DATE 06/30/2008	This Permit Must Be Prominently Posted	ullaing Permit	actruction	000027131
•	This Permit Must Be Prominently Posted			00002/131
APPLICANT THEODOR		PHONE -	904.296.1490	. 22216
ADDRESS 6800	SOUTHPOINT PKWY.# 300	JACKSONVILLE	<u>F</u>	L 32216
OWNER MAROND	A HOMES INC. OF FL	PHONE -	904.296.1490	
ADDRESS 277	SW TIMBER RIDGE DRIVE	LAKE CITY	<u>F</u>	<u>L</u> <u>32024</u>
CONTRACTOR THE	ODORE BROCK	PHONE	904.296.1490	
LOCATION OF PROPERT	90-W TO SR. 247-S,TL TO C-252	2-B,TR TO TIMBER RID	OGE,TL	
	8TH LOT ON R.			-
TYPE DEVELOPMENT	SFD/UTILITY ES	TIMATED COST OF CO	ONSTRUCTION	158350.00
HEATED FLOOR AREA		EA3167.00	HEIGHT 25.00	STORIES 1
FOUNDATION CONC	WALLS FRAMED F	ROOF PITCH	FLOO	R CONC
LAND USE & ZONING	RSF-2	MAX	K. HEIGHT 35	
Minimum Set Back Requir	ments: STREET-FRONT 25.00	REAR	15.00 SI	DE <u>10.00</u>
NO. EX.D.U. 0	FLOOD ZONE XPP	DEVELOPMENT PER	MIT NO.	<u>L</u>
PARCEL ID 10-4S-16-0	02856-108 SUBDIVISIO	N TIMBERLANDS		
LOT 8 BLOCK	PHASE 1 UNIT	TOT	AL ACRES 0.50	
000001626	CBC1256382	MA		
Culvert Permit No.	Culvert Waiver Contractor's License Nur	mber // (//	Applicant/Owner/Co	ntractor
18"X32'MITERED	08-403 BLK		WR	N
Driveway Connection	Septic Tank Number LU & Zoni	ng checked by Ap	proved for Issuance	New Resident
COMMENTS: ELEVATI	ON CONFIRMATION LETTER REQUIRED	D. MFE @ 100.00'.		
			Check # or Cash	910487
Temporary Power	FOR BUILDING & ZONIN		ONLY	910487 (footer/Slab)
Temporary Power	FOR BUILDING & ZONIN			
	FOR BUILDING & ZONING Foundation date/app. by	NG DEPARTMENT	Monolithic	(footer/Slab)  date/app. by
Temporary Power	FOR BUILDING & ZONING Foundation date/app. by	NG DEPARTMENT	Monolithic	(footer/Slab)
Under slab rough-in plumb	FOR BUILDING & ZONIN  Foundation  date/app. by  ing  date/app. by  Rough-in plumbing a	NG DEPARTMENT date/app. by	Monolithic Sheathing/Na	(footer/Slab)  date/app. by
Under slab rough-in plumb Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing  date/app. by  Rough-in plumbing a	date/app. by	Monolithic Sheathing/Na	(footer/Slab)  date/app. by
Under slab rough-in plumb	FOR BUILDING & ZONIN  Foundation  date/app. by  ing  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct	date/app. by  date/app. by  bove slab and below woo	Monolithic Sheathing/Na	(footer/Slab)  date/app. by  iling date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab date/app. by  Rough-in plumbing a p. by  Heat & Air Duct date/app. by	date/app. by	Monolithic Sheathing/Na d floor Peri. beam (Lintel)	(footer/Slab)  date/app. by  iling date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIR  Foundation  date/app. by  ing Slab  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct  date/app. by  C.O. Final	date/app. by  date/app. by  date/app. by  date/app. by	Monolithic Sheathing/Na	(footer/Slab)  date/app. by  iling date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab date/app. by  Rough-in plumbing a p. by  Heat & Air Duct date/app. by  C.O. Final	date/app. by  date/app. by  bove slab and below woo	Monolithic Sheathing/Na d floor Peri. beam (Lintel) Culvert	(footer/Slab)  date/app. by  iling date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab date/app. by  Rough-in plumbing a p. by  Heat & Air Duct date/app. by  C.O. Final	date/app. by  date/app. by  bove slab and below woo  date/app. by	Monolithic Sheathing/Na d floor Peri. beam (Lintel)	(footer/Slab)  date/app. by  iling date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing date/ap  Electrical rough-in  Permanent power da  M/H tie downs, blocking, ele  Reconnection	FOR BUILDING & ZONING  Foundation  date/app. by  ing  date/app. by  Rough-in plumbing a p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  lectricity and plumbing  Pump pole	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  Utility Po	Monolithic Sheathing/Nard floor  Peri. beam (Lintel)  Culvert Pool	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing date/ap  Electrical rough-in  Permanent power da  M/H tie downs, blocking, ele  Reconnection	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  ectricity and plumbing  Pump pole  date/app. by  Travel Trailer	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  Utility Po	Monolithic Sheathing/Nard floor  Peri. beam (Lintel)  Culvert	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  ectricity and plumbing  Pump pole  date/app. by  Travel Trailer	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  Utility Po	Monolithic Sheathing/Na d floor Peri. beam (Lintel) Culvert Pool ble date/app. by Re-roof Sheathing/Na d floor Sheathing/Na d fl	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  ing Slab  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  ectricity and plumbing  Pump pole  date/app. by  Travel Trailer	date/app. by  date/app. by  bove slab and below woo  date/app. by  date/app. by  Utility Po	Monolithic Sheathing/Na d floor Peri. beam (Lintel) Culvert Pool ble date/app. by Re-roof Sheathing/Na d floor Sheathing/Na d fl	(footer/Slab)  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIR  Foundation  date/app. by  ing Slab  date/app. by  Rough-in plumbing a  p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  ectricity and plumbing  pump pole date/app. by  Travel Trailer	date/app. by  date/app. by  bove slab and below woo  date/app. by  date/app. by  Utility Pos/app. by  date/app. by	Monolithic Sheathing/Na d floor Sheathing/Na d floor Peri. beam (Lintel) Culvert Pool date/app. by Re-roof SURCHARGE FI	(footer/Slab)  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Under slab rough-in plumb  Framing	FOR BUILDING & ZONIR  Foundation  date/app. by  ing Slab _  date/app. by  Rough-in plumbing a p. by  Heat & Air Duct  date/app. by  C.O. Final  te/app. by  ectricity and plumbing  date/app  Pump pole date/app. by  Travel Trailer  \$ 795.00 CERTIFICATION FE	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  Utility Polyapp. by  date/app. by  FIRE FEE \$ 0.00	Monolithic Sheathing/Na  d floor Peri. beam (Lintel)  Culvert Pool  date/app. by Re-roof  SURCHARGE FI	(footer/Slab)  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by

DEDMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

### LEGAL DESCRIPTION:

LOT EIGHT (8) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

MARONDA HOMES CERTIFIED TO:



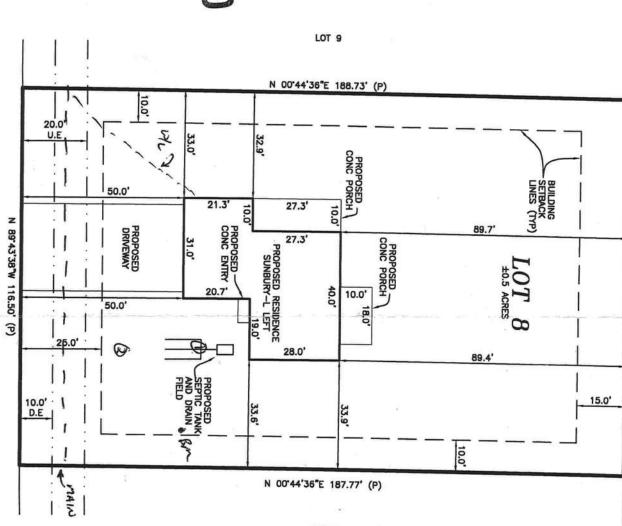
## PROPOSED BUILDING LAYOUT

30

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

N 89"15'24"W 116.50' (P)





LOT 7

JILDING SETBACK INFORMATION FOR IMBERLANDS" IS AS FOLLOWS: FRONT 5', REAR 15', SIDE 10'

1 N.50

UILDING SETBACK NOTE:

# FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND

BUILDING SETBACK LINES DEPICTED HEREON ARE SUBJECT

TO BE ACCEPTABLE TO THIS SURVEYOR.

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

SURVEYOR NOTES:

S.W TIMBER RIDGE DRIVE

2

4607 NW 6th STREET SUITE C.

BRINKMAN SURVEYING &

■ FOUND 1/2" REBAR NO
DENTIFICATION

■ FOUND 1/2" REBAR & CAP

□ SET 1/2" REBAR & CAP

□ SET 1/2" REBAR & CAP

□ FOUND 3/4" IRON PIPE

■ FOUND 3/4" IRON PIPE

■ FOUND 3/4" IRON PIPE

■ FOUND 3/4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

NO IDENTIFICATION

□ SET 4" X 4" CONC. MON.

HE UCE

NO IDENTIFICATION

□ FEN

POUND FOUND

X = FOUND NAIL & DISK P.S.M. 5582

X = FOUND NAIL & DISK

E FOUND 6" X 6" S.R.D.

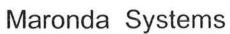
TRANS = TRANS

R/W MON.

HE TYP

TYP

WM = WAT 0 = WOOD POWER POLE Z = TELEPHONE PEDESTA I HEREBY CERTIFY THAT THE SURVEY DATA TRUE AND CORRECT REPRESENTATION OF A MY SUPERVISION OF THE HEREON DESCRIBE THE MINIMUM TECHNICAL STANDARDS AS SE BOARD OF LAND SURVEYORS, PURSUANT TO STATURES AND CHAPTER 61037-6, FLORIDA I I I I I I NOT VALID WITHOUT THE SIGNATURE AND THAT A FLORIDA LICENSED SURVEYOR AND MAPPITO THIS MAP BY ANYONE OTHER THAN THIS CERTIFICATE BRINKMAN, PSM - FLA. CERT# IS NOT A BOUND 20 000 SURVI



Maronda Systems

4005 Maronda Way

Sanford FL 32771

(407) 321-0064

Fax (407) 321-3913

MAY 0 5 2008

29

DATE:

Engineer/Architect of Record:

Tomas Ponce, P.E.

367 Medallion PL.

Chuluota, Fl. 32766

FL PE # 50068,

Design Criteria: TPI

Design: Matrix Analysis

MiTek software

SUNBURY L MODEL

DIV/SUB PLAN JOB # LOT **ADDRESS** 277 SW TIMBER JAX-9TM SUNL3 LEFT 9TM00801 8-1 RIDGE DR

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

I, Tomas Ponce, P.E.

**JGRD** 

12/27/07

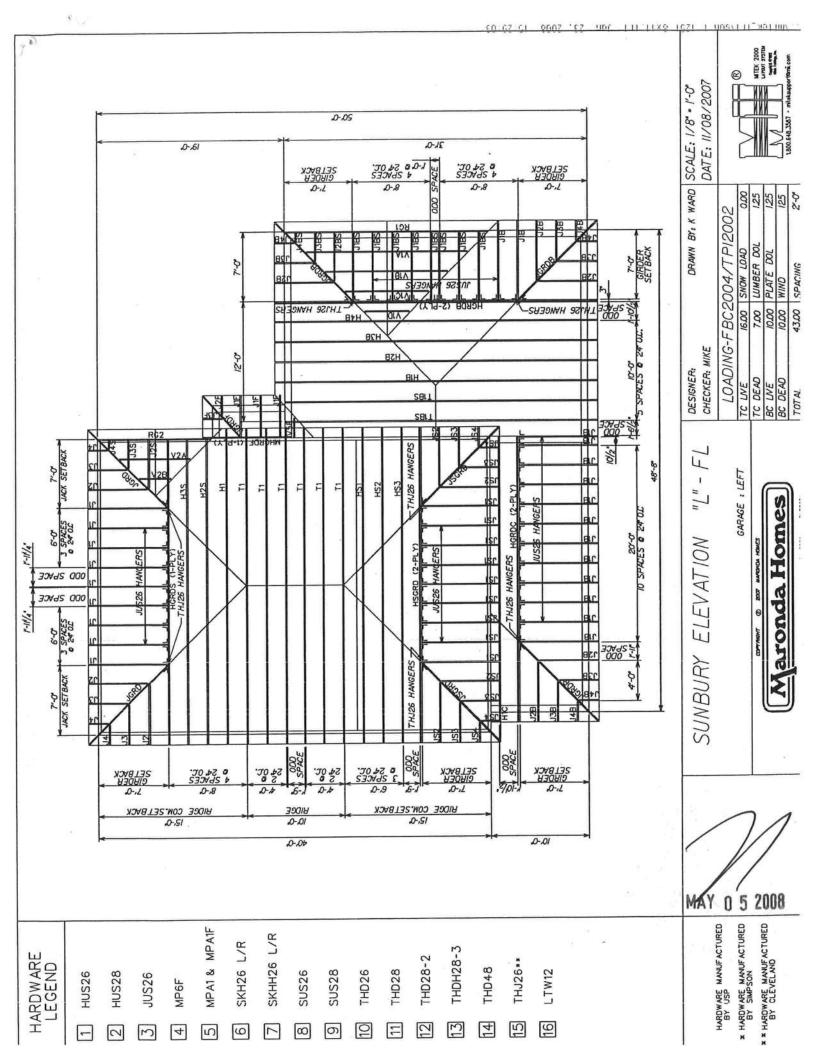
01/23/08

the Architect/Engineer of Record for the above referenced lot

Have reviewed the package and confirmed that it matches the physical and structural

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwgs:
Layout	11/08/07	01/23/08	JGRDB	12/27/07	01/23/08	Roof Loads-
V	07/27/05	01/23/08	JGRDF	12/27/07	01/23/08	TC Live: 16.0 psf
HIP	11/02/06	01/23/08	JS1	12/27/07	01/23/08	TC Dead: 7.0 psf
H1	12/27/07	01/23/08	JS2	12/27/07	01/23/08	BC Live: 10.0 psf
H1B	12/27/07	01/23/08	JS3	12/27/07	01/23/08	BC Dead: 10.0 psf
H1C	12/27/07	01/23/08	JS4	12/27/07	01/23/08	Total 43.0 psf
H2S	12/27/07	01/23/08	JSGRD	12/27/07	01/23/08	
H2B	12/27/07	01/23/08	MHGRDF	12/27/07	01/23/08	DurFac- Lbr: 1.25
H3S	12/27/07	01/23/08	RG1	12/27/07	01/23/08	DurFac- Plt: 1.25
НЗВ	12/27/07	01/23/08	RG2	12/27/07	01/23/08	O.C. Spacing: 24.0"
H4B	12/27/07	01/23/08	T1	12/27/07	01/23/08	
HGRDS	12/27/07	01/23/08	T1BS	12/27/07	01/23/08	
HGRDB	12/27/07	01/23/08	V1A	12/27/07	01/23/08	
HGRDC	12/27/07	01/23/08	V1B	12/27/07	01/23/08	
HS1	12/27/07	01/23/08	V1C	12/27/07	01/23/08	Floor Loads-
HS2	12/27/07	01/23/08	V1D	12/27/07	01/23/08	TC Live: 40.0 psf
HS3	12/27/07	01/23/08	V2A	12/27/07	01/23/08	TC Dead: 10.0 psf
HSGRD	12/27/07	01/23/08	V2B	12/27/07	01/23/08	BC Live: 0.0 psf
J1	12/27/07	01/23/08				BC Dead: 05.0 psf
J1B	12/27/07	01/23/08	Floor Layout	09/26/07	01/23/08	Total 55.0 psf
J1BS	12/27/07	01/23/08	FA	01/15/08	01/23/08	DurFac Lbr: 1.00
J1F	12/27/07	01/23/08	FB	01/15/08	01/23/08	DurFac-Pit: 1,00
J2	12/27/07	01/23/08	FC	01/15/08	01/23/08	O.C. Spacing: 24.0"
J2B	12/27/07	01/23/08	FD	01/15/08	01/23/08	Compile
J2BS	12/27/07	01/23/08	FE	01/15/08	01/23/08	ompliance In
J2F	12/27/07	01/23/08				TANS EXAMINER
J2S	12/27/07	01/23/08				
J3	12/27/07	01/23/08	INV#	DESC	QNTY	
J3B	12/27/07	01/23/08	50060.0114	THD48		
J3BS	12/27/07	01/23/08	50060.0047	THD28		1
J3F	12/27/07	01/23/08	50060.0110	JUS26	32	1
J3S	12/27/07	01/23/08	50060.0058	THJ26	7	
J4	12/27/07	01/23/08	50060.0049	THD28-2		
J4B	12/27/07	01/23/08				
J4BS	12/27/07	01/23/08				
J4S	12/27/07	01/23/08	SEAT PLAT	ES	70	MAY 0 5 2008
						MAY O B ZIIIIN

FLOOR SEAT PLATES



### BCSI-Bl Summaky Sheel

### GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for Individual truss members. Refer to the BCSI-83 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more Information. All other permanent bracing design is the responsibility of the Building Designer.

personal injury or death.

The consequences of Improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious

El resultado de un manejo, levantamiento, Instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.







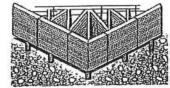
### HANDLING — MANEJO

Avoid lateral bending. — Evite la flexión lateral.



The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses quardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita venti-

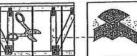
vea el folleto BCSI Guia de Buena Práctica. para el Manejo. Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabaio.

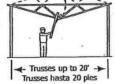
### NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto <u>BCST Guía de Buena Práctica</u> para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para formación más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restriction/Arriostre Permanente de Cuerdas y Miembros Secundados para más Información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.







HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES

Marning! Don't overload the crane.

with trusses up to 45'.

trusses greater than 60'.

paquetes más de 60 pies.

paquetes más de 60 ples.

Trusses 20

or less, sup-

port at peak.

del pico los

trusses de

20 pies o

Levante

trusses up to 60'.

iAdvertencia! iNo sobrecarga la grúa!

Never use banding alone to lift a bundle. Do not lift a group of Individually banded bundles.

RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Nunca use sólo los empaques para levantar un paquete.

No levante un grupo de empaques individuales.

A single lift point may be used for bundles

Usé at least 3 lift points for bundles with

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.

Puede usar dos puntos de levantar para

Use por lo menos tres puntos de levantar para

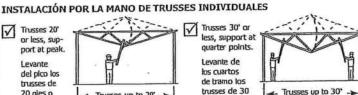
INSTALLATION OF SINGLE TRUSSES BY HAND

Two lift points may be used for bundles with

Trusses 30' or less, support at quarter points. Levante de

estable.

los cuartos de tramo los trusses de 30 nies a menas.



Trusses hasta 30 pies

⚠ Warning! Do not over load supporting

apoyada con el paquete de trusses.

✓ Place truss bundles in stable position.

iAdvertendal No sobrecargue la estructura

Puse paquetes de trusses en una posición

structure with truss bundle.

HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALI

| Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

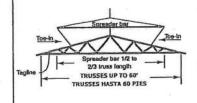
Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté Instalado y el truss está asegurado en los soportes.

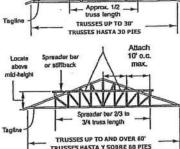
Warning! Using a single pick-point at the peak can damage the truss.

(Advertencial El uso de un solo lugar para levantar en el pico puede hacer daño al truss.

HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES





### TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Refer to BCSI-BZ Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

> Vea el resumen BCSI B2 - Restricción/ Arriostre Temporal y Instalación de los Trusses para más información.

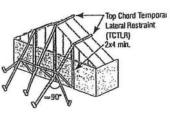
Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

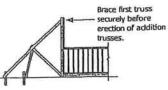
Coloque los arriostres de tierra para el primer truss directamente en linea con cada una de las filas de restricción lateral temporal de la ruenta superior (vea la table en la próxima columna).



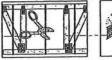
Do not walk on unbraced

No camine en brusses









cables eléctricos o de and airports. aeropuertos. Spreader bar for truss

Use special care in

windy weather or

near power lines

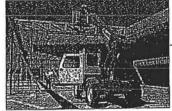


Use equipo apropiado para levantar e improvisar.

Utilice cuidado

especial en días

ventosos o cerca de





No almacene verticalmente los



Do not store on uneven around

No almacene en tierra desigual.



### CHANGE THE TANK OF THE PARTY OF

### *TEPS TO SETTING TRUSSES* AS MEDIDAS DE LA INSTALLACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). Repeat process on groups of four trusses until all trusses are set.
  - Instale los arriostres de tierra.
     Instale el primero truss y ate seguramente al arriostre de tlerra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y amostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses
- Refer to BCSI-B2 Summary Sheet Truss Installation & Temporary Restraint/Bracing for more

Vea el resúmen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

### **ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES** RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas

### TOP CHORD - CUERDA SUPERIOR

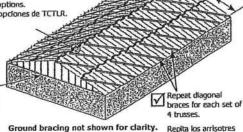
Truss Span	Top Chord Temporary Lateral Restraint (TCTLR) Spacing
Longitud de Tramo	Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 ples	10 ples måximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 ples	6 ples máximo
60' to 80'*	4' o.c. max.
60 a 80 pies*	4 pies máximo

\*Consult a Professional Engineer for trusses longer than 60'. \*Consulte a un Ingeniero para trusses de mas de 60 ples. See <u>BCSI-B2</u> for TCTLR options.

Vea el <u>BCSI-B2</u> para las opciones de TCTLR.

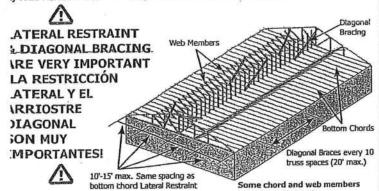
Refer to BCSI-B3 5ummary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/ reinforcement Information.

Para Información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.



Repita los arrisotres diagonales para cada grupo de 4 trusses.

### ) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS



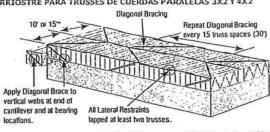
not shown for clarity. 3) BOTTOM CHORD - CUERDA INFERIOR Lateral Restraints - 2x4x12' or greater lapped over two truss Botton chords Diagonal Braces every 10 truss spaces (20' max.) ome chord and web members 10'-15

not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

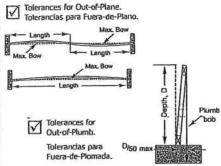
Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/ Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal v Permanente de Trusses de Cuerdas Paralelas para más información.



\*Top chord Temporary Lateral Restraint spacing shall be 10 o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

### INSTALLING — INSTALACION



Personal Property	CAP THEN
D/50	D (ft.)
1/4"	1"
1/2"	2.
3/4"	3,
1*	4"
1-1/4"	5'
1-1/2"	6*
1-3/4"	7.
2-	>8,

Gypsum Board

Asphalt Shingles

Concrete Block

Clay Tile

vood or OSA

S S S	
Max. Bow	Truss Length
3/4*	12.5
7/8*	14,6'
1*	16.7
1-1/8*	18.8
1-1/4"	20.8"
-1-3/8*-	22.9'
1-1/2"	25.0
1-3/4*	29.2
2-	≥33.3'

Helgh

12"

16"

3-4 Wes high

### CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma aproplada v segura.

Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resúmen BCSI-B4 Carga de Construcción para mayor información.



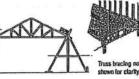


Do not overload small groups or single trusses. No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak Nunca amontone material cerca del pico.

Place loads over as many trusses as possible. Coloque las cargas sobre tantos trusses como sea posible

Position loads over load bearing walls. Coloque las cargas sobre las paredes soportantes.

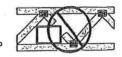


### **ALTERATIONS — ALTERACIONES**

Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors. Vea el resúmen <u>BCSI-B5 Daños de trusses. Modificaciones en la Obra y Errores de Instalación.</u>

Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté especificamente pecnitido en el dibujo del diseño del truss.



Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and vold.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantia del Fabricante de Trusses.

NOTE: The Thus Manufacturer and Thuss Designer raly on the presumption that the Contractor and crane operator (If applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent, party. The methods and procedures outlined in this document are intended to ensure that the overall construction interhylace employed will put the trusses into place SAFELY. These recommendations for handling, installing, restalling, and bracing trusses are based upon the collective experience of leading personnel invoked with trust seeking, manufacture and installation, but mast, thus the returned or responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design prediction for handling, installing, restraining and bracing trusses, and it does not preduce the use of other equivalent methods for restraining/bracing and providing stability for the wells, columns, floors, roots and all the intermetated structural building components as determined by the Contractor. Thus, WTCA and TPI expressy disablem any responsibility for demages artsing from the use, application, or relance on the recommendations and information contained herein.



6300 Enterprise Lanc • Madison, WI 53719 w.sbclndustry.com





MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To:

**Building Department** 

From: Maronda Systems

**Tomas Ponce** 

**Professional Engineer** 

State of Florida #0050068

Subject: Valley Trusses

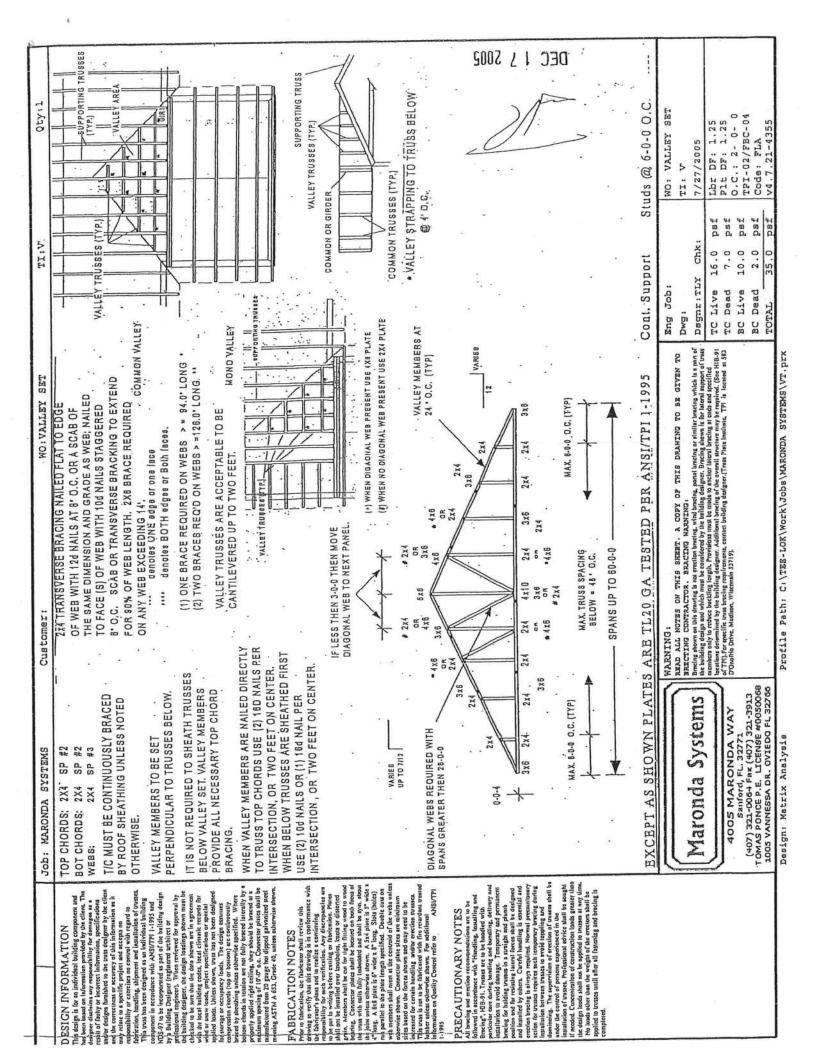
All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

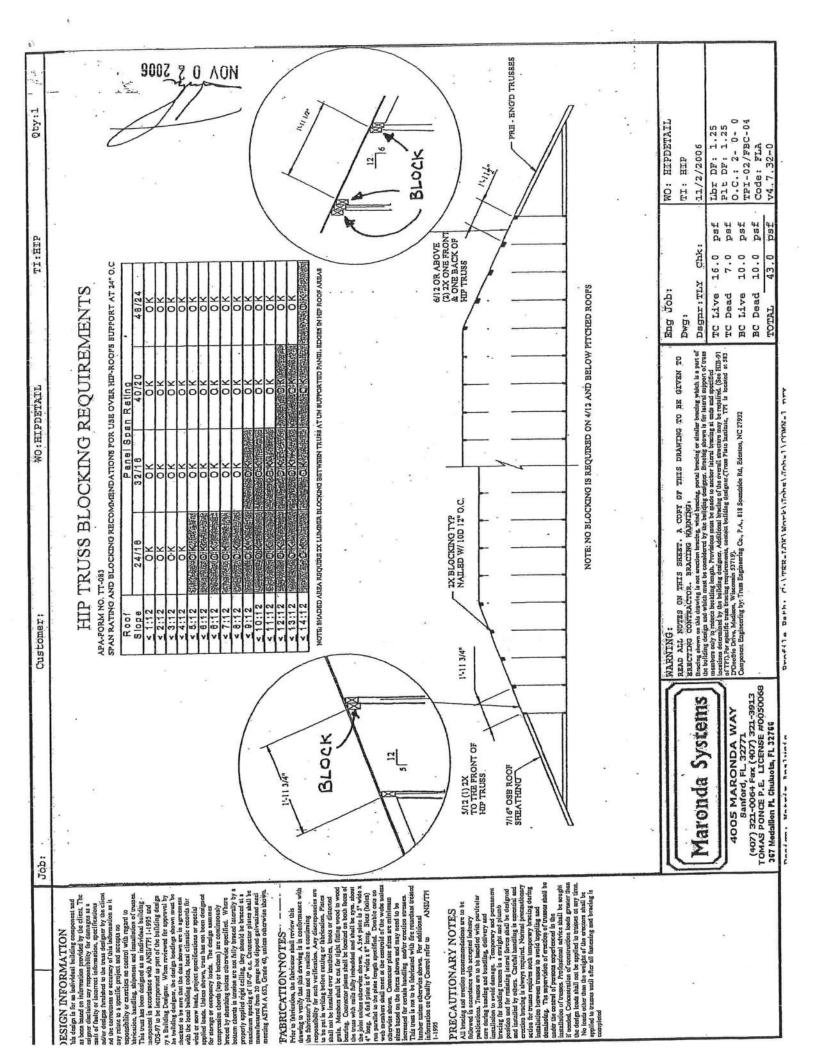
If you have any questions please feel free to call at 407-321-0064.

Sincerely,

Tomas Ponce, P.E.

Date: 11/1/06







Trenco

818 Soundside Rd Edenton, NC 27932

Re: SUNBURY

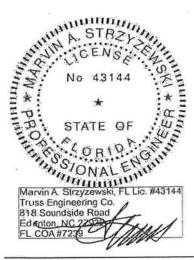
SUNBURY FLORIDA\_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4585782

thru E4585895

My license renewal date for the state of Florida is February 28, 2009.



December 27,2007

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2. Engineering services provided by Truss Engineering Company.

SUNBURY FLORIDA 125 Qty Truss Type Truss Job F4585783 SUNBURY HI Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:39 2007 Page 1 Maronda Homes Inc., Sanford, Florida 30-0-0 31-0-0 17-0-0 23-5-1 13-0-0 1-0-0 6-6-15 6-5-1 6-6-15 6-5-1 4-0-0 1-0-0 6-6-15 Scale = 1:56.9 4x8 = 4x6 = 5 6.00 12 3x4 3x4 3x6 > 11 14 10 3x8 = 13 12 3x8 = 2x4 || 2x4 || 3x4 = 3x4 = 30-0-0 13-0-0 17-0-0 23-5-1 6-6-15 6-5-1 4-0-0 Plate Offsets (X.Y): [2:0-4-12.0-1-8]. [4:0-5-4.0-2-0]. [7:0-2-10.0-1-8] PLATES GRIP L/d DEFL in (loc) Vdefl LOADING (psf) SPACING 2-0-0 -0.14 12-13 240 MT20 244/190 >999 Vert(LL) TCLL 16.0 Plates Increase 1.25 TC 0.30 -0.28 12-13 >999 180 Vert(TL) 0.57 7.0 Lumber Increase 1.25 BC TCDL 0.10 n/a n/a WB 0.53 Horz(TL) BCLL 10.0 Rep Stress Incr YES Weight: 157 lb Code FBC2004/TPI2002 (Matrix) BCDL BRACING LUMBER Structural wood sheathing directly applied or 3-9-11 oc purlins. TOP CHORD

**BOT CHORD** 

Rigid ceiling directly applied or 8-10-10 oc bracing.

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=1364/0-4-0, 7=1360/0-4-0

Max Horz 2=123(LC 6)

Max Uplift2=-300(LC 6), 7=-300(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-2387/712, 3-4=-1747/608, 4-5=-1502/605, 5-6=-1738/608, 6-7=-2377/712, 7-8=0/21

2-13=-494/2080, 12-13=-494/2080, 11-12=-247/1510, 11-14=-247/1510, 10-14=-247/1510, 9-10=-494/2071, **BOT CHORD** 

3-13=0/295, 3-12=-654/279, 4-12=-67/538, 4-10=-140/107, 5-10=-67/517, 6-10=-653/279, 6-9=0/295 WEBS

### NOTES

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2 and 300 lb uplift at joint 7.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not lruss designer. Bracing shown is for tateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the undiding designer, for general guidance regarding of the overall structure is the responsibility of the undiding designer, for general guidance regarding obsciously. ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



SUNBURY\_FLORIDA\_125 Job Truss Type Truss F4585785 SUNBURY HIB Job Reference (optional) Maronda Homes Inc., Sanford, Florida 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:41 2007 Page 1 31-0-0 1-0-0, 7-7-10 1-0-0 7-7-10 7-4-6 1-0-0 7-4-5 7-7-10 1-0-0 Scale = 1:56.3 4×8 = 4x8 = 5.00 12 4 5 5x6 = 5x6 > 3x6 > 3x6 = 11 10 2x4 ii 2x4 || 6x12 = 15-0-0 31-0-0 16-0-0 7-7-10 7-4-6 1-0-0 7-4-6 7-7-10 Plate Offsets (X,Y): [2:0-3-0.0-1-8], [3:0-3-0.0-3-0], [4:0-4-0.0-1-13], [5:0-4-0.0-1-13], [6:0-3-0.0-3-0], [7:0-3-0.0-1-8] PLATES LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) **V**defi L/d GRIP 244/190 TCLL 16.0 Plates Increase 1.25 TC 0.43 Vert(LL) -0 19 10-11 >999 240 MT20 -0.39 10-11 TCDL 7.0 Lumber Increase 1.25 BC 0.68 Vert(TL) >934 180 WB 0.24 0.12 BCLL 100 Rep Stress Incr YES Horz(TL) n/a n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 150 lb BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 3-5-5 oc purlins. BOT CHORD Rigid ceiling directly applied or 8-0-15 oc bracing. BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2 WEBS 3-10, 6-10 WEBS

REACTIONS (lb/size) 2=1368/0-8-0, 7=1368/0-8-0 Max Horz 2=-115(LC 7)

Max Uplift2=-328(LC 6), 7=-328(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-2635/787, 3-4=-1787/616, 4-5=-1623/633, 5-6=-1787/616, 6-7=-2635/787, 7-8=0/18 TOP CHORD

**BOT CHORD** 2-11=-593/2372, 10-11=-593/2372, 9-10=-593/2372, 7-9=-593/2372

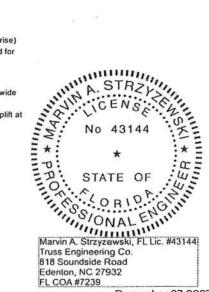
WEBS 3-11=0/332, 3-10=-858/327, 6-10=-858/327, 6-9=0/332, 4-10=-121/591, 5-10=-121/591

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone, Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 2 and 328 lb uplift at joint 7.

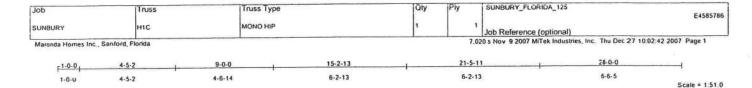
LOAD CASE(S) Standard

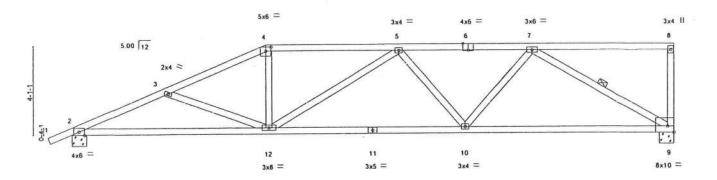


December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Piale Institute. S83 D'Onatio Drive. Modison. WI 53719.







	9-4-4				9-7-12					
Plate Offsets (X.Y); [4:0-3-0.0-2-4], [6:0-3-0.Edge], [9:Edge,0-3-8]										
TCDL 7.0 Lumb BCLL 10.0 • Rep S	cing 2-0-0 s Increase 1.25 er Increase 1.25 Stress Incr YES FBC2004/TPI2002	CSI TC 0.52 BC 0.76 WB 0.39 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.50 -0.42 -0.10	9-10	l/defl >656 >792 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 139 lb	<b>GRIP</b> 244/190	

TOP CHORD

**BOT CHORD** 

WEBS

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 9=1182/0-8-0, 2=1249/0-8-0

Max Horz 2=184(LC 6)

9-0-0

Max Uplift9=-728(LC 5), 2=-711(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension 1-2=0/18, 2-3=-2345/2382, 3-4=-2141/2257, 4-5=-1962/2150, 5-6=-1991/2182, 6-7=-1991/2182, 7-8=-105/120, TOP CHORD

8-9=-149/143

2-12=-2309/2119, 11-12=-2318/2138, 10-11=-2318/2138, 9-10=-1651/1530 **BOT CHORD** 

3-12=-190/204, 4-12=-634/548, 5-12=-208/197, 5-10=-232/215, 7-10=-836/726, 7-9=-1659/1781 WEBS

### NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 728 lb uplift at joint 9 and 711 lb uplift at joint 2

LOAD CASE(S) Standard



28-0-0

Structural wood sheathing directly applied or 3-9-9 oc purlins, except

Rigid ceiling directly applied or 3-8-14 oc bracing.

7-9

1 Row at midpt

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing show is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the rector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobication, qualify control, storage, desivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onolrio Drive, Madison, WI 53719.



SUNBURY FLORIDA 125 Oty Truss Type Job Truss E4585790 SUNBURY H2S Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:44 2007 Page 1 Maronda Homes Inc., Sanford, Florida 30-0-0 31-0-0, 19-0-0 24-2-12 11-0-0 5-9-4 5-2-12 5-2-12 8-0-0 5-9-4 Scale = 1:52.9 4x8 = 4x6 = 3 6.00 12 3×4 = 3x4 > 5-10-3 3x8 = 13 10 3x6 = 12 2x4 || 3×8 = 2×4 11 3x4 = 3×5 = 30-0-0 19-0-0 24-2-12 5-9-4 5-9-4 5-2-12 5-9-4 5-2-12 8-0-0 [1:0-1-13.0-1-8], [3:0-5-4.0-2-0], [4:0-3-8.0-2-4], [6:0-4-12.0-1-8] Plate Offsets (X,Y): **PLATES** GRIP DEFL L/d (loc) **V**defI LOADING (psf) SPACING 2-0-0 CSI 240 MT20 244/190 -0.21 9-11 >999 Vert(LL) 16.0 Plates Increase 1.25 TC 0.40 TCLL >861 180 -0.41 9-11 0.75 Vert(TL) Lumber Increase 1.25 BC TCDL 0.10 n/a n/a Horz(TL) 6 10.0 Rep Stress Incr YES WB 0.25 BCLL Weight: 149 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 3-9-9 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 8-4-1 oc bracing. BOT CHORD 2 X 4 SYP No.2 WEBS 1 Row at midpt 3-9 2 X 4 SYP No.2 WEBS REACTIONS (lb/size) 1=1329/0-4-0, 6=1371/0-4-0

Max Horz 1=-117(LC 7)

Max Uplift1=-212(LC 6), 6=-285(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-2446/759, 2-3=-1998/679, 3-4=-1724/664, 4-5=-1958/675, 5-6=-2423/737, 6-7=0/21 TOP CHORD

1-12=-543/2128, 11-12=-543/2128, 11-13=-355/1761, 10-13=-355/1761, 9-10=-355/1761, 8-9=-521/2099, **BOT CHORD** 

2-12=0/212, 2-11=-437/214, 3-11=-17/578, 3-9=-161/77, 4-9=-11/532, 5-9=-445/193, 5-8=0/213 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

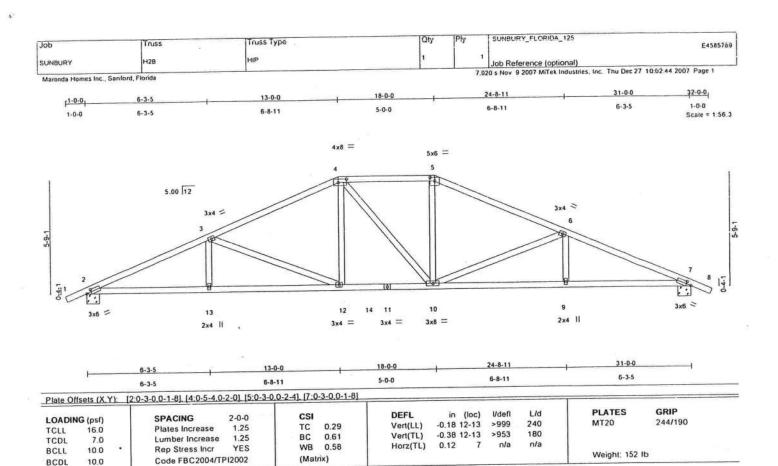
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 212 lb uplift at joint 1 and 285 lb uplift at joint 6.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USB. Design volid for use only with Milek connectors. This design is based only upon parameters and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permonent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobrocation, quality control, storage, delivery, erection and bracing, consult. ANSI/TPI Quality Cifferia, DSB-89 and BCS11 Building Component Safety Information.





TOP CHORD

**BOT CHORD** 

### LUMBER

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

10.0

2 X 4 SYP No.2

REACTIONS (lb/size) 2=1399/0-8-0, 7=1393/0-8-0

Max Horz 2=-103(LC 7)

Max Uplift2=-314(LC 6), 7=-314(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-2802/826, 3-4=-2080/692, 4-5=-1863/693, 5-6=-2065/692, 6-7=-2787/826, 7-8=0/18 TOP CHORD

2-13=-643/2516, 12-13=-643/2516, 12-14=-396/1878, 11-14=-396/1878, 10-11=-396/1878, 9-10=-643/2503, **BOT CHORD** 

7-9=-643/2503

3-13=0/280, 3-12=-704/266, 4-12=-30/530, 4-10=-157/113, 5-10=-30/506, 6-10=-705/266, 6-9=0/281 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 314 lb uplift at joint 2 and 314 lb uplift at joint 7.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-6-11 oc purlins.

Rigid ceiling directly applied or 7-9-13 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not huss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the control permanent bracing of the overall structure is the responsibility of the world structure is the responsibility of the control permanent bracing of the overall structure is the responsibility of the valid property of the control structure is the responsibility of the valid property of the valid pr



Edenion NC 27932

SUNBURY\_FLORIDA\_125 Qty Truss Type Truss Job E4585793 H3S SUNBURY Job Reference (optional) 7 020 5 Nov. 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:46 2007. Page 1 Maronda Homes Inc., Sanford, Florida 25-5-6 30-0-0 31-0-0 15-0-0 21-0-0 4-6-10 9-0-0 1-0-0 6-0-0 6-0-0 4-5-6 4-6-10 4-5-6 4-6-10 Scale = 1:53.0 4x6 = 4x6 = 3×8 = 5 6.00 12 2x4 = 2×4 4-10-3 3x8 = 11 10 3×8 12 3×4 = 3x8 = 3x8 = 2×4 || 21-0-0 30-0-0 15-0-0 9-0-0 9-0-0 6-0-0 6-0-0 9-0-0 Plate Offsets (X,Y): [1:0-8-0.0-0-8], [7:0-8-4.0-0-10] PLATES GRIP CSI DEFL (loc) **Vdefi** L/d SPACING 2-0-0 LOADING (psf) 244/190 -0.20 >999 240 MT20 1.25 TC 0.38 Vert(LL) 1-12 Plates Increase TCLL 16.0 180 1.25 BC 0.81 Vert(TL) -0.451-12 >789 Lumber Increase TCDL 7.0 YES WB 0.36 Horz(TL) 0.10 n/a n/a Rep Stress Incr BCLL 10.0 Weight: 149 lb (Matrix) Code FBC2004/TPI2002 BCDL 10.0 BRACING LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 3-8-9 oc purlins.

Rigid ceiling directly applied or 7-11-15 oc bracing.

2 X 4 SYP No.2

REACTIONS (lb/size) 1=1273/0-4-0, 7=1332/0-4-0

Max Horz 1=-102(LC 7)

Max Uplift1=-194(LC 6), 7=-267(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-2272/828, 2-3=-2057/720, 3-4=-1818/698, 4-5=-1820/690, 5-6=-2058/711, 6-7=-2276/809, 7-8=0/21 TOP CHORD

1-12=-617/1996, 11-12=-564/2107, 10-11=-564/2107, 9-10=-564/2107, 7-9=-594/2001 **BOT CHORD** 

2-12=-225/206, 3-12=-94/658, 4-12=-445/176, 4-10=0/168, 4-9=-441/178, 5-9=-86/657, 6-9=-228/192 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 1 and 267 lb uplift at joint 7.

LOAD CASE(S) Standard



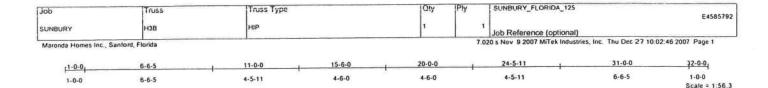
December 27,2007

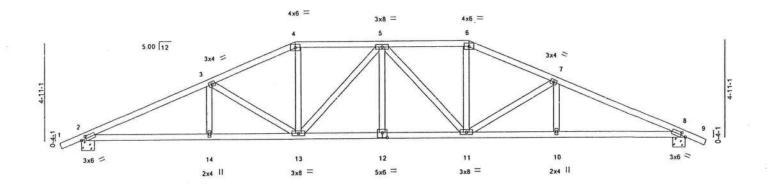
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

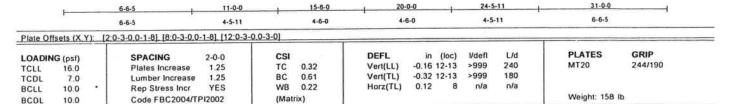
Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not furst designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erection. Additional permanent bracing of the overall structure is the responsibility of the distriction, quality control, storage, delivery, erection and bracing, consult.

ARSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafria Drive, Modison, WI 537 19.









TOP CHORD

**BOT CHORD** 

LUMBER

WEBS

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

REACTIONS (lb/size) 2=1368/0-8-0, 8=1368/0-8-0

Max Horz 2=90(LC 6)

Max Uplift2=-297(LC 6), 8=-297(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-2681/837, 3-4=-2165/766, 4-5=-1976/743, 5-6=-1976/743, 6-7=-2165/766, 7-8=-2681/837, 8-9=0/18 TOP CHORD

2-14=-646/2402, 13-14=-646/2402, 12-13=-551/2154, 11-12=-551/2154, 10-11=-646/2402, 8-10=-646/2402 BOT CHORD

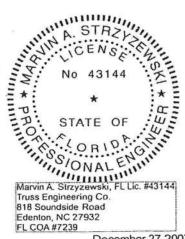
3-14=0/245, 3-13=-524/192, 4-13=-120/622, 5-13=-348/97, 5-12=0/176, 5-11=-348/97, 6-11=-120/622, 7-11=-524/192, WEBS 7-10=0/245

### NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 297 lb uplift at joint 8.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-6-12 oc purlins.

Rigid ceiling directly applied or 7-9-13 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

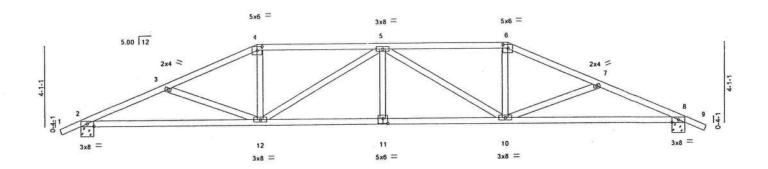
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ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.



818 Soundside Road

Job	Truss	T	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125		E458579
SUNBURY	H4B	н	HP.	1		Job Reference (optional)		
Maronda Homes Inc.,	, Sanford, Florida		3		7	.020 s Nov 9 2007 MiTek Industries, Inc.	Thu Dec 27 10:02:4	7 2007 Page 1
(1-0-0,	4-5-2	9-0-0	15-6-0	22-	0-0	26-6-14	31-0-0	32-0-0
1.0.0	4-5-2	4-6-14	6-6-0	6-6	6-0	4-6-14	4-5-2	1-0-0



1	The state of	9-0-0		1	2-0-0		22-0-0						
9-0-0					6-6-0 6-6-0			9-0-0					
Plate Offsets (X.)	(): [2:0	0-4-2.0-1-8]. [4:0-3-0.0-	2-4]. [6:0-3-0	0.0-2-4]. [7:0	-0-0,0-0-0].[	8:0-4-2.0-1-8].[1	1:0-3-0.	0-3-0]			1		
LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL 16.0		Plates Increase	1.25	TC	0.33	Vert(LL)	0.000	2-12	>999	240	MT20	244/190	
TCDL 7.0	- 1	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.47	2-12	>775	180			
BCLL 10.0	•	Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.13	8	n/a	n/a			
BCDL 10.0		Code FBC2004/TF	212002	(Mati	rix)						Weight: 149	lb	

TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=1368/0-8-0, 8=1368/0-8-0

Max Horz 2=-77(LC 7)

Max Uplift2=-288(LC 4), 8=-288(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-2620/912, 3-4=-2425/814, 4-5=-2229/801, 5-6=-2229/801, 6-7=-2425/814, 7-8=-2620/912, 8-9=0/18 TOP CHORD

2-12=-726/2370, 11-12=-770/2655, 10-11=-770/2655, 8-10=-726/2370 BOT CHORD

3-12=-185/184, 4-12=-63/651, 5-12=-583/220, 5-11=0/205, 5-10=-583/220, 6-10=-63/651, 7-10=-185/185 WEBS

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 288 lb uplift at joint 2 and 288 lb uplift at joint 8.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-6-9 oc purlins.

Rigid ceiling directly applied or 7-1-1 oc bracing.

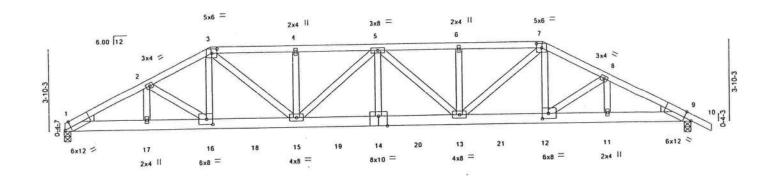
December 27,2007

Scale = 1:56.3

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not furst designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the dispers. For general guidance regarding tobaccation, quality control, storage, delivery, erection and bracing, consult. ARSI/TRI Quality Cifferta, DSB-89 and BCS11 Building Component Safety Information.



Job	Truss	Truss Type		Oty	Ply	SUNBURY_FLORIDA_125		E458580
SUNBURY	HGRDS	HIP		1	1	Job Reference (optional)		
Maronda Homes Inc., Sanfo	ord, Florida				7.020	s Nov 9 2007 MiTek Industries, Inc. 1	Thu Dec 27 10:02:5	5 2007 Page 1
2497	. 7-0-0	11-0-14	, 15-0-8	18-11-2	2	3-0-0 26-0-12	30-0-0	31-0-0
3-11-4	3-0-12	4-0-14	3-11-10	3-10-10	. 4	-0-14 3-0-12	3-11-4	1-0-0



112	3-11-4	7-0-0	11-0-14	15-0-8	10-11-2	-					# 15300101W/T	
1	3-11-4	3-0-12	4-0-14	3-11-10	3-10-10			4-0-14		3-0-12	3-11-4	
ate Off:	sets (X.Y): [1	1:0-3-13.Edge], [3:0-3-0.0	0-2-7]. [7:0-3-0	.0-2-7]. [9:0-4-6.Edge]. [	12:0-3-8,0-3-0]. [	14:0-5-	0.Edge	]. [16:0-:	3-8,0-3-0]			
OADING CLL CDL	G (psf) 16.0 7.0	SPACING Plates Increase	2-0-0 1.25 1.25	CSI TC 0.58 BC 0.91	DEFL Vert(LL) Vert(TL)	in 0.40 -0.71	(loc) 14 14	Vdefl >891 >502	L/d 240 180	PLATE MT20	244/1	
CLL	10.0	Rep Stress Incr	NO 212002	WB 0.38 (Matrix)	Horz(TL)	0.16	9	n/a	n/a	Weight	t: 187 lb	

TOP CHORD

**BOT CHORD** 

23-0-0

### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D 2 X 4 SYP No.2

WEBS WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 1=2911/0-4-0, 9=2962/0-4-0

Max Horz 1=-88(LC 6)

Max Uplift1=-1220(LC 4), 9=-1245(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-5851/2534, 2-3=-5830/2665, 3-4=-6577/3043, 4-5=-6577/3043, 5-6=-6585/3048, 6-7=-6585/3048, 7-8=-5849/2674, TOP CHORD

8-9=-5874/2546, 9-10=0/23

1-17=-2252/5158, 16-17=-2252/5158, 16-18=-2325/5167, 15-18=-2325/5167, 15-19=-3196/7077, 14-19=-3196/7077, BOT CHORD 14-20=-3196/7077, 13-20=-3196/7077, 13-21=-2284/5183, 12-21=-2284/5183, 11-12=-2218/5189, 9-11=-2218/5189

2-17=-81/143, 2-16=-187/173, 3-16=-544/1179, 3-15=-913/1905, 4-15=-165/126, 5-15=-700/342, 5-14=-283/678,

5-13=-696/341, 6-13=-162/124, 7-13=-909/1893, 7-12=-545/1194, 8-12=-187/149, 8-11=-90/153

### NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1220 lb uplift at joint 1 and 1245 lb uplift
- at joint 9. 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 784 lb down and 461 lb up at 7-0-0, 243 lb down and 155 lb up at 9-0-12, 243 lb down and 155 lb up at 11-0-12, 243 lb down and 155 lb up at 13-0-12, 243 lb down and 155 lb up at 15-0-0, 243 lb down and 155 lb up at 16-11-4, 243 lb down and 155 lb up at 18-11-4, and 243 lb down and 155 lb up at 20-11-4, and 784 lb down and 461 lb up at 22-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

Continued on page 2



30-0-0

26-0-12

Structural wood sheathing directly applied or 2-1-14 oc purlins.

Rigid ceiling directly applied or 4-8-1 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

			Oty	TPIv	SUNBURY_FLORIDA_125	
Job	Truss	Truss Type	City	r ly	GONDON (_reomon_ree	E4585804
SUNBURY	HGRDS	ніР	1	1	Job Reference (optional)	
					- 11 - 0.0007 117-1 to d string top. Thu Dog	27 10:02 EE 2007 Page 2

Marenda Homes Inc., Sanford, Florida

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-46, 3-7=-46, 7-10=-46, 1-9=-40

Concentrated Loads (lb)

Vert: 16=-784(F) 15=-243(F) 14=-243(F) 13=-243(F) 12=-784(F) 18=-243(F) 19=-243(F) 20=-243(F) 21=-243(F)



Job		นธร		russ Type	l diy	1.7			E4585801
SUNBURY	н	GROB	н	IP	1		Job Reference (optional)		
Maronda Homes In-	c., Sanford, Flori	da				7,0	20 s Nov 9 2007 MiTek Industries, Inc. Th	hu Dec 27 10:02:53	2007 Page 1
	4.0.5	7-0-0	0	15-6-0		24-0-0	26-11-11	31-0-0	32-0-0

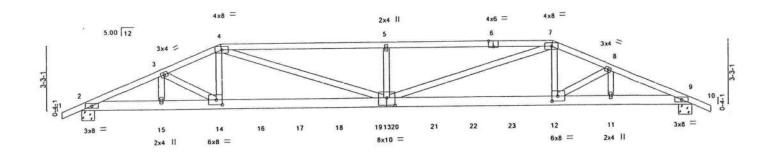


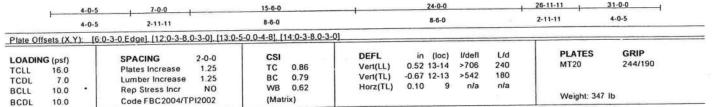
Ply

SUNBURY FLORIDA 125

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 7-11-0 oc bracing.





BRACING

TOP CHORD

**BOT CHORD** 

LUMBER	
TOP CHORD	2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.2 WEBS

2 X 4 SYP No.2

REACTIONS (lb/size) 2=3058/0-8-0, 9=3067/0-8-0

Max Horz 2=-66(LC 6)

Max Uplift2=-1899(LC 3), 9=-1747(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/20, 2-3=-6652/4213, 3-4=-7170/4824, 4-5=-9289/6242, 5-6=-9289/6242, 6-7=-9289/6242, 7-8=-7197/4346,

TOP CHORD

8-9=-6671/3887, 9-10=0/20 2-15=-3792/6042, 14-15=-3792/6042, 14-16=-4347/6574, 16-17=-4347/6574, 17-18=-4347/6574, 18-19=-4347/6574, **BOT CHORD** 

13-19=-4347/6574, 13-20=-3928/6598, 20-21=-3928/6598, 21-22=-3928/6598, 22-23=-3928/6598, 12-23=-3928/6598,

11-12=-3506/6059, 9-11=-3506/6059

3-15=-440/486, 3-14=-696/791, 4-14=-1043/1464, 4-13=-1897/2942, 5-13=-383/275, 7-13=-2348/2916, 7-12=-712/1483,

8-12=-533/801, 8-11=-447/353

### NOTES

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc

Boltom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1899 lb uplift at joint 2 and 1747 lb uplift at joint 9.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 752 lb down and 628 lb up at 7-0-0, 234 lb down and 267 lb up at 9-0-12, 234 lb down and 267 lb up at 11-0-12, 234 lb down and 267 lb up at 13-0-12, 234 lb down and 267 lb up at 15-0-12, 234 lb down and 267 lb up at 15-11-4, 234 lb down and 267 lb up at 17-11-4, 240 lb down and 148 lb up at 19-11-4, and 240 lb down and 148 lb up at 21-11-4, and 759 lb down and 509 lb up at 23-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. Continued on page 2

December 27,2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milet connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not lruss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permonent bracing of the overall structure is the responsibility of the wildling designer, for general guidance regarding designer, during control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI S3719.



			lOty	Plv	SUNBURY FLORIDA 125		
Job	Truss	Truss Type	City	Piy	SUNBURT_FLURIDA_123		E4585801
SUNBURY	HGRDB	HIP	1	2	Job Reference (optional)	*	
	N. C. S.				Job Reference (optional)		7

Maronda Homes Inc., Sanford, Florida

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:53 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf) Vert: 1-4=-46, 4-7=-46, 7-10=-46, 2-9=-40

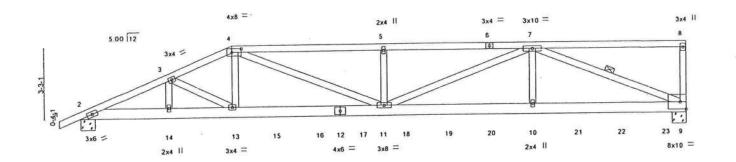
Concentrated Loads (lb)

Vert: 14=-752(B) 12=-759(B) 16=-234(B) 17=-234(B) 18=-234(B) 19=-234(B) 20=-234(B) 21=-234(B) 22=-240(B) 23=-240(B)



818 Soundside Road Edenton, NC 27932

Job	Truss		Truss Type	Qly	SUNBURY_FEORIDA_125	E4585803
SUNBURY	HGRD	С	MONO HIP	' 2	Job Reference (optional)	
Maronda Homes In	nc., Sanford, Florida			7.0	20 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10	1.02:54 2007 Page 1
c1-0-0	4-0-5	7-0-0	14-0-9	20-11-7	28-0-0	
1-0-0	4-0-5	2-11-11	7-0-9	6-10-13	7-0-9	Scale = 1:50.8



1	4-0-5	1-0-0		14-0-3						
	4-0-5			7-0-9	100	6-10-13			7-0-9	
Plate Offsets (X	Y): [2:	0-0-0,0-0-0]. [3:0-0-0.0	0-0] [4:0-5-4	0-2-0], [5:0-0-0,0-0-0], [6	0-0-0.0-0-0]. [7:0-0	0,0-0-0]. [8	3:0-0-0.0	-0-0]		SUBSERVE V
LOADING (psf) FCLL 16.0 FCDL 7.0		SPACING Plates Increase Lumber Increase	2-0-0 1.25 1.25	CSI TC 0.45 BC 0.58	Vert(TL) -0	in (loc) 35 11-13 43 11-13	l/defl >957 >771	L/d 240 180	PLATES MT20	<b>GRIP</b> 244/190
BCLL 10.0 BCDL 10.0		Rep Stress Incr Code FBC2004/TI	NO 212002	WB 0.51 (Matrix)	Horz(TL) 0	.08 9	n/a	n/a	Weight: 325 II	b

LUMBER

TOP CHORD

**BOT CHORD** 

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.2 WEBS

BRACING **BOT CHORD** 

WEBS

TOP CHORD

Structural wood sheathing directly applied or 4-10-1 oc purlins, except

28-0-0

end verticals.

Rigid ceiling directly applied or 8-4-14 oc bracing. 1 Row at midpt

REACTIONS (lb/size) 9=3146/0-8-0, 2=2644/0-8-0

Max Horz 2=150(LC 5)

Max Uplift9=-2411(LC 4), 2=-1704(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/20, 2-3=-5723/3791, 3-4=-5978/4224, 4-5=-7146/5262, 5-6=-7146/5263, 6-7=-7146/5263, 7-8=-220/166,

8-9=-173/125

2-14=-3520/5194, 13-14=-3520/5194, 13-15=-3976/5585, 15-16=-3976/5585, 12-16=-3976/5585, 12-17=-3976/5585,

11-17=-3976/5585, 11-18=-4025/5404, 18-19=-4025/5404, 19-20=-4025/5404, 10-20=-4025/5404, 10-21=-4025/5404,

21-22=-4025/5404, 22-23=-4025/5404, 9-23=-4025/5404

3-14=-237/337, 3-13=-481/363, 4-13=-935/1360, 4-11=-1393/1685, 5-11=-311/224, 7-11=-1341/1888, 7-10=-881/1132,

7-9=-5617/4182

### NOTES

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2411 lb uplift at joint 9 and 1704 lb uplift
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 752 lb down and 628 lb up at 7-0-0, 234 lb down and 267 lb up at 9-0-12, 234 lb down and 267 lb up at 11-0-12, 234 lb down and 267 lb up at 13-0-12, 234 lb down and 267 ib up at 15-0-12, 234 ib down and 267 ib up at 17-0-12, 234 ib down and 267 ib up at 19-0-12, 234 ib down and 267 ib up at 21-0-12, 234 lb down and 267 lb up at 23-0-12, 234 lb down and 267 lb up at 25-0-12, and 234 lb down and 267 lb up at 27-0-12, and 274 lb down and 255 lb up at 27-10-4 on bottom chord. The design/selection of such connection device(s) is the Continued on page 2



December 27,2007

meters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USB. WARNING - Verify design para Design valid for use only with Milek connectors. This design is based only upon parameters hown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobological, quality control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onolrio Drive, Modison, WI 53719.



FL COA #7239

Job	Truss	Truss Type	Oly	Ply	SUNBURY_FLORIDA_125	E4585803
SUNBURY	HGRDC	MONO HIP	1	2	Job Reference (optional)	
SONDON	- 028-75 NOTES			7.02	20 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:02:54 2007 Pa	age 2

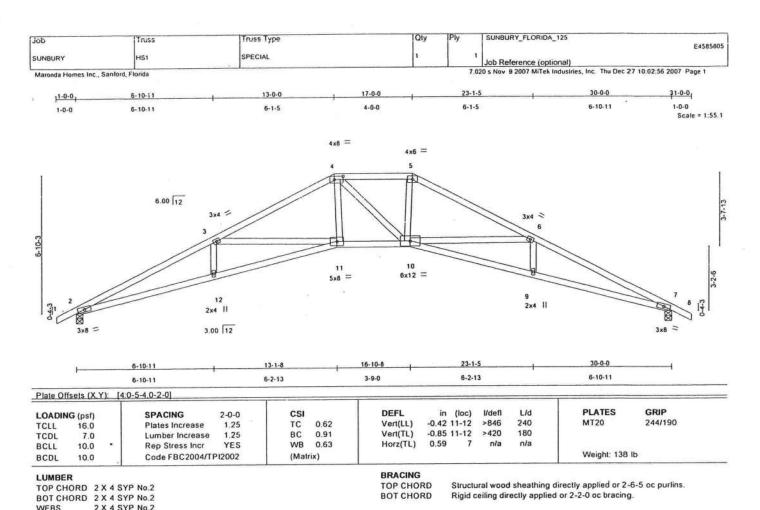
Maronda Homes Inc., Sanford, Florida

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)

Vert: 1-4=-46, 4-8=-46, 2-9=-40

Vert: 9=-274(F) 13=-752(F) 10=-234(F) 15=-234(F) 16=-234(F) 17=-234(F) 18=-234(F) 19=-234(F) 20=-234(F) 21=-234(F) 22=-234(F) 23=-234(F) Concentrated Loads (lb)



17.101.102

REACTIONS (lb/size) 2=1329/0-4-0, 7=1329/0-4-0 Max Horz 2=-121(LC 7)

Max Uplift2=-296(LC 6), 7=-296(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-4252/1190, 3-4=-3118/864, 4-5=-2821/846, 5-6=-3172/875, 6-7=-4249/1191, 7-8=0/18

BOT CHORD 2-12=-955/3873, 11-12=-954/3859, 10-11=-493/2824, 9-10=-956/3862, 7-9=-955/3871

WEBS 3-12=0/309, 3-11=-1005/441, 4-10=-165/156, 6-10=-959/432, 6-9=0/285, 4-11=-188/1150, 5-10=-194/1165

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2 and 296 lb uplift at joint 7.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. 
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not huss designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding stabication, quality control, storage, desivery, erection and bracing, cansul. AMSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onatrio Drive, Madison, WI 53719.



UNBURY	Truss HS2	Truss Type SPECIAL		Oty 1	Ply	SUNBURY_FLORIDA_125  Job Reference (optional)		E4585806
Maronda Homes Inc., S	Sanford, Florida				7.02	20 s Nov 9 2007 MiTek Indust	ries, Inc. Thu Dec 27 10	02.56 2007 Page 1
	5-10-11	11-0-0	, 15-0-0	19-0-0		24-1-5	30-0-0	31-0-0
1-0-0	5-10-11	5-1-5	4-0-0	4-0-0	10	5-1-5	5-10-11	1-0-0 Scale = 1:55.1
						100		
		100	<sub>1×6</sub> =					
			3×		4x6 =			
			4 5 (h) 29					
	6.00 12	x4 : //				<u> </u>		
	3	**				3x4 = 7		
								1 1
			12		11	8		φ
			x12 =		6x12 =	10		3. 6 8
3		13 4 II				2x4 II		, 14.
3x8 =	3.0	00 12						3x8 =
	5-10-11	11-1-8	18-1	0-8	-1	24-1-5	30-0-0	
-	5-10-11	5-2-13	7-9	1-0	9775	5-2-13	5-10-11	

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

TOP CHORD

**BOT CHORD** 

in (loc)

-0.47 11-12

-0.97 11-12

0.59

I/defl

>755

>368

n/a

7-3-3 oc bracing: 11-12.

1 /d

240

180

n/a

LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

16.0

7.0

10.0

10.0

2 X 4 SYP No.2

REACTIONS (lb/size) 2=1329/0-4-0, 8=1329/0-4-0

Max Horz 2=-105(LC 7)

Max Uplift2=-281(LC 6), 8=-281(LC 7)

SPACING

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-4269/1246, 3-4=-3541/1012, 4-5=-3156/962, 5-6=-3156/962, 6-7=-3541/1012, 7-8=-4269/1246, 8-9=0/18

2-13=-1015/3872, 12-13=-1013/3881, 11-12=-753/3255, 10-11=-1013/3881, 8-10=-1015/3872 **BOT CHORD** 

2-0-0

1.25

1.25

YES

3-13=0/195, 3-12=-633/327, 5-12=-252/187, 5-11=-252/186, 7-11=-633/327, 7-10=0/195, 6-11=-254/1328, WEBS

4-12=-254/1328

### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC

BC

WB 0.27

(Matrix)

0.46

0.95

- 3) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 281 lb uplift at joint 2 and 281 lb uplift at joint 8.

LOAD CASE(S) Standard



**PLATES** 

Weight: 138 lb

MT20

Structural wood sheathing directly applied or 2-8-8 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing, Except:

GRIP

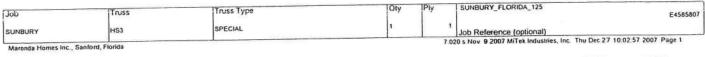
244/190

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE.

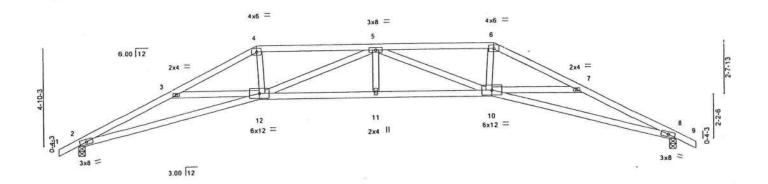
Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of entropy of the building designer. For general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TR1 Quality Criteria, DSB-89 and BCS11 Building Compone Safety Information.









	9-1-8 15-0-0 20-10-8 1 9-1-8 5-10-8 5-10-8		30-0-0		
1			5-10-8	9-1-8	
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	TC 0.49 BC 0.93 WB 0.41	DEFL in (loc) Vert(LL) -0.44 11 Vert(TL) -0.88 11 Horz(TL) 0.58 8		244/190

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

10-12 2 X 4 SYP No.1D

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=1329/0-4-0, 8=1329/0-4-0

Max Horz 2=-90(LC 7)

Max Uplift2=-263(LC 6), 8=-263(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4104/1344, 3-4=-3868/1137, 4-5=-3511/1084, 5-6=-3511/1084, 6-7=-3868/1137, 7-8=-4104/1344, TOP CHORD

2-12=-1111/3750, 11-12=-1086/4058, 10-11=-1086/4058, 8-10=-1111/3750 BOT CHORD

3-12=-177/287, 5-12=-716/290, 5-11=0/170, 5-10=-716/289, 7-10=-177/297, 4-12=-276/1461, 6-10=-276/1461 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2 and 263 lb uplift at

LOAD CASE(S) Standard



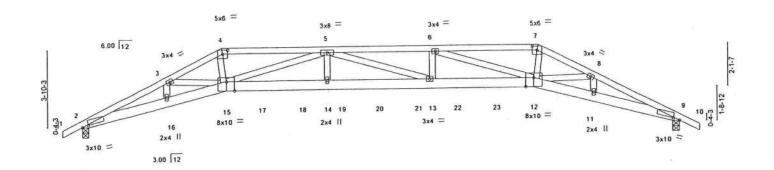
Structural wood sheathing directly applied or 2-7-14 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milex connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown Applicability of design parameters and proper incorporation of component is responsibility of the unit of the responsibility of the insure stability during construction is the responsibility of the is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector, additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the unditing designer. For general guidance regarding erector. Additional permanent procing of the responsibility of the substitute of the permanent permanent procing of the permanent p



Job	Tr	ušš	Truss Type		Oly	Ply	SUNBURY_FLORIDA_125		E4585811
SUNBURY	н	SGRD	SPECIAL		1	2	Job Reference (optional)		
Maronda Homes In	c , Sanford, Flori	da				7.00	20 s Nov 9 2007 MiTek Industries, In	c. Thu Dec 27 10.0	3 00 2007 Page 1
11-0-0	4-2-13	. 7-0-0	12-4-0	17-8-0		23-0	-0 25-9-3	30-0-0	31-0-0
1-0-0	4-2-13	2-9-3	5-4-0	5-4-0	Van	5-4	-0 2-9-3	4-2-13	1-0-0 Scale = 1:55.1



	4-2-13	337	5	-1-0	5-4-0	5-1	-0	3-0-3	4-2-1	3
OADING (psf)	Y): [2:0	0-3-14.0-0-6] [4:0-3-0. SPACING	2-0-0	0,0-2-7], [9:0-3-14.0- CSI TC 0.80	0-6], [12:0-4-12.0-5-4 <b>DEFL</b> Vert(LL)	in (loc) 0.74 13-14	l/defl >481	L/d 240	PLATES MT20	GRIP 244/190
LL 16.0 DL 7.0 LL 10.0		Plates Increase Lumber Increase Rep Stress Incr	1.25 1.25 NO	BC 0.94 WB 0.44	Vert(TL) Horz(TL)	-1.31 13-14 0.65 9	>272 n/a	180 n/a	Mainht 227	

TOP CHORD

**BOT CHORD** 

17-8-0

BCDL LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.2

10.0

REACTIONS (lb/size) 2=2963/0-4-0, 9=2963/0-4-0

Code FBC2004/TPI2002

Max Horz 2=78(LC 5)

Max Uplift2=-1259(LC 5), 9=-1259(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-10338/4556, 3-4=-11145/5131, 4-5=-10559/4887, 5-6=-13646/6285, 6-7=-10595/4855, 7-8=-11184/5097 TOP CHORD

(Matrix)

8-9=-10327/4507, 9-10=0/22

2-16=-4140/9363, 15-16=-4197/9483, 15-17=-6336/13789, 17-18=-6336/13789, 14-18=-6336/13789, **BOT CHORD** 

14-19=-6336/13789, 19-20=-6336/13789, 20-21=-6336/13789, 13-21=-6336/13789, 13-22=-6218/13646,

22-23=-6218/13646, 12-23=-6218/13646, 11-12=-4102/9478, 9-11=-4043/9353

3-16=-387/280, 3-15=-633/1001, 5-15=-3492/1705, 5-14=-378/904, 5-13=-225/124, 6-13=-357/848, 6-12=-3305/1615, WEBS

8-12=-663/1047, 8-11=-410/290, 4-15=-2160/4753, 7-12=-2138/4755

### NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1259 lb uplift at joint 2 and 1259 lb uplift at joint 9.

Continued on page 2



Weight: 327 lb

Structural wood sheathing directly applied or 2-11-4 oc purlins.

Rigid ceiling directly applied or 7-1-14 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USB Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. For a large shown is for lateral support of individual web members only. Additional temporary bracking to insure stability during construction is the responsibility of the certain bracking of the overall structure is the responsibility of the validing designer. For general guidance regarding erector. Additional permanent bracking of the overall structure is the responsibility of the validing designer. For general guidance regarding tobication, quality control, storage, delivery, erection and bracking, consult.

ANSI/TEN Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. Available from Truss Plate Institute, 583 D'Onafrio Drive, Modison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	4585811
SUNBURY	HSGRD	SPECIAL	1		Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 784 lb down and 467 lb up at 7-3-0, 243 lb down and 157 lb up at 9-0-12 . 243 lb down and 157 lb up at 11-0-12, 243 lb down and 157 lb up at 13-0-12, 243 lb down and 157 lb up at 15-0-0, 243 lb down and 157 lb up at 16-11-4, 243 lb down and 157 lb up at 18-11-4, and 243 lb down and 157 lb up at 20-11-4, and 784 lb down and 467 lb up at 22-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 4-7=-46, 7-10=-46, 2-15=-40, 12-15=-40, 9-12=-40

Concentrated Loads (lb)

Vert: 15=-784(B) 12=-784(B) 17=-243(B) 18=-243(B) 19=-243(B) 20=-243(B) 21=-243(B) 22=-243(B) 23=-243(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MID-7473 BEPORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. 
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fluss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of building designer. For general guidance regarding tobrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TELL Quality Criteria, DSB-89 and BCSL1 Building Component Safety Information.



SUNBURY FLORIDA 125 Truss Type Oty Truss Job E4585813 JACK SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:02 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 -1-0-0 2-7-4 1-0-0 4-4-12 Scale: 1/2"=1" 2x4 11 6,00 12 2x4 || 2x4 7-0-0 4-4-12

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

**BOT CHORD** 

-0.10

-0.26

0.00

2-4

2-4

BCDL LUMBER

TCIL

TCDL

BCLL

LOADING (psf)

16.0

7.0

10.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=349/0-4-0, 4=282/Mechanical

Max Horz 2=176(LC 6)

Max Uplift2=-111(LC 6), 4=-100(LC 6)

SPACING

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-105/54

2-4=0/0 **BOT CHORD** 

WEBS 3-4=-149/173

### NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC

BC

WB

(Matrix)

0.44

0.42

0.03

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2-0-0

1.25

1 25

YES

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 100 lb uplift at joint 4.

LOAD CASE(S) Standard



**PLATES** 

Weight: 29 lb

MT20

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

L/d

240

180

nla

**Vdefi** 

>765

>306

n/a

GRIP

244/190

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the during construction is the responsibility of the overall purpose. For general guidance regarding designer, for general guidance regarding of flobrication, qualify control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DS8-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719,



SUNBURY FLORIDA\_125 Oty Truss Type Job Truss E4585814 19 JACK JIB SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:02 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 5-8-6 -1-6-0 5-8-6 Scale = 1:20.8 2x4 || 3 5.00 12 0-4-1 3×4 7-0-0 1-3-10 DEFL PLATES CSI I/defl L/d LOADING (psf) SPACING 2-0-0 244/190 Vert(LL) 0.10 2-4 >800 240 MT20 TC 0.30 TCLL 16.0 Plates Increase 1.25 180 Vert(TL) 0.08 2-4 >999

Horz(TL)

BRACING

TOP CHORD

**BOT CHORD** 

0.00

n/a

n/a

Weight: 33 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.1D BOT CHORD 2 X 6 SYP No.2

7.0

10.0

10.0

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=351/0-8-0, 4=274/Mechanical

Max Horz 2=148(LC 6)

Max Uplift2=-238(LC 6), 4=-190(LC 6)

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-82/45

2-4=0/0 **BOT CHORD** 

WEBS 3-4=-143/151

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

BC

WB 0.02

(Matrix)

0.26

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2 and 190 lb uplift at joint 4.

6)

LOAD CASE(S) Standard



December 27,2007



Truss Type SUNBURY\_FLORIDA\_125 Job Truss E4585816 SUNBURY J1BS JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:03 2007 Page 1 Maronda Homes Inc., Sanford, Florida 7-0-0 5-3-10 5-3-10 1-B-6 Scale = 1:20.8 2x4 11 5.00 12 3 2x4 || 2x4 7-0-0

						5-3-10					1-8-6	
LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES GRIP
TCLL	16.0		Plates Increase	1.25	TC	0.44	Vert(LL)	-0.09	1-3	>841	240	MT20 244/190
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.23	1-3	>336	180	
BCLL	10.0		Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00		n/a	n/a	
BCDL	10.0		Code FBC2004/TF	P12002	(Matr	rix)						Weight: 26 lb

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 1=279/0-8-0, 3=279/Mechanical

Max Horz 1=120(LC 6)

Max Uplift1 = -41(LC 6), 3=-95(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-89/49 **BOT CHORD** 1-3=0/0

WEBS 2-3=-149/169

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 1 and 95 lb uplift at joint 3.

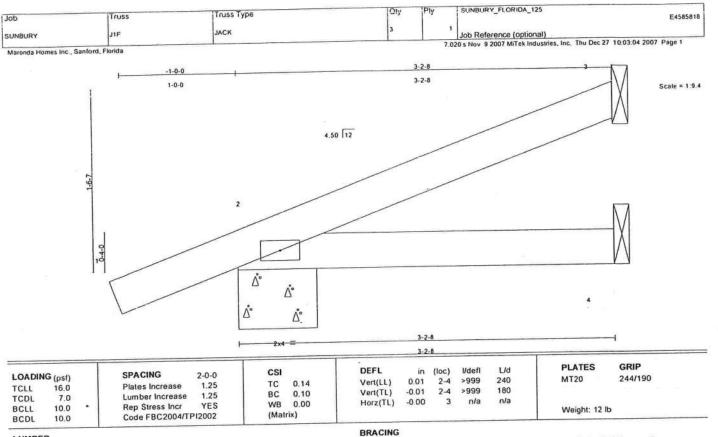
6)

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 7-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=49/Mechanical, 2=200/0-8-0, 4=56/Mechanical Max Horz 2=74(LC 6) Max Uplift3=-36(LC 6), 2=-164(LC 6), 4=-29(LC 4)

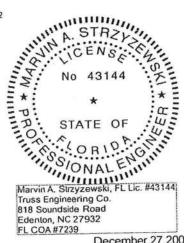
FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/16, 2-3=-32/14 TOP CHORD

2-4=0/0 **BOT CHORD** 

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 164 lb uplift at joint 2 and 29 lb uplift at joint 4.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-2-8 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding are too constructions of the overall structure is the responsibility of the building designer, for general guidance regarding are too constructions of the overall structure is the responsibility of the building designer, for general guidance regarding are structured. ANSI/TRI Quality Criteria, 058-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, 383 D'Onalifo Drive, Madison, WI 53719.



SUNBURY\_FLORIDA\_125 Qty Truss Type Truss Job E4585821 JACK J2 SUNBURY Job Reference (optional)

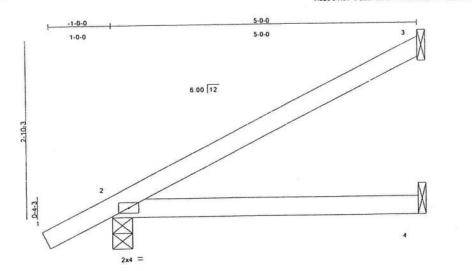
Maronda Homes Inc., Sanford, Florida

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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:17.8



GRIP PLATES DEFL CSI (loc) **Udefl** L/d SPACING 2-0-0 LOADING (psf) MT20 244/190 240 >999 Vert(LL) -0.032-4 TC 0.21 Plates Increase 1 25 16.0 TCLL 180 >842 0.22 Vert(TL) -0.07 2-4 BC 1.25 Lumber Increase TCDL 7.0 n/a n/a WB 0.00 Horz(TL) -0.00YES 10.0 Rep Stress Incr BCLL Weight: 18 lb (Matrix) Code FBC2004/TPI2002 BCDL 10.0

BRACING

TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=102/Mechanical, 2=269/0-4-0, 4=95/Mechanical

Max Horz 2=136(LC 6)

Max Uplift3=-95(LC 6), 2=-104(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

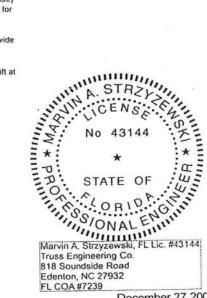
TOP CHORD 1-2=0/21, 2-3=-74/37

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3 and 104 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer. For liture, stocking the parameters are properties of component is responsibility of the sign parameters and proper incorporation of component is responsibility of the sign of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the suit of the support of the s



		Truss Type	Qiy	Ply	SUNBURY_FLORIDA_125	19-41-5-3
Job	Truss	lituss type	120	1	Separation of the separation o	E4585822
SUNBURY	J2B	JACK	5		Job Reference (optional)	
Maronda Homes Inc.,	Sanford, Florida			7.	020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec	27 10:03:05 2007 Page 1
			5-0-0			. 7
	-1-0-0	1			3	
	1-0-0		5-0-0			Scale = 1:13.4
	1					\  \
		20				
	1	5.0	00 12			
	মু					
	~		/ /			
	1	2				
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	14				100000000000000000000000000000000000000	
	191	<u>i</u> .				
		Δ Δ.			12	
		Δ A.				
	-	8				
		2x4 =	5-0-0		- 11	
			5-0-0			

DEFL

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

**BOT CHORD** 

l/defl

n/a

(loc)

2-4 >579

0.10

0.07 2-4 >744

-0.00

L/d

240

180

n/a

LUMBER

LOADING (psf)

16.0

7.0

10.0

10.0

TCIL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical

2-0-0

1.25

1.25

YES

Max Horz 2=113(LC 6)

Max Uplift3=-83(LC 6), 2=-195(LC 6), 4=-47(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-58/31

SPACING

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC

BC 0.27

WB

(Matrix)

0.18

0.00

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 83 lb uplift at joint 3, 195 lb uplift at joint 2 and 47 lb uplift at joint 4.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



PLATES

Weight: 17 lb

MT20

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

GRIP

244/190

December 27,2007

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ANSI/TPT Quality Citleria, DSB-89 and 8CS11 Building Component Safety Information.



Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	E4585824
SUNBURY	J2BS	JACK	1		Job Reference (optional)	
Maronda Homes Inc.	Sanford, Florida			,	7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec	. 10.03.03 2001 103
	1		5-0-0			
	· -		5-0-0 5-0-0		2	Scale = 1:13.4
		- 3			2	Scale = 1:13.4

	5-0-0	
	5-0-0	2
	8	
	5.00 12	
10		
1///		П
		V
44		M
9 0		
Δ-		3
Δ. Δ.		•
2x4 =	5-0-0	
	5-0-0	

LOADING (psf) TCLL 16.0 TCDL 7.0	SPACING Plates Increase Lumber Increase	2-0-0 1.25 1.25 YES	CSI TC 0.22 BC 0.20 WB 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.02 -0.06 -0.00	(loc) 1-3 1-3 2	Vdefl >999 >963 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190
BCLL 10.0 * BCDL 10.0	Rep Stress Incr Code FBC2004/T	100000000000000000000000000000000000000	(Matrix)				The state of the s	100001	Weight: 16 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 1=196/0-8-0, 2=105/Mechanical, 3=91/Mechanical Max Horz 1=87(LC 6) Max Uplift1=-28(LC 6), 2=-95(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-66/35 **BOT CHORD** 1-3=0/0

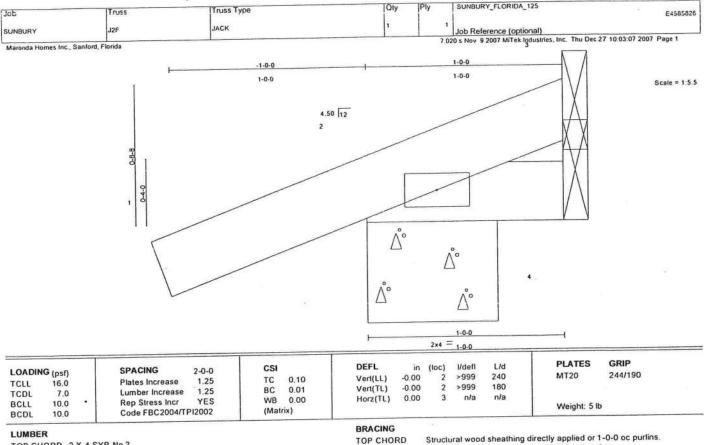
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1 and 95 lb uplift at joint 2.
- Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



December 27,2007





**BOT CHORD** 

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=122/0-8-0, 4=18/Mechanical, 3=-10/Mechanical

Max Horz 2=40(LC 6)

Max Uplift2=-113(LC 6), 3=-10(LC 1)

Max Grav2=122(LC 1), 4=18(LC 1), 3=24(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/16, 2-3=-21/19

**BOT CHORD** 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 2 and 10 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 REFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building a component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fruss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the didding designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the Mildling designer. For general guidance regarding fobication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onolrio Drive, Madison, WI 53719.



SUNBURY FLORIDA\_125 Truss Type E4585828 Truss Job JACK Job Reference (optional)
7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:08 2007 Page 1 J2S SUNBURY Maronda Homes Inc., Sanford, Florida 5-0-0 2 5-0-0 Scale = 1:15.4 6.00 12 GRIP PLATES Lld DEFL (loc)

-0.03

-0.07

-0.00

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

**BOT CHORD** 

1-3 >999

1-3

>865

nla

240

180

n/a

LUMBER

LOADING (psf)

TCLL

TCDL

BCLL

BCDL

16.0

7.0

10.0

10.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=203/0-4-0, 2=109/Mechanical, 3=95/Mechanical

2-0-0

1.25

1.25

YES

Max Horz 1=104(LC 6)

Max Uplift1=-22(LC 6), 2=-106(LC 6)

SPACING

Plates Increase

Rep Stress Incr

Lumber Increase

Code FBC2004/TPI2002

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-81/41 TOP CHORD 1-3=0/0 **BOT CHORD** 

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

CSI

TC

BC 0.21

WB 0.00

(Matrix)

0.23

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 106 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



244/190

MT20

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 16 lb

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component, applicability of design parameters and proper incorporation of component is responsibility of building designer and trust designer. Bracing shown applicability of lesign paramenters and proper incorporation of component is responsibility of building designer and trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding flobication, qualify control, storage, delivery, erection and bracing, consult.

ANSI/TPII Qualify Citledia, DSB-89 and BCS11 Building Component Safety Information. available from fruss Plate Institute. S83 D'Onolno Drive, Madison, WI S3719.



SUNBURY\_FLORIDA\_125 Qty Job Truss Truss Type E4585829 J3 IACK SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:08 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-0-0 3-0-0 1-0-0 Scale = 1:10.8 6.00 12 2 0-4-3 3-0-0 DEFL PLATES GRIP LOADING (psf) CSI **V**defl Lld SPACING in (loc)

LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

16.0

7.0

10.0

10.0

BRACING TOP CHORD BOT CHORD

Vert(LL)

Vert(TL)

Horz(TL)

-0.00

-0.01

-0.00

2-4

>999

>999

n/a

240

180

n/a

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MT20

Weight: 11 lb

REACTIONS (lb/size) 3=51/Mechanical, 2=188/0-4-0, 4=55/Mechanical

1.25

1.25

YES

Max Horz 2=95(LC 6)

Max Uplift3=-44(LC 6), 2=-101(LC 6)

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-36/18

BOT CHORD 2-4=0/0

NOTES (6)

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

TC

BC 0.07

WR

(Matrix)

0.12

0.00

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 3 and 101 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



244/190

December 27,2007

WARNING - Verify deelign parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEPORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. 
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not huss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobications, quality control, storage, defever, erection and bracing, consult. AMSI/TPI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Modison, WI 53719.



818 Soundside Road

SUNBURY\_FLORIDA\_125 Qty Truss Type Job E4585830 JACK SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:08 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3-0-0 1-0-0 3-0-0 1-0-0 5.00 12 1-6-0 а а Δ° ϰ 3-0-0 PLATES GRIP DEFL L/d CSI (loc) LOADING (psf) SPACING 2-0-0 MT20 244/190 0.01 2-4 >999 240 Vert(LL) 0.14 TC Plates Increase 1.25 TCII 16.0 180 Vert(TL) -0.01 2-4 >999 0.09 BC Lumber Increase 1.25 7.0 TCDL Horz(TL) -0.00 n/a n/a 0.00 WB Rep Stress Incr YES 10.0 BCLL Weight: 11 lb Code FBC2004/TPI2002 (Matrix) 10.0 BCDL BRACING LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 3=43/Mechanical, 2=193/0-8-0, 4=52/Mechanical

Max Horz 2=79(LC 6)

Max Uplift3=-33(LC 5), 2=-159(LC 6), 4=-27(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-33/13 TOP CHORD

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for
- members and forces, and for MWFRS for reactions specified. 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3, 159 lb uplift at joint 2
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not furs designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the area of the control support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the area of the control structure is the responsibility of the during designer, for general guidance regarding erection, additional permanent bracing of the overall structure is the responsibility of the during the structure of the control structure, and is consulted to the control structure of the control structure. ANSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information available from Iruss Plate Institute, 583 D'Onolria Drive, Modison, WI 53719.



lob	Truss	Truss Type		Qty	Ply	SUNBURY_FLORIDA_125	E4585833
SUNBURY	звѕ	JACK		1	1	Job Reference (optional)	
Maronda Homes Inc., San	ford, Florida			•	7.0	20 s Nov 9 2007 MiTek Industries, Inc.	Thu Dec 27 10:03:09 2007 Page 1
		1		1-0-0			
				3-0-0			Scale = 1:9.
			70-79-11	_			
			5.00 12				
					9		
	177						
		1	/ /				
	- i					V	
	440					\  \	
	١٩					/\	
		Δ.					
		$\nabla_{\mathbf{e}}$				3	
		Δ° Δ°					
		В					12
				3-0-0			
						DI DI	ATES GRIP
LOADING (psf)	SPACING	2-0-0	CSI	DEFL Ved(LL) -0	in (loc)	rucii Du	

BCDL LUMBER

TCLL

TCDL

BCLL

TOP CHORD 2 X 4 SYP No.2

16.0

7.0

10.0

10.0

BOT CHORD 2 X 4 SYP No.2

BRACING

Vert(LL)

Vert(TL)

Horz(TL)

-0.00

-0.01

-0.00

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 9 lb

>999

>999

n/a

1-3

240

180

n/a

REACTIONS (lb/size) 1=110/0-8-0, 2=59/Mechanical, 3=51/Mechanical Max Horz 1=52(LC 6) Max Uplift1=-14(LC 6), 2=-55(LC 6)

Plates Increase

Lumber Increase

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-40/21 TOP CHORD

BOT CHORD 1-3=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

TC

BC

WB

(Matrix)

0.07

0.06

0.00

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

1.25

YES

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
  5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1 and 55 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints2 and 3.

LOAD CASE(S) Standard

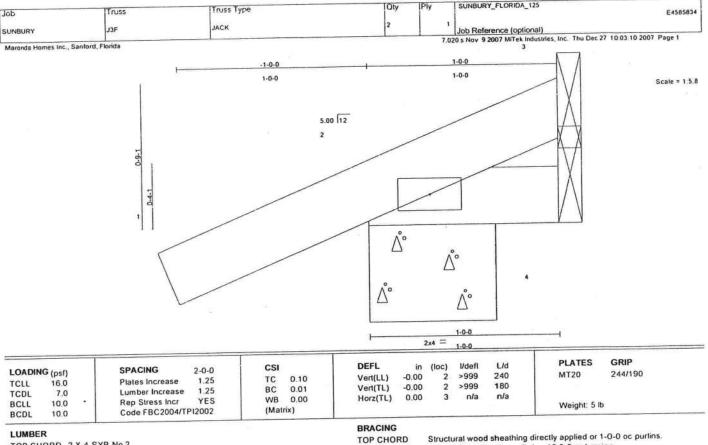


December 27,2007

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not it use designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility to building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding elaboration, quality control, storage, delivery, erection and bracing, cansult.

ANSI/TR11 Quality Criteria, DSB-89 and 8CS11 Building Component Salety Information available from Truss Plate Institute. 583 D'Onotrio Drive, Madison, WI 53719.





TOP CHORD

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=123/0-8-0, 4=18/Mechanical, 3=-10/Mechanical

Max Horz 2=45(LC 6)

Max Uplift2=-114(LC 6), 3=-10(LC 1)

Max Grav2=123(LC 1), 4=18(LC 1), 3=25(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-23/21

**BOT CHORD** 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 2 and 10 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Millet connectors. This design is based only upon parameters shown, and is for an individual building component. 
Applicability of design parameters and recorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the discount of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the discount of individual permanent bracing of the overall structure is the responsibility of the discount of the control purpose. For general guidance regarding designer, or general guidance regarding ANSI/TPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Piate Institute. S83 D'Onalria Drive, Madison, WI S3719.



Job	Truss	Truss Type	Oty	Ply	SUNBURY_FLORIDA_125 E4585836	1
SUNBURY	J3S	JACK	1	1	Job Reference (optional)	

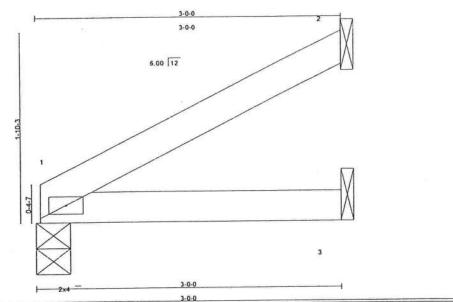
Marenda Homes Inc., Sanford, Florida

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:11 2007 Page 1

Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Scale = 1:10.8



OADING (psf	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d 240	PLATES MT20	GRIP 244/190
CLL 16.0	 Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	1-3	>999	773377	W120	244/150
CDL 7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	1-3	>999	180		
3CLL 10.0	Rep Stress Incr Code FBC2004/TF	YES 912002	WB (Mati	0.00 rix)	Horz(TL)	-0.00	2	n/a	n/a	Weight: 10 lb	)

BRACING

TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=117/0-4-0, 2=63/Mechanical, 3=55/Mechanical

Max Horz 1=63(LC 6)

Max Uplift1=-11(LC 6), 2=-62(LC 6)

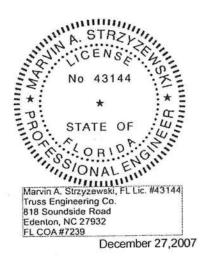
FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-49/24 BOT CHORD 1-3=0/0

(6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 1 and 62 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard





SUNBURY FLORIDA\_125 Truss Type Job Truss E4585837 JACK Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:11 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3 1-0-0 1-0-0 1-0-0 Scale = 1.6.2 6.00 12 1-0-0 2×4 =

OADING (psf) CCLL 16.0 CCDL 7.0 CCLL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC 2004/TPI2002	TC 0.10 BC 0.01 WB 0.00 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 2 2 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES GRIP MT20 244/190 Weight: 5 lb
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BRACING

TOP CHORD

**BOT CHORD** 

#### LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=121/0-4-0, 4=19/Mechanical, 3=-7/Mechanical

Max Horz 2=54(LC 6)

Max Uplift2=-111(LC 6), 3=-7(LC 1)

Max Grav2=121(LC 1), 4=19(LC 1), 3=22(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

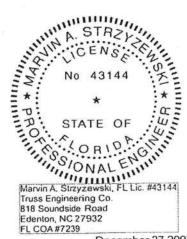
TOP CHORD 1-2=0/21, 2-3=-25/22

BOT CHORD 2-4=0/0

### NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 4) refer to global symbol to the second connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 2 and 7 lb uplift at joint 3
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design poramenters and proper incorporation of component is responsibility of building designer - not furst designer shows is for toleral support of individual wave members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the averall structure is the responsibility of the building designer. For general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, cansult. AMSI/TPI Quality Criteria, DSB-B9 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.



SUNBURY FLORIDA 125 Truss Type Truss Job E4585838 J4B JACK SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:11 2007 Page 1 Maronda Homes Inc., Sanford, Florida 1-0-0 -1-0-0 1-0-0 1-0-0 Scale = 1:5.8 5.00 12 0-9-1 1-0-0 2x4 = 1-0-0 PLATES GRIP DEFL CSI (loc) **Vdefl** SPACING 2-0-0 LOADING (psf) 244/190 MT20 -0.00 >999 240 Vert(LL) 2 TC 0.10 Plates Increase 1.25 TCLL 16.0 Vert(TL) -0.00 2 >999 180 BC 0.01 Lumber Increase 1 25 TCDL 7.0 0.00 Horz(TL) 0.00 3 n/a n/a WB Rep Stress Incr YES BCLL 10.0 Weight: 5 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (Ib/size) 2=123/0-8-0, 4=18/Mechanical, 3=-10/Mechanical

Max Horz 2=45(LC 6)

Max Uplift2=-114(LC 6), 3=-10(LC 1)

Max Grav 2=123(LC 1), 4=18(LC 1), 3=25(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-23/21

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 2 and 10 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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SUNBURY\_FLORIDA\_125 Qty Truss Type russ E4585840 Job JACK J4BS Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:12 2007 Page 1 SUNBURY Maronda Homes Inc., Sanford, Florida 2 5.00 12 Scale = 1:5.8 0-4-4 1-0-0 2×4 = PLATES GRIP DEFL in (loc) Vdef1 L/d CSI LOADING (psf) SPACING 2-0-0 244/190 240 MT20 Vert(LL) -0.00 >999 TC 0.01 Plates Increase 1.25 TCLL 16.0 >999 180 Vert(TL) -0.00 BC 0.01 7.0 Lumber Increase 1.25 TCDL n/a Horz(TL) -0.00 2 n/a WB 0.00 Rep Stress Incr YES 10.0 BCLL Weight: 3 lb (Matrix) Code FBC2004/TPI2002 10.0 BCDL

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** 

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=38/0-8-0, 3=18/Mechanical, 2=20/Mechanical Max Horz 1=18(LC 6)

Max Uplift1=-5(LC 6), 2=-19(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-14/7 1-3=0/0 **BOT CHORD** 

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

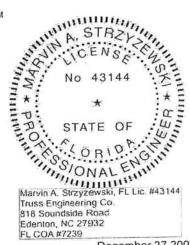
This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 1 and 19 lb uplift at joint
- Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



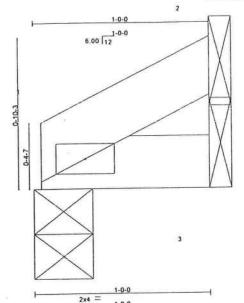
December 27,2007

SUNBURY\_FLORIDA\_125 Qty Truss Type Truss Job E4585842 JACK 145 SUNBURY Job Reference (optional)

Maronda Homes Inc., Sanford, Florida

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:13 2007 Page 1

Scale = 1:6.2



1-0-0 DEFL CSI **Udefi** 1/d LOADING (psf) SPACING 2-0-0 240 Vert(LL) -0.00 >999 TC 0.01 Plates Increase 1 25 TCLL 16.0 180 >999 Vert(TL) -0.00 BC 0.01 1.25 7.0 Lumber Increase TCDL n/a 0.00 Horz(TL) -0.00 n/a YES WB 10.0 Rep Stress Incr BCLL Code FBC2004/TPI2002 BCDL 10.0

**PLATES** GRIP 244/190 MT20

Weight: 3 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

Structural wood sheathing directly applied or 1-0-0 oc purlins. TOP CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** 

REACTIONS (lb/size) 1=39/0-4-0, 3=18/Mechanical, 2=21/Mechanical Max Horz 1=22(LC 6)

Max Uplift1=-3(LC 6), 2=-22(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-17/8 **BOT CHORD** 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 1 and 22 lb uplift at joint 2
- Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 2 and 3.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer. For large shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance responsibility of the building designer, For general guidance responsibility of the building designer. For general guidance responsibility of the subdising designer and subdising designer.



SUNBURY\_FLORIDA\_125 Qty Truss Type Truss Job E4585847 MONO TRUSS JGRD Job Reference (optional) SUNBURY 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:15 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 5-1-15 1-5-0 4-8-1 5-1-15 1-5-0 Scale = 1:23.6 3x4 11 4.24 12 3x4 = 9-3-14 12 11 5 3x5 = 2x4 11 2×4 = 9-10-5-1-15 4-8-1 5-1-15 Plate Offsets (X,Y): [2:0-1-15.0-0-7] GRIP PI ATES L/d DEFL (loc) 2-0-0 CSI SPACING LOADING (psf) 244/190 5-6 >999 240 MT20 -0.03 Vert(LL) 1.25 TC 0.24 Plates Increase TCLL 16.0 5-6 >999 180 -0.06 Vert(TL) 1.25 BC 0.33 Lumber Increase TCDL 7.0 0.01 5 n/a n/a Horz(TL) NO WB 0.29 Rep Stress Incr

BRACING

TOP CHORD

**BOT CHORD** 

end verticals.

BCDL LUMBER

BCLL

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

10.0

10.0

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 5=576/Mechanical, 2=536/0-5-11

Code FBC2004/TPI2002

Max Horz 2=174(LC 3)

Max Uplift5=-237(LC 3), 2=-169(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/21, 2-7=-905/228, 7-8=-870/250, 3-8=-830/229, 3-9=-123/50, 4-9=-55/0, 4-5=-129/142

2-10=-302/815, 10-11=-302/815, 6-11=-302/815, 6-12=-302/815, 5-12=-302/815 **BOT CHORD** 

3-6=0/293, 3-5=-812/290 WEBS

### NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 5 and 169 lb uplift at
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5 lb down and 33 lb up at 4-4-0.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Concentrated Loads (lb)

Vert: 8=-10(F=-5, B=-5) 9=-113(F=-56, B=-56) 10=42(F=21, B=21) 11=-31(F=-15, B=-15) 12=-111(F=-55, B=-55)

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No 43144

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Marvin A. Strzyzewski, FL Lic. #43144

Truss Engineering Co. Truss Engineering Co. 818 Soundside Road Edenton, NC 27932

Weight: 46 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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Edenton, NC 27932

FL COA #7239

Truss Type Qly SUNBURY FLORIDA 125 Job Truss E4585849 MONO TRUSS SUNBURY **JGRDB** Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:16 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 5-2-15 -1-5-0 4-7-2 5-2-15 1-5-0 3x4 |Scale = 1:20.8 3.54 12 3x4 = 6 14 12 13 3x5 = 2x4 || 9-10-1 4-7-2 5-2-15 PLATES GRIP LOADING (psf) SPACING CSI DEFL (loc) I/defl L/d 2-0-0 240 MT20 244/190 TC Vert(LL) -0.03 2-6 >999 0.24 TCLL 16.0 Plates Increase 1.25

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

100

10.0

WEBS 2 X 4 SYP No.2

BRACING TOP CHORD

Vert(TL)

Horz(TL)

-0.07

0.02

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Weight: 45 lb

end verticals.

5-6

5

>999

BOT CHORD Rigid ceiling directly applied or 8-11-0 oc bracing.

180

n/a

REACTIONS (lb/size) 5=576/Mechanical, 2=486/1-0-1

Max Horz 2=144(LC 3)

Max Uplift5=-306(LC 3), 2=-219(LC 3)

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-7=-1022/437, 7-8=-1020/438, 8-9=-999/449, 3-9=-967/436, 3-10=-127/62, 4-10=-70/16, 4-5=-129/131

BC 0.34

WB 0.31

(Matrix)

BOT CHORD 2-11=-490/957, 11-12=-490/957, 12-13=-490/957, 6-13=-490/957, 6-14=-490/957, 5-14=-490/957

1.25

NO

WEBS 3-6=-17/280, 3-5=-933/466

### NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4,2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- (a) Near to grader(s) for tross to tross connection.
  (b) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 5 and 219 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 18 lb down and 25 lb up at 1-6-12, 1 lb down and 19 lb up at 4-4-12, 14 lb down and 48 lb up at 4-4-12, 50 lb down and 84 lb up at 7-2-11, and 60 lb down and 101 lb up at 7-2-11, and 40 lb down at 1-6-13 on top chord, and 22 lb up at 1-6-13, 22 lb up at 1-6-12, 12 lb down and 49 lb up at 4-4-12, 12 lb down at 4-4-12, and 52 lb down and 78 lb up at 7-2-11, and 52 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-46, 2-7=-14, 4-7=-46, 2-11=-20, 5-11=-40

Concentrated Loads (lb

Vert: 8=25(F) 9=-11(F=-14, B=3) 10=-110(F=-60, B=-50) 12=43(F=22, B=22) 13=-24(F=-12, B=-12) 14=-104(F=-52, B=-52)

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Marvin A Sirzyzewski, FL Lic. #43144
Truss Engineering Co.

Marvin A. Strzyzewski, FL Lic. #431-Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

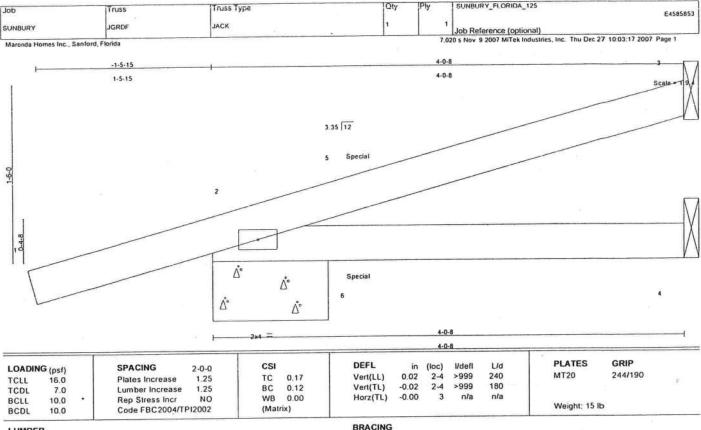
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818 Soundside Road Edenton, NC 27932



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

Structural wood sheathing directly applied or 4-0-8 oc purlins. TOP CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD

REACTIONS (lb/size) 3=62/Mechanical, 2=249/0-11-15, 4=67/Mechanical

Max Horz 2=72(LC 3)

Max Uplift3=-34(LC 7), 2=-207(LC 7), 4=-37(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/16, 2-5=-33/0, 3-5=-18/14

**BOT CHORD** 2-6=0/0, 4-6=0/0

NOTES (8)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3, 207 lb uplift at joint 2 and 37 lb uplift at joint 4.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 48 lb down at 1-3-5 on top chord, and 22 lb up at 1-3-5 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 8) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-46, 2-4=-40 Concentrated Loads (lb) Vert: 6=22(B)

A. STRZI Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

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ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from fruss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.



SUNBURY\_FLORIDA\_125 Qty Truss Type Job russ E4585855 MONO SCISSOR SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:18 2007 Page 1 Maronda Homes Inc., Sanford, Florida -1-0-0 7-0-0 7-0-0 1-0-0 Scale: 1/2"=1" 2x4 11 6.00 12 2×4 = 3.00 12 3x4 = 7-0-0 **PLATES** GRIP DEFL LOADING (psf) SPACING CSI l/defl L/d 2-0-0 MT20 244/190 1.25 TC 0.44 Vert(LL) -0.11 2-4 >742 240 Plates Increase TCLL 16.0 -0.27>297 180 1.25 BC 0.42 Vert(TL) 2-4 Lumber Increase TCDL 7.0 0.00 n/a nla Rep Stress Incr YES WB 0.03 Horz(TL) 10.0 BCLL Weight: 27 lb Code FBC2004/TPI2002 (Matrix) BCDL BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD

**BOT CHORD** 

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=346/0-4-0, 4=283/Mechanical

Max Horz 2=173(LC 6)

Max Uplift2=-105(LC 6), 4=-102(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-108/55

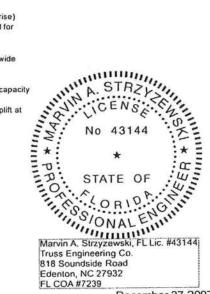
**BOT CHORD** 2-4=-26/26

3-4=-149/176 WEBS

### NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 2 and 102 lb uplift at

LOAD CASE(S) Standard



Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_125	E4585856
IBURY	JS2	MONO SCISSOR	4	,	Job Reference (optional)	v :
ronda Homes Inc., Sa				7.0	20 s Nov 9 2007 MiTek Industries, Inc. Thu Dec	27 10:03:19 2007 Page 1
tonda riomes me., ou	*				21	
	-1-0		5-0-0			
	1-0	-0	5-0-0			Scale = 1.16.
		6.	00 12	/	1.83	
	2.10-3					
	The second	2			1.1.10	
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3×4 =	3.00 1	2	11 -	
		-	5-0-0 5-0-0			

2-4 >999

-0.03

-0.07

-0.00

Vert(LL)

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

**BOT CHORD** 

>817

240

180

n/a

MT20

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 18 lb

244/190

LUMBER

TCLL

TCDL

BCLL

BCDL

LOADING (psf)

16.0

7.0

10.0

10.0

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=103/Mechanical, 2=265/0-4-0, 4=95/Mechanical

Max Horz 2=133(LC 6) Max Uplift3=-98(LC 6), 2=-98(LC 6)

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-76/38 TOP CHORD

**BOT CHORD** 2-4=-19/19

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

TC

BC

WB 0.00

(Matrix)

0.21

0.22

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

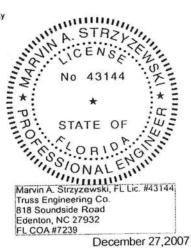
1.25

1.25

YES

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 3 and 98 lb uplift at joint 2.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 BEFORE USE.

Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding consults of the building designer, For general guidance regarding above the building designer. For general guidance regarding above to the support of individual web members only. Ansigner is the responsibility of the building designer. For general guidance regarding above the support of individual values of the control of the control of the control of the support of the support of the support of the control of the control of the support of the



Job	Truss	Truss Type	Qty	Ply	SUNBURY_FLORIDA_12	5	E4585857
SUNBURY	JS3	MONO SCISSOR	4	1	Job Reference (options	st)	2100000
Maronda Homes Inc., Sanfo	ord, Florida			7.0	20 s Nov 9 2007 MiTek Indu	stries, Inc. Thu Dec	27 10:03:19 2007 Page 1
	-1-0-0		3-0-0	15		4	
	1-0-0		3-0-0		3	1.7	
	1						Scale = 1:10.8
						M	
					/ /	4	
		6.00	12			5.3	
		0,001	/			4	
						W	
	1-10-3					TXI	
	7		/ /			VN .	
						71	
		2				00	
	1	10			4	0-8-0	
	[5]					١	
	19					1 1	
	' /		3	.00 12			
		3×4 =	3-0-0				
		-	3-0-0				
	T		2000			PLATES	GRIP
LOADING (psf)	SPACING 2-0 Plates Increase 1.	-0 CSI 25 TC 0.11	DEFL Vert(LL) -0.0	in (loc) 00 2-4	I/defl L/d >999 240	MT20	244/190

Vert(TL)

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

-0.01

>999

180

n/a

Weight: 12 lb

Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

10.0

REACTIONS (lb/size) 3=52/Mechanical, 2=184/0-4-0, 4=55/Mechanical

Lumber Increase

Rep Stress Incr

Code FBC2004/TPI2002

Max Horz 2=92(LC 6) Max Uplift3=-48(LC 6), 2=-94(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-38/19 TOP CHORD

BOT CHORD 2-4=-11/11

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

BC 0.07

WB 0.00

(Matrix)

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

YES

- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 3 and 94 lb uplift at joint 2.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007



SUNBURY FLORIDA\_125 russ Truss Type Job E4585858 MONO SCISSOR JS4 SUNBURY Job Reference (optional)

7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10.03.19 2007 Page 1 Maronda Homes Inc., Sanford, Florida 3 1-0-0 -1-0-0 1-0-0 1-0-0 Scale = 1:6.2 6.00 12 0.10-3 0-4-3 3x4 = 3.00 12 4 1-0-0 1-0-0 GRIP **PLATES** l/defl L/d CSI in (loc) LOADING (psf) SPACING 2-0-0 >999 240 MT20 244/190 -0.00 Vert(LL) 0.09 Plates Increase 1.25 TC TCLL 160 -0.00 >999 180 Vert(TL) Lumber Increase 1.25 BC 0.01 7.0 TCDL nia 0.00 Horz(TL) Rep Stress Incr YES WB 0.00 10.0 BCLL Weight: 5 lb Code FBC2004/TP12002 (Matrix) BCDL 10.0

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=114/0-4-0, 4=19/Mechanical, 3=-4/Mechanical

Max Horz 2=51(LC 6)

Max Uplift2=-98(LC 6), 3=-7(LC 5)

Max Grav2=114(LC 1), 4=19(LC 1), 3=14(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

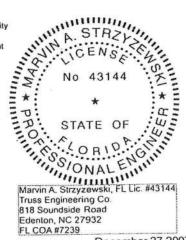
TOP CHORD 1-2=0/17, 2-3=-25/17

2-4=-4/4 BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 2 and 7 lb uplift at joint
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for on individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the execution. Additional permanent bracing of the averall structure is the responsibility of the diditional permanent bracing of the averall structure is the responsibility of the diditional permanent bracing of the averall structure is the responsibility of the support of individual permanent process. For general guidance regarding a stability of the control storage, defivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-B9 and BCS11 Building Component Safety Information.



SUNBURY\_FLORIDA\_125 Oty Truss Type Job Truss E4585864 MONO SCISSOR ISGRD SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:22 2007 Page 1 Maronda Homes Inc., Sanford, Florida 9-10-1 -1-5-0 5-4-0 4-6-1 1-5-0 Scale = 1:23.4 3x4 11 4.24 12 4x6 = 5x6 = 12 9-3-19 2x4 || 2.12 12 10 3x4 = 9-10-1 DEFL PLATES. GRIP LOADING (psf) 2-0-0 CSI SPACING (loc) 1/defl 240 MT20 244/190 Vert(LL) -0.06 2-6 >999 1 25 TC 0.30 TCLL Plates Increase >999 180 Vert(TL) -0.11 BC 0.46 TCDL Lumber Increase 1.25 7.0 0.02 n/a NO WB 0.36 Horz(TL) 5 n/a BCLL 10.0 Rep Stress Incr Weight: 42 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER

TOP CHORD

**BOT CHORD** 

end verticals.

REACTIONS (lb/size) 2=536/0-5-11, 5=580/Mechanical

Max Horz 2=173(LC 3)

2 X 4 SYP No.2

Max Uplift2=-180(LC 3), 5=-248(LC 3)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/19, 2-7=-1496/472, 7-8=-1456/490, 3-8=-1423/473, 3-9=-210/95, 4-9=-143/35, 4-5=-110/132 TOP CHORD

2-10=-543/1378, 10-11=-534/1381, 6-11=-533/1403, 6-12=-547/1371, 5-12=-539/1407 **BOT CHORD** 

3-6=0/273, 3-5=-1212/455 WEBS

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

### NOTES

WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2 and 248 lb uplift at joint 5.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 6 lb down and 38 lb up at 4-4-12. 6 lb down and 38 lb up at 4-4-12, 57 lb down and 103 lb up at 7-2-11, 57 lb down and 103 lb up at 7-2-11, and 35 lb down at 1-6-12, and 35 lb down at 1-6-12 on top chord, and 21 lb up at 1-6-12, 21 lb up at 1-6-12, 15 lb down at 4-4-12, 15 lb down at 4-4-12, and 55 lb down at 7-2-11, and 55 lb down at 7-2-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 8=-13(F=-6, B=-6) 9=-114(F=-57, B=-57) 10=42(F=21, B=21) 11=-31(F=-15, B=-15) 12=-111(F=-55, B=-55)

A STRZY Truss Engineering Co. 818 Soundside Road Edenton, NC 27932

Structural wood sheathing directly applied or 5-1-0 oc purlins, except

Rigid ceiling directly applied or 8-5-5 oc bracing

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding laborication, quality control, storage, delivery, erection and bracing, consult. AMS/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Edenion NC 27932

FL COA #7239

SUNBURY\_FLORIDA\_125 Truss Type Qly Job Truss E4585868 SUNBURY MHGRDF MONO HIP Job Reference (optional) 9 2007 MiTek Industries, Inc. Thu Dec 27 10 03:24 2007 Page 1 Maronda Homes Inc., Sanford, Florida 1-0-0 2-9-7 5-2-9 2-9-7 1-0-0 Scale = 1:15.8 4x8 = 42x4 II 5.00 12 Ġ À. Q. V. 2x4 || 6 5 À. 4x6 = 3x4 = 2-9-7 2-9-7 5-2-9 Plate Offsets (X.Y): [3:0-5-0.0-2-0] DEFL l/defl L/d **PLATES** GRIP LOADING (ost) SPACING 2-0-0 244/190 1.25 TC 0.32 Vert(LL) 0.02 5-6 >999 240 MT20 16.0 Plates Increase TCLL 1.25 BC 0.21 Vert(TL) -0 03 5-6 >999 180 TCDL 7.0 Lumber Increase NO WR 0.23 Horz(TL) 0.00 5 n/a n/a 10.0 Rep Stress Incr BCLL Weight: 42 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 end verticals

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2 X 6 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 5=368/0-8-0, 2=445/0-8-0

Max Horz 2=78(LC 5)

Max Uplift5=-231(LC 8), 2=-327(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/20, 2-3=-655/387, 3-7=0/0, 7-8=0/0, 4-8=0/0, 4-5=-121/96

2-6=-360/563, 6-9=-370/591, 9-10=-370/591, 5-10=-370/591 BOT CHORD

3-6=-82/239, 3-5=-606/380 WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Concentrated loads from layout are not present in Load Case(s): #9 1st unbalanced Regular, #10 2nd unbalanced Regular.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 5 and 327 lb uplift at
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 76 lb up at 2-9-7,
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Provide adequate drainage to prevent water ponding.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Concentrated loads from layout are not present in Load Case(s): #9 1st unbalanced Regular, #10 2nd unbalanced Regular.

This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 5 and 327 lb uplift at joint 2.

Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 76 lb up at 2-9-7, and 16 lb down at 4-10-3, and 4 lb down and 42 lb up at 6-10-3 on top chord, and 44 lb down at 2-9-7, and 16 lb down at 4-10-3 and 16 lb down at 6-10-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

OAD CASE(S) Standard

Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-46, 3-4=-46, 2-5=-40

Concentrated Loads (lb)

Vert: 3=-19(B) 6=-44(B) 7=-4(B) 8=-4(B) 9=-16(B) 10=-16(B)

Marvin A. Strzyzewski, FL Lic. #43144

Truss Engineering Co. Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII.7473 BEFORE USE.

Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss-designer, Bracing sho is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the certain control permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. ANSI/TRI Quality Cilleria, DSB-87 and 8CS11 Building Component Safety Information.



lob	Truss	Truss Type	Oty	Ply		SUNBURY_FLORIDA_1	25	E4585870
SUNBURY	RG1	GABLE	1		1	Job Reference (option	al)	
Maronda Homes Inc., S	anlord Florida				7.02	0 s Nov 9 2007 MiTek Ind	ustries, Inc. Thu Dec 2	7 10 03 25 2007 Page 1
Maiorida Florines ins., e-						20-5-7		
		-2-11				10-2-12		
	10	-2-11				10.2.13		Scale = 1:34,9
			4×6 =					
			5					
4		2x4 1		2x4	Ĺ			
İ	5.00	12		6				
	III.NP227.	2x4 II	/ 11 \	Ja -	\	2×4 11		
		3		15	/	7		
	2x4 II			- 11		NQ.	2x4	
25				- 11			8	
4	2			11			Tal.	
	10	11 11		- 11				
		11 11		- 11				9
1 1		- le le		<u> </u>		- FIB		- O 4
I XXX		[e] [e]	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		XX		XXXXXX	××××××××××××××××××××××××××××××××××××××
2 XXXX	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX	$\times \times $	$\times \times $	XX.	$\times \times $	$x^{4}_{8}xxxxx$	3x4 >
3x4	= 17	2x4 II 2x4	II 2x4 II	2x4	H)	3x4 =	2x4	3,4
	2x4 II	2x4 II 2x4		-		2x4 ii		
			20-5-7					
-0- <del>  -0</del>			20-5-7					
0-1-0							PLATES	GRIP
LOADING (psf)	SPACING	2-0-0 CSI	DEFL		loc)	I/defl L/d n/a 999	MT20	244/190
TCLL 16.0	Plates Increase	1.25 TC 0.11 1.25 BC 0.10		n/a n/a		n/a 999 n/a 999	WIIZO	244/100
TCDL 7.0	Lumber Increase	1.25 BC 0.10 NO WB 0.03		0.00	9	n/a n/a	100-44-17 (MICH. 0041)	
BCLL 10.0 BCDL 10.0	<ul> <li>Rep Stress Incr Code FBC2004/TF</li> </ul>						Weight: 85	b
			BRACING					
LUMBER			TOP CHOR	D SI	ructu	ral wood sheathing o	irectly applied or 6	-0-0 oc purlins.
TOP CHORD 2			BOT CHOR		igid c	eiling directly applied	or 10-0-0 oc braci	ng.
DOI CHURD Z	1 4 0 11 110.2							

2 X 4 SYP No.2 REACTIONS (lb/size) 1=127/20-6-7, 9=124/20-6-7, 14=159/20-6-7, 15=186/20-6-7, 16=116/20-6-7, 17=311/20-6-7, 13=188/20-6-7, 11=119/20-6-7, 10=303/20-6-7

Max Horz 1=-72(LC 7)

Max Uplift1=-38(LC 7), 9=-37(LC 7), 15=-104(LC 6), 16=-69(LC 6), 17=-178(LC 6), 13=-106(LC 7), 11=-70(LC 7), 10=-174(LC 7)

Max Grav 1=127(LC 1), 9=124(LC 1), 14=159(LC 1), 15=189(LC 10), 16=116(LC 1), 17=311(LC 10), 13=191(LC 11), 11=119(LC 1), 10=304(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-76/31, 2-3=-34/86, 3-4=-23/118, 4-5=-25/174, 5-6=-25/174, 6-7=-22/113, 7-8=-32/64, 8-9=-45/31

1-17=0/78, 16-17=0/78, 15-16=0/78, 14-15=0/78, 13-14=0/78, 12-13=0/78, 11-12=0/78, 10-11=0/78, 9-10=0/78 5-14=-83/0, 4-15=-101/138, 3-16=-65/109, 2-17=-162/218, 6-13=-103/142, 7-11=-66/110, 8-10=-159/214 BOT CHORD

### NOTES

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4,2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 37 lb uplift at joint 9, 104 lb uplift at joint 15, 69 lb uplift at joint 16, 178 lb uplift at joint 17, 106 lb uplift at joint 13, 70 lb uplift at joint 11 and 174 lb uplift at joint 10.
- 8) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and receptor in corporation of component is responsibility of building designer in oil furs designer. Bracing shown a for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the during designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the flobrication, quality control, strange, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DS8-89 and 8CS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onafrio Drive, Madison, WI 53719.

Job		Truss	Truss Type		0	iy	Ply	SUNBL	JRY_FLORIDA_125		5.0000
		RG2	GABLE		1		1				E458587
SUNBURY		RGZ	GAUCE						eference (optional)		
Maronda Homes	s Inc., Sanford, F	lorida					7.0	020 s Nov	9 2007 MiTek Industr	ries, Inc. Thu Dec 2	7 10:03:25 2007 Page 1
		5-6							11-0-13		20
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					11-0-13-						20
1	-				11-0-13						
					11-0-13						
LOADING (	nsf)	SPACING 2-0	-0 CSI	7.2	DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
	6.0		25 TC	0.11	Vert(LL)	n/a		n/a	999	MT20	244/190
	7.0	Lumber Increase 1.	25 BC	0.07	Vert(TL)	n/a		n/a	999		
	10.0		NO MB	0.03	Horz(TL)	0.00	5	n/a	n/a	141-1-14- 40 11	
BCDL 1	10.0	Code FBC2004/TPI20	02 (Mat	rix)						Weight: 40 lb	)
LUMBER	-				BRACING						
	D 2 X 4 SYF	No 2			TOP CHO		Structi	ıral wood	d sheathing direc	tly applied or 6-	0-0 oc purlins.
	D 2X4SY				вот сно				rectly applied or		
OTHERS	2 X 4 SY					125		13.50			70
UTILING	27.1011	C. 17. C. 17.									
DEACTION	C (lh/sizo)	1=106/11-1-13 5=103/11.	1-13 7=105/11-1-13	8=268/11-1-13	6=264/11-1-	13					

Max Horz 1=-46(LC 4)

Max Uplift1=-35(LC 7), 5=-38(LC 7), 8=-167(LC 6), 6=-165(LC 7)

Max Grav 1=106(LC 1), 5=103(LC 1), 7=105(LC 1), 8=269(LC 10), 6=264(LC 11)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-34/34, 2-3=-34/117, 3-4=-33/114, 4-5=-25/30

BOT CHORD

1-8=0/54, 7-8=0/54, 6-7=0/55, 5-6=0/55

WEBS

3-7=-56/7, 2-8=-141/234, 4-6=-139/233

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 38 lb uplift at joint 5, 167 lb uplift at joint 8 and 165 lb uplift at joint 6.
- 8) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



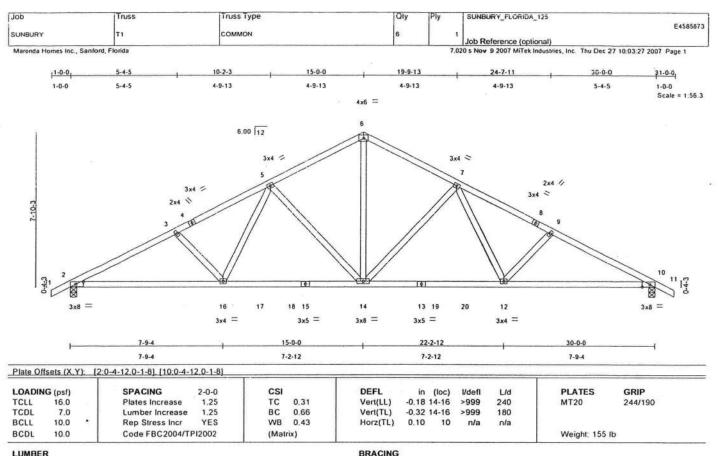
December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer and truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the world structure in the responsibility of the support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the support of individual permanent processing of the overall structure is the responsibility of the subdiction, quality control, storage, delivery, erection and bracing, consult.

ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onolrio Drive, Modison, WI 53719.





TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.2

REACTIONS (lb/size) 2=1392/0-4-0, 10=1392/0-4-0

Max Horz 2=-137(LC 7)

Max Uplift2=-309(LC 6), 10=-309(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/20, 2-3=-2441/724, 3-4=-2287/666, 4-5=-2233/682, 5-6=-1589/568, 6-7=-1589/568, 7-8=-2233/682, TOP CHORD

8-9=-2287/666, 9-10=-2441/724, 10-11=0/20

**BOT CHORD** 2-16=-519/2136, 16-17=-342/1760, 17-18=-342/1760, 15-18=-342/1760, 14-15=-342/1760, 13-14=-342/1760,

13-19=-342/1760, 19-20=-342/1760, 12-20=-342/1760, 10-12=-519/2136

WEBS

3-16=-205/215, 5-16=-70/561, 5-14=-568/272, 6-14=-314/1144, 7-14=-568/272, 7-12=-70/561, 9-12=-205/215

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 10.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-8-8 oc purlins.

Rigid ceiling directly applied or 8-8-14 oc bracing.

December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shows is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional temporary bracing is insured stability during construction is the responsibility of the building designer. For general guidance regarding labely control, storage, delivery, erection and bracing, consult. ASSI/TETI Quality Criteria, 058-89 and BCSI1 Building Component Stability Criteria, 058-89 and BCSI1 Building Component. labrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Qua Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



SUNBURY\_FLORIDA\_125 Oly Truss Type Lob Truss E4585878 COMMON TIBS SUNBURY Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Thu Dec 27 10:03:30 2007 Page 1 Maronda Homes Inc., Sanford, Florida 22-11-2 31-0-0 1-0-0, 8-0-14 8-0-14 7-5-2 7-5-2 1-0-0 Scale = 1:54.7 5x6 = 5.00 12 5 3x4 = 3x4 = 1 6 3x4 3 14 3x6 3x6 = 10 11 5x8 = 2x4 || 2x4 11 31-0-0 22-11-2 15-6-0 8-0-14 8-0-14 7-5-2 8-0-14 7-5-2 Plate Offsets (X.Y): [2:9-3-0.0-1-8]. [8:0-3-0.0-1-8]. [10:0-4-0.0-3-0] PLATES GRIP DEFL SPACING 2-0-0 CSI in (loc) **Vdefi** L/d LOADING (psf) 240 MT20 244/190 -0.20 >999 Vert(LL) 8-9 Plates Increase 1.25 TC 0.53 TCLL 16.0 -0.42 >874 180 8-9 TCDL 7.0 Lumber Increase 1 25 BC 0.83 Vert(TL) 0.12 n/a n/a WB 0.89 BCLL 10.0 Rep Stress Incr YES Horz(TL) Weight: 140 lb BCDL 10.0 Code FBC2004/TPI2002 (Matrix) BRACING LUMBER TOP CHORD Structural wood sheathing directly applied or 3-3-6 oc purlins. TOP CHORD 2 X 4 SYP No.2

BOT CHORD

Rigid ceiling directly applied or 7-5-6 oc bracing

BOT CHORD 2 X 4 SYP No.2

WERS

2 X 4 SYP No.2

REACTIONS (lb/size) 2=1370/0-8-0, 8=1303/0-8-0

Max Horz 2=126(LC 6)

Max Uplift2=-332(LC 6), 8=-250(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-2604/783, 3-4=-1774/597, 4-5=-1706/619, 5-6=-1706/620, 6-7=-1775/598, 7-8=-2613/804 TOP CHORD

2-11=-626/2342, 10-11=-626/2342, 9-10=-649/2351, 8-9=-649/2351 **BOT CHORD** 

3-11=0/340, 3-10=-869/328, 5-10=-190/986, 7-10=-880/352, 7-9=0/345 WEBS

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 2 and 250 lb uplift at

LOAD CASE(S) Standard



December 27,2007



Job	Truss	Truss Type	Qly	Ply	SUNBURY_F	LORIDA_125		E4585881
SUNBURY	VIA	VALLEY	1	- 1	Job Referen	nce (optional)		
Maronda Homes Inc., Sanford	1, Florida			7.0	20 s Nov 9 200	7 MiTek Industrie	s, Inc. Thu Dec 2	7 10:03:32 2007 Page 1
						16-6-7		
1	8-3-4 8-3-4					8-3-4		-
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3x4 =	2x4 11	3x4 =	2×4			2x4		244
-			16-6-7					
			16-6-7			T		
LOADING (psf)	SPACING 2-0-0		DEFL	in (loc)		/d	PLATES	GRIP 244/190
TCLL 16.0 TCDL 7.0	Plates Increase 1.25 Lumber Increase 1.25			n/a - n/a -		99 99	MT20	244/190
BCLL 10.0 -	Rep Stress Incr YES	WB 0.03		.00 5	n/a n	la		
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)					Weight: 56 lb	)
LUMBER			BRACING			1000 Val. 10	72: V 72:	
TOP CHORD 2 X 4 S			TOP CHORD BOT CHORD				y applied or 6- 0-0-0 oc bracin	0-0 oc purlins.
BOT CHORD 2 X 4 S OTHERS 2 X 4 S	YP No.2 YP No.2		BOT CHOKE	itigid c	oig on oon	,		

REACTIONS (lb/size) 1=110/16-6-7, 5=110/16-6-7, 7=359/16-6-7, 6=359/16-6-7, 9=359/16-6-7

Max Horz 1=48(LC 6)

Max Uplift1=-18(LC 7), 5=-17(LC 7), 7=-23(LC 6), 6=-123(LC 7), 9=-123(LC 6) Max Grav 1=110(LC 1), 5=110(LC 1), 7=359(LC 1), 6=369(LC 11), 9=369(LC 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-41/35, 2-3=-54/86, 3-4=-54/86, 4-5=-33/26

BOT CHORD

1-9=0/38, 8-9=0/38, 7-8=0/38, 6-7=0/38, 5-6=0/38

WEBS

3-7=-186/110, 4-6=-201/184, 2-9=-201/184

### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone, Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1, 17 lb uplift at joint 5, 23 lb uplift at joint 7, 123 lb uplift at joint 6 and 123 lb uplift at joint 9.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 REPORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the usilizing designer, for general guidance regarding labrication, quality control, storage, delivery, erection and bracing, consult.

ANSI/TP1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information.



Job	Truss	Truss Typ	oe .	Ot	y F	²ly	SUNBU	RY_FLORIDA_1	25	E458588
SUNBURY	V1B	VALLEY		1		,	Joh Re	ference (option	nal)	2.13550
Maronda Homes Inc.,	Sanford, Florida					7.0				7 10:03:32 2007 Page 1
								12-6-7		
		6-3-4 6-3-4						6-3-4		
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(A A A A	3x4 =			2×4						3×4 =
1				12-6-7						
				12-6-7						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.22	Vert(LL)	n/a	4.004	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	n/a		n/a	999		
BCLL 10.0 BCDL 10.0	Rep Stress Incr Code FBC2004/T	YES PI2002	WB 0.03 (Matrix)	Horz(TL)	0.00	3	n/a	n/a	Weight: 39 I	ь
	0300100200111		decommond.							
LUMBER				TOP CHOR	n.	Structu	ral woo	d sheathing d	irectly applied or 6	-0-0 oc purlins
TOP CHORD 2	X 4 SYP No.2			POT CHOP					or 10-0-0 oc braci	

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 OTHERS

REACTIONS (lb/size) 1=190/12-6-7, 3=190/12-6-7, 4=572/12-6-7

Max Horz 1=35(LC 6)

Max Uplift1=-50(LC 6), 3=-56(LC 7), 4=-83(LC 6) Max Grav 1=198(LC 10), 3=198(LC 11), 4=572(LC 1)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-58/53, 2-3=-58/51

BOT CHORD

1-4=0/28, 3-4=0/28

WEBS

2-4=-289/208

### NOTES

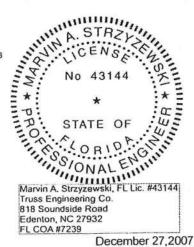
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gusl); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 56 lb uplift at joint 3 and 83 lb uplift at joint 4.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the didding designer, for general guidance regarding a flobrication, qualify control, storage, delivery, erection and bracing, consult.

ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onalrio Drive, Modison, WI 537 19.



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UNBURY	VIC	GABLE		1		1 lob Do	eference (optio	nal\	2,000
Maronda Homes Inc., 5	Sanford Florida					7.020 s Nov	9 2007 MiTek In	dustries, Inc. Thu Dec 2	27 10:03:33 2007 Page 1
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc	) I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increa		TC 0.13	Vert(LL)	n/a -	n/a	999	MT20	244/190
TCDL 7.0	Lumber Incre		BC 0.12 WB 0.02	Vert(TL) Horz(TL)	n/a - 0.00 :	n/a 3 n/a	999 n/a		
BCLL 10.0 BCDL 10.0	Rep Stress In Code FBC20		(Matrix)	11012(12)			1110	Weight: 26 I	b
		A CONTRACTOR OF THE STATE OF TH		BRACING					
LUMBER TOP CHORD 2	X 4 SYP No 2			TOP CHORD				irectly applied or 6	
BOT CHORD 2				<b>BOT CHORD</b>	Rigio	l ceiling di	rectly applied	or 10-0-0 oc braci	ng.

OTHERS 2 X 4 SYP No.2

REACTIONS (lb/size) 1=139/8-6-7, 3=139/8-6-7, 4=330/8-6-7

Max Horz 1=-22(LC 7)

Max Uplift1=-45(LC 6), 3=-49(LC 7), 4=-27(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension 1-2=-38/34, 2-3=-38/32

TOP CHORD

1-4=0/15, 3-4=0/15

**BOT CHORD** 

WEBS

2-4=-153/125

# NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 1, 49 lb uplift at joint 3 and 27 lb uplift at joint 4.
- Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



December 27,2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the effect. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labrication, quality control, storage, defivery, erection and bracing, consult.

ANSI/TRI Quality Criteria, DSB-89 and BCS11 Building Component Safety Information.



Job	Truss	Truss Type		Qty	Ply	SUNBURY_FLORIDA_1	25	E4585884
SUNBURY	V1D	VALLEY		1	,	Job Reference (option	nal)	
Maronda Homes Inc.,	Sanlord Florida		3x4 =		7.0	20 s Nov 9 2007 MiTek In	dustries, Inc. Thu Dec 27	7 10:03:33 2007 Page 1
Maiolida Floriles Inc.,	Samore, Florida					(D) Series		
-	2-3-4		2			4-6-7		
	2-3-4	1				2-3-4		Scale = 1:7.6
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1		5.00 12						
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		•						
							3	
10								
0-11-5	1				\	~	_	
6		\				_ / `		
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	*256200.10					2*	. >	
	2x4 =		4-6-7					
-			4-6-7					
Plate Offsets (X.	Y): [2:0-2-0,Edge]					eren ver english		
LOADING (psf)	SPACING 2	-0-0 CSI	DEFI			I/defl L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25 TC	0.03 Vert(			n/a 999	MT20	244/190
TCDL 7.0		1.25 BC	0.10 Vert(			n/a 999 n/a n/a	1	
BCLL 10.0		YES WB	0.00 Horz	(TL) 0.00	3	n/a n/a	Weight: 12 lt	<b>Y</b>
BCDL 10.0	Code FBC2004/TPI2	002 (Mate	nx)				vveigin. 12 it	
LUMBER			BRA	CING				
LUMBER	Y A SVP No 2			CHORD	Structu	ral wood sheathing d	lirectly applied or 4-	6-7 oc purlins,

**BOT CHORD** 

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=132/4-6-7, 3=132/4-6-7

Max Horz 1=10(LC 6)

Max Uplift1=-26(LC 6), 3=-26(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-103/88, 2-3=-103/88

1-3=-63/85 **BOT CHORD** 

# NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 1 and 26 lb uplift at joint 3.

LOAD CASE(S) Standard



Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the elector. Additional permanent bracing of the overall structure is the responsibility of the distribution of the property of the elector. Additional permanent bracing of the overall structure is the responsibility of the distribution designer. For general guidance regarding also control to the property of 


Job	Truss	Truss Type		Qty	Ply	SUNBU	RY_FLORIDA_125		E4585885
SUNBURY	V2A	VALLEY		1		,			C4303003
		17/05/95/05/05		1		Job Re	ference (optional)		
Maronda Homes Inc., Sa	nford, Florida				7.	020 s Nov	9 2007 MiTek Industri	es, Inc. Thu Dec 2	7 10:03:34 2007 Page 1
		3-6-14				7	7-1-13		
	1						3-6-15		
541		3-6-14					3-0-13		Scale = 1:14.4
			3x4 =						
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		6.00 12							
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	2x4 =							2x4 >	
			7-1-13						
			7-1-13						
Plate Offsets (X,Y):	[2:0-2-0.Edge]								
		Notice to the second se		7.00		NEW OF STREET		DIATES	CDID
LOADING (psf)		-0-0 CSI	DEFL		(loc)	l/defl	L/d	PLATES MT20	GRIP 244/190
TCLL 16.0			0.10 Vert(LI 0.36 Vert(TI			n/a n/a	999	MIZU	244/190
TCDL 7.0 BCLL 10.0			0.00 Ven(1)			n/a	n/a		
BCLL 10.0 BCDL 10.0	Code FBC2004/TP12		30 M 10 M 1 M 1 M 1 M 1 M 1 M 1 M 1 M 1 M	-, 0.00			5000	Weight: 20 lb	,
BCDL 10.0	Code FBC2004/1F12	(Matri	.,					orgin. 20 ic	
LUMBER			BRACI	NG					

TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=254/7-1-13, 3=254/7-1-13

Max Horz 1=-24(LC 4)

Max Uplift1=-48(LC 6), 3=-48(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-176/143, 2-3=-176/143

**BOT CHORD** 1-3=-87/136

## NOTES

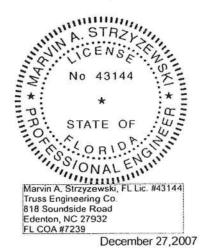
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 48 lb uplift at joint 3.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON TEIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REPORE USE.

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Job	Truss	Truss Type	3x4 =	Oty Ply	SUNBURY_FLORIDA_12	5 E4585886
SUNBURY	V2B	VALLEY		1	Job Reference (optional	
Maronda Homes Inc.	Santord Florida				7.020 s Nov 9 2007 MiTek Indu	stries, Inc. Thu Dec 27 10:03:34 2007 Page 1
Maidida ridiles ille.	Samora, Torras		2			
1		1-6-14			3-1-13	
		1-6-14	ter a		1-6-15	
1		6.00 12				Scale = 1:5.9
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14	CANANANANANANANANANANANANANANANANANANAN	VVVVVVVVV	~~~~~~~	/VVVVV	//////////////////////////////////////	XXXXXXXXXXX 12/4
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R	******	XXXXXXXXXX	$\times\!\!\times\!\!\times\!\!\times\!\!\times\!\!\times\!\!\times$	$\longleftrightarrow$	<b>*********</b>	***************************************
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K	*****	$\times\!\!\times\!\!\times\!\!\times\!\!\times\!\!\times\!\!\times$	****	$\sim$	<b>**********</b>	^^^
2			3-1-13	<u> </u>		
-		2×4 =	3-1-13		2×4 >	
						The second secon
Plate Offsets (X.	r): [2:0-2-0.Edge]					
LOADING / O	SPACING	2-0-0 CSI	DEFL	in (loc	c) I/defl L/d	PLATES GRIP
LOADING (psf)	Plates Increase		0.02 Vert(LI		- n/a 999	MT20 244/190
TCLL 16.0 TCDL 7.0	Lumber Increase		0.04 Vert(T		- n/a 999	
BCLL 10.0	Rep Stress Incr		0.00 Horz(T		3 n/a n/a	
BCDL 10.0	Code FBC2004/TPI			-ta - 5000		Weight: 8 lb
- 10.0	3.55. 2.55. 0.11	and the same of th				

TOP CHORD

**BOT CHORD** 

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=82/3-1-13, 3=82/3-1-13

Max Horz 1=-8(LC 4)

Max Uplift1=-16(LC 6), 3=-16(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-57/48, 2-3=-57/48

BOT CHORD 1-3=-29/44

## NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 1 and 16 lb uplift at joint 3.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 3-1-13 oc purlins.

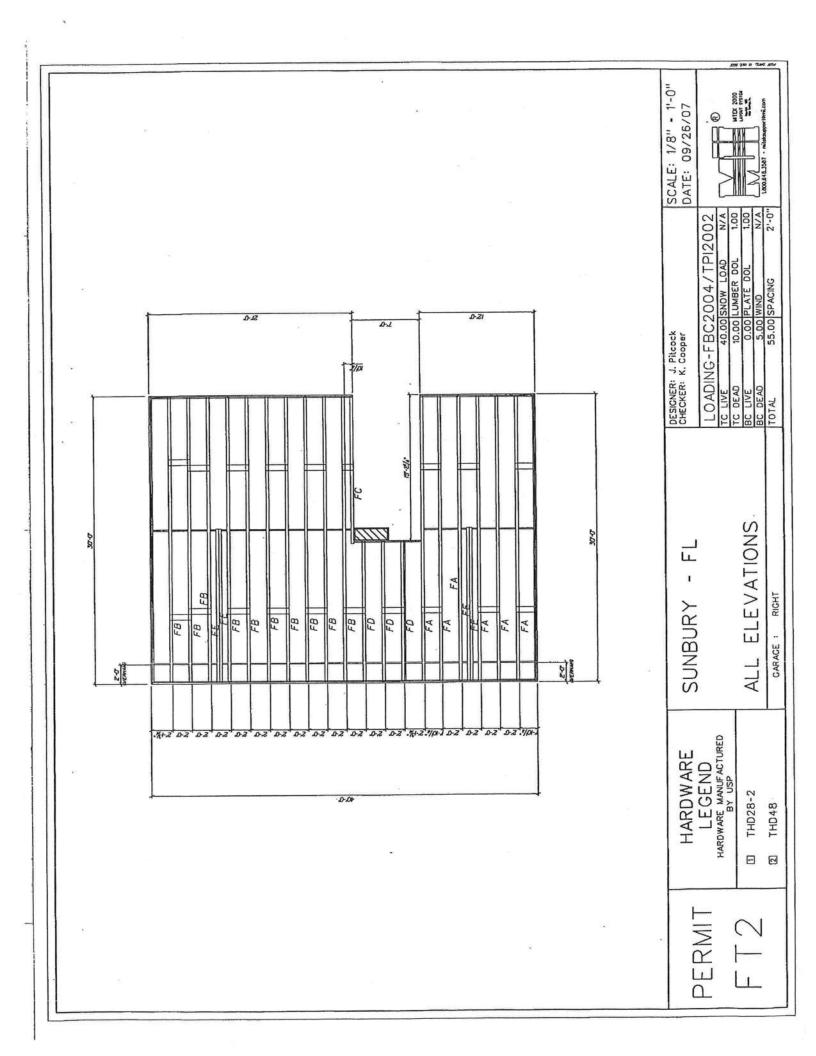
Rigid ceiling directly applied or 10-0-0 oc bracing.

December 27,2007

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RE: ELEV\_F - SUNBURY FL

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Project Customer: Maronda Homes Project Name: Sunbury FL

Lot/Block: Sanford

Subdivision: Sanford

Address: Sanford

State: FL

City: Sanford

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Design Program: MiTek 20/20 7.0

Wind Code: N/A

Wind Speed: N/A mph

Floor Load: 55.0 psf

Roof Load: N/A psf

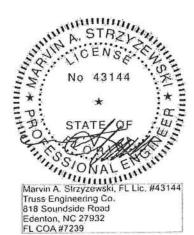
This package includes 5 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules. This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	
1	E4617771	FA	1/15/08	
2	E4617772	FB	1/15/08	
3	E4617773	FC	1/15/08	
4	E4617774	FD	1/15/08	
5	E4617775	FE	1/15/08	

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Strzyzewski, Marvin My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



January 15,2008

SUNBURY FL Qty Truss Type Job Trus E4617771 SUNBURY FL ALL ELEVATIONS FLOOR ELEV F Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:14 2008 Page 1 Maronda Homes, Inc. Pittsburgh, PA 9-7-8 9-7-8-1/8 Scale = 1:52.2 1-0-14 9-9-141 9-10-21 1-9-12 P-11-19 H 9-10-8 1-0-0 TRUSS IS DESIGNED TO SUPPORT A SMALL CONCENTRATED LOAD AT ITS CANTILEVERED END(S). 4x6 = 1.5x4 || 1.5x4 II 3x6 FP = 3x6 = 3x6 = 3x8 = 4x6 = 3x4 || 4x6 = 4x8 = 3x8 =3x4 = 3x6 = 3x6 = 4x6 1.5x4 II 3x4 = 3×4 19 20 21 22 23 17 15 16 9 12 13 14 M M 25 33 32 3130 29 27 26 36 37 42 40 39 38 50 48 3x6 = 3x6 FP = 1.5x4 II 3x6 = 3x4 4x8 II 5x6 = 5x6 5x8 1.5x4 II 1.5x4 || 1.5x4 II 5x6 = 3x6 3x6 11 1.5x4 II 3x6 = 3x6 FP = NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 27-10-8 27-9-0 21-7-8 21-2-6 8-6-0 7-7-2 0-1-8 1-10-8 0-5-2 5-8-6 4-5-10 0-10-14 7-7-2 0-5-2 0-10-14 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [8:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4 Edge], [32:0-1-8.Edge], [37:0-2-8.Edge], [42:0-1-8.Edge], [43:0-1-8.Edge], [45:0-1-8.Edge], [47:0-3-0.Edge], [48:0-1-8.Edge], [50:Edge,0-1-8] PLATES in (loc) I/defi 1/4 2-0-0 CSI LOADING (psf) SPACING 244/190 MT20 Vert(LL) -0.27 44-45 >616 360 1.00 1.00 TC 40.0 Plates Increase TCLL Vert(TL) -0 39 44-45 >424 240 0.71 BC Lumber Increase 1.00 TCDL 10.0 n/a Horz(TL) 0.03 39 n/a Rep Stress Incr NO WB 0.77 BCII 0.0 Weight: 161 lb Code FBC2004/TPI2002 (Matrix) 5.0 BCDL BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD TOP CHORD 4 X 2 SYP No.2 \*Except\* 13-23 4 X 2 SYP No.1D end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing, Except: **BOT CHORD** 4 X 2 SYP No.2 \*Except\* BOT CHORD 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D WEBS 4 X 2 SYP No.3 REACTIONS (lb/size) 50=2156/0-6-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (Ib) - Maximum Compression/Maximum Tension 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0, 6-7=-3375/274, 7-8=-3375/274, 8-9=-2177/949, 9-10=-142/1901, 10-11=-142/1901, 11-12=0/3301, 12-13=-231/1277, TOP CHORD 13-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, BOT CHORD 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 36-37=-1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30=-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, WEBS 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0, 7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44, 18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163 NOTES Unbalanced floor live loads have been considered for this design. Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face 3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means. CAUTION, Do not erect truss backwards. 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554 Ib down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others. In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Truss Engineering Co. 818 Soundside Road LOAD CASE(S) Standard Edenton, NC 27932 FL COA #7239 Continued on page 2 January 15,2008

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ANSI/TPIT Quality Criteria, DS\$-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onotria Drive. Madison. WI 53719.



Job	Truss	Truss Type	Qty P	y SUNBURY FL	E4617771	
ELEV_F	FA	FLOOR	6	1 SUNBURY FL ALL ELEVATIONS Job Reference (optional)		
	and the second second second		2 020 c tan 2 2008 MiTek Industries Inc. Tue Jan 15 12 03:14 2006			

Maronda Homes, Inc. Pittsburgh, PA

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 24-50=-10, 1-23=-100
Concentrated Loads (lb)
Vert: 23=-1554(F) 1=-1554(F)



SUNBURY FL Qty Truss Type Tob Truss E4617772 SUNBURY FL ALL ELEVATIONS 10 ELEV\_F FLOOR FB Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:16 2008 Page 1 Maronda Homes, Inc., Pittsburgh, PA 0-10-4 0-1-8 9-7-8 9-7-8-1-8 Scal 9-9-141 1-0-14 9-10-21 1-9-12 9-11-19 H 9-10-8 1-0-0 NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION. 1.5x4 || 1.5x4 II 1.5x4 II 3x6 FP = 3x6 4x6 = 3x6 = 3x6 = 3x8 = 4x6 = 3x4 | | 4x8 = 3x4 = 3x6 = 4x8 = 3x8 = 4x6 = 3x6 =1.5x4 || 3x4 3x4 19 20 21 22 23 15 16 18 17 9 10 11 12 13 14 4 2 3 36 33 3130 29 27 26 25 39 37 35 32 40 46 45 43 42 50 3x6 FP = 4x6 = 4x8= 3x6 = 5x6 4x8 = 3x6 = 3x6 = 3x4 = 1.5x4 4x6 | 1 5x6 5x8 3x4 = 1.5x4 II 1.5x4 II 3x6 = 3x6 = 1.5x4 II 5x6 = 3x6 II 3x8 = 3x4 = 3x6 = 3x6 FP = 1.5x4 II 29-9-0 9-4-14 27-10-8 21-7-8 13-10-8 21-2-6 8-6-0 7-7-2 7-3-14 0-5-2 5-8-6 0-1-8 1-10-8 0-10-14 4-5-10 7-7-2 0-5-2 0-10-14 Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-2-0,Edge], [5:0-1-8,Edge], [6:0-1-8,Edge], [12:0-3-0,Edge], [16:0-1-8,Edge], [18:0-2-12,Edge], [23:0-3-0,Edge], [26:0-2-4 Edge], [32:0-1-8.Edge], [37:0-2-8.Edge], [42:0-1-8.Edge], [43:0-1-8.Edge], [45:0-1-8.Edge], [47:0-3-0.Edge], [48:0-1-8.Edge], [50:Edge,0-1-8] GRIP PLATES DEFL in (loc) I/defi 1 /d CSI 2-0-0 SPACING LOADING (psf) 244/190 MT20 Vert(LL) -0 27 44-45 >616 360 TC 1.00 1.00 TCLL 40.0 Plates Increase 240 0.71 Vert(TL) -0.39 44-45 >424 1.00 BC TCDL 10.0 Lumber Increase 0.03 39 n/a n/a WB 0.77 Horz(TL) Rep Stress Incr NO BCLL Weight: 161 lb Code FBC2004/TPI2002 (Matrix) BCDL 5.0 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD TOP CHORD 4 X 2 SYP No.2 \*Except\* end verticals. 13-23 4 X 2 SYP No.1D Rigid ceiling directly applied or 6-0-0 oc bracing, Except: **BOT CHORD** BOT CHORD 4 X 2 SYP No.2 \*Except\* 10-0-0 oc bracing: 49-50,48-49,47-48,46-47,45-46. 41-50 4 X 2 SYP No.1D, 41-48 4 X 2 SYP No.1D 4 X 2 SYP No.3 WEBS REACTIONS (lb/size) 50=2156/0-3-8, 39=1576/0-3-8, 26=2595/0-8-0 Max Grav 50=2222(LC 4), 39=1795(LC 2), 26=2654(LC 3) FORCES (lb) - Maximum Compression/Maximum Tension 50-51=-2249/0, 1-51=-2245/0, 24-52=0/6, 23-52=0/6, 1-2=-1421/0, 2-3=-1421/0, 3-4=-3096/0, 4-5=-3766/0, 5-6=-3969/0 TOP CHORD 6-7=-3375/274, 7-8=-3375/274, 8-9=-2177/949, 9-10=-142/1901, 10-11=-142/1901, 11-12=0/3301, 12-13=-231/1277, 13-14=-231/1277, 14-15=-1561/1325, 15-16=-2271/1671, 16-17=-2356/1983, 17-18=-2356/1983, 18-19=-1487/2899, 19-20=0/3875, 20-21=0/4396, 21-22=0/4393, 22-23=0/2157 49-50=0/428, 48-49=0/2299, 47-48=0/2302, 46-47=0/3615, 45-46=0/3969, 44-45=-274/3375, 43-44=-274/3375, BOT CHORD 42-43=-949/2177, 41-42=-1272/1229, 40-41=-1272/1225, 39-40=-3301/0, 38-39=-1912/0, 37-38=-1912/0, 36-37=-1241/1034, 35-36=-1448/2085, 34-35=-1448/2085, 33-34=-1983/2356, 32-33=-1983/2356, 31-32=-2514/2031, 30-31=-2514/2031, 29-30=-2514/2031, 28-29=-3317/904, 27-28=-3317/904, 26-27=-3875/0, 25-26=-2157/0, 24-25=-1/0 2-49=-96/0, 3-49=-1012/0, 3-48=-103/0, 3-47=0/909, 4-47=-602/21, 4-46=-99/175, 5-46=-231/91, 5-45=-279/0, WEBS 6-45=0/1260, 11-40=0/1687, 10-40=-93/15, 9-40=-1364/0, 9-42=0/1158, 8-42=-804/0, 8-43=0/1911, 12-39=-1581/0, 12-38=-25/3, 12-37=0/1223, 14-37=-1128/32, 14-36=-101/729, 15-36=-725/144, 15-35=-27/19, 6-44=-552/0, 7-43=-386/0, 11-39=-965/0, 16-33=-278/117, 17-32=-243/1, 21-26=-197/0, 15-34=-261/416, 16-34=-441/497, 20-26=-1584/0, 20-27=0/715, 19-27=-1491/0, 19-28=0/18, 19-29=0/1026, 18-29=-960/0, 18-31=-85/44, 18-32=-175/1044, 22-25=0/1553, 22-26=-2766/0, 23-25=-2681/0, 1-49=0/1163 NOTES

1) Unbalanced floor live loads have been considered for this design.

2) Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

4) CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0, and 1554 lb down at 29-6-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2



818 Soundside Road Edenton, NC 27932 FL COA #7239

January 15,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USB. Design valid for use only with Milek connectors. This design is based only upon parameters have, and is for an individual building component. Applicability of design parameters and reper incorporation of component is responsibility of building designer - not lruss designer. Bracing shis for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of each of the construction of the overall structure is the responsibility of building designer, for general guidance regarding designer, for general guidance regarding area of the overall structure is the responsibility of the additional permanent bracing of the overall structure is the responsibility of the properties of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure in the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall structure is the responsibility of the overall guidance regarding area of the overall guidance of the overall guidanc



Job	Truss	Truss Type	City	Ply	SUNBURYFL	E4617772
ELEV_F	FB	FLOOR	10		1 SUNBURY FL ALL ELEVATIONS Job Reference (optional)	
	Burney Br				7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:16	2008 Page 2

Maronda Homes, Inc, Pittsburgh, PA

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (pIf)
Vert: 24-50=-10, 1-23=-100
Concentrated Loads (lb)

Vert: 23=-1554(F) 1=-1554(F)

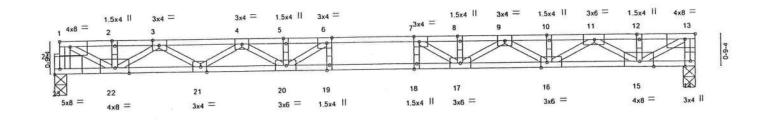
Job	Truss	Truss Type	Qty	Ply	SUNBURY FL E4	617773
ELEV_F	FC	FLOOR	1		SUNBURY FL ALL ELEVATIONS Job Reference (optional)	

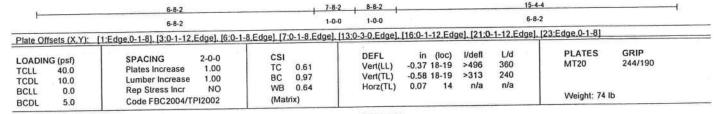
Maronda Homes, Inc., Pittsburgh, PA

0-1-8 H 1-0-0

1 0-11-2 2-0-0 0-11-2

Scale = 1:26.3





LUMBER

TOP CHORD 4 X 2 SYP No.2

BOT CHORD 4 X 2 SYP No.1D

4 X 2 SYP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-12 oc purlins, except

end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** 

REACTIONS (lb/size) 23=1239/0-3-8, 14=834/0-3-8 Max Grav 23=2248(LC 2), 14=834(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

23-24=-2276/0, 1-24=-2272/0, 13-14=-823/0, 1-2=-1606/0, 2-3=-1606/0, 3-4=-3362/0, 4-5=-4469/0, 5-6=-4469/0, 6-7=-4793/0, 7-8=-4450/0, 8-9=-4450/0, 9-10=-3317/0, 10-11=-3317/0, 11-12=-1412/0, 12-13=-1412/0 TOP CHORD

**BOT CHORD** 

22-23=0/433, 21-22=0/2645, 20-21=0/4045, 19-20=0/4793, 18-19=0/4793, 17-18=0/4793, 16-17=0/3986, 15-16=0/2455, 14-15=0/0

6-19=-126/139, 7-18=-107/157, 1-22=0/1558, 2-22=-99/0, 3-22=-1216/0, 3-21=0/854, 4-21=-813/0, 4-20=0/495, 5-20=-131/103, 13-15=0/1607, 12-15=-121/0, 11-15=-1220/0, 11-16=0/1010, 10-16=-101/0, 9-16=-782/0, 9-17=0/543,

8-17=-144/113, 7-17=-835/65, 6-20=-768/125

### NOTES

WEBS

Unbalanced floor live loads have been considered for this design.

2) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

3) CAUTION, Do not erect truss backwards.

4) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1554 lb down at 0-3-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 14-23=-10, 1-13=-100

Concentrated Loads (lb)

Vert: 1=-424(F)

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932

FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters and KEAD ROTES ON 14115 AND INCLUDED RITTER REPRESENCE FACE BILLY AND BILLY BIL



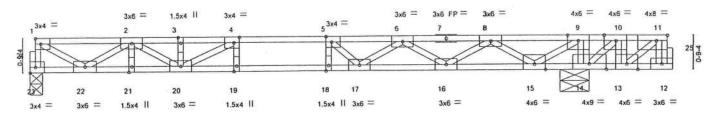
Edenton, NC 27932

Job	Truss	Truss Type	Qty Ply SUNBURY FL E4617774
ELEV_F	FD	FLOOR	3 1 SUNBURY FL ALL ELEVATIONS Job Reference (optional)
Maranda Homes In	r Pittshumh PA		7.030 s Jan 3 2008 MiTek Industries, Inc. Tue Jan 15 12:03:18 2008 Page 1

Maronda Homes, Inc. Pittsburgh, PA

H 1-0-0

1-1-14 2-0-0 1-0-7-14 | 0-7-0 | 0-8-0 | 0-β-β | Scale = 1:25.1



NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION.

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 6-0-0 oc bracing.

T-	4-9-6		5-9-6	6-9-6		1	13	2-8-4		12-9-12	14-8-4
	4-9-6		1-0-0	1-0-0			5-	10-14		0-1-8	1-10-8
Plate Offsets (X,Y):	[4:0-1-8.Edge], [5:0-1-8.E	dge], [11:0-3	-0.Edge]								
OADING (psf)	SPACING	2-0-0	-CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC	0.96	Vert(LL)	-0.20	18	>768	360	MT20	244/190
CDL 10.0	Lumber Increase	1.00	BC	0.80	Vert(TL)	0.22	17-18	>693	240		
BCLL 0.0	Rep Stress Incr	NO	WB	0.55	Horz(TL)	-0.03	14	n/a	n/a	1 100 000 000 EARWAY	
3CDL 5.0	Code FBC2004/TF	212002	(Mat	rix)						Weight: 72 lb	

BRACING

TOP CHORD

**BOT CHORD** 

end verticals.

LUMBER

TOP CHORD 4 X 2 SYP No.1D \*Except\*

1-7 4 X 2 SYP No.2

BOT CHORD 4 X 2 SYP No.1D WEBS

4 X 2 SYP No.3

REACTIONS (lb/size) 23=586/0-3-8, 14=1595/0-8-0

Max Uplift23=-42(LC 3)

Max Grav 23 = 596(LC 2), 14 = 2302(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

23-24=-590/45, 1-24=-588/45, 12-25=0/6, 11-25=0/6, 1-2=-876/108, 2-3=-2133/375, 3-4=-2133/375, 4-5=-2473/1091,

5-6=-2187/1692, 6-7=-1172/2634, 7-8=-1172/2634, 8-9=0/3716, 9-10=0/4331, 10-11=0/2240

22-23=-4/57, 21-22=-250/1672, 20-21=-250/1672, 19-20=-1091/2473, 18-19=-1091/2473, 17-18=-1091/2473, **BOT CHORD** 

16-17=-2196/1810, 15-16=-3136/495, 14-15=-4331/0, 13-14=-2240/0, 12-13=-1/0 WEBS

9-14=-849/0, 4-19=-291/73, 5-18=-64/449, 1-22=-122/958, 2-22=-931/166, 9-15=0/1300, 8-15=-1212/0, 2-21=-48/20,

2-20=-145/532, 3-20=-206/42, 4-20=-570/802, 8-16=0/850, 6-16=-775/0, 6-17=0/913, 5-17=-1165/0, 11-13=-2740/0,

10-13=0/1537, 10-14=-2645/0

### NOTES

1) Unbalanced floor live loads have been considered for this design.

- Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face
- 3) Attach ribbon block to truss with 3-10d nails applied to flat face.
- 4) One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) 23.
- 5) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1547 lb down at 14-5-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 12-23=-10, 1-11=-100

Concentrated Loads (lb)

Vert: 11=-612(F)



FL COA #7239

January 15,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing show is for toteral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the exercitor. Additional permanent bracing of the averall structure is the responsibility of the diding designer, For general guidance regarding tobication, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Salely Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



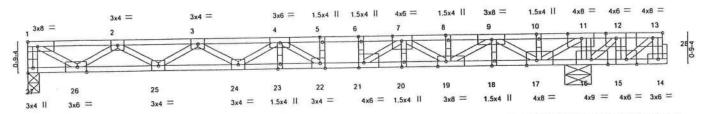
			IQIv   P(v   SUNBURY FL
Job	Truss	Truss Type	City Pily SUNBORT FL E4617775
ELEV_F	FE	FLOOR	4 1 SUNBURY FL ALL ELEVATIONS Job Reference (optional)
1			7 020 - top 2 2008 MiTak Industries Inc. Tue Jan 15 12:03:18 2008 Page 1

Maronda Homes, Inc. Pittsburgh, PA 1-0-0

0-11-6 0-10-4 0-11-6

0-7-0 0-8-0 0-1-8

Scale = 1:27.5



NOTE: DUE TO THE OVERALL CANTILEVER LENGTH THE FLOOR MAY EXHIBIT OBJECTIONABLE VIBRATION AND/OR DEFLECTION. BUILDING DESIGNER TO CONSIDER PROVIDING MEANS TO DAMPEN POSSIBLE FLOOR VIBRATION.

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 6-0-0 oc bracing, Except:

		8-3-10			
	7-5-6	7-10-8	14-0-0	14118	16-0-0
-	7-5-6	0-5-2	5-8-6	0-1-8	1-10-8
		0-5-2			

LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0	SPACING 2-0-0 Plates Increase 1.00 Lumber Increase 1.00 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.95 BC 0.81 WB 0.69 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in (loc) -0.25 22-23 -0.32 22-23 0.04 16	l/defl >658 >522 n/a	L/d 360 240 n/a	PLATES GRIP MT20 244/190 Weight: 82 lb
--	---	--	--	---	-------------------------------	--------------------------	--

BRACING

TOP CHORD

**BOT CHORD** 

end verticals.

10-0-0 oc bracing: 26-27.

LUMBER

TOP CHORD 4 X 2 SYP No.1D

BOT CHORD 4 X 2 SYP No.1D

WEBS

4 X 2 SYP No.3

REACTIONS (lb/size) 27=562/0-3-8, 16=2658/0-8-0

Max Grav27=708(LC 2), 16=2658(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD

1-27=-699/0, 14-28=0/6, 13-28=0/6, 1-2=-1062/24, 2-3=-2548/191, 3-4=-3309/553, 4-5=-3329/1301, 5-6=-3329/1301, 6-7=-3329/1301, 7-8=-2158/2382, 8-9=-2158/2382, 9-10=-200/3578, 10-11=-200/3578, 11-12=0/4198, 12-13=0/2181

26-27=-0/0, 25-26=-71/1984, 24-25=-347/3079, 23-24=-774/3540, 22-23=-774/3540, 21-22=-1301/3329,

**BOT CHORD** 20-21=-1929/2780, 19-20=-1929/2780, 18-19=-2943/1292, 17-18=-2943/1292, 16-17=-4198/0, 15-16=-2181/0,

14-15=-1/0

5-22=-61/204, 6-21=-354/0, 11-16=-1007/0, 1-26=-28/1225, 2-26=-1098/56, 2-25=-143/671, 3-25=-633/185, 3-24=-246/273, 4-24=-270/259, 4-23=0/135, 4-22=-952/158, 11-17=0/1722, 10-17=-101/9, 9-17=-1564/0, 9-18=0/19, WEBS

9-19=0/1293, 8-19=-130/0, 7-19=-975/0, 7-20=-117/15, 7-21=0/1281, 13-15=-2667/0, 12-15=0/1493, 12-16=-2559/0

### NOTES

1) Unbalanced floor live loads have been considered for this design.

2) Posi-Strut webs to be applied to both sides of truss unless otherwise noted. FF=Front Face, BF=Back Face

3) Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

CAUTION, Do not erect truss backwards.

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1500 lb down at 15-9-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

### LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 14-27=-10, 1-13=-100

Concentrated Loads (lb)

Vert: 13=-1500(F)

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932

January 15,2008

WARNING · Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown as for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the undiding designer, For general guidance regarding fobrication, quality controls, storage, delivery, erection and bracing, consult. ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



Edenton, NC 27932

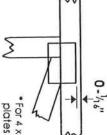
FL COA #7239

### Symbols

### PLATE LOCATION AND ORIENTATION



and fully embed teeth Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss offsets are indicated. Center plate on joint unless x, y



For 4 x 2 orientation, locate edge of truss. plates 0- 1/46" from outside

• This symbol indicates the required direction of slots in connector plates.

\*Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 × 4

width measured perpendicular the length parallel to slots. to slots. Second dimension is The first dimension is the plate

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

### BEARING



number where bearings occur (supports) occur. Icons vary but reaction section indicates joint Indicates location where bearings

### ANSI/TPI1: Industry Standards:

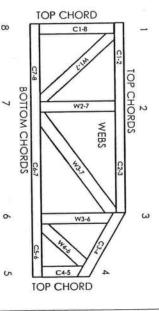


Installing & Bracing of Metal Plate Connected Wood Trusses. Building Component Safety Information. Guide to Good Practice for Handling. Plate Connected Wood Truss Construction. National Design Specification for Metal Design Standard for Bracing.

DSB-89

### Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

ICC-ES Reports:

95110, 84-32, 96-67, ER-3907, 9432A ESR-1311, ESR-1352, ER-5243, 9604B NER-487, NER-561 9730, 95-43, 96-31, 9667A

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MiTek Engineering Reference Sheet: MII-7473

## General Safety Notes

### Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII
- may require bracing, or alternative  $\mathsf{T}$ ,  $\mathsf{I}$ , or Eliminator bracing should be considered. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- designer, erection supervisor, property owner and all other interested parties. Provide copies of this truss design to the building
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of tabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chards must be sheathed or purins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all partions of this design (front, back, words and pictures) before use, Reviewing pictures alone is not sufficient
- Design assumes manufacture in accordance with ANSI/TPI I Quality Criteria.

### COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006 Supplements and One (1) and Two (2) Family Dwellings

### ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ------110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

### **GENERAL REQUIREMENTS;**

- Y Two (2) complete sets of plans containing the following:
- All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

### Site Plan information including:

- Dimensions of lot or parcel of land
  - Dimensions of all building set backs
- Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- Provide a full legal description of property.

### Wind-load Engineering Summary, calculations and any details required:

- / Plans or specifications must meet state compliance with FRC Chapter 3
  - The following information must be shown as per section FRC
  - · Basic wind speed (3-second gust), miles per hour
  - Wind importance factor and nature of occupancy
- Wind exposure if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
  - The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

### **Elevations Drawing including:**

- All side views of the structure
- Roof pitch
  - Overhang dimensions and detail with attic ventilation
  - Location, size and height above roof of chimneys
- Location and size of skylights with Florida Product Approval Number of stories
- e) Building height from the established grade to the roofs highest peak

Floor Plan including:	
Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck,	
balconies and raised floor surfaces located more than 30 inches above the floor or grade	
All exterior and interior shear walls indicated	
Shear wall opening shown (Windows, Doors and Garage doors	
<ul> <li>Emergency escape and rescue opening in each bedroom (net clear opening shown)</li> </ul>	
✓ Safety glazing of glass where needed	
o Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10	
/ of FRC)	
Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC	
311)	
Plans must show and identify accessibility of bathroom (see FRC 322)	
All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida	
product approval number and mfg. installation information submitted with the plans (see Florida product	
approval form)	
Foundation Plans Per FRC 403:	
a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and	4
/ type of reinforcing.	*
b) All posts and/or column footing including size and reinforcing	
c) Any special support required by soil analysis such as piling.	
o d) Assumed load-bearing valve of soil (psf)	
e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)	
CONCRETE SLAB ON GRADE Per FRC R506	
Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	
Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	
PROTECTION AGAINST TERMITES Per FRC 320:	
Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit	
other approved termite protection methods. Protection shall be provided by registered termiticides	
Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606	
Show all materials making up walls, wall height, and Block size, mortar type	
Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	
Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof.	
Engineer or Architect	
Floor Framing System: First and/or second story	
Floor truss package shall including layout and details, signed and sealed by Florida Registered	
Professional Engineer	
Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem	
walls and/or priers  Girder type, size and spacing to load bearing walls, stem wall and/or priers	
Attachment of joist to girder	
Wind load requirements where applicable	
Show required under-floor crawl space	

Show required amount of ventilation opening for under-floor spaces

Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &

Show fireproofing requirements for garages attached to living spaces, per FRC section R309

Show the required access opening to access to under-floor spaces

Provide live and dead load rating of floor framing systems (psf).

Show required covering of ventilation opening.

intermediate of the areas structural panel sheathing Show Draft stopping, Fire caulking and Fire blocking

2

### WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
  - Fastener schedule for structural members per table R602.3 (1) are to be shown.
- Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- Indicate where pressure treated wood will be placed.
- Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

### **ROOF SYSTEMS:**

- Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- Provide dead load rating of trusses

### Conventional Roof Framing Layout Per FRC 802:

- Rafter and ridge beams sizes, span, species and spacing
- Connectors to wall assemblies' include assemblies' resistance to uplift rating.
  - Valley framing and support details
- Provide dead load rating of rafter system.

### **RØOF SHEATHING FRC Table R602,3(2) FRC 803**

Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

### **ROOF ASSEMBLIES FRC Chapter 9**

Include all materials which will make up the roof assembles covering; with Florida Product Approval numbers for each component of the roof assembles covering.

### FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

### HXAC information shown

- Manual J sizing equipment or equivalent computation
- Exhaust fans locations in bathrooms

### Plumbing Fixture layout shown

All fixtures waste water lines shall be shown on the foundation plan

### Mectrical layout shown including:

- Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- // /Ceiling fans
  - Smoke detectors
  - Service panel, sub-panel, location(s) and total ampere ratings

On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.

Appliances and HVAC equipment and disconnects Arc Fault Circuits (AFCI) in bedrooms

Notarized Disclosure Statement for Owner Builders

Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice
 <u>Of Commencement is required to be filed with the building department Before Any
 Inspections Will Be Done.</u>

### **Private Potable Water**

- Size of pump motor
- Size of pressure tank
- Cycle stop valve if used

### THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

<u>Building Permit Application:</u> A current Building Permit Application form is to be completed and submitted for all residential projects.

<u>Parcel Number:</u> The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

- <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED. A development permit will also be required. The permit cost is \$50.00.
- <u>Driveway Connection:</u> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

### PRODUCT APPROVAL SPECIFICATION SHEET

Location:		Project Name:	
which you are applying for a	n the building comp building permit or the product approva	da Administrative Code 9B-72, please provide conents listed below if they will be utilized on the or after April 1, 2004. We recommend you all number for any of the applicable listed product at a second contact and applicable listed products.	ne construction project fo
Category/Subcategory	Manufacturer	Product Description	Approval Number(s
A. EXTERIOR DOORS			
Swinging			
2. Sliding			
Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
Single hung			
Horizontal Slider			
3. Casement			
Double Hung			
5. Fixed			
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
Membrane     Greenhouse			
10. Other			
1. Asphalt Shingles			
Asphalt Shingles     Underlayments			
Roofing Fasteners			
Non-structural Metal Rf			
Built-Up Roofing			
Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			
12. Nooning State			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
<ol> <li>Cements-Adhesives – Coatings</li> </ol>			
15. Roof Tile Adhesive			
16. Spray Applied			
Polyurethane Roof			
17. Other			
E. SHUTTERS			
Accordion			
2. Bahama			
Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
Skylight			
2. Other			
G. STRUCTURAL			
COMPONENTS			
<ol> <li>Wood connector/anchor</li> </ol>			
2. Truss plates			
Engineered lumber			
4. Railing			
5. Coolers-freezers			
Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR			
ENVELOPE PRODUCTS			
1.			
2.			
obsite; 1) copy of the production of these pro obsite; 1) copy of the production and certified to comply with, 3	oducts, the follow t approval, 2) the s) copy of the app	e product approval at plan review. wing information must be available e performance characteristics whic plicable manufacturers installation emoved if approval cannot be dem	to the inspector on the ch the product was tested requirements.
ontractor or Contractor's Audit			
ontractor or Contractor's Authorized	Agent Signature	Print Name	Date
ocation		Permit # (FOR STAF)	F USE ONLY)

Effective April 1, 2004

Website:

02/02/04 - 2 of 2

### GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

### FIELD DENSITY WORKSHEET

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COMPACTION REQUIREMENT (%)	15/	_ • Mo	odified P	roctor	-0	1378,0	K_FIELD	CONTACT
TOTAL ON-SITE TIME					MILES	FROM OF	FICE	
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*	LAB PR	OCTOR	TEST	PROBE	0/	WET	DRY	0,
TEST LOCATION	DENS.	OMC	DEPTH	DEPTH	MOIST.	(PCF)	(PCF)	% COMP.
CTR. Of PAD	103.7	120	Fb	12"	6.6	108.1	101.4	97.8
CAR. Of H. FAC.		1	1	1	7.6	106.3	98.8	95.3
CHR of S. Ftb.	1	1	1	1	6.9	106.4	99.6	96.0
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### STATE OF FLORIDA DEPARTMENT OF HEALTH

08-403

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

	Permit Application Num	ber
	- PART II - SITE PLAN	
Scale: Each block represents 5 feet and 1 inch = 50	) feet.	
Notes:		18
		4
Sito Blan automitted by		
Site Plan submitted by:	Nulson Signature	AGONT Title
Plan Approved X	Not Approved CFD	Date c/a/8
By PCUP >	Character 100	_ County Health Departm

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### **Detail by Entity Name**

### Florida Profit Corporation

MARONDA HOMES, INC. OF FLORIDA

### Filing Information

**Document Number 562259** 

FEI Number

251336949

Date Filed

03/17/1978

State

FL

Status

**ACTIVE** 

### Principal Address

202 PARK WEST DR PITTSBURGH PA 15275 US

Changed 04/11/2002

### Mailing Address

202 PARK WEST DR PITTSBURGH PA 15275 US

Changed 04/11/2002

### Registered Agent Name & Address

VON DREELE, WAYNE J 3993 WEST FIRST STREET SANFORD FL 32771 US

Name Changed: 02/09/2000

Address Changed: 03/01/2006

### Officer/Director Detail

### Name & Address

Title CEOD

WOLF, WILLIAM J. 202 PARK WEST DRIVE PITTSBURGH PA 15275

Title VPSD

WOLF, RONALD W. 202 PARK WEST DRIVE PITTSBURGH PA 15275

Title P

VON DREELE, WAYNE J 3993 WEST FIRST STREET SANFORD FL 32771

Title VP

FALCK, MARK 3993 WEST FIRST ST SANFORD FL 32771

Title V.P.

ROUSCH, WILLIAM 4610 LIPSCOMB STREET, SUITE 300 PALM BAY FL 32905

Title V.P.

HOWARD, SCOTT 955 KELLER ROAD, SUITE 1500 **ALTAMONTE SPRINGS FL 32714** 

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### Report Year Filed Date

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03/07/2007

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10/29/2007 01/02/2008

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Inst. Number: 200712024085 Book: 1134 Page: 1698 Date: 10/26/2007 Time: 4:45:00 PM Page 1 of 2

This instrument prepared by: William J. Haley, Esquire Brannon, Brown, Haley & Bullock, P. A. P. O. Box 1029 Lake City, FL 32056-1029

> Inst:200712024085 Date:10/26/2007 Time:4:45 PM Doc Stamp-Deed:3276.00 \_\_\_\_\_\_\_DC,P.DeWitt Cason,Columbia County Page 1 of 2

### SPECIAL WARRANTY DEED

THIS INDENTURE, made this \_/Oth \_ day of October, 2007, between RML HOLDINGS, INC., a Florida corporation, having a mailing address of 703 NW Blackberry Circle, Lake City, Florida 32055, hereinafter referred to as Grantor, and MARONDA HOMES, INC. OF FLORIDA, a Florida corporation, having a mailing address of 3993 West First Street, Sanford, Florida 32771, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of \$10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, towit:

Lot(s) 2, 3, 4, 5, 6, 7, 21, 22, 23, 25, and 26, TIMBERLANDS, PHASE 1, according to the plat thereof, as recorded in Plat Book 9, pages 26-27, public records of Columbia County, Florida.

PARCEL NO.

Part of 10-4S-16-02856-000 and 15-4S-16-02990-000

SUBJECT TO:

Taxes and special assessments for the year 2007 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

### **Columbia County Building Department Culvert Permit**

### Culvert Permit No.

000001626

DATE 06/30	0/2008 PARCEL ID #	10-4S-16-02856-108	
APPLICANT	THEODORE BROCK	PHONE	904.296.1490
ADDRESS 6	800 SOUTHPOINTE PKWY # 300	JACKSONVILLE	FL 32216
OWNER MA	ARONDA HOMES INC. OF FL	PHONE	904.296.1490
ADDRESS 27	77 SW TIMBER RIDGE DRIVE	LAKE CITY	FL 32024
CONTRACTO	R THEODORE BROCK	PHONE	407.227.3504
LOCATION OF	F PROPERTY 90-W TO SR. 247-S,TL TO	C-252-B,TR TO TIMBER R	IDGE,TL
8TH LOT ON R.	<del></del>		
SUBDIVISION/	/LOT/BLOCK/PHASE/UNIT TIMBERL	ANDS	8 1
/	19 1814	10	
x	INSTALLATION REQUIREMENT Culvert size will be 18 inches in diameter driving surface. Both ends will be mitered thick reinforced concrete slab.	— er with a total lenght of 3	32 feet, leaving 24 feet of see and poured with a 4 inch
	INSTALLATION NOTE: Turnouts will be a) a majority of the current and existing b) the driveway to be served will be partially to the concrete or paved concrete or paved driveway, whiches current and existing paved or concre	ng driveway turnouts are aved or formed with con d a minimum of 12 feet we ever is greater. The width	crete. wide or the width of the
	Culvert installation shall conform to the	e approved site plan sta	ndards.
	Department of Transportation Permit in	nstallation approved sta	ndards.
	Other		
ALL DDODED CA	FETY REQUIREMENTS SHOULD BE FOLI	OWED	650

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

DURING THE INSTALATION OF THE CULVERT.

Amount Paid 25.00



### Columbia County Building Permit Application

12/15/ 22	13 /6 40 2713
For Office Use Only Application # 0806.33 Date Received 6	A C
Zoning Official Date 20.0608 Flood Zone X Polit FEM	
Land Use Elevation NA MFE 100.0 River NA Plan	ns Examiner Date 6/20/08
Comments Elevation Confirmation Lello Reguird	• 19 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
NOC EH Deed or PA Site Plan State Road Info Parent Parcel	
Dev Permit # In Floodway - Letter of Authorization	the state of the s
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Tow	A TOTAL PROPERTY OF THE PARTY O
Septic Permit No.	Fax (904)-332-6367
Name Authorized Person Signing Permit Theodore C. Brock / ATRICK Will	Suple Phone (904)-296-1490
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216	
Owners Name Maronda Homes Inc. of Florida	Phone (904)-296-1490
911 Address 277 SW Timber Ridge Dr. , LAN	
	Phone (407)-227-3504
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216	Const. Find the second of the
Fee Simple Owner Name & Address N/A	THE
Bonding Co. Name & AddressN/A	
Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way S	Sanford, FL 32771
Mortgage Lenders Name & Address_Bank of America 250 Park Ave. S	
Circle the correct power company - FL Power & Light - Clay Elec. Sur	The state of the s
Property ID Number 10-45-10-02850-108 Estimated Co.	st of Construction \$100,035.00
그는 그렇게 그가 없었다면서 이번 경기를 하는 것이 되었다면 하다면 가장 하는 것이 되었다면 하는데 그렇게 되었다면 그렇게	ot 8/1 Block Unit Phase
Driving Directions Hwy 90, Left on 247 South; Right on 252B; Lef	t on Timber Kruge Skipe
8th lot on Right	- 1350 - 1865 N A
Number of Exis	sting Dwellings on Property
Construction of Residential Single Family Dwelling	otal Acreage 50 _ Lot Size
Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive	
	(1) Side 34'(1) Rear89'
Number of Stories 2 Heated Floor Area 2223 Total Floor Area	
Number of Stories Heated Floor Area LILL Total Floor Area	a 3 V Roof Pitch
Application is hereby made to obtain a permit to do work and installations as installation has commenced prior to the issuance of a permit and that all wo	s indicated. I certify that no work or
of all laws regulating construction in this jurisdiction.	ACCOUNT OF THE PROPERTY OF T
# 9	26.68

Page 1 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

# 926.68

Revise

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Commission Country Sestering Carrons whitegrands

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WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certif	y that all the fore	igoing information	n is accurate and all	work will be
done in compliance with all applicable	laws and regulat	ing construction	and zoning. I further	understand
the above whiten responsibilities in Co	lumbia County for	or obtaining this E	Building Permit.	
Owners Signature BHE HOCK	<del></del>		3 3 × "	
Swilding Blove + (CCB)		: : : : : : : : : : : : : : : : : : :	*	
•	*,			
CONTRACTORS AFFIDAVIT: By my signa	turo Lundorstani	d and arroo that I	have informed and	novided this
written statement to the owner of all the	e above written r	esponsibilities in	Columbia County for	roptaining
this Building Pemit.	, a			*2
Jec &		Contractor's Licen	se Number (BC)2	56382
Contractor's Signature (Permitee)	*	Columbia County		
. Theodore C. Brock		Competency Card	Number	
¥			*	9
	7). (f)		0.4	
Affirmed under penalty of perjury to by the	Contractor and su	bscribed before me	this 29 day of M	VC 2008
Personally known XXX or Produced Identification				
nial AMADA	RI	a contract of	(.*	
Melma i Magne	SE	AL:	200	
State of Florida Notary Signature (For the C Melissa L. McKague	ontractor)	200	Notary Public State of Florid Melissa L McKague My Commission DD493647	1
			Expires 11/22/2009	1

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Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

51/2

NOTICE OF COMMENCEMENT

Inst: 200812012528 Date: 7/2/2008 Time: 10:39 AM
DC, P. DeWitt Cason, Columbia County Page 1 of 1 B:1153 P:2198

Tax Parcel Identification Number 10-45-10-02850-108 County Clerk's Office Stamp or Seal
THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.
1. Description of property (legal description): 811 TMD/ GMD  a) Street (job) Address:
a) Street (job) Address:  2. General description of improvements:  ONSTRUCTOR of a STRATE FARM OF AWAITING
a) Name and address: MIONA HOMES INC. OF LOOD SOUTH POINT PKWY #300 WX FL 30016 b) Name and address of fee simple titleholder (if other than owner) c) Interest in property
4. Contractor Information a) Name and address: MAYONGA HOMES INC & PL USOD SOUTHPOINT PICKY \$300. Dr. TO 32210 b) Telephone No. (104) 2910 - 1990 Fax No. (Opt.) (104) 332-103-15  5. Surety Information
a) Name and address:
n) Name and address: b) Amount of Bond: c) Telephone No.: Fax No. (Opt.)
The Loridon
a) Name and address: b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:  a) Name and address: SWTTLYN TITLE HOWING OD LUC 3943 BAYMEMOWS Rd JAX FL 32227  b) Telephone No.: (904) 739-2205 Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(I)(b), Florida Statutes:
a) Name and address:
b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.
STATE OF FLORIDA COUNTY OF COLUMBIA  10.
Signature of Owner or Ordor's Authorized Office/Director/Purtner/Manager
Print Name C- Brock
The foregoing instrument was acknowledged before me, a Plorida Notary, this
MALYONG LANGE OF TAXABLE OF TAXABLE OF TAXABLE ACTION (type of authority, e.g. officer, trustice, attorney
Personully Known OR Produced Identification Type Notary Public State of Florida
Notary Signature MCKOffe  Notary Stamp or Scal: Stamp of Scal: McKague My Commission DD493647 Expires 11/22/2009
I. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.
Signature of Natural Person Signing in Line (10 obaye)

### FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: SUNBURY GAINESVILLE Address: 27 SW TIMBUR RIME Dr. City, State: LANC CITY, R. 32055 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: COLUMDIA Permit Number: Jurisdiction Number:
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit  b. N/A  c. N/A  13. Heating systems a. Electric Heat Pump  b. N/A  c. N/A  14. Hot water systems a. Electric Resistance  b. N/A  c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)  15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)  Cap: 40.5 kBtu/hr  Cap: 40.5 kBtu/hr  HSPF: 8.10  Cap: 50.0 gallons  EF: 0.90  PT,
Glass/Floor Area: U.12	points: 30122 PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: Mllma Mycan

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL:	
DATE:	

### **SUMMER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,

PERMIT #:

	BAS	E		AS-BUILT									
GLASS TYPE .18 X Condi Floor	tioned X	BSPM =	Points	Type/SC		erhang Len		Area X	SPI	мх	SOF	= Points	
.18 22	23.0	18.59	7439.0	1.Single, Clear	Е	1.0	6.0	30.0	47.9	92	0.97	1394.0	
				2.Single, Clear	E	1.0	15.0	30.0	47.9		1.00	1432.0	
				3.Single, Clear	E	1.0	4.0	9.0	47.9		0.91	393.0	
				4.Single, Clear	E	1.0	4.0	6.0	47.9		0.91	262.0	
				5.Single, Clear	E	1.0	4.0	6.0	47.9		0.91	262.0	
				6.Single, Clear	N	1.0	15.0	30.0	21.		1.00	649.0	
				7.Single, Clear	W	1.0	16.0	40.0	43.8		1.00	1748.0	
				8.Single, Clear	W	1.0	15.0	30.0	43.8		1.00	1311.0	
				9.Single, Clear	W	1.0	13.0	9.0	43.8		1.00	393.0	
				10.Single, Clear	w	1.0	5.5	15.0	43.8		0.96	632.0	
				11.Single, Clear	W	1.0	5.5	15.0	43.8	34	0.96	632.0	
			7/	12.Single, Clear	W	1.0	5.5	30.0	43.8	34	0.96	1265.0	
			ii el	13.Single, Clear	E	4.0	2.5	8.0	47.9	92	0.42	161.0	
				14.Single, Clear	E	4.0	7.0	5.0	47.9	92	0.68	162.0	
			ı.	As-Built Total:				263.0				10696.0	
WALL TYPES	S Area	X BSPM	= Points	Туре		R-	-Value	e Area	X	SP	M =	Points	
Adjacent	159.0	0.70	111.3	1. Concrete, Int Insul, Exteri	or		4.1	720.0		1.13		817.2	
Exterior	1712.0	1.70	2910.4	2. Frame, Wood, Exterior			13.0	992.0		1.50		1488.0	
				3. Frame, Steel, Adjacent			13.0	159.0		0.90		143.1	
Base Total:	1871.0	)	3021.7	As-Built Total:			nais:	1871.0		2011/10		2448.3	
DOOR TYPES	S Area	X BSPM	= Points	Туре	0 (0.4,0)		e =0	Area	X	SP	M =	Points	
Adjacent	17.0	2.40.	40.8	1.Exterior Insulated			-	20.0		4.10		82.0	
Exterior	20.0	6.10	122.0	2.Adjacent Wood				17.0		2.40		40.8	
Base Total:	37.0	)	162.8	As-Built Total:				37.0				122.8	
CEILING TYP	ES Area	X BSPM	= Points	Туре		R-Valu	ie ,	Area X S	SPM	ХS	CM =	Points	
Under Attic	1120.0	1.73	1937.6	1. Under Attic			19.0	1120.0	2.34 >	< 1.00		2620.8	
Base Total:	1120.0	)	1937.6	As-Built Total:		9		1120.0				2620.8	
FLOOR TYPE	S Area	X BSPM	= Points	Туре		R-	Value	e Area	X	SPI	M =	Points	
Slab Raised	136.0(p) 0.0	-37.0 0.00	-5032.0 0.0	1. Slab-On-Grade Edge Insu	ılation	,:	0.0	136.0(p		41.20		-5603.2	
Base Total:			-5032.0	As-Built Total:				136.0				-5603.2	

### **SUMMER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

	BASE		AS-BUILT							
INFILTRATION	Area X BSP	M = Points	Area X SPM = Points							
	2223.0 10.2	1 22696.8	2223.0 10.21 22696.8							
Summer Bas	e Points: 30	225.9	Summer As-Built Points: 32981.5							
Total Summer ) Points	C System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)							
30225.9	0.3250	9823.4	(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)         32982       1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 8732.2         32981.5       1.00 1.072 0.260 0.950 8732.2							

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

DAGE		10.71								
BASE	AS-BUILT									
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Ov Type/SC Ornt	verhang t Len Hg	t Area X	WPM X	( WOF	= Points				
.18 2223.0 20.17 8071.0	1.Single, Clear E	1.0 6.0	30.0	26.41	1.02	804.0				
	2.Single, Clear E			26.41	1.00	795.0				
	3.Single, Clear E			26.41	1.04	246.0				
	4.Single, Clear E	1.0 4.0	6.0	26.41	1.04	164.0				
	5.Single, Clear E	1.0 4.0	6.0	26.41	1.04	164.0				
	6.Single, Clear N	1.0 15.0	30.0	33.22	1.00	996.0				
	7.Single, Clear W	1.0 16.0		28.84	1.00	1154.0				
	8.Single, Clear W	1.0 15.0	30.0	28.84	1.00	866.0				
	9.Single, Clear W	1.0 13.0	9.0	28.84	1.00	259.0				
	10.Single, Clear W	1.0 5.5	15.0	28.84	1.01	436.0				
	11.Single, Clear W			28.84	1.01	436.0				
	12.Single, Clear W			28.84	1.01	873.0				
	13.Single, Clear E			26.41	1.41	297.0				
	14.Single, Clear E			26.41	1.15	151.0				
	As-Built Total:		263.0			7641.0				
WALL TYPES Area X BWPM = Points	Туре	R-Val	ue Area	X WP	M =	Points				
Adjacent 159.0 3.60 572.4	1. Concrete, Int Insul, Exterior	4.1	720.0	6.42	,	4622.4				
Exterior 1712.0 3.70 6334.4	2. Frame, Wood, Exterior	13.0		3.40		3372.8				
	3. Frame, Steel, Adjacent	13.0		4.90		779.1				
Base Total: 1871.0 6906.8	As-Built Total:		1871.0			8774.3				
DOOR TYPES Area X BWPM = Points	Туре	3.60 9	Area	X WP	M =	Points				
Adjacent 17.0 11.50 195.5	1.Exterior Insulated		20.0	8.40	)	168.0				
Exterior 20.0 12.30 246.0	2.Adjacent Wood		17.0	11.50		195.5				
Base Total: 37.0 441.5	As-Built Total:		37.0	11100	5.	363.5				
CEILING TYPES Area X BWPM = Points		R-Value		/DM Y \//	CM =	Points				
Under Attic 1120.0 2.05 2296.0	1. Under Attic	19.0		2.70 X 1.00	III JII O SANS	3024.0				
12		10.0		2.70 % 1.00	* 5					
Base Total: 1120.0 2296.0	As-Built Total:		1120.0		A	3024.0				
FLOOR TYPES Area X BWPM = Points	Туре	R-Val		X WPI	eri i mendida	Points				
Slab         136.0(p)         8.9         1210.4           Raised         0.0         0.00         0.0	Slab-On-Grade Edge Insulation	0.0	136.0(p	18.80	)	2556.8				
Base Total: 1210.4	As-Built Total:		136.0			2556.8				

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE	AS-BUILT							
INFILTRATION Area X BWPM = Poin	Area X WPM = Points							
2223.0 -0.59 -1311	.6 2223.0 -0.59 -1311.6							
Winter Base Points: 17614	1 Winter As-Built Points: 21048.0							
Total Winter X System = Heating Points Multiplier Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)							
17614.1 0.5540 9758	(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 21048.0 1.000 (1.060 x 1.169 x 0.88) 0.421 0.950 9215.8 21048.0 1.00 1.095 0.421 0.950 9215.8							

### **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

BASE					AS-BUILT								
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X	( Multiplier	X Cred		: Total
4		2635.00		10540.0	50.0	0.90	4		1.00	2693.56	1.00		10774.2
					As-Built To	otal:							10774.2

	CODE COMPLIANCE STATUS												
	BASE						AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
9823		9758		10540		30122	8732		9216		10774		28722

**PASS** 



### **Code Compliance Checklist**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot:,	Sub: ,	Plat:,,,	PERMIT #:

### **6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	<b>/</b>
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	1
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	/
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	V
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	./
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	1

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK			
Vater Heaters 612.1		Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.				
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.				
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.				
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	√ 			
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	/			
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	V			

### ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

### ESTIMATED ENERGY PERFORMANCE SCORE\* = 85.3

The higher the score, the more efficient the home.

ELECTRIC, Lot:, Sub:, Plat:,,,

1.	New construction or existing	New	·-	12.	Cooling systems		
2.	Single family or multi-family	Single family		a.	Central Unit	Cap: 40.5 kBtu/hr	
3.	Number of units, if multi-family	1				SEER: 13.00	
4.	Number of Bedrooms	4	-	b.	N/A	-	
5.	Is this a worst case?	Yes					_
6.	Conditioned floor area (ft²)	2223 ft <sup>2</sup>		C.	N/A		
7.	Glass type 1 and area: (Label reqd. by	y 13-104.4.5 if not default)			The same of the second to the same	A4 = A 0 S	
a.	U-factor:	Description Area		13.	Heating systems	_	
b.	(or Single or Double DEFAULT) 7 SHGC:		_		Electric Heat Pump	Cap: 40.5 kBtu/hr _ HSPF: 8.10 _	
	(or Clear or Tint DEFAULT)	7b. (Clear) 263.0 ft <sup>2</sup>	_	b.	N/A		
8.	Floor types					¥ ,	
a.	Slab-On-Grade Edge Insulation	R=0.0, 136.0(p) ft	Pa_ ==	c.	N/A		
b.	N/A		10.10				
c.	N/A			14.	Hot water systems	_	_
9.	Wall types			a.	Electric Resistance	Cap: 50.0 gallons	
a.	Concrete, Int Insul, Exterior	R=4.1, 720.0 ft <sup>2</sup>	P-0-1-0			EF: 0.90	
b.	Frame, Wood, Exterior	R=13.0, 992.0 ft <sup>2</sup>		b.	N/A		
C.	Frame, Steel, Adjacent	R=13.0, 159.0 ft <sup>2</sup>					
	N/A			c.	Conservation credits		
	N/A		_		(HR-Heat recovery, Solar		
10.	Ceiling types				DHP-Dedicated heat pump)		
a.	Under Attic	R=19.0, 1120.0 ft <sup>2</sup>		15.	HVAC credits	PT, _	
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A		-		HF-Whole house fan,		
11.	Ducts				PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Con. AH(Sealed):In	terior Sup. R=6.0, 150.0 ft			MZ-C-Multizone cooling,		
Ъ.	N/A				MZ-H-Multizone heating)		
	*				Committing that are account of the transfer of		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: MIMMA MMA

Address of New Home: 27 SW TIMBER RICHERDEZ

r Rider Drzip: Lake City, Fr 32055

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar<sup>TM</sup>designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

### **Duct System Summary**

### Entire House

### **MARONDA HOMES**

Job: SUNBURY Date:

Date:

By: ... G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

### **Project Information**

For:

SUNBURY

	He	eating			C	ooling
External static pressure		in H2O			0.60	in H2O
Pressure losses	0.00	in H2O			0.00	in H2O
Available static pressure	0.60	in H2O			0.60	in H2O
Supply / return available pressure	0.48/0.12	in H2O			0.48 / 0.12	in H2O
Lowest friction rate	1.935	in/100ft			1.935	in/100ft
Actual air flow	1240	cfm			1240	cfm
Total effective length (TEL)			31	ft		

Supply Branch Detail Table

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
KITCHEN	h	423 5	150	150	1.935	6	x00	VIFx	25.0	0.0	st1
FAMILY ROOM	h	424 1	140	140	1.935	6	$\mathbf{x}$	VIFx	25.0	0.0	ST2
DINING ROOM	h	3991	150	150	1.935	6	$\mathbf{x}$	VlFx	25.0	0.0	st1
LIVING ROOM	c	3329	140	140	9.677	6	$\mathbf{x}$	VlFx	5.0	0.0	ST3
UTILITY ROOM	h	829	25	25	9.677	4	$\mathbf{x}$	VlFx	5.0	0.0	ST3
BATHROOM	c	143 2	25	25	9.677	4	$\mathbf{x}$	VIFx	5.0	0.0	ST3
MASTER BEDROOM	c	3303	140	140	3.226	6	$\mathbf{x}$	VlFx	15.0	0.0	ST2
MASTER BATHROOM	c	2516	100	100	3.226	5	x00	VIFx	15.0	0.0	st5
HALL BATHROOM	c	1109	25	25	4.839	4	$\mathbf{x}$	VlFx	10.0	0.0	ST2
TOILET	c	1023	25	25	3.226	4	$\mathbf{x}$	VlFx	15.0	0.0	st5
BEDROOM #3	h	4511	110	110	4.839	5	x00	VlFx	10.0	0.0	st1
BEDROOM #2	c	242 2	100	100	4.839	5	x00	VIFx	10.0	0.0	ST4
BEDROOM #4	C	268 1	110	110	4.839	5	x@	VIFx	10.0	0.0	st1

### **Supply Trunk Detail Table**

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	520	520	1.935	515	12	0 x 0	VinlFlx	
ST2	Peak AVF	620	620	1.935	613	12	0 x 0	VinlFlx	
ST3	Peak AVF	190	190	9.677	569	7	0 x 0	VinlFlx	st5
ST4	Peak AVF	100	100	4.839	510	5	0 x 0	VinlFlx	
st5	Peak AVF	315	315	3.226	632	10	0 x 0	VinlFlx	st2

Bold/talic values have been manually overridden

### Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	:	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	1240	1240	6.0	1.935	568	18	х0	0		VlFx	



### **Project Summary Entire House MARONDA HOMES**

Job: SUNBURY

Date:

G. CARMACK By:

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

### **Project Information**

For:

SUNBURY

Notes:

### **Design Information**

Weather: Gainesville, FL, US

### Winter Design Conditions

### **Summer Design Conditions**

Outside db Inside db Design TD	33 70 37	Outside db Inside db Design TD	92 75 17	°F °F
		Daily range Relative humidity	M 50	%
		Moisture difference	52	gr/lb

### **Heating Summary**

### Sensible Cooling Equipment Load Sizing

Structure Ducts	39178 7466	Btuh	Structure Ducts	19600 9255	Btuh Btuh
Central vent (100 cfm)	4048		Central vent (100 cfm)	1860	Btuh
Humidification Piping	0	Btuh Btuh	Blower	0	Btuh
Equipment load	50692	Btuh	Use manufacturer's data	n	
Infiltration	n		Rate/swing multiplier Equipment sensible load	0.97 29794	Btuh

Method Construction quality		Average	Latent Cooling Equipme	nt Load	Sizin
Fireplaces		Average 0	Structure	2842	
			Ducts	2144	Btun
	Heating	Cooling	Central vent (100 cfm)	3516	Btuh
Area (ft²) Volume (ft³)	2223 17517	2223 17517	Equipment latent load	8502	Btuh
Air changes/hour	0.32	0.16	Equipment total load	38295	Btuh
Equiv. AVF (cfm)	93	47	Req. total capacity at 0.76 SHR	3.3	ton

### **Heating Equipment Summary**

### Cooling Equipment Summary

Trade HEAT PUMP Model N4H342AKA		Make TEMPSTAR Trade HEAT PUMP Cond N4H342AKA	8
Efficiency Heating input Heating output Temperature rise Actual air flow Air flow factor Static pressure Space thermostat	8.1 HSPF  0 Btuh @ 47°F 0 °F 1240 cfm 0.027 cfm/Btuh 0.60 in H2O	Coil FSU4X4200A Efficiency Sensible cooling Latent cooling Total cooling Actual air flow Air flow factor Static pressure Load sensible heat ratio	13 SEER 30780 Btuh 9720 Btuh 40500 Btuh <b>1240</b> cfm 0.043 cfm/Btuh 0.60 in H2O 0.78

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.





### **Building Analysis** *Entire House* **MARONDA HOMES**

Job: SUNBURY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

### **Project Information**

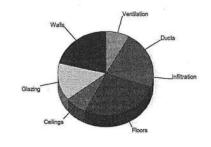
For:

SUNBURY

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 151 ft Latitude: 30°N  Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6  Simplified Average 0	Cooling 75 17 50 52.0

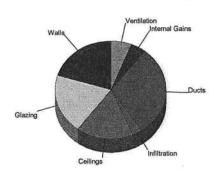
### Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.1	10723	21.2
Glazing Doors	47.0 0.0	7283 0	14.4
Ceilings Floors	1.3 28.1	3724 13667	7.3
Infiltration	1.4	3782	7.5
Ducts Piping	,	7466 0	14.7 0.0
Humidification Ventilation		4048	0.0 8.0
Adjustments <b>Total</b>		<b>50692</b>	100.0



### Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments Total	2.4 38.9 0.0 1.8 0.0 0.3	6218 6036 0 5097 0 869 9255 1860 1380 0 0	20.2 19.7 0.0 16.6 0.0 2.8 30.1 6.1 4.5 0.0



Overall U-value = 0.160 Btuh/ft2-°F

Data entries checked.

Inst. Number: 200712024085 Book: 1134 Page: 1699 Date: 10/26/2007 Time: 4:45:00 PM Page 2 of 2

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Signed, sealed and delivered in the presence of:

RML HOLDINGS, INC., a Florida corporation

Print Name: Linda Brown

Margaret Lardizabal
Vice President

Print Name: <u>Debbie G. Moore</u>

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 104h day of October, 2007, by Margaret Lardizabal, as Vice President of RML Holdings, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me.

Notary Public, State of Florida

Debbie G. Moore
Commission # DD400475
Expires March 16, 2009
Expires March 16, 2009
Expires March 16, 2009



# OCCUPANC

# **COLUMBIA COUNTY, FLORIDA**

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-108

Building permit No. 000027131

Use Classification SFD/UTILITY

Permit Holder THEODORE BROCK

Fire: 70.62

Waste: 184.25

Owner of Building MARONDA HOMES INC. OF FL

Date: 11/14/2008

Location:

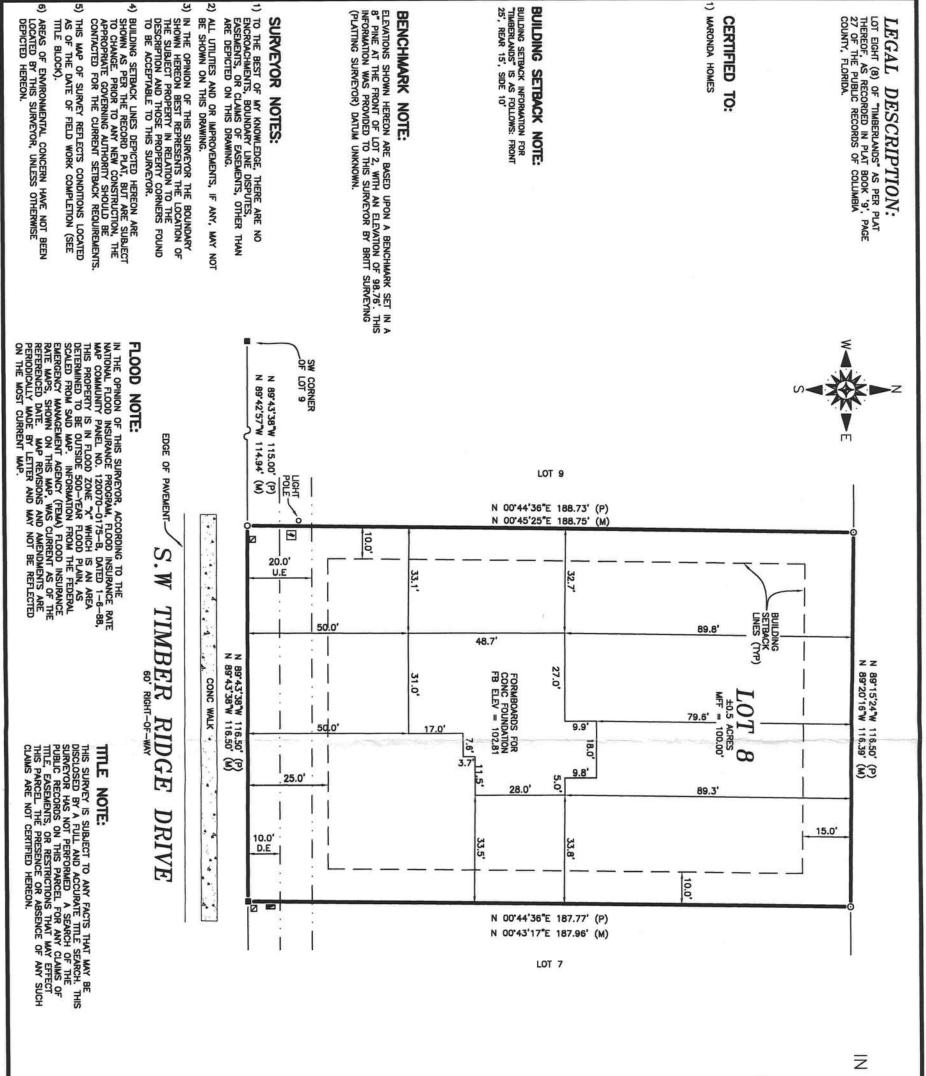
277 SW TIMBERRIDGE DRIVE, LAKE CITY, FL

254.87

Total:

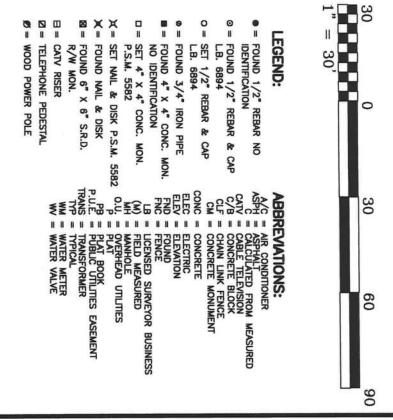
**Building Inspector** 

POST IN A CONSPICUOUS PLACE (Business Places Only)



### OUNDARY SURVEY

SECT W EAST, COLUMBIA COUNTY, FLORIDA TION 10, TOWNSHIP 4 SOUTH, RANGE



CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATLYES, AND CHAPTER, 61G17-6, FLORIDA ADMINISTRATIVE CODE.



### DATE: SCALE: FELD WORK COMPLETED ON 7/8/08 7/14/08 1" = 30' 4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609 PHONE: (352) 374-7707 BRINKMAN SURVEYING & MAPPING INC. "THE BENCHMARK IN QUALITY SERVICE" FIELDBOOK 96, FAX: (352) 374-8757 PAGE 66 CHECKED BY: J.B. DRAWN BY: ZL DRAWING NUMBER 097-08

PREPA

RED FOR: MARONDA

LOT EIGHT (8) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. LEGALDESCRIPTION:

CERTIFIED TO:

1) MARONDA HOMES

### BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25", REAR 15", SIDE 10"

### SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

20.0' U.E

25.0

10.0' D.E

2)

3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

4

- BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS. THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE
- AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

6)

5



### PROPOSED BUILDING LAYOUT

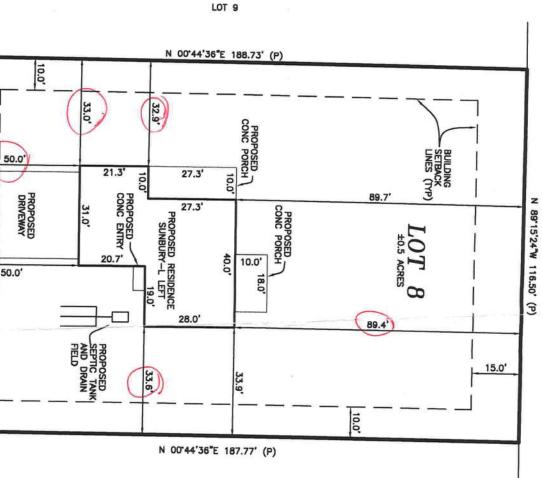
IN SECTION 10, 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

30

0

60

90



# TIMBER RIDGE

DRIVE

89'43'38'W 116.50' (P)

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED

### THE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH

LEGEND:

- . FOUND 1/2" REBAR NO DENTIFICATION FOUND 1/2" REBAR & CAP
- 0 0 LB. 6894 SET 1/2" REBAR & CAP LB. 6894
- \_ 9 11
- XX
- **S** (1) ⊠ II SET VAIL & DISK P.S.M. 5582

  SET VAIL & DISK P.S.M. 5582

  FOUND NAIL & DISK P.S.M. 5582

  FOUND NAIL & DISK P.S.M. 5782

  FOUND NAIL & WW = TELEY B.S.M. 5582

  TELEY B.S.M. 5582

  TELEY B.S.M. 5582

  TRANS B.S.M. 5582

WOOD POWER POLE

ELEPHONE PEDESTAL

LOT 7

- ABBREVIATIONS:

  A/C = AIR CONDITIONER

  A/C = AIR CONDITIONER

  ASPH = ASPHALT

  C = CALCULATED FROM MEASURED

  CATV = CABLE TELEVISION

  C/B = CONCRETE BLOCK

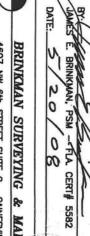
  CLF = CHAIN LINK FENCE

  CM = CONCRETE MONUMENT
- = ELECTRIC = ELEVATION = FOUND = FENCE = CONCRETE
- B = LICENSED SURVEYOR BUSINESS
  W) = FIELD MEASURED
  (H = MANHOLE
  U. = OVERHEAD UTILITIES
  P = PLAT
- = PLAT BOOK = PUBLIC UTILITIES EASEMENT = TRANSFORMER = TYPICAL WATER METER WATER VALVE

### CERTIFICATE OF SURVEYOR: 'HIS IS NOT A BOUNDARY SURVEY

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER-61(3)7—6, FLORIDA ADMINISTRATIVE CODE.



# BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

DATE: 5/19/08 1" = 30' PHONE: (352) 374-7707 "THE BENCHMARK IN QUALITY SERVICE" FAX: (352) 374-8757 DRAWN BY: ZL

SCALE:

FIELD WORK COMPLETED ON \*\*\*\* FIELDBOOK \*\*, PAGE \*\*

PREPARED FOR: MARONDA

DRAWING NUMBER 097-08

CHECKED BY: J.B.

TOTAL FEES CHARGED 3, 063.67 CHECK NUMBER	SCHOOL IMPACT FEE 500.00	CORRECTIONS IMPACT FEE 4409. 16	FIRE PROTECTION IMPACT FEE 78.63	EMS IMPACT FEE 27, 88	ROAD IMPACT FEE 1,046.00 CODE 211 UNIT	FEES:
					VIT /	