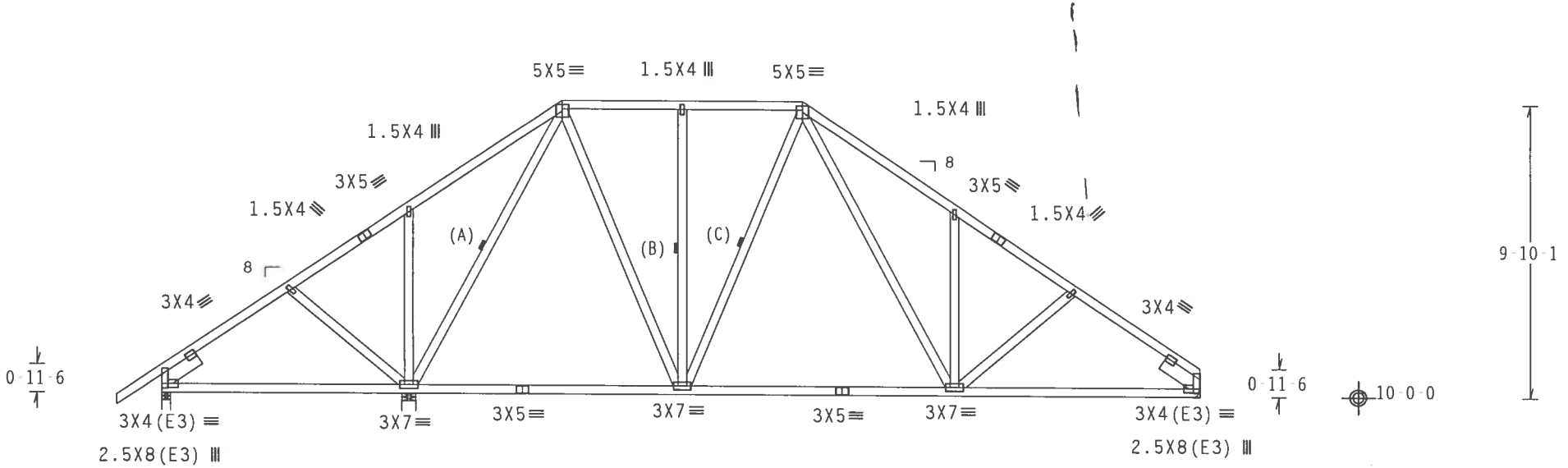
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

H - recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(C) Continuous lateral bracing equally spaced on member. Or 2x4 SP #3 or better "T" brace. 80% length of web member. Attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



R=371 U=180 W=3.5" R=1561 U=536 W=5.5"

R=1084 U=366 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:6 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27200
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363154
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21355
DUR.FAC.	1.25	

(5 547 MIKE BENNETT - A5G)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #2:
Webs 2x4 SP #3

110 mph wind, 17.18 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

(A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

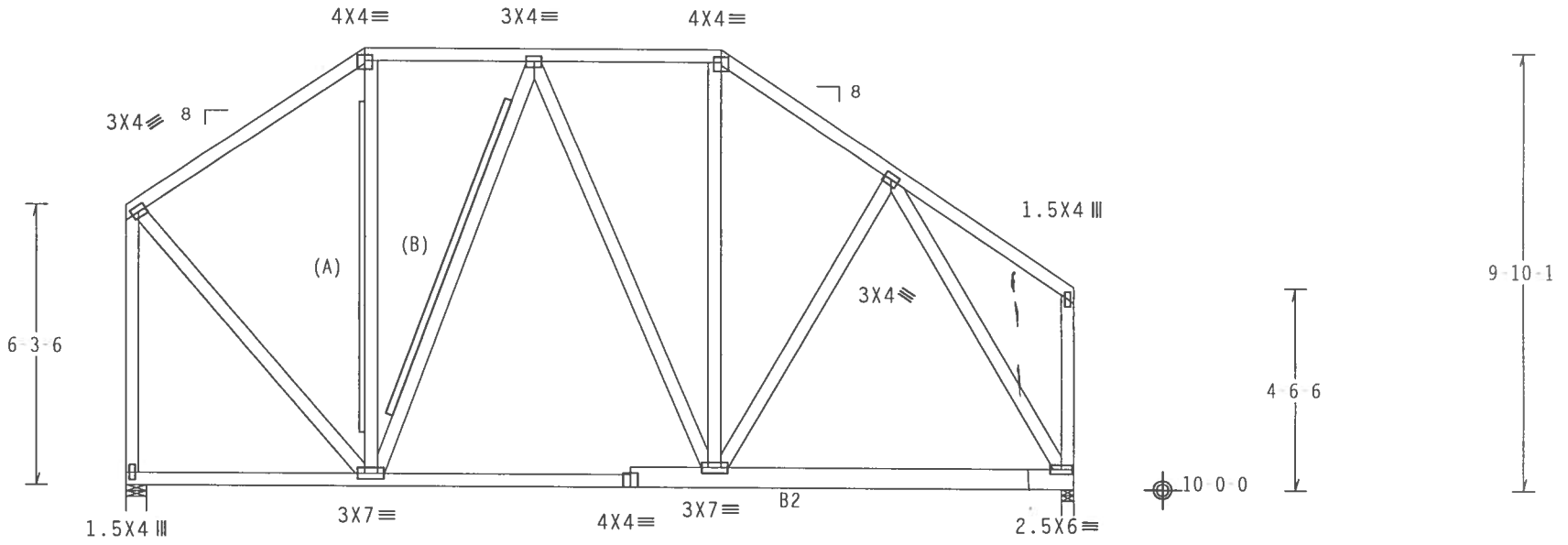
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

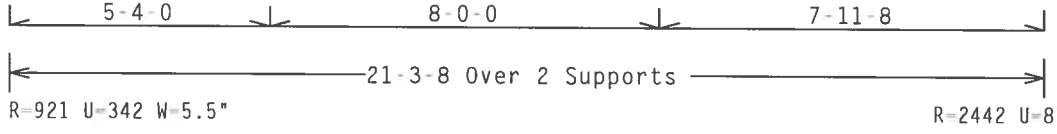
SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at 0.00 to 64 PLF at 21.29
BC - From 20 PLF at 0.00 to 20 PLF at 21.29
BC - 1573 LB Conc. Load at 20.94

End verticals not exposed to wind pressure.

(B) 2x4 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.25(1.25)/0(0) 7.22.1 [illegible] KEY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27201
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363155
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 71299
DUR.FAC.	1.25	

(5 547 MIKE BENNETT - A6)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbed Wedge 2x8 SP SS:

H - recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(A) Continuous lateral bracing equally spaced on member. Or 2x6 SP #3 or better "T" brace. 80% length of web member. Attached with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

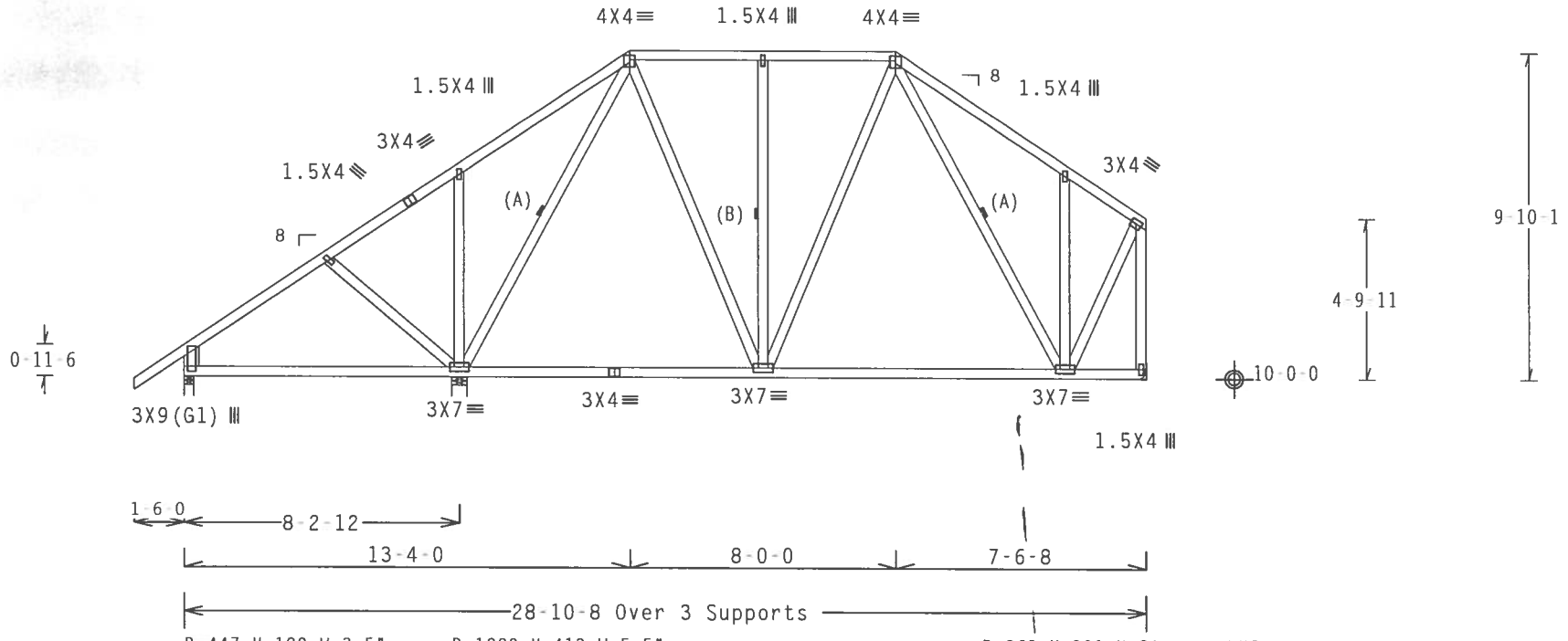
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

(B) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.11 TY:3 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No. 53687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27202
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363156
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21418
DUR.FAC.	1.25		

(J 947 MIKE BENNETT - A7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

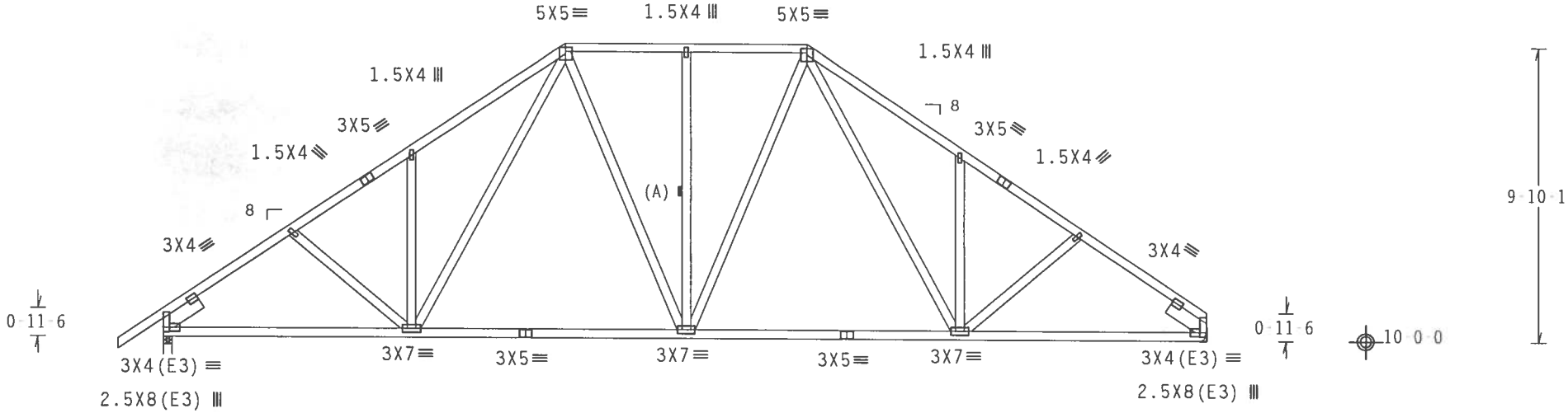
(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

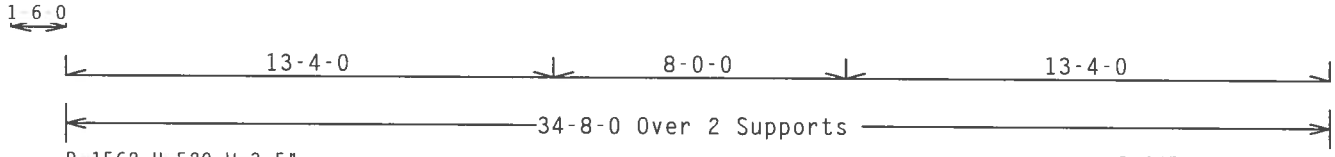
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.





R=1562 U=529 W=3.5"

R=1454 U=493 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
...der is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.25(1.25)/0(0) 7.22.1 ...TY:6 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27203
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363157
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21345
DUR.FAC.	1.25	

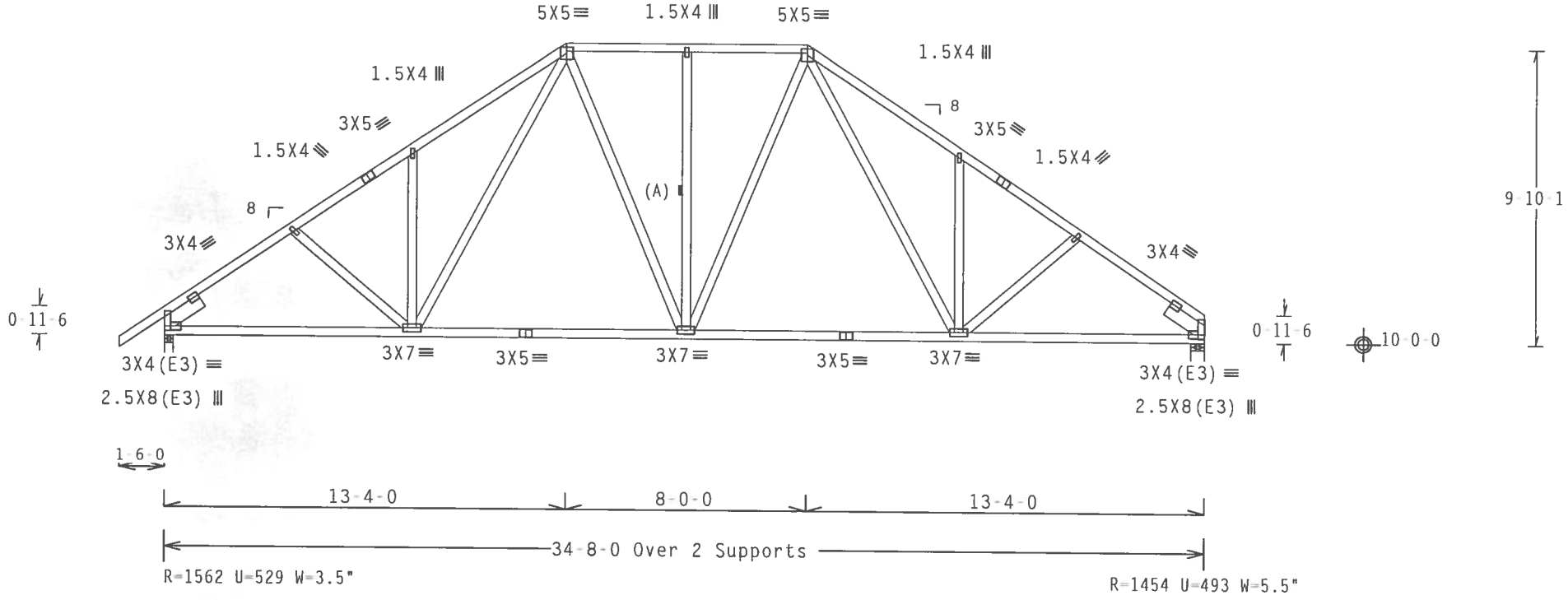
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

(A) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3 or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.11 ...TY:3 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27204
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363158
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21370
DUR.FAC.	1.25		

(5 547 MIKE BENNETT - A9 GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

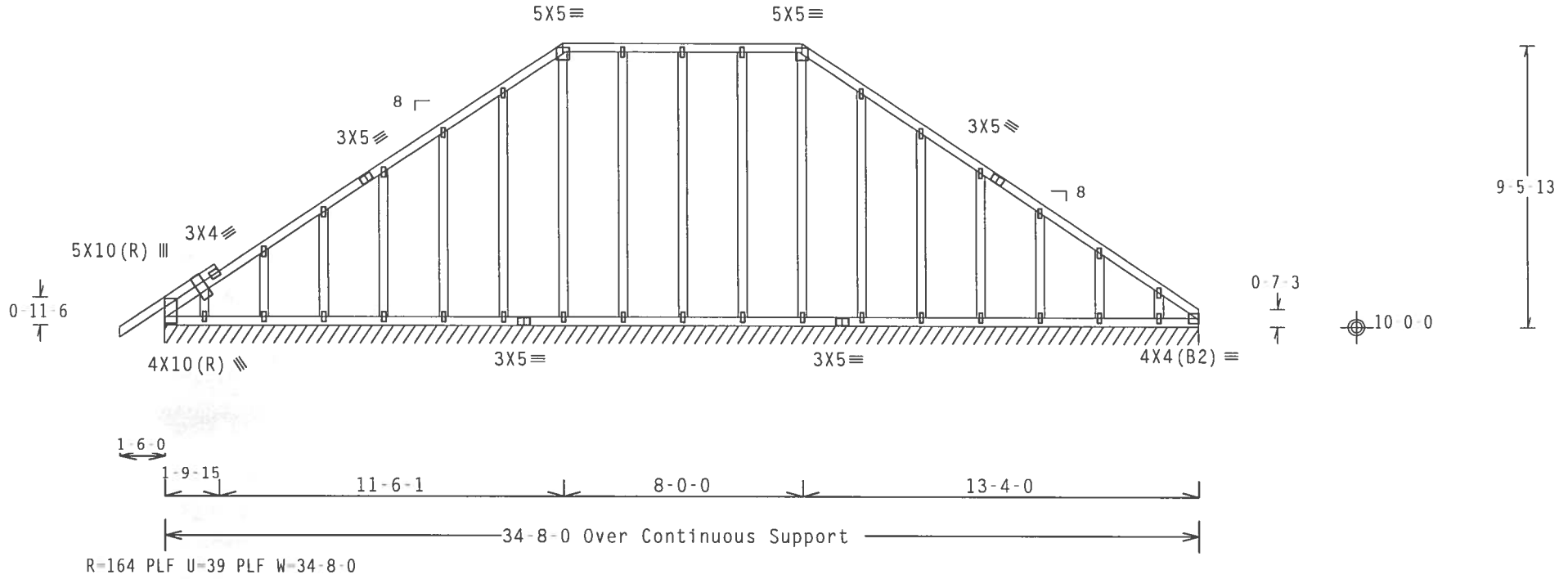
Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27205
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363182
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21384 REV
DUR.FAC.	1.25		

(5 547 MIKE BENNETT 81)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	61 PLF	at	1.50	to	61 PLF at 7.50
TC	From	30 PLF	at	7.50	to	30 PLF at 9.90
TC	From	51 PLF	at	9.90	to	51 PLF at 19.17
BC	From	4 PLF	at	1.50	to	4 PLF at 0.00
BC	From	20 PLF	at	0.00	to	20 PLF at 7.50
BC	From	151 PLF	at	7.50	to	151 PLF at 19.17
BC		569 LB Conc. Load at 7.50				

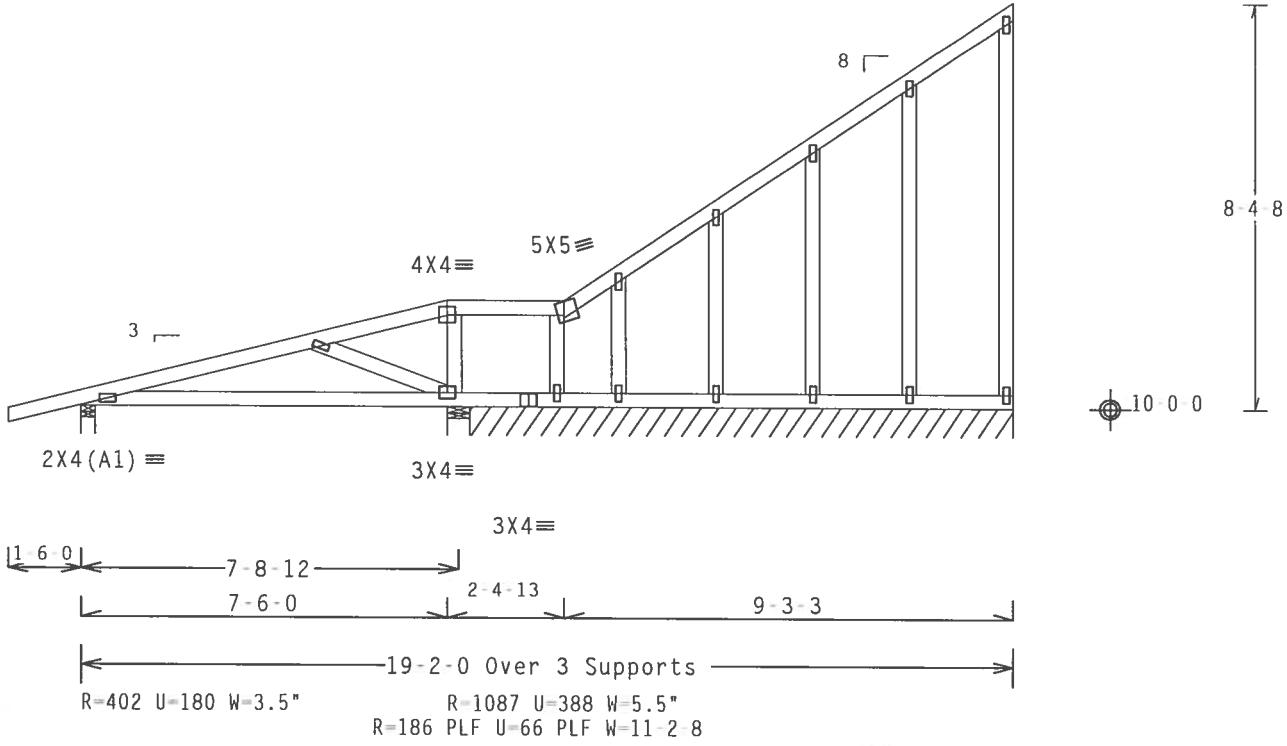
Refer to drawing GE 003 or HC26294035 for stud verticals and bracing details. Gable end has been designed to transfer all horizontal wind loads into the roof and ceiling diaphragms. Connection and design of these systems are the responsibility of the building designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22. QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0	PSF	REF R487-- 27206
TC DL	10.0	PSF	DATE 12/29/05
BC DL	10.0	PSF	DRW HCUSR487 05363159
BC LL	0.0	PSF	HC-ENG DAL/AF
TOT.LD.	40.0	PSF	SEQN- 21155
DUR.FAC.	1.25		

(5 547 MIKE BENNETT - B2)

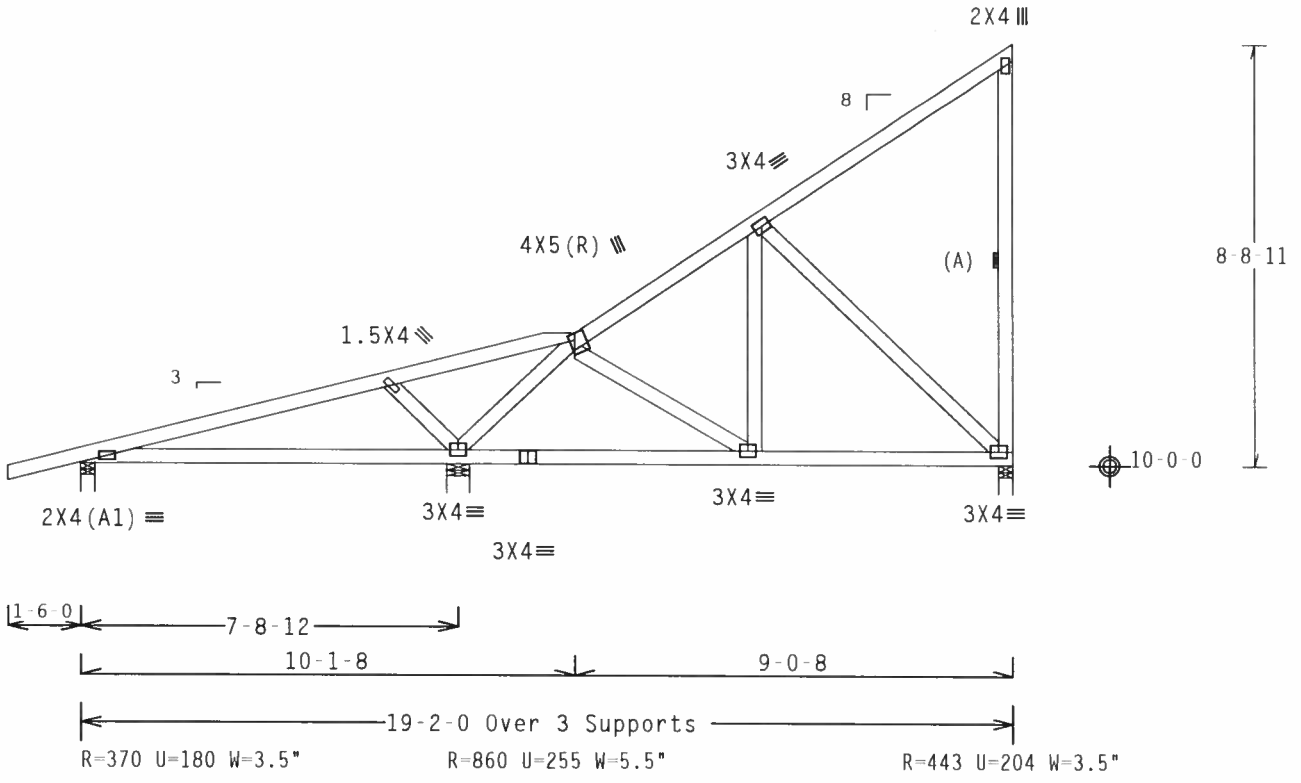
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0)

PLT TYP. Wave

7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27207
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363160
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21189
DUR.FAC.	1.25	

(5 547 MIKE BENNETT B3)

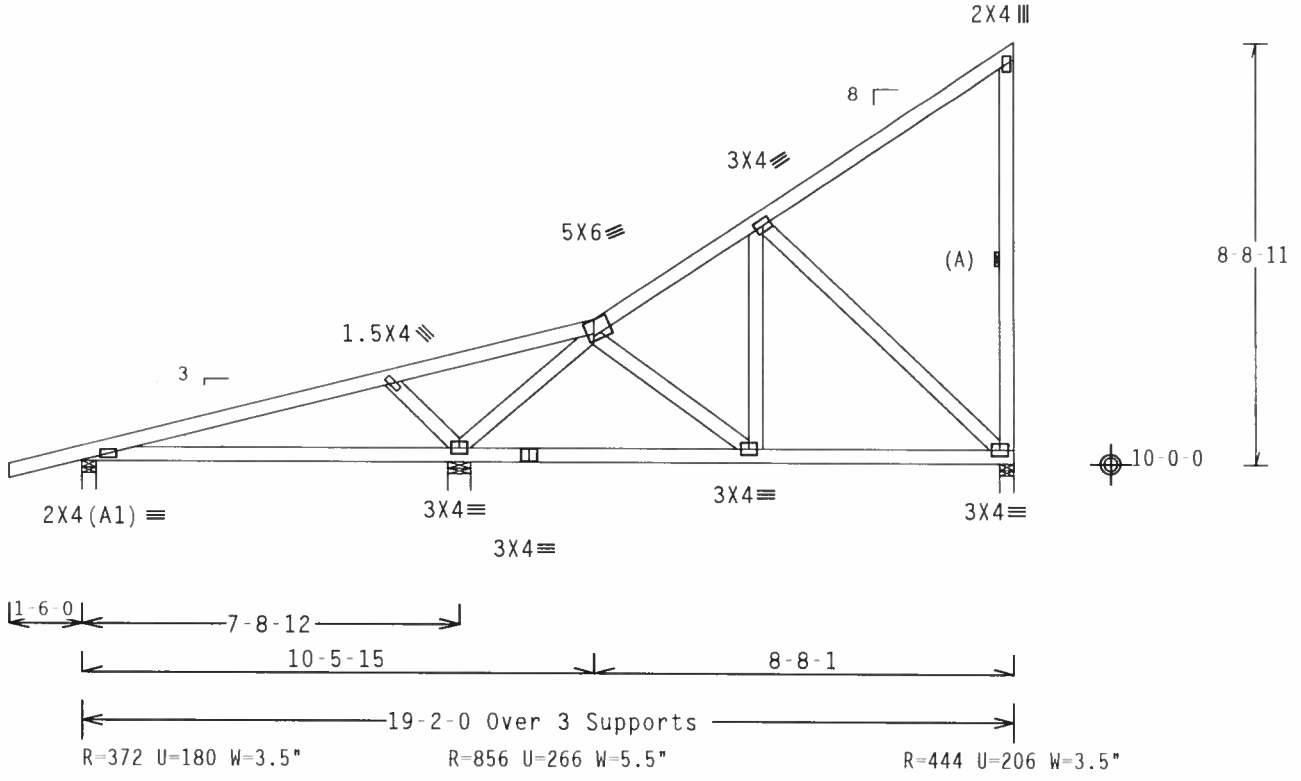
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:6 FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

ALPINE

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TC LL	20.0 PSF	REF	R487-- 27208
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363151
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21174
DUR.FAC.	1.25		

(5 547 MIKE BENNETT - B4G)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

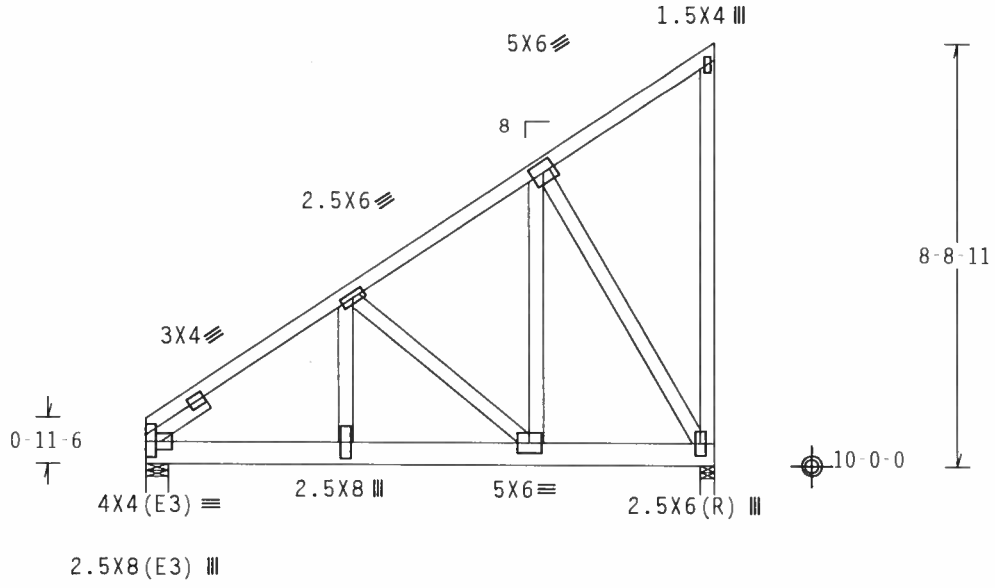
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d Common (0.148"x3", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 5.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

Girder supports 34-8-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.



11-8-0 Over 2 Supports

R=4458 U=1515 W=5.5" R=4458 U=1515 W=3.5"

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 [illegible] FL/-/4/-/-/R/- Scale =.25"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W, K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY

TC LL	20.0 PSF	REF R487-- 27209
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363161
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21295
DUR.FAC.	1.25	

(5 547-MIKE BENNETT C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 61 PLF at -1.50 to 61 PLF at 7.50
TC - From 30 PLF at 7.50 to 30 PLF at 9.90
TC - From 51 PLF at 9.90 to 51 PLF at 19.46
BC - From 4 PLF at -1.50 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 7.50
BC - From 151 PLF at 7.50 to 151 PLF at 19.46
BC - 569 LB Conc. Load at 7.50

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

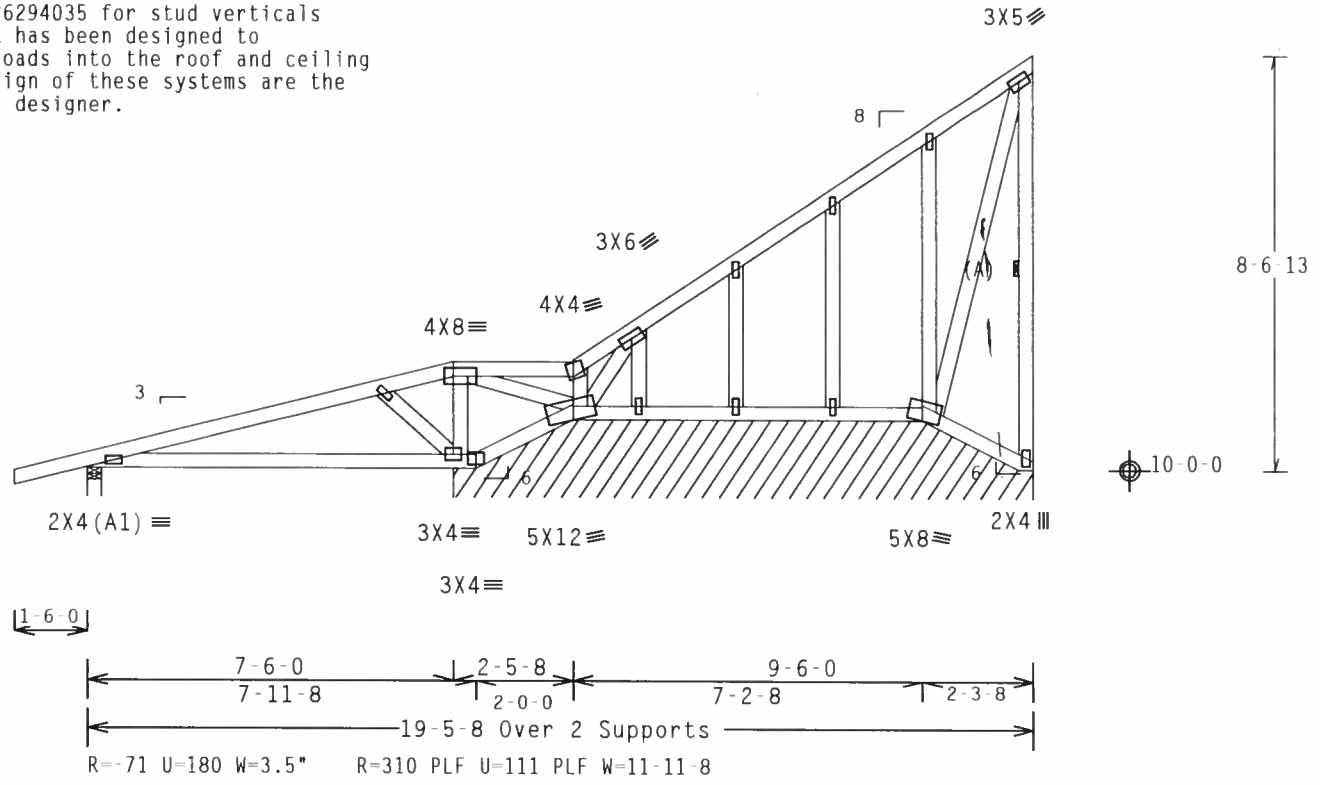
Refer to drawing GE-003 or HC26294035 for stud verticals and bracing details. Gable end has been designed to transfer all horizontal wind loads into the roof and ceiling diaphragms. Connection and design of these systems are the responsibility of the building designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22 ... TY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30, 05

TC LL	20.0 PSF	REF R487-- 27210
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363162
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21202
DUR.FAC.	1.25	

(5 547 MIKE BENNETT - C2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

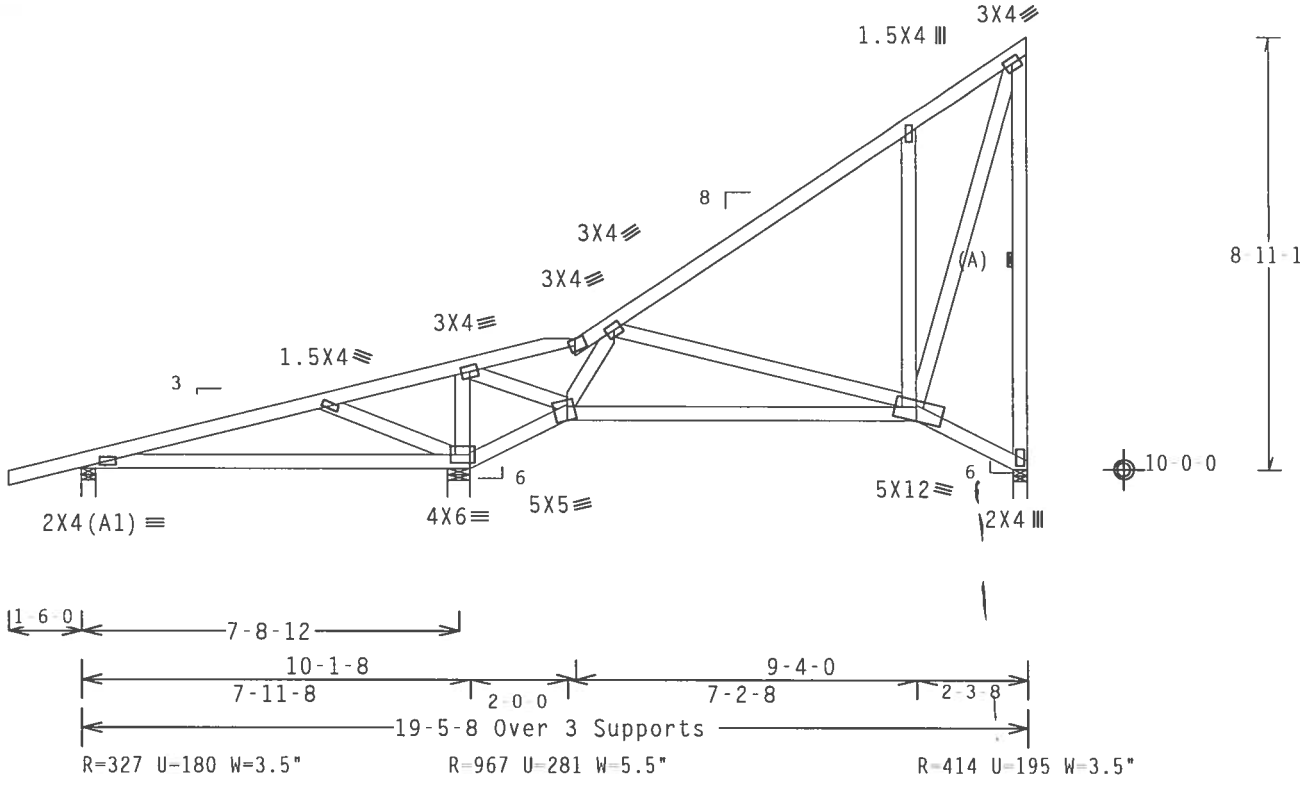
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

Shim all supports to solid bearing.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.1 KEY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

PLT TYP. Wave

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30,05

TC LL	20.0 PSF	REF R487-- 27211
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363163
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21237
DUR.FAC.	1.25	

(5 547 MIKE BENNETT - C3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

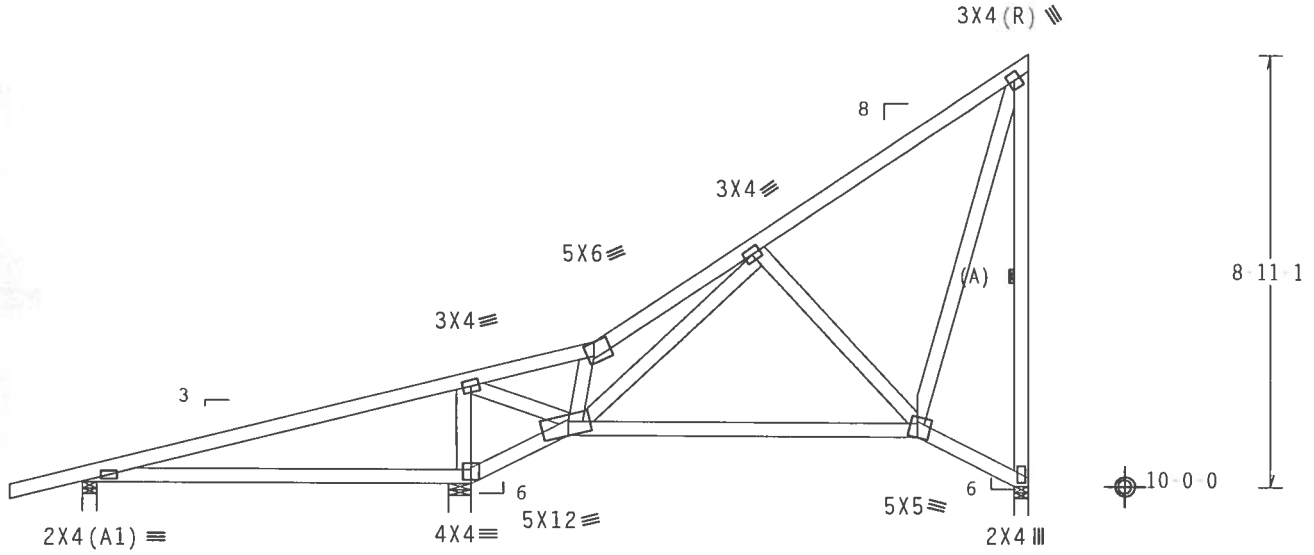
(A) Continuous lateral bracing equally spaced on member.

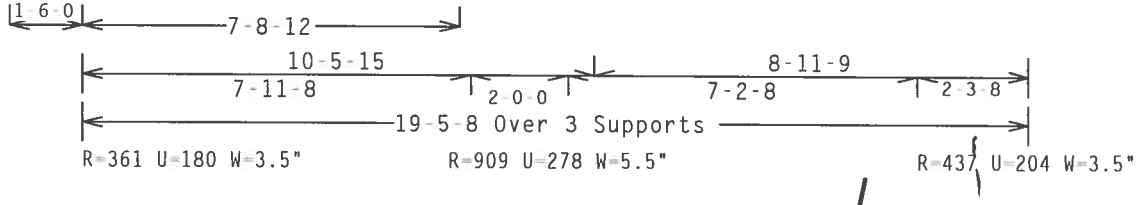
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

Shim all supports to solid bearing.





Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.1

PLT TYP. Wave

TY:2 FL/-/4/-/-/R/- Scale =.25"/Ft.

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1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27212
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363164
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21250
DUR.FAC.	1.25	

(5 547 MIKE BENNETT C4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

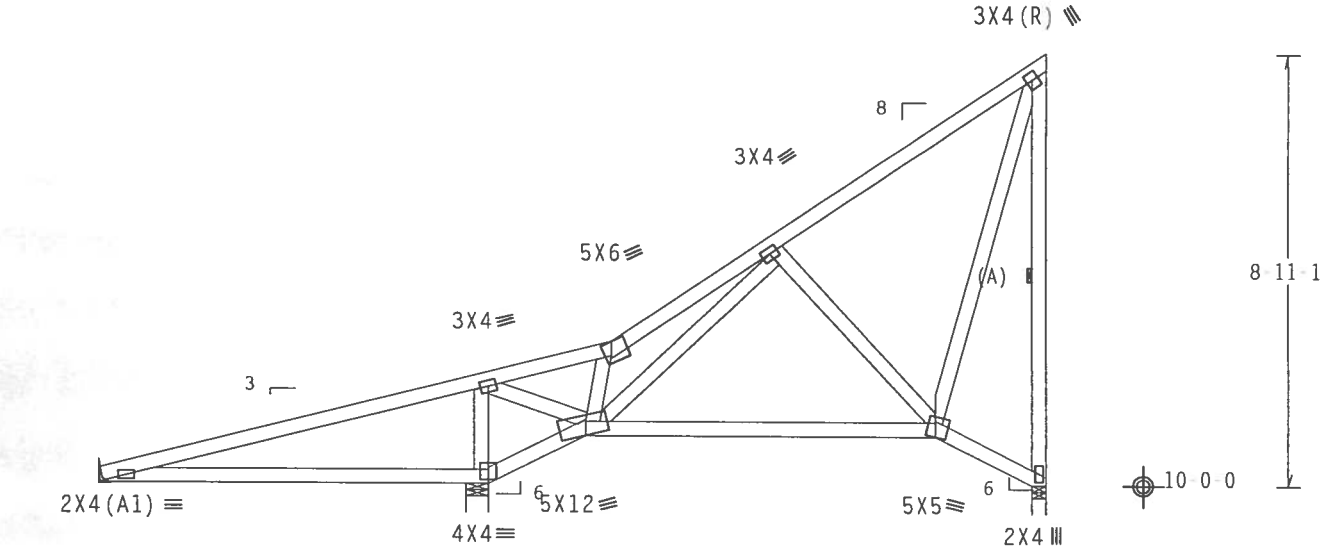
Shim all supports to solid bearing.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=253 U=180 H=Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R=918 U=281 W=5.5"

R=438 U=204 W=3.5"

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.11 D: QTY:2 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 20 '05

TC LL	20.0 PSF	REF	R487-- 27213
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363165
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21257
DUR.FAC.	1.25		

(3 947 MIKE BENNETT C5)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

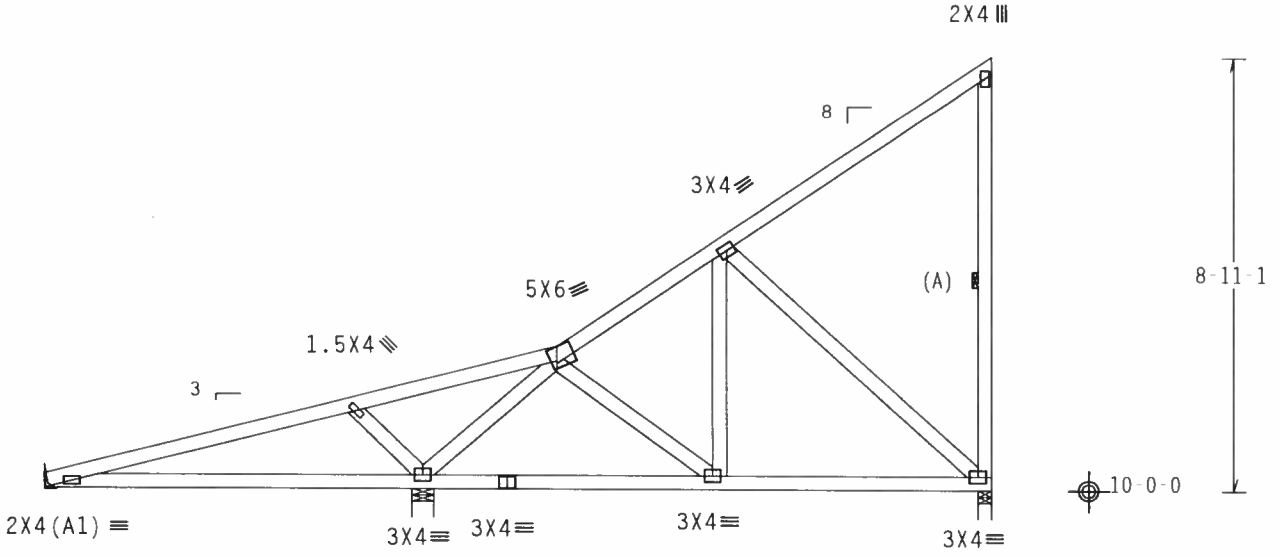
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:2 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER LICENSE No.39687 STATE OF FLORIDA PROFESSIONAL ENGINEER

DEC 30 '05

TC LL	20.0 PSF	REF R487-- 27214
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363166
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21271
DUR.FAC.	1.25	

(5 547 MIKE BENNETT C6G)

Top chord 2x4 SP #2
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

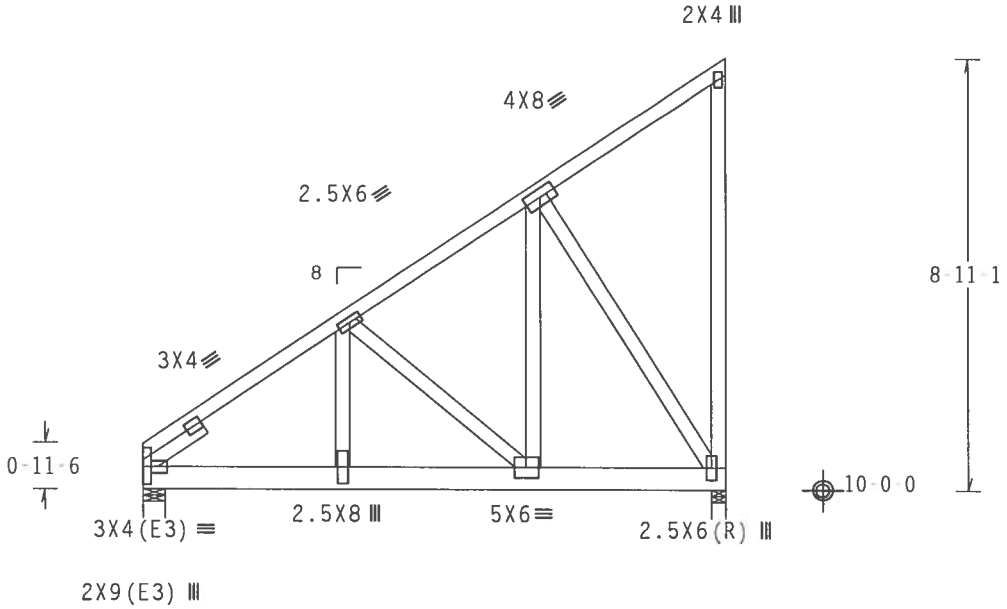
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d Common (0.148"x3", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 7.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

Girder supports 26-8-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.



11-11-8 Over 2 Supports

R=3564 U=1211 W=5.5" R=3564 U=1211 W=3.5"

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27215
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363167
BC LL	0.0 PSF	HC-ENG TCE/AF
TOT.LD.	40.0 PSF	SEQN- 71694 REV
DUR.FAC.	1.25	

(5-547-MIKE BENNETT - FGA)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

THE ARCHITECT OR ENGINEER OF RECORD SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS".)

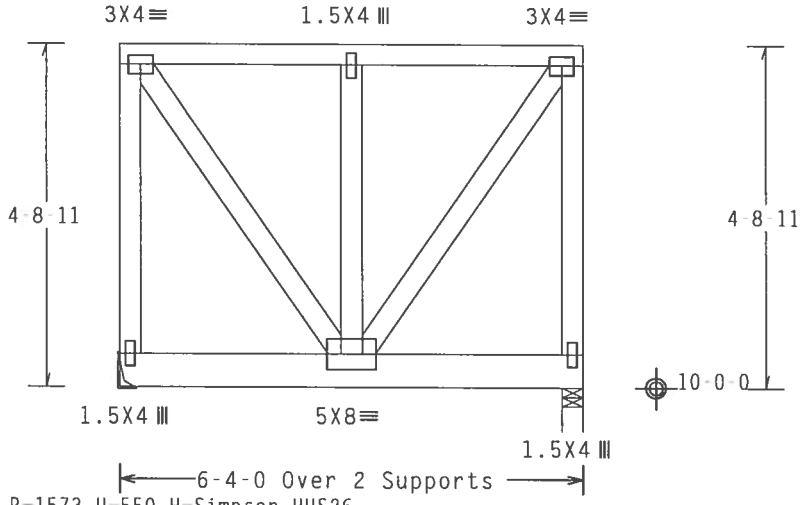
SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at 0.00 to 60 PLF at 6.33
BC - From 20 PLF at 0.00 to 20 PLF at 6.33
BC - 863 LB Conc. Load at 1.10, 3.10, 5.10

End verticals not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

NOT DESIGNED FOR ANY LOAD FROM HAND FRAMING



R=1573 U=550 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R=1522 U=532 W=3.5"

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF
Florida
PROFESSIONAL ENGINEER
Dec 3[illegible] '05

TC LL	20.0 PSF	REF R487-- 27216
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363168
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21427
DUR.FAC.	1.25	

(5-547-MIKE BENNETT - D1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

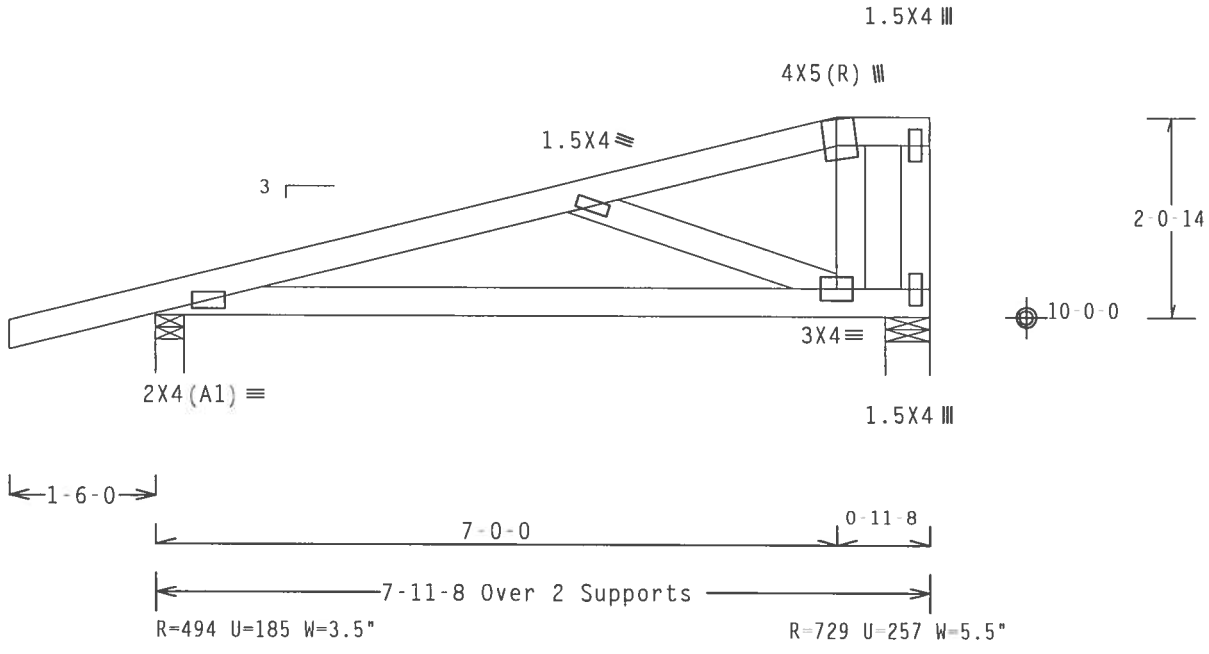
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27217
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363169
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21144
DUR.FAC.	1.25	

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

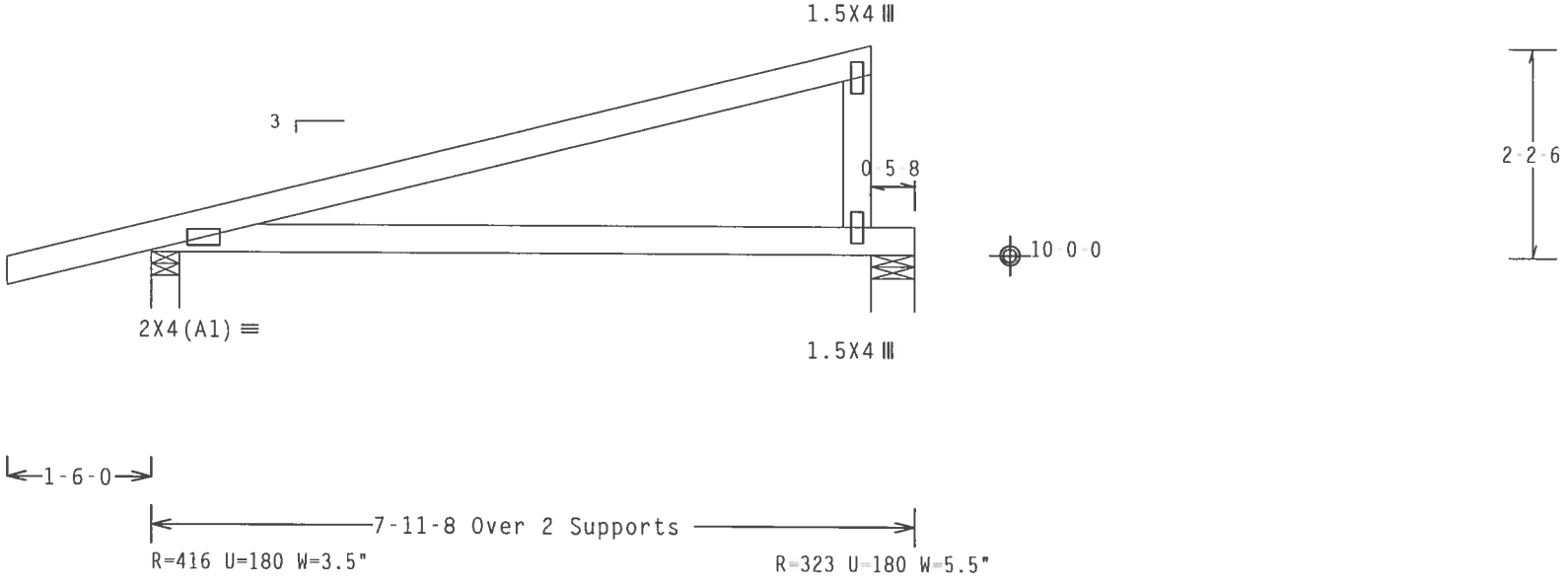
(5 547 MIKE BENNETT - D2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 TY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec 30 '05

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

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TC LL	20.0 PSF	REF R487-- 27218
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363170
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21071
DUR.FAC.	1.25	

(5 547 MIKE BENNETT - 03)

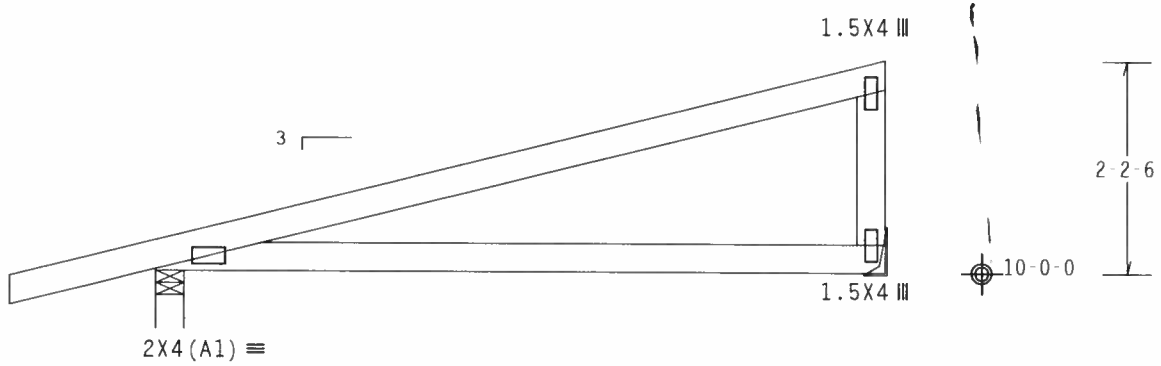
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0

7-6-0 Over 2 Supports

R=417 U=180 W=3.5"

R=284 U=180 H=Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder: 2-2x4 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:12 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No.59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Dec 30 05

TC LL	20.0 PSF	REF R487-- 27219
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363180
BC LL	0.0 PSF	HC-ENG DAL/AF *
TOT.LD.	40.0 PSF	SEQN- 21064
DUR.FAC.	1.25	

(J 947-MIKE BENNETT - 04G)

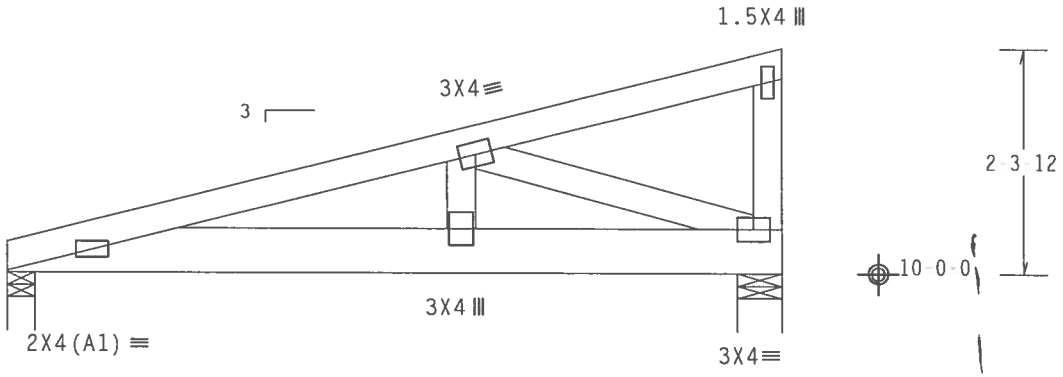
Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 61 PLF at 0.00 to 61 PLF at 7.96
BC - From 20 PLF at 0.00 to 20 PLF at 7.96
BC - 253 LB Conc. Load at 0.73, 2.73
BC - 263 LB Conc. Load at 4.73, 6.73

Right end vertical not exposed to wind pressure.



7-11-8 Over 2 Supports

R=881 U=275 W=3.5"

R=793 U=244 W=5.5"

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27220
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363171
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21395
DUR.FAC.	1.25		

(5 547 MIKE BENNETT - HJ7)

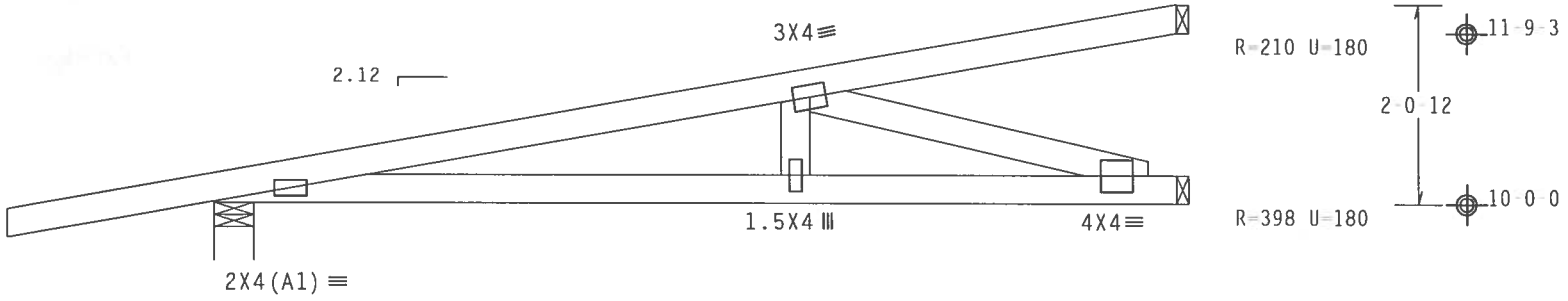
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

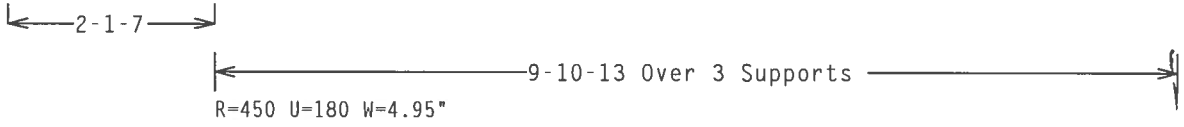
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27221
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363172
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21124
DUR.FAC.	1.25	

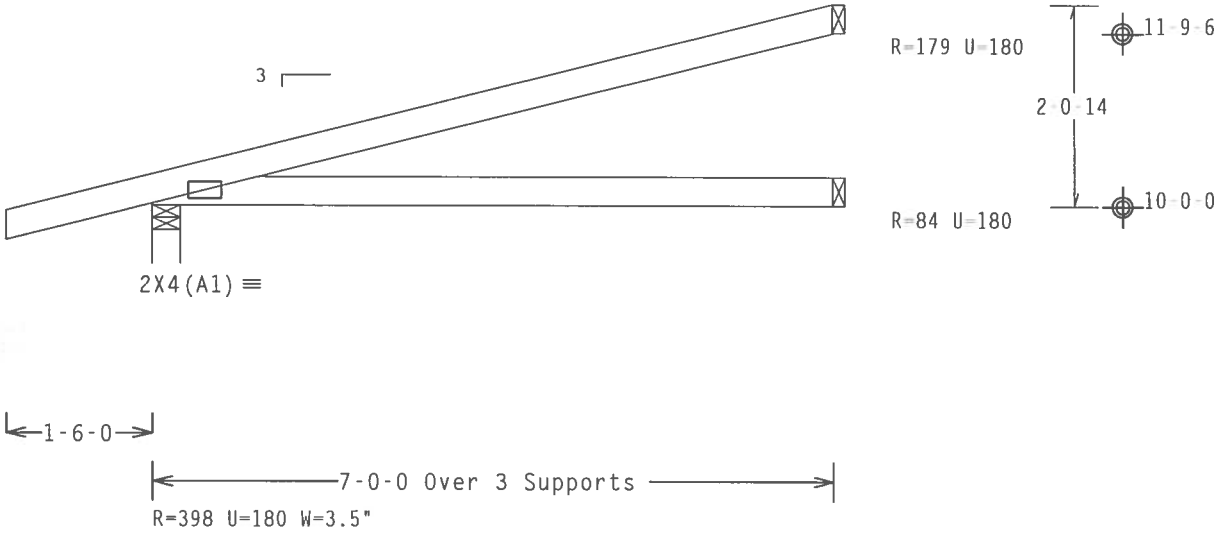
(5 547 MIKE BENNETT - EJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
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1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27222
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363173
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21076
DUR.FAC.	1.25	

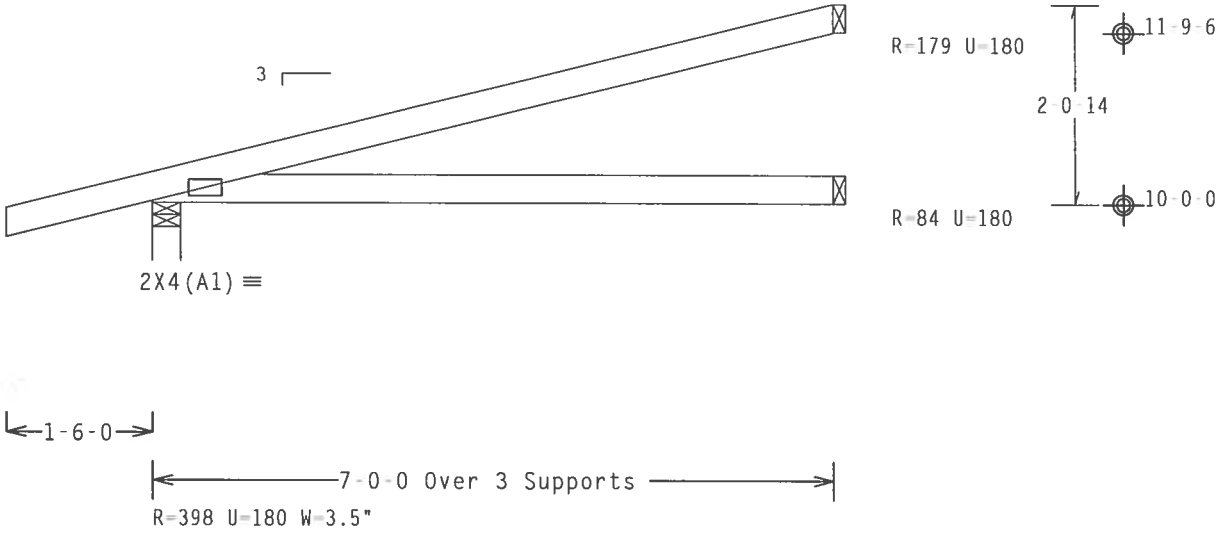
(5 547 MIKE BENNETT J7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 TY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27223
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363174
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21074
DUR.FAC.	1.25	

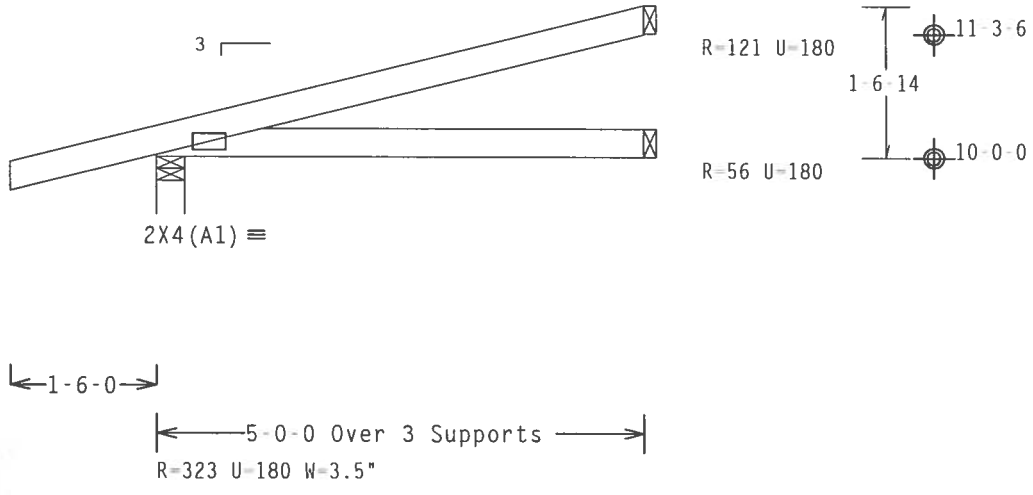
(5 547 MIKE BENNETT J5)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No.59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27224
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363175
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21054
DUR.FAC.	1.25	

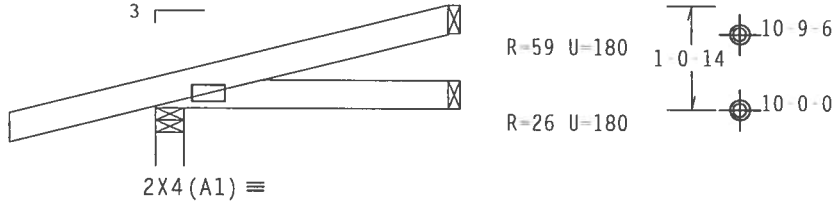
(5 547-MIKE BENNETT J3)

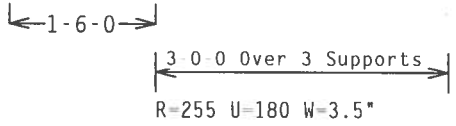
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.1 [illegible] TY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No. 59987
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF R487-- 27225
TC DL	10.0 PSF	DATE 12/29/05
BC DL	10.0 PSF	DRW HCUSR487 05363176
BC LL	0.0 PSF	HC-ENG DAL/AF
TOT.LD.	40.0 PSF	SEQN- 21058
DUR.FAC.	1.25	

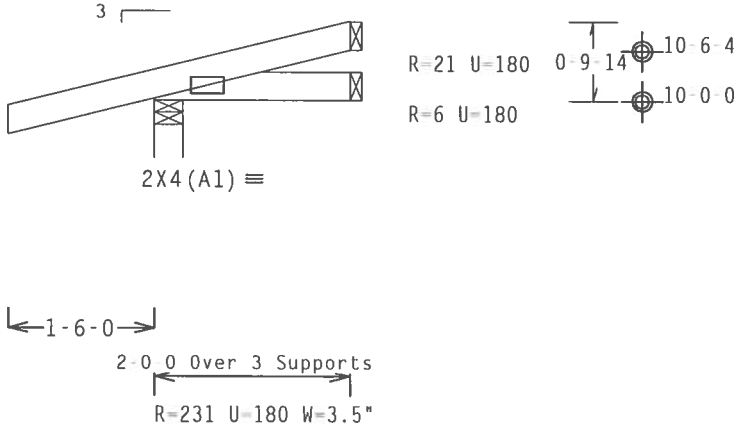
(5 547 MIKE BENNETT - J1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

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ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

ARTHUR B. FISHER LICENSE No.59887 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27226
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363177
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21132
DUR.FAC.	1.25		

(5-547-MIKE BENNETT - HJD)

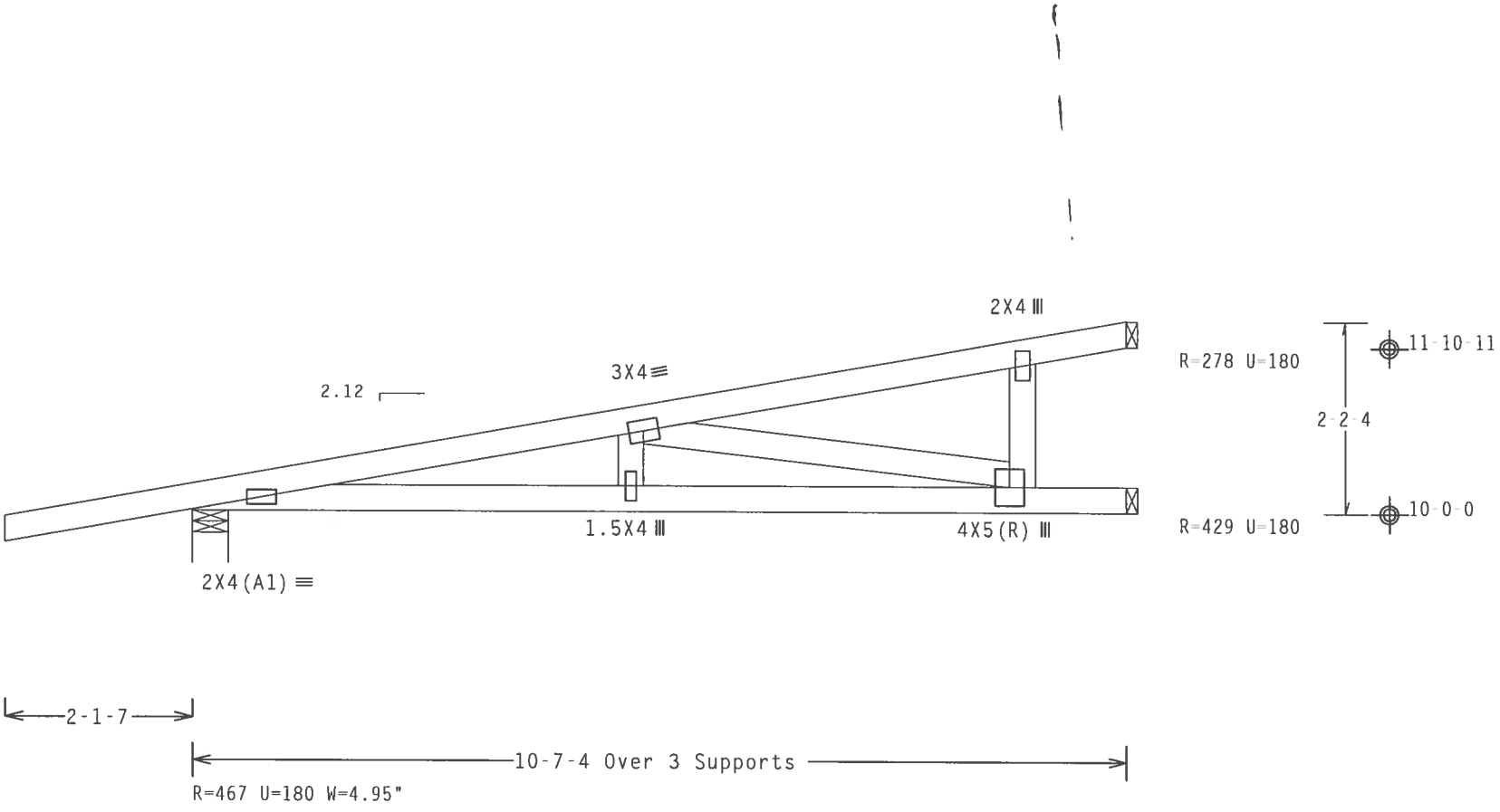
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.

Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
PROVIDE HANGER OR SPECIAL CONNECTION AT BOTTOM CHORD.

Hipjack supports 7-6-0 setback jacks. Jacks up to 7' have no webs. Longer jacks supported to BC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.25(1.25)/0(0) 7.22.1 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27227
TC DL	10.0 PSF	DATE	12/29/05
BC DL	10.0 PSF	DRW	HCUSR487 05363178
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	40.0 PSF	SEQN-	21116
DUR.FAC.	1.25		

(5 547 MIKE BENNETT AP1)

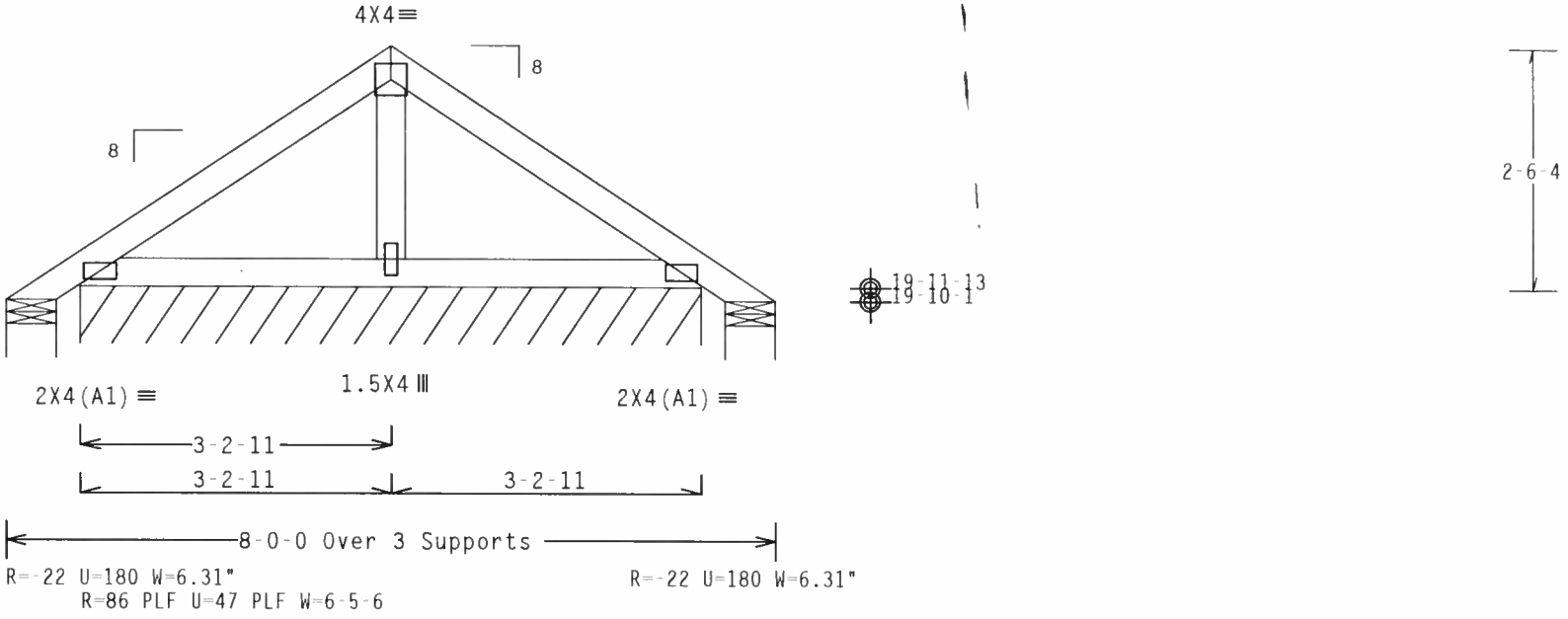
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA1103 or PIGBACKB0204 for piggyback details.

PORTION OF TRUSS UNDER PIGGYBACK IS TO BE BRACED @ 24" OC, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 21.17 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=0.0 psf, wind BC DL=0.0 psf.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.25(1.25)/0(0) 7.22.11 QTY:31 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

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ARTHUR R. FISHER LICENSE No. 59687 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec 30 '05

TC LL	20.0 PSF	REF	R487-- 27228
TC DL	10.0 PSF	DATE	12/29/05
BC DL	2.0 PSF	DRW	HCUSR487 05363179
BC LL	0.0 PSF	HC-ENG	DAL/AF
TOT.LD.	32.0 PSF	SEQN-	21284
DUR.FAC.	1.25		

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

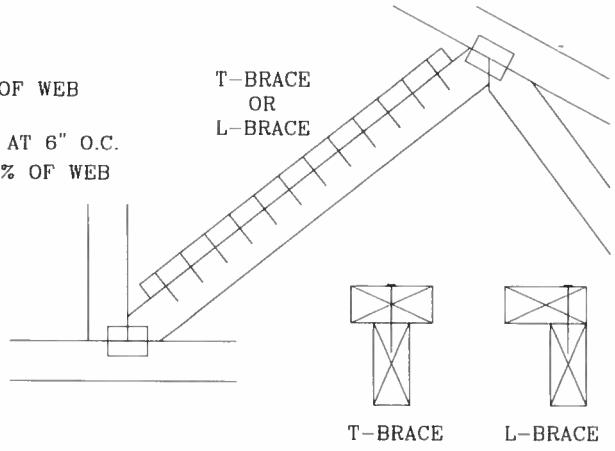
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

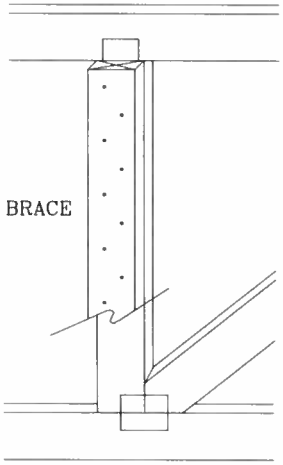
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE
ATTACH WITH 16d NAILS AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d OR .128"x3" GUN NAILS AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

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ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF FLORIDA
PROFESSIONAL ENGINEER

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCLBSUB1103
BC LL	PSF	ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			