COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A
THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 7/1/18

	Website: http://www.columbiacountyfla.com/BuildingandZoning.asp GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Each B Cir	cled as dicable	l be
1	Two (2) complete sets of plans containing the following:	0			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	0			
3	Condition space (Sq. Ft.) 1727 Total (Sq. Ft.) under roof 2 H 2 9	Ye	s	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

19

20

Number of stories

	ILC I HAR INIOI Mation including.	-		
4	Dimensions of lot or parcel of land	-	0	
5	Dimensions of all building set backs		0	
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed	_	6	
	well and septic tank and all utility easements.		0	
7	Provide a full legal description of property.	-	0	

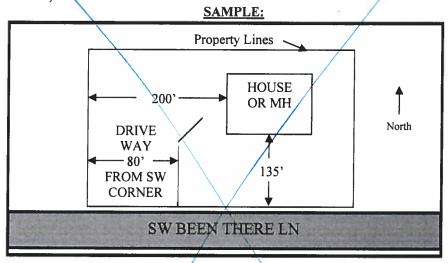
Wind-load Engineering Summary, calculations and any details are required.

Building height from the established grade to the roofs highest peak

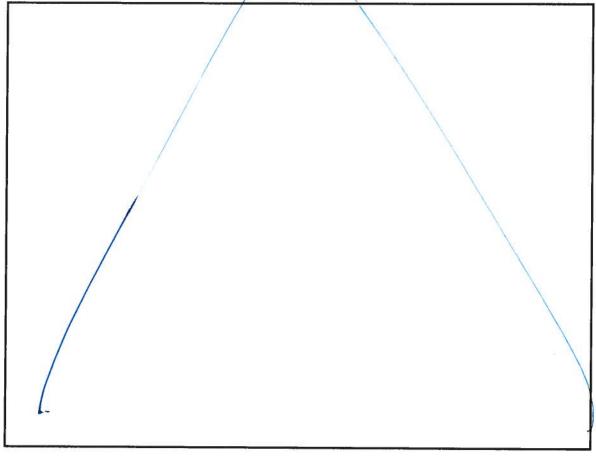
	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Each C	to Inclui Box shal ircled as plicable	
8	Plans or specifications must show compliance with FBCR Chapter 3	Ye	s	No	NA
		Sele	et Fro	m Drop	down
9	Basic wind speed (3-second gust), miles per hour	<u> </u>	0		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	-	0		
11	Wind importance factor and nature of occupancy	-	0		
12	The applicable internal pressure coefficient, Components and Cladding		8		
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	-	0		
Ele	evations Drawing including:				
14	All side views of the structure	-	0		
15	Roof pitch	-	Ü		
16	Overhang dimensions and detail with attic ventilation	-	U		
17	Location, size and height above roof of chimneys		0	-	C
18	Location and size of skylights with Florida Product Approval				
			7 3		- 1

Page 2, Site Plan for 9-1-1 Address Application From

- 1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
- 2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
- 3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
- 4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).



SITE PLAN BOX:



Floor Plan Including:

21 Dimensioned area plan showing rooms, attached gara deck, balconies 22 Raised floor surfaces located more than 30 inches abo 23 All exterior and interior shear walls indicated 24 Shear wall opening shown (Windows, Doors and Gara	ve the floor or grade	
 Raised floor surfaces located more than 30 inches abo All exterior and interior shear walls indicated Shear wall opening shown (Windows, Doors and Gara 	ve the floor or grade	
 All exterior and interior shear walls indicated Shear wall opening shown (Windows, Doors and Gara 		
24 Shear wall opening shown (Windows, Doors and Gara		
	age doors) - r	
25 Show compliance with Section FBCR 310 Emergence	cy escape and rescue opening shown in each	
bedroom (net clear opening shown) and Show complopening of an operable window is located more than 7 below, the lowest part of the clear opening of the window is inches shall be fixed or have openings through which is	2 inches above the finished grade or surface dow shall be a minimum of 24 inches above located. Glazing between the floor and 24	
26 Safety glazing of glass where needed	- 6	
Fireplaces types (gas appliance) (vented or non-vented (see chapter 10 and chapter 24 of FBCR)	l) or wood burning with Hearth	0
28 Show stairs with dimensions (width, tread and riser an	d total run) details of guardrails, Handrails	0
29 Identify accessibility of bathroom (see FBCR SECTIO	ON 320) - 6	

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS:
APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

	Circles Applica	
FBCR 403: Foundation Plans	Select From	Drop dowr
30 Location of all load-bearing walls footings indicated as standard, monolithic, dimensions and type of reinforcing.	s, size	
31 All posts and/or column footing including size and reinforcing	- 0	
32 Any special support required by soil analysis such as piling.	ļ -	ق
33 Assumed load-bearing valve of soil Pound Per Square Foot	-	0
Location of horizontal and vertical steel, for foundation or walls (include # size and type with foundation which establish new electrical utility companies service connection a C Encased Electrode will be required within the foundation to serve as an grounding electroper the National Electrical Code article 250.52.3	Concrete	
FBCR 506: CONCRETE SLAB ON GRADE		
35 Show Vapor retarder (6mil. Polyethylene with 'pints la pap 6 inches and sealed)	- 0	
36 Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement ar	nd Speprts - O	
FBCR 318: PROTECTION AGAINST TERMITES		
Indicate on the foundation plan if soil treatment is used for subterranean termite prevent Submit other approved termite protection methods. Protection shall be provided by reg		
37 Submit other approved termite protection methods. Protection shall be provided by reg termiticides		
FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)		
termiticides	- 0	

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Items to Include-

Each Box shall be

Floor Framing System: First and/or second story Floor truss package shall including layout and details, signed and sealed by Florida Registered 0 40 Professional Engineer Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, 0 stem walls and/or priers 42 Girder type, size and spacing to load bearing walls, stem wall and/or priers 0 43 Attachment of joist to girder 0 44 Wind load requirements where applicable 0 45 Show required under-floor crawl space 0 Show required amount of ventilation opening for under-floor spaces 0 Show required covering of ventilation opening 0 Show the required access opening to access to under-floor spaces 0 Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & 0 49 intermediate of the areas structural panel sheathing 50 Show Draftstopping, Fire caulking and Fire blocking 0 51 Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6 0 52 Provide live and dead load rating of floor framing systems (psf). 0 FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION Items to Include-**GENERAL REQUIREMENTS:** Each Box shall be APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL Circled as Applicable Select from Drop down 53 Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls 0 O 54 Fastener schedule for structural members per table FBC-R602.3.2 are to be shown Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural 55 0 panel sheathing Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or 0 rafter systems Show sizes, type, span lengths and required number of support jack studs, king studs for 0 shear wall opening and girder or header per FBC-R602.7. 58 Indicate where pressure treated wood will be placed 0 Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas 60 A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

F	SCR :ROOF SYSTEMS:		,
61	Truss design drawing shall meet section FBC-R 802.10.1 Wood trusses	- 0	
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	- 0	
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	- 0	
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	- 0	
65	Provide dead load rating of trusses	- 0	

FBCR 802:Conventional Roof Framing Layout

66 Rafter and ridge beams sizes, span, species and spacing

67 Connectors to wall assemblies' include assemblies' resistance to uplift rating

68 Valley framing and support details

69 Provide dead load rating of rafter system

6 CO

FI	FBCR 803 ROOF SHEATHING						
	Include all materials which will make up the roof decking, identification of structural panel	_ 0					
	sheathing, grade, thickness		1				
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	- 0					

ROOF ASSEMBLIES FRC Chapter 9

	Include all materials which will make up the roof assembles covering	I -	0	
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	-	0	

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600 A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Each Box shall be Circled as Applicable			
	S	elec	t from	Drop	Down
74	Show the insulation R value for the following areas of the structure	Ţ-	0		
75	Attic space	-	O		
76	Exterior wall cavity	-	O		
77	Crawl space	<u> -</u>			0
н	VAC information				
78		-	0		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or	-	0		
	20 cfm continuous required	-		ļ	
80	Show clothes dryer route and total run of exhaust duct	-	0	<u> </u>	
	umbing Fixture layout shown				
81	All fixtures waste water lines shall be shown on the foundationplan	-	0		
82		-	0		
Pr	ivate Potable Water				
	Pump motor horse power	-			6
	Reservoir pressure tank gallon capacity	-			0
	Rating of cycle stop valve if used	-			0
	ectrical layout shown including				
86		T-	0		
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected	1-			
07	by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	-	Ø	-	
88	Show the location of smoke detectors & Carbon monoxide detectors	-	0		
89	Show service panel, sub-panel, location(s) and total ampere ratings	-	0		
0,	Short service paner, see paner, recarrently and real ampere rannings				
	On the electrical plans identify the electrical service overcurrent protection device for the main			1	
	electrical service. This device shall be installed on the exterior of structures to serve as a				
90	disconnecting means for the utility company electrical service. Conductors used from the exterior		0		
,,,		1			
70	disconnecting means to a panel of sub-panel shall have lour-wire conductors, of which one	1	2.3		
,,	disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance	-			
,,,	conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	-			
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91	conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3 Appliances and HVAC equipment and disconnects	-			

Items to Include-

Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as
	Applicable

ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT. Select from Drop down 93 **Building Permit Application** A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. 0 There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed. 94 Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office 0 (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com Environmental Health Permit or Sewer Tap Approval A copy of a approved 95 Columbia County Environmental Health (386) 758-1058 City of Lake City A City Water and/or Sewer letter. Call 386-752-2031 96 0 0 97 Toilet facilities shall be provided for all construction sites 98 Town of Fort White (386) 497-2321 If the parcel in the application for building permit is 0 within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit. 99 Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood - O elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com) CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot 0 Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required. A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00 |-**Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. ٥ If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. 911 Address: An application for a 911 address must be applied for and received through the Columbia 103 County Emergency Management Office of 911 Addressing Department (386) 758-1125. 0

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

**This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - http://www.columbiacountyfla.com/BuildingandZoning.asp

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Plast Pro Plast Pro Naynor		15220.4 F1 15213.14 FL 14092-110
Plast Pro Plast Pro Naynor		F1 15213.14
Plast Pro Naynur		
Naynor		FL 14092-110
YKK AP	Viesidentia 1	73778
Hardie		F1 889
F1 15213.14.0		F1 15213.14
Town & Country		1-6-11398-14
/		
Tanko	ASTM D 3462 Acolalt Shinls	18355.1
	Hardie Fl 152+3.14. Town & Country Tamko	Hardie FI 15213.14.0 Town & Country

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES:	



LOS ANGELES, CA 90045 5200 W. CENTURY BLVD.

Smooth / Wood Grain / White Wood Grain Series N Fiberglass Door Rustic / Mahogany **INSWING / OUTSWING** "NON-IMPACT"

74.00" MAX. OVERALL FRAME WIDTH

GENERAL NOTES

- This product has been evaluated and is in compliance with the 6th Edition (2017) Florida Building Code (FBC) structural requirements including the "High Velocity Hurricane Zone" (HVHZ).
- Product anchors shall be as listed and spaced as shown on details. Anchor embedment to base material shall be beyond wall dressing or stucco.
- When used in the "HYHZ" this product is required to be protected with an impact resistant covering that complies with Section 1626 of the FBC.
- When used in areas requiring wind borne debris protection, this product is required to be protected with an impact resistant covering that compiles with FBC Sections 1609.1.2 & R301.2.1.2.
- For 2x stud construction, anchoring of these units shall be the same as that shown for 2x buck masonry
- Site conditions that deviate from the details of this drawing require further engineering analysis by a licensed engineer or registered architect.
- Outswing configurations meet water infiltration requirements for "HVHZ"
- Inswing configurations do not meet the water infiltration requirements for the "TVHZ". Inswing units shall be installed only in non-habitable areas or at habitable locations protected by an overhang or canopy such that the angle between the edge of canopy or overhang to sill is less than 45 degrees.

#EEEE

DESCRIPTION

TABLE OF CONTENTS

Typical elevations, design pressures & general notes Door panel details and glazing detail Horizontal & Verlical Cross Sections (2X Buck)

Horizontal & Vertical Cross Sections (Direct to Masonry)
Horizontal & Vertical Cross Sections (Direct to Masonry) Horizontal & Vertical Cross Sections (1X Buck)

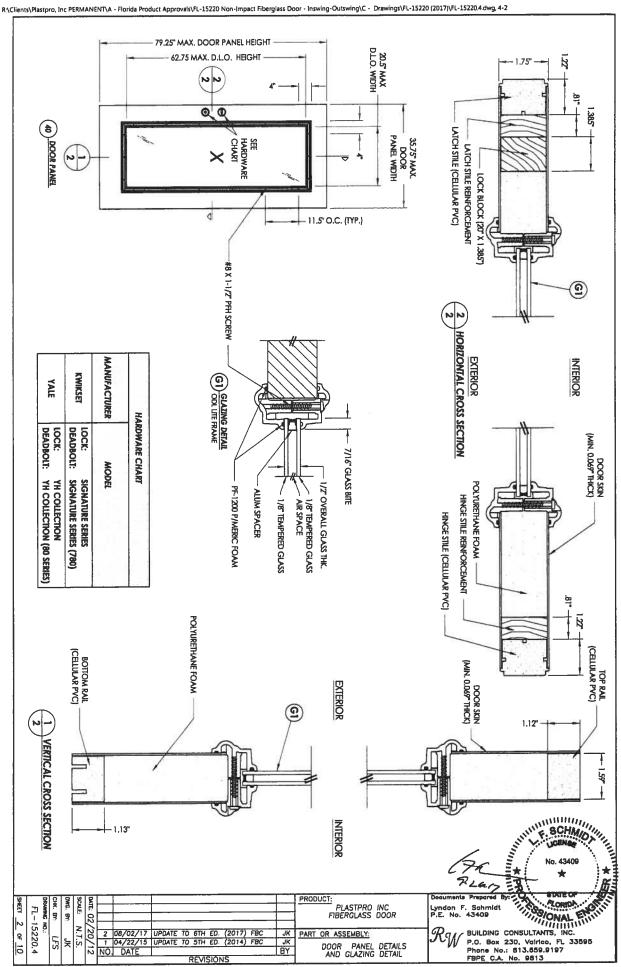
Bill of materials & components **Buck and frame anchoring** Vertical Cross Sections (Thresholds)

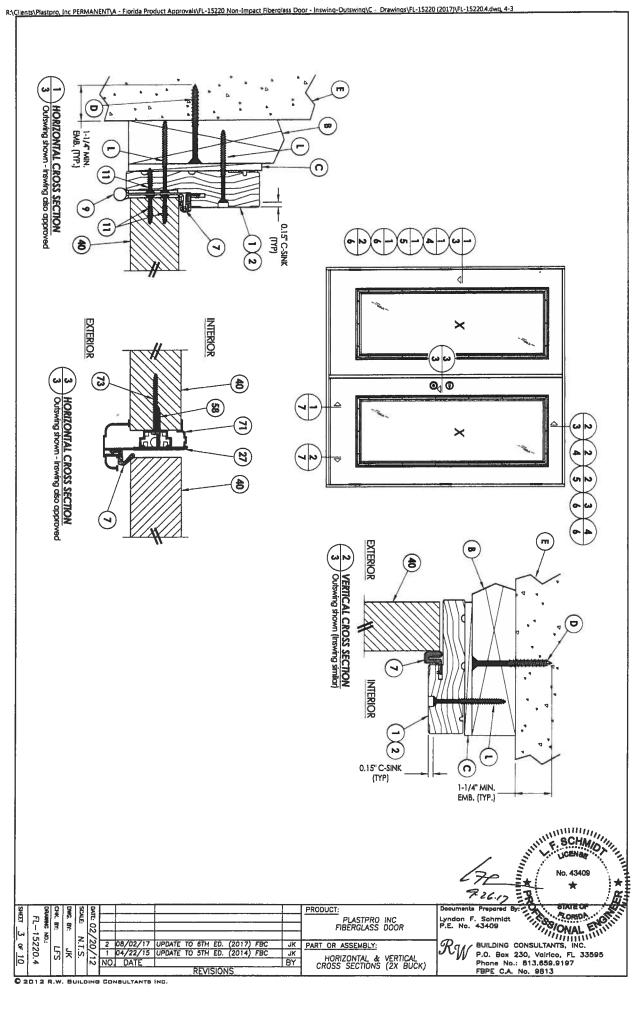
Astragal details

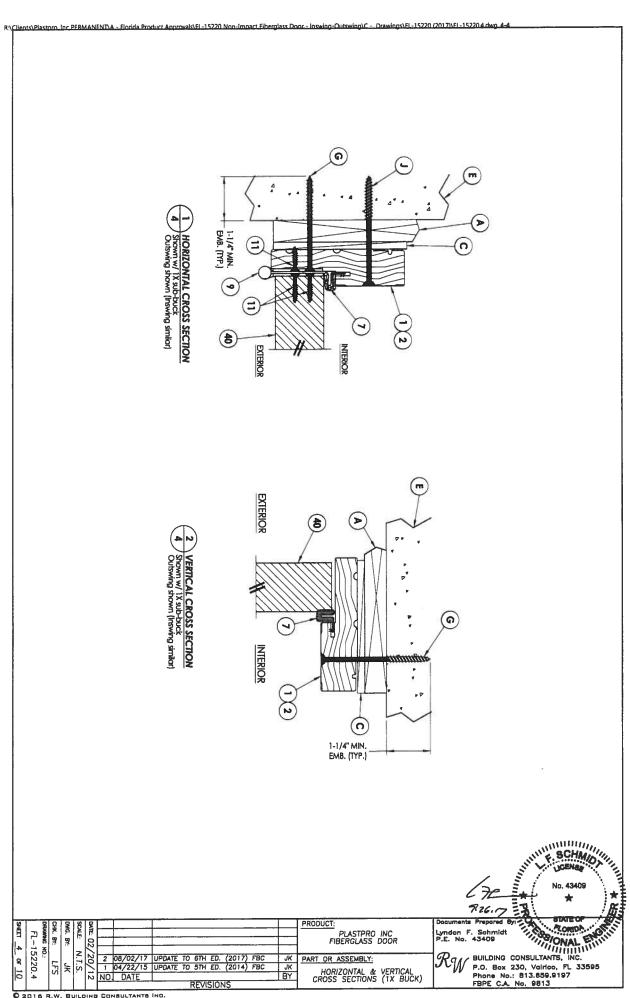
74.0" X 81.63"	FRAME	81.63" OVERALL FRAME HEIGHT - INSWING 80.5" OVERALL FRAME HEIGHT - OUTSWING
20.5" X 62.75"	D.L.O. DIMENSION	×
ତ୍ର	GLASS	O O
+50.0	POSITIVE	×
-50.0	POSITIVE NEGATIVE	

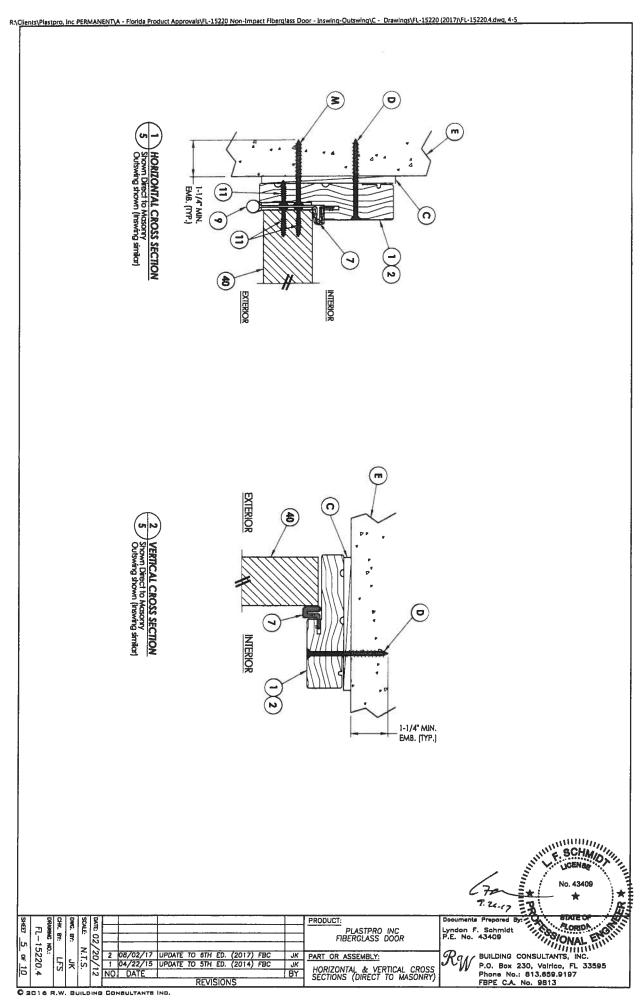
-50.0	+50.0	GI	20.5" X 62.75"	74.0" X 81.63"
NEGATI	POSITIVE	TYPE	DIMENSION	DIMENSION
ESSURE (P	DESIGN PRESSURE (P	GLASS	OVERALL	OVERALL

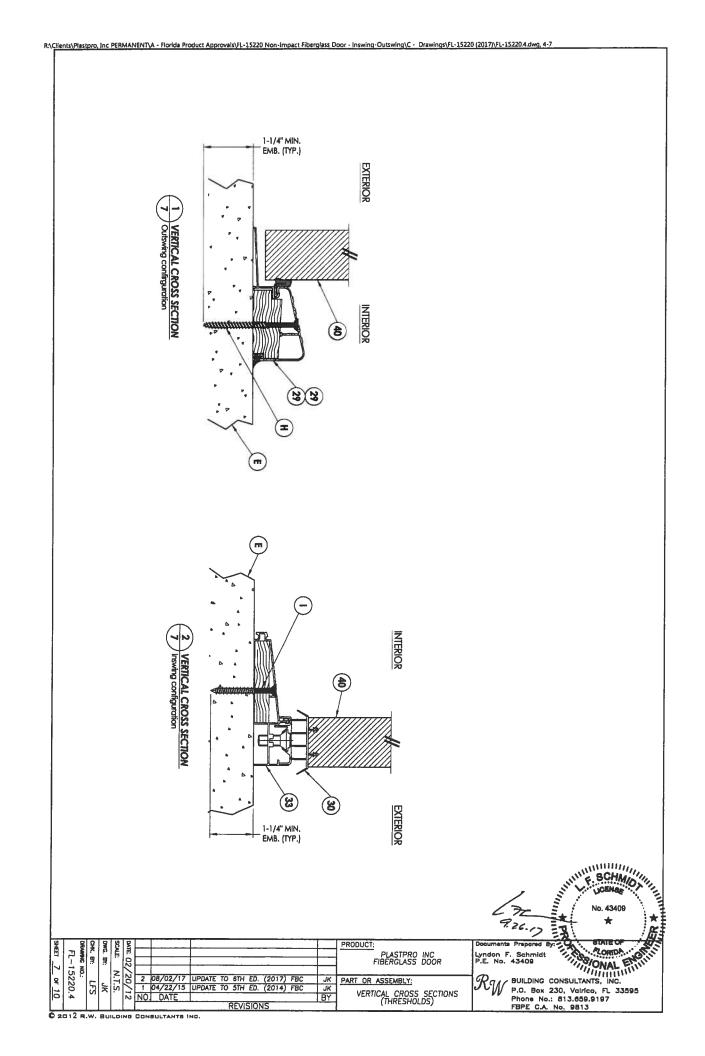
SCHMO JOENSE No. 43408 9.26. PRODUCT: 왕 HEET 1 or 10 JWG. BY: MTE: 02/ HAWING NO.: BUILDING CONSULTANTS, INC.
P.O. Box 230, Vairloo, FL 33595
Phone No.: 813.859.9197
FBPE C.A. No. 9813 FL-15220.4 Lyndon F. Schmidt P.E. No. 43409 PLASTPRO INC FIBERGLASS DOOR /20/12 08/02/17 UPDATE TO 6TH ED. (2017) FBC 04/22/15 UPDATE TO 5TH ED. (2014) FBC JK PART OR ASSEMBLY S JK TYPICAL ELEVATION, DESIGN PRESSURES & GENERAL NOTES NO. DATE BY REVISIONS

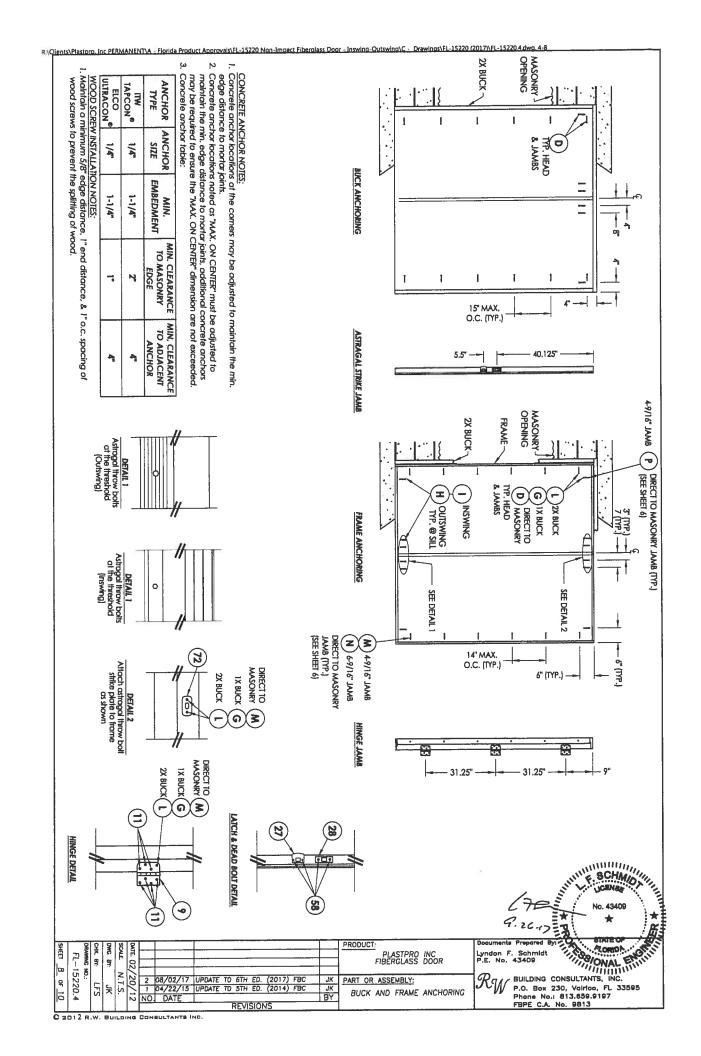












	73	72	71	8	8	ස	ఆ	29	28	27	12	11	9	8	7	2	-	P	z	3	_	~	_	-	Ŧ	G	п	n	0	ဂ	8	>	TTEM #	
CONCEALED SHEEP BY PLASIPRO (SINY) 6 ALLMINUM	#8 X Z' PH SCREW	ASTRAGAL STRIKE PLATE	ASTRAGAL BY DLP (6063-T5 ALUMINUM)	#8 X 2" PFH WOOD SCREW	DOOR PANEL - SEE DOOR PANEL DETAIL SHEET FOR CONSTRUCTION DETAILS	INSWING ADJUSTABLE ALUMINUM THRESHOLD BY DLP	INSWING VINYL DOOR BOTTOM SWEEP	OUTSWING THRESHOLD BY DLP	DEADBOLT STRIKE PLATE	LATCH STRIKE PLATE	#10 X 3" PFH WOOD SCREW	#9 X 3/4" PFH WOOD SCREW	4" X 4" BUTI HINGE	COMPRESSION WEATHERSTRIP QLON 650 BY SCHLEGEL	FORCE 5 WEATHERSTRIPPING BY ENDURA	FINGER JOINTED PINE FRAME	POLY FIBER JAMB	1/4" X 2-1/4" PFH ELCO CONCRETE SCREW	1/4" x 3-1/4" PFH ITW CONCRETE SCREW	3/16" X 2-1/4" ITW CONCRETE SCREW	#10 X 2-1/2" PFH WOOD SCREW (1.15" MIN. EMBEDMENT)	#7 X 2" PFH DRYWALL SCREW	1/4" X 3-3/4" PFH ELCO OR ITW CONCRETE SCREW	1/4" X 2-1/4" PFH ELCO OR ITW CONCRETE SCREW	1/4" x 3-1/4" PFH ELCO OR ITW CONCRETE SCREW	3/16" X 3-1/4" PFH ITW CONCRETE SCREW	301 OR HOLLOW BLOCK CONFORMING TO ASTM C90	MASONRY - 3,000 PSI MIN. CONCRETE CONFORMING TO ACI	1/4" X 2-3/4" ELCO OR ITW CONCRETE SCREW	1/4" MAX. SHIM SPACE	2X BUCK SG >= 0.55	1X BUCK SG >= 0.55	DESCRITION	BILL OF MATERIALS
ALUM.	STEEL	STEEL	ALUM.	STEEL	e	ALUM. / WOOD	VINYL	ALUM./WOOD	STEEL	STEEL	STEEL	STEEL	STEEL	FOAM	FOAM	WOOD	COMP. / VINYL	STEEL	STEEL	HEIS	TEILS	STEEL	STEEL	STEEL	STEEL	STEEL	COMCRETE	CONCRETE	STEEL		WOOD	WOOD	MATERIAL	

29 OUTSWING THRESHOLD BY DLP	Da SIDE JAMB Boar 1.75	No. 43409
DAILE 02/20 12 08/02/17 UPDATE TO 6TH ED. (2017) FBC 1 04/22/15 UPDATE TO 5TH ED. (2014) FBC 1 04/22/15 UPDATE TO 5TH ED. (201	PRODUCT: PLASTPRO INC FIBERGLASS DOOR	Lyndon F. Schmidt P.E. No. 43409
002/2 004/3 (1904) VO CEL SD (2013) EBS		
2 08/02/17 UPDATE TO 6TH ED. (2017) FBC 1 04/22/15 UPDATE TO 5TH ED. (2014) FBC	JK PART OR ASSEMBLY:	Ry Building consultants, inc. P.O. Box 230, Valdeo, FL 33595
NO. DATE REVISIONS	BILL OF MATERIALS & COMPONENTS	Phone No.: 813.659.9197 FBPE C.A. No. 9813
D 2012 R.W. BUILDING CONSULTANTS INC.		

VENEER, AND SIDING.

INSTALLATION NOTES

- ONE (1) INSTALLATION ANCHOR IS REQUIRED AT EACH ANCHOR
- 2. THE NUMBER OF INSTALLATION ANCHORS DEPICTED IS THE MINIMUM NUMBER OF ANCHORS TO BE USED FOR PRODUCT INSTALLATION.
- 3. INSTALL INDIVIDUAL INSTALLATION ANCHORS WITHIN A TOLERANCE OF ±1/2 INCH OF THE DEPICTED LOCATION IN THE ANCHOR LAVOUT DETAIL (I.E., WITHOUT CONSIDERATION OF TOLERANCES). TOLERANCES ARE NOT CUMULATIVE FROM ONE INSTALLATION ANCHOR TO THE NEXT.
- FOR INSTALLATION INTO WOOD FRAMING USE #8 WOOD SCREWS SCREWS OF SUFFICIENT LENGTH TO ACHIEVE 3/4 INCH MINIMUM EMBEDMENT INTO WOOD SUBSTRATE.
- 5. FOR INSTALLATION INTO METAL STUD USE #8 PAN HEAD SCREWS THROUGH THE FRAME OF SUFFICIENT LENGTH TO ACHIEVE A MINIMUM OF 3 THREADS PENETRATION BEYOND METAL FRAME . MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDE WALL FINISHES, INCLUDING BUT NOT LIMITED TO STUCCO, FOAM, BRICK SUBSTRATE.
- INSTALLATION ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH ANCHOR MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND ANCHORS SHALL NOT BE USED IN SUBSTRATES WITH STRENGTHS LESS THAN THE MINIMUM STRENGTH SPECIFIED BY THE ANCHOR MANUFACTURER. INSTALLATION ANCHORS AND ASSOCIATED HARDWARE MUST BE MADE OF CORROSION RESISTANT MATERIAL OR HAVE A CORROSION RESISTANT COATING.
- INSTALLATION ANCHOR CAPACITIES FOR PRODUCTS HEREIN ARE BASED ON SUBSTRATE MATERIALS WITH THE FOLLOWING PROPERTIES:
- A. WOOD MINIMUM SPECIFIC GRAVITY OF 0.55.
 B. STEEL MINIMUM YIELD STRENGTH OF 33 KSI. MINIMUM 18 GA. WALL THICKNESS.

YKK AP RESIDENTIAL

StyleView Single Hung Window

GENERAL NOTES

- 1. THE PRODUCT SHOWN HEREIN IS DESIGNED AND MANUFACTURED TO COMPLY WITH THE CURRENT EDITION FLORIDA BUILDING CODE FREQ. EXCLUDING HYPIZ AND HAS BEEN EVALUATED ACCORDING TO THE FOLLOWING: AAMA/WDMA/CSA 101/I.S.2/A440-05
- 2. ADEQUACY OF THE EXISTING STRUCTURAL
 CONCRETE/MASDANRY, ZX FRAMING AND METAL STUD
 FRAMING AS A MAIN WIND FORCE RESISTING SYSTEM CAPABLE
 OF WITHSTANDING AND TRANSFERRING APPLED PRODUCT
 LOADS TO THE FOLUNDATION IS THE RESPONSIBILITY OF THE
 ENGINEER OR ARCHITECT OF RECORD FOR THE PROJECT OF
- ZX BUCKS (WHEN USED) SHALL BE DESIGNED AND ANCHORED TO PROPERLY TRANSFER ALL LOADS TO THE STRUCTURE, BUCK DESIGN AND INSTALLATION IS THE RESPONSIBILITY OF THE ENGINEER OR ARCHITECT OF RECORD FOR THE PROJECT OF INSTALLATION.
- 4. THE INSTALLATION DETAILS DESCRIBED HEREIN ARE GENERIC
 AND MAY NOT REFLECT ACTUAL CONDITIONS FOR A SPECIFIC
 SITE. IF SITE CONDITIONS CAUSE INSTALLATION TO DEVIATE
 FROM THE REQUIREMENTS DETAILED HEREIN, A LICENSED
 ENGINEER OR ARCHITECT SHALL PREPARE SITE SPECIFIC
 DOCUMENTS FOR USE WITH THIS DOCUMENT.
- APPROVED IMPACT PROTECTIVE SYSTEM IS REQUIRED ON THIS PRODUCT IN AREAS REQUIRING IMPACT RESISTANCE.
- WINDOW FRAME MATERIAL: PVC
- DESIGNATIONS "X" AND "O" STAND FOR THE FOLLOWING: X: OPERABLE PANEL O: FIXED PANEL
- GLAZING MEETS ASTM £1300 REQUIREMENTS, SEE SHEET 3 FOR GLAZING DETAILS.

HORIZONTAL SECTION	4
VERTICAL SECTION & GLAZING DETAIL	3
ELEVATION & ANCHOR SCHEDULE	Ν
INSTALLATION & GENERAL NOTES	1
SHEET DESCRIPTION	SHEET
TABLE OF CONTENTS	

0/x	CONFIGURATION
+50 / -50 PSF	CONFIGURATION DESIGN PRESSURE
47.5" × 71.5"	MAXIMUM SIZE
NON-IMPACT	MISSILE IMPACT RATING

Date: 2015.06.24 18:33:11 -04'00' Reason: I am approving this document Digitally signed by Hermes F. Norero, P.E.

SHEET

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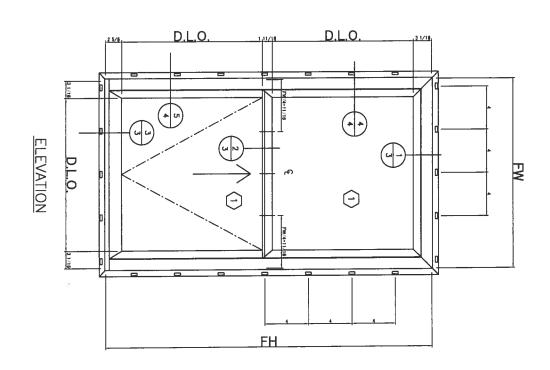
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DATE: 06.04.1 DWN BY CHK BY HFN SCALE NT5 FIRE COST, OF ALMACUEZATION NO. 29879

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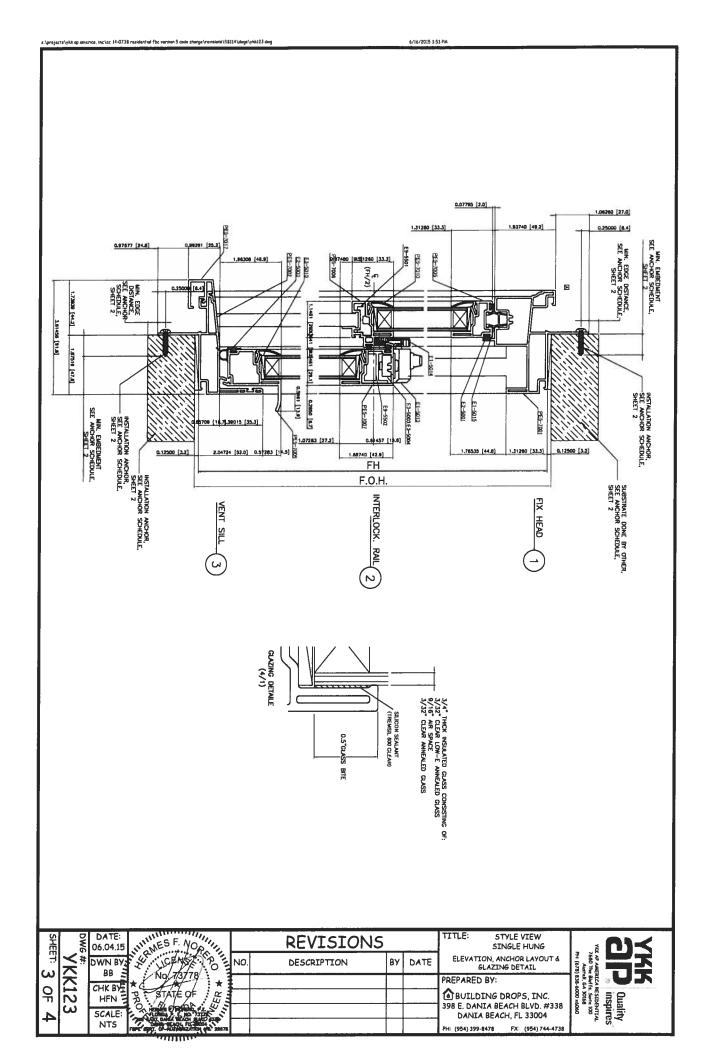
WES F. VO		REVISIONS			TITLE: STYLE VIEW SINGLE HUNG
E CENSTO	NO.	DESCRIPTION	ВУ	DATE	INSTALLATION & GENERAL NOTES
NO. 1/3/19 *					PREPARED BY:
STATE OF C					BUILDING DROPS, INC.
					398 E. DANIA BEACH BLVD. #338 DANIA BEACH, FL 33004
CONT. OF AUTHORIZATION NO. 20070					PH: (954) 399-8478 FX (954) 744-4738

YKK AP AMERICA RESIDENTIAL 7680 The Bluffs, Suite 100 Austell, 6A 30168 TE5 inspires Quality 138



	H	ANCHOR SCHEDULE	Ш	
 METHOD	SUBSTRATE	ANCHOR	MIN. EMBEDMENT	MIN. EDGE DISTANCE
	MIN. S.G. = 0.55 WOOD #8 WOOD SCREW	#8 WOOD SCREW	1.5"	0.75"
NAIL FIN	NAIL FIN 18 GAUGE STEEL MIN	#8 PAN HEAD	3 THREADS	
	33 65	CODEW	PENETRATION	0.75"
	1 y 1 33 ASI	GCALAA	BEYOND METAL	

SHEE	DW6	DATE: 06.04.15	WINES F. NODIN		REVISIONS			TITLE: STYLE VIEW SINGLE HUNG	. ,
9	 	DWN BY	E CONTRACTOR	NO.	DESCRIPTION	ВУ	DATE	ELEVATION & ANCHOR SCHEDULE	AP AMER 680 The Austr
12		CHK BYE	* No. 73/78 *					PREPARED BY:	Buffa R
유	12:	HFN	TO STATE OF THE	_		_		BUILDING DROPS, INC. 398 E. DANIA BEACH BLVD. #338	Ouali Superior Superi
4	ω	SCALE: NTS	PLOTON AND AND AND AND AND AND AND AND AND AN	\vdash				DANIA BEACH, FL 33004 PH: (954) 399-8478 FX: (954) 744-4738	°° i Bi



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INSTALLATION NOTES:

- $\mathbf{1}_{\circ}$ ONE (1) INSTALLATION ANCHOR IS REQUIRED AT EACH ANCHOR LOCATION SHOWN.
- THE NUMBER OF INSTALLATION ANCHORS DEPICTED IS THE MINIMUM NUMBER OF ANCHORS TO BE USED FOR PRODUCT
- INSTALL INDIVIDUAL INSTALLATION ANCHORS WITHIN A TOLERANCE OF £1/2 INCH OF THE DEPICTED LOCATION IN THE ANCHOR LAYOUT DEFTAIL (I.E., WITHOUT CONSIDERATION OF TOLERANCES). TOLERANCES ARE NOT CUMULATIVE FROM ONE INSTALLATION ANCHOR TO THE NEXT.
- FOR INSTALLATION INTO WOOD FRAMING USE #10 WOOD SCREWS OF SUFFICIENT LENGTH TO ACHIEVE 1 1/2 INCH MINIMUM EMBEDMENT INTO WOOD SUBSTRATE.
- MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDE WALL FINISHES, INCLUDING BUT NOT LIMITED TO STUCCO, FOAM, BRICK VENEER, AND SIDING. FOR INSTALLATION INTO METAL STUD USE #10 TEK SCREWS THROUGH THE FRAME OF SUFFICIENT LENGTH TO ACHIEVE A MINIMUM OF 3 THREADS PENETRATION BEYOND METAL FRAME
- INSTALLATION ANCHORS AND ASSOCIATED HARDWARE MUST BE MADE OF CORROSION RESISTANT MATERIAL OR HAVE A CORROSION RESISTANT COATING.
- INSTALLATION ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH ANCHOR MANUFACTURER'S INSTALLATION INSTRUCTIONS, AND ANCHORS SHALL NOT BE USED IN SUBSTRATES WITH STRENGTHS LESS THAN THE MINIMUM STRENGTH SPECIFIED BY THE ANCHOR MANUFACTURER.
- INSTALLATION ANCHOR CAPACITIES FOR PRODUCTS HEREIN ARE BASED ON SUBSTRATE MATERIALS WITH THE FOLLOWING
- PROPERTIES:

 A. WOOD. ANIMUM SPECIFIC GRAVITY OF 0.55.

 B. STEEL MINIMUM YELD STRENGTH OF 33 KSI. MINIMUM 18

 GA. WALL THICKNESS.

GENERAL NOTES:

- THE PRODUCT SHOWN HEREIN IS DESIGNED AND
 MANUFACTURED TO COMPTY WITH THE CURRENT EDITION
 FLORIDA BILLIONING CODE FIRED, EXCLUDING HVPLA AND HAS
 BEEN EVALUATED ACCORDING TO THE FOLLOWING: AAMA/WDMA/CSA 101/I.S.2/A440-11
- ADEQUACY OF THE EXISTING STRUCTURAL
 CONCRETE/MASSONRY, 2X FRAMING AND METAL STUD
 FRAMING AS A MAIN WIND FORCE RESISTING SYSTEM CAPABLE
 OF WITHSTANDING AND TRANSFERRING APPLIED PRODUCT
 LOADS TO THE EVOLUNDATION IS THE RESPONSIBILITY OF THE
 ENGINEER OR, ARCHITECT OF RECORD FOR THE PROJECT OF INSTALLATION.
- ZX BUCKS (WHEN USED) SHALL BE DESIGNED AND ANCHORED TO PROPERLY TRANSFER ALL LOADS TO THE STRUCTURE. BUCK DESIGN AND INSTALLATION IS THE RESPONSIBILITY OF THE ENGINEER OR ARCHITECT OF RECORD FOR THE PROJECT OF INSTALLATION.
- THE INSTALLATION DETAILS DESCRIBED HEREM ARE GENERIC AND MAY NOT REFLECT ACTUAL CONDITIONS FOR A SPECIFIC STELL IF SITE CONDITIONS CAUSE INSTALLATION TO DEVIATE FROM THE RECURREMENTS DETAILED HEREIN, A LICENSED ENGINEER OR ARCHITECT SHALL PREPARE SITE SPECIFIC DOCUMENTS FOR USE WITH THIS DOCUMENT IN NON-HYHZ AREAS.
- APPROVED IMPACT PROTECTIVE SYSTEM IS REQUIRED ON THIS PRODUCT IN AREAS REQUIRING IMPACT RESISTANCE.
- WINDOW FRAME MATERIAL: PVC
- DESIGNATIONS "X" AND "O" STAND FOR THE FOLLOWING:
 X: OPERABLE PANEL
 O: FIXED PANEL
- GLAZING SHALL MEET ASTM £1300 REQUIREMENTS, SEE SHEET 2 FOR GLAZING DETAILS.

		TABLE OF CONTENTS
знеет	REVISION	SHEET DESCRIPTION
1	>	INSTALLATION & GENERAL NOTES
2	٨	ELEVATION, ANCHOR SCHEDULE & GLAZING DETAIL
ω	>	VERTICAL & HORIZONTAL SECTIONS

0	CONFIGURATION	
+50 / -50 PSF	DESIGN PRESSURE MAXIMUM SIZE	UNIT DIA
+50 / -50 PSF 79 1/2" X 79 1/2"	MAXIMUM SIZE	UNIT DIMENSIONS
NON-IMPACT	MISSILE IMPACT RATING	

Digitally signed by Hermes F. Norero, F Reason: I am approving this document Date: 2016.05:30 13:06:20 -04'00'

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	SHOP OF THE PROPERTY BLAZE COME CHARA BEACH, P. 3300A PRIME CERT, OF PATHONNEL AND HO. 28578		

MINIMIN.	-				
CENSON IN		REVISIONS			TITLE: STYLEVIEW PICTURE WINDOW
No. 73778	NO.	DESCRIPTION	ВУ	DATE	INSTALLATION & GENERAL NO
STATE OF C	ĒA	ADDITIONAL 80" X 80" UNIT	L5	05.18.16	PREPARED BY:
FLORDA H.	F				BUILDING DROPS, INC.
PARTIES P. MOPERIO, P. S.					398 E. DANIA BEACH BLVD. #3 DANIA BEACH, FL 33004
CAMEN BENEFIT STORM					PH: (954) 399-8478 FX: (954) 744-

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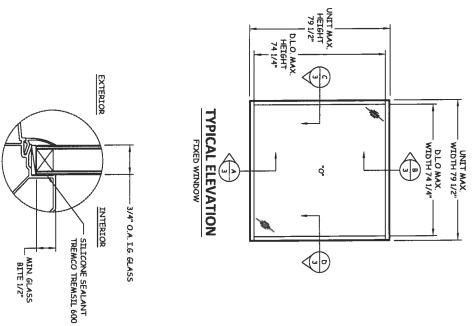
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YKK AP AMERICA RESIDENTIAL 270 Riverside Portsooy, Suite A Austell, GA 30168 PH: (678) 838-6000 x6060 Quality inspires

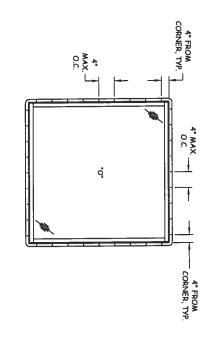
GLAZING NOTES:
GLASS THICKNESS AND TYPE SHALL COMPLY
WITH ASTM E1300 GLASS STRENGTH
REQUIREMENTS.

GLAZING DETAIL

ALL GLAZING CONFIGURATIONS SHALL COMPLY WITH SAFETY GLAZING REQUIREMENTS OUTLINED IN CURRENT FBC.

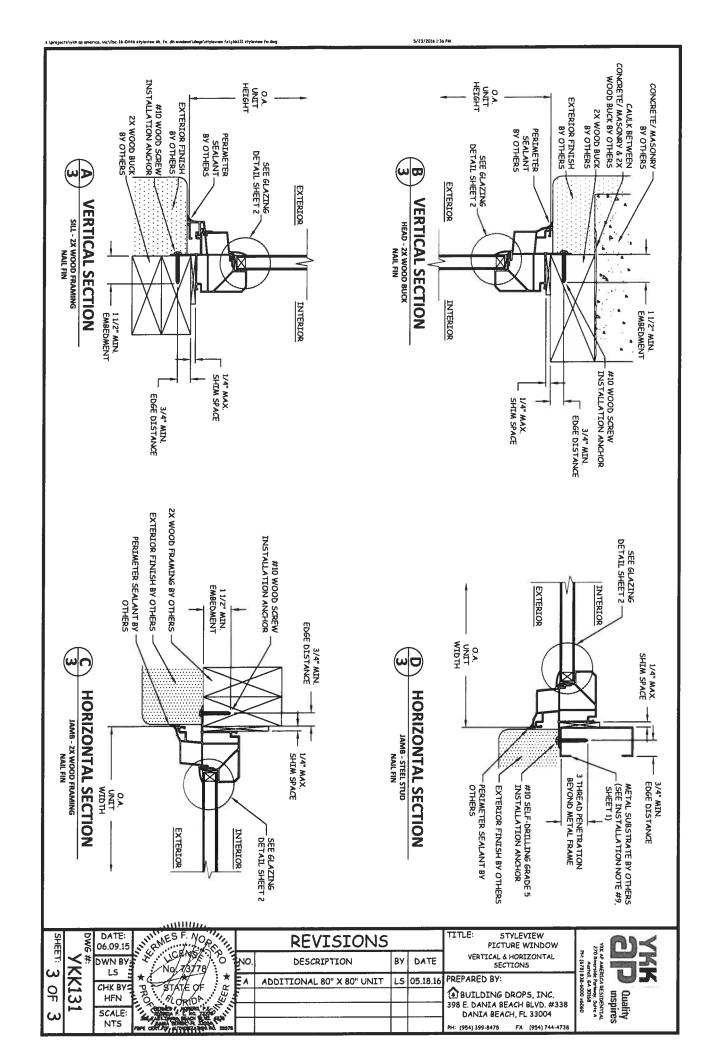


		ANCHOR SCHEDULE	m	
метнор	SUBSTRATE	ANCHOR	MIN. EMBEDMENT	MIN. EDGE DISTANCE
	MIN. S.G. = 0.55 WOOD #10 WOOD SCREW	#10 WOOD SCREW	1.5*	0.75
NAIL FIN	NAIL FIN 18 GAUGE STEEL, MIN fy = 33 ksi	#10 TEK SCREW	3 THREADS PENETRATION BEYOND METAL	0.75"



SHEE	DW6	DATE: 06.09.15	WILLIAMES F. NO.	14	REVISIONS			TITLE: STYLEVIEW PICTURE WINDOW	, gặ (1) ≼
ET:	∠ #	DWN BY	No 73778	NO.	DESCRIPTION	ВУ	DATE	ELEVATION, ANCHOR SCHEDULE. & GLAZING DETAIL	D Riverside Control
2	 	CHK BY:	TO STATE OF	* 1	ADDITIONAL 80" X 80" UNIT	LS	05,18.16	PREPARED BY:	EL PORTE
유	<u> 3</u>	HFN	ON FLORIDA	$\bar{y}_{\bar{y}} = 0$				BUILDING DROPS, INC. 398 E. DANIA BEACH BLVD. #338	Quali inspir
ω		SCALE: NTS	SHEAL SHOW THE ATTER	19576				DANIA BEACH, FL 33004 PH: (954) 399-8478 FX: (954) 744-4738	Sau Sau

ANCHOR LAYOUT





Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2135432 - CHEMERY CONST. - LOT 7 FWS

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Chemery's Const. Project Name: Spec House Model: Custom

Subdivision: Fort White Station

Lot/Block: 7 Address: N/A, N/A

City: Columbia Cty

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special

Loading Conditions):

Design Code: FBC2017/TPI2014

Wind Code: ASCE 7-10

Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.2

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 19 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No. 12345678910112314	Seal# T18737148 T18737149 T18737150 T18737151 T18737153 T18737155 T18737156 T18737157 T18737158 T18737160 T18737160 T18737160	Truss Name CJ01 CJ03 CJ05 EJ01 HJ10 T01 T02 T03 T04 T04G T05 T05G T06 T07	Date 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19 11/22/19
11	T18737158	T05	11/22/19
12	T18737159	T05G	11/22/19
13	T18737160	T06	11/22/19
17	T18737164	T10	11/22/19
18	T18737165	T11	11/22/19
19	T18737166	T11G	11/22/19



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

	φļ	(8)

CHEMERY CONST. - LOT 7 FWS Qty Truss Type Job Truss T18737148 1 CJ01 Jack-Open 2135432 Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 22 12 21 48 2019 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:5lppGi3mlXfNNmnlqREYFDz9Eln-kW?KFWe?d?RH8dlbxtgzfdUmn0A?QjrNB?YfW3yGdp1 1-0-0 Scale = 1:9.5 3 6.00 12 0-10-8 0-10-B 0-5-13 0-4-8 4 1-0-0 Plate Offsets (X,Y)-[2:0-1-4,0-1-9] **PLATES** GRIP DEFL. in I/defl I /d CSI. (loc) SPACING-2-0-0 LOADING (psf) 244/190 MT20 0.32 Vert(LL) 0.00 >999 240 Plate Grip DOL 1.25 TC TCLL 20.0 0.07 Vert(CT) 0.00 >999 180 1.25 BC Lumber DOL TCDL 7.0

LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 2

0.0

10.0

BRACING-

Horz(CT)

0.00

2 n/a

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.

Weight: 7 lb

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

REACTIONS. (lb/size) 3=-27/Mechanical, 2=254/0-3-8, 4=-46/Mechanical

YES

Max Horz 2=66(LC 12)

Max Uplift 3=-27(LC 1), 2=-162(LC 12), 4=-46(LC 1) Max Grav 3=25(LC 16), 2=254(LC 1), 4=44(LC 16)

Code FBC2017/TPI2014

Rep Stress Incr

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-MP

0.00

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 162 lb uplift at joint 2 and 46 lb uplift at joint 4.



FT = 20%

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

November 22,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply CHEMERY CONST. - LOT 7 FWS T18737149 2135432 CJ03 Jack-Open Job Reference (optional) **Builders FirstSource** Jacksonville, FL - 32244, 8.240 s Jul 14 2019 MiTek Industries, Inc. Fri Nov 22 12 21:50 2019 Page 1 ID_5lppGi3mlXfNNmnlqREYFDz9Eln-hv74gBfG9dh_OxS_3HiRk2Z6GqsTudLgfJ2lbyyGdp?

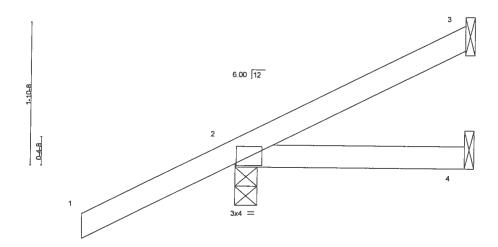


Plate Offsets (X,Y)- [2:0-0-3,0-0-5]			3-0-0			1	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.32 BC 0.07 WB 0.00 Matrix-MP	DEFL. in Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) 0.00	(loc) 4-7 4-7 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 13 lb	GRIP 244/190 FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=52/Mechanical, 2=253/0-3-8, 4=20/Mechanical

Max Horz 2=113(LC 12)

Max Uplift 3=-48(LC 12), 2=-126(LC 12)

Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3 and 126 lb uplift at joint 2.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, and a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314



6904 Parke East Blvd Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	CHEMERY CON	ST - LOT 7 FWS	
			,	'	1		T18737150
2135432	CJ05	Jack-Open	4	1	Job Reference (o	ntinnel\	
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s	Jul 14 2019 MiTek li	ndustries, Inc. Fri Nov 22	2 12 21:51 2019 Page 1
		200	ID:5lppGi3mI)	(fNNmnlqRE	EYFDz9Eln-95hTtXg	uwwpr?51Ad?DgHG6H0	EAHd4bqtznJ7OyGdp_
	 	-2-0-0 2-0-0	<u>5-</u>	D-0 D-0			
						3 🖂	Scale = 1 19 5
	1					M	
			_		//		
			6.00 12				
	eq.			/ /			
	2-10-A						
	ر ا	2 /				М	
	0.4-B		***			XI	
	14					<u>_</u>	
						4	
	1/	3x4 =					
		3,4 —					
			5-0	LN			
			5-0	H-0			
LOADING (psf)	SPACING- 2-	0-0 CSI.	DEEL	in (1)	Ira-et Ira	PLATEO	ODID.
TCLL 20.0		.25 TC 0.32	DEFL. Vert(LL) 0	in (loc) .03 4-7	l/defl L/d >999 240	PLATES MT20	GRIP 244/190
TCDL 7.0	Lumber DOL 1	.25 BC 0.23		.05 4-7	>999 180	.01120	277150
BCLL 0.0 *		ES WB 0.00	Horz(CT) 0	.00 3	n/a n/a		
BCDL 10.0	Code FBC2017/TPI201	4 Matrix-MP				Weight: 19 I	b FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=108/Mechanical, 2=313/0-3-8, 4=53/Mechanical

Max Horz 2=162(LC 12)

Max Uplift 3=-98(LC 12), 2=-137(LC 12), 4=-1(LC 12) Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 3, 137 lb uplift at joint 2 and 1 lb uplift at joint 4.



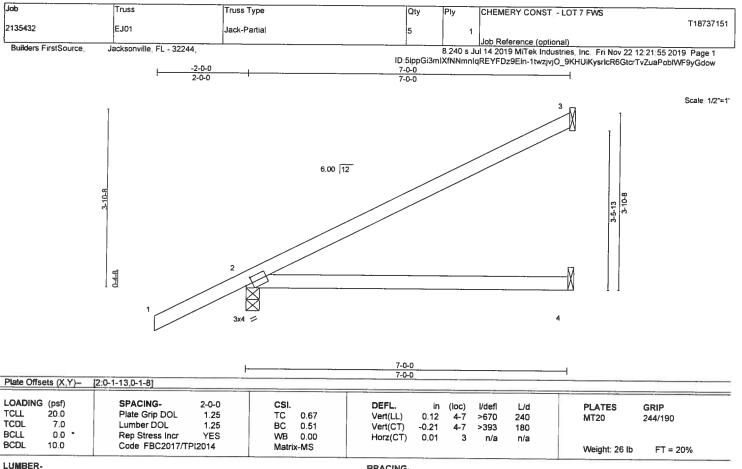
MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, cerction and bracing of trusses and truss systems, see ANSUTP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=160/Mechanical, 2=380/0-3-8, 4=81/Mechanical

Max Horz 2=144(LC 12)

Max Uplift 3=-94(LC 12), 2=-81(LC 12)

Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3 and 81 lb uplift at ioint 2.



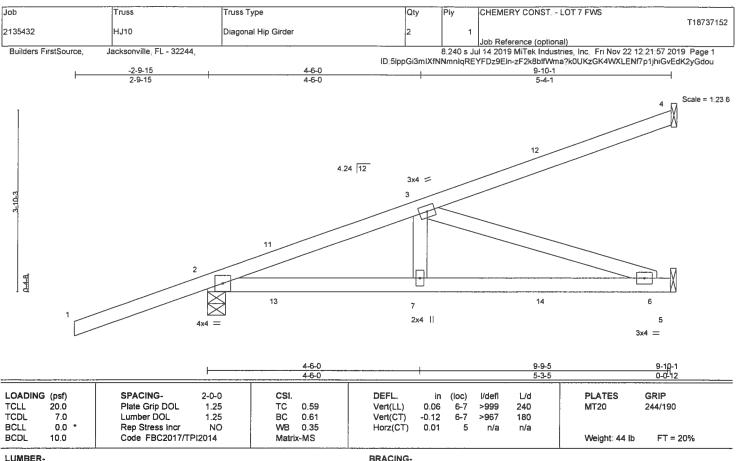
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

📤 WARNING - Verify design perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property demage. For general guidance regarding the tabrication, storage, delivery, eraction and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.3 WEBS

(lb/size) 4=150/Mechanical, 2=463/0-4-9, 5=251/Mechanical

Max Horz 2=233(LC 22)

Max Uplift 4=-141(LC 4), 2=-264(LC 4), 5=-103(LC 8) Max Grav 4=150(LC 1), 2=463(LC 1), 5=266(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-628/252

BOT CHORD 2-7=-327/573, 6-7=-327/573

WEBS 3-6=-603/345

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 4, 264 lb uplift at joint 2 and 103 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 86 lb down and 103 lb up at 1-6-1, 86 lb down and 103 lb up at 1-6-1, 26 lb down and 38 lb up at 4-4-0, 26 lb down and 38 lb up at 4-4-0, and 50 lb down and 97 lb up at 7-1-15, and 50 lb down and 97 lb up at 7-1-15 on top chord, and 36 lb down and 74 lb up at 1-6-1, 36 lb down and 74 lb up at 1-6-1, 28 lb down and 2 lb up at 4-4-0, 28 lb down and 2 lb up at 4-4-0, and 44 lb down and 15 lb up at 7-1-15, and 44 lb down and 15 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 13=70(F=35, B=35) 14=-49(F=-24, B=-24)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-9-0 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

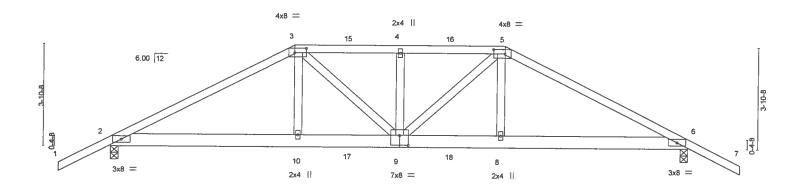
November 22,2019

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Jab	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
2135432	T01	Hip Girder	1	1	**	T18737153
7 71 5					Job Reference (optional)	
Builders FirstSource	Jacksonville, FL - 32244				al 14 2019 MiTek Industries, Inc. Fri Nov 22	
			ID:5lppGi3mIX	fNNmnlqRl	EYFDz9Eln-weAUYGmv1QqjzJej5gMYcyR\	V7SoqVfl?jDjkOxyGdos
-2-0-0	7-0-0	11-0-0	15-0-0		22-0-0	24-0-0
2-0-0	7-0-0	4-0-0	4-0-0	-	7-0-0	2-0-0

Scale = 1 42 4



⊢	7-0-0 7-0-0			1-0-0 4-0-0		5-0-0 4-0-0				2-0-0 7-0-0	→
Plate Offsets (X,Y)-	[3:0-5-4,0-2-0], [5:0-5-4,0	-2-0], [9:0-4-0,				*****	*			7-0-0	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 NO PI2014	BC (0.95 0.70 0.24 MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.11 -0.20 0.06	(loc) 9 9 6	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 122 lb	GRIP 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 8-2-9 oc bracing.

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS

2x4 SP No.3

(lb/size) 2=1586/0-3-8, 6=1613/0-3-8

Max Horz 2=-61(LC 6)

Max Uplift 2=-495(LC 8), 6=-534(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=2831/898, 3-4=-2831/954, 4-5=-2831/954, 5-6=-2890/982 BOT CHORD 2-10=-752/2460, 9-10=-755/2482, 8-9=-800/2534, 6-8=-797/2513 WEBS 3-10=-93/624, 3-9=-267/564, 4-9=-465/267, 5-9=-143/480, 5-8=-91/623

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 495 lb uplift at joint 2 and 534 lb uplift at ioint 6.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 100 lb up at 7-0-0, 106 lb down and 100 lb up at 9-0-12, 106 lb down and 100 lb up at 11-0-0, and 106 lb down and 100 lb up at 12-11-4, and 227 ib down and 250 lb up at 15-0-0 on top chord, and 294 lb down and 131 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-0, and 85 lb down at 12-11-4, and 294 lb down and 131 lb up at 14-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) in the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

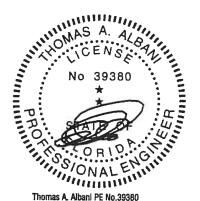
LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=54, 3-5=54, 5-7=-54, 2-6=20

Concentrated Loads (lb)

Vert: 3=-106(F) 5=-180(F) 10=-284(F) 9=-61(F) 4=-106(F) 8=-284(F) 15=-106(F) 16=-106(F) 17=-61(F) 18=-61(F)



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

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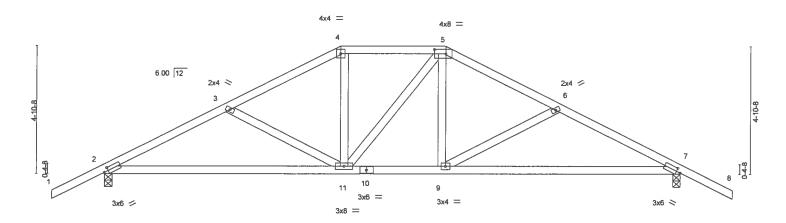
ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component states of the property damage. For general guidance regarding the tablication storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component states and property damage.



Job	Truss	Truss Type		Qty	Ply	CHEMERY CO	NST LOT 7 FWS	
	1							T18737154
2135432	T02	Hip		1	1			
					1	Job Reference	(optional)	
Builders FirstSource,	Jacksonville, FL - 32244,				8 240 s Ju	il 14 2019 MiTel	Industries, Inc. Fri Nov 22 1	2.22.02.2019 Page 1
			ID:5lpj	oGi3mIXfl	NmnlqRE	YFDz9Eln-KDro	iBlpnKICHqnMlmpwFDa38fgj	oci0zRPByO?FyGdop
2-0-0	4-9-8	9-0-0	13-0-0	1	17-2	8	22-0-0	24-0-0
2-0-0	4-9-8	4-2-8	4-0-0	- 1	4-2-	-8	4-9-8	2-0-0

Scale = 1 42 4



	-		-0-0 -0-0			13-0-0 4-0-0	+			22-0-0 9-0-0		
Plate Offse	ets (X,Y)-	[2:0-1-15,0-1-8], [5:0-5-4,		15,0-1-8]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.16	9-17	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.69	Vert(CT)	-0.32	9-17	>823	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.04	7	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	-MS	, ,					Weight: 111 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

REACTIONS. (lb/size) 2=922/0-3-8, 7=922/0-3-8

Max Horz 2=-75(LC 10) Max Uplift 2=-199(LC 12), 7=-199(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1378/742, 3-4=-1115/596, 4-5=-952/583, 5-6=-1114/595, 6-7=-1378/742

2-11=-513/1207, 9-11=-294/951, 7-9=-539/1207 BOT CHORD

3-11=-300/281, 4-11=-90/318, 5-9=-97/318, 6-9=-300/281 **WEBS**

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 2 and 199 lb uplift at joint 7.



Structural wood sheathing directly applied or 4-10-0 oc purlins.

Rigid ceiling directly applied or 7-11-15 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019



Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
2135432	тоз	Common	6	1	T18	3737155
		<u> </u>			Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8 240 s J	ul 14 2019 MiTek Industries, Inc. Fri Nov 22 12 22 04 2019 Par	ae 1
			ID:5lppGi3mIX	fNNmnlqR	EYFDz9Eln-GbzNc_q2swS?34WgtEyjJ?8TPTYIAtLktVRV48yG	don
-2-0-0	5-8-5	11-0-0		-3-11	22-0-0 , 24-0-0	
2-0-0	5-8-5	5-3-11	5-	3-11	5-8-5 2-0-0	—

Scale = 1 41.9

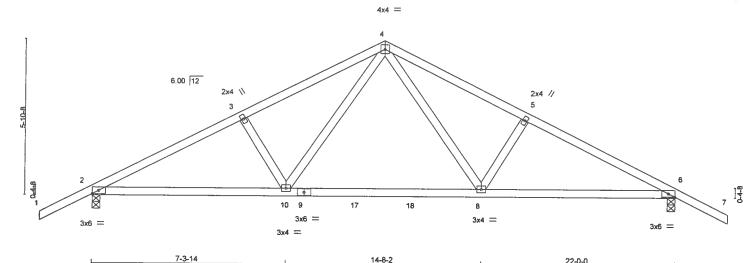


Plate Offsets (X,Y)-	7-3-14 7-3-14 [6:0-2-15,Edge]	<u> </u>	14-8-2 7-4-5	22-0-0 7-3-14	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2017/TPI2014	CSI. TC 0.47 BC 0.46 WB 0.35 Matrix-MS	DEFL. in (loc) Vert(LL) 0.22 8-10 Vert(CT) -0.36 8-10 Horz(CT) 0.04 6	>999 240 MT: >742 180 n/a n/a	ATES GRIP 20 244/190 ight: 105 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP M 31 WERS

2x4 SP No 3

REACTIONS. (lb/size) 2=1143/0-3-8, 6=1143/0-3-8

Max Horz 2=88(LC 11)

Max Uplift 2=-267(LC 12), 6=-267(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1913/1009, 3-4=-1770/1009, 4-5=-1770/1009, 5-6=-1913/1009 TOP CHORD

BOT CHORD 2-10=-745/1655, 8-10=-396/1111, 6-8=-765/1655

WEBS 4-8=-406/757, 5-8=-268/285, 4-10=-406/757, 3-10=-268/285

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2 and 267 lb uplift at
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20



Structural wood sheathing directly applied or 4-0-8 oc purlins.

Rigid ceiling directly applied or 8-9-10 oc bracing.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

November 22,2019

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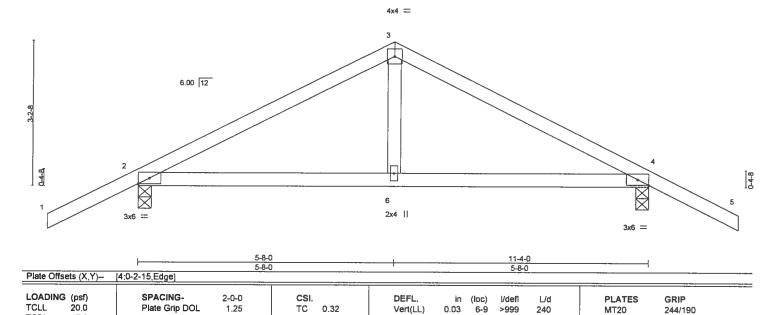
ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandra, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

	lob	Truss	Truss Type	Qty	Ply	CHEMERY CONST - LOT 7 FWS	
1.	105.400	<u> </u>				h.	T18737156
- 1	2135432	T04	Common	3	1		
L		<u> </u>				Job Reference (optional)	
	Builders FirstSource, J	acksonville, FL - 32244,			8,240 s Ju	I 14 2019 MiTek Industries, Inc. Fri Nov 22	12 22 08 2019 Page 1
				5lppGi3ml)	(fNNmnlqF	REYFDz9Eln-9NCuRLtYw8zRYiqR631fTrJA	Ak4xm6lJKn7PiDvyGdoi
	-2-0-0	1	5-8-0		•	11-4-0	13-4-0
	2-0-0	1	5-8-0			5-8-0	2-0-0

Scale = 1.24.6



-0.05

0.01

6-12

>999

n/a

180

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 47 lb

FT = 20%

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

7.0

0.0

10.0

REACTIONS. (lb/size) 2=527/0-3-8, 4=527/0-3-8

Max Horz 2=81(LC 16)

Max Uplift 2=-228(LC 12), 4=-228(LC 13)

Lumber DOL

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-543/294, 3-4=-543/294 BOT CHORD 2-6=-102/430, 4-6=-102/430

WEBS 3-6=0/251

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

вс 0.32

WB 0.10

Matrix-MS

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 2 and 228 lb uplift at ioint 4.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

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Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	1
2135432	T04G	Common Supported Gable	1	1	T18737157	
					Job Reference (optional)	
Builders FirstSource, Ja	cksonville, FL - 32244,			8 240 s Ju	Il 14 2019 MiTek Industries, Inc. Fri Nov 22 12 22 10 2019 Page 1	_
			ID:5lppGi3mlXfNNrr	nlqREYF	Dz9Eln-5lKes1vpSmD9n?_qEU37YGOXMuhzagTdFQupHoyGdoh	
-2-0-0		5-8-0			11-4-0	
2-0-0	,	5-8-0			5-8-0 2-0-0	

Scale = 1.25 4

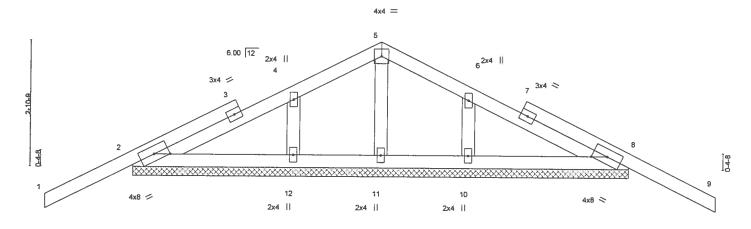


Plate Offsets (X,Y)-	[2:0-4-0,0-1-15], [8:0-4-0,0-1-15]		11-4-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.31 BC 0.09 WB 0.05 Matrix-S	DEFL. in (loc) I/defl L/d PLATES GRIP Vert(LL) -0.02 9 n/r 120 MT20 244/190 Vert(CT) -0.02 9 n/r 120 Horz(CT) 0.00 8 n/a n/a Weight: 56 lb FT = 20%

11-4-0

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 OTHERS

2x4 SP No.3

REACTIONS. All bearings 11-4-0.

(lb) - Max Horz 2=47(LC 11) Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 12, 10

Max Grav All reactions 250 lb or less at joint(s) 11, 12, 10 except 2=258(LC 23), 8=258(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Extenor(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 10.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTe& connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see

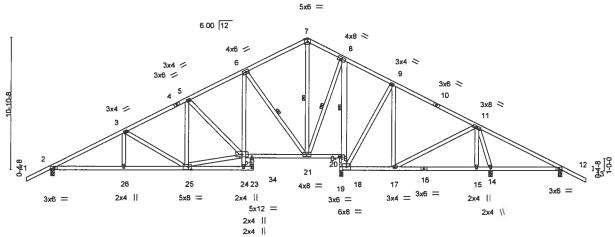
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 216 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Job	Trus	S		Tn.	ıss Typ	•				Qty	Ply	CHEMERY CO	NST LOT 7 FWS		
2135432	T05			Ro	of Spec	ial				1		1			T18737158
											1	Job Reference	(optional)		
Builders FirstSource,	Jacks	onville, FL	- 32244,										Industries, Inc. Fr		
									ID.	5lppGi3n	iXfNNm	nlqREYFDz9Eln-S	7XvlzxGlrRunso11	IfIFK5LCvGrFphM	Oibay?yGdoc
	-0-0	6-0-0		11-2-4		16-0-0	1	21-0-0	23-8-8	28-1-12	! .	35-0-0	42-0-0	144-0-0	
12	-0-0 '	6-0-0		5-2-4	-1	4-9-12	- 1	5-0-0	2-8-8	4-5-4	,	6-10-4	7-0-0	2-0-0	

Scale = 1:91.1



		6-0-0	11-2-4	16-0-0	16-8-0	21-0-0	23-10-4 23-8-8	28-1-12	-	35-0-0	36-3-8	42-0-0	
Plate Offse	ets (X,Y)-	[12:0-2-15,Edge], [25:0-2	5-2-4 -12,0-3-0]	4-9-12	018-0	4-4-0	1 2-8-8 0-1112	4-3-8		6-10-4	1-3-81	5-8-8	
LOADING	(psf)	SPACING-	2-0-0	CSI.			DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.50		Vert(LL)	0.10	25-26	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.62		Vert(CT)	-0.10	25-26	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.57	1	Horz(CT)	0.02	12	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS							Weight: 291 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

6-24: 2x4 SP No.3, 8-18: 2x6 SP No.2

WEBS 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins. Rigid ceiling directly applied or 4-9-15 oc bracing. Except:

1 Row at midpt

6-0-0 oc bracing: 18-20

10-0-0 oc bracing: 22-24

WEBS 1 Row at midpt 6-21, 7-21, 8-21

REACTIONS. All bearings 0-3-8.

(lb) - Max Horz 2=242(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-427(LC 9), 18=-527(LC 9), 14=-317(LC 13), 12=-189(LC

13)

Max Grav All reactions 250 lb or less at joint(s) except 2=923(LC 1), 18=1593(LC 1), 14=589(LC 24), 12=321(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1389/1599, 3-5=-949/1181, 5-6=-636/889, 6-7=-175/426, 7-8=-172/441, 8-9=32/395, 9-11=202/278

BOT CHORD 2-26=-1369/1186, 25-26=-1369/1186, 6-22=-729/579, 21-22=-364/581, 20-21=-242/482,

18-20=-1181/1081, 8-20=-1138/1011, 17-18=-98/255

3-25=-458/609, 5-25=-312/241, 22-25=-785/776, 5-22=-386/556, 6-21=-766/945, WEBS

8-21=-764/881, 9-18=-457/386, 9-17=-101/298, 11-14=-478/272

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Extenor(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2, 527 lb uplift at joint 18, 317 lb uplift at joint 14 and 189 lb uplift at joint 12.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

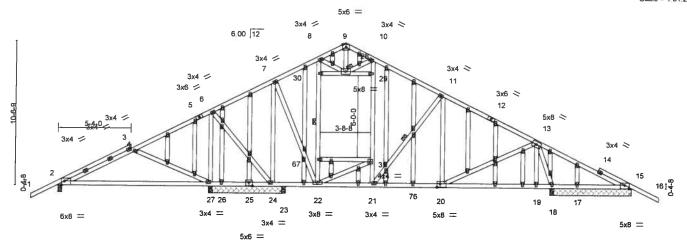
November 22,2019

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Job Truss Truss Type Qty Ply CHEMERY CONST. - LOT 7 FWS T18737159 2135432 T05G GABLE Job Reference (optional) 8 240 s Jul 14 2019 MiTek Industries Inc. Fn Nov 22 12 22 20 2019 Page 1 ID.5lppGi3mlXfNNmnlqREYFDz9Eln-ogxQyS045qTk_YklpbETyNoBswwfw3L5Y_JLeDyGdoX Builders FirstSource Jacksonville, FL - 32244 21-0-0 23-0-0 25-10-14 28-1-1 2-0-0 2-0-0 2-10-14 2-2-14 15-10-4

Scale = 1 81.2



2000	5-8-0 5-8-0	11-0-8 5-4-8	11-9-4 0-1 ¹ / ₁ 2 2-8-0	15-10-4	19-0-0 2-7-8	21-0-0 23-0-0	28-1-12 5-1-12		35-0-0 6-10-4	36-0-9	42-0-0
Plate Offsets (X,Y)-	[2:0-2-13,Edge], [15:0-4-0	,0-3-1], [20	0:0-2-12,0-3-0],				-0]		0-10-4	1-0-0	6-0-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TP	2-0-0 1.25 1.25 YES PI2014	CSI. TC BC WB Matr	0.57 0.83 0.54 ix-MS		DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.27 27-70 -0.36 27-70 0.02 18	l/defi >488 >366 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 3	244/190
LUMBER-						BRACING-					

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 **WEBS** 2x4 SP No.3 OTHERS 2x4 SP No.3 **BRACING-**TOP CHORD **BOT CHORD WEBS JOINTS**

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing. 1 Row at midpt 11-21, 8-22

1 Brace at Jt(s): 28

REACTIONS. All bearings 0-3-8 except (jt=length) 24=5-7-8, 27=5-7-8, 27=5-7-8, 17=6-0-0, 15=6-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0, 26=5-7-8, 27=5-0-0

Max Horz 2=235(LC 16) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 17, 15, 23 except 2=229(LC 8), 24=227(LC 12), 27=482(LC 9),

26=-361(LC 3), 18=-616(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 17, 15, 26, 23, 15 except 2=585(LC 1), 24=420(LC 23), 27=1131(LC 1), 27=1131(LC 1), 18=1312(LC 1), 18=1312(LC 1)

FORCES. (lb) - Max, Comp./Max, Ten. - All forces 250 (lb) or less except when shown 2-4=-618/539, 4-6=-307/239, 6-7=-468/390, 7-8=-555/505, 8-9=-355/342, 9-10=-342/340, 10-11=-658/526, 11-13=-794/483, 13-15=-356/621 TOP CHORD

BOT CHORD 2-27=-361/554, 23-24=0/383, 22-23=0/383, 21-22=0/564, 20-21=-92/631,

17-18=-487/446, 15-17=-487/446

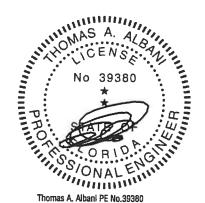
9-28=-259/235, 13-20=-292/703, 13-19=0/266, 7-22=-59/271, 6-24=-95/318, 6-27=-629/358, 4-27=-434/515, 7-24=-485/261, 13-18=-1403/774, 21-31=-116/251,

10-28=-373/338

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 15, 23, 15 except (jt=lb) 2=229, 24=227, 27=482, 26=361, 18=616.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd

Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
2135432	то6	Roof Special	3	1	T1873	37160
2.00.02	100	Trool openial			Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244			8.240 s Ju	ul 14 2019 MiTek Industries, Inc. Fri Nov 22 12 22 28 2019 Page	1
			ID 5lppGi3mIXfNNmn	IqREYFDz	9EIn-ZDQSeB75CIUbynMHHGOLH38XP8gKocZGOEFmwlyGdo	P

28-1-12

21-0-0

44-0-0 6-10-4

42-0-0

Structural wood sheathing directly applied or 3-0-1 oc purlins.

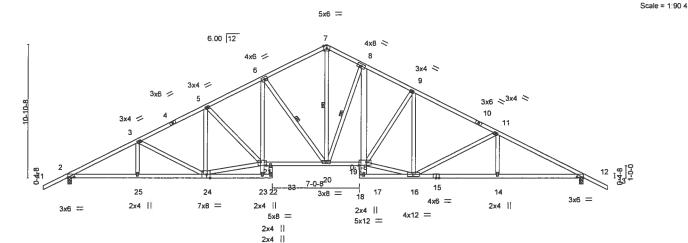
6-20, 7-20, 8-20

Rigid ceiling directly applied or 5-3-4 oc bracing. Except:

10-0-0 oc bracing: 21-23, 17-19

1 Row at midpt

35-0-0



		5-8-0 5-8-0	11-2-12 5-6-12	16-0-0 16-8- 4-9-4 0-8-	 23-8-8	28-1-12 4-5-4	35-0-0 6-10-4		42-0-0 7-0-0	
Plate Offse	ets (X,Y)-	[2:0-6-0,0-0-3], [12:0-6-0			 	4-0-4	0-10-4		7-0-0	
LOADING TCLL	20.0	SPACING- Plate Grip DOL	2-0-0 1.25	CSI. TC 0.6	 DEFL. Vert(LL)	in (loc) 0.24 22	>999	L/d 240	PLATES MT20	GRIP 244/190
TCDL BCLL BCDL	7.0 0.0 * 10.0	Lumber DOL Rep Stress Incr Code FBC2017/T	1.25 YES Pl2014	BC 0.73 WB 0.84 Matrix-MS	Vert(CT) Horz(CT)	-0.44 20-21 0.18 12	>999 n/a	180 n/a	Weight: 291 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 *Except*

6-23: 2x4 SP No.3, 8-17: 2x6 SP No.2

2x4 SP No.3

(lb/size) 2=1675/0-3-8, 12=1672/0-3-8 REACTIONS.

Max Horz 2=-242(LC 13)

Max Uplift 2=-645(LC 12), 12=-646(LC 13)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-3=-3037/1569, 3-5=-2617/1421, 5-6=-2541/1422, 6-7=-2000/1214, 7-8=-1954/1229,

8-9=-2296/1349, 9-11=-2433/1347, 11-12=-2991/1550 **BOT CHORD**

11-2-12

5-6-12

16-0-0

2-25=-1234/2659, 24-25=-1234/2659, 6-21=-310/671, 20-21=-856/2228, 19-20=-714/2003,

8-19=-413/715, 14-16=-1233/2609, 12-14=-1233/2609 **WEBS** 3-24=-436/313, 21-24=-946/2217, 6-20=-840/552, 7-20=-896/1493, 8-20=-766/512,

16-19=-815/1976, 9-19=-276/297, 11-16=-583/417, 11-14=0/286

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=645, 12=646.

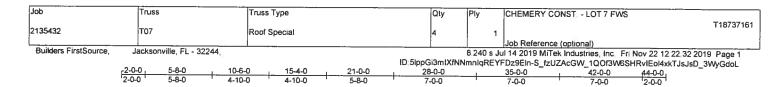


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This fall incl. Local miles Reference Page Miles 13 to 1, 100 and 1, 100 at 1





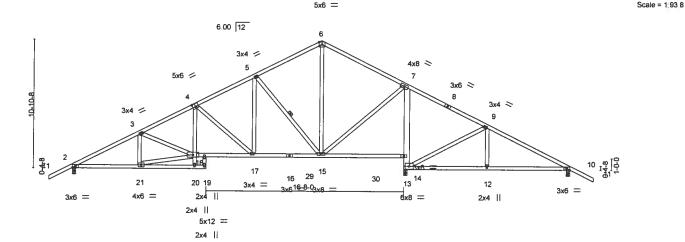


Plate Offsets (X,Y)-		1-4-0 15-4-0 21-0-0 0-10-0 4-0-0 5-8-0	28-0-0 28-1 _F 12 35-0-0 7-0-0 0-1-12 6-10-4	42-0-0 7-0-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.57 BC 0.61 WB 0.67 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) -0.08 19 >999 240 Vert(CT) -0.16 17-18 >999 180 Horz(CT) 0.02 13 n/a n/a	PLATES GRIP MT20 244/190 Weight: 257 lb FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

BOT CHORD

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD

4-20: 2x4 SP No.3, 7-13: 2x6 SP No.2

WEBS 2x4 SP No.3

(lb/size) 2=1087/0-3-8, 13=1797/0-3-8, 10=459/0-3-8

Max Horz 2=-156(LC 10)

Max Uplift 2=-268(LC 12), 13=-281(LC 12), 10=-188(LC 13) Max Grav 2=1087(LC 1), 13=1797(LC 1), 10=518(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-1753/975, 3-4=-1635/958, 4-5=-1104/756, 5-6=-553/542, 6-7=-580/530, 7-9=0/427, 9-10=-473/319

2-21=-696/1513, 4-18=-147/439, 13-14=-1462/679, 7-14=-1388/694, 12-13=-123/361,

10-12=-123/361, 17-18=-571/1425, 15-17=-265/965, 14-15=-323/289

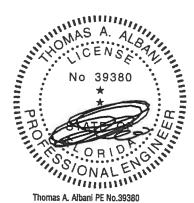
18-21=-654/1423, 4-17=-636/407, 5-15=-795/532, 7-15=-259/981, 9-13=-611/420,

9-12=0/318. 5-17=-247/573

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ri=lb) 2=268, 13=281, 10=188.



Structural wood sheathing directly applied or 4-2-11 oc purlins.

Rigid ceiling directly applied or 5-8-7 oc bracing. Except:

10-0-0 oc bracing: 18-20

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

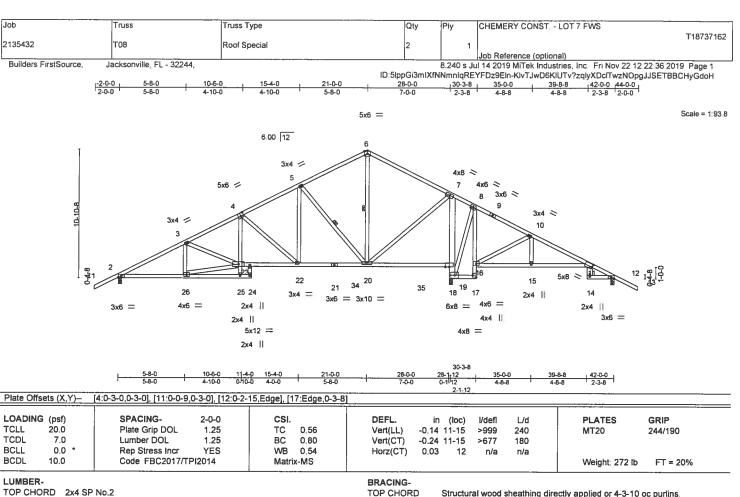
November 22,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and inidividual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Practing individual to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPT! Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandra, VA 22314



6904 Parke East Blvd Tampa, FL 36610



BOT CHORD

WEBS

REACTIONS.

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

4-25,8-17: 2x4 SP No.3, 7-18,11-14: 2x6 SP No.2

WEBS 2x4 SP No.3

(lb/size) 2=1041/0-3-8, 18=1938/0-3-8, 12=368/0-3-8

Max Horz 2=156(LC 11)

Max Uplift 2=-271(LC 12), 18=-309(LC 13), 12=-154(LC 13) Max Grav 2=1059(LC 23), 18=1938(LC 1), 12=387(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - Ali forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1692/959, 3-4=-1560/938, 4-5=-1033/736, 5-6=-483/523, 6-7=-465/509, 7-8=0/611,

8-10=0/521, 10-11=-146/298

BOT CHORD 2-26=-681/1459, 4-23=-146/434, 18-19=-1538/667, 7-19=-1460/681, 17-18=-389/271, 8-16=-150/300, 22-23=-534/1356, 20-22=-224/900, 19-20=-543/372

WEBS 23-26=-640/1374, 4-22=-629/406, 5-22=-247/570, 5-20=-794/531, 7-20=-327/1129, 8-18=-369/243, 10-16=-553/374

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=271, 18=309, 12=154,



Structural wood sheathing directly applied or 4-3-10 oc purlins,

5-20, 6-20

Rigid ceiling directly applied or 5-4-8 oc bracing. Except:

10-0-0 oc bracing: 23-25

1 Row at midpt

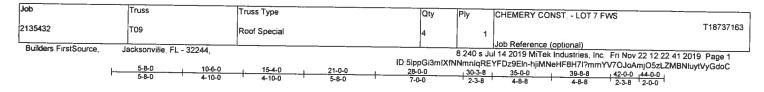
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

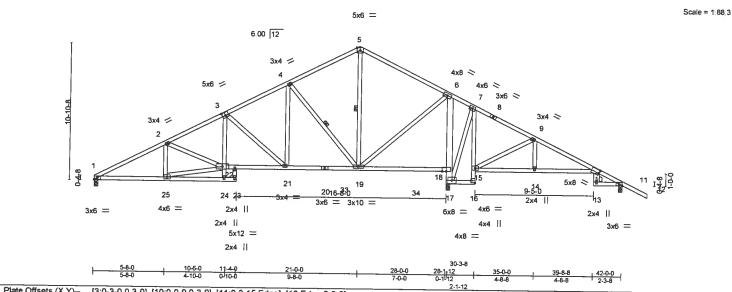
November 22,2019

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTel® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see.

ANTIFY! Quality Criteria, DSB-89 and BCSI Building Composition available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







LOADING (psf) TCIL 20.0 TCDL 7.0 BCIL 0.0 BCOL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.56 BC 0.81 WB 0.55	, ,	in (loc) -0.14 10-14 -0.24 10-14 0.03 11	l/defi >999 >677 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
	00de 1 BC2017/1F12014	Matrix-MS					Weight: 268 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER.

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

3-24,7-16: 2x4 SP No.3, 6-17,10-13: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 1=929/0-3-8, 17=1942/0-3-8, 11=368/0-3-8

Max Horz 1=-159(LC 10)

Max Uplift 1=-232(LC 12), 17=-309(LC 13), 11=-154(LC 13) Max Grav 1=947(LC 23), 17=1942(LC 1), 11=387(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1731/994, 2-3=-1577/953, 3-4=-1041/743, 4-5=-486/526, 5-6=-468/512, 6-7=0/611,

7-9=0/521, 9-10=-145/297 **BOT CHORD**

1-25=-719/1499, 3-22=-160/438, 17-18=-1542/675, 6-18=-1465/688, 16-17=-390/272.

7-15=-149/300, 21-22=-549/1371, 19-21=-232/905, 18-19=-543/373

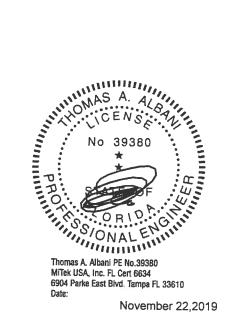
WEBS 22-25=-683/1418, 3-21=-639/415, 4-21=-253/575, 4-19=-801/538, 6-19=-335/1133,

7-17=-369/243, 9-15=-553/373

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=232, 17=309, 11=154.



Structural wood sheathing directly applied or 4-2-0 oc purlins.

4-19, 5-19

Rigid ceiling directly applied or 5-4-6 oc bracing. Except:

10-0-0 oc bracing: 22-24

1 Row at midpt

November 22,2019

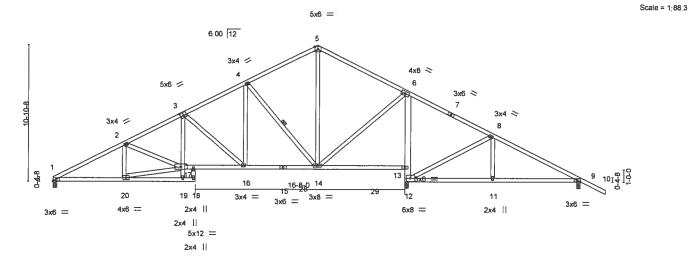
📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Suite 312, Alexandria. VA 22314



6904 Parke East Blvd Tampa, FL 36610

Job	Truss		Truss Type		Qty	Ply	CHEMERY CO	NST LOT 7 FWS		
	L									T18737164
2135432	T10		Roof Special		5	1 1				
			<u> </u>				Job Reference (optional)		
Builders FirstSource,	Jacksonville, F	FL - 32244,				8,240 s Ju	114 2019 MiTek	Industries, Inc. Fri I	Nov 22 12 22 44	2019 Page 1
					ID 5lppGi3mlXfNi	NmnlqREYF	Dz9Eln-5IOV?f.	J8RCVKsEaMDdg5v	wRoHmbAgYuld	3j7cUqyGdo9
	5-8-0	10-6-0	15-4-0	, 21-0-0	28-0-0		35-0-0	42-0-0	44-0-0	
	5-8-0	4-10-0	4-10-0	5-8-0	7-0-0	1	7-0-0	7_0_0	2-0-0	



			U-0-U 11-4-U	10-4-0	21-0-0	28-0-0	28-1 ₁ 12	35-0-0	1	42-0-0	
		<u>' 5-8-0 ' 4-</u>	<u>-10-0 0-10-b</u>	4-0-0	5-8-0	7-0-0	0-1-12	6-10-4	1	7-0-0	
Plate Offs	sets (X,Y)-	[3:0-3-0,0-3-0], [9:0-2-15	,Edge)								
				1						1	· :
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.57	Vert(LL)	-0.09 18	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.61	Vert(CT)	-0.16 16-17	>999	180		
BCLL	0.0 °	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.02 12	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	-MS					Weight: 253 lb	FT = 20%
						1				Traight 200 ib	2370

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 *Except*

3-19: 2x4 SP No.3, 6-12: 2x6 SP No.2

WEBS

(lb/size) 1=975/0-3-8, 12=1802/0-3-8, 9=459/0-3-8 Max Horz 1=159(LC 10)

Max Uplift 1=-229(LC 12), 12=-283(LC 12), 9=-188(LC 13) Max Grav 1=975(LC 1), 12=1802(LC 1), 9=518(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1792/1010, 2-3=-1651/973, 3-4=-1111/762, 4-5=-555/544, 5-6=-582/532,

6-8=0/428, 8-9=-473/317 1-20=-757/1552, 3-17=-162/443, 12-13=-1468/688, 6-13=-1393/703, 11-12=-120/361,

9-11=-120/361, 16-17=-596/1439, 14-16=-276/970, 13-14=-325/291

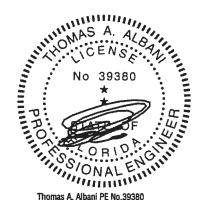
17-20=-718/1467, 3-16=-646/418, 4-16=-255/578, 4-14=-802/540, 6-14=-268/985,

8-12=-611/420, 8-11=0/318

NOTES.

WEBS

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=229, 12=283, 9=188,



Structural wood sheathing directly applied or 4-1-0 oc purlins.

Rigid ceiling directly applied or 5-8-5 oc bracing. Except:

10-0-0 oc bracing: 17-19

1 Row at midpt

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

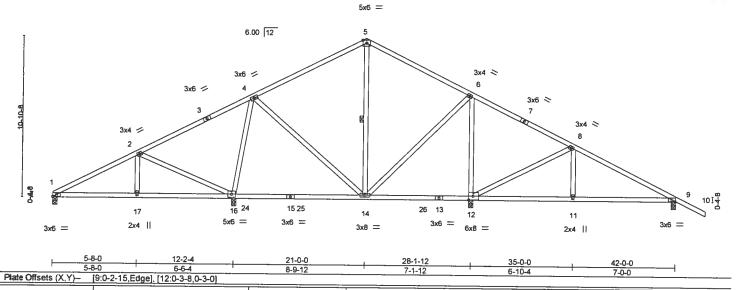
🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTeM® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N Lee Street, Saile 312. Alexandra, VA 22314



Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS		* "
2135432	T11	Common	2	1	27		T18737165
Builders FirstSource.	Jacksonville, FL - 32244				Job Reference (optional)	V. Zź	
,	ID 5lppGi3mIXfNNmnlqREYFDz9Ein-W33ddhLQK7ivih.ixvmDoY30m0C7ME93bil C59cdc45						
<u>5-8-0</u> 5-8-0	13-4-0 7-8-0	21-0-0 7-8-0	28-1-12 7-1-12		35-0-0 6-10-4	42-0-0 7-0-0	44-0-0 2-0-0

Scale = 1.74.9



LUMBER-

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2x4 SP No.2 2x4 SP No.2

BOT CHORD WEBS 2x4 SP No.3

20.0

7.0

0.0

10.0

BRACING-

DEFL

Vert(LL)

Vert(CT)

Horz(CT)

in (loc)

-0.14 14-16

-0.25 14-16

0.01

TOP CHORD BOT CHORD WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

PLATES

Weight: 230 lb

MT20

GRIP

244/190

FT = 20%

Rigid ceiling directly applied or 6-0-0 oc bracing.

L/d

240

180

n/a

1 Row at midpt 5-14

I/defl

>999

>756

n/a

REACTIONS. All bearings 0-3-8.

(lb) - Max Horz 1=-262(LC 17)

Max Uplift All uplift 100 b or less at joint(s) except 1=-144(LC 9), 12=-475(LC 13), 16=-539(LC 12), 9=-272(LC 13)

CSI.

TC

вс

WB 0.72

Matrix-MS

0.67

0.52

Max Grav All reactions 250 lb or less at joint(s) except 1=355(LC 23), 12=1171(LC 1), 16=1154(LC 1), 9=551(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-456/483, 2-4=-163/316, 4-5=-266/259, 8-9=-538/517 **BOT CHORD**

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

1-17=-298/380, 16-17=-298/380, 14-16=-92/361, 12-14=-147/424, 11-12=-317/420, 9-11=-317/420

WEBS

6-14=-75/348, 6-12=-766/509, 8-12=-596/805, 8-11=-307/296, 4-16=-765/533,

2-0-0

1.25

1.25

YES

2-16=-588/797, 2-17=-275/235

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 1, 475 lb uplift at joint 12, 539 lb uplift at joint 16 and 272 lb uplift at joint 9.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the tabrication, storage, delivery, erection and bracing of trusses and truss systems, see

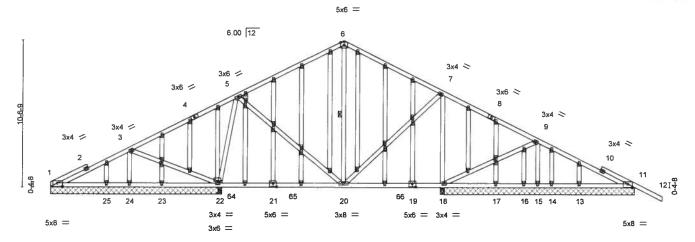
ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

Jc	b	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	\neg
21	35432	T11G	GABLE	1	1	T187371	66
			OABEC		'	Job Reference (optional)	
- 1	Builders FirstSource, J	lacksonville, FL - 32244,			8.240 s Ju	il 14 2019 MiTek Industries, Inc. Fri Nov 22 12 22 52 2019 Page 1	
				ID:5lppGi3mIXfNN	ImnlqREY	FDz9Eln-sqtWgOP9ZgVBqTBvhJpzF77depvoQXlovz31mMyGdo1	
	<u> </u>	5-8-0 13	4-0 21-0-0	28-1-1	2	35-0-0 42-0-0 44-0-0	
		5-8-0 7-	3-0 7-8-0	7-1-12	!	6-10-4 7-0-0 2-0-0	

Scale = 1 79 7



		5-8-0	12-2-4	12-4-0	21-0-0			28-1 _r 12	35-0-0	42-0-0	
	'	5-8-0 '	6-6-4	0-1 <u>'</u> 12	8-8-0	· · · · · · · · · · · · · · · · · · ·	7-0-0	0-1-12	6-10-4	7-0-0	
Plate Offse	ets (X,Y)-	[1:0-4-0,0-3-1], [11:0-4-0	<u>,0-3-1], [19:0</u>	-3-0,0-3-0], [21	(0-3-0,0-3-0),	[22:0-2-4,0-0-4],	[30:0-1-14,0-	1-0], [41:0-	1-12,0-1-0]		
LOADING TCLL TCDL BCLL	(psf) 20.0 7.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC BC WB	0.64 0.53 0.60	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.12 20-22 -0.23 20-22 0.01 11	>999 >831	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	()	3			Weight: 359 lb	FT = 20%

LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS **OTHERS** 2x4 SP No.3

BRACING. TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 6-20

REACTIONS. All bearings 14-0-0 except (it=length) 1=12-4-0, 24=12-4-0, 22=12-4-0, 22=12-4-0, 25=12-4-0, 23=12-4-0, 1=12-4-0.

(lb) - Max Horz 1=-256(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 1, 14, 16, 17, 25, 23 except 24=-232(LC 12), 22=-359(LC 12),

18=-318(LC 13), 15=-329(LC 13), 11=-122(LC 13), 13=-114(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 14, 16, 17, 25, 23, 1 except 24=375(LC 23),

22=847(LC 2), 22=829(LC 1), 18=767(LC 2), 18=762(LC 1), 15=512(LC 1), 11=254(LC 1), 11=254(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 5-6=-370/345, 6-7=-361/348 **BOT CHORD** 18-20=-63/252

3-24-364/287, 5-22-677/436, 7-20-56/298, 7-18-700/457, 9-15-464/376

WEBS NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 14, 16, 17, 25, 23, 1 except (jt=lb) 24=232, 22=359, 18=318, 15=329, 11=122, 13=114, 11=122.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

November 22,2019

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and in individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property incorporate this design into the overall building designer must verify the applicability of design parameters and property designers and property and property and property and property and property and property designers and property and property designers and property and property and property designers and property and property

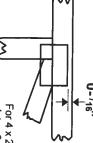


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹16" from outside edge of truss.

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This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE



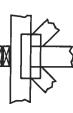
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



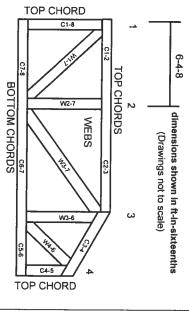
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards: ANSI/TPI1: National D

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing. Building Component Safety Information

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet; MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

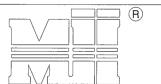
- . Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MiTek USA, Inc. Page 1 of 1

Brace Size

MII-T-BRACE 2



MiTek USA, Inc.



Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

V mill of Villimite							
Nailing Pattern							
T-Brace size	Nail Size	Nail Spacing					
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.					

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

	for One	for One-Ply Truss				
	Specified Continuous Rows of Lateral Bracing					
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

Nails	
WEB	_ SPACING
	T-BRACE
Nails Section Detail T-Brace Web	

Me	Web	
Nails Web	I-Brace	
Nails		

		Brace Size for Two-Ply Truss					
		Continuous Iteral Bracing					
Web Size	1	2					
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace					
2x6	2x6 T-Brace	2x6 I-Brace					
2x8	2x8 T-Brace	2x8 I-Brace					

T-Brace / I-Brace must be same species and grade (or better) as web member.



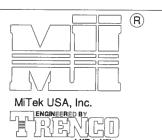
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SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1



Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.

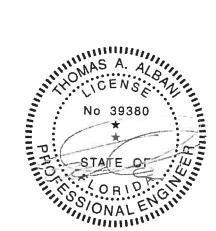
MAXIMUM WEB AXIAL FORCE = 2500 lbs MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE MINIMUM WEB GRADE OF #3

Nails Section Detail

Scab-Brace

Scab-Brace must be same species grade (or better) as web member.

Web

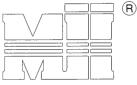


Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

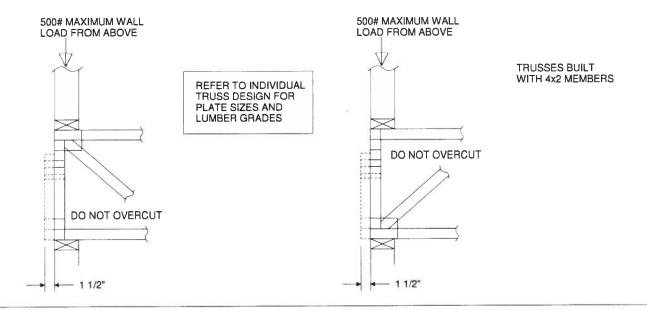
MII-REP05

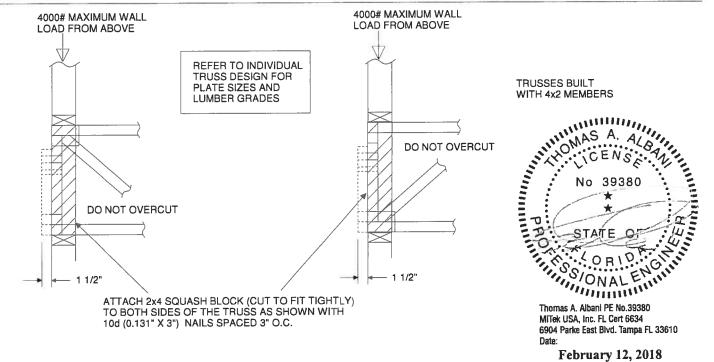
MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY 컰 A MITek Affiliate

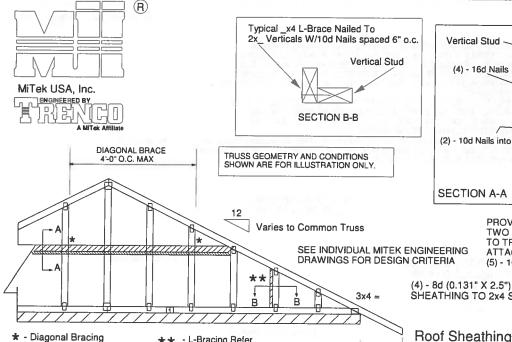
- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION, THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED. THE LOADS INDICATED.
- ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY.
- 6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





Standard Gable End Detail

MII-GE130-D-SP



- L-Bracing Refer

to Section B-B

MiTek USA, Inc. Page 1 of 2 Vertical Stud DIAGONAL BRACE (4) - 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2x4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

NAILS

1'-3"

Max.

24" Max

Refer to Section A-A

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT,
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG, ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

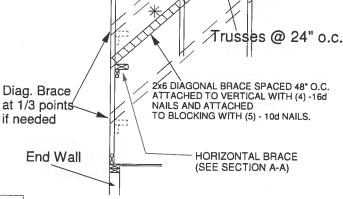
10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

MALES DESIGNATED TO ARE (0.131 X 3.3)									
Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7			
2x4 SP No. 3 / Stud		3-5-4	3-6-8	5-0-2	6-10-8	10-3-13			
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPI ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.





(2) - 10d NAILS

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

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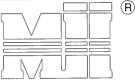
- L-Bracing Refer

to Section B-B

MII-GE130-SP

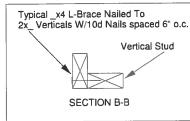
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Page 1 of 2



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ENGINEERED BY 집회



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

Vertical Stud DIAGONAL (4) - 16d Nails BRACE 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d NAILS

Roof Sheathing

1'-3"

Max.

End Wall

Refer to Section A-A NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

STACING OF ROOF STATEM.

A: "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" C DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A DIAGRAM OF THE STATEMENT O

DIAGONAL BRACE 4'-0" O.C. MAX

- Diagonal Bracing

2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

S. ID	24" Max	
o.c.		
	Diag. Bra at 1/3 po if needed	ints

3x4 =

2x6 DIÀGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

(2) - 10d NAILS

∕Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
	Maximum Stud Length						
12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6		
16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1		
24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15		
	12" O.C. 16" O.C.	Spacing Brace 12" O.C. 4-0-7 16" O.C. 3-8-0	Spacing Brace L-Brace Maximul 12" O.C. 4-0-7 4-5-6 16" O.C. 3-8-0 3-10-4	Spacing Brace L-Brace L-Brace Maximum Stud Let 12" O.C. 4-0-7 4-5-6 6-3-8 16" O.C. 3-8-0 3-10-4 5-5-6	Stud Spacing Brace L-Brace L-Brace BRACE		

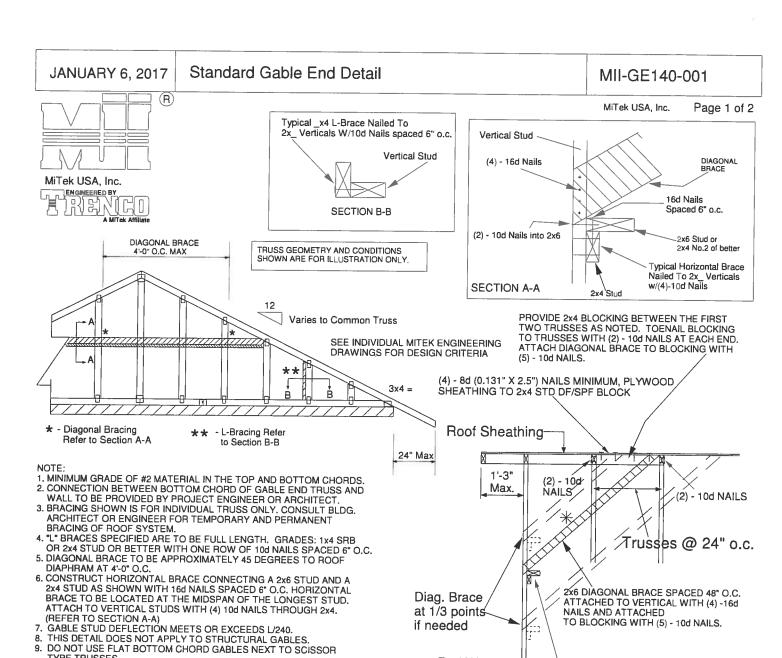
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



End Wall

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade	Maximum Stud Length					
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



HORIZONTAL BRACE

(SEE SECTION A-A)

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

January 19, 2018

(R)

Standard Gable End Detail

MII-GE170-D-SP



TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(5) - 10d NAILS.

SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD,

(2) - 100

NAILS

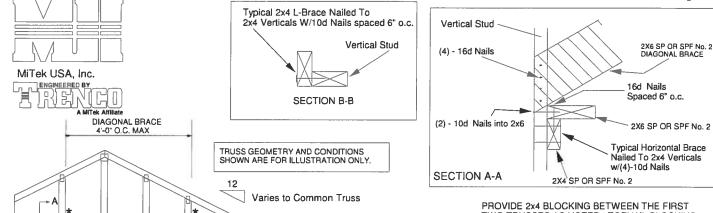
Page 1 of 2

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED

48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO



SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

24" Max

Diag. Brace

at 1/3 points

if needed

3x4 =

* - Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

NOTE

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
 CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

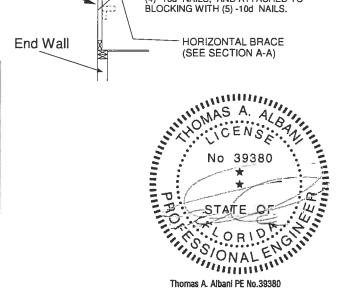
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIG	NATED 16	ARE (0.13	11" X 3.5")		
Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximum St	ud Length	
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

Standard Gable End Detail

MII-GE180-D-SP

Page 1 of 2

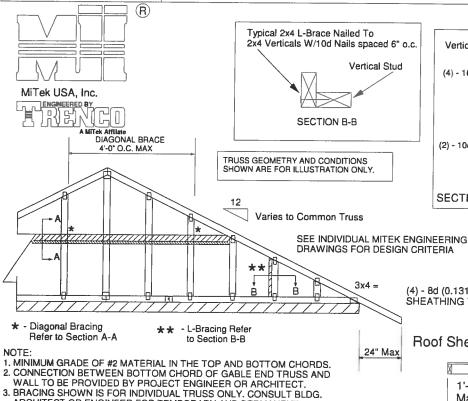
2X6 SP OR SPF No. 2

(2) - 10d NAILS

∕Trusses @ 24" o.c.

Typical Horizontal Brace Nailed To 2x4 Verticals

MiTek USA, Inc.



2X6 SP OR SPF No. 2 DIAGONAL BRACE 16d Nails Spaced 6" o.c.

w/(4)-10d Nails **SECTION A-A** 2X4 SP OR SPF No. 2 PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END.

ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Vertical Stud

(4) - 16d Nails

(2) - 10d Nails into 2x6

Roof Sheathing

1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

10d

NÁILS

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Diag. Brace at 1/3 points if needed	/
End Wall	

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

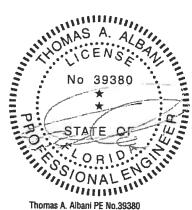
> HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Stud Size Spacing		Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximum St	ud Length	
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

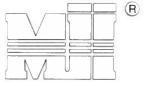
MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

ENGINEERED BY

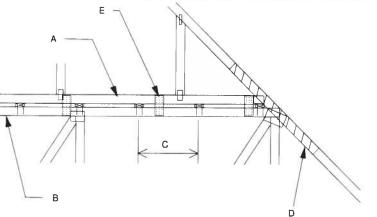
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C **ASCE 7-10 DURATION OF LOAD INCREASE: 1.60**

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) (0.131" X 3.5") TOS-NAILED.
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
 D 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
 ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
 IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
 DIRECTIONS AND:
- IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT, IN BOTH DIRECTIONS AND:

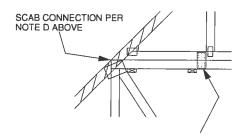
 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

 E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH
- MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

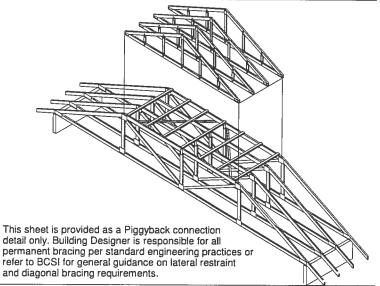


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

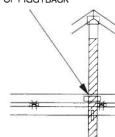
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48° O.C. W/ (4) (0.131° X 1.5°) PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5° EDGE DISTANCE.



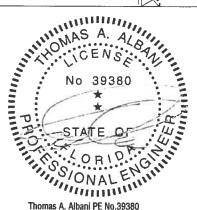
VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- AS SHOWN IN DETAIL.

 ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- (MINIMUM 24)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
 FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS.
- NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10

DURATION OF LOAD INCREASE: 1.60 DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

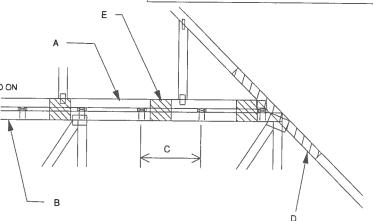


A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (00.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X _ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN OR

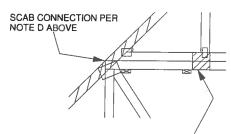
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAX PIGGYBACK SPAN OF 12 ft. - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

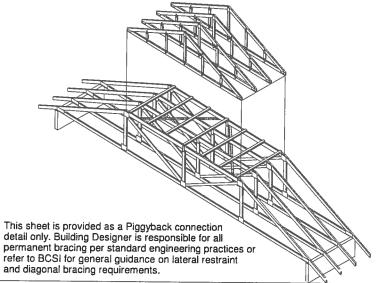


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

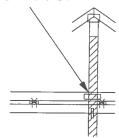
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

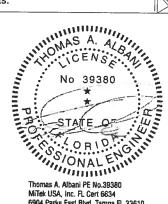


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DELAIL.
ATTACH 2 x _ x 4'-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.
FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

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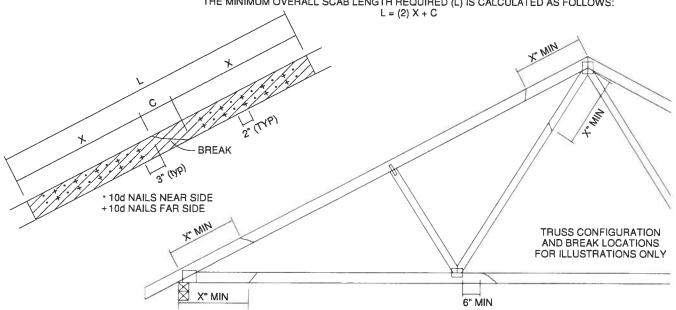


	OTAL NUMBER OF		MAXIMUM FORCE (Ibs) 15% LOAD DURATION									
NAILS EACH SIDE OF BREAK *		X INCHES	S	SP		DF		PF	HF			
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6		
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028		
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608		
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187		
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767		
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347		

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID

- THE END DISTANCE, EDGE DISTANCE AND SPACING OF INAILS SHALL BE GOOD AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.
 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610

January 19, 2018

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

MiTek USA, Inc.

Page 1 of 1



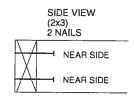
MiTek USA, Inc.

NOTES:

- TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



OE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) SPF-S DIAM. HF SPF .131 88.0 80.6 69.9 68.4 59.7 3.5" LONG .135 93.5 85.6 74.2 72.6 63.4 108.8 99.6 86.4 84.5 73.8 .162 3.25" LONG .128 74.2 57.6 67.9 50.3 58.9 75.9 69.5 60.3 59.0 51.1 .131 81.4 64.6 63.2 .148 74.5 52.5

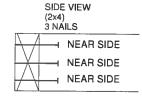
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

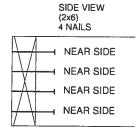
EXAMPLE:

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

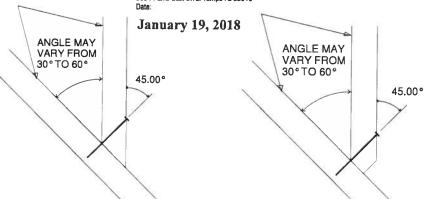
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

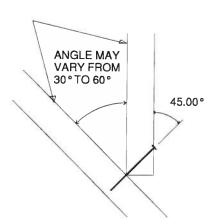






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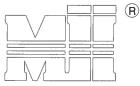


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1



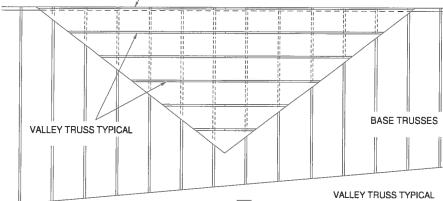
MiTek USA, Inc.

ENGINEERED BY RIST

GABLE END, COMMON TRUSS OR GIRDER TRUSS

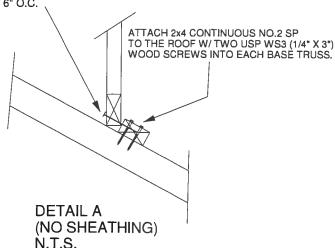
GENERAL SPECIFICATIONS

- NAIL SIZE 10d (0.131" X 3")
 WOOD SCREW = 3" WS3 USP OR EQUIVALENT
 DO NOT USE DRYWALL OR DECKING TYPE SCREW
 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
 SECULE VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 $\,$ 146 MPH WIND DESIGN PER ASCE 7-10 $\,$ 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

No 39380

STAILE OF THE STAIL OF THE

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

AUGUST 1, 2016 TRUSSED VALLEY SET DETAIL MII-VALLEY HIGH WIND2 R MiTek USA, Inc. Page 1 of 1 GENERAL SPECIFICATIONS 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES. MiTek USA, Inc. 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A GABLE END, COMMON TRUSS OR GIRDER TRUSS ENGINEERED BY 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS. 6. NAILING DONE PER NDS-01 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. BASE TRUSSES VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C. WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C ATTACH 2x4 CONTINUOUS NO.2 SP WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF TO THE ROOF W/ TWO USP WS45 (1/4" X 4.5") WOOD SCREWS INTO EACH BASE TRUSS. MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

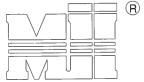
ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS45 (1/4" X 4.5") WOOD SCREWS INTO EACH BASE TRUSS.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

MiTek USA, Inc.

Page 1 of 1



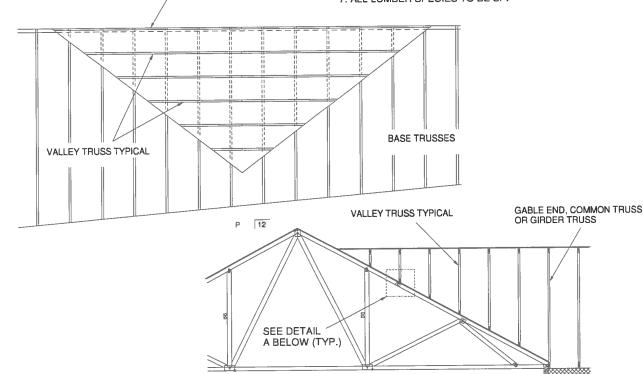
MiTek USA, Inc.

ENGINEERED BY

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



SECURE VALLEY TRUSS W/ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO 16d NAILS INTO EACH BASE TRUSS. **DETAIL A**

(MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B
WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF

ON THE TRUSSES

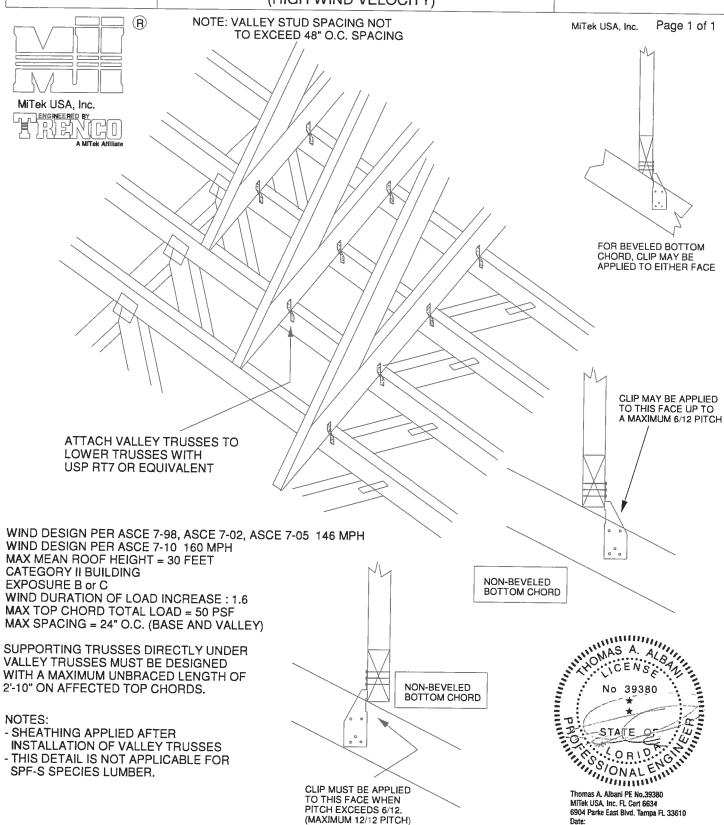
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TRUSSED VALLEY SET DETAIL (HIGH WIND VELOCITY)

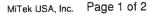
MII-VALLEY

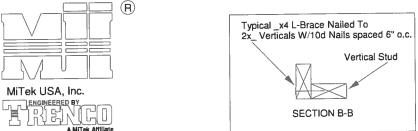


January 19, 2018

Standard Gable End Detail

MII-GE146-001





DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 =- Diagonal Bracing - L-Bracing Refer

to Section B-B

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails **SECTION A-A**

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

> > (2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

(2) - 10¢

NAILS

Roof Sheathing

1'-3"

Max.

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

A. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:

2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. 8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. 9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
Species and Grade		Maxin	num Stud L	ength			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1

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MiTek USA, Inc.

ENGINEERED BY

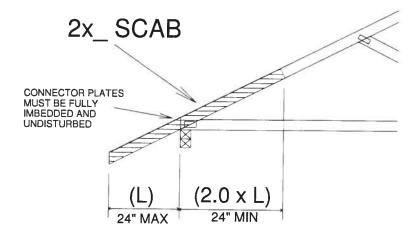
TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

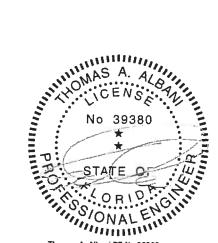
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

(B)

MiTek USA, Inc.

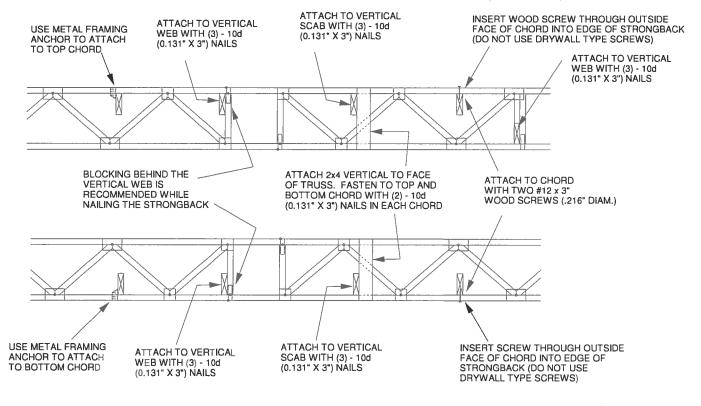
MiTek USA, Inc.

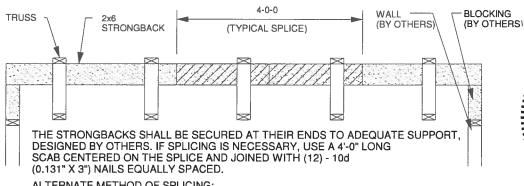
Page 1 of 1

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d
(0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

CP-CC-FWS-7 HVAC Load Calculations

for

Chemerys Construction 2025 NW County Road 236 High Springs, FL 32643



Prepared By:

Ken Fonorow Florida H.E.R.O., Inc. 15220 NW 5th Ave Newberry, Fl 32669 (352) 472-5661 Friday, November 8, 2019

Rhvac is an ACCA approved Manual J, D and S computer program.

Calculations are performed per ACCA Manual J 8th Edition, Version 2, and ACCA Manual D.

Rhvac - Residential & Light Commercial HVAC Loads

Florida H.E.R.O.

Newberry, FL 32669

Elite Software Development, Inc. CP-CC-FWS-7 Page 2

Project Report

General Project Information

Project Title: CP-CC-FWS-7
Designed By: Ken Fonorow
Project Date: 11/7/2019
Project Comment: Custom home

Client Name: Chemerys Construction
Client Address: 2025 NW County Road 236
Client City: High Springs, FL 32643
Client Phone: 352 222-6964

Client E-Mail Address: cjchemerys@hotmail.com
Company Name: Florida H.E.R.O., Inc.

Company Representative: Ken Fonorow
Company Address: 15220 NW 5th Ave
Company City: Newberry, Fl 32669
Company Phone: (352) 472-5661
Company E-Mail Address: ken@floridahero.com
Company Website: www.floridahero.com

Design Data

Reference City: Gainesville, Florida
Building Orientation: Front door faces South

Daily Temperature Range: Medium Latitude: 29 Degrees Elevation: 152 ft.

Altitude Factor: 0.995

	Outdoor	Outdoor	Outdoor	Indoor	Indoor	Grains
	<u>Dry Bulb</u>	Wet Bulb	Rel.Hum	<u>Rel.Hum</u>	Dry Bulb	<u>Difference</u>
Winter:	33	30.8	n/a	n/a	72	n/a
Summer:	92	77	51%	50%	75	52

Check FiguresTotal Building Supply CFM:1,000CFM Per Square ft.:0.535Square ft. of Room Area:1,867Square ft. Per Ton:873Volume (ft³):18,674

Building LoadsTotal Heating Required Including Ventilation Air:26,053Btuh26.053MBHTotal Sensible Gain:19,926Btuh78%

Total Latent Gain: 5,731 Btuh 22 %

Total Cooling Required Including Ventilation Air: 25,656 Btuh 2.14 Tons (Based On Sensible + Latent)

Notes

Rhvac is an ACCA approved Manual J, D and S computer program.

Calculations are performed per ACCA Manual J 8th Edition, Version 2, and ACCA Manual D.

All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads according to the manufacturer's performance data at your design conditions.

Rhvac - Residential & Light C Florida H.E.R.O. Newberry, FL 32669	ommercial h	IVAC Load	ls					Elite	Software Deve	Plopment, Inc CP-CC-FWS-7 Page 3
Miscellaneous Re	port									
System 1 Whole House Input Data				tdoor Bulb	Outdoor Wet Bulb		Outdoor el.Hum	Indoor Rel.Hum		Grains Difference
Winter: Summer:				33 92	30.8 77		80% 51%	n/a 50%	72	n/a 51.69
Duct Sizing Inputs										
	Main Trunk Yes Yes 0.15000				Runouts Yes Yes 0.15000					
Pressure Drop: Minimum Velocity: Maximum Velocity:	0.1000 650 900	in.wg./10 ft./min ft./min	00 ft.		0.1000 450 750	ft./mi	n			
Minimum Height: Maximum Height:	0	in. in.				in. in.				
Outside Air Data										
Infiltration Specified:			AC/hr CFM				AC/hr CFM			
Infiltration Actual: Above Grade Volume:		18,674 5,416	AC/hr Cu.ft. Cu.ft./h	nr	X 18 1	,674 ,037	AC/hr Cu.ft. Cu.ft./hr			
Total Building Infiltration: Total Building Ventilation:	ž		CFM CFM		X 0.	17	CFM CFM			
System 1 Infiltration & Ventilation Se Infiltration & Ventilation La Infiltration & Ventilation Se Winter Infiltration Specified Summer Infiltration Specifie	ent Gain M nsible Loss : 0.260	lultiplier: Multiplier AC/hr (8	: 11 CFM)	34.96 42.66 , Const	= (0.68 X)	0.998 0.998 mi-Ti	5 X 51.69 5 X 39.00 ght	Grains Dif	Temp. Differer fference) mp. Differenc	
Duct Load Factor Scenario	s for Syste	m 1								
Duct Load I actor Occharic										

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Return

1

1

Main

Main

Rhyac - Residential & Light Commercial HVAC Loads	Elite Software Development, Inc.
Florida H.E.R.O.	CP-CC-FWS-7
Newberry, FL 32669	Page 4

Room or Duct Name	Source	Minimum Velocity	Maximum Velocity	Rough, Factor	Design L/100	SP Loss	Duct Velocity	Duct Length	Htg Elow	Clg Flow	Act. Flow	Duct Size
System 1												
Supply Runouts												
Zone 1												
1-Master Bedroom	Built-In	450	750	0.15	0.1		385		97	151	151	2
2-Master Bath	Built-In	450	750	0.15	0.1		349.6		125	69	69	1(
3-Master WIC	Built-In	450	750	0.15	0.1		91.8		4	8	8	1
4-Office	Built-In	450	750	0.15	0.1		427.5		115	84	84	1(
5-Laundry	Built-In	450	750	0.15	0.1		391.8		36	34	34	1
6-Foyer	Built-In	450	750	0.15	0.1		378.8		52	33	33	1
7-Bath	Built-In	450	750	0.15	0.1		172.2		31	15	15	1
8-Bedroom 3	Built-In	450	750	0.15	0.1		343.5		149	67	67	1(
9-Bedroom 2	Built-In	450	750	0.15	0.1		583.3		82	115	115	1-(
10-Kitchen	Built-In	450	750	0.15	0.1		740.1		163	198	198	1-7
11-Living Room	Built-In	450	750	0.15	0.1		576		147	226	226	2
Other Ducts in System 1												
Supply Main Trunk	Built-In	650	900	0.15	0.1		734.7		1,000	1,000	1,000	14x1

Summary

System 1

Heating Flow: 1000 Cooling Flow: 1000

Rhvac - Residential & Light Commercial HVAC Loads Florida H.E.R.O. Newberry, FL 32669			Elite S		Plopment, Inc. CP-CC-FWS-7 Page 5
Total Building Summary Loads					
Component	Area	Sen	Lat	Sen	Total
Description	Quan	Loss	Gain	Gain	Gain
VYN 34 23: Glazing-Dbi Pn Vyn Fr U .34 SHGC .23,	60	796	0	910	910
ground reflectance = 0.23, outdoor insect screen with 50% coverage, medium color blinds at 45° with 25% coverage, U-value 0.34, SHGC 0.23					
VYN 34 23: Glazing-Dbl Pn Vyn Fr U .34 SHGC .23, ground reflectance = 0.23, U-value 0.34, SHGC 0.23	6	80	0	64	64
VYN 34 23: Glazing-Dbl Pn Vyn Fr U .34 SHGC .23, ground reflectance = 0.32, U-value 0.34, SHGC 0.23	4	53	0	42	42
10C-f: Glazing-French door, double pane low-e glass (e = 0.40), insulated fiberglass frame, ground reflectance = 0.32, medium color blinds at 45° with 100%	20	351	0	262	262
coverage, U-value 0.45, SHGC 0.43 VYN 34 23: Glazing-Dbl Pn Vyn Fr U .34 SHGC .23, ground reflectance = 0.32, outdoor insect screen with	15	199	0	526	526
50% coverage, U-value 0.34, SHGC 0.23 VYN 34 23: Glazing-Dbl Pn Vyn Fr U .34 SHGC .23, ground reflectance = 0.23, outdoor insect screen with 50% coverage, medium color blinds at 45° with 50%	15	199	0	111	111
coverage, U-value 0.34, SHGC 0.23 VYN 34 23: Glazing-Dbl Pn Vyn Fr U .34 SHGC .23, ground reflectance = 0.32, outdoor insect screen with	30	398	0	228	228
50% coverage, medium color blinds at 45° with 25% coverage, U-value 0.34, SHGC 0.23 10C-f: Glazing-French door, double pane low-e glass (e = 0.40), insulated fiberglass frame, ground reflectance = 0.32, medium color blinds at 45° with 25%	40	702	0	524	524
coverage, U-value 0.45, SHGC 0.43 11P: Door-Metal - Polyurethane Core, U-value 0.29	20	226	0	162	162
12E-0sw: Wall-Frame, R-19 insulation in 2 x 6 stud cavity, no board insulation, siding finish, wood studs, U-value 0.068	1570.3	4,163	0	2,071	2,071
R20 UV: Roof/Ceiling-Roof Joists Between Roof Deck and Ceiling or Foam Encapsulated Roof Joists, Custom, Open cell foam R 20, U-value 0.047	1870.4	3,428	0	4,572	4,572
22A-pl: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, light dry soil, U-value 0.989	198	7,637	0	0	0
		18,232	0	9,472	9,472
Subtotals for structure: People:	6	10,232	1,200 1,300	1,380 3,800	2,580 5,100
Equipment:	0		1,300		
Lighting:	0	0.004	250	0 2 742	4.007
Ductwork:		2,904	356	3,742	4,097
Infiltration: Winter CFM: 90, Summer CFM: 17 Ventilation: Winter CFM: 25, Summer CFM: 65 Exhaust: Winter CFM: 65, Summer CFM: 25		3,851 1,067	603 2,272	323 1,209	926 3,481
Total Building Load Totals:		26,053	5,731	19,926	25,656
Check Figures					
Total Building Supply CFM: 1,000		er Square ft.:			0.535
Square ft. of Room Area: 1,867 Volume (ft³): 18,674	Square	ft. Per Ton:			873
Total Sensible Gain: 19	,053 Btuh ,926 Btuh	78	MBH %		
	,731 Btuh ,656 Btuh		% Tons (Based 0	On Sensible	+ Latent)

Rhvac - Residential & Light Commercial HVAC Loads

Florida H.E.R.O.

Newberry, FL 32669

Elite Software Development, Inc. CP-CC-FWS-7 Page 6

Total Building Summary Loads (cont'd)

Notes

Rhvac is an ACCA approved Manual J, D and S computer program.

Calculations are performed per ACCA Manual J 8th Edition, Version 2, and ACCA Manual D.

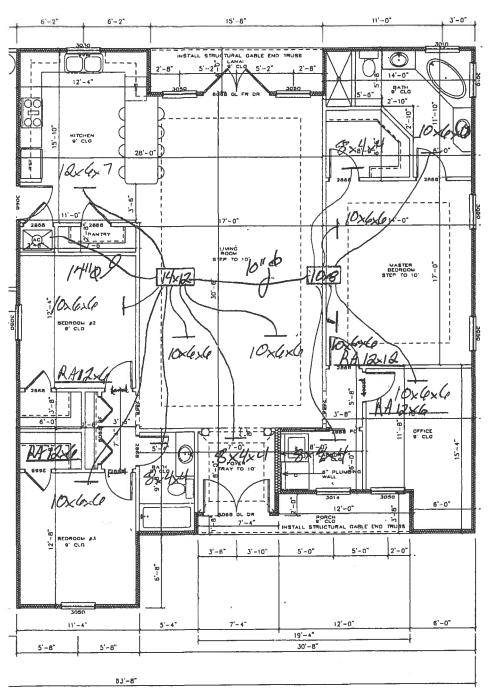
All computed results are estimates as building use and weather may vary.

Be sure to select a unit that meets both sensible and latent loads according to the manufacturer's performance data at your design conditions.

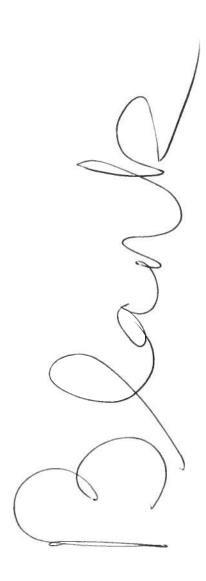
Rhvac - Residential & Light Co Florida H.E.R.O. Newberry, FL 32669	mmercial H	VAC Loads					Elite Software		oment, Inc CC-FWS-7 Page 7
System 1 Room Lo	ad Sun	nmary							
		Htg	Min	Run	Run	Clg	Clg	Min	Ac
Room	Area	Sens	Htg	Duct	Duct	Sens	Lat	Clg	Sys
No Name	SF	Btuh	CFM	Size	Vel	Btuh	Btuh	CFM	CFM
Zone 1									
1 Master Bedroom	238	2,397	44	2-6	385	2,670	452	122	151
2 Master Bath	130	3,079	56	1-6	350	1,212	442	55	69
3 Master WIC	49	100	2	1-4	92	142	0	6	8
4 Office	150	2,847	52	1-6	428	1,482	78	68	84
5 Laundry	48	877	16	1-4	392	604	374	28	34
6 Foyer	56	1,270	23	1-4	379	584	27	27	33
7 Bath	52	761	14	1-4	172	265	272	12	15
8 Bedroom 3	185	3,681	67	1-6	344	1,191	105	54	67
9 Bedroom 2	181	2,011	37	1-6	583	2,023	49	92	115
10 Kitchen	216	4,010	73	1-7	740	3,493	656	160	198
11 Living Room	563	3,619	66	2-6	576	3,994	648	183	226
Ventilation		1,067				1,209	2,272		
Duct Latent						4.050	195		
Return Duct		333				1,056	161		
System 1 total	1,867	26,053	451			19,926	5,731	807	1,000
System 1 Main Trunk Size:		14x14 in.							
Velocity:		735 ft./							
Loss per 100 ft.:		0.347 in.	wg						
Cooling System Summary									
	Cooling	Sensib	ole/Latent		Sensible		Latent		Total
	Tons		Split		Btuh		Btuh		Btuh
Net Required:	2.14		3% / 22%		19,926		5,731		25,656
Actual:	2.38	77	7% / 23%		22,022		6,578		28,600
Equipment Data									
_		Heating				Cooling S			
Type:			ce Heat Pum	р			e Heat Pump		
Model:		CH14NE	8030*0**A*				030*0**A*		
Indoor Model:		44.0555)30L+TXV		
Brand:		14 SEEF		_		14 SEER			
Description:			ce Heat Pum	р		14 SEER	e Heat Pump		
Efficiency:		8.2 HSP	Г						
Sound:		0) tub			0 28,600 Bt	u.b		
Capacity:		28,600 E	otuli						
Sensible Capacity:		n/a				22,022 Bt 6,578 Btu			
Latent Capacity:		n/a				9162428	11		
AHRI Reference No.:		n/a				3102420			

This system's equipment was selected in accordance with ACCA Manual S.

Manual S equipment sizing data: SODB: 92F, SOWB: 77F, WODB: 33F, SIDB: 75F, SIRH: 50%, WIDB: 72F, Sen. gain: 19,926 Btuh, Lat. gain: 5,731 Btuh, Sen. loss: 26,053 Btuh, Entering clg. coil DB: 77.1F, Entering clg. coil WB: 63.9F, Entering htg. coil DB: 70.7F, Clg. coil TD: 20F, Htg. coil TD: 50F, Req. clg. airflow: 807 CFM, Req. htg. airflow: 451 CFM



11/8/2019



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: Ft. White station Street: City, State, Zip: Ft. White, FL. Owner: Design Location: FL, Gainesville	,	Builder Name: Chemery construction Permit Office: Permit Number: Jurisdiction: County: Alachua (Florida Climat	e Zone 2)
 New construction or existing Single family or multiple family Number of units, if multiple family Number of Bedrooms Is this a worst case? Conditioned floor area above graded Conditioned floor area below graded Conditioned Floor Short Conditioned floor area below graded Conditioned Floor Conditioned Floor Conditioned Floor Conditioned Floor Conditioned Floor Short Conditioned Floor Short Conditioned Floor Conditioned	e (ft²) 0 otion Area .33 121.00 ft² .22 ft² ft² ft²	9. Wall Types (1788.0 sqft.) a. Frame - Wood, Exterior b. N/A c. N/A d. N/A 10. Ceiling Types (1867.0 sqft.) a. Under Attic (Vented) b. N/A c. N/A 11. Ducts a. Sup: Attic, Ret: Attic, AH: Main 12. Cooling systems a. Central Unit 13. Heating systems a. Electric Heat Pump 14. Hot water systems a. Electric b. Conservation features None 15. Credits	Insulation Area R=13.0 1788.00 ft² R= ft² R= ft² R= ft² Insulation Area R=40.0 1867.00 ft² R= ft² R= ft² R= ft² R= ft² R= ft² Cap: 40 gallons EF: 0.930 CF, Pstat
Glass/Floor Area: 0.065	Total Proposed Modifi Total Baselin		PASS
I hereby certify that the plans and this calculation are in compliance Code. PREPARED BY: DATE: I hereby certify that this building, a with the Florida Energy Code. OWNER/AGENT:	with the Florida Energy Suncoast insulators as designed, is in compliance	Review of the plans and specifications covered by this calculation Indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes. BUILDING OFFICIAL:	THE STATE OF THE S

- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.
- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

RM R	405-2	017		_INPUT S	SUMMA	RY CHE	CKL	IST F	REPORT					
			•		e disease in the Add # to-Inquiring in the	W/	ALLS						-	b
V/4	_Ornt_	Adjace	ent Wall	Туре	Space	Cavity R-Value	Wie Et	dth In	Height Et In	Area		g Framing Fraction	Solar Absor	Below Grade
- 	UIII_ S	Exterior		me - Wood	Main	13	6	~	9	54.0 ft ²		0.23	0.75	0
2	W	Exterior	Fra	me - Wood	Main	13	3	8	9	33.0 ft²		0.23	0.75	0
3	s	Exterior	Fra	me - Wood	Main	13	12		9	108.0 ft²		0.23	0.75	0
4	N	Exterior	Fra	me - Wood	Main	13	7	4	9	66.0 ft²		0.23	0.75	0
5	Е	Exterior	Fra	me - Wood	Main	13	1	8	9	15.0 ft²		0.23	0.75	C
6	s	Exterior	Fra	me - Wood	Main	13	5	4	9	48.0 ft ²		0.23	0.75	C
7	Ε	Exterior	Fra	me - Wood	Main	13	6	8	9	60.0 ft ²		0.23	0.75	(
8	s	Exterior	Fra	me - Wood	Main	13	11	4	9	102.0 ft ²		0.23	0.75	(
9	W	Exterior	Fra	me - Wood	Main	13	50	8	9	456.0 ft ²		0.23	0.75	(
10	N	Exterior	Fra	me - Wood	Main	13	12	4	9	111.0 ft ²		0.23	0.75	(
11	E	Exterior	Fra	me - Wood	Main	13	4		9	36.0 ft ²		0.23	0.75	(
12	N	Exterior	Fra	me - Wood	Main	13	15	8	9	141.0 ft ²		0.23	0.75	(
13	sw	Exterior	Fra	me - Wood	Main	13	4		9	36.0 ft ²		0.23	0.75	(
14	N	Exterior	Fra	me - Wood	Main	13	14		9	126.0 ft²	•	0.23	0.75	(
15	Ε	Exterior	Fra	me - Wood	Main	13	44		9	396.0 ft²		0.23	0.75	(
						DO	ORS							
$\sqrt{}$	#	Ornt		Door Type	Space			Storms	U-Val	ue F	Width t In	Height Ft	In	Area
	1	N		Insulated	Main			None	.46	6	3	6	8 4	40 ft²
	2	W		Insulated	Main			None	.46	3	3	6	8 2	20 ft²
- Time	3	N		Insulated	Main			None	.46	6	3	6	8 4	40 ft²
_				Orio	entation show	WINI	DOWS	Pronose	d orientatio	n.				
,		Wall			STREET OF OTHER	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					rhang			
\checkmark	# C		Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Depth	Separation	Int Sha	de S	Screenin
	1	S 3	Vinyl	Low-E Double	Yes	0.33	0.22	N	15.0 ft²	5 ft 8 in	1 ft 4 in	Drapes/b		None
	2	S 3	Vinyl	Low-E Double	Yes	0.33	0.22	N	4.0 ft²	5 ft 8 in	1 ft 4 in	Drapes/b	linds	None
	3	S 8	Vinyl	Low-E Double	Yes	0.33	0.22	N	15.0 ft ²	2 ft 0 in	1 ft 4 in	Drapes/b	linds	None
	4	W 9	Vinyl	Low-E Double	Yes	0.33	0.22	N	15.0 ft²		1 ft 4 in	Drapes/b	linds	None
	5	N 10	Vinyl	Low-E Double	Yes	0.33	0.22	N	9.0 ft²		1 ft 4 in	Drapes/b		None
	6	N 12	Vinyl	Low-E Double	Yes	0.33	0.22	N	30.0 ft²	6 ft 0 in	1 ft 4 in	Drapes/b		None
	7	N 14	Vinyl	Low-E Double	Yes	0.33	0.22	N	3.0 ft²	2 ft 0 in	1 ft 4 in	Drapes/b		None
	8	E 15	Vinyl	Low-E Double	Yes	0.33	0.22	N	30.0 ft²	2 ft 0 in	1 ft 4 in	Drapes/b	linds	None
						GAF	RAGE					<u>-</u>		
\vee	#	Floor	Area	Ceiling	Area	Exposed V	Vall Per	imeter		all Height	Expos	ed Wall Ins	ulation	
	1	528	l ft²	528 1	[[2	•	34 ft		9) ft		1		

INPUT SUMMARY CHECKLIST REPORT

8		The second secon	The second second		INI	FILTRAT	TION						49-b 1-94	
¥	Scope	Metho	d	SLA	CFM 5	0 EL	A E	qLA	ACH	ACH	1 50			
1 V	Vholehouse	Proposed	ACH(50)	.000286	1400.3	3 76.8	37 14	14.57	.1128	5	5			
<u></u>					HEAT	TING SY	STEM							
	#	System Type		Subtype			Efficienc	у	Capacity			Block	D	ouct
	_ 1	Electric Heat P	'ump/	None			HSPF:8.	2 2	3.6 kBtu/hr			1	S	ys#
					COOL	LING SY	STEM							
	#	System Type		Subtype			Efficiency	Сара	city Air	Flow SI	HR	Block	D	uct
	_ 1	Central Unit/		None			SEER: 14	28.6 kB	tu/hr 858	cfm 0.	85	1	S	ys#
					нот w	ATER S	YSTEM							
	#	System Type	SubType	Location	n EF	(Сар	Use	SetPnt		Con	servatio	ก	
	_ 1	Electric	None	Garage	0.93	40) gal	60 gal	120 deg		ı	None		
				so	DLAR HO	T WATE	R SYSTE	M						
V	FSEC Cert #	Company N	lame		System	Model #	Co	ollector Me		ollector Area	Storaç Volum	-	FEF	
	None	None								ft²				
						DUCTS								
\checkmark	#	Sup Location R		R Locatio	eturn n Area	Leak	age Type	Ai Hand		CFM25 OUT	QN	RLF	HV. Heat	
10000	1	Attic	6 373.4 ft	Attic	93.35 ft	Defau	It Leakage	Main	(Default)	(Default)			1	•
					TEMI	PERATL	JRES							
rog	ramable The	ermostat: Y		(Ceiling Fans	3:								
oolir eatir	ng []Ja ng [X]Ja ng []Ja	n Feb in X Feb	[] Mar [X] Mar [X] Mar	Apr Apr X Apr	May May May	X Jun Jun Jun	[X] Jul Jul Jul	[X] AL AL AL	g [X] Ser g [] Ser g [] Ser		t [Nov X Nov X Nov	X	De De De

FORM R405-2017		IN	PUT S	UMMA	RY CH	IECKLI	ST RE	PORT					
Thermostat Schedule:	HERS 200	6 Referer	ice -		فيفذا حسبها أن		Н	ours					-
Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM PM	78 80	78 80	78 78	78 78	78 78	78 78	78 78	78 78	80 78	80 78	80 78	80 78
Cooling (WEH)	AM PM	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78	78 78
Heating (WD)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
Heating (WEH)	AM PM	66 68	66 68	66 68	66 68	66 68	68 68	68 68	68 68	68 68	68 68	68 66	68 66
					ļ	MASS							
Mass Type			Ar	ea	7	hickness		Furniture F	raction	S	pace		
Default(8 lbs/sc	ą.ft.	5-2-5-	0 1	1 2		0 ft		0.3			Main		

RESIDENTIAL ENERGY CONSERVATION CODE DOCUMENTATION CHECKLIST

Florida Department of Business and Professional Regulation Simulated Performance Alternative (Performance) Method

Applications for compliance with the 2017 Florida Building Code, Energy Conservation via the residential Simulated Performance Method shall include:

	This checklist
	A Form R405 report that documents that the Proposed Design complies with Section R405.3 of the Florida Energy Code. This form shall include a summary page indicating home address, e-ratio and the pass or fail status along with summary areas and types of components, whether the home was simulated as a worst-case orientation, name and version of the compliance software tool, name of individual completing the compliance report (one page) and an input summary checklist that can be used for field verification (usually four pages/may be greater).
	Energy Performance Level (EPL) Display Card (one page)
	HVAC system sizing and selection based on ACCA Manual S or per exceptions provided in Section R403.7
	Mandatory Requirements (five pages)
Req	uired prior to CO for the Performance Method:
	Air Barrier and Insulation Inspection Component Criteria checklist (Table R402.4.1.1 - one page)
	A completed Envelope Leakage Test Report (usually one page)
	If Form R405 duct leakage type indicates anything other than "default leakage", then a completed Form R405 Duct Leakage Test Report (usually one page)

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 100

The lower the Energy Performance Index, the more efficient the home.

1. New home or, addition	1. New (From Plans)	12. Ducts, location & insulation level
2. Single-family or multiple-family	2. Single-family	a) Supply ducts R 6.0 b) Return ducts R 6.0 c) AHU location Main
3. No. of units (if multiple-family)	31	Of Arto location
4. Number of bedrooms	43_	13. Cooling system: Capacity 28.6 a) Split system SEER
5. Is this a worst case? (yes/no)	5. <u>No</u>	b) Single package SEER c) Ground/water source SEER/COP
6. Conditioned floor area (sq. ft.)	6. <u>1867</u>	d) Room unit/PTAC EER
7. Windows, type and area a) U-factor:(weighted average) b) Solar Heat Gain Coefficient (SHGC) c) Area	7a. 0.330 7b. 0.220 7c. 121.0	14. Heating system: Capacity 28.6 a) Split system heat pump HSPF b) Single package heat pump HSPF
8. Skylights a) U-factor:(weighted average) b) Solar Heat Gain Coefficient (SHGC)	8a. <u>NA</u> 8b. <u>NA</u>	c) Electric resistance COP d) Gas furnace, natural gas AFUE e) Gas furnace, LPG AFUE f) Other 8.20
9. Floor type, insulation level: a) Slab-on-grade (R-value) b) Wood, raised (R-value) c) Concrete, raised (R-value) 10. Wall type and insulation:	9a0.0_ 9b 9c	15. Water heating system a) Electric resistance
A. Exterior: 1. Wood frame (Insulation R-value) 2. Masonry (Insulation R-value) B. Adjacent: 1. Wood frame (Insulation R-value)	10A113.0 10A2	d) Solar system with tank EF e) Dedicated heat pump with tank EF f) Heat recovery unit HeatRec% g) Other
2. Masonry (Insulation R-value) 11. Ceiling type and insulation level a) Under attic b) Single assembly c) Knee walls/skylight walls d) Radiant barrier installed	11a. 40.0 11b. 11c. Yes	16. HVAC credits claimed (Performance Method) a) Ceillng fans Yes b) Cross ventilation No c) Whole house fan No d) Multizone cooling credit e) Multizone heating credit f) Programmable thermostat Yes
*Label required by Section R303.1.3 of the Flo	orida Building Code, Ener	rgy Conservation, if not DEFAULT.
I certify that this home has complied with the saving features which will be installed (or excedisplay card will be completed based on installed).	eeded) in this home befor	e final inspection. Otherwise, a new EPL
Builder Signature:		Date:
Address of New Home:		City/FL Zip:Ft. White, FL

Florida Building Code, Energy Conservation, 6th Edition (2017) Mandatory Requirements for Residential Performance, Prescriptive and ERI Methods

Αľ	DDRESS:					Permit Number	or:				
			hite , FL ,	20							
MAN	NDATOR'	Y REQ	UIREMENT	S See individu	ıal code se	ections for fu	ill details.				
/				SECT	ON R401	GENERA	L				
	display card (Section 553 nonpresold	be completed by the complete services between the complete services by	eted and certified orida Statutes) re- buildings. The El wait. The building	PL) display card (I by the builder to be quires the EPL displey card con official shall verify that code compliants	accurate and lay card to be it tains information that the EPL dis	correct before find the control of t	nal approval of ddendum to ea energy perfor eleted and sign	the building to ach sales contr mance level an ed by the build	r occupancy. r act for both pre d efficiencies o er accurately re	iorida law sold and f components iflects the plar	าร
	R402.4 Air I Sections		Mandatory). through R402.4.	The building therm 5.	nal envelope sh	nall be construct	ed to limit air l	eakage In acco	rdance with the	requirements	of
			on: Dwelling un with Section C40	its of R-2 Occupand 2.5.	les and multip	le attached sing	le family dwelli	ngs shall be pe	rmitted to		
	R402. The se	4.1 Bulldi ealing met	n g thermal enve hods between dis	lo pise building therr similar materlals sh	nal envelope s all allow for dif	hall comply with ferential expans	Sections R40 lon and contra	2.4.1.1 and R4 ction.	02.4.1.2.		
	with th	ne manufa	cturer's instruction	ponents of the build ns and the criteria li third party shall insp	sted in Table F	₹402.4.1.1, as a	pplicable to the	.4.1.1 shall be e method of cor	installed in acconstruction. Whe	ordance re required	
	chang accord individ	es per hou dance with luals as de proved thir	ur in Climate Zone ANSI/RESNET/I efined in Section to d party. A written	ng or dwelling unit sets 1 and 2, and three CC 380 and reported 553.993(5) or (7), Floreport of the results dat any time after controls.	e air changes point at a pressure orida Statutes, a of the test sha	per hour in Clim e of 0.2 inch w.g , or individuals lic all be signed by	ate Zones 3 th . (50 pascals). censed as set the party cond	rough 8. Testing Testing shall b forth in Sectlon ucting the test	g shall be cond be conducted by 489.105(3)(f),	lucted in / either (g) or (i) or	
	Excep buildin	otion: ngs in whic	Testing is not re	quired for additions uction is less than 8	, alterations, re 5 percent of th	enovations, or re e building therm	pairs, of the bual envelope.	ilding thermal	envelope of exi	sting	
	1. Extended other in 2. Dan infiltration 3. Inte	infiltration inpers inclution control information doors, information doors ating and o	control measures uding exhaust, int of measures. If installed at the for continuous ve cooling systems, i	eplace and stove do . ake, makeup air, ba time of the test, sh entilation systems a f installed at the time	ackdraft and flu all be open. nd heat recove e of the test, sl	e dampers shall ery ventilators sh hall be turned of	be closed, but nall be closed a f.	t not sealed be		or	
	using tight-fit	tting doors	on factory-built f	aing fireplaces shall ireplaces listed and on masonry firepla	labeled in acc	ordance with UL	, 127, the door	s shall be teste	a and listed for	e the	
	square foot (1.5 L/s/m	and swinging (dows, skylights and doors no more than an accredited, indep	0.5 cfm per sq	juare foot (2.6 L/	/s/m2), when to	ested according	to NFRC 400	m per or	
	Excep	otion:	Site-built window	vs, skylights and do	ors.	MAT (see					

MAN	DATORY	REQUIREMENTS - (Con	tinued)	
open of enclose of Tab	combustion fue ed in a room, i le R402.1.2, wi asketed and an	burning appliances, the appliances and comb colated from inside the thermal envelope. Such are the walls, floors and cellings shall meet n y water lines and ducts in the room insulated tioned space to a minimum of R-8.	te Zones 3 through 8, where open combustion air ducts provide corpustion air opening shall be located outside the building thermal enter rooms shall be sealed and insulated in accordance with the enversion less than the basement wall R-value requirement. The door into in accordance with Section R403. The combustion air duct shall be	velope or lope requirements the room shall be
	1.	Direct vent appliances with both intake and ex	chaust pipes installed continuous to the outside. R402.4.2 and Section R1006 of the Florida Bullding Code, Resider	ntial.
betwee	en conditioned han 2.0 cfm (0.	and unconditioned spaces. All recessed lumin	ne building thermal envelope shall be sealed to limit air leakage raires shall be IC-rated and labeled as having an air leakage rate no M E283 at a 1.57 psf (75 Pa) pressure differential. All recessed ing and the interior wall or ceiling covering.	ot
R403.1	Controls.	SECTIO	N R403 SYSTEMS	
		stat provision (Mandatory). At lea	ast one thermostat shall be provided for each separate heating and	cooling system.
R40	3.1.3 Heat pu	np supplementary heat (Mandatory).	leat pumps having supplementary electric-resistance heat shall have also have the heat pump compressor can meet the heating load.	
	3.3.2 Sealing	(Mandatory) All ducts, air handlers, filte	r boxes and building cavities that form the primary air containment enum chambers, shall be constructed and sealed in accordance will shall be shown to meet duct tightness criteria below.	passageways h Section
	(7), Florida S	tatutes, or individuals licensed as set forth in with Section R403.3.3.	ith ANSI/RESNET/ICC 380 by either individuals as defined in Secti Section 489.105(3)(f), (g) or (i), Florida Statutes, to be "substantiall	y leak free" in
	of the de	sign airflow rate when tested in accordance w	ave a manufacturer's designation for an air leakage of no more than with ASHRAE 193.	
	R403.3.3 Du	ct testing (Mandatory). Ducts shall be press	sure tested to determine air leakage by one of the following method	S:
	1.	manufacturer's air handler enclosure if Insta	red with a pressure differential of 0.1 inch w.g. (25 Pa) across the silled at the time of the test. All registers shall be taped or otherwise	sealed during the te
	2.	Postconstruction test: Total leakage shall be entire system, including the manufacturer's test.	e measured with a pressure differential of 0.1 Inch w.g. (25 Pa) acro air handler enclosure. Registers shall be taped or otherwise sealed	ss the during the
	Ex	ceptions:		
		thermal envelope.	ired where the ducts and air handlers are located entirely within the	building
		2. Duct testing is not mandatory for buildin		official
			be signed by the party conducting the test and provided to the code	official.
R403.3	.5 Building ca	vities (Mandatory). Building framing cavities	s shall not be used as ducts or plenums.	
R403.4 or belov	Mechanical s v 55°F (13°C)	stem piping insulation (Mandatory). Methall be insulated to a minimum of R-3.	chanical system piping capable of carrying fluids above 105°F (41°	C)
	caused by si	otection of piping insulation. Piping insulation, P	ulation exposed to weather shall be protected from damage, includir if wind, and shall provide shielding from solar radiation that can caus rmitted.	ng that se
	accordance of Automatic co	vith Section R403.5.1.1. Heat trace temperatuntrols, temperature sensors and pumps shall	intenance systems (Mandatory). Heated water circulation system ure maintenance systems shall be in accordance with Section R403 be accessible. Manual controls shall be readily accessible.	5.5.1.2.
	pir Co oc	e shall be a dedicated return pipe or a cold w	er circulation systems shall be provided with a circulation pump. The ater supply pipe. Gravity and thermosiphon circulation systems sha shall start the pump based on the identification of a demand for ho off the pump when the water in the circulation loop is at the desired	II be prohibited. t water within the
	R4 sh	03.5.1.2 Heat trace systems. Electric heat to	race systems shall comply with IEEE 515.1 or UL 515. Controls for heat tracing to maintain the desired water temperature in the pipin	such systems g in accordance

with the times when heated water is used in the occupancy.

MANDATORY REQUIREMENTS - (Continued) R403.5.5 Heat traps (Mandatory). Storage water heaters not equipped with integral heat traps and having vertical pipe risers shall have heat traps installed on both the inlets and outlets. External heat traps shall consist of either a commercially available heat trap or a downward and upward bend of at least 3 1/2 inches (89 mm) in the hot water distribution line and cold water line located as close as possible to the storage tank. R403.5.6 Water heater efficiencies (Mandatory). Service water-heating systems shall be equipped with automatic temperature controls capable R403.5.6.1.1 Automatic controls. of adjustment from the lowest to the highest acceptable temperature settings for the intended use. The minimum temperature setting range shall be from 100°F to 140°F (38°C to 60°C). A separate switch or a clearly marked circuit breaker shall be provided to permit the power supplied to electric service systems to be turned off. A separate valve shall be provided to permit the energy supplied to the main burner(s) of combustion types of service water-heating systems to be turned off. Water-heating equipment installed in residential units shall meet the minimum efficiencies of R403.5.6.2 Water-heating equipment. Table C404.2 in Chapter 4 of the Florida Building Code, Energy Conservation, Commercial Provisions, for the type of equipment installed. Equipment used to provide heating functions as part of a combination system shall satisfy all stated requirements for the appropriate water-heating category. Solar water heaters shall meet the criteria of Section R403.5.6.2.1. Solar systems for domestic hot water production are rated by the annual solar energy R403.5.6.2.1 Solar water-heating systems. factor of the system. The solar energy factor of a system shall be determined from the Florida Solar Energy Center Directory of Certified Solar Systems. Solar collectors shall be tested in accordance with ISO Standard 9806, Test Methods for Solar Collectors, and SRCC Standard TM-1, Solar Domestic Hot Water System and Component Test Protocol. Collectors in installed solar water-heating systems should meet the following criteria: 1. Be installed with a tilt angle between 10 degrees and 40 degrees of the horizontal; and 2. Be installed at an orientation within 45 degrees of true south. The building shall be provided with ventilation that meets the requirements of the R403.6 Mechanical ventilation (Mandatory). Florida Building Code, Residential, or Florida Building Code, Mechanical, as applicable, or with other approved means of ventilation Including: Natural, Infiltration or Mechanical means. Outdoor air intakes and exhausts shall have automatic or gravity dampers that close when the ventilation system is not operating. R403.6.1 Whole-house mechanical ventilation system fan efficacy. When installed to function as a whole-house mechanical ventilation system, fans shall meet the efficacy requirements of Table R403.6.1. Exception: Where whole-house mechanical ventilation fans are integral to tested and listed HVAC equipment, they shall be powered by an electronically commutated motor. R403.6.2 Ventilation air. Residential buildings designed to be operated at a positive indoor pressure or for mechanical ventilation shall meet the following criteria:

 The design air change per hour minimums for residential buildings in ASHRAE 62.2, Ventilation for Acceptable Indoor Air Quality, shall be the maximum rates allowed for residential applications.

 No ventilation or air-conditioning system make-up air shall be provided to conditioned space from attics, crawlspaces, attached enclosed garages or outdoor spaces adjacent to swimming pools or spas.

If ventilation air is drawn from enclosed space(s), then the walls of the space(s) from which air is drawn shall be
insulated to a minimum of R-11 and the ceiling shall be insulated to a minimum of R-19, space permitting, or R-10
otherwise.

R403.7 Heating and cooling equipment (Mandatory).

R403.7.1 Equipment sizing. Heating and cooling equipment shall be sized in accordance with ACCA Manual S based on the equipment loads calculated in accordance with ACCA Manual J or other approved heating and cooling calculation methodologies, based on building loads for the directional orientation of the building. The manufacturer and model number of the outdoor and indoor units (if split system) shall be submitted along with the sensible and total cooling capacities at the design conditions described in Section R302.1. This Code does not allow designer safety factors, provisions for future expansion or other factors that affect equipment sizing. System sizing calculations shall not include loads created by local intermittent mechanical ventilation such as standard kitchen and bathroom exhaust systems. New or replacement heating and cooling equipment shall have an efficiency rating equal to or greater than the minimum required by federal law for the geographic location where the equipment is installed.

TABLE R403.6.1 WHOLE-HOUSE MECHANICAL VENTILATION SYSTEM FAN EFFICACY

FAN LOCATION	AIRFLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY ^a (CFM/WATT)	AIRFLOW RATE MAXIMUN (CFM)
Range hoods	Any	2,8 cfm/watt	Any
In-line fan	Any	2.8 cfm/watt	Апу
Bathroom, utility room	10	1.4 cfm/watt	<90
Bathroom, utility room	90	2.8 cfm/watt	Any

For SI: 1 cfm = 28.3 L/min.

. When tested in accordance with HVI Standard 916

N/I A	IANDATORY REQUIREMENTS - (Continued)	3 80
	R403.7.1.1 Cooling equipment capacity. Cooling only equipment shall be selected so that its total than the calculated total load but not more than 1.15 times greater than the total load calculated according to the procession 403.7, or the closest available size provided by the manufacturer's product lines. The corresponding latent capacity and the calculated latent load.	cedure selected in apacity of the
	The published value for AHRI total capacity is a nominal, rating-test value and shall not be used for equipment sizing expanded performance data shall be used to select cooling-only equipment. This selection shall be based on the o temperature for the load calculation (or entering water temperature for water-source equipment), the blower CFM properformance data, the design value for entering wet-bulb temperature and the design value for entering dry-bulb temperature.	utdoor design dry-bulb ovided by the expanded
	Design values for entering wet-bulb and dry-bulb temperatures shall be for the indoor dry bulb and relative humidity calculation and shall be adjusted for return side gains if the return duct(s) is installed in an unconditioned space.	used for the load
	Exceptions:	
	 Attached single- and multiple-family residential equipment sizing may be selected so that its coolin calculated total sensible load but not less than 80 percent of that load. 	
	When signed and sealed by a Florida-registered engineer, in attached single- and multiple-family equipment may be sized in accordance with good design practice. R403.7.1.2 Heating equipment capacity.	units, the capacity of
	R403.7.1.2.1 Heat pumps. Heat pump sizing shall be based on the cooling requirements as calculated R403.7.1.1, and the heat pump total cooling capacity shall not be more than 1.15 times greater than the design heating load is 1.15 times greater than the design cooling load.	ed according to Section ign cooling load even if
		15
	R403.7.1.2.2 Electric resistance furnaces. Electric resistance furnaces shall be sized within 4 kW of the requirements calculated according to the procedure selected in Section R403.7.1.	e design
	R403.7.1.2.3 Fossil fuel heating equipment. The capacity of fossil fuel heating equipment with natural drashall not be less than the design load calculated in accordance with Section R403.7.1.	aft atmospheric burners
	R403.7.1.3 Extra capacity required for special occasions. Residences requiring excess cooling or heating intermittent basis, such as anticipated additional loads caused by major entertainment events, shall have equipart continuous space cooling or heating within that space by one or more of the following options:	oment sized or controlled to
	 A separate cooling or heating system is utilized to provide cooling or heating to the major entertain. 	inment areas.
	 A variable capacity system sized for optimum performance during base load periods is utilized. 	
	R403.8 Systems serving multiple dwelling units (Mandatory). Systems serving multiple dwelling units shall c403 and C404 of the IECC—Commercial Provisions in lieu of Section R403.	comply with Sections
	R403.9 Snow melt and ice system controls (Mandatory) Snow- and ice-melting systems, supplied through energy shall include automatic controls capable of shutting off the system when the pavement temperature is above 50° precipitation is falling and an automatic or manual control that will allow shutoff when the outdoor temperature is	F (10°C), and no
	R403.10 Pools and permanent spa energy consumption (Mandatory). Shall be in accordance with Sections R403.10.1 through R403.10.5.	ols and permanent spas
	R403.10.1 Heaters. The electric power to heaters shall be controlled by a readily accessible or integral part of the heater mounted on the exterior of the heater, or external to and within 3 feet (914 mm) of such switch shall not change the setting of the heater thermostat. Such switches shall be in addition to a power to the heater. Gas-fired heaters shall not be equipped with continuously burning ignition pilots.	of the heater. Operation
	R403.10.2 Time switches. Time switches or other control methods that can automatically turn off and on accesshall be installed for heaters and pump motors. Heaters and pump motors that have built-in time switches shall section.	ording to a preset schedule I be in compliance with this
	Exceptions:	
	 Where public health standards require 24-hour pump operation. Pumps that operate solar- and waste-heat-recovery pool heating systems. 	
	Where pumps are powered exclusively from on-site renewable generation.	
	R403.10.3 Covers. Outdoor heated swimming pools and outdoor permanent spas shall be equipped with a the water surface or a liquid cover or other means proven to reduce heat loss.	100000
	Exception: Where more than 70 percent of the energy for heating, computed over an operation senergy, such as from a heat pump or solar energy source, covers or other vapor-retardant means R403.10.4 Gas- and oil-fired pool and spa heaters. All gas- and oil-fired pool and spa heaters shall have efficiency of 82 percent for heaters manufactured on or after April 16, 2013, when tested in accordance with All gas- and oil-fired pool and spa heaters manufactured on or after April 16, 2013, when tested in accordance with All gas- and oil-fired pool and spa heaters manufactured on or after April 16, 2013, when tested in accordance with All gas- and oil-fired pool and spa heaters manufactured on or after April 16, 2013, when tested in accordance with All gas- and oil-fired pool and spa heaters.	shall not be required. a minimum thermal
	heaters fired by natural or LP gas shall not have continuously burning pilot lights.	1 100 100 110

	R403.10.5 Heat pump pool heaters. Heat pump pool heaters shall have a minimum COP of 4.0 when tested in accordance with AHRI 1160, Table 2, Standard Rating Conditions-Low Air Temperature. A test report from an independent laboratory is required to verify procedure compliance. Geothermal swimming pool heat pumps are not required to meet this standard.
	R403.11 Portable spas (Mandatory) he energy consumption of electric-powered portable spas shall be controlled by the requirements of APSP-14.
	SECTION R404
El	ECTRICAL POWER AND LIGHTING SYSTEMS
	R404.1 Lighting equipment (Mandatory). Not less than 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps or not less than 75 percent of the permanently installed lighting fixtures shall contain only high-efficacy lamps.
	Exception: Low-voltage lighting.
	PAGE 4.4.1 Lighting agulament (Mandatory). Fuel gas lighting systems shall not have continuously burning pilot lights.

2017 - AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA

TABLE 402.4.1.1 AIR BARRIER AND INSULATION INSPECTION COMPONENT CRITERIA

Project Name:

Ft. White station #7

Builder Name: Chemery construction

Street:

City, State, Zip:

Ft. White, FL,

Permit Office: Permit Number:

Owner:

Jurisdiction:

CHECK Design Location: FL, Gainesville **INSULATION INSTALLATION CRITERIA** AIR BARRIER CRITERIA COMPONENT A continuous air barrier shall be installed in the building envelope. Air-permeable insulation shall General The exterior thermal envelope contains a continuous air barrier. not be used as a sealing material. requirements Breaks or joints in the air barrier shall be sealed. The air barrier in any dropped ceiling/soffit shall be aligned with The insulation in any dropped ceiling/soffit Celling/attic the Insulation and any gaps in the air barrier shall be sealed. shall be aligned with the air barrier. Access openings, drop down stairs or knee wall doors to unconditioned attic spaces shall be sealed. Cavities within corners and headers of frame walls The junction of the foundation and sill plate shall be sealed. Walls shall be insulated by completely filling the cavity The junction of the top plate and the top of exterior walls shall be with a material having a thermal resistance of R-3 sealed. per inch minimum. Knee walls shall be sealed. Exterior thermal envelope insulation for framed walls shall be installed in substantial contact and continuous alignment with the air barrier. The space between window/door jambs and framing, and Windows, skylights skylights and framing shall be sealed. and doors Rim joists shall be insulated. Rim joists shall include the air barrier. Rim joists The air barrier shall be installed at any exposed edge of Floor framing cavity insulation shall be installed to Floors maintain permanent contact with the underside of (including subfloor decking, or floor framing cavity insulation above-garage shall be permitted to be in contact with the top side and cantilevered of sheathing, or continuous insulation installed on floors) the underside of floor framing and extends from the bottom to the top of all perimeter floor framing members. Where provided instead of floor insulation, insulation Exposed earth in unvented crawl spaces shall be covered with Crawl space walls shall be permanently attached to the crawlspace a Class I vapor retarder with overlapping joints taped. Duct shafts, utility penetrations, and flue shafts opening to Shafts, penetrations exterior or unconditioned space shall be sealed. Batts in narrow cavities shall be cut to fit, or narrow cavities shall be filled by insulation that on Narrow cavities installation readily conforms to the available cavity spaces. Air sealing shall be provided between the garage and conditioned spaces. Garage separation Recessed light fixtures installed in the building thermal envelope Recessed light fixtures installed in the building Recessed lighting thermal envelope shall be air tight and IC rated. shall be sealed to the drywall. Batt insulation shall be cut neatly to fit around wiring Plumbing and wiring and plumbing in exterior walls, or insulation that on installation readily conforms to available space shall extend behind piping and wiring Exterior walls adjacent to showers and tubs shall The air barrier installed at exterior walls adjacent to showers and Shower/tub he insulated tubs shall separate them from the showers and tubs. on exterior wall The air barrier shall be installed behind electrical or communication Electrical/phone box or boxes or air-sealed boxes shall be installed. exterior walls HVAC register boots that penetrate building thermal envelope shall HVAC register boots be sealed to the sub-floor or drywall. When required to be sealed, concealed fire sprinklers shall only be Concealed sealed in a manner that is recommended by the manufacturer. sprinklers Caulking or other adhesive sealants shall not be used to fill voids between fire sprinkler cover plates and walls or ceilings.

a. In addition, inspection of log walls shall be in accordance with the provisions of ICC-400.

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance 2017 Florida Building Code, Energy Conservation, 6th Edition

	Jurisdiction:	Permi	it #:				
Jol	Information	and the second of the second o					
Bui	Ider: Chemery construction	Community:	Lot: 7	_			
Add	dress:		74				
City	r: Ft. White	State: FL	Zip:				
Air	Leakage Test Results P	assing results must meet either the Pe	erformance, Prescriptive, or ERI Method				
C	PRESCRIPTIVE METHOD-The buildin changes per hour at a pressure of 0.2	g or dwelling unit shall be tested and verifinch w.g. (50 Pascals) in Climate Zones 1	led as having an air leakage rate of not exceeding 7 air and 2.	STATE OF THE PARTY			
the	selected ACH(50) value, as shown on For	e building or dwelling unit shall be tested a m R405-2017 (Performance) or R406-201 Form R405-2017-Energy Calc (Performance)	and verified as having an air leakage rate of not exceedin 17 (ERI), section labeled as infiltration, sub-section ACH5 ce) or R406-2017 (ERI); 5.000	g 0.			
	x 60 + 16803 Building Vo	= lume ACH(50)	Method for calculating building volume: Retrieved from architectural plans				
		Mechanical Ventilation installation partment.	Code software calculated Field measured and calculated				
Tes 489	ting shall be conducted by either individual .105(3)(f), (g), or (i) or an approved third pa	s as defined in Section 553.993(5) or (7), a arty. A written report of the results of the te	380 and reported at a pressure of 0.2 inch w.g. (50 Pasc Florida Statues.or individuals licensed as set forth in Sec est shall be signed by the party conducting the test and enetrations of the building thermal envelope.	als tior			
1. E cont 2. D mea	trol measures.	air, back draft and flue dampers shall be	, beyond the intended weatherstripping or other infiltration closed, but not sealed beyond intended infiltration control				
4. E 5. H	 Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed. Heating and cooling systems, if installed at the time of the test, shall be turned off. Supply and return registers, if installed at the time of the test, shall be fully open. 						
Te	sting Company			100000			
I h	Company Name: Phone: I hereby verify that the above Air Leakage results are in accordance with the 2017 6th Edition Florida Building Code Energy Conservation requirements according to the compliance method selected above.						
Sig	gnature of Tester:		Date of Test:				
Pri	nted Name of Tester:	Production 10 to the state					
Lic	ense/Certification #:	Issuing A	Authority:				

Florida Product Approval HardiePanel[®] Siding

- For use inside HVHZ:
 - HardiePanel Siding fastener types, fastening schedule, and installation shall be in accordance with the Miami-Dade County Florida NOA 07-0418.04. Consult the HardiePanel product installation instructions on the follow pages for all other installation requirements.
- For use outside of HVHZ,
 - HardiePanel Siding fastener types, fastening schedule, and installation shall be in accordance with Engineering Evaluation Report RIO-2297-11.
 Consult the HardiePanel product installation instructions on the follow pages for all other installation requirements.



Hardie Pane FILTO Vertical Siding



INSTALLATION REQUIREMENTS - PRIMED & COLORPLUS® PRODUCTS

Visit www.dameshardie.com for the most recent version.

SMOOTH - CEDARMILL® - SELECT SIERRA 8 - STUCCO

IMPORTANT: FAILURE TO INSTALL AND FINISH THIS PRODUCT IN ACCORDANCE WITH APPLICABLE BUILDING CODES AND JAMES HARDIE WRITTEN APPLICATION INSTRUCTIONS MAY LEAD TO PERSONAL INJURY, AFFECT SYSTEM PERFORMANCE, VIOLATE LOCAL BUILDING CODES. AND VOID THE PRODUCT ONLY WARRANTY, BEFORE INSTALLATION, CONFIRM THAT YOU ARE USING THE CORRECT HARDIEZONE INSTRUCTIONS. INSTALLATION OF HZ10™ PRODUCTS OUTSIDE AN HZ10™ LOCATION WILL VOID YOUR WARRANTY. TO DETERMINE WHICH HARDIEZONE APPLIES TO YOUR LOCATION, VISIT WWW.HARDIEZONE.COM OR CALL 1-866-942-7343 (866 9HARDIE)

STORAGE & HANDLING:

Store flat and keep dry and covered prior to installation. Installing siding wet or saturated may result in shrinkage at butt joints. Carry product on edge. Protect edges and corners from breakage. James Hardie is not

responsible for damage caused by improper storage and handling of the product.

CUTTING INSTRUCTIONS INDOORS

OUTDOORS

- Position cutting station so that wind will blow dust away from user and others in working area.
- 2. Use one of the following methods a. Best:
 - Score and snap
 - ii. Shears (manual, electric or pneumatic) i. Dust reducing circular saw equipped with a
 - HardieBlade* saw blade and HEPA vacuum extraction
- i. Dust reducing circular saw with a HardieBlade saw blade c Good (only use for low to moderate cutting)

NEVER use a power saw indoors

NEVER use a circular saw blade that does not carry the HardieBlade saw blade trademark

Out only using score and snap, or shears (manual, electric or pneumatic)
 Position cutting station in well-ventilated area

NEVER dry sweep - Use wet suppression or HÉPA Vacuum

Important Note: For maximum protection (lowest respirable dust production), James Hardie recommends always using "Best"-level cutting methods where feasible

NIOSH-approved respirators can be used in conjunction with above cutting practices to further reduce dust exposures. Additional exposure information is available at www.jameshardic.com to help you determine the most appropriate cutting method for your job requirements. If concern still exists about exposure levels or you do not comply with the above practices, you should always consult a qualified inclustrial hygienist or contact James Hardie for further information.

GENERAL REQUIREMENTS:

- HardiePanel® vertical siding can be installed over braced wood or steel studs spaced a maximum of 24" o.c. Irregularities in framing and sheathing can mirror through the finished application.
- HardiePanel vertical siding can also be installed over foam insulation/sheathing up to 1" thick. When using foam insulation/sheathing, avoid over-driving nails (fasteners), which can result in dimpling of the siding due to the compressible nature of the foam insulation/sheathing. Extra caution is necessary if power-driven nails (fasteners) are used for attaching siding over foam insulation/sheathing.
- A water-resistive barrier is required in accordance with local building code requirements. The water-resistive barrier must be appropriately installed with penetration and junction flashing in accordance with local building code requirements. James Hardie will assume no responsibility for water infiltration. James Hardie does manufacture HardieWrap™ Weather Barrier, a non-woven non-perforated housewrap1, which complies with building code requirements
- When installing James Hardie products all clearance details in figs. 5,6,7,8,9,10&11 must be followed.
- Adjacent finished grade must slope away from the building in accordance with local building codes typically a minimum of 6" in the first 10".
- Do not install James Hardie products, such that they may remain in contact with standing water.
- HardiePanel vertical siding may be installed on vertical wall applications only.
- For larger projects, including commercial and multi-family projects, where the span of the wall is significant in length, the designer and/or architect should take into consideration the coefficient of thermal expansion and moisture movement of the product in their design. These values can be found in the Technical Bulletin "Expansion Characteristics of James Hardie® Siding Products" at www.JamesHardie.com.
- DO NOT use stain on James Hardie® products.

INSTALLATION:

Fastener Requirements

Position fasteners 3/8" from panel edges and no closer than 2" away from corners. Do not nail into corners.

HardiePanel Vertical Siding Installation

- Framing must be provided at horizontal and vertical edges for nailing.
- · HardiePanel vertical siding must be joined on stud.
- · Double stud may be required to maintain minimum edge nailing distances

Figure 1 water-resistive barrier HardiePanel sheathing tical siding keep nails 3/8" min. from panel edaes olate moderate contact keep fasteners 2" away from comers

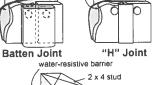
Joint Treatment

 Vertical Joints - Install panels in moderate contact (fig. 1), alternatively joints may also be covered with battens, PVC or metal jointers or caulked (Not applicable to ColorPlus* Finish) (fig. 2).

Figure 3

Horizontal Joints - Provide Z-flashing at all horizontal joints (fig. 3).

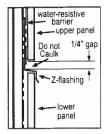
Figure 2 water-resistive barrier water-resistive barrier 2 x 4 stud

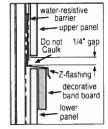


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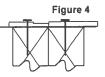
leave appropriate gap between panels, then caulk* Caulk Joint

(Not applicable to ColorPlus® Finish) *Apply caulk in accordance with caulk manufacturer's written application instructions.





Recommendation When installing Sierra 8, provide a double stud at nanel oints to avoid nailing through grooves.



For additional information on HardieWrap™ Weather Barrier, consult James Hardie at 1-866-4Hardie or www.hardiewrap.com

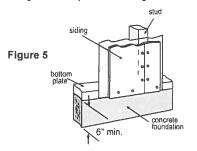
WARNING: AVOID BREATHING SILICA DUST

WARNING: AVOID BREATHING SILICA DUST
James Hardie* products contain respirable crystalline silica, which is known to the State of California to cause cancer and is considered by IARC and NIOSH to be a cause of cancer from some occupational sources.
Breathing excessive amounts of respirable silica dust can also cause a disabling and potentially fatal lung disease called silicosis, and has been linked with other diseases. Some studies suggest smoking may increase these risks. During installation or handling (1) work in outdoor areas with ample ventilation; (2) use fiber cement shears for cutting or, where not feasible, use a HardieBlade* saw blade and dust-reducing circular saw attached to a HEPA vacuum; (3) warn others in the immediate area; (4) wear a properly-fitted. NIOSH-approved dust mask or respirator (e.g., N-95) in accordance with applicable government regulators and manufacturer instructions to further limit respirable silica exposures. During clean-up, use HEPA vacuums or wet cleanup methods - never dry sweep. For further information, refer to our installation instructions and Material Safety Data Sheet available at www.jameshardie com or by calling 1-800-9HARDIE (1-800-942-7343). FAILURE TO ADHERE TO OUR WARNINGS MSDS. AND INSTALLATION INSTRUCTIONS MAY LEAD TO SERIOUS. PERSONAL INJURY OR DEATH.

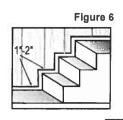


CLEARANCES

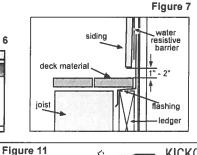
Install siding and trim products in compliance with local building code requirements for clearance between the bottom edge of the siding and the adjacent finished grade.



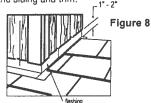
Maintain a 1" - 2" clearance between James Hardie" products and paths, steps and driveways.



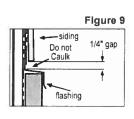
Maintain a 1" - 2" clearance between James Hardle products and decking material.



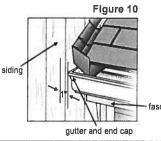
At the juncture of the roof and vertical surfaces, flashing and counterflashing shall be installed per the roofing manufacturer's instructions. Provide a 1" - 2" clearance between the roofing and the bottom edge of the siding and trim.



Maintain a 1/4" clearance between the bottom of James Hardie products and horizontal flashing. Do not caulk gap.



Maintain a minimum 1" gap between gutter end caps and siding & trim.



KICKOUT FLASHING

Because of the volume of water that can pour down a sloped roof, one of the most critical flashing details occurs where a roof intersects a sidewall. The roof must be flashed with step flashing. Where the roof terminates, install a kickout to deflect water away from the siding.

It is best to install a self-adhering membrane on the wall before the subfascia and trim boards are nailed in place, and then come back to install the kickout.

Figure 11, Kickout Flashing: To prevent water from dumping behind the siding and the end of the roof intersection, install a "kickout" of sufficient length and angle to direct the water running down the roof away from the siding.

GENERAL FASTENING REQUIREMENTS

Fasteners must be corrosion resistant, galvanized, or stainless steel. Electro-galvanized are acceptable but may exhibit premature corrosion. James Hardie recommends the use of quality, hot-dipped galvanized nails. James Hardie is not responsible for the corrosion resistance of fasteners. Stainless steel fasteners are recommended when installing James Hardie* products near the ocean, large bodies of water, or in very humid climates.

PNEUMATIC FASTENING

James Hardie products can be hand nailed or fastened with a pneumatic tool. Pneumatic fastening is highly recommended. Set air pressure so that the fastener is driven snug with the surface of the siding. A flush mount attachment on the pneumatic tool is recommended. This will help control the depth the nail is driven. If setting the nail depth proves difficult, choose a setting that under drives the nail. (Drive under driven nails snug with a smooth faced hammer - Does not apply for installation to steel framing).

- Consult applicable code compliance report for correct fasteners type and placement to achieve specified design wind loads.
- NOTE Published wind loads may not be applicable to all areas where Local Building Codes have specific jurisdiction. Consult James Hardie Technical Services if you are unsure of applicable compliance documentation.
- · Drive fasteners perpendicular to siding and framing.
- Fastener heads should fit snug against siding (no air space). (fig. A)
- Do not over-drive nail heads or drive nails at an angle.

- Drip edge

- If nail is countersunk, caulk nail hole and add a nail. (fig. B)
- For wood framing, under driven nails should be hit flush to the plank with a hammer (For steel framing, remove and replace nail).
- Do not use aluminum fasteners, staples, or clipped head nails.



Slep flas









Figure A

CAULKING

For best results use an Elastomeric Joint Sealant complying with ASTM C920 Grade NS, Class 25 or higher or a Latex Joint Sealant complying with ASTM C834. Caulking/Sealant must be applied in accordance with the caulking/sealant manufacturer's written instructions or ASTM C1193.

PAINTING

DO NOT use stain on James Hardie® products. James Hardie products must be painted within 180 days for primed product and 90 days for unprimed. 100% acrylic topcoats are recommended. Do not paint when wet. For application rates refer to paint manufacturers specifications. Back-rolling is recommended if the siding is sprayed.

*The illustration (figure 11) was reprinted with permission of THE JOURNAL OF LIGHT CONSTRUCTION. For subscription information, visit www.jlconline.com.

RECOGNITION: In accordance with ICC-ES Legacy Report NER-405, HardiePanel® vertical siding is recognized as a suitable alternate to that specified in: the BOCA National Building Code/1999, the 1997 Standard Building Code, the 1997 Uniform Building Code, the 1998 International One- and Two-Family Dwellings. HardiePanel vertical siding is also recognized for application in the following. City of Los Angeles Research Report No. 24862, State of Florida listing FL#889, Dade County, Florida NOA No. 02-0729.02, U.S. Dept. of HUD Materials Release 1263c, Texas Department of Insurance Product Evaluation EC-23, City of New York MEA 223-93-M, and California DSA PA-019. These documents should also be consulted for additional information concerning the suitability of this product for specific applications.

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Additional Installation Information, Warranties, and Warnings are available at www.jameshardie.com





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Product Approval Menu > Product or Application Search > Application List > Application Detail

Hot Topics

FL# FL18355-R4 Application Type Revision Code Version 2017 **Application Status** Approved

Comments Archived

Product Manufacturer TAMKO Building Products, Inc.

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Quality Assurance Representative

Address/Phone/Email

Roofing Category Asphalt Shingles Subcategory

Evaluation Report from a Florida Registered Architect or a Licensed Compliance Method

Florida Professional Engineer

Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed

Referenced Standard and Year (of Standard)

the Evaluation Report

Zachary R. Priest

PE-74021 Florida License Quality Assurance Entity UL LLC Quality Assurance Contract Expiration Date 01/07/2019 Validated By Locke Bowden

√ Validation Checklist - Hardcopy Received

FL18355 R4 COI TBP15001.4 2017 FBC Eval Report Shingles final.pdf Certificate of Independence

Standard <u>Year</u> 2016 **ASTM D 3161 ASTM D 3462** 2010 **ASTMD D 7158** 2011 1995 TAS 100

Equivalence of Product Standards

Certified By

Approved Testing Lab

FL18355 R4 Equiv UL letter for standards for FBC 2017.pdf

Sections from the Code

Page 2 of 2 Florida Building Code Online

> Method 1 Option D Product Approval Method

09/25/2017 Date Submitted 09/27/2017 Date Validated 10/08/2017 Date Pending FBC Approval 12/12/2017 Date Approved

Summary of Products

FL # Model, Number or Name		Description			
18355.1	Glass-Seal, Elite Glass-Seal, Heritage, Heritage Premium, Heritage Woodgate, Heritage Vintage, Hip and Ridge, and Vintage Hip and Ridge Shingles	ASTM D 3462 asphalt shingles and hip and ridge shingles			
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other: See evaluation report for limits of use.		Installation Instructions FL18355 R4 II TBP15001.4 2017 FBC Eval Report Shingle final.pdf Verified By: Zachary R. Priest PE-74021 Created by Independent Third Party: Yes Evaluation Reports FL18355 R4 AE TBP15001.4 2017 FBC Eval Report Shingles final.pdf Created by Independent Third Party: Yes			



Contact Us :: 2601 Blair Stone Road, Tallahassee FL 32399 Phone: 850-487-1824

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olastprc

5200 W. CENTURY BLVD. LOS ANGELES, CA 90045

Smooth / Wood Grain / White Wood Grain Series N Fiberglass Door **INSWING / OUTSWING** Rustic / Mahogany "NON-IMPACT"

GENERAL NOTES

- This product has been evaluated and is in compliance with the 6th Edition (2017) Rarida Building Code (FBC) structural requirements excluding the "High Velocity Hurricane Zone" (HYHZ).
- Product anchors shall be as listed and spaced as shown on details. Anchor embedment to base material shall be beyond wall dressing or stucco.
- When used in areas requiring wind borne debris protection this product is required to be protected with an impact resistant covering that complies with FBC Sections 1609.1.2 & R301.2.1.2.
- by a licensed engineer or registered architect. Site conditions that deviate from the details of this drawing require further engineering analysis

For 2x stud framing construction, anchoring of these units shall be the same as that shown for 2x buck masonry construction.

_	_	_	_	_	-			_
7	6	5	4	ω	2	_	SHEET #	
Bill of Materials, glazing detail & components	Frame anchoring - 1X buck masonry construction	Buck and frame anchoring - 2X buck masonry construction	Vertical cross sections	Horizontal cross sections	Door panel details	Typical elevation, design pressures & general notes	DESCRIPTION	TABLE OF CONTENTS

OUTSWING INSWING SWING

37.50" x 80.50" 37.50" x 82.00"

+50.0 +47.0

<u>Q</u>

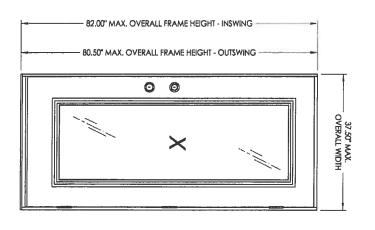
MAX. FRAME DIMENSION

GLASS

POSITIVE

NEGATIVE -50.0 -50.0

DESIGN PRESSURE (PSF)



PRODUCT: CHIC BY: E S DWG. BY: SCALE FL-15213.14 une 02/16/12 WING NO PLASTPRO INC. 1 of 7 N.T.S. FIBERGLASS DOOR 2 10/19/17 UPDATE TO 6TH ED. (2017) FBC 1 04/22/15 UPDATE TO 5TH ED. (2014) FBC PART OR ASSEMBLY. S ¥ TYPICAL ELEVATION, DESIGN PRESSURES & GENERAL NOTES NO DATE REVISIONS

October 19, 2017 Lyndon F. Schmidt P.E. No. 43409

SCHMID

SCHMID

No. 43409

A Prepared By:

SIMILE OF

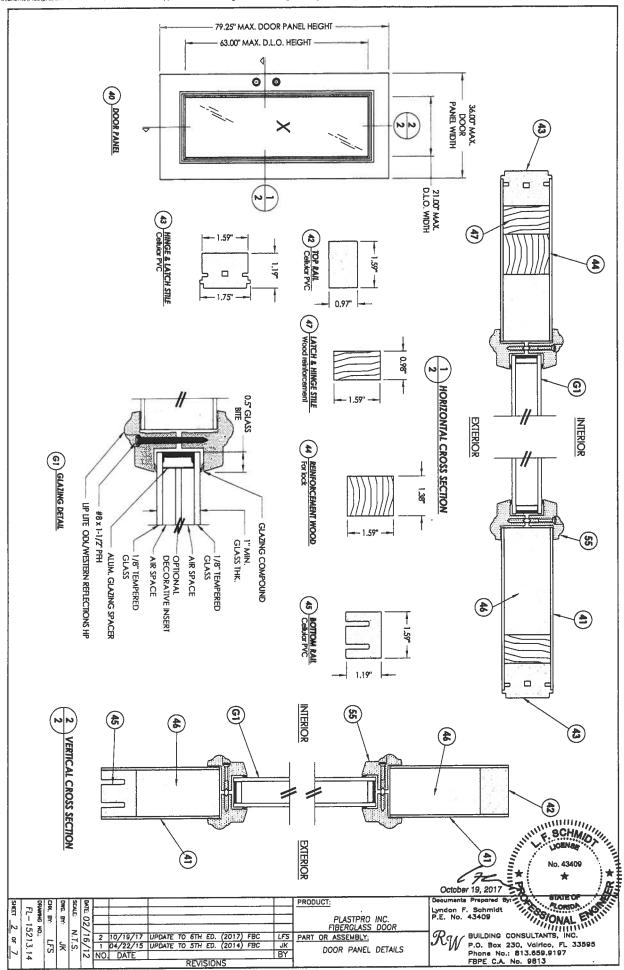
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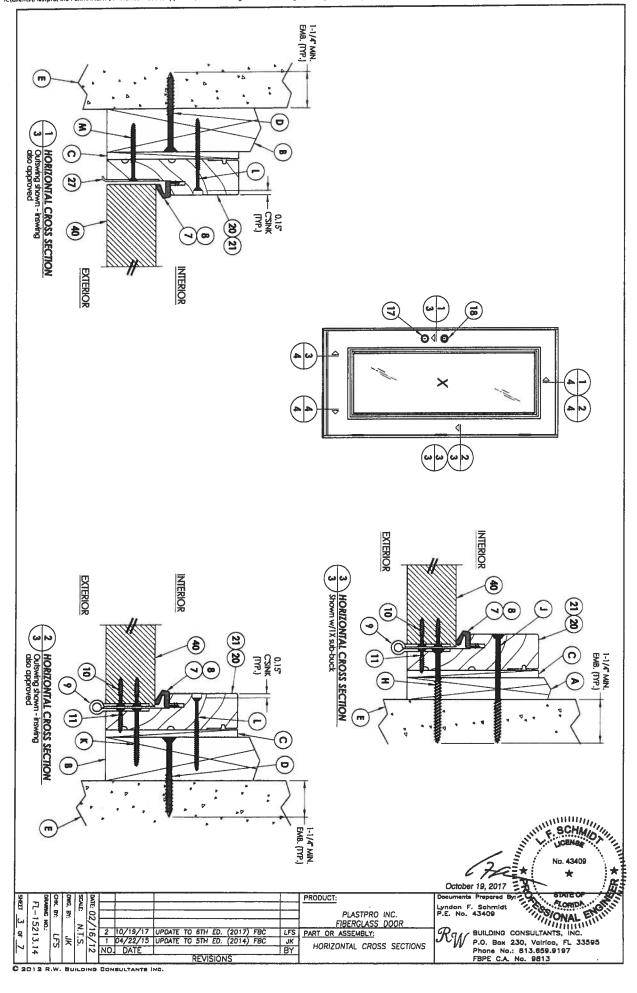
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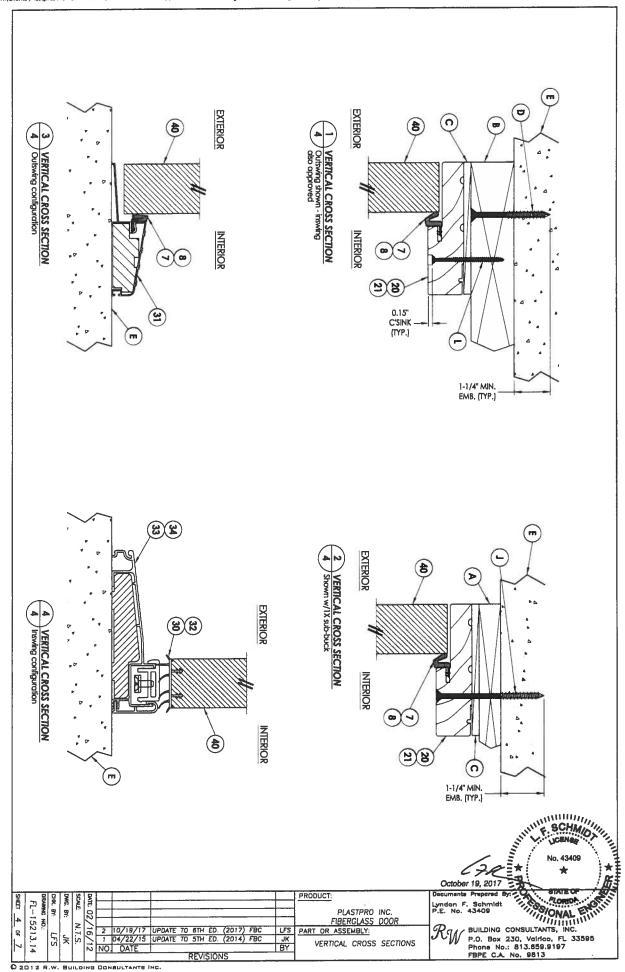
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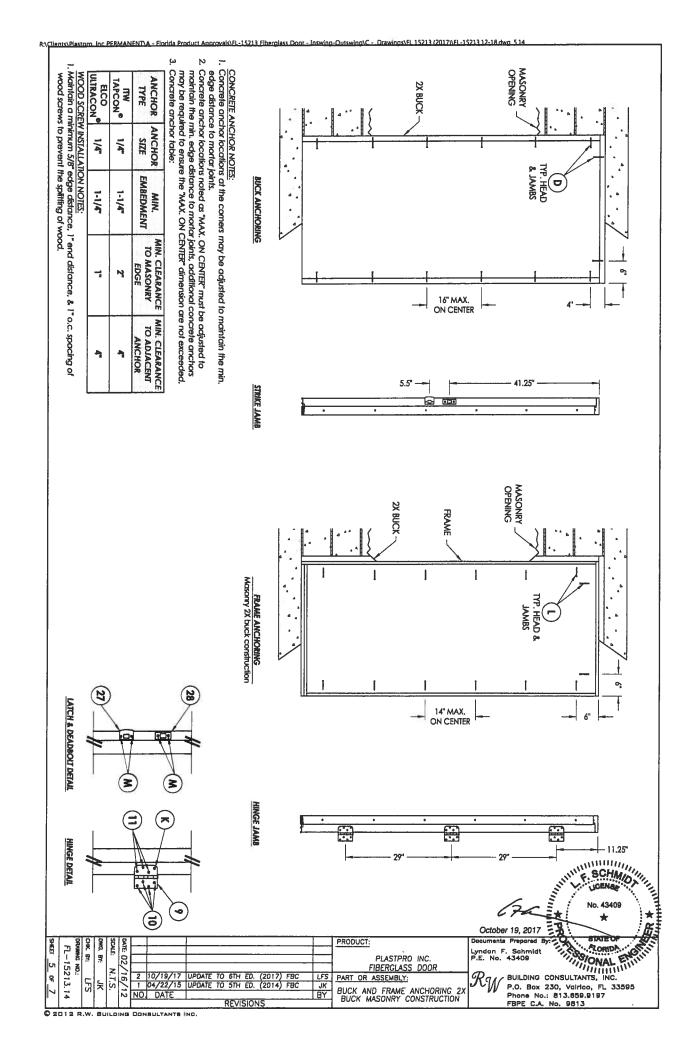
BUILDING CONSULTANTS, INC.

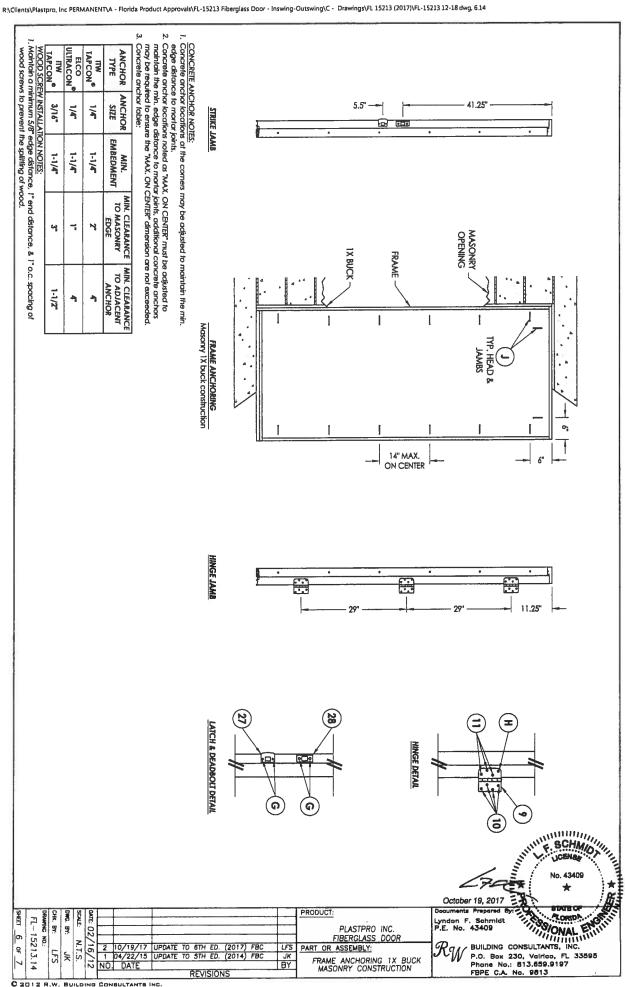
P.O. Box 230, Voirioo, FL 33595 P.O. Box 230, Vairiao, FL 33595 Phone No.: 813.659.9197 FBPE C.A. No. 9813











Business & Professional Regulation







Product Approval Menu > Product or Application Search > Application List > Application Detail

FOFFICE OF THE SECRETARY

FL # FL14092-R10
Application Type Revision
Code Version 2017
Application Status Approved

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Comments Archived

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Raynor Garage Doors

Authorized Signature

Gary Wedekind garyw@raynor.com

Technical Representative Address/Phone/Email Gary Wedekind 1101 E River Road Dixon, IL 61021 garw@raynor.com

Quality Assurance Representative

Address/Phone/Email

Mike Bariether 1101 East River Road Dixon, IL 61021 (815) 285-7312 mbariether@raynor.com

Category

Exterior Doors

Subcategory

Sectional Exterior Door Assemblies

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed

Florida Professional Engineer

Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed

the Evaluation Report

PE-65940

Florida License Quality Assurance Entity

UL LLC

Scott A. Brown

Quality Assurance Contract Expiration Date

07/31/2020

Validated By

John E. Scates, PE

Certificate of Independence

☑ Validation Checklist - Hardcopy Received

Referenced Standard and Year (of Standard)

 Standard
 Year

 ANSI/DASMA 108
 2012

 ANSI/DASMA 115
 2012

 TAS 201
 1994

 TAS 202
 1994

 TAS 203
 1994

FL14092 R10 COI Letter of independence 082812.pdf

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FL11398-R4 **Application Type** Revision Code Version 2017

Comments Archived

Product Manufacturer

Application Status

Town and Country Industries, Address/Phone/Email 400 West McNab Road Ft. Lauderdale, FL 33309 (954) 493-8551 tomj@tc-alum.com

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Quality Assurance Representative Address/Phone/Email

Category Subcategory **Shutters** Storm Panels

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed

Florida Professional Engineer

Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed

the Evaluation Report

PE-41961

Quality Assurance Entity

Quality Assurance Contract Expiration Date

National Accreditation and Management Institute 12/31/2020

Validated By

ORLANDO L. BLANCO, P.E.

John W. Knezevich

Validation Checklist - Hardcopy Received

Certificate of Independence

FL11398 R4 COI CERTIFICATE OF INDEPENDENCE SS.pdf

Referenced Standard and Year (of Standard)

Standard Year TAS 201 1994 TAS 202 1994 **TAS 203** 1994

Equivalence of Product Standards Certified By

Sections from the Code

