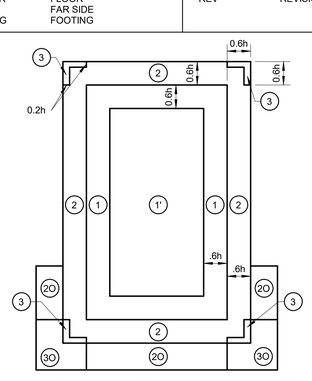
STRUCTURAL ABBREVIATIONS

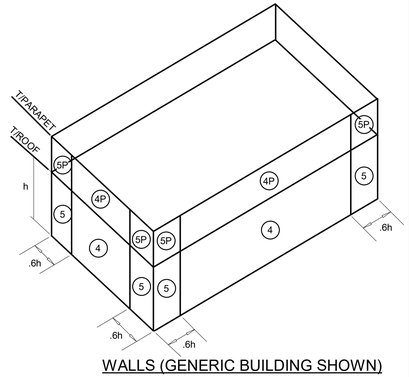
#	NUMBER OR POUND
Ø	ROUND OR DIAMETER
□	SQUARE
&	AND
@	AT
ALUM	ALUMINUM
AR	ANCHOR ROD
ARCH	ARCHITECTURE
ASSY	ASSEMBLY
B/	BOTTOM OF
BETWN	BETWEEN
BLDG	BUILDING
BM	BEAM
BOT	BOTTOM
BPL	BASE PLATE
BRG	BEARING
C TO C	CENTER TO CENTER
CANT	CANTILEVER
CJ	CONSTRUCTION JOINT
CL	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTION
CTR	CENTER
CTRD	CENTERED
DBA	DEFORMED BAR ANCHOR
DEFL	DEFLECTION
DET	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIFF	DIFFERENT
DIM	DIMENSION
DO	DITTO
DWG	DRAWING
EA	EACH
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
ENGR	ENGINEER
EOR	ENGINEER OF RECORD
EQ	EQUAL
EQUIP	EQUIPMENT
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
EW	EACH WAY
FB	FLAT BAR
FD	FLOOR DRAIN
FDN	FOUNDATION
FF	FINISH FLOOR
FLR	FLOOR
FS	FAR SIDE
FTG	FOOTING
GA	GAGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GT	GIRDER TRUSS
HGT	HEIGHT
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HT	HIP TRUSS
IF	INSIDE FACE
INFO	INFORMATION
INT	INTERIOR
JNT	JOINT
JST	JOIST
LB	POUND
LG	LONG
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSH	LONG SIDE HORIZONTAL
LSV	LONG SIDE VERTICAL
MANUF	MANUFACTURER
MAT'L	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NO	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
O/O	OUT TO OUT
OPNG	OPENING
OPP	OPPOSITE
PEMB	PRE-ENGINEERED METAL BUILDING
PERP	PERPENDICULAR
PL	PLATE
PNL	PANEL
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST TENSIONED
QTY	QUANTITY
R OR RAD	RADIUS
RD	ROUND
REF	REFERENCE
REINF	REINFORCE(MENT)
REQ	REQUIRE
REQ'D	REQUIRED
RET	RETAINING
REV	REVISION
SCHED	SCHEDULE
SECT	SECTION
SIM	SIMILAR
SOG	SLAB ON GRADE
SPA	SPACE
SPEC	SPECIFICATION
SQ	SQUARE
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STR	STRAIGHT
STRUCT	STRUCTURAL
SYM	SYMMETRICAL
T&B	TOP & BOTTOM
T/	TOP OF
T/U PANEL	TILT-UP PANEL
TE	THICKENED EDGE
THK	THICK
THRD	THREADED
TRANSV	TRANSVERSE
TS	THICKENED SLAB
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
VER	VERIFY
VERT	VERTICAL
w/	WITH
w/O	WITHOUT
WP	WORK POINT
WS	WATERSTOP
WWF	WELDED WIRE FABRIC

GENERAL SYMBOLS

	PLAN, SECTION OR DETAIL NO SHEET NUMBER
N	NORTH ARROW
1	KEYED NOTE TO PLAN
1	FOUNDATION TYPE
1	REVISION NUMBER
$	FOOTING STEP



ROOF PLAN (GENERIC BUILDING SHOWN)



WALLS (GENERIC BUILDING SHOWN)

COMPONENT & CLADDING DESIGN WIND PRESSURES (PSF)

ROOF SURFACE PRESSURES

AREA	10 SF	20 SF	50 SF	100 SF	200 SF	350 SF	500 SF	1000 SF
NEGATIVE ZONE 1	-51.5	-48.1	-43.6	-40.2	-36.8	-34.1	-32.3	-32.3
NEGATIVE ZONE 1'	-29.6	-29.6	-29.6	-29.6	-25.5	-22.1	-20.0	-16.0
NEGATIVE ZONE 2	-68.0	-63.6	-57.8	-53.5	-49.1	-45.6	-43.3	-43.3
NEGATIVE ZONE 3	-68.0	-63.6	-57.8	-53.5	-49.1	-45.6	-43.3	-43.3
POSITIVE ZONE 1 & 1'	16.0	16.0	16.0	16.0	16.0	16.0	16.0	16.0
POSITIVE ZONES 2 & 3	29.6	28.3	26.6	25.2	23.9	22.9	22.2	20.9
OVERHANG ZONE 1 & 1'	-46.6	-45.8	-44.7	-43.9	-36.8	-31.1	-27.4	-27.4
OVERHANG ZONE 2	-63.0	-57.2	-49.5	-43.7	-37.9	-33.1	-30.2	-30.2
OVERHANG ZONE 3	-63.0	-57.2	-49.5	-43.7	-37.9	-33.1	-30.2	-30.2

PARAPET SURFACE PRESSURES

SOLID PARAPET PRESSURE		10 SF	20 SF	50 SF	100 SF	200 SF	500 SF
CASE A	ZONE 2:	90.1	84.3	76.5	70.7	64.9	57.2
	ZONE 3:	90.1	84.3	76.5	70.7	64.9	57.2
CASE B	INTERIOR ZONE:	-53.2	-50.5	-47.0	-44.3	-41.6	-38.0
	CORNER ZONE:	-60.8	-56.8	-51.4	-47.4	-43.4	-38.0

WALL SURFACE PRESSURES

AREA	10 SF	100 SF	200 SF	500 SF
NEGATIVE ZONE 4	-32.1	-27.7	-26.4	-24.7
NEGATIVE ZONE 5	-39.5	-30.8	-28.1	-24.7
POSITIVE ZONE 4 & 5	29.6	25.2	23.9	22.2

NOTES:
1) TABLE PRESSURES ARE FOR THE SQUARE FOOT (SF) TRIBUTARY AREA SHOWN. FOR OTHER TRIBUTARY AREAS, LINEARLY INTERPOLATE BETWEEN VALUES SHOWN ABOVE.
2) POSITIVE PRESSURES ACT TOWARD THE BUILDING. NEGATIVE PRESSURES ACT AWAY FROM THE BUILDING.
3) SEE DIAGRAMS FOR LOCATION OF ZONES.
4) PRESSURES SHOWN ARE ULTIMATE PRESSURES, MULTIPLY VALUES BY 0.6 FOR NOMINAL PRESSURES.

a=8.80 FT

2 / S0.1 WIND PRESSURE DIAGRAMS

SCALE: NTS

DESIGN CRITERIA

DESIGN PER 2020 FLORIDA BUILDING CODE, UNLESS OTHERWISE NOTED.

LIVE LOADS:
ROOFS AND CANOPIES (REDUCIBLE) . . . . . 20 PSF

DEAD LOADS (SUPER IMPOSED):
ROOF . . . . . 15 PSF

WIND LOADS:
ULTIMATE WIND SPEED: (ASCE 7-16) . . . . . 117 MPH
NOMINAL WIND SPEED . . . . . 91 MPH
MEAN ROOF . . . . . 28 FT
HEIGHT RISK CATEGORY II
WIND EXPOSURE C
ENCLOSURE CLASSIFICATION . . . . . ENCLOSED
INTERNAL PRESSURE COEFFICIENT . . . . . ± 0.18
DIRECTIONALITY FACTOR (Kd) 0.85
SHAPE FACTORS . . . . . PER CODE

RAIN LOADS:
RAIN LOAD . . . . . 30 PSF
RAIN INTENSITY . . . . . 4.25 IN/HR

CONCRETE (DESIGN PER CURRENT EDITION ACI 318):
SLAB ON GRADE . . . . . F'C= 4000 PSI
FOOTINGS . . . . . F'C= 3000 PSI
ALL OTHER CONCRETE . . . . . F'C= 3000 PSI

ALL REINFORCING STEEL ASTM A615 GRADE 60. REINFORCING STEEL SHOWN ON THESE DRAWINGS TO BE WELDED SHALL BE ASTM A706 AND WELDING SHALL BE IN ACCORDANCE WITH AWS D1.4.

WELDED WIRE FABRIC . . . . . ASTM A1064

CONCRETE MASONRY (DESIGN PER CURRENT EDITION ACI 530)
COMPRESSIVE STRENGTH . . . . . F'M= 1900 PSI

STRUCTURAL STEEL (DESIGN PER CURRENT EDITION AISC), UNLESS OTHERWISE NOTED (UON) MATERIALS SHALL BE AS FOLLOWS:
W-SHAPES . . . . . ASTM 992, Fy=50 KSI
OTHER SHAPES & PLATES . . . . . ASTM A36, Fy=36 KSI
HSS SQUARE & RECTANGULAR SHAPES . . . . . ASTM A500 GRADE B, Fy=46 KSI
HSS ROUND SHAPES . . . . . ASTM A500 GRADE B, Fy=42 KSI
STEEL PIPES . . . . . ASTM A53 GRADE B, Fy=35 KSI
WELDING ELECTRODES . . . . . AWS A5.1 OR A5.5 SERIES E70
HIGH-STRENGTH BOLTS . . . . . 3/4Ø ASTM A325
ANCHOR RODS . . . . . GRADE 36 ASTM F1554
WELDED STUDS . . . . . ASTM A108
DEFORMED BARS . . . . . ASTM A496
WELDABLE BARS . . . . . ASTM A706
PAINT & PROTECTION . . . . . SSPC PAINT 25
GROUT - NONMETALLIC, SHRINKAGE-RESISTANT . . . . . ASTM C1107

OPEN WEB STEEL JOIST (DESIGN PER CURRENT EDITION SJI WITH AN ALLOWABLE TENSILE STRESS OF 30,000 PSI)

SOIL BEARING (ASSUMED MAXIMUM). . . . . 2000 PSF

GENERAL NOTES

FOUNDATION

IF FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED, UNSTABLE, OR UNSUITABLE SOIL, THE ENGINEER SHALL BE NOTIFIED.

THE BOTTOM OF ALL FOUNDATIONS SHALL EXTEND A MINIMUM OF 18 INCHES BELOW THE TOP OF FINISH GRADE.

STEPS IN WALL FOOTINGS SHALL NOT EXCEED A SLOPE OF ONE (1) VERTICAL TO TWO (2) HORIZONTAL.

CONCRETE

UNLESS OTHERWISE NOTED (UON) ON THE DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:
FOOTINGS . . . . . 3"
SLABS ON GRADE . . . . . 2" FROM TOP

ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 315 DURING THE PLACING OF THE CONCRETE.

UNLESS OTHERWISE NOTED, SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS FOLLOWS:
WELDED WIRE FABRIC . . . . . WIRE SPACING PLUS 6"
REINFORCING BARS . . . . . 60 BAR DIAMETERS

ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE NOTED.

DOWELS FROM FOUNDATIONS OR SLABS TO WALLS SHALL MATCH WALL REINFORCING, UNLESS OTHERWISE NOTED. DOWELS SHALL BE PLACED BEFORE CONCRETE IS POURED. THEY SHALL NOT BE PUSHED INTO THE CONCRETE.

MASONRY

ALL LOAD BEARING WALLS AND EXTERIOR WALLS SHALL BE COMPOSED OF ASTM C90 HOLLOW CONCRETE MASONRY UNITS WITH ASTM C270 TYPE "S" MORTAR. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 AND HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

ALL EXTERIOR CMU WALLS SHALL BE REINFORCED FULL HEIGHT IN A GROUT FILLED CELL WITH 1-#5 AT:
- EA CORNER, WALL ENDS, WALL INTERSECTIONS
- EA SIDE OF CONTROL JOINTS AND
- AT A MAXIMUM SPACING OF 4'-0" OC, UON
- SEE DETAIL 1/S3.1 FOR TYPICAL REINFORCING AT WALL OPENINGS.
- AT BEAM & JOIST GIRDER BEARING LOCATIONS ADD REINFORCING AS SHOWN IN PLAN.

LAPPED BARS SHALL BE SECURED WITH WIRE TIES OR OTHER MEANS TO ENSURE THAT THE BAR IS NOT DISPLACED DURING GROUT PLACEMENT OUTSIDE THE TOLERANCES ESTABLISHED BY ACI 530. LAP BARS WITH THE FOLLOWING MINIMUM LENGTH.

BAR SIZE	BAR EA FACE	BARS CTR'D 8" CMU	BARS CTR'D 12" CMU
#3	16"	16"	16"
#4	26"	21"	21"
#5	40"	26"	26"
#6	54"	43"	40"
#7	63"	60"	46"

GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS. DO NOT BEGIN PLACEMENT OF GROUT UNTIL ALIGNMENT OF CELLS ARE INSPECTED AND APPROVED.

FILL ALL CELLS BELOW FINISHED GRADE.

PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UON. IN ADDITION, INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE & BELOW OPENINGS, EXTENDING AT LEAST 24 INCHES BEYOND THE OPENING. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN PARAPETS AND FREE STANDING WALLS & 8" OC VERTICALLY. LAP JOINT REINFORCEMENT 6" MINIMUM. HORIZONTAL REINFORCING SHALL CONSIST OF AT LEAST TWO W1.7 WIRES OR GREATER.

SEE ARCHITECTURAL DRAWING FOR EXPANSION OR CONTROL JOINTS. IF NOT SHOWN, LOCATE VERTICAL CONTROL JOINTS AT 25'-0" OC MAXIMUM, BUT NOT LESS THAN 2'-0" FROM A JOIST OR BEAM BEARING PLATE. AT BUILDING CORNERS, PROVIDE ONE JOINT IN ONE OF THE TWO WALL SIDES NO MORE THAN 5'-0" FROM THE BUILDING CORNER. HORIZONTAL REINFORCING SHALL CONSIST OF W1.7 JOINT REINFORCEMENT OR GREATER.

ALL PRECAST OR POURED LINTELS SHALL BE REINFORCED WITH TWO #4 TOP & BOTTOM WITH #3 TIES @ 12" AS A MINIMUM AND HAVE A MINIMUM MASONRY END BEARING OF 8".

BOND/TIE BEAM REINFORCEMENT SHALL BE CONTINUOUS ACROSS CONTROL JOINTS.

16" U-BLOCK OR BOND BEAM SHALL CONSIST OF TWO 8" KNOCK-OUT BLOCKS.

BARS SPECIFIED TO BE EA FACE SHALL BE HELD IN PLACE WITH SPACERS AND SHALL BE LOCATED 2 3/8" FROM EA FACE TO THE CENTER OF THE BAR.

MASONRY WORK SHALL BE INSPECTED IN ACCORDANCE WITH TMS 402 QUALITY ASSURANCE LEVEL 2.

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

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CODY FRAZIER LICENSE No. 85369 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been electronically signed and sealed by Cody Frazier, PE on the date adjacent to the seal using a digital signature.

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FASTENERS

EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3, SIMPSON STRONG-TIE STRONG-BOLT2, DEWALT POWER-STUD+ SD1 OR APPROVED EQUAL, UON. EMBEDMENT DEPTH INTO CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 7 TIMES THE BOLT DIAMETER, UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

SCREW ANCHORS SHALL BE HILTI KWIK HUS-EZ, SIMPSON STRONG-TIE TITEN HD, DEWALT SCREW-BOLT+ OR APPROVED EQUAL, UON. EMBEDMENT IN CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 9 TIMES THE BOLT DIAMETER, UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

POWER ACTUATED FASTENERS (PAF) SHALL BE 0.157" DIAMETER HILTI X-U, SIMPSON STRONG-TIE PDPA, DEWALT CSI PIN OR EQUAL, UON. EMBED MIN 1-1/4" INTO CONCRETE AND CMU, UON. DO NOT PLACE WITHIN 1" OF CMU MORTAR JOINT. PAF SHALL COMPLETELY PENETRATE STRUCTURAL STEEL.

ADHESIVE ANCHORING (EPOXY):

- ADHESIVE ANCHORING FOR CONCRETE SHALL BE HILTI RE-500 V3 CARTRIDGE SYSTEM, SIMPSON STRONG-TIE SET-3G, DEWALT PURE 110+ (OR EQUIVLANT ACRYLIC AC200+, HY 200, OR ATXP) OR APPROVED EQUAL, UON. EMBEDMENT DEPTH SHALL BE AT LEAST 12 TIMES THE INSERT DIAMETER, UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. THE HOLE SHALL BE CLEANED PER MANUFACTURER'S RECOMMENDATIONS BY BRUSHING OUT WITH WIRE BOTTLE BRUSH AND BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP (EXCEPT WHERE PERMITTED WHEN USING A DUST EXTRACTION SYSTEM IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS).

- ADHESIVE ANCHORING FOR MASONRY SHALL BE HILTI HIT-HY 70 OR HY 270 CARTRIDGE SYSTEM, SIMPSON STRONG-TIE SET-XP, DEWALT AC100+ OR APPROVED EQUAL, UON. EMBEDMENT DEPTH INTO SOLID GROUTED MASONRY SHALL BE AT LEAST 9 TIMES THE INSERT DIAMETER, UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. HOLES SHALL NOT BE PLACED WITHIN 1" OF A VERTICAL MORTAR JOINT. CLEAN HOLE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS.

- GENERAL - ANCHORS SHALL MEET THE REQUIREMENTS OF ACI 355.4. INSTALLATION SHALL BE IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS AND PERFORMED BY AN INSTALLER TRAINED BY THE MANUFACTURER. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY, WHICH SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER IN ACCORDANCE WITH ACI318 AND CONTINUOUSLY INSPECTED PER ACI318. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

- CAPACITIES - UON, DESIGN BOND STRENGTH OF ANCHORS HAVE BEEN BASED ON CRACKED CONCRETE, ACI 355.4 TEMPERATURE CATEGORY B, AND INSTALLATIONS INTO DRY HOLES DRILLED WITH A ROTARY IMPACT DRILL OR ROCK DRILL INTO CONCRETE THAT HAS CURED AT LEAST 21 DAYS AND HAS A CONCRETE TEMPERATURE OF AT LEAST 50 DEGREES F AT TIME OF ANCHOR INSTALLATION.

WOOD

CLIPS, CONNECTIONS, HANGERS, HOLD-DOWNS, ETC. SHOWN ON THESE DRAWINGS ARE SIMPSON STRONG-TIE CONNECTORS, UON. FASTENERS OF OTHER MANUFACTURERS MAY BE SUBSTITUTED PROVIDED THE LOAD VALUES OF THE SUBSTITUTED FASTENER FOR GROUP II WOOD SPECIES EQUALS OR EXCEEDS THE SPECIFIED FASTENER.

ALL NAILS SHALL BE COMMON WIRE NAILS UNLESS SHOWN OTHERWISE OR MANUFACTURE'S CONNECTOR LITERATURE SPECIFIES OTHERWISE

NAILING OF ALL MEMBERS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. SEE CODE FOR TABLE.

ALL LUMBER USED IN EXTERIOR APPLICATIONS, INCLUDING: BALCONY DECK BOARDS, LEDGER, JOISTS, BEAMS, WOOD IN CONTACT WITH EXTERIOR MASONRY OR CONCRETE SLABS OR WALLS, AND SILL PLATES EXPOSED TO CONCRETE SHALL BE TREATED IN ACCORDANCE WITH AWPA U1. USE CATEGORY 2 FOR SILL PLATES, CATEGORY 3B FOR EXTERIOR MEMBERS, AND CATEGORY 4A FOR WOOD IN GROUND CONTACT. SEE AWPA U1 FOR ALL OTHER CASES.

SILL PLATE BOLT AND ANCHOR BOLT WASHERS SHALL BE 1/8"x2"x2" AT BEARING LOCATIONS WITH UPLIFT.

ROOF SHEATHING SHALL BE 19/32" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 32/16 SPAN RATING. HOWEVER, 7/16" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 24/16 SPAN RATING MAY BE USED FOR ASPHALT SHINGLED OR STANDING SEAM METAL ROOFS.

ROOF DECKING SHALL BE NAILED WITH 8D NAILS AT 7/16"& 1/2" DECK AND 10d NAILS AT 5/8" & 3/4" DECK. SPACE NAILS AT 6" AT SUPPORTED EDGES OF DECK (4" AT EXTERIOR WALLS) AND 12" SPACING AT INTERMEDIATE SUPPORTS. AT GABLE ENDS, NAIL ROOF DECK AT 4" AT PANEL EDGES AND AT 6" AT INTERMEDIATE SUPPORTS FOR A DISTANCE OF ?'-?" FROM THE END WALL.

PROVIDE 2x4 BLOCKING FOR SUPPORT OF ROOF SHEATHING AT HIPS AND VALLEYS.

WALL SHEATHING SHALL BE 7/16" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 24/16 SPAN RATING. SHEATHING MAY BE ORIENTED VERTICALLY OR HORIZONTALLY FOR FLEXIBLE WALL FINISHES. SHEATHING MUST BE ORIENTED HORIZONTALLY FOR BRITTLE WALL FINISHES (STUCCO) UNLESS STRUCTURAL 1 RATED SHEATHING OR 15/32" 5-PLY/5-LAYER PLYWOOD OR 15/32" OSB IS USED.

HOLES AND NOTCHES MUST BE APPROVED BY THE ENGINEER. IF APPROVED THE NOTCHES ON THE ENDS OF JOISTS SHALL NOT EXCEED ONE-FOURTH THE DEPTH. HOLES BORED FOR PIPE OR CABLE SHALL NOT BE WITHIN THE TOP OR BOTTOM THIRD OF THE JOIST DEPTH AND THE DIAMETER OF SUCH HOLE SHALL NOT EXCEED ONE-THIRD THE JOIST DEPTH NOTCHES FOR PIPES IN THE TOP OR BOTTOM OF JOISTS SHALL NOT EXCEED ONE-SIXTH THE JOIST DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE ONE-THIRD OF THE SPAN.

STRUCTURAL GLUED LAMINATED TIMBER DESIGN PER CURRENT NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION "NDS" BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, STANDARD SPECIFICATION FOR STRUCTURAL GLUED LAMINATED TIMBER OF SOFTWOOD, AND AITC 119 BY THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC). ALL MEMBERS MUST BE MANUFACTURED IN ACCORDANCE WITH THE CURRENT EDITION OF THE U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD PS.56 "STRUCTURAL GLUED LAMINATED TIMBER".

LVL SHALL BE MICROLLAM 2.0E BY "TRUS JOIST" OR EQUAL, PSL SHALL BE PARALLAM 2.0E BY "TRUS JOIST" OR EQUAL; FOR COLUMNS – PARALLAM 1.8E BY "TRUS JOIST" OR EQUAL, AND LSL SHALL BE TIMBERSTRAND 1.5E BY "TRUS JOIST" OR EQUAL. PSL BEAMS GREATER THAN 18" DEEP SHALL BE PARALLAM 2.2E BY "TRUS JOIST" OR EQUAL.

STRESS GRADE: SOUTHERN PINE NO. 2 OR ENGINEER APPROVED EQUAL. ALL DESIGN VALUES ARE UNDER NORMAL LOADING AND IN DRY CONDITIONS OF SERVICE.

PRESSURE-TREAT LUMBER IN ACCORDANCE WITH THE MANUAL OF RECOMMENDED PRACTICE OF THE AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA).

ALL FASTENERS AND NAILS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE MADE OF TYPE 304 OR TYPE 316 STAINLESS STEEL OR ASTM A653 TYPE G185 ZINC COATED STEEL UNLESS THE LUMBER IS TREATED WITH CCA, MCA, MCQ, uCA OR SBX (DOT). [BUT NOT SBX (DOT) WITH SODIUM SILICATE (NaSiO2)]. EXCEPT AT SWIMMING POOLS AND WITHIN 5 MILES OF SALT WATER STAINLESS STEEL MUST BE USED IF IN CONTACT WITH COPPER BASED PRESERVATIVES.

UNLESS OTHERWISE NOTED, USE THE FOLLOWING MINIMUM GRADE OF LUMBER FOR FRAMING.

FRAMING	MINIMUM GRADE
SILL ON FOUNDATION WALLS OR SLAB ON GRADE	NO. 3 SYP
JOISTS, RAFTERS & HEADERS	NO. 2 SYP
PLATES, CAPS & BUCKS	NO. 2 SYP
STUDS	SEE SCHEDULE
POSTS & COLUMNS (INCLUDE ALL VERTICAL MEMBERS SPECIFICALLY CALLED OUT, I.E. 3 - 2x4)	NO. 2 SYP

WOOD CONT

CONNECT OVER FRAMING (SUCH AS VALLEY TRUSSES) TO MAIN ROOF FRAMING BELOW WITH SIMPSON VTCR WITH 4-10d NAILS INTO TRUSS AND 5-10d x 1 1/2" NAILS INTO OVERFRAMING OR 1 1/4"x16 ga TWIST STRAP @ 48" MAX w/ 4-10d NAILS EA END OF STRAP.

POST BASE AND CAPS FOR 4x4 AND 6x6 POST SHALL BE SIMPSON CB OR CBQ SERIES AT BASE AND CC OR CCQ SERIES AT CAP.

JOIST HANGERS SHALL BE SIMPSON SERIES LUS, UON.

CONVENTIONAL FASTENING AND STRAPPING HAVE BEEN SHOWN ON THESE DRAWINGS TO RESIST WIND LOADING. AN ALTERNATE SYSTEM USING FULL HEIGHT BOLTED RODS, CABLES, ETC. (SUCH AS QUICK-TIE). MAY BE SUBMITTED AS AN ALTERNATE. SUBMIT DETAILED SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A REGISTERED ENGINEER FOR APPROVAL PRIOR TO FABRICATION. THE SUBMITTAL SHALL ADDRESS THE FOLLOWING:

- THE TOP PLATE MUST BE CONSIDERED AS TWO SEPARATE MEMBERS FOR TRANSFERRING UPLIFT FORCES TO THE ALTERNATE SYSTEM, UNLESS IT IS NAILED TOGETHER TO ACT AS ONE MEMBER (I.E. vQ/It CALCULATION REQUIRED).
- WOOD WALL SHEATHING MAY BE USED TO TRANSFER THE UPLIFT FORCES PROVIDED ADDITIONAL NAILING IS PROVIDED AT THE TOP PLATE (SEE "SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC" BY AWC FOR ACCEPTABLE TABLES AND DETAILING). NAIL SPACING AT SHEARWALLS WILL NEED TO BE INCREASED.
- GYPSUM BOARD WALL SHEATHING (EXCEPT AT GYPBOARD SHEARWALLS) MAY BE USED TO TRANSFER UPLIFT FORCES PROVIDED SPECIAL NAILING IS PROVIDED. IF WALL SHEATHING IS USED TO TRANSFER UPLIFT THE HURRICANE CLIPS SHALL BE INSTALLED UNDER THE SHEATHING AND ON THE SAME SIDE OF THE WALL.
- SHEARWALL SILL ANCHOR BOLT SPACING MUST BE PER THE SHEARWALL SCHEDULE. HOWEVER, EACH BOLT FOR THE ALTERNATE SYSTEM MAY REPLACE ONE SCHEDULED ANCHOR BOLT.
- SHEARWALL HOLDOWNS AND FLOOR TO FLOOR STRAPPING AT END POSTS MUST BE PROVIDED AS SCHEDULED UNLESS AN ICC REPORT OR OTHER TESTING IS PROVIDED SHOWING THAT LATERAL SHEARWALL DEFLECTION IS WITHIN ACCEPTABLE LIMITS.
- CABLE SYSTEMS MUST BE PRE-TENSIONED TO RESIST UPLIFT LOADING. SUBMIT TENSIONING PROCEDURE FOR APPROVAL.
- TRUSSES AND CONVENTIONAL FRAMING WILL BE FASTENED WITH CONVENTIONAL FASTENERS AS SHOWN ON THESE DRAWINGS, UNLESS AN ALTERNATE PROCEDURE IS SUBMITTED FOR APPROVAL.
- RODS OR CABLES SHALL BE TIED OFF @ EA FLOOR. IF NOT, END POSTS SPECIFIED AT THE GROUND FLOOR SHALL BE USED FULL HEIGHT.
- FOR BUILDINGS OVER A SINGLE STORY, TAKE UP DEVICES SHALL BE USED WITH ROD SYSTEMS TO ACCOMMODATE WOOD SHRINKAGE AND CABLE SYSTEMS SHALL BE DESIGNED AND DETAILED TO ACCOMMODATE WOOD SHRINKAGE.

WOOD TRUSSES

TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS INDICATING ACTUAL TRUSS LAYOUT, DESIGN, WIND UPLIFT AT BEARING LOCATIONS, NUMBER AND TYPES OF TRUSSES, ETC. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. TRUSS MANUFACTURER SHALL COORDINATE AND VERIFY ALL TRUSS DIMENSIONS AND DESIGNS WITH ARCHITECT'S DRAWINGS.

ROOF FRAMING PLAN AND TRUSS TYPES ARE DIAGRAMMATIC AND ARE INTENDED TO INDICATE DESIGN CONCEPT ONLY FOR ROOF CONFIGURATION.

TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH ANSI/TPI1 "NATIONAL DESIGN STANDARDS FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".

ROOF TRUSS DESIGN CRITERIA:

LIVE LOAD	SEE DESIGN CRITERIA THIS SHEET
DEAD LOAD - ASPHALT SHINGLES	10 PSF TOP CHORD 10 PSF BOT CHORD
TILE ROOF	20 PSF TOP CHORD 10 PSF BOT CHORD
MIN DEAD LOAD (FOR UPLIFT) - ASPHALT SHINGLES	8 PSF
TILE ROOF	15 PSF
WIND UPLIFT	PER CODE

BRACE BOTTOM CHORD AS REQUIRED FOR WIND UPLIFT.

COORDINATE TRUSS LOCATIONS/CONFIGURATION WITH PLUMBING WALLS AND HVAC EQUIPMENT SO AS TO AVOID CONFLICTS. SEE MECHANICAL DRAWINGS FOR EXACT LOCATIONS OF EQUIPMENT, DUCTS, STACKS, PIPES, ETC. GENERAL CONTRACTOR SHALL ENSURE TRUSS CONFIGURATION ACCOMMODATES ALL EQUIPMENT, DUCTS, ETC.

TEMPORARY TRUSS BRACING SHALL BE INSTALLED IN ACCORDANCE WITH "RECOMMENDED DESIGN SPECIFICATIONS FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" (TPI-DSB) AND "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (TPI-HIB). INSTALL ALL WEB BRACING REQUIRED BY THE TRUSS DESIGNER. TEMPORARY BOTTOM CHORD AND WEB BRACING SHALL REMAIN PERMANENTLY IN PLACE. THE BOTTOM CHORD BRACING SHALL NOT EXCEED 10' SPACING FOR TRUSSES WHERE NO SHEATHING IS ATTACHED TO THE TRUSS BOTTOM CHORD OR WITH TRUSS BOTTOM FILLER. PROVIDE 2x4 LATERAL BRACING @ 36" UNDER PIGGYBACK TRUSSES. ALL BRACING SHALL BE NAILED WITH 2-16d NAILS TO TRUSSES.

AT TRUSSES REQUIRING HORIZONTAL WEB BRACING, PROVIDE 2x4 DIAGONAL BRACE (APPROX.45 DEGREES) @ 20' MAXIMUM SPACING. NAIL THE TOP END OF DIAGONAL TO WEB OF TRUSS AT ROOF, NAIL MIDDLE OF DIAGONAL TO TRUSS WEB AT HORIZONTAL LATERAL BRACING LOCATION AND THE BOTTOM END OF DIAGONAL TO BOTTOM OF WEB OF TRUSS AT CEILING.

SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

MCVEIGH & MANGUM ENGINEERING, INC OR ANY OF ITS EMPLOYEES SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

VISUAL OBSERVATIONS OF THE STRUCTURAL SYSTEM BY MCVEIGH & MANGUM ENGINEERING FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE INTERNATIONAL BUILDING CODE.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS OF EXISTING STRUCTURE AND SITES THAT ARE AFFECTED BY NEW WORK BEFORE PROCEEDING WITH FABRICATION AND CONSTRUCTION.

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

SHOP DRAWINGS AND SUBMITTALS

SHOP DRAWING SUBMITTALS ARE ONLY REVIEWED FOR GENERAL CONFORMANCE WITH THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR MUST REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO THEIR SUBMITTAL TO THE ARCHITECT. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP SHALL BE RETURNED WITHOUT REVIEW. ANY REQUESTED CHANGES TO THE CONTRACT DOCUMENTS SHALL BE COMMUNICATED IN WRITING PRIOR TO SUBMITTING THE SHOP DRAWINGS AND CLOUDED ON THE SHOP DRAWINGS.

SHOP DRAWINGS MUST BE SUBMITTED FOR ENGINEER'S REVIEW OF THE FOLLOWING ITEMS: (S/S = SIGNED & SEALED SHOP DRAWING WITH CALCS, SD = SHOP DRAWING FOR REVIEW ONLY)

• CONCRETE REINFORCING LAYOUT	S/S☐	SD■
• CONCRETE CONSTRUCTION JOINT LAYOUT	S/S☐	SD■
• MASONRY REINFORCEMENT LAYOUT	S/S☐	SD■
• CONCRETE MIX DESIGNS	S/S☐	SD■
• WOOD TRUSS SYSTEMS	S/S■	SD☐
• MISC STEEL FABRICATIONS	S/S☐	SD■
• EXTERIOR CLADDING (CURTAINWALLS)	S/S■	SD☐

COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM OF RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. VENDOR-DESIGNED COMPONENT SHOP DRAWINGS SHALL BE APPROVED BY THE COMPONENT DESIGNER ENGINEER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASE STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. THE CONTRACTOR SHALL SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR APPROVAL.

SPECIFICATIONS

CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION). EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE TESTING.

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES - ACI 530.1/ASCE 6" (LATEST EDITION), EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM MASONRY TESTING.

ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF AISC "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS", AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AND AWS D1.1" "STRUCTURAL WELDING CODE", EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

GALVANIZING: CONFORM TO ASTM STANDARDS A 123, A 386, AND A 153 AS APPLICABLE WHEREVER SURFACES ARE INDICATED OR SPECIFIED TO BE GALVANIZED. GALVANIZE AFTER FABRICATION UNLESS OTHERWISE INDICATED OR SPECIFIED. REPAIR ALL GALVANIZED COATINGS THAT BECOME DAMAGED IN HANDLING, TRANSPORTING, WELDING, OR BOLTING. MAKE THE REPAIRS BY APPLICATION OF A GALVANIZING REPAIR PAINT CONFORMING TO ASTM A 780. CLEAN ALL AREAS THAT ARE TO BE REPAIRED; REMOVE SLAG FROM WELDS. APPLY REPAIR PAINT TO COLD SURFACES.

A GEOTECHNICAL ENGINEER SHALL BE EMPLOYED TO CONFIRM BEARING PRESSURE STATED PRIOR TO CONSTRUCTION. THE ENGINEER SHALL DEVELOP & ENSURE IMPLEMENTATION OF A SITE PREPARATION PROGRAM AS HE DEEMS NECESSARY TO ACHIEVE THE STATED BEARING PRESSURE.

FOOTING AND SLAB SUBGRADE PREPARATION SHALL BE IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

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BUSY Bee

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License No.: 85369

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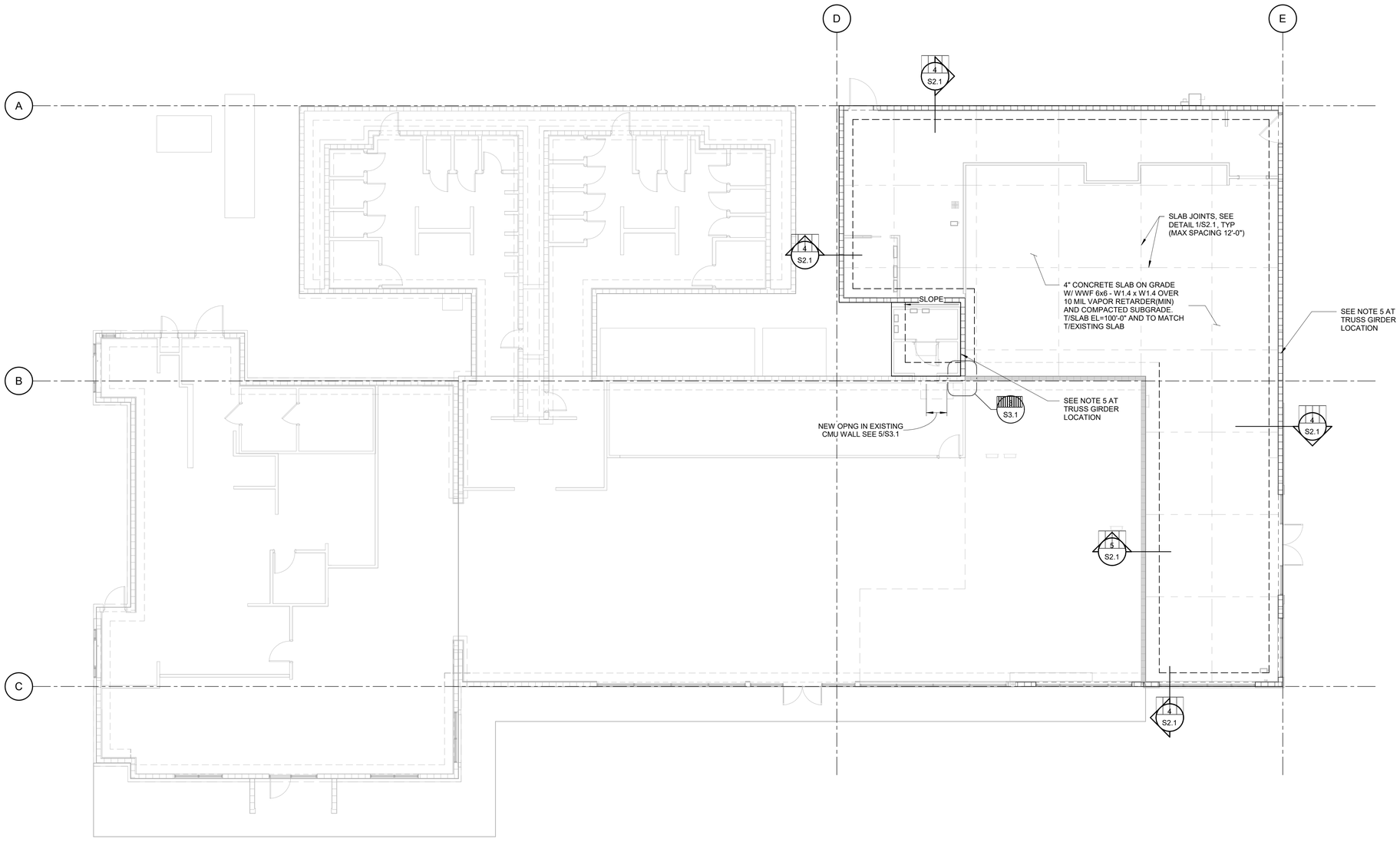
S0.2



1 / S1.1 **FOUNDATION PLAN**

SCALE: 1/8" = 1'-0"

PLAN NOTES:

1) FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEETS S0.1 & S0.2.
2) ELEVATIONS ARE BASED ON DATUM EL 100'-0". SEE CIVIL DRAWINGS FOR ACTUAL ELEVATIONS.
3) T/FTG EL = 98'-0" MIN, SEE 4/S2.1 FOR MIN REQ'D EL BELOW GRADE.
4) FOR ELEVATIONS, WALL SECTIONS AND DIMENSIONS, SEE ARCHITECTURAL.
5) PROVIDE (3) FULL HEIGHT FULLY GROUTED CELLS WITH #5 VERT BAR AT TRUSS GIRDER BEARING LOCATON
6) SEE DETAIL 2/S2.1 FOR REINFORCEMENT @ FOOTING CORNERS (TYP).
7) SEE DETAIL 3/S2.1 WHERE PIPE PENETRATES PERIMETER OF BUILDING. SEE MECH, PLUMBING, AND ARCH FOR PIPE LOCATIONS, SIZES AND ELEVATIONS.
8) ▭ DENOTES 8" CMU WALL FOR TYP CMU WALL REINFORCEMENT & DETAILS, SEE GENERAL NOTES ON S0.1 & DETAIL 1/S3.1.
9) ▨ DENOTES EXISTING 8" CMU WALL TO BE REMOVED.
10) PROVIDE 2-#4x3'-0" MID SLAB @ ALL SOG RE-ENTRANT CORNERS WITHOUT SLAB JOINT.
11) CONTRACTOR TO FIELD VERIFY EXISTING FOUNDATION SYSTEM AND NEW FOUNDATION SYSTEM TO MATCH. NOTIFY MME IF DIFFERENT THAN SHOWN ON THESE DRAWINGS.

Columbia County Building Department — Plans Reviewed for Code Compliance — State of Florida

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.

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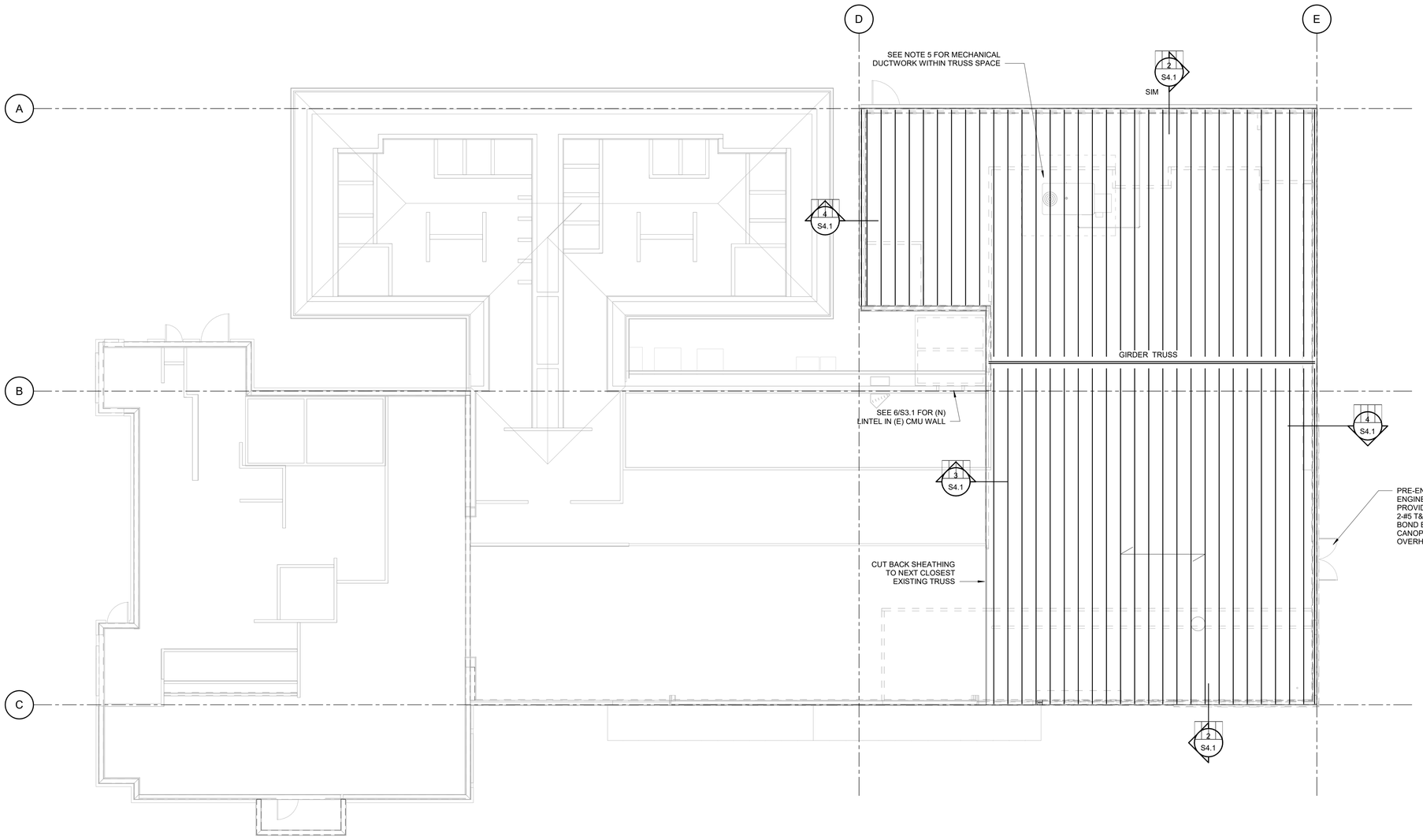
S1.1



1 / S1.2 ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

PLAN NOTES:

1) FOR DESIGN CRITERIA AND GENERAL NOTES, SEE SHEETS S0.1 & S0.2.
2) NEW ROOF FRAMING CONSISTS OF FLAT WOOD ROOF TRUSSES WITH TOP CHORD SLOPE SPACED AT 2'-0"OC. COORD TOP CHORD SLOPE REQUIREMENTS WITH ARCH.
3) FOR SHEATHING ATTACHMENT PATTERN, SEE 5/S4.1.
4) <——→ INDICATES SPAN DIRECTION OF (N) 3/4 PLYWOOD SHEATHING. COORD FINISH REQUIREMENTS W/ ARCH.
5) TRUSS ENGINEER TO COORDINATE WITH MECH DUCTWORK TO ACCOMODATE DUCTS WITHIN WEBS AND BETWEEN TRUSSES.

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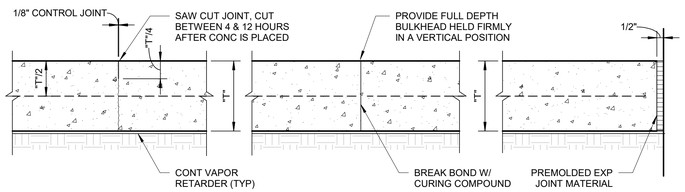
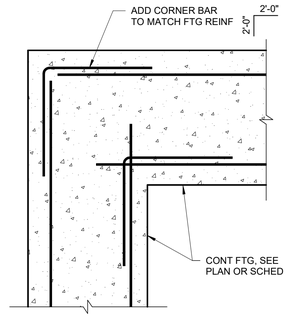
S1.2



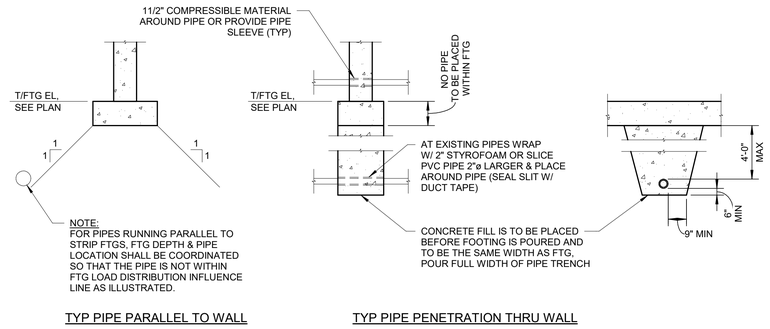
CRACK CONTROL JOINT | CONSTRUCTION JOINT | EXPANSION JOINT AT WALL

NOTE:
1) USE CONSTRUCTION JOINT INSTEAD OF CRACK CONTROL JOINT WHEREVER CONSTRUCTION IS STOPPED OR WHERE CALLED FOR ON PLAN.

1 S2.1 TYP SLAB ON GRADE JOINTS
SCALE: NTS

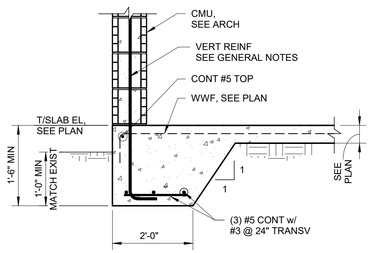


2 S2.1 TYP CORNER FTG REINF
SCALE: NTS

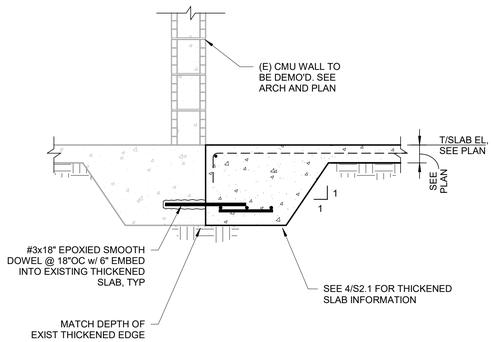


TYP PIPE PARALLEL TO WALL | TYP PIPE PENETRATION THRU WALL

3 S2.1 PIPE PENTERATION AT WALL FOOTING
SCALE: NTS



4 S2.1 TYP EXTERIOR FTG
SCALE: NTS



5 S2.1 SLAB ON GRADE CONN (EXIST TO NEW)
SCALE: NTS

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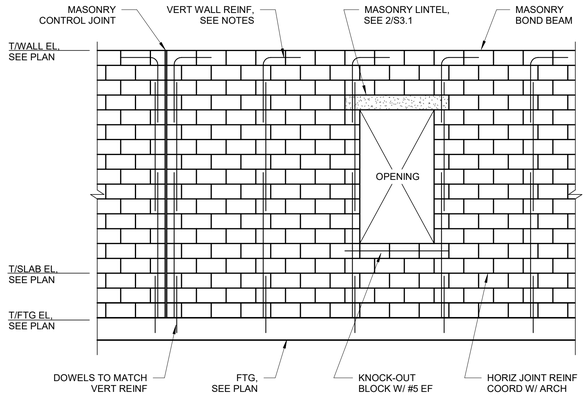
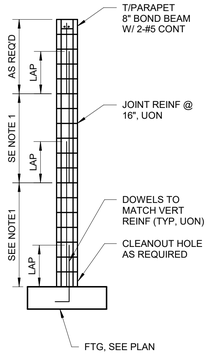
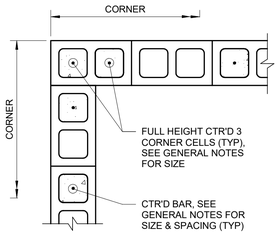
S2.1



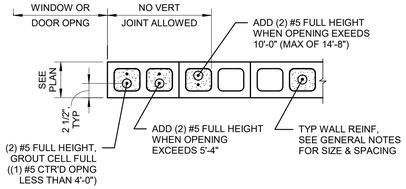
WALL ELEVATION



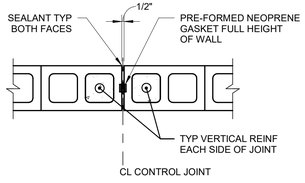
WALL SECTION



CORNER REINF

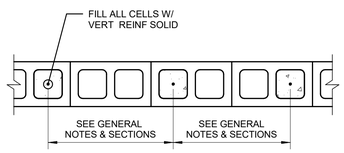


TYP WALL OPENING



CMU CONTROL JOINT

NOTE:
JOINT REINFORCEMENT SHALL BE STOPPED EACH SIDE OF JOINT. BOND BEAM REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONTROL JOINT.

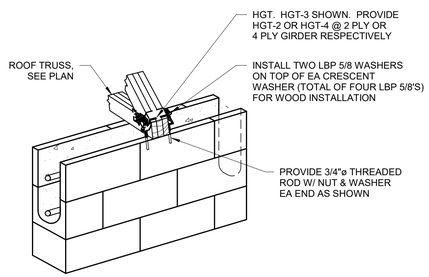


PLAN

NOTES:
1) 5' MAX IF LOW LIFT GROUTING IS USED.
2) IF HIGH LIFT GROUTING IS USED, REINFORCING SHALL BE FULL HEIGHT & A CLEANOUT HOLE IS REQ'D @ CELLS W/ REBAR. GROUT SHALL BE PLACED IN LIFTS TO PREVENT BLOWOUTS.
3) SEE GENERAL NOTES FOR BAR LAPS.
4) SEE GENERAL NOTES FOR HORIZONTAL JOINT REINFORCING & LAP LENGTH.
5) HORIZONTAL REINFORCING ABOVE AND BELOW OPENING SHALL EXTEND A MINIMUM OF 24" BEYOND OPENING

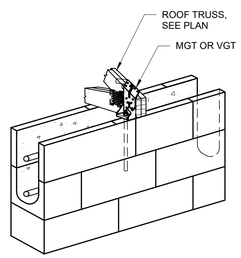
1 S3.1 TYP REINF MASONRY WALL

SCALE: NTS



HGT

FASTENERS:	CAPACITY:
HGT-2: 16-10d NAILS	10950#
HGT-3: 16-10d NAILS	10500#
HGT-4: 16-10d NAILS	9250#



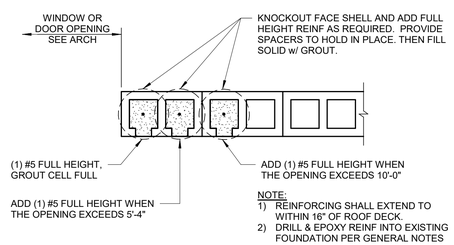
MGT OR VGT

FASTENER:	CAPACITY:
MGT: 22-10d NAILS	MGT = 3900#
	VGT = 4900#
	2-VGT = 7150#

4 S3.1 TYP GIRDER HOLDDOWNS @ CMU

SCALE: NTS

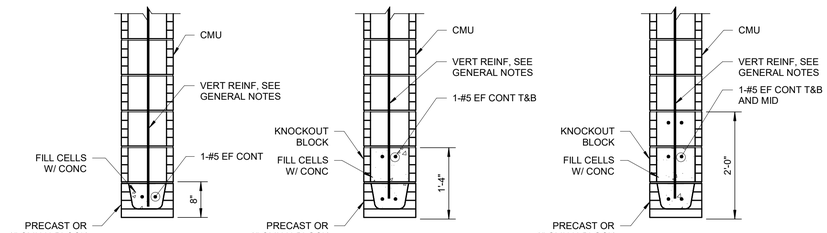
NOTES:
1) SEE ARCH FOR OPENING DIMENSIONS.
2) HIGH LIFT GROUTING MUST BE USED WHEN FILLING CELLS. PROVIDE KNOCKOUT AT BOTTOM OF WALL TO ENSURE PROPER FILLING OF CELLS.
3) REINFORCING SHALL EXTEND TO WITHIN 16" OF ROOF DECK.
4) DRILL & EPOXY REINF INTO EXISTING FOUNDATION PER GENERAL NOTES
5) SEE 6/S3.1 FOR LINTEL INFORMATION



NOTE:
1) REINFORCING SHALL EXTEND TO WITHIN 16" OF ROOF DECK.
2) DRILL & EPOXY REINF INTO EXISTING FOUNDATION PER GENERAL NOTES

5 S3.1 JAMBS @ NEW CMU OPENING

SCALE: NTS

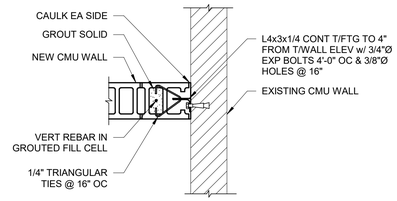


MASONRY OPNG LESS THAN 5'-0" | MASONRY OPNG 5'-0" OR MORE (MAX OPNG SIZE 8'-0") | MASONRY OPNG 8-0" OR MORE (MAX OPNG SIZE 14'-8")

NOTES:
1) FOR OPNG LOCATIONS SEE ARCH DWGS.
2) PROVIDE 8" BEARING EA SIDE OF OPNG.

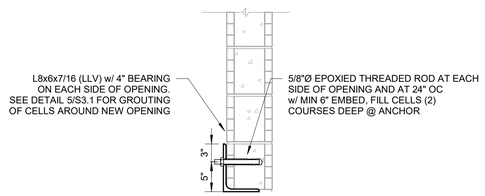
2 S3.1 TYP MASONRY LINTEL DETAIL

SCALE: NTS



3 S3.1 NEW CMU WALL TO EXIST WALL CONN B

SCALE: NTS



INSTALLATION PROCEDURE IS AS FOLLOWS:
PRIOR TO REMOVAL OF ENTIRE BLOCKOUT FOR NEW OPENING, VERTICALLY SAWCUT AT OPENING EDGES KNOCKOUT REQUIRED BLOCK FACES AND PROVIDE GROUT AS REQUIRED ABOVE. NEXT, PROVIDE HORIZONTAL SAWCUT AND INSTALL LINTEL ASSEMBLY PER DETAIL. REMOVE REMAINDER OF NEW BLOCKOUT WHEN INSTALLATION OF OPENING REINFORCEMENT IS COMPLETE.

6 S3.1 SECTION OF NEW LINTEL

SCALE: NTS

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S3.1

1 FASTENER SCHEDULE

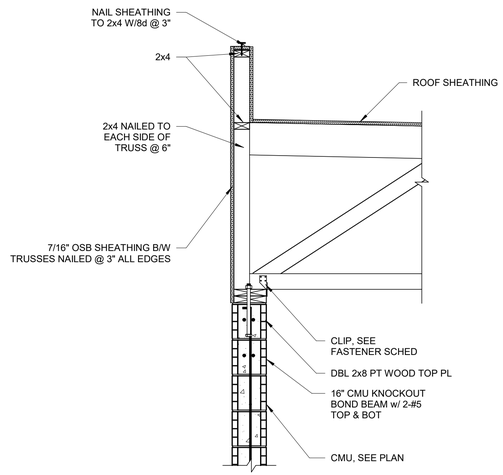
LOCATION	UPLIFT	FASTENER (1)	CONNECTION TRUSS	CONNECTION PLATE
ROOF TRUSS (2)	<455#	1-H5	4-8d	4-8d
	<600#	1-H2.5A	5-8d	5-8d
	<1200#	2-H2.5A	5-8d	5-8d
	>1200#	SEE DETAIL 4/S3.1		
SILL PLATE TO CMU WALLS		1/2"Ø ANCHOR BOLT w/ 2x2x1/8" PL WASHER OR "MASA" @ 32"		

NOTES:
1) ALL CONNECTORS LISTED ARE SIMPSON STRONG-TIE, UON. OTHER MANUFACTURERS MAY BE SUBSTITUTED. NAIL SIZE AND NUMBER SHALL BE IN ACCORDANCE WITH MANUFACTURER'S CATALOG. ROOF TRUSS CLIPS SHALL BE SELECTED TO PROVIDE THE UPLIFT RESISTANCE SHOWN ON THE ROOF TRUSS SHOP DRAWINGS.
2) IN ADDITION TO SHEDULED HOLD DOWN, PROVIDE 3-10d TOE NAILS.
3) EMBEDMENT OF ANCHOR BOLTS SHALL BE AS FOLLOWS:

ANCHOR BOLT TYPE	1/2"Ø	5/8"Ø	3/4"Ø	7/8"Ø
EMBEDDED ANCHOR @ INTERIOR	7"	7"	7"	7"
EMBEDDED ANCHOR @ EDGE	7"	7"	8"	10"
EMBEDDED ANCHOR IN TOP OF CMU WALL	7"	9"	13"	18"
EPOXIED THREADED ROD	SEE GENERAL NOTES--			
EXPANSION ANCHORS	SEE GENERAL NOTES--			

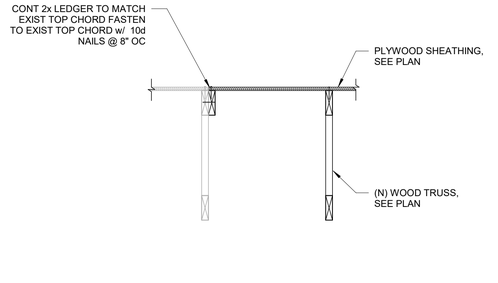
EDGE DISTANCE FOR SILL PLATE BOLTS SHALL BE A MIN OF 1/2 OF SILL WIDTH. EDGE DISTANCE FOR HOLDDOWNS AND ALL OTHERS SHALL BE 2 1/2" MIN EMBEDDED ANCHOR BOLTS SHALL BE HEADED OR BE THREADED RODS WITH A NUT ATTACHED TO THE EMBEDED END. J-BOLTS GREATER THEN 1/2"Ø ARE NOT PERMITTED.

1 S4.1 **FASTENER SCHEDULE**
SCALE: NTS

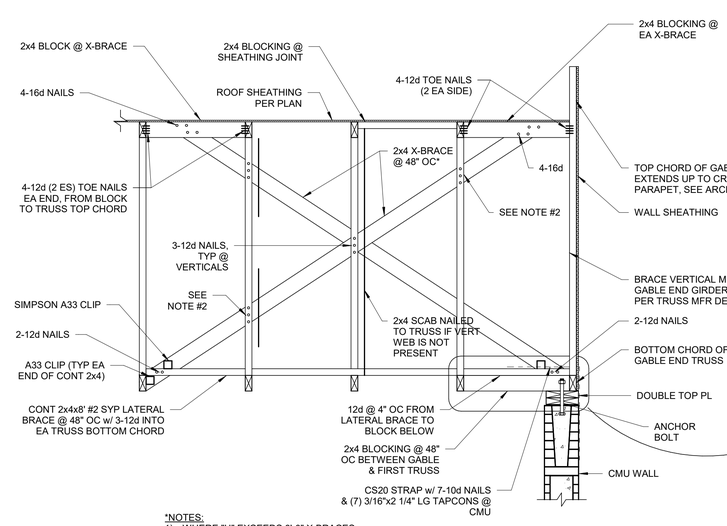


NOTE:
AT SIM, NO PARAPET

2 S4.1 **TYP TRUSS BEARING**
SCALE: NTS

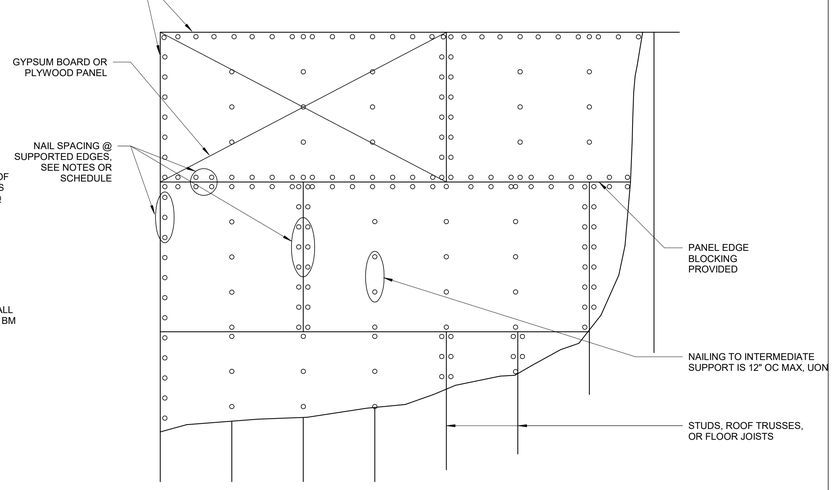


3 S4.1 **SECTION AT EXISTING TRUSS**
SCALE: NTS



*NOTES:
1) WHERE "H" EXCEEDS 8'-0" X-BRACES SHALL BE 2x6.
2) WHERE "H" EXCEEDS 10'-6" X-BRACE MEMBERS SHALL BE NAILED TO VERTICAL WEB (OR SCAB) @ EA TRUSS.
3) CMU REINF NOT SHOWN FOR CLARITY

4 S4.1 **GABLE END TRUSS ON CMU**
SCALE: NTS



NOTES:
1) ROOF SHEATHING SHALL BE STAGGERED AS SHOWN WITH LONG DIRECTION OF PLYWOOD TRANSVERSE TO TRUSSES OR JOISTS.
2) UNSUPPORTED (UNBLOCKED) PANEL EDGE. PROVIDE BLOCKING IF REQ'D BY NOTES OR SCHEDULE.

5 S4.1 **NAILING REQUIREMENTS FOR ROOF SHEATHING**
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S4.1