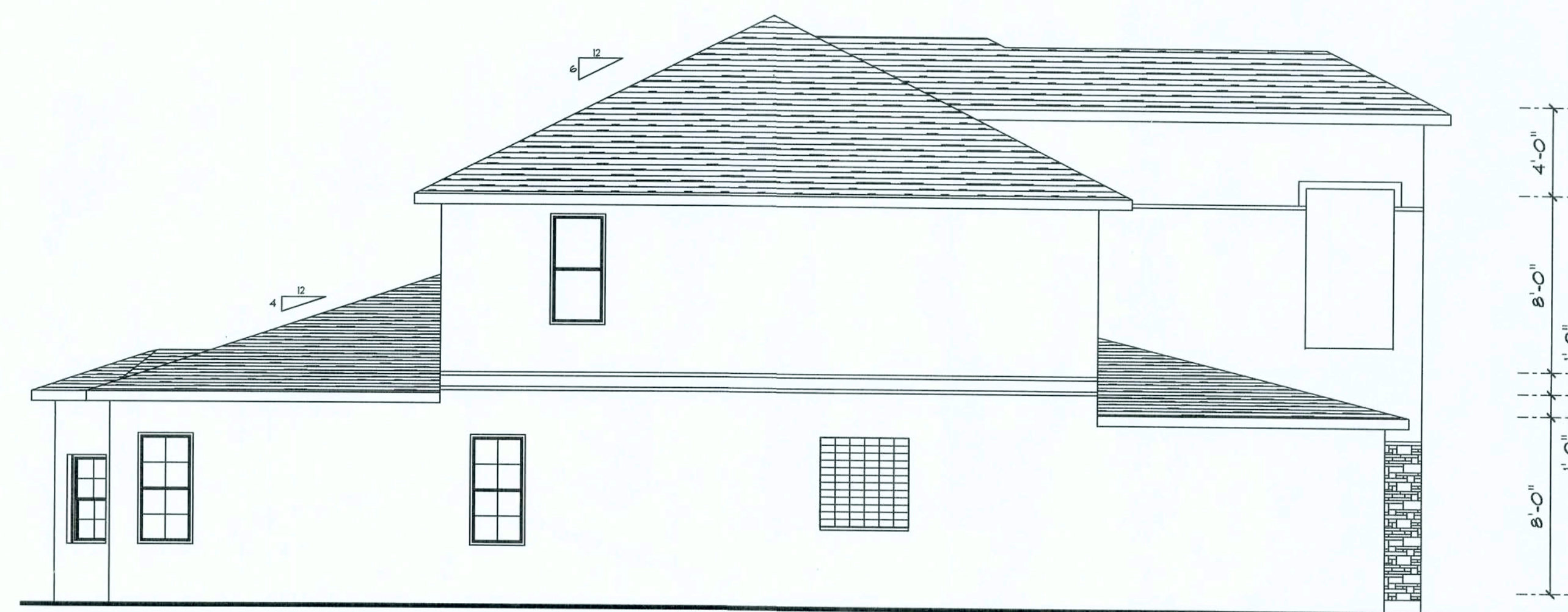


REVISIONS

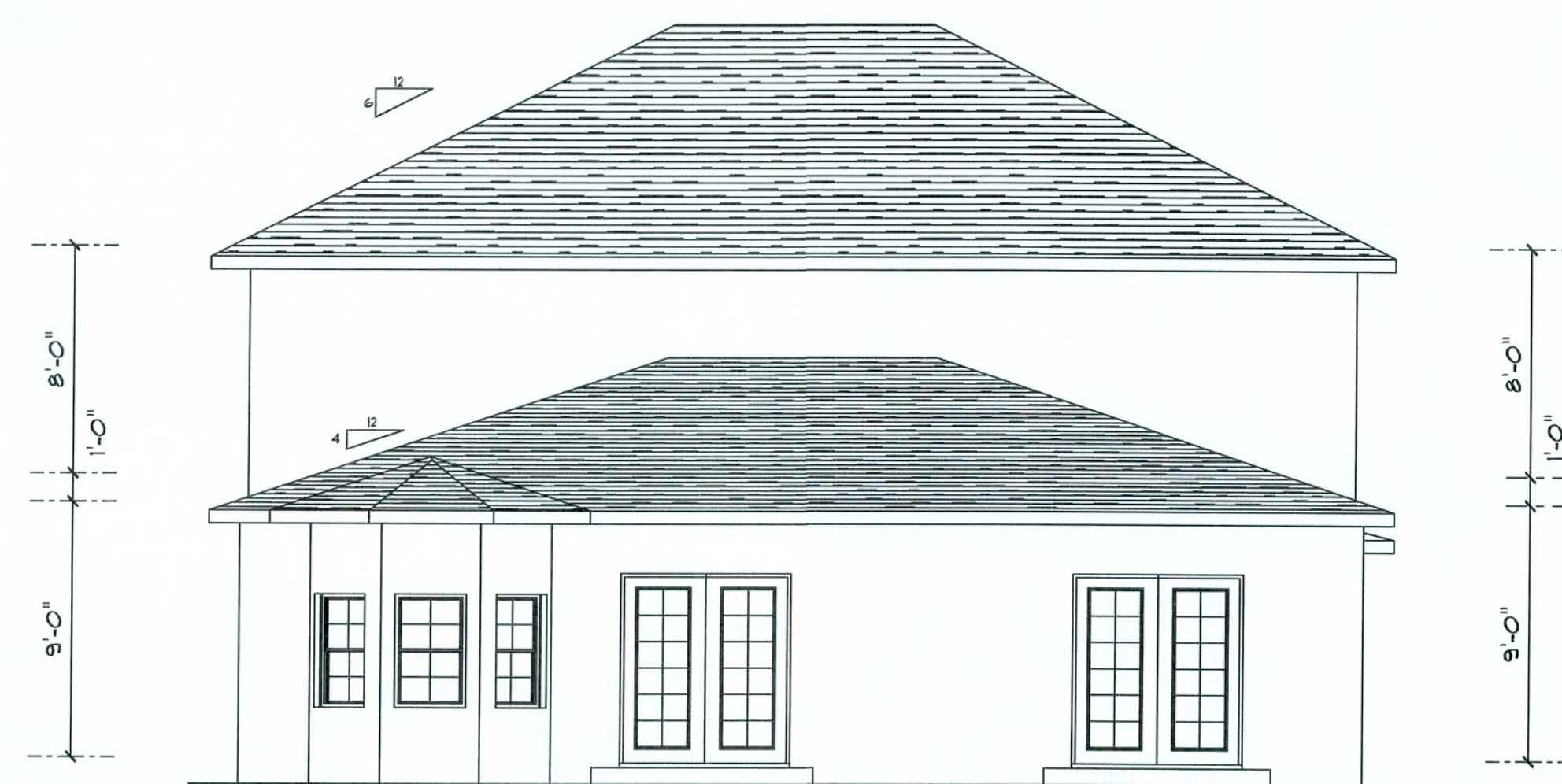
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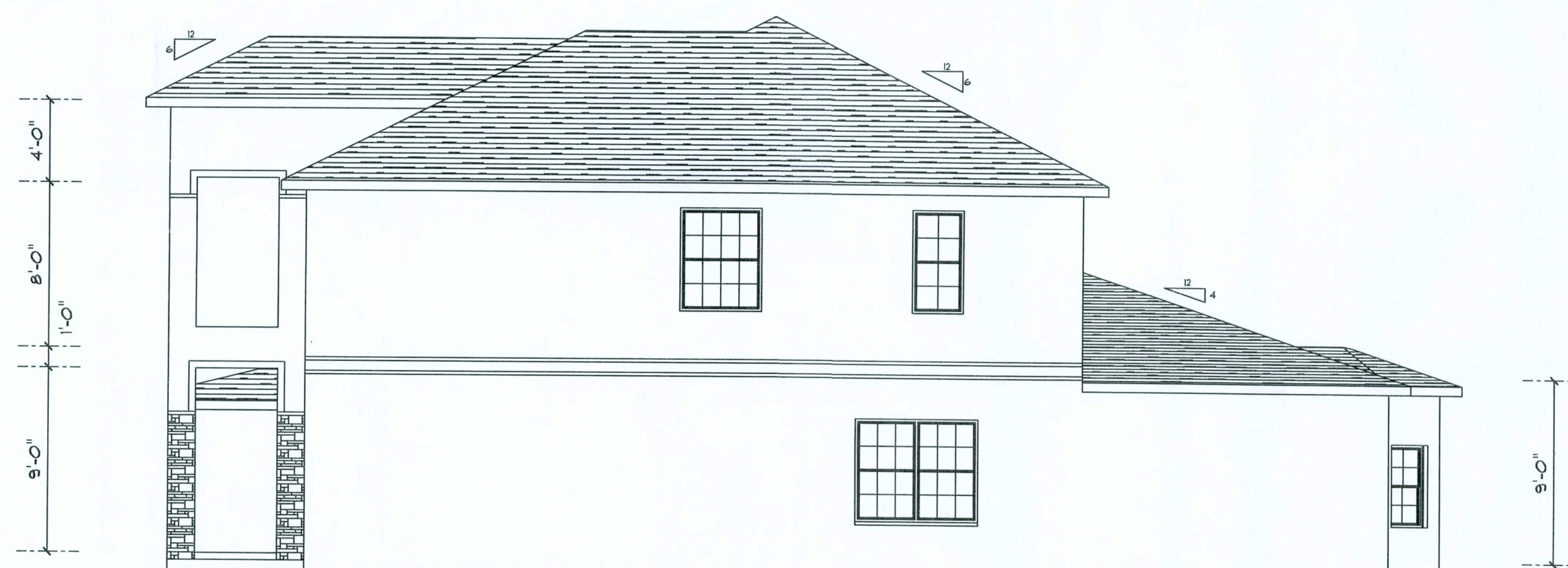
FRONT ELEVATION



LEFT ELEVATION



REAR ELEVATION



RIGHT ELEVATION

Lipscomb Eagle
Development, Inc.

Mediterranean Model

ADDRESS:
Lot 115 Preserve S/D
Columbia County, Florida

Lipscomb Eagle
Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone: (386) 719 - 6960
Fax: (386) 719 - 9586

PRINTED DATE:
December 14, 2007

DRAWN BY: T.J. Clanton
CHECKED BY: SUSAN HOLTON

DESIGNED BY:

FINALES DATE:
14 / Dec / 07

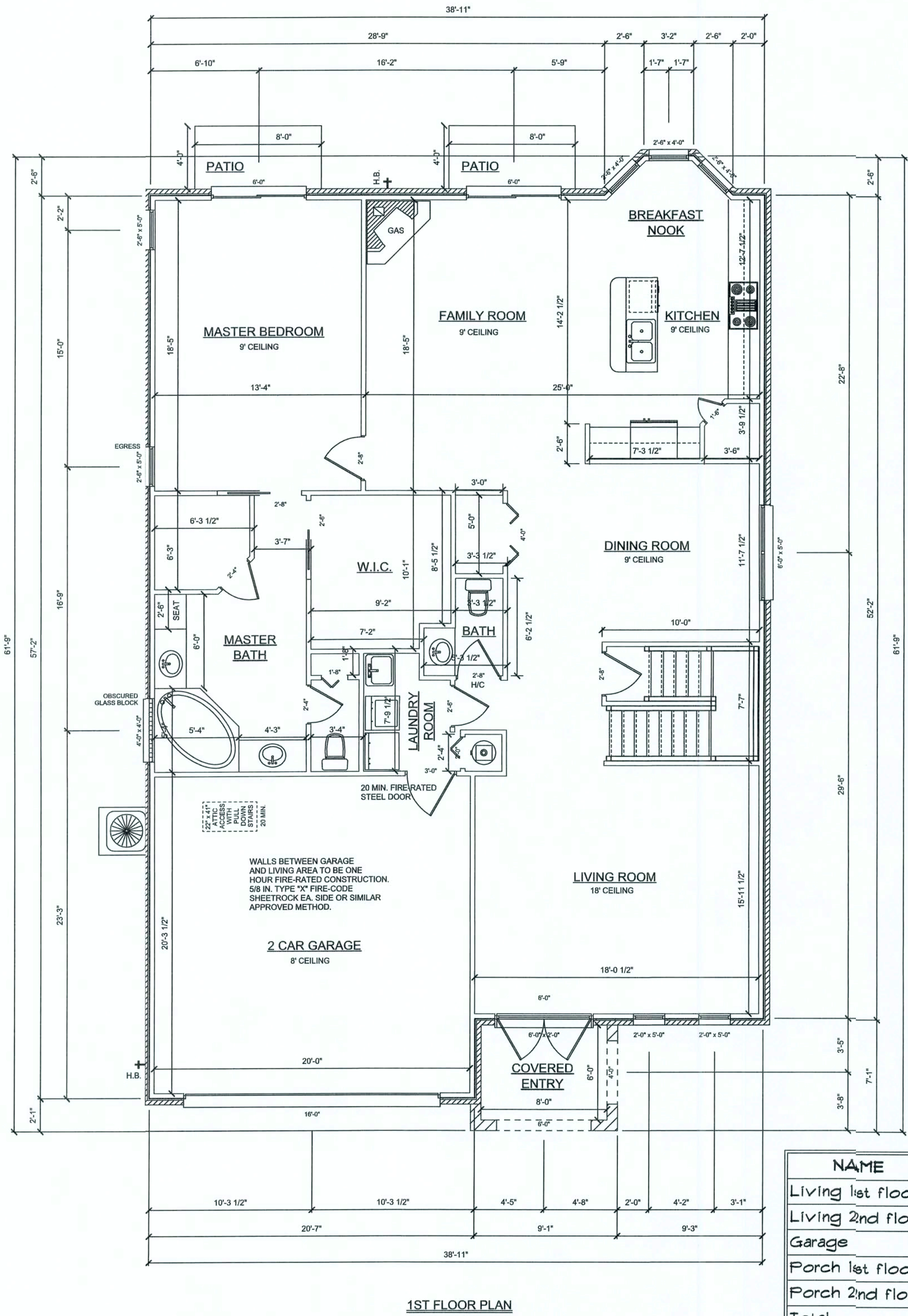
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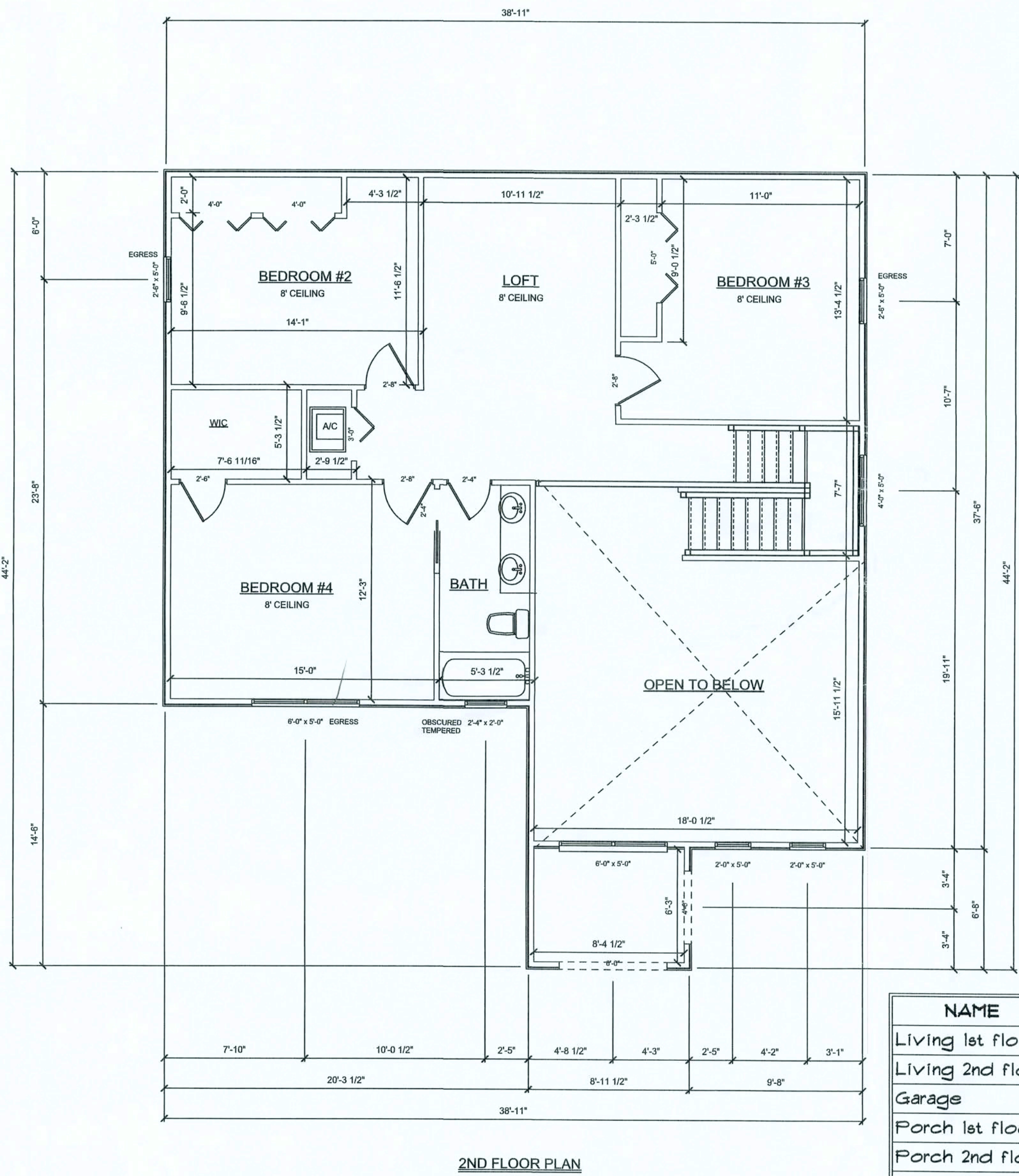
A-1

1 OF 3 SHEETS

| REVISIONS | |
|-----------|--|
| | |
| | |
| | |



| NAME | AREA |
|------------------|-----------|
| Living 1st floor | 1791 S.F. |
| Living 2nd floor | 950 S.F. |
| Garage | 435 S.F. |
| Porch 1st floor | 64 S.F. |
| Porch 2nd floor | 64 S.F. |
| Total | 3310 S.F. |



| NAME | AREA |
|------------------|-----------|
| Living 1st floor | 1791 S.F. |
| Living 2nd floor | 950 S.F. |
| Garage | 435 S.F. |
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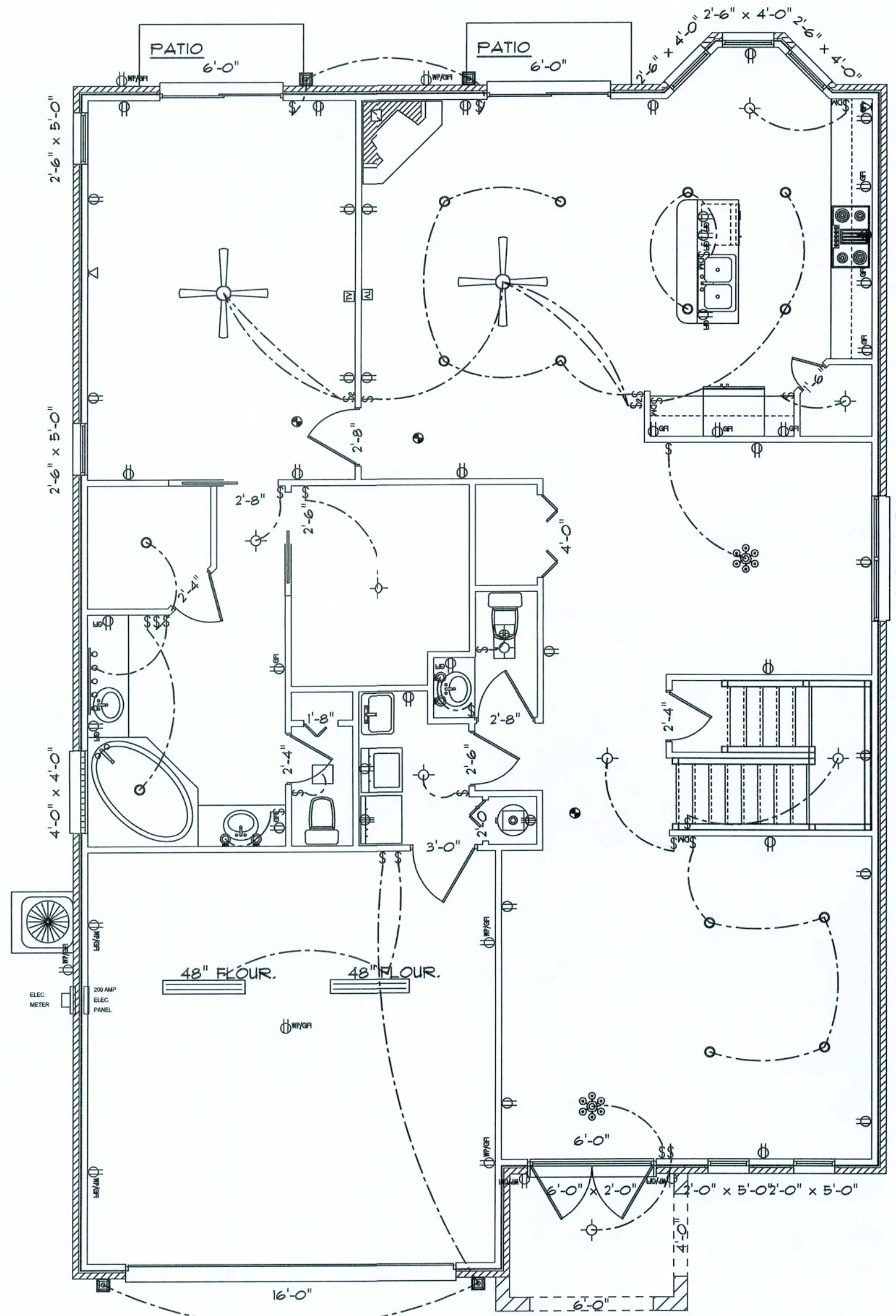
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DRAWING NUMBER

A-2

2 OF 3 SHEETS

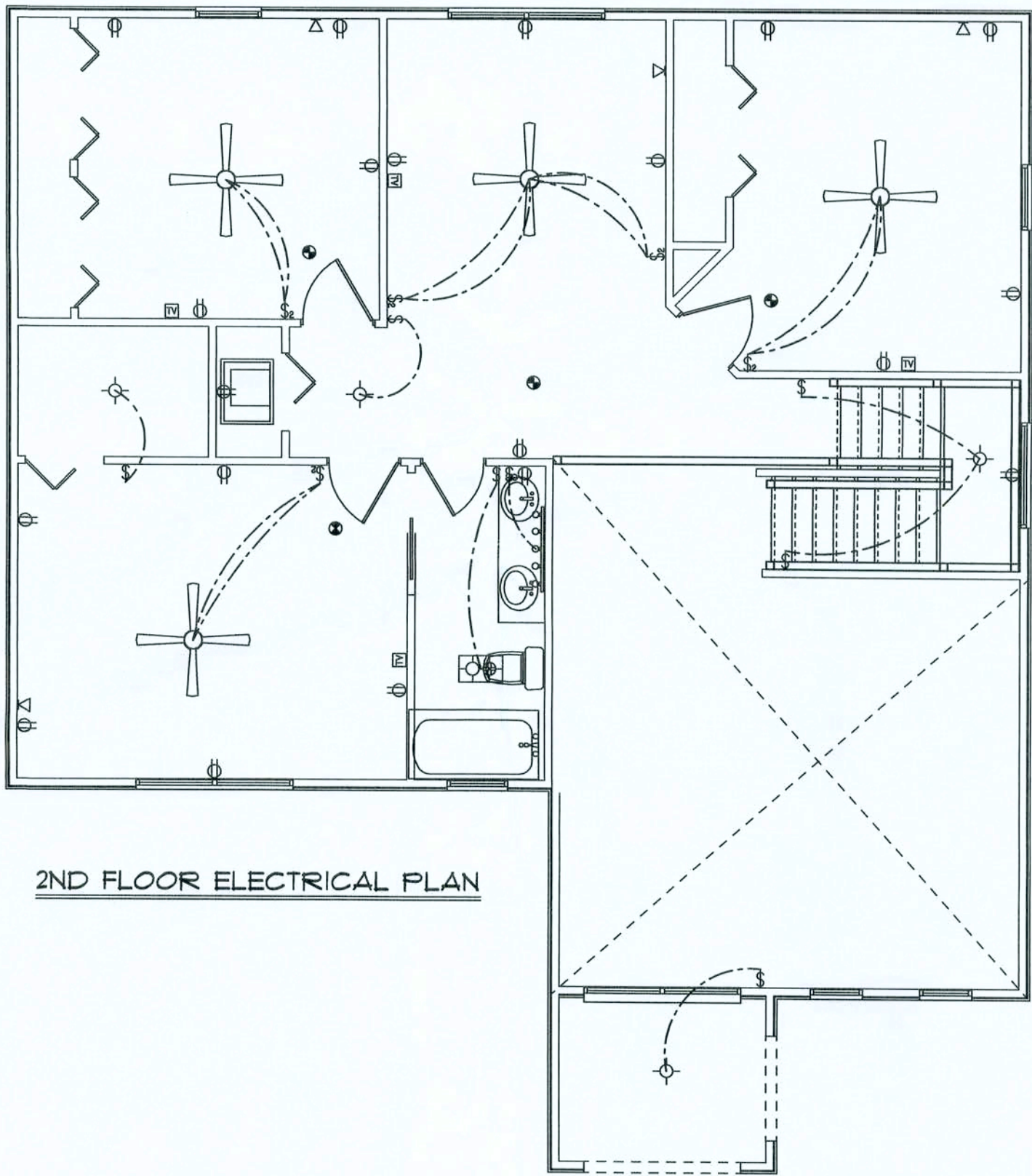
| REVISIONS | |
|-----------|--|
| | |
| | |
| | |



| ELECTRICAL | COUNT | SYMBOL |
|---------------------------|-------|--------|
| ceiling fan globe 1 | 2 | |
| ceiling light vent square | 2 | |
| chandelier | 2 | |
| fluorescent fixture | 1 | |
| pot light | 14 | |
| vanity bar light | 1 | |
| wall mount 1 | 4 | |
| wall mount 2 | 4 | |
| electrical panel | 2 | |
| cable tv outlet | 2 | |
| dimmer switch | 4 | |
| light | 8 | |
| outlet | 27 | |
| outlet gfi | 23 | |
| smoke detector | 2 | |
| switch | 22 | |
| switch double | 2 | |
| telephone | 2 | |

1ST FLOOR ELECTRICAL PLAN

| ELECTRICAL | COUNT | SYMBOL |
|----------------------|-------|--------|
| ceiling fan globe 1 | 4 | |
| vanity bar light | 1 | |
| Fan Light - bathroom | 1 | |
| cable tv outlet | 4 | |
| light | 4 | |
| outlet | 19 | |
| outlet gfi | 1 | |
| smoke detector | 4 | |
| switch | 7 | |
| switch double | 5 | |
| telephone | 4 | |



2ND FLOOR ELECTRICAL PLAN

Lipscomb Eagle Development, Inc.

Mediterranean Model

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PRINTED DATE:
December 14, 2007

| | |
|---------------------------|-----------------------------|
| DRAWN BY: T.J. Clanton | CHECKED BY: SUSAN HOLTON |
|---------------------------|-----------------------------|

DESIGNED BY:

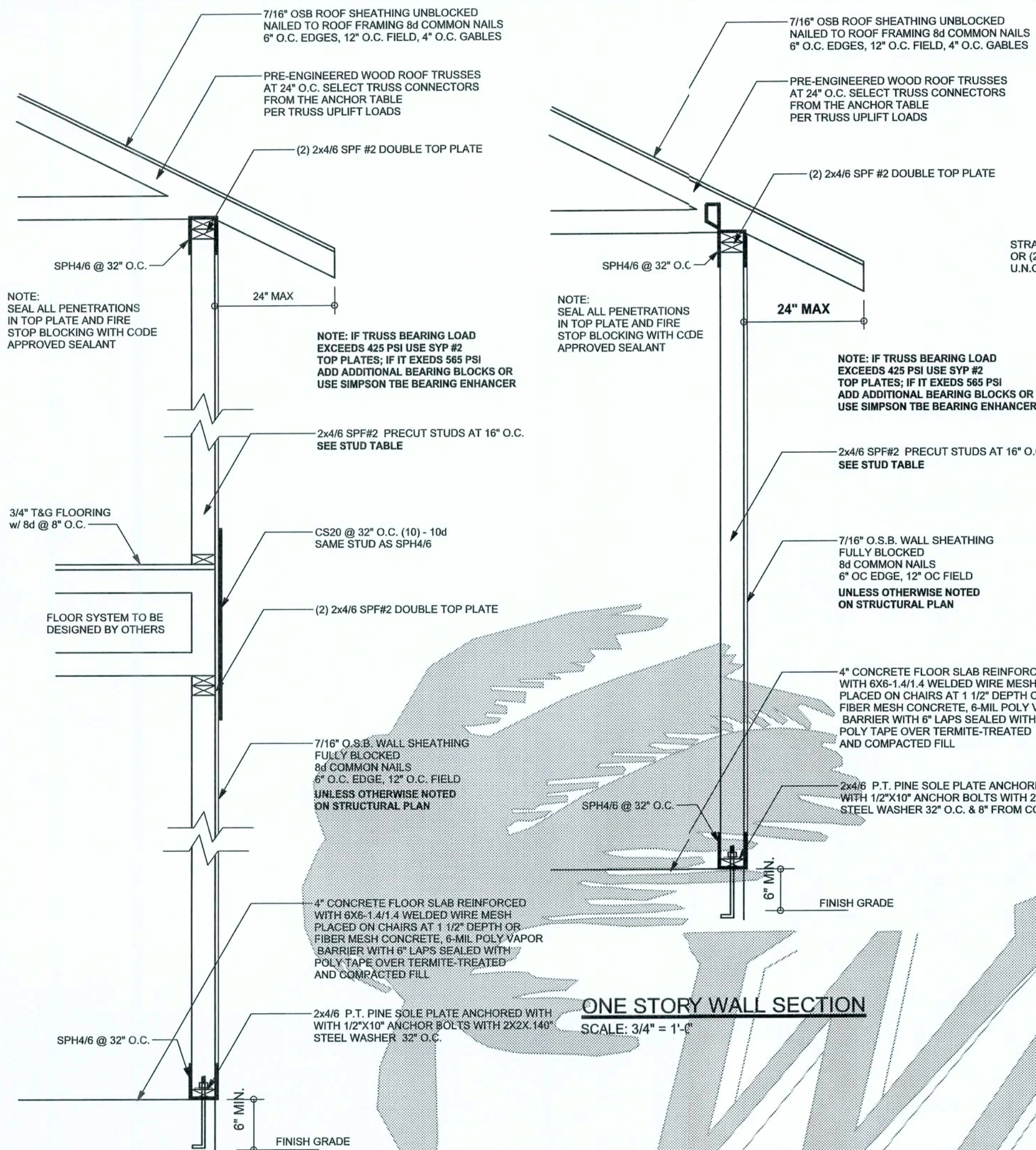
FINALES DATE:
14 / Dec / 07

JOB NUMBER:

DRAWING NUMBER

A-3

3 OF 3 SHEETS



INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

2 STORY INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

| | |
|--------------------|-----------------------|
| (1) 2x4 @ 16" O.C. | TO 11'-9" STUD HEIGHT |
| (1) 2x4 @ 12" O.C. | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" O.C. | TO 18'-0" STUD HEIGHT |
| (1) 2x6 @ 12" O.C. | TO 20'-0" STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 1.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TWO STORY WALL SECTION
SCALE: 3/4" = 1'-0"

INTERIOR SHEAR WALL DETAIL
SCALE: 1/2" = 1'-0"

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

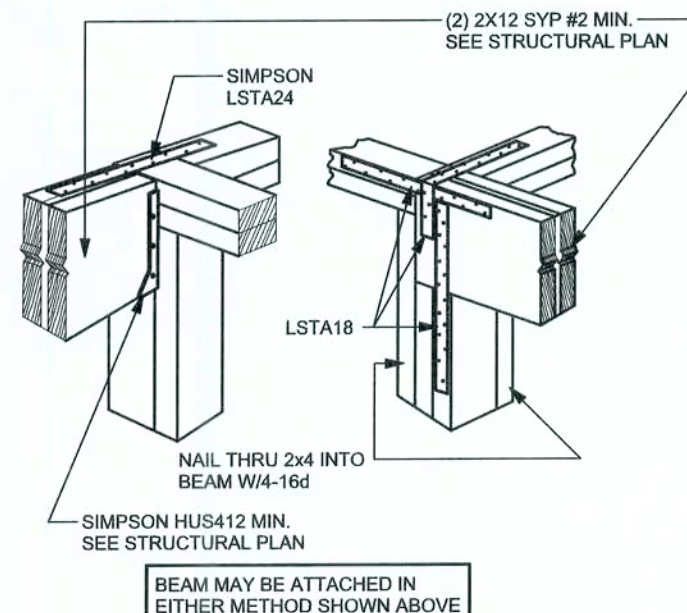
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHERS. LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" ON PER TABLE BELOW:

| DOOR WIDTH | 3/8" x 4" LAG | 16d STAGGER | (2) ROWS OF .131 x 3 1/4" ON |
|------------|---------------|-------------|------------------------------|
| 8' - 10' | 24" O.C. | 5" O.C. | 5" O.C. |
| 11' - 15' | 18" O.C. | 4" O.C. | 4" O.C. |
| 16' - 18' | 16" O.C. | 3" O.C. | 3" O.C. |

GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.

SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

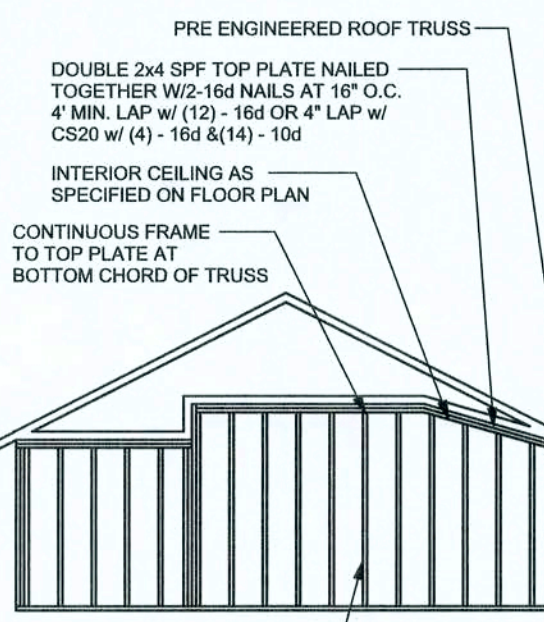
SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

TYPICAL HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

| | | Fb (psi) | E (10 ⁶ psi) |
|------|--------------|----------|-------------------------|
| 2x8 | SYP #2 | 1200 | 1.8 |
| 2x10 | SYP #2 | 1050 | 1.6 |
| 2x12 | SYP #2 | 975 | 1.6 |
| GLB | 24F-V3 SP | 2400 | 1.8 |
| LVL | TIMBERSTRAND | 1700 | 1.7 |
| LVL | MILICROLAM | 1600 | 1.9 |
| PSL | PARALAM | 2900 | 2.0 |

CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS DETAILING, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 418LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186 LOCATED IN MIDDLE OF THE SLAB; SUPPLIED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH CUT JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 24ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ALL BEAMS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE MEMBERS, CABLE EDGES AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO MEET THE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS DETAILING, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE TRUSS DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. A DESIGNER'S RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 8/MS 02). THE CONTRACTOR AND MASON MUST IMMEDIATELY BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

| ACI530.1-02 Section | Specific Requirements |
|--|---|
| 1.4A Compressive strength | 8" block bearing walls $F_m = 1500$ psi |
| 2.1 Mortar | ASTM C 270, Type N, UNO |
| 2.2 Grout | ASTM C 476, admixtures require approval |
| 2.3 CMU standard | ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block |
| 2.3 Clay brick standard | ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5" |
| 2.4 Reinforcing bars, #3 - #11 | ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia. (30" for #5) |
| 2.4F Coating for corrosion protection | Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GB60, 0.60 oz/lb or 304SS |
| 2.4F Coating for corrosion protection | Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS |
| 3.3.E.2 Pipes, conduits, and accessories | Any not shown on the project drawings require engineering approval |
| 3.3.E.7 Movement joints | Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings. |

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP | UPLIFT LBS. SPF | TRUSS CONNECTOR* | TO PLATES | TO RAFTER/TRUSS | TO STUDS |
|-------------------------------|-----------------|-----------------------|----------------|-----------------|-----------------------------------|
| < 420 | < 245 | H5A | 3-8d | 3-8d | |
| < 455 | < 265 | H5 | 4-8d | 4-8d | |
| < 360 | < 235 | H4 | 4-8d | 4-8d | |
| < 455 | < 320 | H3 | 4-8d | 4-8d | |
| < 415 | < 365 | H2.5 | 5-8d | 5-8d | |
| < 600 | < 535 | H2.5A | 5-8d | 5-8d | |
| < 950 | < 820 | H8 | 8-8d | 8-8d | |
| < 745 | < 565 | H8 | 5-10d, 1 1/2" | 5-10d, 1 1/2" | |
| < 1465 | < 1050 | H14-1 | 13-8d | 12-8d, 1 1/2" | |
| < 1465 | < 1050 | H14-2 | 15-8d | 12-8d, 1 1/2" | |
| < 990 | < 650 | H10-1 | 8-8d, 1 1/2" | 8-8d, 1 1/2" | |
| < 760 | < 655 | H10-2 | 6-10d | 6-10d | |
| < 1470 | < 1265 | H16-1 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1470 | < 1265 | H16-2 | 10-10d, 1 1/2" | 2-10d, 1 1/2" | |
| < 1000 | < 860 | MTS24C | 7-10d 1 1/2" | 7-10d 1 1/2" | |
| < 1450 | < 1245 | HTS24 | 12-10d 1 1/2" | 12-10d 1 1/2" | |
| < 2900 | < 2480 | 2-HTS24 | | | |
| < 2050 | < 1785 | LG2 | 14-16d | 14-16d | |
| HEAVY GIRDER TIEDOWNS* | | | | | |
| < 3965 | < 3330 | MGT | | 22-10d | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980 | < 6485 | HGT-2 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530 | < 9035 | HGT-3 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250 | < 9250 | HGT-4 | | 16-10d | 2-5/8" THREADED ROD 12" EMBEDMENT |
| STUD STRAP CONNECTOR* | | | | | |
| < 435 | < 435 | SSP DOUBLE TOP PLATE | 3-10d | | 4-10d |
| < 455 | < 420 | SSP SINGLE SILL PLATE | 1-10d | | 4-10d |
| < 825 | < 825 | DSP DOUBLE TOP PLATE | 6-10d | | 8-10d |
| < 825 | < 600 | DSP SINGLE SILL PLATE | 2-10d | | 8-10d |
| < 885 | < 760 | SP4 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH4 | | | 10-10d, 1 1/2" |
| < 885 | < 760 | SP6 | | | 6-10d, 1 1/2" |
| < 1240 | < 1065 | SPH6 | | | 10-10d, 1 1/2" |
| < 1235 | < 1165 | LSTA18 | 14-10d | | |
| < 1235 | < 1235 | LSTA21 | 16-10d | | |
| < 1030 | < 1030 | CS20 | 18-8d | | |
| < 1705 | < 1705 | CS16 | 28-8d | | |
| STUD ANCHORS* | | | | | |
| < 1350 | < 1305 | LTT19 | 8-16d | | 1/2" AB |
| < 2310 | < 2310 | LTT131 | 48-10d, 1 1/2" | | 1/2" AB |
| < 2775 | < 2570 | HDOA | 2-3/8" BOLTS | | 5/8" AB |
| < 4175 | < 3695 | HTT16 | 18-16d | | 5/8" AB |
| < 1400 | < 1400 | PHAD2 | 16-16d | | |
| < 3335 | < 3335 | HPH22 | 16-16d | | |
| < 2200 | < 2200 | ABJ44 | 12-16d | | 1/2" AB |
| < 2300 | < 2300 | ABJ66 | 12-16d | | 1/2" AB |
| < 2320 | < 2320 | ABJ88 | 18-16d | | 2-5/8" AB |

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBTSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

| | Zone | Effective Wind Area (F2) |
|---|-------------|--------------------------|
| | 10 | 100 |
| | 1 | 19.9 - 21.8 |
| | 2 | 19.9 - 25.5 |
| 2 0thg | 40.8 | -40.8 |
| 3 | 19.9 - 25.5 | 18.1 - 21.8 |
| 3 0thg | -68.3 | -42.4 |
| 4 | 21.8 - 23.6 | 18.5 - 20.4 |
| 5 | 21.8 - 29.1 | 18.5 - 22.6 |
| Doors & Windows Worst Case (Zone 5, 10 ft2) | 21.8 | -29.1 |
| 8x7 Garage Door | 19.5 | -22.9 |
| 16x7 Garage Door | 18.5 | -21.0 |

DESIGN LOADS

| | |
|--|--|
| FLOOR | 40 PSF (ALL OTHER DWELLING ROOMS) |
| | 30 PSF (SLEEPING ROOMS) |
| | 30 PSF (ATTICS WITH STORAGE) |
| | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF | 20 PSF (FLAT OR <4:12) |
| | 16 PSF (4:12 TO <12:12) |
| | 12 PSF (12:12 AND GREATER) |
| STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) | |
| SOIL BEARING CAPACITY 1000PSF | |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) | |

REVISIONS

| | | |
|--|--|--|
| | | |
| | | |
| | | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No.53915, P.O.B. 668, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
SEALED
10/20/07

Lipscomb Eagle
Development

Mediterranean Model

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Fax: (386) 269 - 4871

PRINTED DATE:
December 14, 2007

DRAWN BY: David Disoway

CHECKED BY:

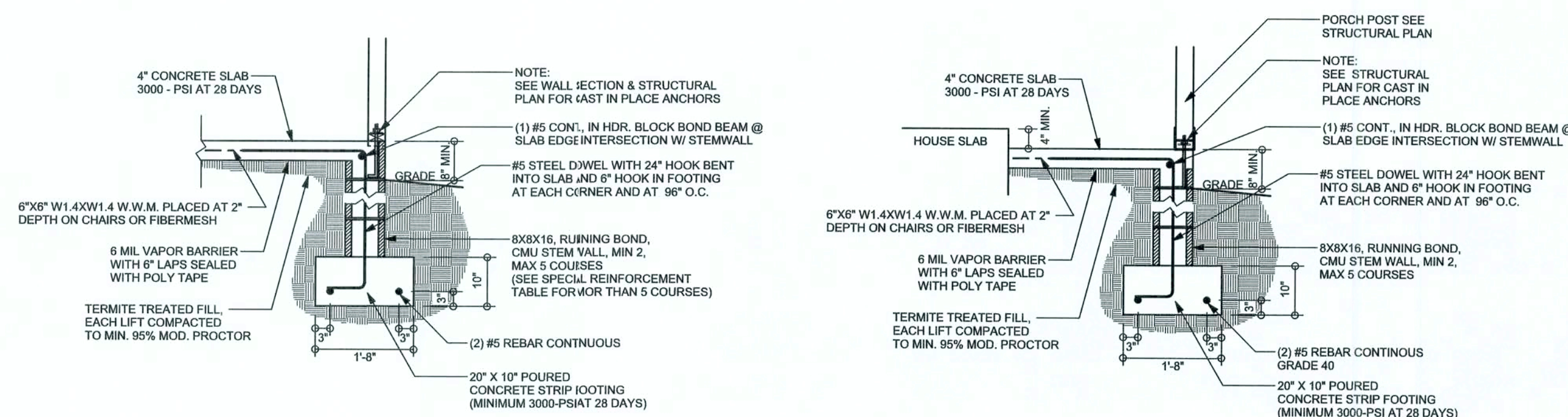
FINALS DATE:
14 / Dec / 07

JOB NUMBER:
711296

DRAWING NUMBER

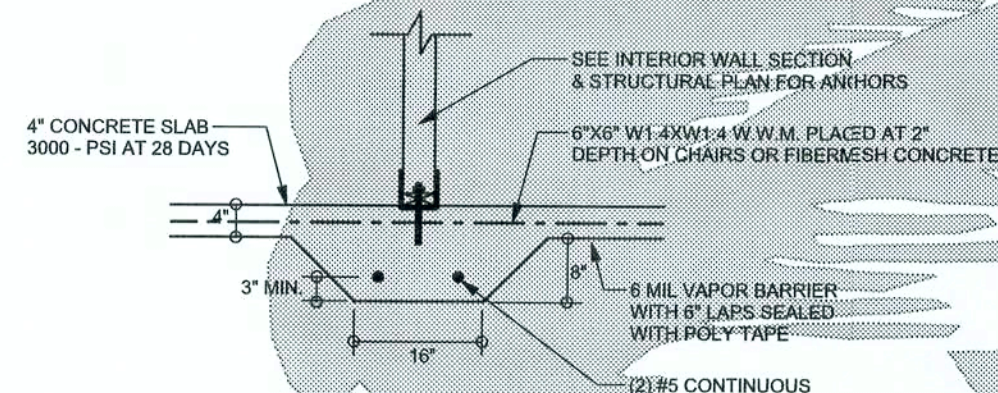
S-1

OF 3 SHEETS

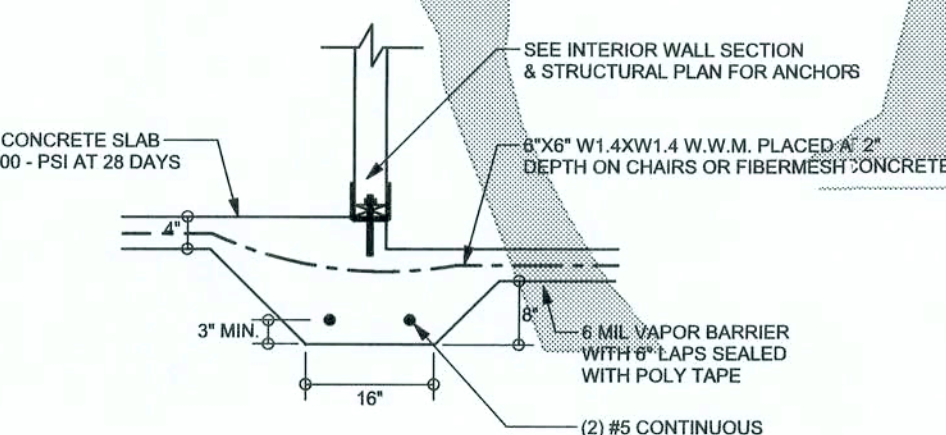


F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

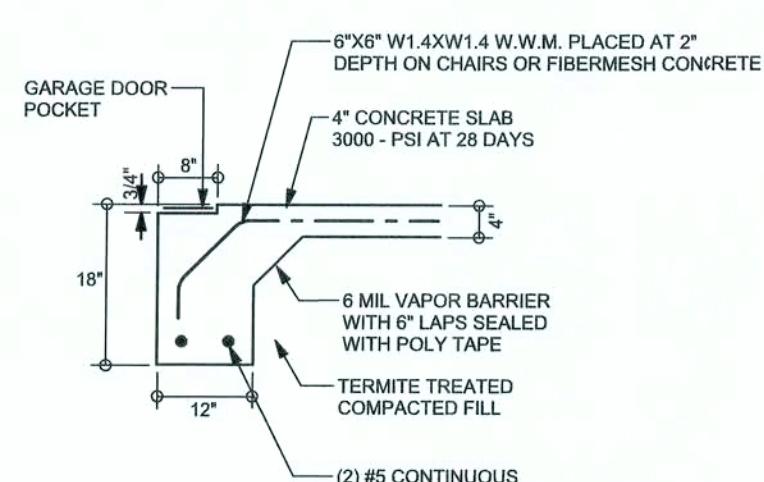
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



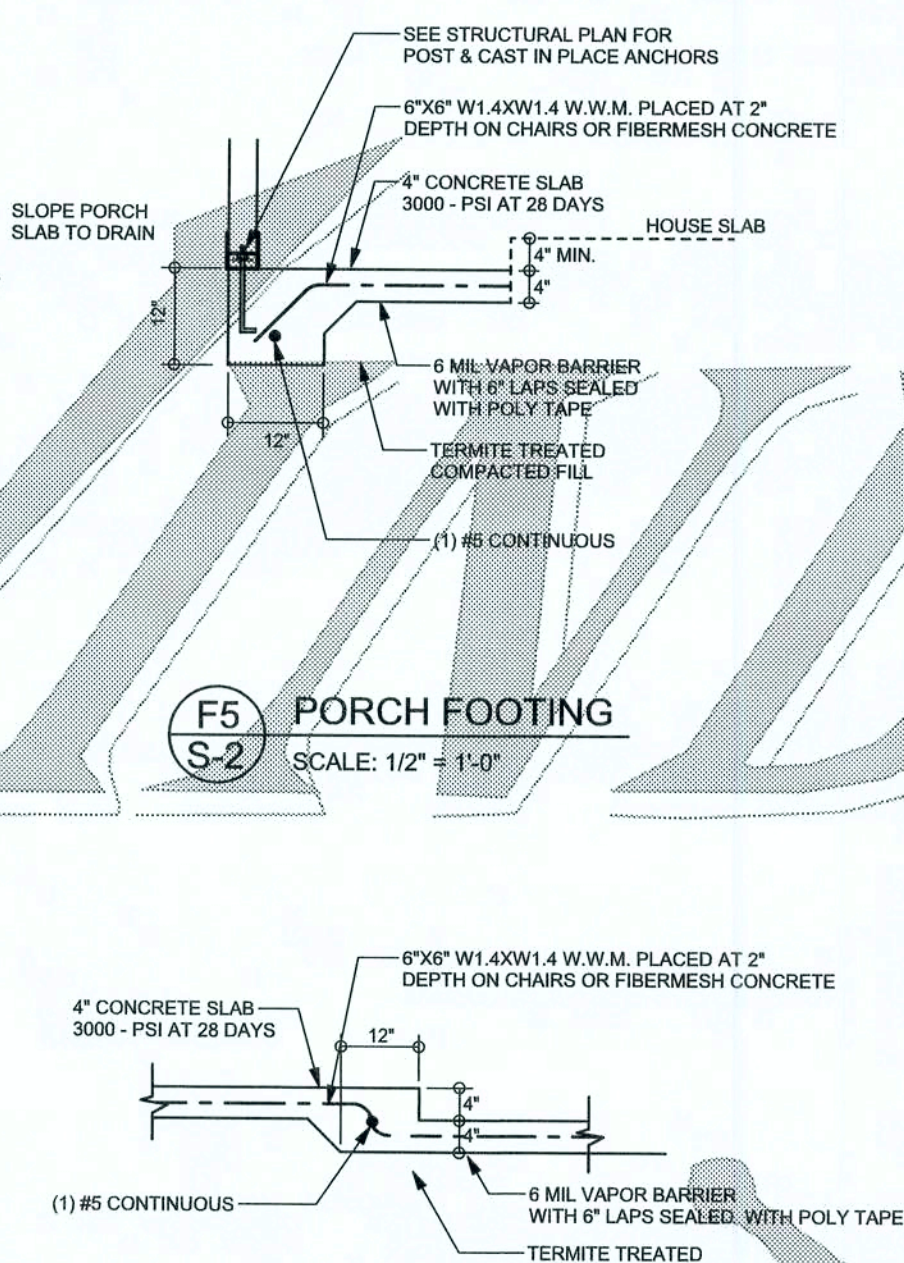
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



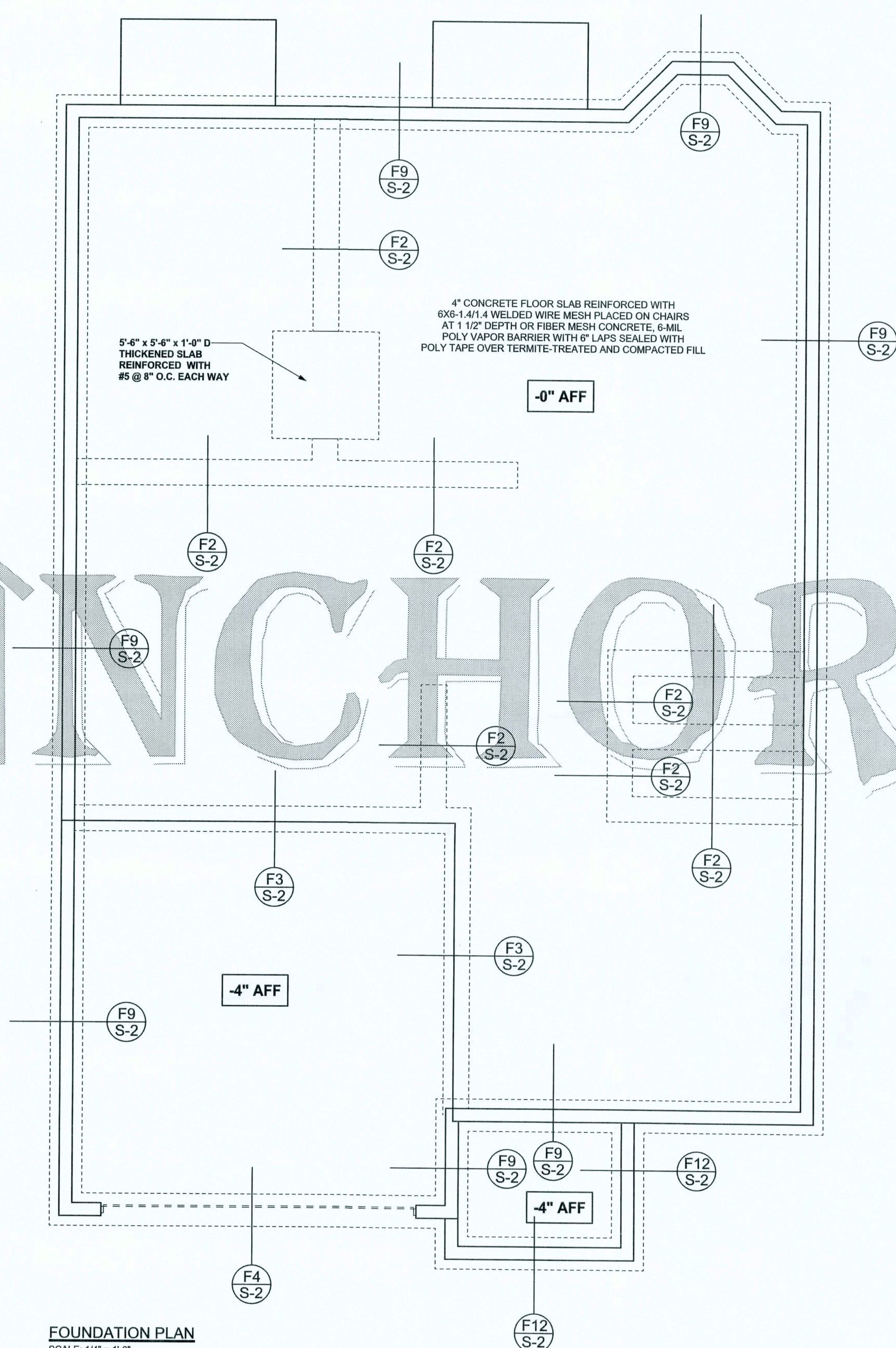
F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

F6 S-2 TYPICAL NON-BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, with 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall, 12" CMU may be used with reinforcement as shown in the table below.

| STEM WALL HEIGHT (FEET) | UNBALANCED BACKFILL HEIGHT | VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.) | | | VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.) | | |
|-------------------------|----------------------------|---|----|----|--|----|----|
| | | #5 | #7 | #8 | #5 | #7 | #8 |
| 3.3 | 3.0 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.0 | 3.7 | 96 | 96 | 96 | 96 | 96 | 96 |
| 4.7 | 4.3 | 88 | 96 | 96 | 96 | 96 | 96 |
| 5.3 | 5.0 | 56 | 96 | 96 | 96 | 96 | 96 |
| 6.0 | 5.7 | 40 | 80 | 96 | 80 | 96 | 96 |
| 6.7 | 6.3 | 32 | 56 | 80 | 56 | 96 | 96 |
| 7.3 | 7.0 | 24 | 40 | 56 | 40 | 80 | 96 |
| 8.0 | 7.7 | 16 | 32 | 48 | 32 | 64 | 80 |
| 8.7 | 8.3 | 8 | 24 | 32 | 24 | 48 | 64 |
| 9.3 | 9.0 | 8 | 16 | 24 | 16 | 40 | 48 |



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53815, P.O. Box 868, Lake City, FL 32065, 386-754-4419
DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F501.2.1, Florida building code residential 2004, to the best of my knowledge.
LIMITATION: This design is valid for one building, at specified location.
MARK DISOWAY
P.E. 53815
Mark Disoway
12/Dec/07

Lipscomb Eagle Development

Mediterranean Model

ADDRESS:
Lot 115 Preserve S/D
Columbia County, Florida
Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

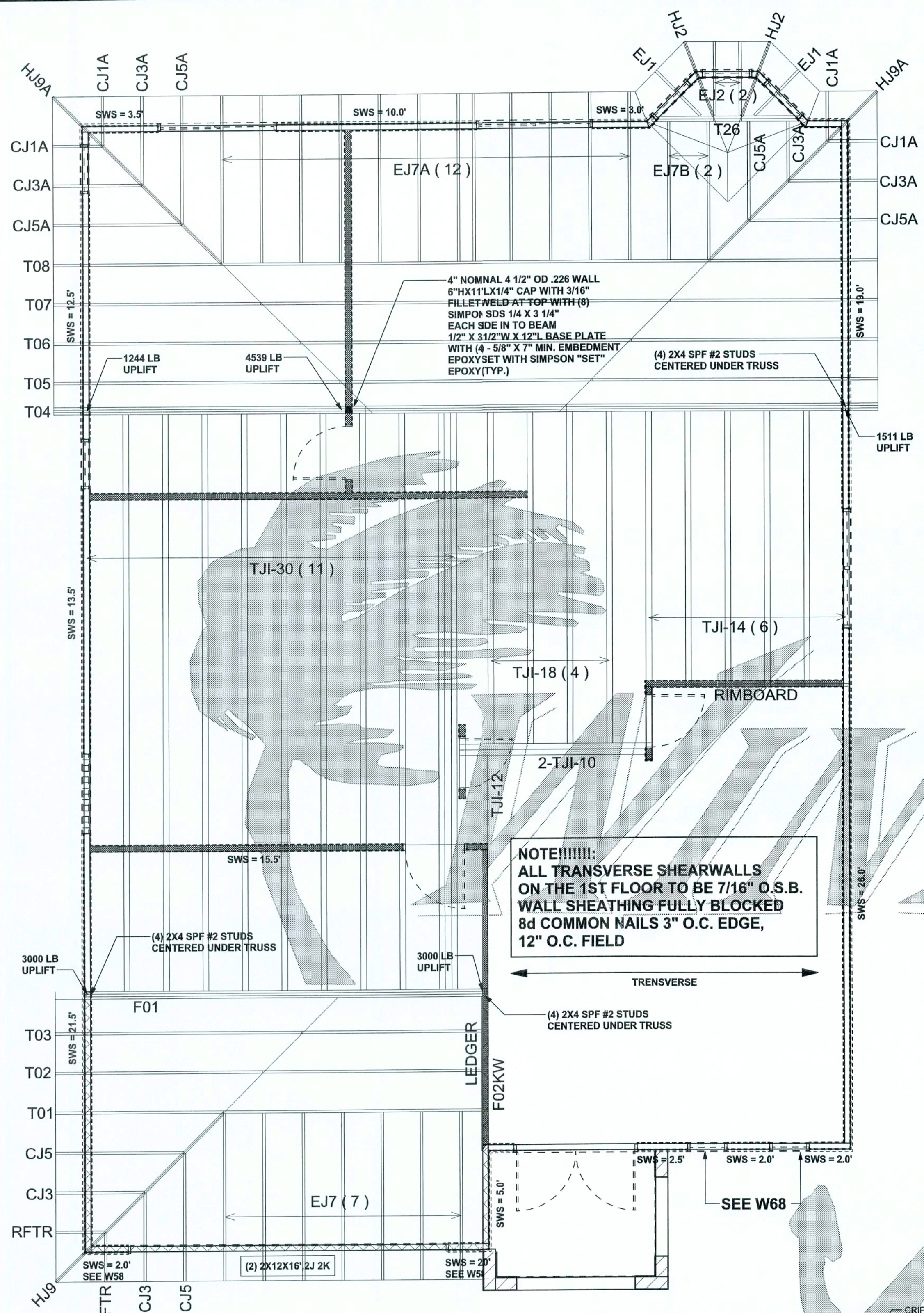
PRINTED DATE:
December 14, 2007
DRAWN BY:
David Disoway
CHECKED BY:

FINALS DATE:
14 / Dec / 07

JOB NUMBER:
711296

DRAWING NUMBER
S-2

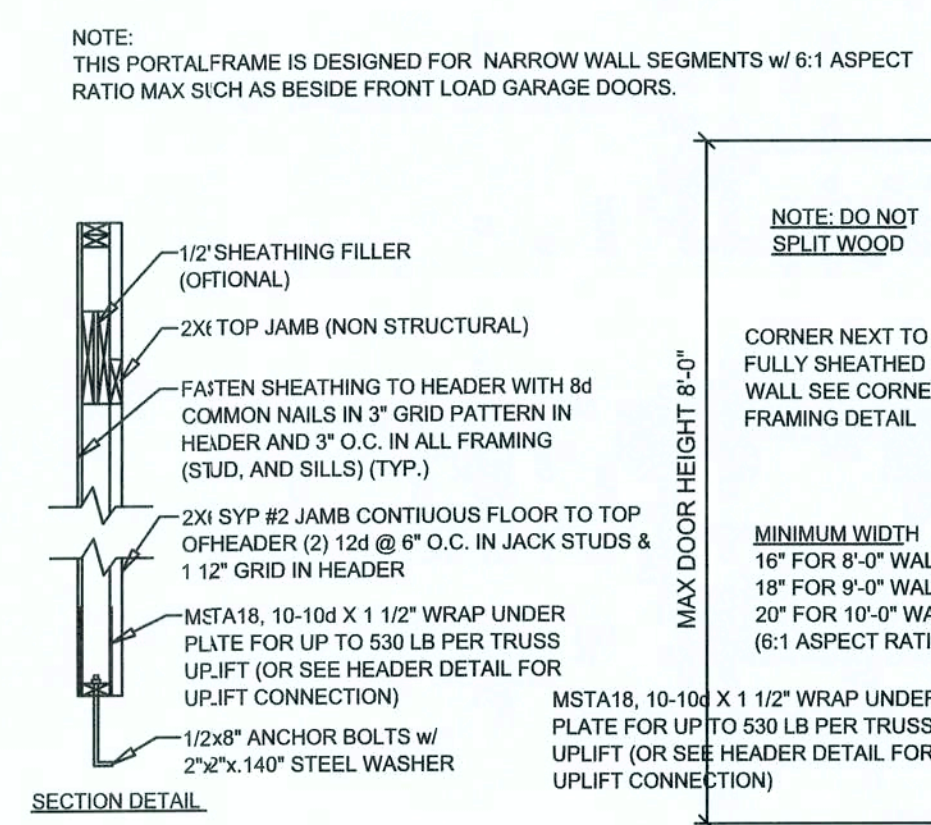
OF 3 SHEETS



STRUCTURAL FLOOR PLAN
SCALE: 1/4" = 1'-0"

1ST FLOOR TOTAL SHEAR WALL SEGMENTS
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE | 37.9' | 42.5' |
| LONGITUDINAL | 42.0' | 97.5' |



W58 - PORTAL FRAME SHEARWALL DETAIL
SCALE: 1/2" = 1'-0"

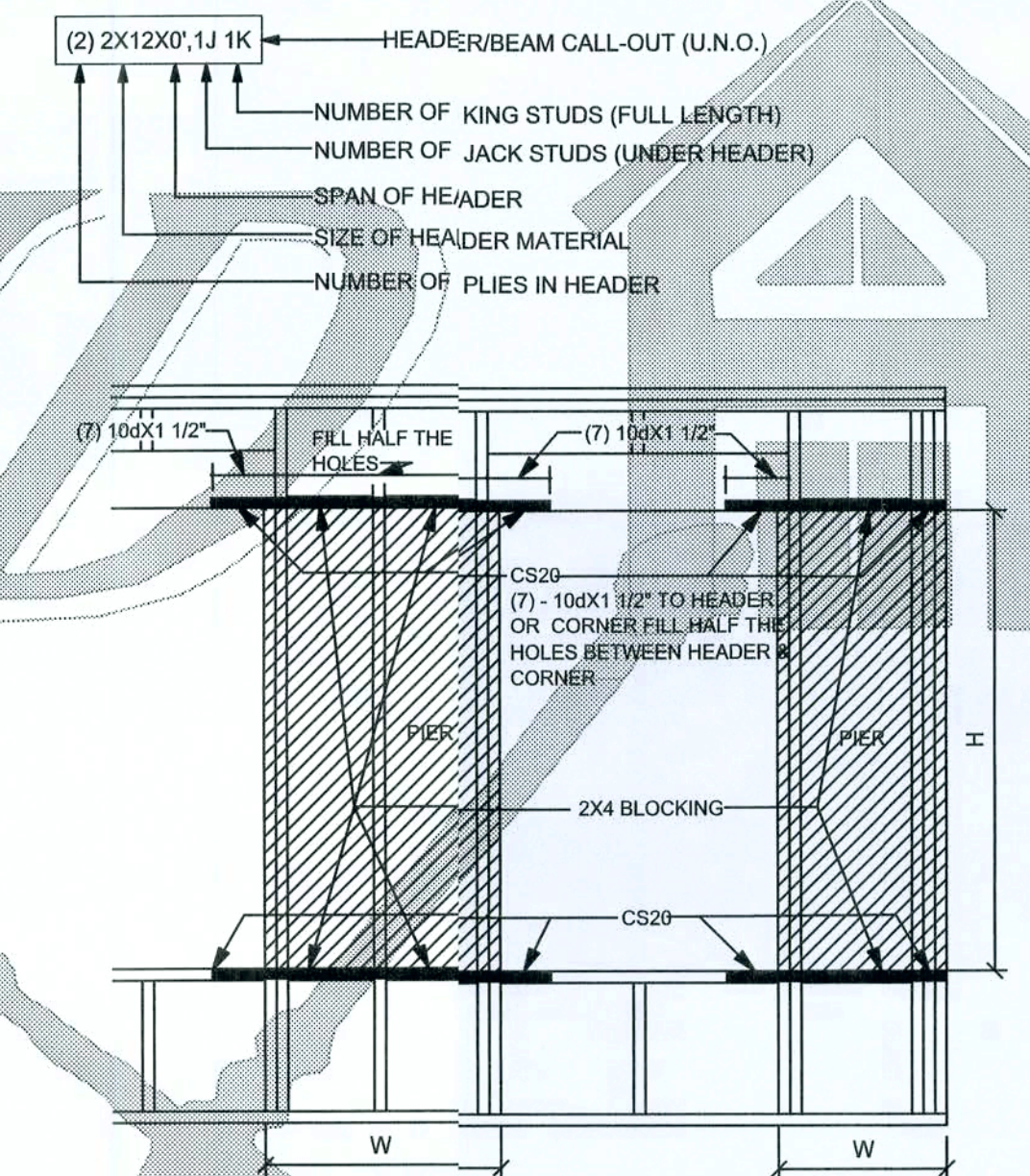
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

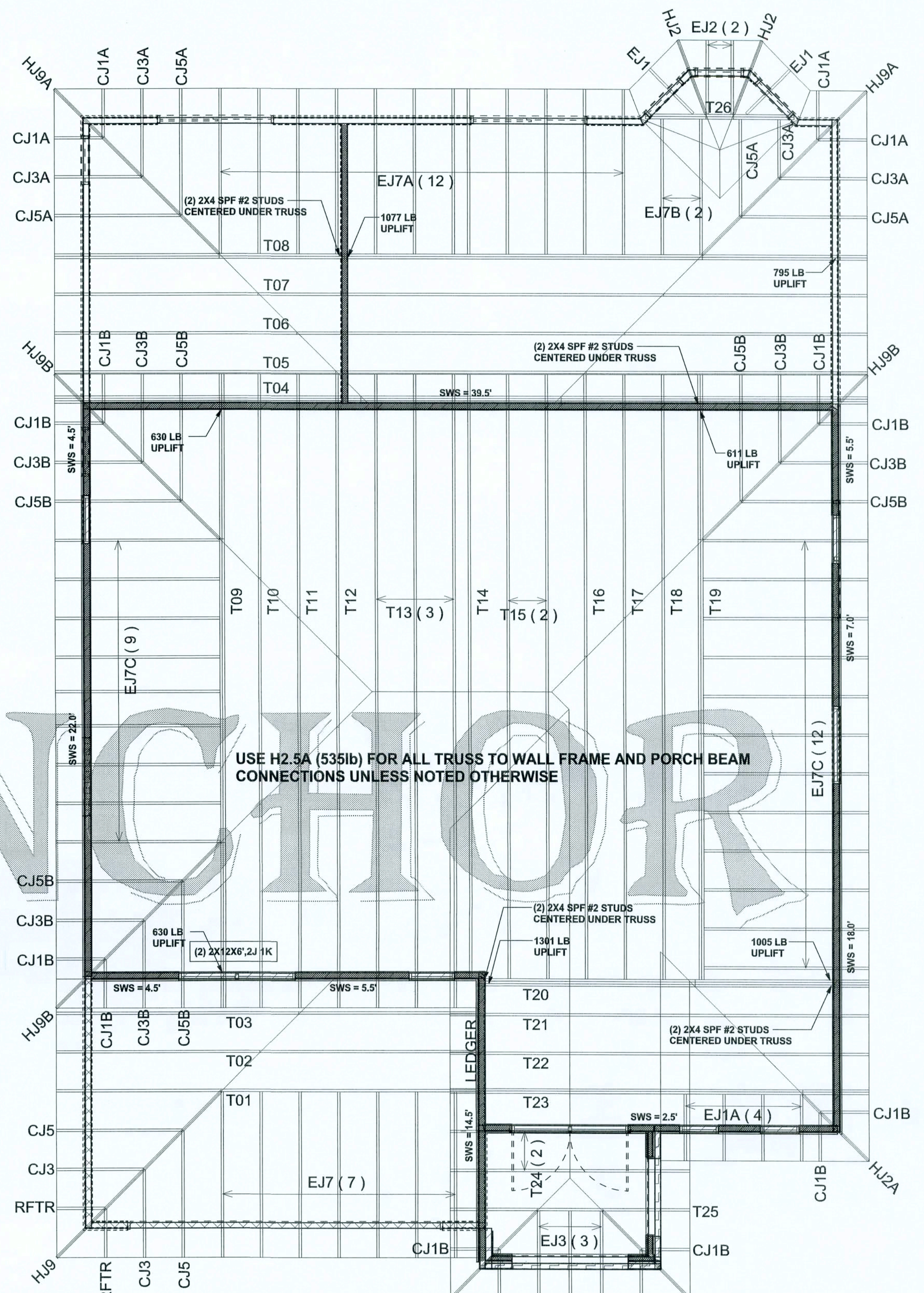
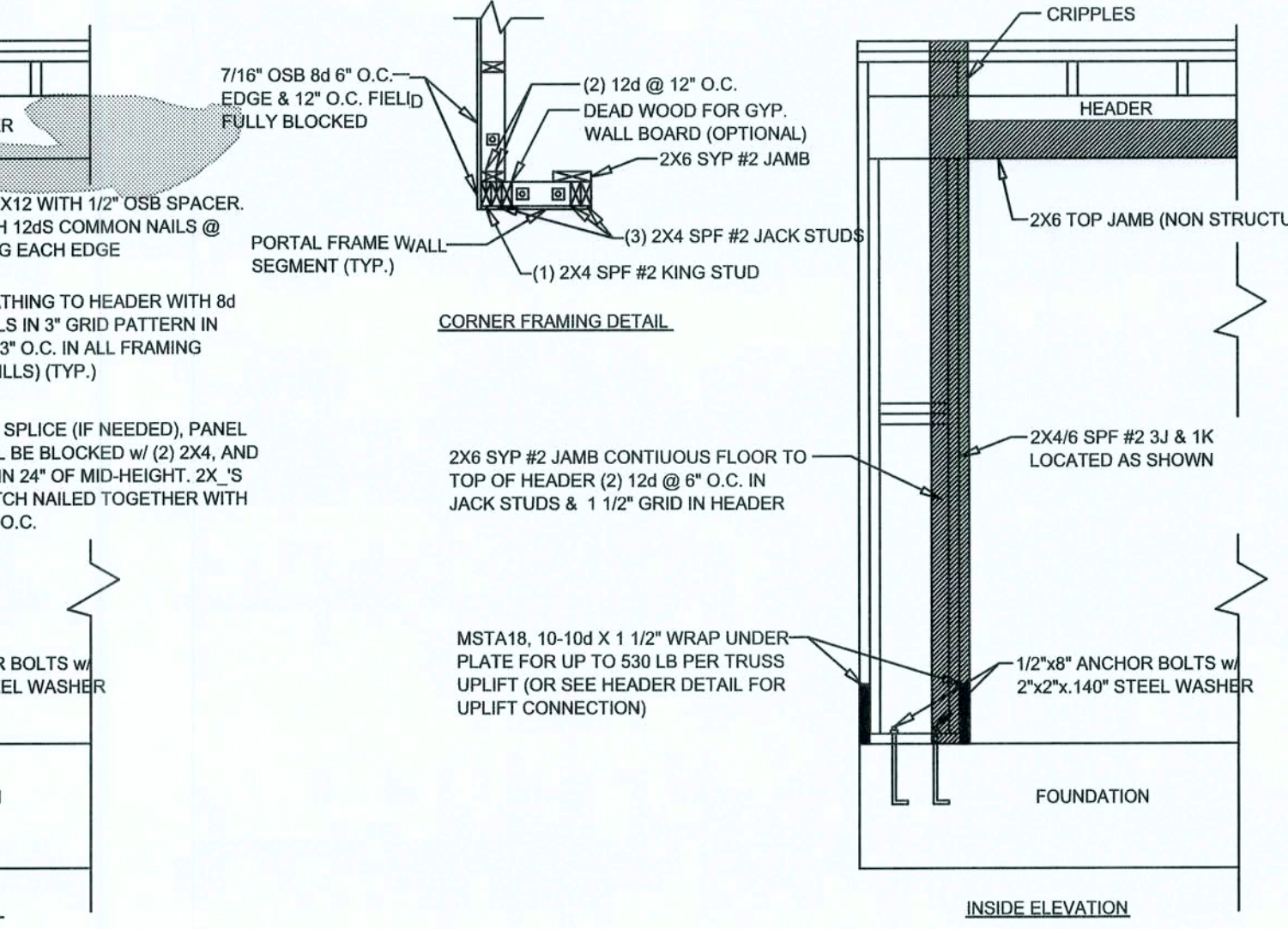
| | |
|------------|---------------------------------|
| SWS = 0.0' | 1ST FLOOR EXTERIOR WALL |
| SWS = 0.0' | 2ND FLOOR EXTERIOR WALL |
| IBW | 1ST FLOOR INTERIOR BEARING WALL |
| IBW | 2ND FLOOR INTERIOR BEARING WALL |

HEADER LEGEND



NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER RESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT HW < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGH OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

W68 - OPENING FORCE TRANSFER
SCALE: 1/2" = 1'-0"



STRUCTURAL ROOF PLAN
SCALE: 1/4" = 1'-0"

2ND FLOOR TOTAL SHEAR WALL SEGMENTS
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

| | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE | 26.8' | 52.0' |
| LONGITUDINAL | 15.4' | 71.5' |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L235554

REVISIONS

| NO. | DESCRIPTION |
|-----|-------------|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
12/14/07
SEAL

Lipscomb Eagle Development

Mediterranean Model

ADDRESS: Lot 115 Preserve S/D Columbia County, Florida

Mark Disoway P.E.
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Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: December 14, 2007

DRAWN BY: David Disoway **CHECKED BY:**

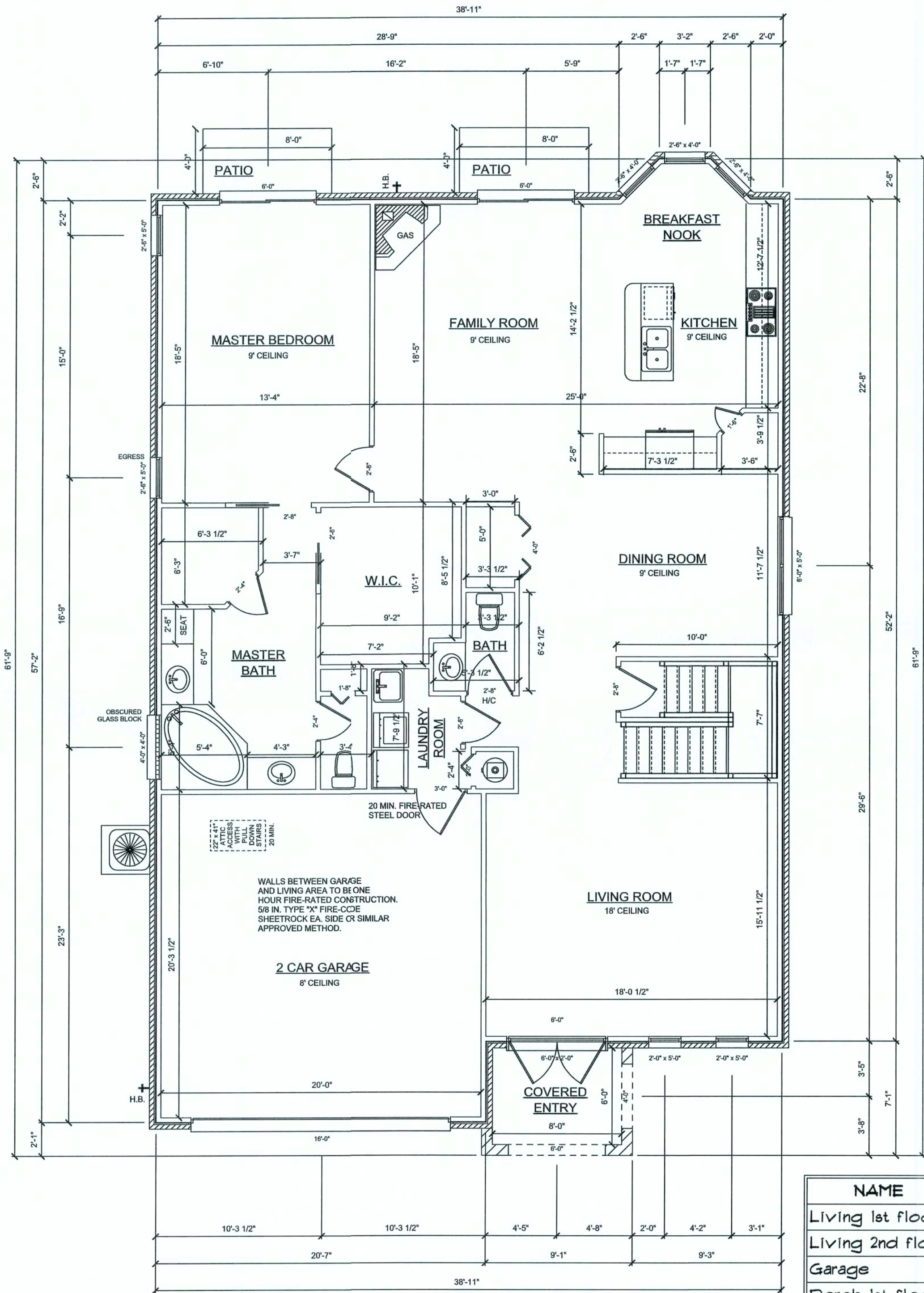
FINALS DATE: 14 / Dec / 07

JOB NUMBER: 711296

DRAWING NUMBER
S-3
OF 3 SHEETS

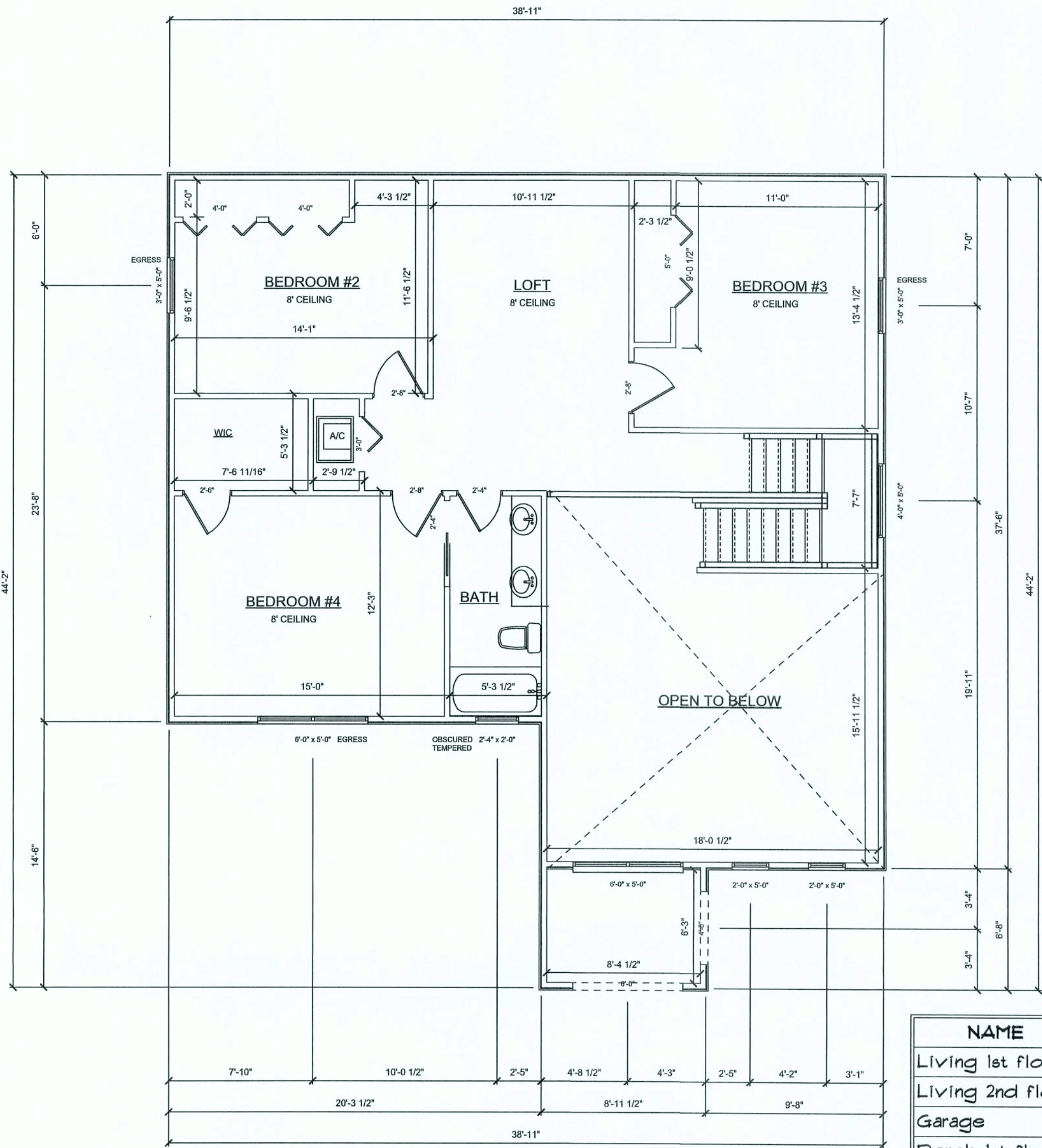
| REVISIONS | |
|-----------|--|
| | |
| | |
| | |

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



1ST FLOOR PLAN

| NAME | AREA |
|------------------|-----------|
| Living 1st floor | 1791 S.F. |
| Living 2nd floor | 950 S.F. |
| Garage | 435 S.F. |
| Porch 1st floor | 64 S.F. |
| Porch 2nd floor | 64 S.F. |
| Total | 3310 S.F. |



2ND FLOOR PLAN

| NAME | AREA |
|------------------|-----------|
| Living 1st floor | 1791 S.F. |
| Living 2nd floor | 950 S.F. |
| Garage | 435 S.F. |
| Porch 1st floor | 64 S.F. |
| Porch 2nd floor | 64 S.F. |
| Total | 3310 S.F. |

Addendum to permit # 0712-55

Lipscomb Eagle
Development, Inc.

Mediterranean Model

ADDRESS:
Lot 115 Preserve S/D
Columbia County, Florida

Lipscomb Eagle
Development, Inc.
872 SW Jaguar Drive
Lake City, Florida 32025
Phone: (386) 719 - 6960
Fax: (386) 719 - 9586

PRINTED DATE:
January 07, 2008
DRAWN BY: TJ Canton
CHECKED BY: SUSAN HOLTON
DESIGNED BY:

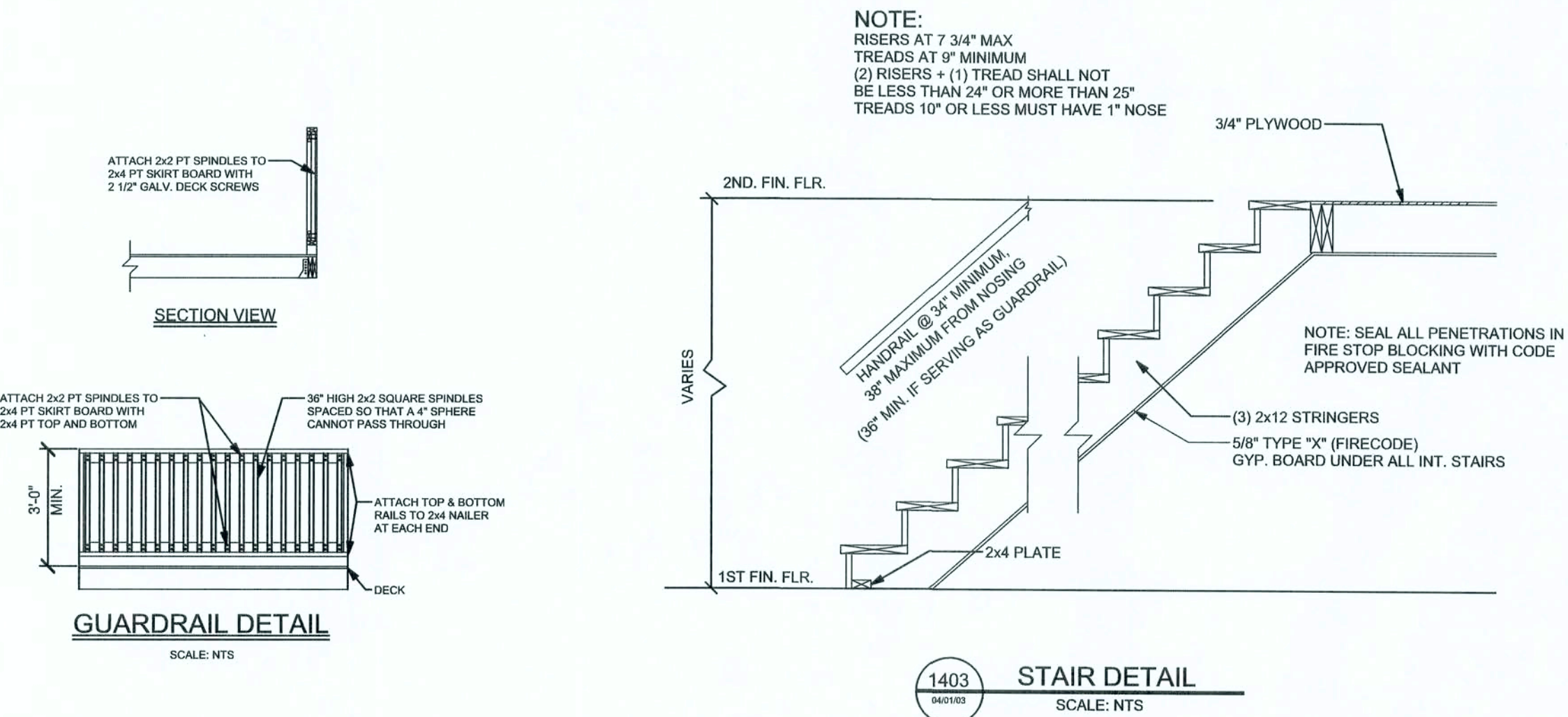
FINALS DATE:
14 / Dec / 07

JOB NUMBER:

DRAWING NUMBER

A-2

2 OF 3 SHEETS



NOTE:
RISERS AT 7 3/4" MAX
TREADS AT 9" MINIMUM
(2) RISERS + (1) TREAD SHALL NOT
BE LESS THAN 24" OR MORE THAN 25"
TREADS 10" OR LESS MUST HAVE 1" NOSE

ATTACH 2x2 FT SPINDLES TO
2x4 FT SHORT BOARD WITH
2 1/2" GALV. DECK SCREWS

SECTION VIEW

ATTACH 2x2 FT SPINDLES TO
2x4 FT SHORT BOARD WITH
2x4 FT TOP AND BOTTOM

38" HIGH 2x2 SQUARE SPINDLES
SPACED SO THAT A 4" SPHERE
CANNOT PASS THROUGH

ATTACH TOP & BOTTOM
RAILS TO 2x4 NAILED
AT EACH END

GUARDRAIL DETAIL

SCALE: NTS

2ND. FIN. FLR.

VARIES

1ST FIN. FLR.

3/4" PLYWOOD

NOTE: SEAL ALL PENETRATIONS IN
FIRE STOP BLOCKING WITH CODE
APPROVED SEALANT

(3) 2x12 STRINGERS

5/8" TYPE "X" (FIRECODE)
GYP. BOARD UNDER ALL INT. STAIRS

2x4 PLATE

STAIR DETAIL

SCALE: NTS