DATE 03/18/2013

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000030852

APPLICANT REBECCA THOMAS PHONE 386.623.5079

ADDRESS 547 SW DYAL AVENUE LAKE CITY FL 32024

OWNER MIKE ROBERTS PHONE 386.755.9476

ADDRESS 524 SW GERALD CONNER DRIVE LAKE CITY FL 32024

CONTRACTOR REBECCA THOMAS PHONE 386.623.5079

LOCATION OF PROPERTY 90-W TO C-341,TL TO KICKLIGHTER,L TO GERALD CONNER,TR AND IT'S 1/2 MILE ON R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 106850.00

HEATED FLOOR AREA 1495.00 TOTAL AREA 2137.00 HEIGHT 17.70 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 23-4S-16-03095-106 SUBDIVISION CANNON CREEK PLACE

LOT 6 BLOCK PHASE UNIT 2 TOTAL ACRES 0.51

000001995		CBC1256094	Rebecca Thomas	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PWD	1300-73	BLK	TC	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: 6" ABOVE ROAD. ELEVATION DETERMINATION LETTER RECEIVED. NOC ON FILE.

Check # or Cash 2744

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 535.00 CERTIFICATION FEE $ 10.69 SURCHARGE FEE $ 10.69

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 656.38

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY.

NOTICE: ALL OTHER APPLICABLE STATE OR FEDERAL PERMITS SHALL BE OBTAINED BEFORE COMMENCEMENT OF THIS PERMITTED DEVELOPMENT.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Prepared by:
Abstract Trust Title, LLC
PO Box 7175
Lake City, FL 32055

Inst:201312001838 Date:2/5/2013 Time:2:39 PM
Doc Stamp-Deed:73.50
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1249 P:274

ATS# 4-5498

Warranty Deed

Individual to Individual

THIS WARRANTY DEED made the 5th day of February, 2013, Peter W. Giebeig, .A Single Person, hereinafter called the grantor, to Mike Roberts whose post office address is: 657 SW Catherine Lane, Lake City, FL 32025 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of $10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, Florida, viz: Parcel ID# R03095-106

Lot 6, Cannon Creek Place, Unit 2, a subdivision according to the plat thereof as recorded in Plat Book 8, Pages 130-131, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2012.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Witness:

Printed Name: TRACI LANDRY

Witness:

Printed Name: Michael Harrell

Peter W. Giebeig

STATE OF FLORIDA

COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 5th day of February, 2013 by PETER W. GIEBEIG, A Single Person personally known to me or, if not personally known to me, who produced _____ for identification and who did not take an oath.

Notary Public

(Notary Seal)

MICHAEL H. HARRELL
Notary Public, State of Florida
My Comm. Expires April 8, 2013
Commission No. DD 869555

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 23-4S-16-03095-106 **Building permit No.** 000030852

Use Classification SFD/UTILITY **Fire:** 183.32

Permit Holder REBECCA THOMAS **Waste:** 193.00

Owner of Building MIKE ROBERTS **Total:** 376.32

Location: 524 SW GERALD CONNER DR, LAKE CITY, FL 32024

Date: 10/16/2013

Troy Cu

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

New Construction Subterranean Termite Service Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company. 30552

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential, therefore, no assurance of confidentiality is provided.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Company and builder, unless stated otherwise.

Section 1: General Information (Pest Control Company Information)

Company Name Aspen Pest Control, inc.
Company Address P.O. Box 1795 City Lake City State FL Zip 32056
Company Business License No. JB102948 Company Phone No. 386-755-3511
FHA/VA Case No. (if any) _____

Section 2: Builder Information

Company Name _____ Phone No. _____

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) _____

Section 4: Service Information

Date(s) of Service(s) 3-5-15
Type of Construction (More than one box may be checked) ☐ Slab ☐ Basement ☐ Crawl ☐ Other _____

Check all that apply:

☐ A. Soil Applied Liquid Termiticide
Brand Name of Termiticide: Bifen IT EPA Registration No. 279-3206
Approx. Dilution (%): 0.06 Approx. Total Gallons Mix Applied: 40 Treatment completed on exterior: ☐ Yes ☐ No

☐ B. Wood Applied Liquid Termiticide
Brand Name of Termiticide: _____ EPA Registration No. _____
Approx. Dilution (%): _____ Approx. Total Gallons Mix Applied: _____

☐ C. Bait System Installed
Name of System _____ EPA Registration No. _____ Number of Stations Installed _____

☐ D. Physical Barrier System Installed
Name of System _____ Attach installation information (required)

Service Agreement Available? ☐ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List) _____

Comments _____

Name of Applicator(s) Crosby Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All materials and methods used comply with state and federal regulations.

Authorized Signature _____ Date 3-5-2015

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

30852

Lake City • (386) 755-3633
Fax • (386) 752-5456

Jacksonville • (904) 381-8901
Fax • (904) 381-8902

Cal-Tech Testing, Inc. — FOUNDATIONS • ASPHALT • SOILS • CONCRETE — ENGINEERING • GEOTECHNICAL • ENVIRONMENTAL

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
450 SR. 13 N.• Suite 206-308 • Jacksonville, FL 32259

JOB NO.:
DATE TESTED: 2-28-13

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ (D-2922) Nuclear _____ (D-2937) Drive Cylinder _____ Other

PROJECT: Cayman Creek Place Lot #6

CLIENT: Mike Roberts Framing

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): 1 SPECIFICATION REQUIREMENTS: 95 %

TECHNICIAN: C. Day

MODIFIED (ASTM D-1557): ✓ STANDARD (ASTM D-698): _____

TEST NO.	TEST LOCATION	TEST: ___DEPTH ___ELEV. ___LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
1	S.E. Corner of pad 10' west & 10' north	12"	1	106.1	99.9	6.3	96
2	Approx. center of pad	12"	1	112.6	105.0	7.3	101
3	N.W. corner of pad 12' east & 12' South	12"	1	108.9	101.3	7.5	97

REMARKS: _____

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
1	Tannish Brown Sand	104.0	
	Richland Sand Pit		

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

CH# 2749 - ROBERTS . EX CARD
Thomas . W/C OR EXEMPTION
JORDAN . LIAB

Columbia County Building Permit Application

For Office Use Only Application # 1302-40 Date Received 2/25 By W Permit # 1995/30851
Zoning Official BLK Date 13 MARCH 2013 Flood Zone X Land Use Res. Low Den Zoning RSF-2
FEMA Map # N/A Elevation N/A MFE 6" above Rd River N/A Plans Examiner T.C. Date 3-13-13
Comments Elevation Determination Letter Received
☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan □ State Road Info ☑ Well letter ☑ 911 Sheet □ Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Auth. from Contractor □ F W Comp. letter
IMPACT FEES: EMS Fire Corr ☑ Sub VF Form
Road/Code School = TOTAL (Suspended) □ Ellisville Water ☑ App Fee Paid

Septic Permit No. 130073 Fax 386-755-1933

Name Authorized Person Signing Permit Rebecca Thomas Phone 386-623-5079

Address 547 SW Dyal Avenue Lake City FL 32024

Owners Name Mike Roberts Phone 386-755-9476

911 Address 524 SW Gerald Conner Drive, L.C. FL 32024

Contractors Name Rebecca Thomas Phone 386-623-5079

Address 547 SW Dyal Avenue, L. C. FL 32024

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Mark Disosway

Mortgage Lenders Name & Address NA

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 24-4S-16 R03095-106 Estimated Cost of Construction $100,000

Subdivision Name Cannon Creek Place Lot 6 Block Unit 2 Phase

Driving Directions Hwy 90 to Sisters Welcome Rd. Turn Left onto Kicklighter Rd. 1 1/2 miles to Cannon Creek Place Sub. turn Right onto Gerald Conner 1/2 mile on Right. Number of Existing Dwellings on Property 0

Construction of Residential - "SFD" Total Acreage .51 AC Lot Size 116' x 190'

Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive Total Building Height 17' 7"

Actual Distance of Structure from Property Lines - Front 27' Side 25' Side 41' Rear 110'

Number of Stories 1 Heated Floor Area 1495 Total Floor Area 2137 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. **CODE:** Florida Building Code 2010 and the 2008 National Electrical Code.

TW = left page for return call. 3.13.13
- Rebecca returned call 3.15.13 -

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

(Owners Must Sign All Applications Before Permit Issuance.)

Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Rebecca Thomas
Contractor's Signature (Permitee)

Contractor's License Number CBC1256094
Columbia County
Competency Card Number 1173

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 25th day of February 2013.

Personally known ✓ or Produced Identification

La Hodson
State of Florida Notary Signature (For the Contractor)

SEAL: LAURIE HODSON
MY COMMISSION # EE 214728
EXPIRES: July 14, 2016
Bonded Thru Notary Public Underwriters

(4) 2

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1302-40 CONTRACTOR Rebecca Thomas PHONE 623.5079

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 76	Print Name MATTHEWS ELECTRIC INC License #: ER-0014352	Signature Phone #: 386-344-2029
MECHANICAL/ A/C 802	Print Name Wilson Heating & Air License #: CAC057886	Clinton Wilson (386)
PLUMBING/ GAS 298	Print Name HometTown Plumbing License #: RF11067418	Signature Phone #: 386-754-6140
ROOFING 1173	Print Name Thomas Construction License #: CBC1256094	Signature Rebecca Thomas Phone #: 386 623 5079
SHEET METAL	Print Name N-A License #:	Signature Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name N-A License#:	Signature Phone #:
SOLAR	Print Name N-A License #:	Signature Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	N-A		
CONCRETE FINISHER	000218	TONY E JORDAN SR	Tony E Jordan Sr
FRAMING	000101	Mike Roberts	Mike Roberts
INSULATION 793		Spicer Insulation LLC	Brian Spicer
STUCCO	no	—	—
DRYWALL	no	—	—
PLASTER 1173	CBC1256094	Thomas Construction	Rebecca Thomas
CABINET INSTALLER 1173	CBC1256094	Thomas Construction	Rebecca Thomas
PAINTING	000848	Mike Roberts	Mike Roberts
ACOUSTICAL CEILING	no	—	—
GLASS	no	—	—
CERAMIC TILE	000849	Mike Roberts	Mike Roberts
FLOOR COVERING	—	—	—
ALUM/VINYL SIDING	no		
GARAGE DOOR	000619	Lake City Glass - Carl Bullard JR	Carl Bullard Jr
METAL BLDG ERECTOR	no		

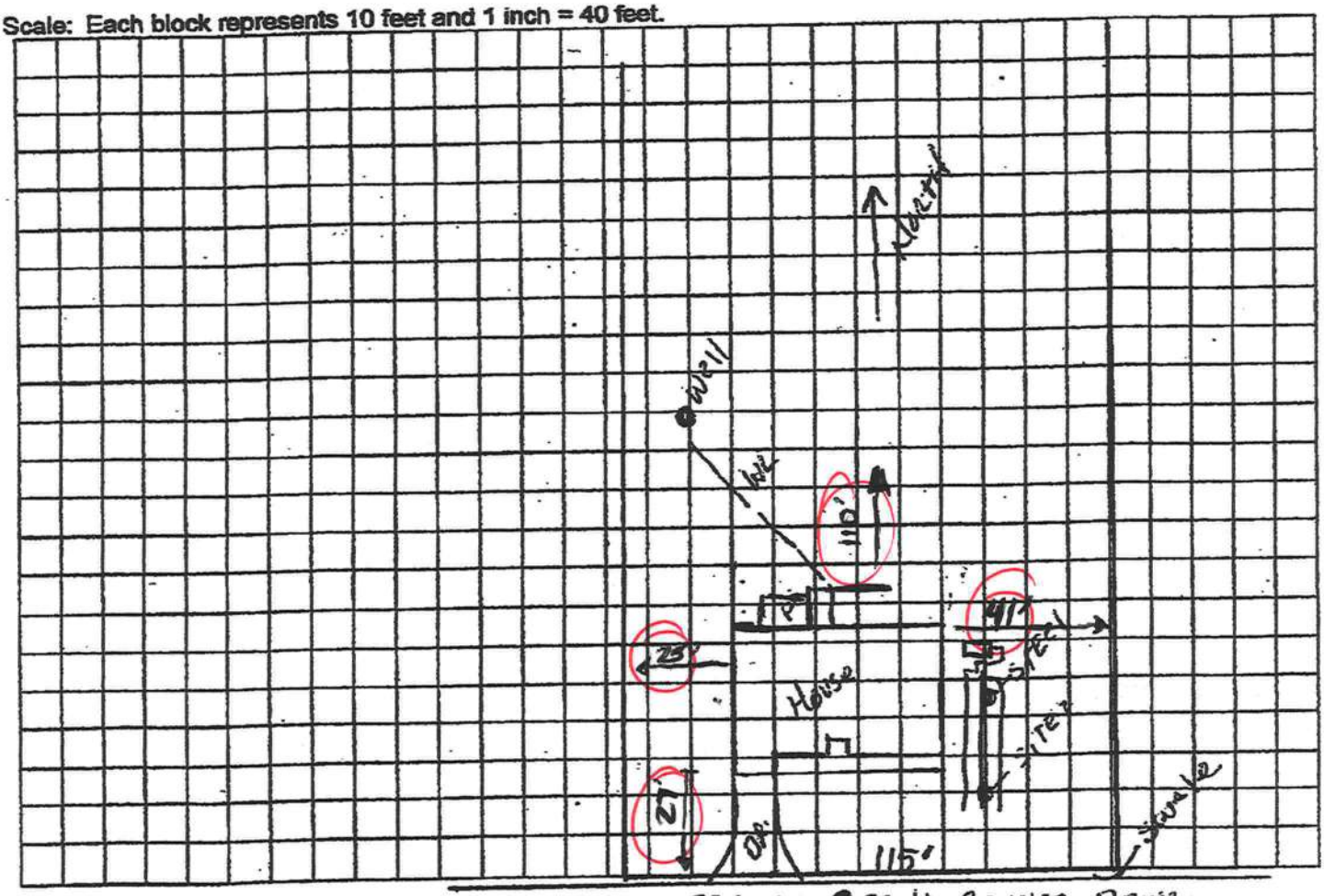
F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number_____

---------- PART II - SITEPLAN ----------

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes: 524 SW Emerald Corner Drive.
BM.
Mike Roberts
Lot 6 Cannon Creek Dr. Unit 2
23-4S-16-03095-016

Site Plan submitted by: Robert W Jack 2-11-13

Plan Approved_____ Not Approved_____ Date_____

By_____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 13-0073
DATE PAID: 2/11/13
FEE PAID: 310.00
RECEIPT #: 1097156

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Pete Giebler (mike Roberts)

AGENT: Robert Ford NEST INC TELEPHONE: 755-6372

MAILING ADDRESS: 580 NW Gverdon Rd LC Fla 32055

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 6 BLOCK/PH: 2 SUBDIVISION: CANNON CREEK P/I PLATTED: 2006

PROPERTY ID #: 23-4S-16-03095-106 ZONING: SF I/M OR EQUIVALENT: [Y (N)

PROPERTY SIZE: 0.510 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y /(N)] DISTANCE TO SEWER: NA FT

PROPERTY ADDRESS: 524 SW Gerald Connor DR.

DIRECTIONS TO PROPERTY: Hwy 90 West to Sister Welcome TL Follow to Kicklighter TL Follow to SW Cannon Creek DR. TL Turn Right on Gerald Connor Follow to Prop on Right

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	SF Res	3	1495	
2				
3				
4				

[ ] Floor/Equipment Drains [ ] Other (Specify) _____

SIGNATURE: Robert W Ford DATE: 2-11-13

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

:2-08-11:03:25PM:

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
CONSTRUCTION PERMIT

PERMIT NO. 13-0873
DATE PAID: 2/11/13
FEE PAID: 394.00
RECEIPT #: 1099156

CONSTRUCTION PERMIT FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Peter Giebick (mike Roberts)

PROPERTY ADDRESS: 524 SW Gerald Conner DR.

LOT: 6 BLOCK: PH 2 SUBDIVISION: Cannon Creek Place
[SECTION, TOWNSHIP, RANGE, PARCEL NUMBER]

PROPERTY ID #: 03095-106 [OR TAX ID NUMBER]

SYSTEM MUST BE CONSTRUCTED IN ACCORDANCE WITH SPECIFICATIONS AND STANDARDS OF SECTION 381.0065, F.S., AND CHAPTER 64E-6, F.A.C. DEPARTMENT APPROVAL OF SYSTEM DOES NOT GUARANTEE SATISFACTORY PERFORMANCE FOR ANY SPECIFIC PERIOD OF TIME. ANY CHANGE IN MATERIAL FACTS, WHICH SERVED AS A BASIS FOR ISSUANCE OF THIS PERMIT, REQUIRE THE APPLICANT TO REVISE THE PERMIT APPLICATION. SUCH MODIFICATIONS MAY RESULT IN THIS PERMIT BEING MADE NULL AND VOID. ISSUANCE OF THIS PERMIT DOES NOT EXEMPT THE APPLICANT FROM COMPLIANCE WITH OTHER FEDERAL, STATE, OR LOCAL PERMITTING REQUIRED FOR DEVELOPMENT OF THIS PROPERTY.

SYSTEM DESIGN AND SPECIFICATIONS

T [900] GALLONS / CPD SEPTIC TANK/AEROBIC UNIT CAPACITY MULTI-CHAMBERED/IN-SERIES [ ]
A [ ] GALLONS / CPD _____ CAPACITY MULTI-CHAMBERED/IN-SERIES [ ]
N [ ] GALLONS GREASE INTERCEPTOR CAPACITY [MINIMUM CAPACITY SINGLE TANK: 1250 GALLONS]
K [ ] GALLONS DOSING TANK CAPACITY [ ] GALLONS @ [ ] DOSES PER 24 HRS # PUMPS [ ]

D [375] SQUARE FEET PRIMARY DRAINFIELD SYSTEM
R [ ] SQUARE FEET _____ SYSTEM
A TYPE SYSTEM: [X] STANDARD [ ] FILLED [ ] MOUND [ ] _____
I CONFIGURATION: [X] TRENCH [ ] BED [ ] _____
N
F LOCATION OF BENCHMARK: Painted Spot Center of Road
I ELEVATION OF PROPOSED SYSTEM SITE [5] [INCHES/FT] [ABOVE/BELOW] BENCHMARK/REFERENCE POINT
E BOTTOM OF DRAINFIELD TO BE [35] [INCHES/FT] [ABOVE/BELOW] BENCHMARK/REFERENCE POINT
L
D FILL REQUIRED: [0] INCHES EXCAVATION REQUIRED: [18] INCHES
O MUST Remove Top 18" of SL Fill and Replace
T
E
E
R

SPECIFICATIONS BY: Robert W Fuller TITLE: Master

APPROVED BY: Willie Ford TITLE: Env Health Director Columbia CHD

DATE ISSUED: 2-20-13 EXPIRATION DATE: 8-20-14

DH 4016, 08/09 (Obsoletes all previous editions which may not be used)
Incorporated: 64E-6.003, FAC

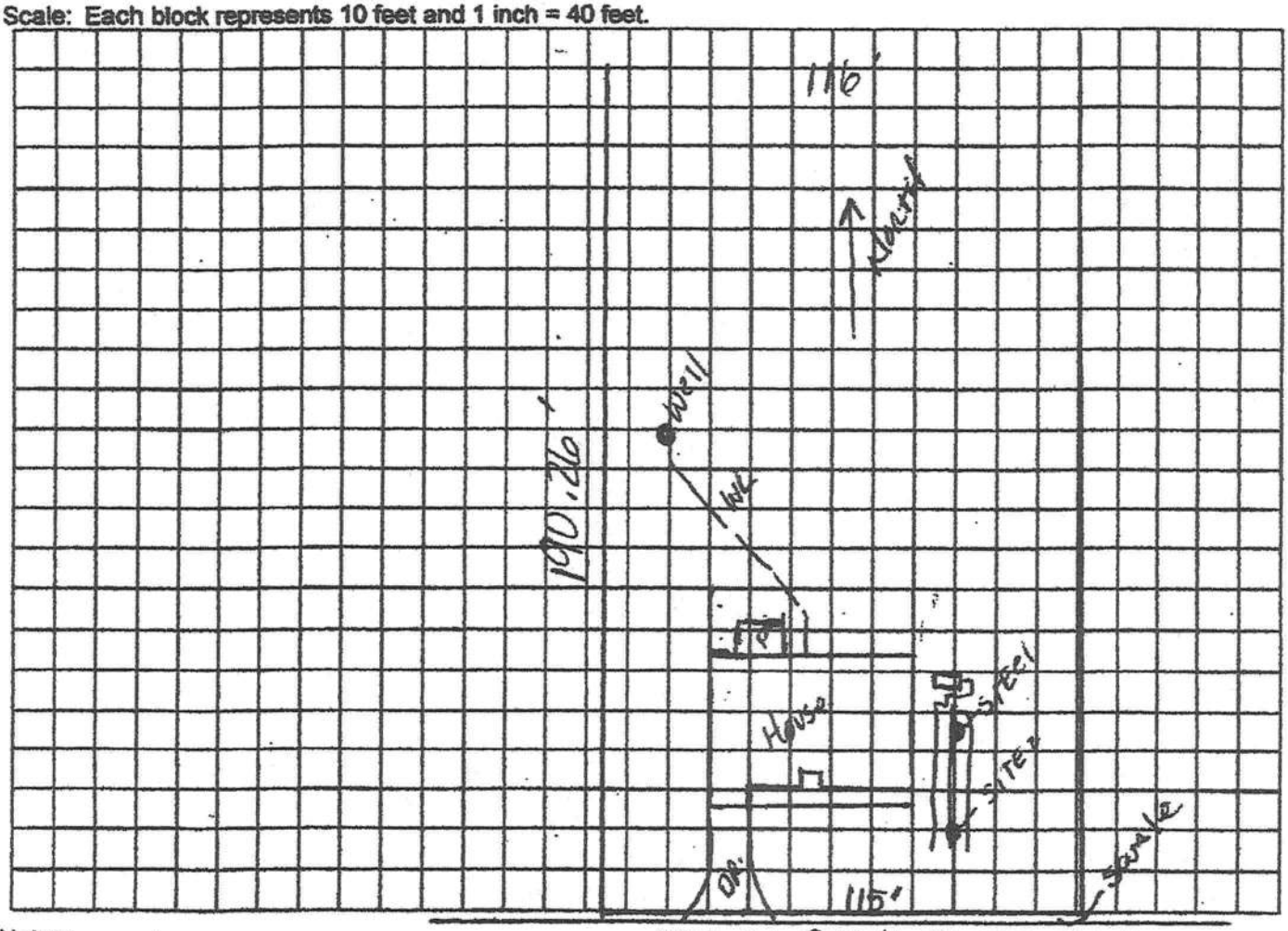
SE

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number 13-0073

---------- PART II - SITEPLAN ----------

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes: 524 SW Emerald Conner Drive.

Mike Roberts

Lot 6 Cannon Creek Dr. U.U.T.Z

23-4S-16-03095-016

Site Plan submitted by: Robert W Ford 2-11-13 Agent

Plan Approved ✓ Not Approved ____ Date 2-20-13

By Sallie Ford Env Health Director Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 2/5/2013 **DATE ISSUED:** 2/6/2013

ENHANCED 9-1-1 ADDRESS:

524 SW GERALD CONNER DR

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

23-4S-16-03095-106

Remarks:

ADDRESS FOR PROPOSED STRUCTURE ON PARCEL.

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

2458

Elevation Determination Letter

CES
Crews Engineering Services, LLC

Crews Engineering Services, LLC
P.O. Box 970
Lake City, FL 32056
(Phone) 386.623.4303
brett@crewsengineeringservices.com

Finish Floor Elevation Recommendation

For: Mike Roberts

Parcel Number: 23-4S-16-03095-106

Description of Project:

The project is located on 0.5 acres off of SW Gerald Conner Dr in Columbia County, FL (Lot 6, Cannon Creek Place, Unit 2). The owner is proposing to construct a site built home +/-50 feet from SW Gerald Conner Dr.

Existing Conditions:

A site visit was conducted on 02-05-2013 to the project site.

The topography of the property is such that the land slopes away from toward SW Gerald Conner Dr. and eventually to an existing retention pond to the west.

Recommendation:

Although the proposed building location is lower than the nearest road, there is adequate drainage to avoid damage from storm water runoff if the finish floor is not set one foot above.

The proposed building shall be protected from storm water runoff.

The finish floor elevation of the new building shall be set as discussed at +/-6" in above SW Gerald Conner Dr at the building location). The area around the proposed structure shall be graded such as to convey all storm water runoff away from and around the proposed structure.

The above recommendation is based on the structure's location as described above.

No topographical survey was used.

2-11-2013

Brett A. Crews, P.E. 65592

Suwannee River Water Management District
Effective Flood Information Report

Effective Flood Zones described on Page 2

SFHA - AE w/Floodway

SFHA - Zones AE, AH, AO

SFHA - Zone VE

SFHA - Zone A

0.2 % (shaded X)

Wetlands

FIRM Panel

State Lands

Counties

SRWMD

Parcels

Depressions

BFE

Cross Sections

LOCATION

Date: 02-11-2013

Parcel: 23-4S-16-03095-106

County: Columbia

STR: S023 T04 R16

Columbia Flood Hazard Areas Status: Effective: 02/04/2009

FLOOD INFORMATION

FIRM Panel(s): 12023C0293C

Parcel In Special Flood Hazard Area? (SFHA): No

Flood Zone(s):

1% Annual Chance Flood Elev (BFE): Not Available

Floodway: No

10% Annual Chance Flood Elev: Not Available

50% Annual Chance Flood Elev: Not Available

Note: Elevations are based on NAVD88

The Federal Emergency Management Agency (FEMA) maintains information about map features, such as street locations and names, in or near designated flood hazard areas. The information herein represents the best available data as of the effective date shown. The applicable Flood Insurance Study and a Digital Flood Insurance Rate Map is available online (http://www.srwmdfloodreport.com). To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to also consult the FEMA Map Service Center at 1-800-358-9616 (http://www.msc.fema.gov) for information on available products associated with this FIRM panel. Available products from the Map Service Center may include previously issued Letters of Map Change.

Requests to revise flood information in or near designated flood hazard areas may be provided to FEMA during the community review period on preliminary maps, or through the Letter of Map Change process for effective maps.

Base Flood Elevation (BFE)

The elevation shown on the Flood Insurance Rate Map for Zones AE, AH, A1-A30, AR, AO, V1-V30, and VE that indicates the water surface elevation resulting from a flood that has a one percent chance of equaling or exceeding that level in any given year.

A

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Because detailed analyses are not performed for such areas; no depths or base flood elevations are shown within these zones.

AE, A1-A30

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. In most instances, base flood elevations derived from detailed analyses are shown at selected intervals within these zones.

AH

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Usually areas of ponding with flood depths of 1 to 3 feet. Base Flood Elevations are determined.

AO

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Usually areas of sheet flow on sloping terrain with flood depths of 1 to 3 feet. Base Flood Elevations are determined.

Supplemental Information:

10%-chance flood elevations (10-year flood-risk elevations) and 50%-chance flood elevations (2-year flood-risk elevations), are calculated during detailed flooding studies but are not shown on FEMA Digital Flood Insurance Rate Maps (FIRMs). They have been provided as supplemental information in the Flood Information section of this report.

AE FW (FLOODWAYS)

The channel of a river or other watercourse and the adjacent land areas that must be reserved in order to discharge the base flood (1% annual chance flood event). The floodway must be kept open so that flood water can proceed downstream and not be obstructed or diverted onto other properties.

Please note, if you develop within the regulatory floodway, you will need to contact your Local Government and the Suwannee River Water Management District prior to commencing with the activity. Please contact the District at 800.226.1066.

VE

Areas with a 1% annual chance of flooding over the life of a 30-year mortgage with additional hazards due to storm-induced velocity wave action. Base Flood Elevations (BFEs) derived from detailed analyses.

X 0.2 PCT (X Shaded, 0.2 PCT ANNUAL CHANCE FLOOD HAZARD)

Same as Zone X; however, detailed studies have been performed, and the area has been determined to be within the 0.2 percent annual chance floodplain (also known as the 500-year flood zone). Insurance purchase is not required in this zone but is available at a reduced rate and is recommended.

X

All areas outside the 1-percent annual chance floodplain are Zone X. This includes areas of 1% annual chance sheet flow flooding where average depths are less than 1 foot, areas of 1% annual chance stream flooding where the contributing drainage area is less than 1 square mile, or areas protected from the 1% annual chance flood by levees. No Base Flood Elevations or depths are shown within this zone. Insurance purchase is not required in these zones.

LINKS

FEMA:

http://www.fema.gov

SRWMD:

http://www.srwmd.state.fl.us

CONTACT

SRWMD
9225 County Road 49
Live Oak, FL 32060

(386) 362-1001

Toll Free:
(800) 226-1066

NOTICE OF COMMENCEMENT

Clerk's Office Stamp

Inst 201312002159 Date:2/12/2013 Time:11:24 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1249 P:1181

Tax Parcel Identification Number:

23-45-16-03095-106

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: Lot 6 Cannon Creek Place unit 2
   a) Street *(job)* Address: 524 SW Gerald Conner DR Lake City FL
2. General description of improvements: New Construction
3. Owner Information
   a) Name and address: Mike Roberts
   b) Name and address of fee simple titleholder (if other than owner)
   c) Interest in property 100%
4. Contractor Information
   a) Name and address: Thomas Construction of Lake City
   b) Telephone No.: 623-5084 Fax No. (Opt.)
5. Surety Information
   a) Name and address
   b) Amount of Bond:
   c) Telephone No.: Fax No. (Opt.)
6. Lender
   a) Name and address:
   b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
   a) Name and address:
   b) Telephone No Fax No. (Opt.)
9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified):**

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [signature]
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Mike Roberts
Printed Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 12th day of Feb., 20 13, by: Mike Roberts as Owner **(type of authority, e.g. officer, trustee, attorney fact) for** **(name of party on behalf of whom instrument was executed).**

Personally Known ____ OR Produced Identification ✓ Type FL D/L

Notary Signature F Voncile Dow   Notary Stamp or Seal

F VONCILE DOW
Notary Public - State of Florida
My Comm. Expires Oct 3, 2014
Commission # EE 27026
Bonded Through National Notary Assn.

---AND---

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief

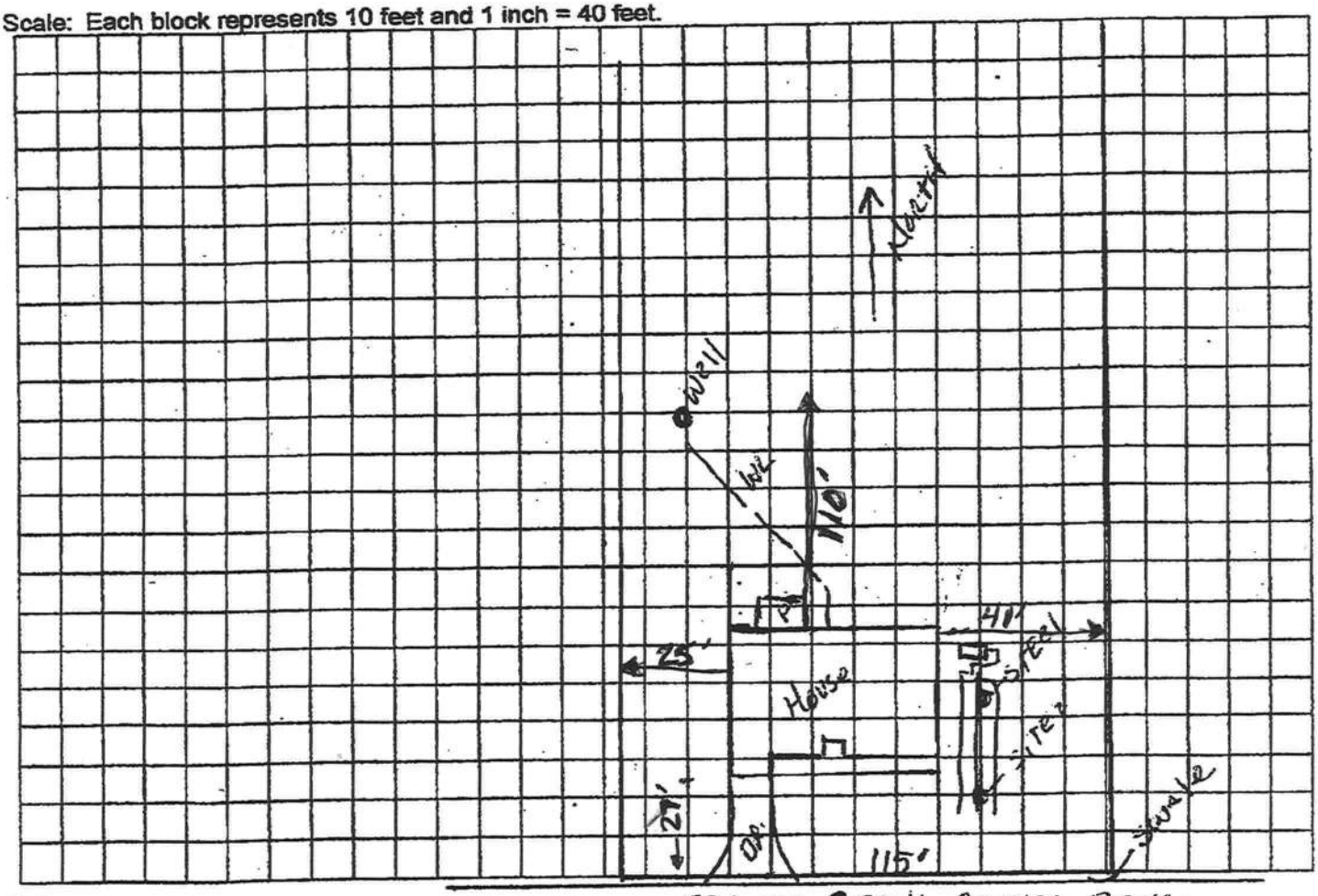
[signature]
Signature of Natural Person Signing (in line #10 above.)

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number_____

---------- PART II - SITEPLAN ----------

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes: 524 SW Emerald Corner Drive.
BM.

Mike Roberts

Lot 6 Cannon Creek Dr. Unit 2

23-45-16-03095-016

Site Plan submitted by: Robert W Jack 2-11-13

Plan Approved_____ Not Approved_____ Date_____

By_____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit # _____ Owner's Name Mike Robert

Well Depth 120 Ft. Casing Depth 100 Ft. Water Level 58 Ft.

Casing Size (4 inch Steel) Pump Installation: Deep Well Submersible

Pump Make Schaefer Pump Model 20SV1S4 HP 1

System Pressure (PSI) 30 On 30 Off 50 Average Pressure 50

Pumping System GPM at average pressure and pumping level _____ (GPM)

Tank Installation: (Bladder)/Galvanized Make DST
Model _____ Size 86

Tank Draw-down per cycle at system pressure 27 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

2/13/2013
Date

Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456

450 SR 13 N., Suite 206, #308, Jacksonville, FL 32259 • Tel(904)381-8901 • Fax(904)381-8902

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 13-00086-01
DATE TESTED: 2/28/13
DATE REPORTED: 3/1/13

PROJECT:	Cannon Creek Place Lot # 6, Spec House
CLIENT:	Mike Roberts Framing, 657 SW Catherine Lane, Lake City, Florida 32025
GENERAL CONTRACTOR:	Mike Roberts Framing
EARTHWORK CONTRACTOR:	Mike Roberts Framing
INSPECTOR:	C. Day

ASTM METHOD	SOIL USE
(D-6938) Nuclear	BUILDING FILL

SPECIFIED REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	MAXIMUM DENSITY
1		Southeast Corner of Pad 10' West, 10' North	0-12"	106.1	6.3	99.8	1	104.0	96%
2		Approximate Center of Pad	0-12"	112.6	7.3	104.9	1	104.0	101%
3		Northwest Corner of Pad 12' East, 12' South	0-12"	108.9	7.5	101.3	1	104.0	97%

REMARKS: The Above Tests Meet Specified Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Tannish Brown Sand (SP) (Refer to CTI Job No.: 12-00234-01, Richardson's Site Prep, Ft. White Pit)	104.0	10.0	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

Reviewed By: NABIL O. HMEDI, LICENSED, No 57842, STATE OF FLORIDA, PROFESSIONAL ENGINEER

Date: 3/1/13

Licensed, Florida No: 57842

COPY

COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2010 EFFECTIVE 15 MARCH 2012 AND THE NATIONAL ELECTRICAL 2008 EFFECTIVE 1 OCTOBER 2009

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT 2010 FLORIDA BUILDING CODES RESIDENTIAL, EFFECTIVE 15 MARCH 2012. NATIONAL ELECTRICAL CODE 2008 EFFECTIVE 1 OCTOBER 2009. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL			Items to Include- Each Box shall be Circled as Applicable		
				Yes	No	N/A
1	Two (2) complete sets of plans containing the following:			✓		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void			✓		
3	Condition space (Sq. Ft.)		Total (Sq. Ft.) under roof	IIIIIIII	IIIIIIII	IIIII

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	✓		
5	Dimensions of all building set backs	✓		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	✓		
7	Provide a full legal description of property.	✓		

Wind-load Engineering Summary, calculations and any details are required.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
8	**Plans or specifications must show compliance with FBCR Chapter 3**	IIIIII	IIIII	IIIIII
		YES	**NO**	**N/A**
9	Basic wind speed (3-second gust), miles per hour	✓		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	✓		
11	Wind importance factor and nature of occupancy	✓		
12	The applicable internal pressure coefficient, Components and Cladding	✓		
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	✓		

Elevations Drawing including:

14	All side views of the structure	✓		
15	Roof pitch	✓		
16	Overhang dimensions and detail with attic ventilation	✓		
17	Location, size and height above roof of chimneys	✓		
18	Location and size of skylights with Florida Product Approval	✓		
18	Number of stories	✓		
20A	Building height from the established grade to the roofs highest peak	✓		

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	✓		
21	Raised floor surfaces located more than 30 inches above the floor or grade	✓		
22	All exterior and interior shear walls indicated	✓		
23	Shear wall opening shown (Windows, Doors and Garage doors)	✓		
24	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	✓		
25	Safety glazing of glass where needed	✓		
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)			✓
27	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails			✓
28	Identify accessibility of bathroom (see FBCR SECTION 320)	✓		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plan (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

FBCR 403: Foundation Plans

		YES	NO	N/A
29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	✓		
30	All posts and/or column footing including size and reinforcing	✓		
31	Any special support required by soil analysis such as piling.	✓		
32	Assumed load-bearing valve of soil_____ Pound Per Square Foot	✓		
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type) **For** structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	✓		

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	✓		
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	✓		

FBCR 318: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. **Protection shall be provided by registered termiticides**	✓		

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type	✓		
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	✓		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer			✓
40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers			✓
41	Girder type, size and spacing to load bearing walls, stem wall and/or priers			✓
42	Attachment of joist to girder			✓
43	Wind load requirements where applicable			✓
44	Show required under-floor crawl space			✓
45	Show required amount of ventilation opening for under-floor spaces			✓
46	Show required covering of ventilation opening			✓
47	Show the required access opening to access to under-floor spaces			✓
48	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & inter of the areas structural panel sheathing			✓

49	Show Draftstopping, Fire caulking and Fire blocking			✓
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6			✓
51	Provide live and dead load rating of floor framing systems (psf).			✓

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53	Fastener schedule for structural members per table IRC 602.3 are to be shown	✓		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per IRC Table 502.5 (1)	✓		
57	Indicate where pressure treated wood will be placed	✓		
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.1.6.1 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses	✓		

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing	✓		
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating	✓		
67	Valley framing and support details	✓		
68	Provide dead load rating of rafter system	✓		

FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	✓		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	✓		

ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assembles covering	✓		
72	Submit Florida Product Approval numbers for each component of the roof assembles covering	✓		

FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter11 Residential buildings compliance methods. **Two of the required forms are to be submitted**, *N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.*

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
		YES	**NO**	**N/A**
73	Show the insulation R value for the following areas of the structure	✓		
74	Attic space	✓		
75	Exterior wall cavity	✓		
76	Crawl space	✓		

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	✓		
78	Exhaust fans shown in bathrooms **Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required**	✓		
79	Show clothes dryer route and total run of exhaust duct	✓		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan	✓		
81	Show the location of water heater	✓		

Private Potable Water

82	Pump motor horse power	✓		
83	Reservoir pressure tank gallon capacity	✓		
84	Rating of cycle stop valve if used	✓		

Electrical layout shown including

85	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	✓		
86	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by **Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A**	✓		
87	Show the location of smoke detectors & Carbon monoxide detectors	✓		
88	Show service panel, sub-panel, location(s) and total ampere ratings	✓		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. **For structures** with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	✓		

90	Appliances and HVAC equipment and disconnects	✓		
91	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed **Combination arc-fault circuit interrupter,** Protection device.	✓		

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

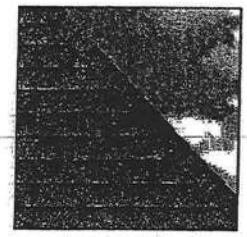
A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	**Building Permit Application** A current On-Line Building Permit Application www.ccpermit.com is to be completed, by following the Checklist all supporting documents must be submitted. There is a **$15.00** application fee.	✓		
93	**Parcel Number** The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also requested. www.columbiacountyfla.com	✓		
94	**Environmental Health Permit or Sewer Tap Approval** A copy of a approved Columbia County Environmental Health (386) 758-1058	✓		
95	**City of Lake City** A permit showing an approved waste water sewer tap 386-752-2031			✓
96	**Toilet facilities shall be provided for all construction sites**	✓		
97	**Town of Fort White** (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.			✓
98	**Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations	✓		
99	**CERTIFIED FINISHED FLOOR ELEVATIONS** will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	✓		
100	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is **$50.00**			
101	**Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	✓		
102	**911 Address:** An application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125 Ext. 3	✓		

ELK ROOFING PRODUCTS SPECIFICATIONS – TUSCALOOSA, AL



PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE®

Prestique Plus *High Definition* and Prestique Gallery Collection™

Product size 13¼"x 39⅜"
Exposure 5⅝"
Pieces/Bundle 16
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***

Raised Profile

Product size 13¼"x 38⅞"
Exposure 5⅝"
Pieces/Bundle 22
Bundles/Square 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

Prestique I *High Definition*

Product size 13¼"x 39⅜"
Exposure 5⅝"
Pieces/Bundle 16
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*™

Size: 12"x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Vented RidgeCrest™ w/*FLX*™

Size: 13"x13¼"
Exposure: 9¼"
Pieces/Box: 26
Coverage: 5 boxes = 100 linear feet

Prestique *High Definition*

Product size 13¼"x 38⅞"
Exposure 5⅝"
Pieces/Bundle 22
Bundles/Square 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood. Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard® treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL® Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

*See actual limited warranty for conditions and limitations.
** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.
***For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

SPECIFICATIONS

SCOPE: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

PREPARATION OF ROOF DECK: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes[4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

SOUTHEAST & ATLANTIC OFFICE:
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

PLANT LOCATION:
800.945.5545

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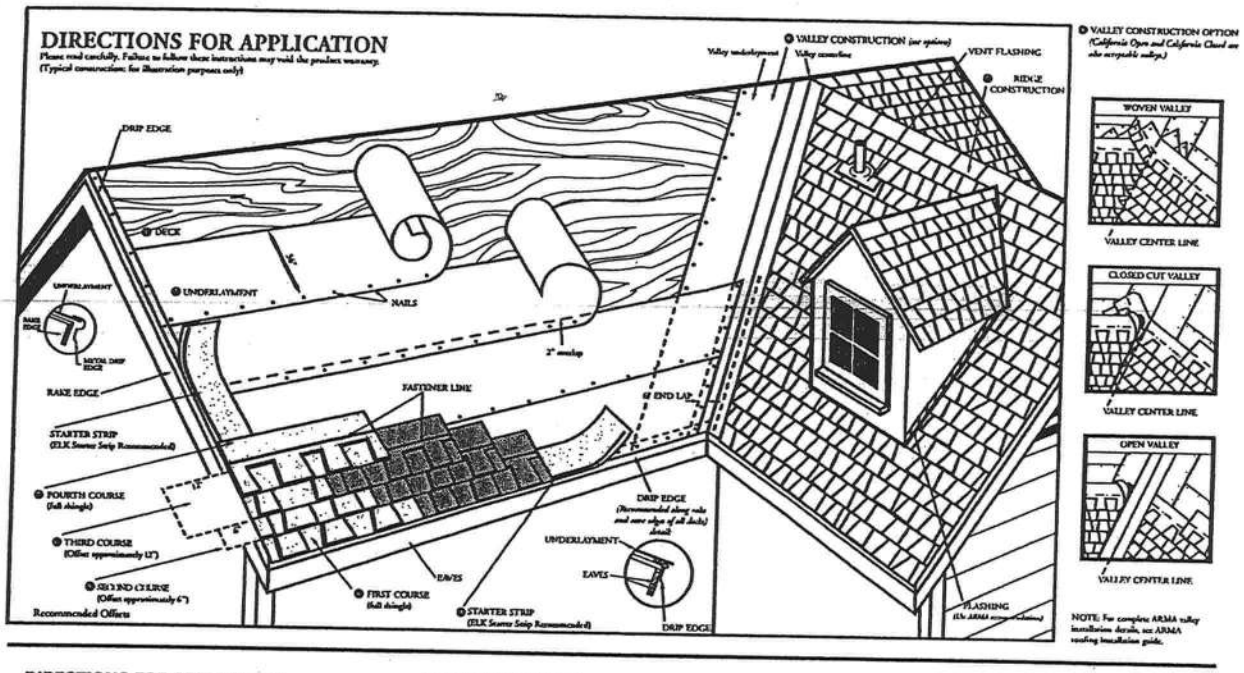
www.elkcorp.com

SS00T 06/04



DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

❺ SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

❻ THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z®Ridge or Seal-A-Ridge® with formula *FLX*™ or RidgeCrest™ with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

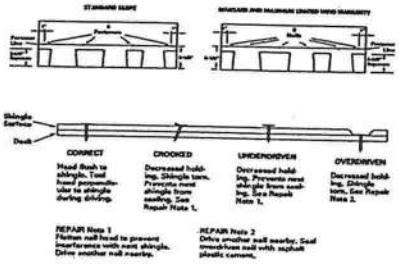
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

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www.elkcorp.com

FLORIDA DEPARTMENT OF Community Affairs

Community Affairs

- COMMUNITY PLANNING
- HOUSING & COMMUNITY DEVELOPMENT
- EMERGENCY MANAGEMENT
- OFFICE OF THE SECRETARY

Product Approval

USER: Public User

FL #	FL1214-R1
Application Type	Revision
Code Version	2004
Application Status	Approved
Comments	
Archived	☐

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Product Manufacturer Address/Phone/Email	Alenco 615 Carson Bryan, TX 77802 (979) 779-7770 ext 343 mkoppers@alenco.com
Authorized Signature	Martin Koppers mkoppers@alenco.com
Technical Representative Address/Phone/Email	Martin Koppers 615 Carson St. Bryan, TX 77802 mkoppers@alenco.com
Quality Assurance Representative Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	National Accreditation & Management Institute,
Referenced Standard and Year (of	**Standard**

Standard)	AAMA/NWWDA 101/I.S.2
Equivalence of Product Standards Certified By	
Sections from the Code	1707.4.2.1
Product Approval Method	Method 1 Option A
Date Submitted	06/08/2005
Date Validated	08/04/2005
Date Pending FBC Approval	06/18/2005
Date Approved	08/05/2005

Summary of Products

FL #	Model, Number or Name	Description
1214.1	1111	Vinyl Tilt Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 1111: 48X72 R(35) Tested with DS annealed,44X72 R(40)Tested with SS annealed. For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** PTID 1214 R1 I FL INSTALLATION INSTRUCTIONS - Aluminum B.pdf PTID 1214 R1 I INSTALLATION INSTRUCTIONS - Vinyl B.pdf Verified By:
1214.2	3753	Aluminum Tilt Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 3753:44X72 R(40) Tested with Tested with DS annealed.For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** Verified By:
1214.3	4710F	Aluminum Single Hung
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** 4710F:48X72 R(40)/DP(50), Tested with DS annealed glass.For smaller window sizes, glass to comply with ASTM E1300-02.		**Certification Agency Certificate** **Installation Instructions** Verified By:

Back Next

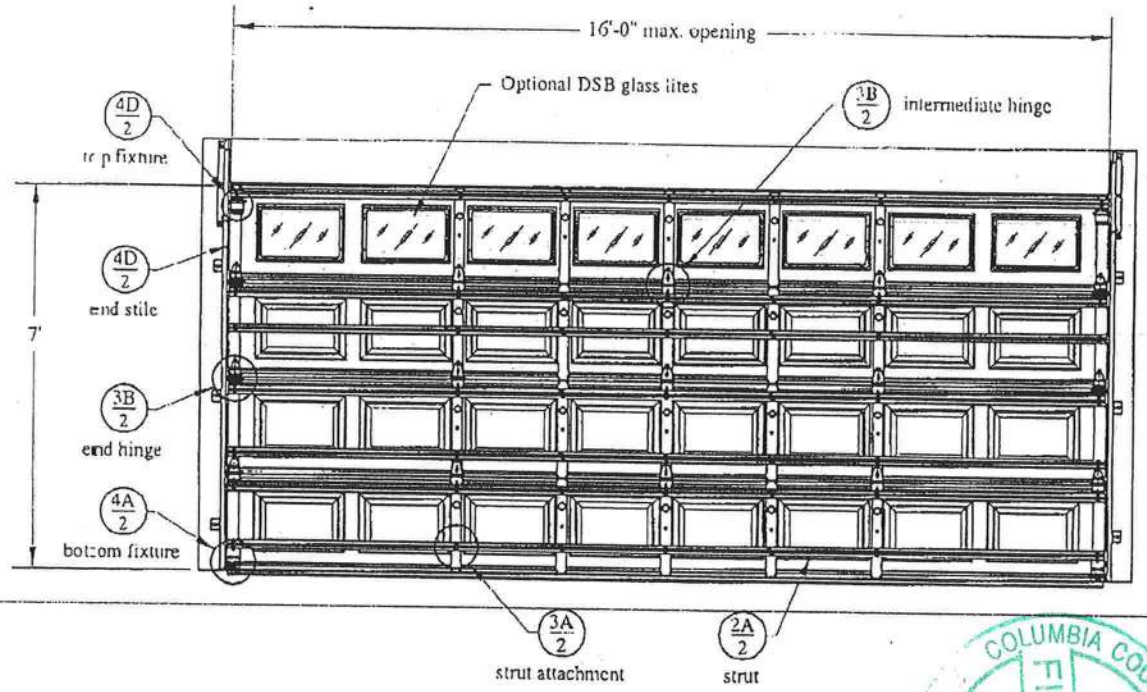
DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

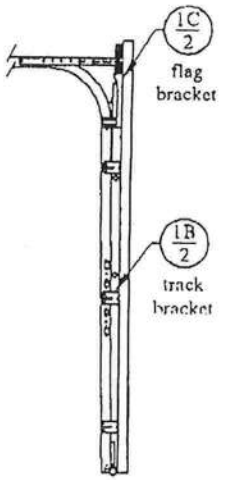
Door Model	Gauge	Decimal
2250/2251	25	.0185
2240/2241	24	.0225

Window Restriction: Standard window options are available.



door height	section quantity	strut quantity	trk brkt per side
6'-6" to 7'-0"	4	7	3
7'-6" to 8'-0"	5	9	4
8'-3" to 8'-9"	5	9	4
9'-0" to 10'-6"	6	11	5
10'-9" to 12'-3"	7	13	6
12'-6" to 14'-0"	8	15	7

Refer to Supplemental Instructions for strut placement on doors over 7'-0" high



This door has been tested in accordance with ANSI/DASMA 108-2002 & 108-2005
Design Pressure (DP): 30.1 pos / 33.5 neg
Test Pressure (TP): 45.2 pos / 50.3 neg

Per 2004 FBC Table 1609.6E, DP meets or exceeds basic wind speed of:
V = 140 MPH for Exposure B and mean roof height of 30' or less
V = 118 MPH for Exposure C and mean roof height of 30' or less

Maximum door size: 16'-0" wide by 14'-0" tall

Glazing and door have not been tested for windborne debris.

Wood back and supporting structural elements shall be designed by a registered professional engineer for wind loads shown on this drawing.

If door is not electrically operated, a lock must be installed.

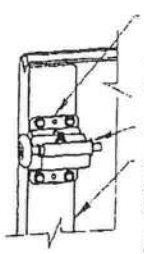
Details for door heights up to 14'-0" are contained in the Supplemental Instructions, which are required in addition to this drawing for installation. Do not install door using only this drawing.

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

FL 10201

C.H.I. | SCALE nts | DATE 01-18-2008
Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z6-1607-01200



16 gauge (.059) galvanized steel top fixture. Each fixture attached with four 1/4" x 3/4" screws.

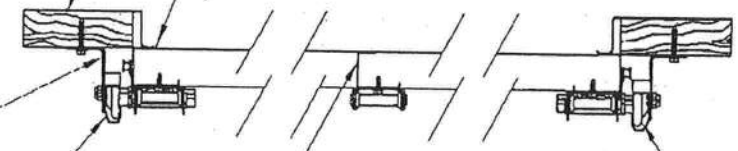
push nut

20 gauge (.034) end stile manufactured by C.H.I.

Strut, if applicable, not shown for clarity.

The 2x6 vertical wood jambs are to be grade 2 or better southern pine. Fasteners may be countersunk to provide a flush mounting surface.

2" x 7/16" (nominal) Stop molding required (not supplied by C.H.I.) To be secured with minimum 2-1/2" long nail or screw on a 5" spacing.



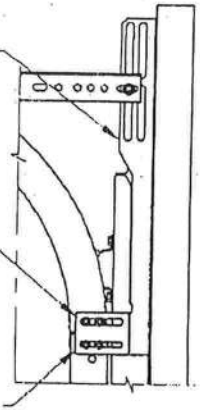
12 gauge (.102) galvanized steel track bracket.

20 gauge (.034) center stile manufactured by C.H.I.

2" steel roller

2" x .0485 min. galvanized steel track fastened to track brackets. Each track bracket attached with one 1/4" x 5/8" track bolt and nut.

Details on some views may have been omitted for clarity.

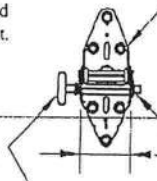


13 gauge (.086) galvanized steel flag bracket fastened to wood jamb with three 5/16" x 1-5/8" wood lag screws.

Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.

Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts.

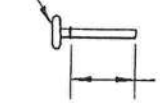
End Hinge
14 gauge (.069) galvanized steel end hinge fastened to section with six 1/4" x 3/4" screws.



Push nut

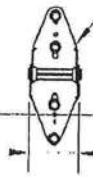
2 3/4"

2" steel track roller.



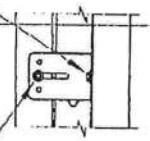
3-1/2" (min.) stem

Intermediate Hinge
16 gauge (.058) galvanized steel intermediate hinge fastened to section with four 1/4" x 3/4" screws.

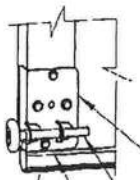


2 3/4"

12 gauge (.102) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.



Each track bracket attached with one 1/4" x 5/8" track bolt and nut. Or two 1/4" x 11/32" rivets.

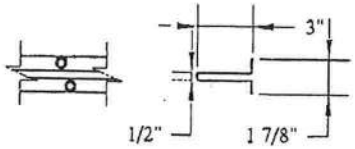


12 gauge (.102) galvanized steel bottom bracket manufactured by C.H.I. Each bracket attached with four red 1/4" x 3/4" screws.

Vinyl weatherstrip

push nut

Aluminum extrusion



17 gauge (.051) 50 ksi galvanized steel 3" strut attached with two 1/4" x 3/4" screws per stile or hinge plate.

Design Load: 30.1 pos / 33.5 neg
Test Load: 45.2 pos / 50.3 neg
page 2 of 2

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

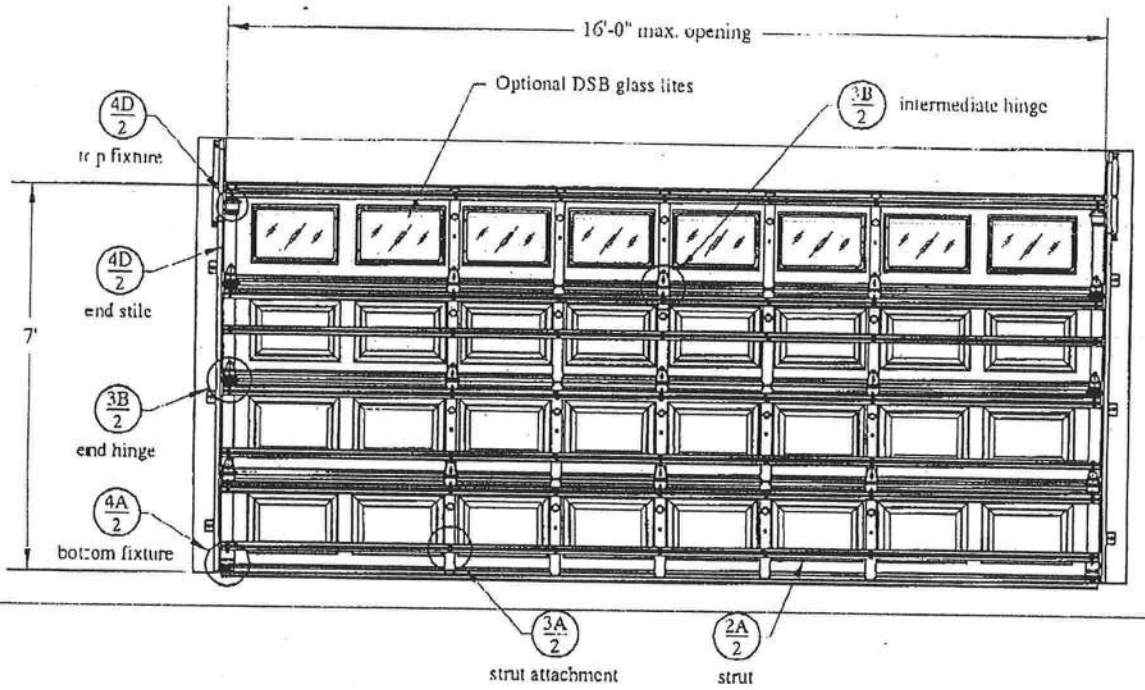
C.H.I.

SCALE nts
DATE 01-18-2008

Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z6-1607-01200

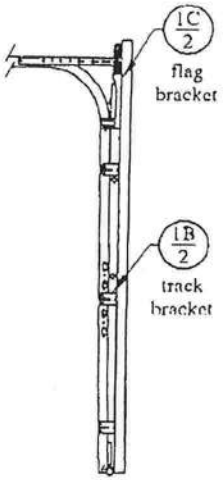
Door Model	Gauge	Decimal
2250/2251	25	.0185
2240/2241	24	.0225

Window Restriction: Standard window options are available.



door height	section quantity	strut quantity	trk brkt per side
6'-6" to 7'-0"	4	7	3
7'-6" to 8'-0"	5	9	4
8'-3" to 8'-9"	5	9	4
9'-0" to 10'-6"	6	11	5
10'-9" to 12'-3"	7	13	6
12'-6" to 14'-0"	8	15	7

Refer to Supplemental Instructions for strut placement on doors over 7'-0" high



This door has been tested in accordance with ANSI/DASMA 108-2002 & 108-2005
Design Pressure (DP): 30.1 pos / 33.5 neg
Test Pressure (TP): 45.2 pos / 50.3 neg

Per 2004 FBC Table 1609.6E, DP meets or exceeds basic wind speed of:
V = 140 MPH for Exposure B and mean roof height of 30' or less
V = 118 MPH for Exposure C and mean roof height of 30' or less

Maximum door size: 16'-0" wide by 14'-0" tall

Glazing and door have not been tested for windborne debris.

Wood back and supporting structural elements shall be designed by a registered professional engineer for wind loads shown on this drawing.

If door is not electrically operated, a lock must be installed.

Professional Engineer's seal provided only for verification of windload construction details

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Carrollton, Texas 75007
Florida P.E. # 51737

Details for door heights up to 14'-0" are contained in the Supplemental Instructions, which are required in addition to this drawing for installation. Do not install door using only this drawing.

FL 10201

C.H.I. OVERHEAD DOORS

SCALE nts
DATE 01-18-2008
Model 2250/51 (16'-0" wide)
C.H.I. Drawing: Z6-1607-01200

Product Approval
USER: Public User

FL #	FL4645
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Product Manufacturer	C.H.I. Overhead Doors
Address/Phone/Email	1485 Sunrise Drive Arthur, IL 61911 (217) 543-2135 ext 4309 canzelmo@chiohd.com
Authorized Signature	Chris Anzelmo canzelmo@chiohd.com
Technical Representative	Patrick J. Hunter
Address/Phone/Email	PO Box 260 1485 Sunrise Drive, IL 61911 (217) 543-2762 phunter@chiohd.com
Quality Assurance Representative	Jerod Price
Address/Phone/Email	1485 Sunrise Drive PO Box 260 Arthur, IL 61911 (217) 543-2135 jprice@chiohd.com
Category	Exterior Doors
Subcategory	Sectional Exterior Door Assemblies
Compliance Method	Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer ☑ Evaluation Report - Hardcopy Received
Florida Engineer or Architect Name who developed the Evaluation Report	John E. Scates, P.E.
Florida License	PE- 51737
Quality Assurance Entity	Architectural Testing, Inc.
Validated By	Gordon Thomas, P.E.
Certificate of Independence	

Referenced Standard and Year (of Standard)	**Standard**	**Year**

ANSI/DASMA 108-2002	2002
ASTM D 1929	2001
ASTM D 2843	1999
ASTM E 330-02	2002

Equivalence of Product Standards Certified By

Sections from the Code

Product Approval Method: Method 1 Option D

Date Submitted: 06/09/2005
Date Validated: 08/01/2005
Date Pending FBC Approval: 06/20/2005
Date Approved: 08/05/2005

Summary of Products

Go to Page [ ] GO Page 1 / 3

FL #	Model, Number or Name	Description
4645.1	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation

Limits of Use (See Other)
Approved for use in HVHZ:
Approved for use outside HVHZ:
Impact Resistant:
Design Pressure: +/-
Other: C.H.I. Drawing: Z7-1007-03000 Non impact rated Design load: +35.7 / -41.0 Test load: +53.6 / -61.5 9'-1" thru 10'-0" wide

Installation Instructions
Verified By:
Evaluation Reports
PTID_4645_T_all-instructions.pdf
PTID_4645_T_Cert_of_Independence_Scates_06_09_05.pdf
PTID_4645_T_Evaluation_Report_06_09_05.pdf
PTID_4645_T_polycarbonate_building_compliance.pdf
PTID_4645_T_res-instruct.pdf
PTID_4645_T_Z1-0907-04000s.pdf
PTID_4645_T_Z1-1007-01000s.pdf
PTID_4645_T_Z1-1007-02000s.pdf
PTID_4645_T_Z1-1007-03000s.pdf
PTID_4645_T_Z1-1607-02000.pdf
PTID_4645_T_Z1-1607-04000s.pdf
PTID_4645_T_Z1-1807-01000s.pdf
PTID_4645_T_Z1-1807-02000s.pdf
PTID_4645_T_Z1-1807-03000s.pdf
PTID_4645_T_Z2-1007-01000s.pdf
PTID_4645_T_Z2-1007-02000s.pdf
PTID_4645_T_Z2-1007-03000s.pdf
PTID_4645_T_Z2-1807-02000s.pdf
PTID_4645_T_Z2-1807-03000s.pdf
PTID_4645_T_Z3-0907-04000s.pdf
PTID_4645_T_Z3-1007-02000s.pdf
PTID_4645_T_Z3-1007-03000s.pdf
PTID_4645_T_Z3-1607-04000s.pdf
PTID_4645_T_Z3-1807-02000s.pdf
PTID_4645_T_Z3-1807-03000s.pdf
PTID_4645_T_Z4-1007-01000s.pdf
PTID_4645_T_Z4-1007-02000s.pdf
PTID_4645_T_Z4-1007-03000s.pdf
PTID_4645_T_Z4-1607-04000s.pdf
PTID_4645_T_Z4-1807-02000s.pdf
PTID_4645_T_Z4-1807-03000s.pdf
PTID_4645_T_Z5-0907-01000.pdf
PTID_4645_T_Z5-0907-04000s.pdf
PTID_4645_T_Z5-1007-01000s.pdf
PTID_4645_T_Z5-1007-02000s.pdf

	PTID_4645_T_Z5-1007-03000s.pdf PTID_4645_T_Z5-1607-04000s.pdf PTID_4645_T_Z5-1807-02000s.pdf PTID_4645_T_Z5-1807-03000s.pdf PTID_4645_T_Z6-0907-04000s.pdf PTID_4645_T_Z6-1007-02000s.pdf PTID_4645_T_Z6-1607-04000.pdf PTID_4645_T_Z6-1807-02000s.pdf PTID_4645_T_Z7-0907-04000.pdf PTID_4645_T_Z7-1007-01000s.pdf PTID_4645_T_Z7-1007-02000s.pdf PTID_4645_T_Z7-1007-03000s.pdf PTID_4645_T_Z7-1607-04000.pdf

4645.2	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z1-1007-03000 Non impact rated Design load: +12.8 / -14.8 Test load: -19.2 / -22.2 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.3	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z2-1007-03000 Non impact rated Design load: +15.9 / -18.2 Test load: +23.9 / -27.3 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.4	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z3-1007-03000 Non impact rated Design load: +19.2 / -22.0 Test load: +28.8 / -33.0 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.5	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z4-1007-03000 Non impact rated Design load: +22.9 / -26.3 Test load: +34.4 / -39.5 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.6	Model: 2216, 2217 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z5-1007-03000 Non impact rated Design load: +26.9 / -30.8 Test load: +40.4 / -46.2 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**

4645.7	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z5-1807-03000 Non impact rated Design Load: +25.9 / -28.8 Test Load: +38.9 / -43.2 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.8	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z4-1807-03000 Non impact rated Design Load: +22.0 / -24.5 Test Load: +33.0 / -36.8 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.9	Model: 2216, 2217, 4216 and 5216	26 ga. min. ext. and 27 ga.min. int. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z1-1807-03000 Non impact rated Design load: +12.4 / -13.8 Test load: +18.6 / -20.7 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.10	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z2-1807-03000 Non impact rated Design load: +15.3 / -17.0 Test load: +23.0 / -25.5 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.11	Model: 2216, 2217, 4216 and 5216	26 ga. ext. min. 27 ga. int. min. with foamed in place polyurethane insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z3-1807-03000 Non impact rated Design load: +18.5 / -20.7 Test load: +27.8 / -31.1 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.12	Model: 2250, 2251, 2240 and 2241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I.Drawing: Z5-1007-01000 Non impact rated Design Load: +26.9 / -30.8 Test Load: +40.4 / -46.2 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.13	Model: 2250, 2251, 2240 and 2241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z1-1007-01000 Non impact rated Design load: +12.8 / -14.8 Test load: +19.2 / -22.2 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.14	Model: 2250, 2251, 2240 and 2241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z2-1007-01000 Non impact rated Design load: +15.9 / -18.2 Test load: +23.9 / -27.3 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.15	Model: 2250, 2251, 2240 and 2241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z4-1007-01000 Non impact rated Design load: +22.9 / -26.3 Test load: +34.4 / -39.5 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.16	Model: 2250, 2251, 2240 and 2241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z7-1007-01000 Non impact rated Design load: +35.7 / -41.0 Test load: +53.6 / -61.5 9'-1" thru 10'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.17	Model: 2250, 2251, 4250, 4251, 2240, 2241, 4240, 4241, 5240 and 5241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z1-1807-01000 Non impact rated Design Load: +12.4 / -13.8 Test Load: +18.6 / -13.8 16'-1" thru 18'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.18	Model: 2250, 2251, 4250, 4251, 2240, 2241, 4240, 4241, 5240 and 5241	Steel pan (25 ga. min.) hollow or laid in place polystyrene insulation
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z5-0907-01000 Non impact rated Design Load: +26.9 / -30.8 Test Load: +40.4 / -46.2 Thru 9'-0" wide		**Installation Instructions** Verified By: **Evaluation Reports**
4645.19	Model: 2283, 2284, 2285 and 2286	27 ga. int. min. 27 ga. ext. min. with polystyrene insulation

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z2-1007-02000 Non impact rated Design load: +15.9 / -18.2 Test load: +23.9 / -27.3 9'-1" thru 10'-0" wide	**Installation Instructions** Verified By: **Evaluation Reports**

4645.20	Model: 2283, 2284, 2285 and 2286	27 ga. int. min. 27 ga.ext. min. with polystyrene insulation

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** C.H.I. Drawing: Z3-1007-02000 Non impact rated Design load: +19.2 / -22.0 Test load: +28.8 / -33.0 9'-1" thru 10'-0" wide	**Installation Instructions** Verified By: **Evaluation Reports**

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Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

PRODUCT APPROVAL SPECIFICATION SHEET

Location: Project Name:

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			FL 4242-R
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			FL 5108
2. Horizontal Slider			FL 5451
3. Casement			
4. Double Hung			
5. Fixed			FL 5418
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			FL 889-R2
2. Soffits			FL 4899
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			FL 3820-R1
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			FL 586-R2
2. Underlayments			FL 1814-R1
3. Roofing Fasteners			
4. Non-structural Metal Rf			FL 7518.1
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			FL. 1960-R1
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			FL 451-R1
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			FL. 474-R1
2. Truss plates			
3. Engineered lumber			FL 1008-R1
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

S. H. 2

[redacted]

Short Form
Entire House

Ahlbrandt Ref. Inc.

Job:
Date: May 16,
By:

P O Box 1945, Alachua, Fl 32616 Phone: 352-225-1308 Fax: 386-418-0549

Project Information

For: Mike Roberts

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	-	50
Moisture difference (gr/lb)	-	52

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

HEATING EQUIPMENT

Make	York
Trade	Guarden
Model	HP030X1321

Efficiency	8 HSPF	
Heating input		
Heating output	30000	Btuh @ 47°F
Temperature rise	29	°F
Actual air flow	933	cfm
Air flow factor	0.035	cfm/Btuh
Static pressure	0.10	in H2O
Space thermostat		

COOLING EQUIPMENT

Make	York
Trade	Guarden
Cond	HP030X1321
Coil	G2FD036S17+1TV0701

Efficiency	13 SEER	
Sensible cooling	19600	Btuh
Latent cooling	8400	Btuh
Total cooling	28000	Btuh
Actual air flow	933	cfm
Air flow factor	0.048	cfm/Btuh
Static pressure	0.10	in H2O
Load sensible heat ratio	0.69	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Dining rm	131	3391	1820	119	87
Kitchen	108	1408	2284	50	109
Bath	52	975	495	34	24
Bedrm 2	182	5143	3325	181	159
Bedrm 3	186	3195	2343	112	112
Fam rm	357	4398	4219	155	202
Master bedrm	219	3379	3010	119	144
Master bath	78	2926	1128	103	54
W.I.c.	48	1531	569	54	27
core	132	189	314	7	15

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House	1494	26536	19507	933	933
Other equip loads		5651	2596		
Equip. @ 0.97 RSM			21440		
Latent cooling			9979		
TOTALS	1494	32187	31419	933	933

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

CES
Crews Engineering Services, LLC

Crews Engineering Services, LLC
P.O. Box 970
Lake City, FL 32056
(Phone) 386.623.4303
brett@crewsengineeringservices.com

Finish Floor Elevation Recommendation

For: Mike Roberts

Parcel Number: 23-4S-16-03095-106

Description of Project:

The project is located on 0.5 acres off of SW Gerald Conner Dr in Columbia County, FL (Lot 6, Cannon Creek Place, Unit 2). The owner is proposing to construct a site built home +/-50 feet from SW Gerald Conner Dr.

Existing Conditions:

A site visit was conducted on 02-05-2013 to the project site.

The topography of the property is such that the land slopes away from toward SW Gerald Conner Dr. and eventually to an existing retention pond to the west.

Recommendation:

Although the proposed building location is lower than the nearest road, there is adequate drainage to avoid damage from storm water runoff if the finish floor is not set one foot above.

The proposed building shall be protected from storm water runoff.

The finish floor elevation of the new building shall be set as discussed at +/-6" in above SW Gerald Conner Dr at the building location). The area around the proposed structure shall be graded such as to convey all storm water runoff away from and around the proposed structure.

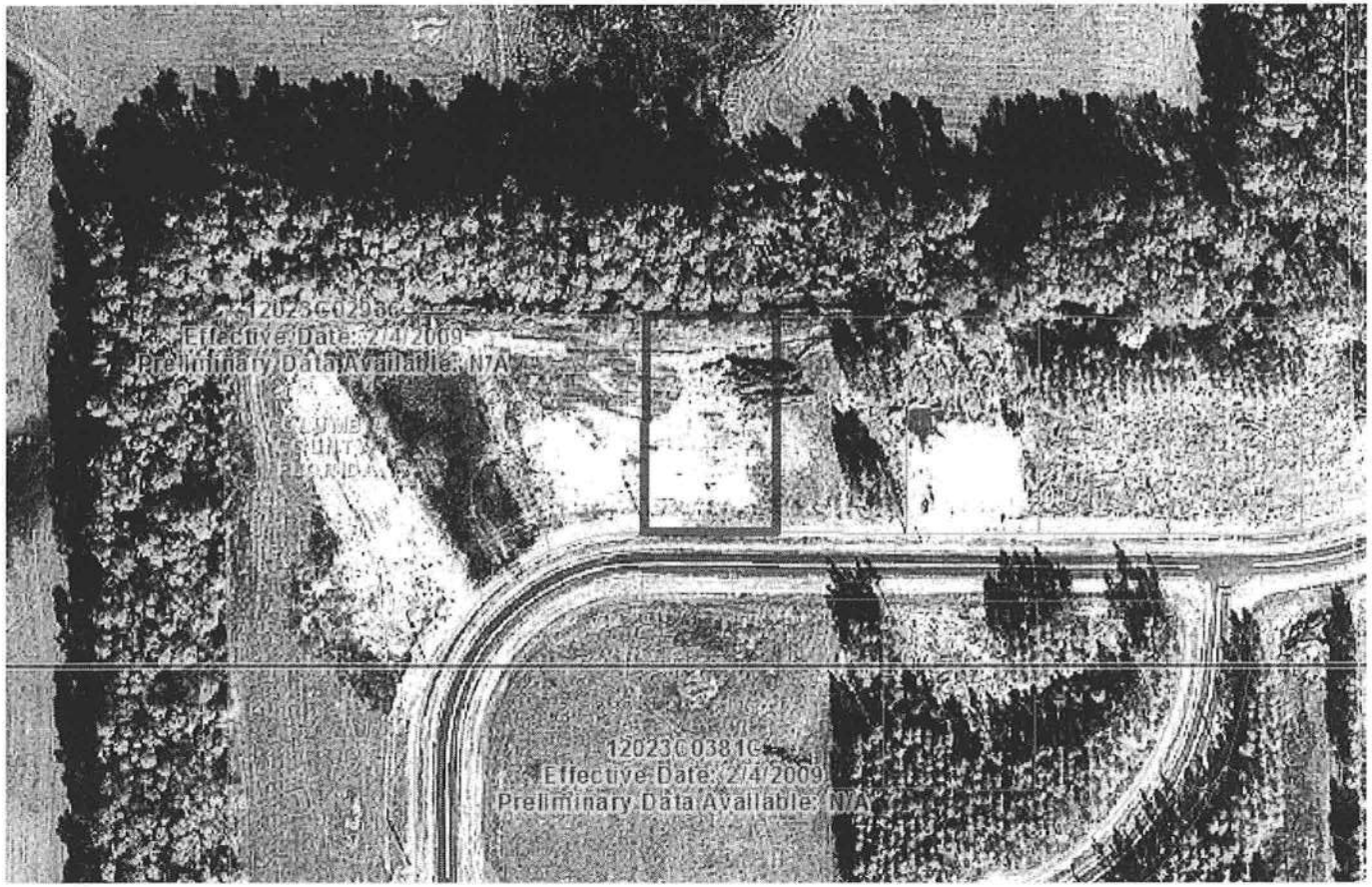
The above recommendation is based on the structure's location as described above.

No topographical survey was used.

2-11-2013

Brett A. Crews, P.E. 65592

Suwannee River Water Management District
Effective Flood Information Report



Effective Flood Zones described on Page 2

- SFHA - AE w/Floodway
- SFHA - Zones AE, AH, AO
- SFHA - Zone VE
- SFHA - Zone A
- 0.2 % (shaded X)
- Wetlands
- FIRM Panel
- State Lands
- Counties
- SRWMD
- Parcels
- Depressions
- BFE
- Cross Sections

LOCATION

Date: 02-11-2013

Parcel: 23-4S-16-03095-106

County: Columbia

STR: S023 T04 R16

Columbia Flood Hazard Areas Status: Effective: 02/04/2009

FLOOD INFORMATION

FIRM Panel(s): 12023C0293C

Parcel In Special Flood Hazard Area? (SFHA): No

Flood Zone(s):

1% Annual Chance Flood Elev (BFE): Not Available

Floodway: No

10% Annual Chance Flood Elev: Not Available

50% Annual Chance Flood Elev: Not Available

Note: Elevations are based on NAVD88

The Federal Emergency Management Agency (FEMA) maintains information about map features, such as street locations and names, in or near designated flood hazard areas. The information herein represents the best available data as of the effective date shown. The applicable Flood Insurance Study and a Digital Flood Insurance Rate Map is available online (http://www.srwmdfloodreport.com). To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to also consult the FEMA Map Service Center at 1-800-358-9616 (http://www.msc.fema.gov) for information on available products associated with this FIRM panel. Available products from the Map Service Center may include previously issued Letters of Map Change.

Requests to revise flood information in or near designated flood hazard areas may be provided to FEMA during the community review period on preliminary maps, or through the Letter of Map Change process for effective maps.

Base Flood Elevation (BFE)

The elevation shown on the Flood Insurance Rate Map for Zones AE, AH, A1-A30, AR, AO, V1-V30, and VE that indicates the water surface elevation resulting from a flood that has a one percent chance of equaling or exceeding that level in any given year.

A

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Because detailed analyses are not performed for such areas; no depths or base flood elevations are shown within these zones.

AE, A1-A30

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. In most instances, base flood elevations derived from detailed analyses are shown at selected intervals within these zones.

AH

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Usually areas of ponding with flood depths of 1 to 3 feet. Base Flood Elevations are determined.

AO

Areas with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. Usually areas of sheet flow on sloping terrain with flood depths of 1 to 3 feet. Base Flood Elevations are determined.

Supplemental Information:

10%-chance flood elevations (10-year flood-risk elevations) and 50%-chance flood elevations (2-year flood-risk elevations), are calculated during detailed flooding studies but are not shown on FEMA Digital Flood Insurance Rate Maps (FIRMs). They have been provided as supplemental information in the Flood Information section of this report.

AE FW (FLOODWAYS)

The channel of a river or other watercourse and the adjacent land areas that must be reserved in order to discharge the base flood (1% annual chance flood event). The floodway must be kept open so that flood water can proceed downstream and not be obstructed or diverted onto other properties.

Please note, if you develop within the regulatory floodway, you will need to contact your Local Government and the Suwannee River Water Management District prior to commencing with the activity. Please contact the District at 800.226.1066.

VE

Areas with a 1% annual chance of flooding over the life of a 30-year mortgage with additional hazards due to storm-induced velocity wave action. Base Flood Elevations (BFEs) derived from detailed analyses.

X 0.2 PCT (X Shaded, 0.2 PCT ANNUAL CHANCE FLOOD HAZARD)

Same as Zone X; however, detailed studies have been performed, and the area has been determined to be within the 0.2 percent annual chance floodplain (also known as the 500-year flood zone). Insurance purchase is not required in this zone but is available at a reduced rate and is recommended.

X

All areas outside the 1-percent annual chance floodplain are Zone X. This includes areas of 1% annual chance sheet flow flooding where average depths are less than 1 foot, areas of 1% annual chance stream flooding where the contributing drainage area is less than 1 square mile, or areas protected from the 1% annual chance flood by levees. No Base Flood Elevations or depths are shown within this zone. Insurance purchase is not required in these zones.

LINKS

FEMA:

http://www.fema.gov

SRWMD:

http://www.srwmd.state.fl.us

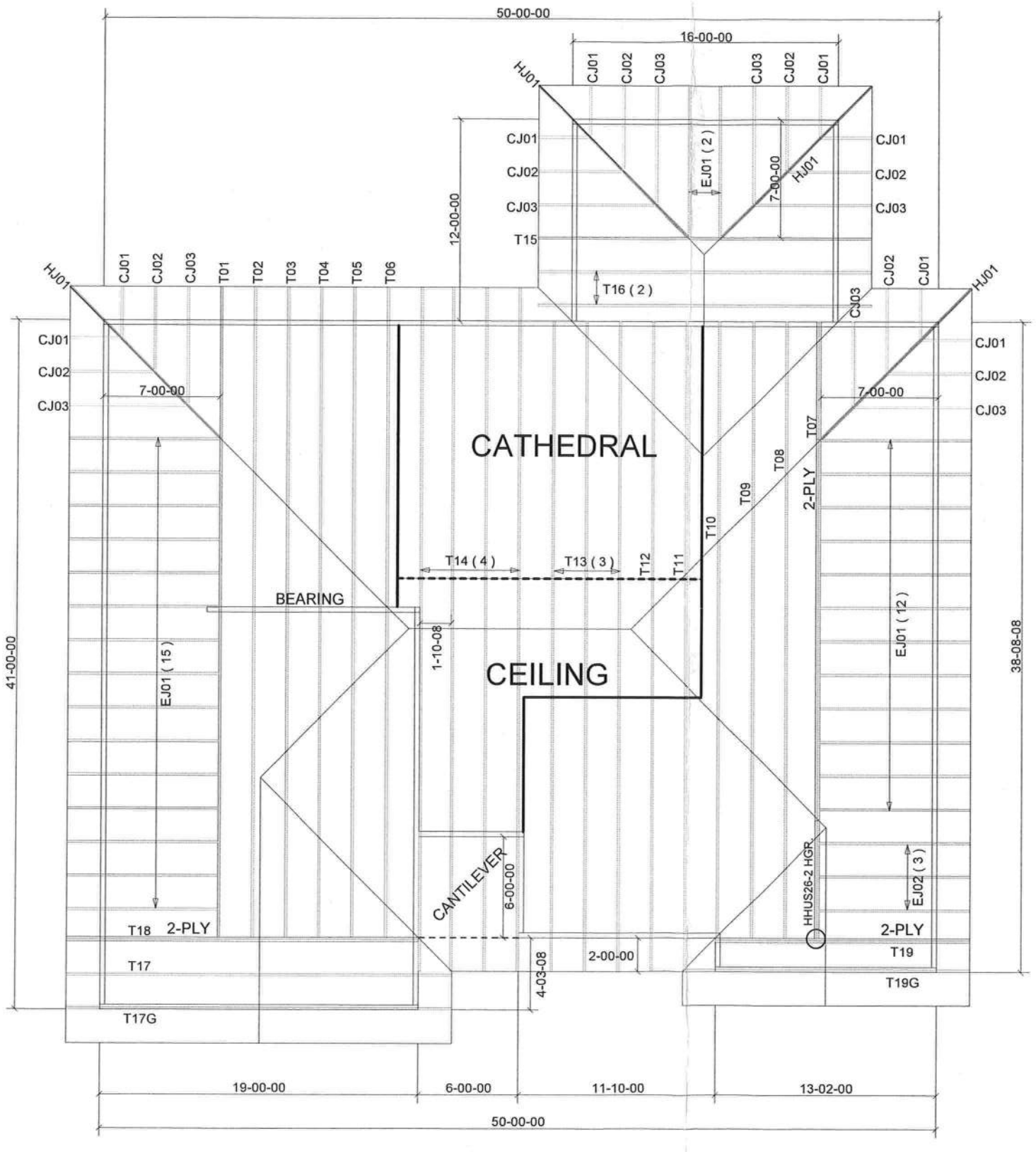
CONTACT

SRWMD
9225 County Road 49
Live Oak, FL 32060

(386) 362-1001

Toll Free:
(800) 226-1066

6/12 PITCH - 24" 0/H



BEARING HEIGHT SCHEDULE

BASE

NOTES:

1) REFER TO HIB-91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

Builders FirstSource

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Tampa
PHONE: 813-621-9831 FAX: 813-628-8956

Freeport
PHONE: 850-835-4541 FAX: 850-835-6835

BUILDER: MIKE ROBERTS			
LEGAL ADDRESS: SPEC HSE			
MODEL: CUSTOM			Revision: Rev. By:
DATE: 2-8-13	DESIGNER: KLH		Original Reference #: 466999
1st Level Job #:	2nd Level Job #:		Roof Job #: 466999

MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERHAUSER PRODUCT #'s 1630.2 - 1630.10

JULIUS LEE PE.

RE: 466999 - MIKE ROBERTS - SPEC HSE

1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435

Site Information:
Project Customer: Mike Roberts - O/B Project Name: 466999 Model: Custom
Lot/Block: 6 Subdivision: Cannon Creek
Address:
City: Columbia Cty State: FL

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: Unknown at time of seal License #: Unknown at time of seal
Address: Unknown at time of seal
City: Unknown at time of seal State: Unknown at time of seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
FBC 2010/TPI 2007 Design Program: MiTek 20/20 7.3
ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 27 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I6380624	CJ01	2/8/013	18	I6380641	T12	2/8/013
2	I6380625	CJ02	2/8/013	19	I6380642	T13	2/8/013
3	I6380626	CJ03	2/8/013	20	I6380643	T14	2/8/013
4	I6380627	EJ01	2/8/013	21	I6380644	T15	2/8/013
5	I6380628	EJ02	2/8/013	22	I6380645	T16	2/8/013
6	I6380629	HJ01	2/8/013	23	I6380646	T17	2/8/013
7	I6380630	T01	2/8/013	24	I6380647	T17G	2/8/013
8	I6380631	T02	2/8/013	25	I6380648	T18	2/8/013
9	I6380632	T03	2/8/013	26	I6380649	T19	2/8/013
10	I6380633	T04	2/8/013	27	I6380650	T19G	2/8/013
11	I6380634	T05	2/8/013				
12	I6380635	T06	2/8/013				
13	I6380636	T07	2/8/013				
14	I6380637	T08	2/8/013				
15	I6380638	T09	2/8/013				
16	I6380639	T10	2/8/013				
17	I6380640	T11	2/8/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2013.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

JULIUS S.K. LEE
LICENSE
No 34869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

February 8,2013

Julius Lee

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	CJ01	Jack-Open Truss	8	1	I6380624
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:35 2013 Page 1

ID:jRhrov9QzLs40H7EpCZ11VyVpE7-I9ale4Y_69tuDdOp8ZRGSMQG7I?0zLFIOVNxe2znBkY

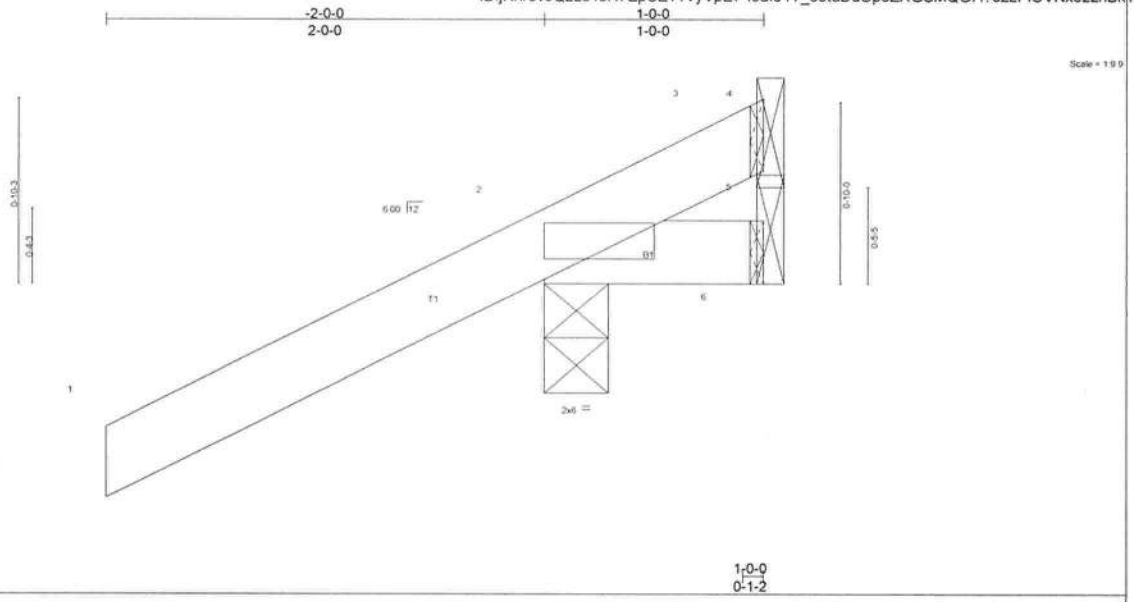


Plate Offsets (X,Y): [2:0-6-0,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 7 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=187/0-3-8 (min. 0-1-8), 3=-16/Mechanical, 6=-30/Mechanical
Max Horz 2=67(LC 12)
Max Uplift2=-141(LC 12), 3=-20(LC 2), 6=-38(LC 2)
Max Grav 2=228(LC 2), 3=19(LC 10), 6=33(LC 16)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 2, 20 lb uplift at joint 3 and 38 lb uplift at joint 6.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	CJ03	Jack-Open Truss	8	1	I6380626
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:37 2013 Page 1

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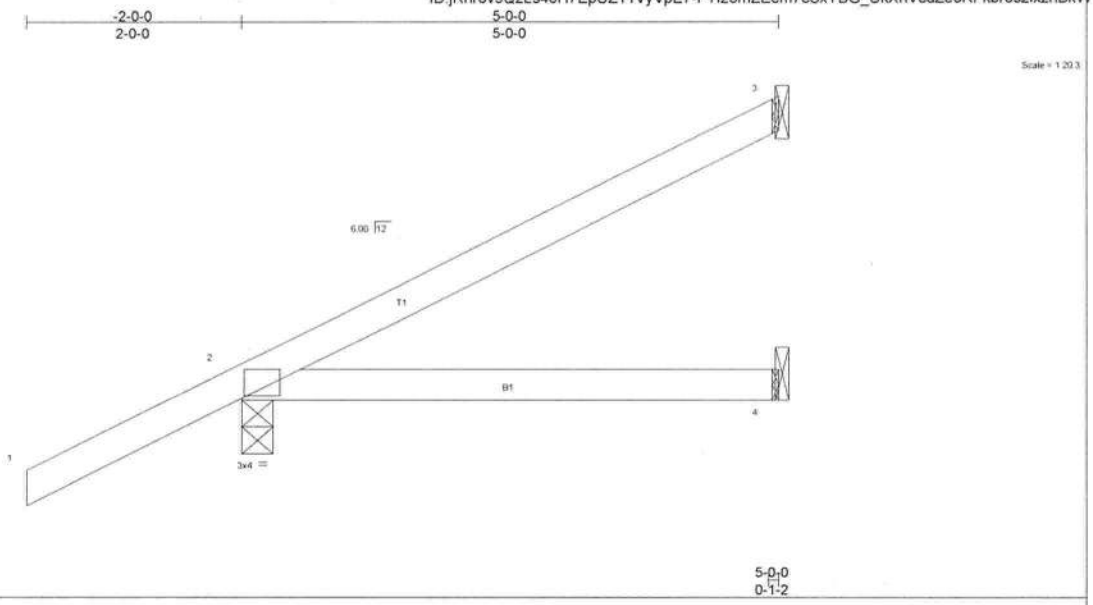


Plate Offsets (X,Y): [2:0-4-4,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.07	4-7	>904	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.06	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 19 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

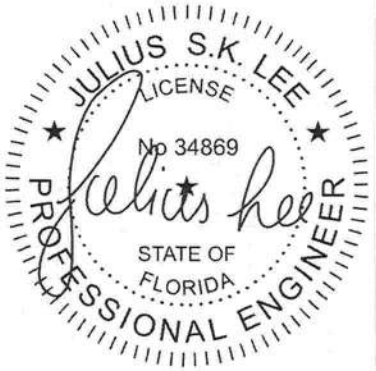
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=79/Mechanical, 2=253/0-3-8 (min. 0-1-8), 4=23/Mechanical
Max Horz 2=162(LC 12)
Max Uplift3=-93(LC 12), 2=-148(LC 12), 4=-35(LC 9)
Max Grav 3=97(LC 2), 2=304(LC 2), 4=56(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-824/589
BOT CHORD 2-4=-952/1106

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3, 148 lb uplift at joint 2 and 35 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE	I6380628
466999	EJ02	Jack-Partial Truss	3	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:39 2013 Page 1

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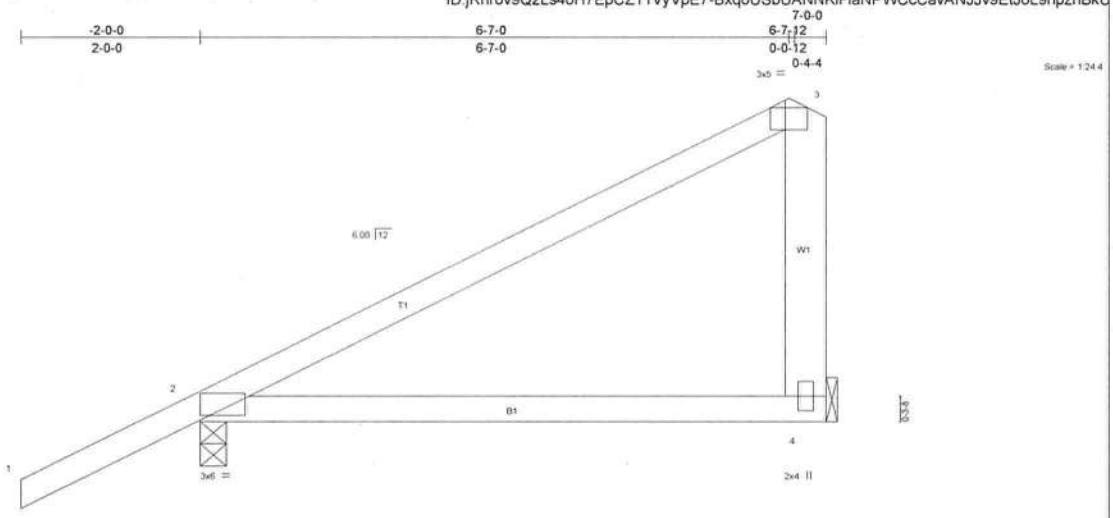

Plate Offsets (X,Y): [2:0-6-0,0-0-10], [3:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.05	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.10	4-7	>800	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 33 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x6 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=141/Mechanical, 2=312/0-3-8 (min. 0-1-8)
Max Horz 2=142(LC 12)
Max Uplift 4=-74(LC 12), 2=-95(LC 12)
Max Grav 4=167(LC 2), 2=374(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1175/489
BOT CHORD 2-4=-972/1639

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 4 and 95 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	HJ01	Diagonal Hip Girder	4	1	I6380629
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:40 2013 Page 2

ID:jRhrov9QzLs40H7EpCZ11VyVpE7-f7NBhoc6xhVBJPGmx61R9Q73wmcteXs1Ym5iJGznBkT

LOAD CASE(S) Standard

Concentrated Loads (lb)

Vert: 11=33(F=17, B=17) 12=4(F=2, B=2) 13=-70(F=-35, B=-35) 14=33(F=16, B=16) 15=-2(F=-1, B=-1) 16=-26(F=-13, B=-13)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T01	Half Hip Truss	1	1	I6380630
					Job Reference (optional)

NOTES (11-14)

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-3=-44, 3-10=-44, 11-19=-10
   Concentrated Loads (lb)
   Vert: 3=-69(F) 6=-69(F) 18=-205(F) 5=-69(F) 15=-22(F) 8=-69(F) 22=-69(F) 23=-69(F) 24=-69(F) 25=-69(F) 26=-69(F) 27=-69(F) 28=-69(F) 29=-69(F) 30=-69(F) 31=-69(F) 32=-69(F) 33=-22(F) 34=-22(F) 35=-22(F) 36=-22(F) 37=-22(F) 38=-22(F) 39=-22(F) 40=-22(F) 41=-22(F) 42=-22(F) 43=-22(F) 44=-22(F) 45=-22(F)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T03	Hip Truss	1	1	I6380632
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:44 2013 Page 1

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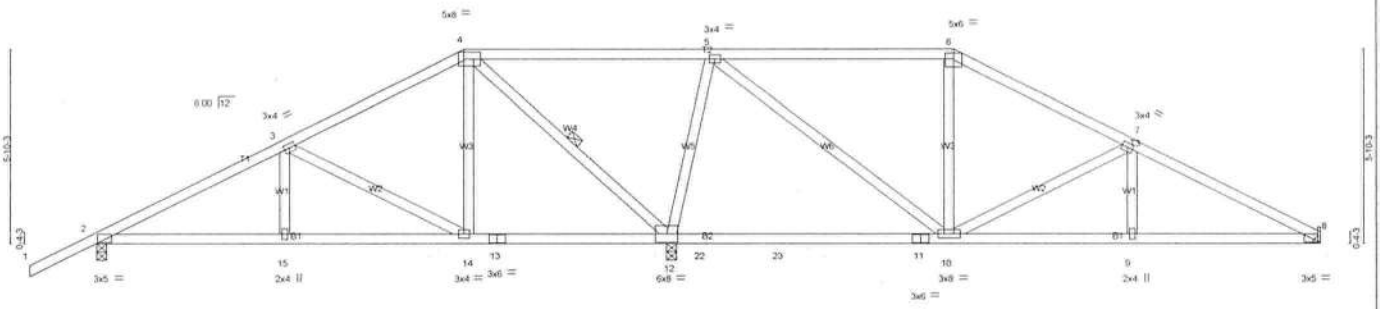
-2-0-0	5-7-11	11-0-0	18-4-9	25-8-8	31-0-13	36-8-8
2-0-0	5-7-11	5-4-5	7-4-9	7-3-15	5-4-5	5-7-11

Scale = 1:65.4



5-7-11	11-0-0	17-2-12	25-8-8	31-0-13	36-8-8
5-7-11	5-4-5	6-2-12	8-5-12	5-4-5	5-7-11

Plate Offsets (X,Y): [2:0-5-4,0-0-4], [4:0-6-0,0-2-8], [6:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.74	Vert(LL)	-0.10	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.17	10-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 190 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 10-12.
WEBS 1 Row at midpt 4-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=424/Mechanical, 2=428/0-3-8 (min. 0-1-8), 12=1289/0-3-8 (min. 0-1-14)
Max Horz 2=96(LC 16)
Max Uplift 8=-147(LC 13), 2=-157(LC 12), 12=-296(LC 9)
Max Grav 8=527(LC 28), 2=550(LC 27), 12=1445(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-860/322, 4-5=-122/450, 5-6=-336/303, 6-7=-446/278, 7-8=-817/449
BOT CHORD 2-15=-223/1055, 14-15=-201/502, 12-22=-316/292, 22-23=-316/292, 11-23=-316/292, 10-11=-316/292, 9-10=-314/679, 8-9=-547/852
WEBS 3-14=-458/330, 4-14=-119/280, 4-12=-766/442, 5-12=-933/592, 5-10=-377/725, 7-10=-427/343

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 8, 157 lb uplift at joint 2 and 296 lb uplift at joint 12.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

February 8,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

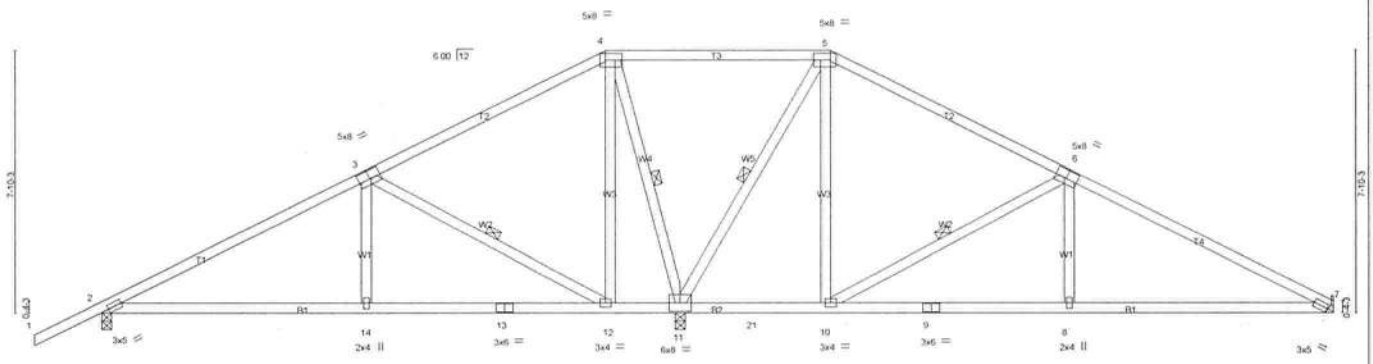
Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T05	Hip Truss	1	1	I6380634
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:58:47 2013 Page 1

ID:jRhrov9QzLs40H7EpCZ11VyVpE7-yTlq9BhVHrOBfTJ6r4f4xuvFCb11nh839MHa3MznBkM

-2-0-0	7-10-8	15-0-0	21-8-8	28-10-0	36-8-8
2-0-0	7-10-8	7-1-8	6-8-8	7-1-8	7-10-8

Scale = 1:65.0



7-10-8	15-0-0	17-2-12	21-8-8	28-10-0	36-8-8
7-10-8	7-1-8	2-2-12	4-5-12	7-1-8	7-10-8

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-4-0,0-3-0], [4:0-6-0,0-2-8], [5:0-6-0,0-2-8], [6:0-4-0,0-3-0], [7:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.06	8-20	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.11	8-20	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 198 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-12, 4-11, 5-11, 6-10

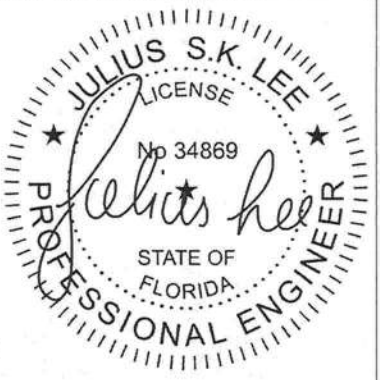
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=404/0-3-8 (min. 0-1-8), 11=1326/0-3-8 (min. 0-1-15), 7=404/Mechanical
Max Horz 2=120(LC 16)
Max Uplift 2=-154(LC 12), 11=-288(LC 12), 7=-154(LC 13)
Max Grav 2=548(LC 27), 11=1500(LC 2), 7=520(LC 28)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1109/367, 3-4=-39/332, 4-5=-69/448, 6-7=-972/639
BOT CHORD 2-14=-587/1491, 13-14=-111/372, 12-13=-111/372, 11-12=-297/338, 9-10=-176/536, 8-9=-176/536, 7-8=-921/1320
WEBS 3-12=-627/463, 4-12=-169/343, 4-11=-832/478, 5-11=-889/479, 5-10=-177/396, 6-10=-617/466

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 2, 288 lb uplift at joint 11 and 154 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



February 8,2013

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T07	HIP TRUSS	1	2	I6380636
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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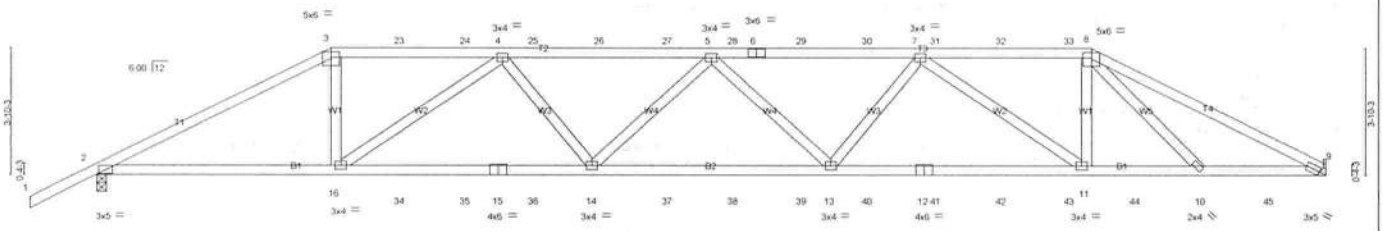
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-IR6jCule6N0UIFB4eeFGeyd8Fcd2Syfole?LkZznBkH

-2-0-0	7-0-0	12-1-8	18-4-4	24-7-0	29-8-8	36-8-8
2-0-0	7-0-0	5-1-8	6-2-12	6-2-12	5-1-8	7-0-0

Scale = 1:65.2



7-0-0	14-9-8	21-11-0	29-8-8	32-10-9	36-8-8
7-0-0	7-9-8	7-1-8	7-9-8	3-2-1	3-9-15

Plate Offsets (X,Y): [3:0-3-0,0-2-0], [8:0-3-0,0-2-0], [9:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.28	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.55	13-14	>796	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.37	Horz(TL)	0.16	9	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 353 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-8 oc bracing.

REACTIONS (lb/size) 9=1920/Mechanical, 2=1818/0-3-8 (min. 0-1-8)
Max Horz 2=71(LC 8)
Max Uplift 9=-843(LC 4), 2=-859(LC 8)
Max Grav 9=2279(LC 2), 2=2156(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4005/1696, 3-23=-3550/1562, 23-24=-3550/1562, 4-24=-3550/1562, 4-25=-5365/2144, 25-26=-5365/2144, 26-27=-5365/2144, 5-27=-5365/2144, 5-28=-5349/2079, 6-28=-5349/2079, 6-29=-5349/2079, 29-30=-5349/2079, 7-30=-5349/2079, 7-31=-3556/1375, 31-32=-3556/1375, 32-33=-3556/1375, 8-33=-3556/1375, 8-9=-4187/1593
BOT CHORD 2-16=-1500/3494, 16-34=-2066/5012, 34-35=-2066/5012, 15-35=-2066/5012, 15-36=-2066/5012, 14-36=-2066/5012, 14-37=-2239/5664, 37-38=-2239/5664, 38-39=-2239/5664, 13-39=-2239/5664, 13-40=-1924/5004, 12-40=-1924/5004, 12-41=-1924/5004, 41-42=-1924/5004, 42-43=-1924/5004, 11-43=-1924/5004, 11-44=-1272/3503, 10-44=-1272/3503, 10-45=-1363/3662, 9-45=-1363/3662
WEBS 3-16=-587/1392, 4-16=-1856/712, 4-14=-92/615, 5-14=-451/215, 5-13=-475/310, 7-13=-159/633, 7-11=-1839/806, 8-11=-464/1332

NOTES (12-15)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 843 lb uplift at joint 9 and 859 lb uplift at joint 2.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

Continued on page 2

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T08	Hip Truss	1	1	I6380637
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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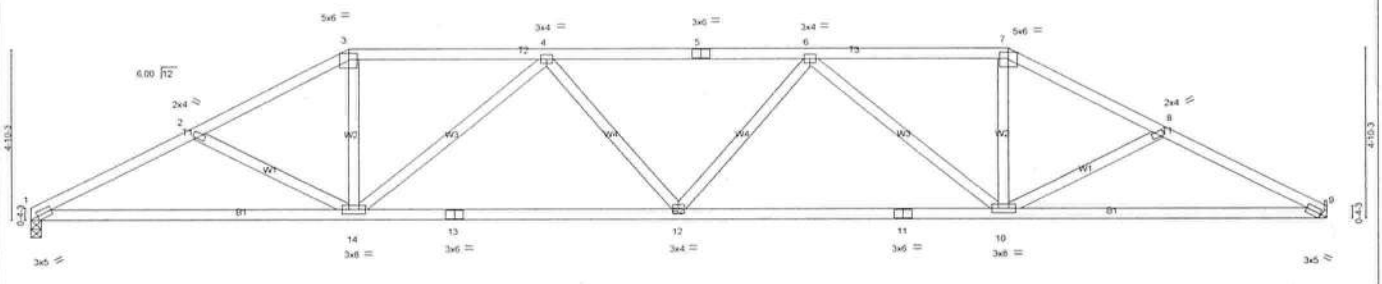
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-FpETdanue_GC?YLSm3HkjNiSNPJNwlZ5myUSpSznBkF

4-9-5	9-0-0	14-7-9	22-0-15	27-8-8	31-11-3	36-8-8
4-9-5	4-2-11	5-7-9	7-5-6	5-7-9	4-2-11	4-9-5

Scale = 1:61.8



9-0-0	18-4-4	27-8-8	36-8-8
9-0-0	9-4-4	9-4-4	9-0-0

Plate Offsets (X,Y): [1:0-2-10,0-1-8], [3:0-3-0,0-2-0], [7:0-3-0,0-2-0], [9:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.27	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.46	10-12	>961	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.14	9	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 177 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-10 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=991/0-3-8 (min. 0-1-8), 9=991/Mechanical
Max Horz 1=61(LC 11)
Max Uplift 1=-254(LC 9), 9=-254(LC 8)
Max Grav 1=1175(LC 2), 9=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2279/1314, 2-3=-2042/1171, 3-4=-1822/1107, 4-5=-2387/1403, 5-6=-2387/1403, 6-7=-1822/1107, 7-8=-2042/1171, 8-9=-2279/1314
BOT CHORD 1-14=-1094/1976, 13-14=-1183/2284, 12-13=-1183/2284, 11-12=-1183/2284, 10-11=-1183/2284, 9-10=-1094/1976
WEBS 2-14=-267/271, 3-14=-331/643, 4-14=-743/414, 6-10=-743/414, 7-10=-331/643, 8-10=-267/271

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 254 lb uplift at joint 9.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE	I6380639
466999	T10	Hip Truss	1	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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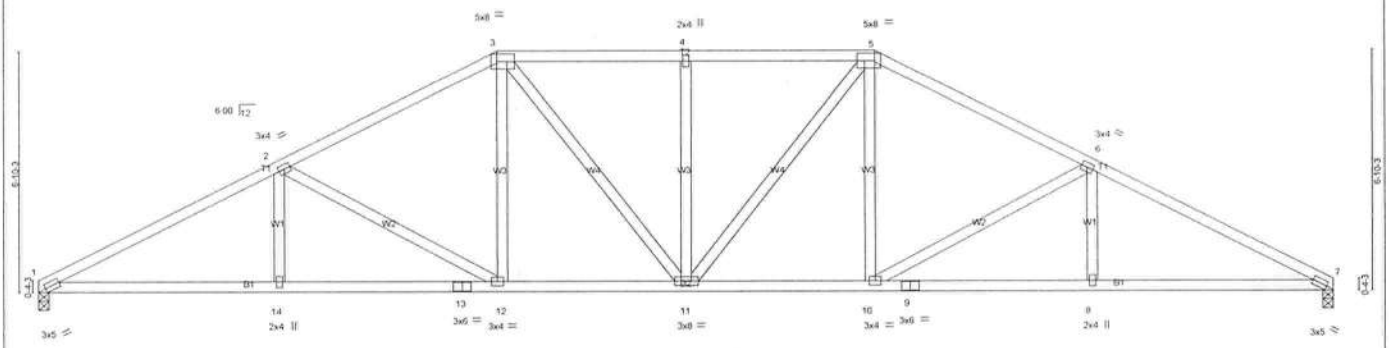
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-7bT_TyqPhDndUAeE?uMguDt8B1lasdjghaSfyDznBkB

6-9-14	13-0-0	18-4-4	23-8-8	29-10-10	36-8-8
6-9-14	6-2-2	5-4-4	5-4-4	6-2-2	6-9-14

Scale = 1:61.8



6-9-14	13-0-0	18-4-4	23-8-8	29-10-10	36-8-8
6-9-14	6-2-2	5-4-4	5-4-4	6-2-2	6-9-14

Plate Offsets (X,Y): [1:0-2-10,0-1-8], [3:0-6-0,0-2-8], [5:0-6-0,0-2-8], [7:0-2-10,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.20	11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.29	11-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.12	7	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 196 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=991/0-3-8 (min. 0-1-8), 7=991/0-3-8 (min. 0-1-8)
Max Horz 1=-88(LC 8)
Max Uplift 1=-256(LC 12), 7=-256(LC 13)
Max Grav 1=1175(LC 2), 7=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2246/1261, 2-3=-1814/1084, 3-4=-1733/1118, 4-5=-1733/1118, 5-6=-1814/1084, 6-7=-2246/1261
BOT CHORD 1-14=-1068/1928, 13-14=-1021/1928, 12-13=-1021/1928, 11-12=-687/1496, 10-11=-687/1496, 9-10=-1021/1928, 8-9=-1021/1928, 7-8=-1068/1928
WEBS 2-12=-500/384, 3-12=-153/323, 3-11=-130/355, 4-11=-322/230, 5-11=-130/355, 5-10=-153/323, 6-10=-500/384

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 1 and 256 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T12	Hip Truss	1	1	I6380641
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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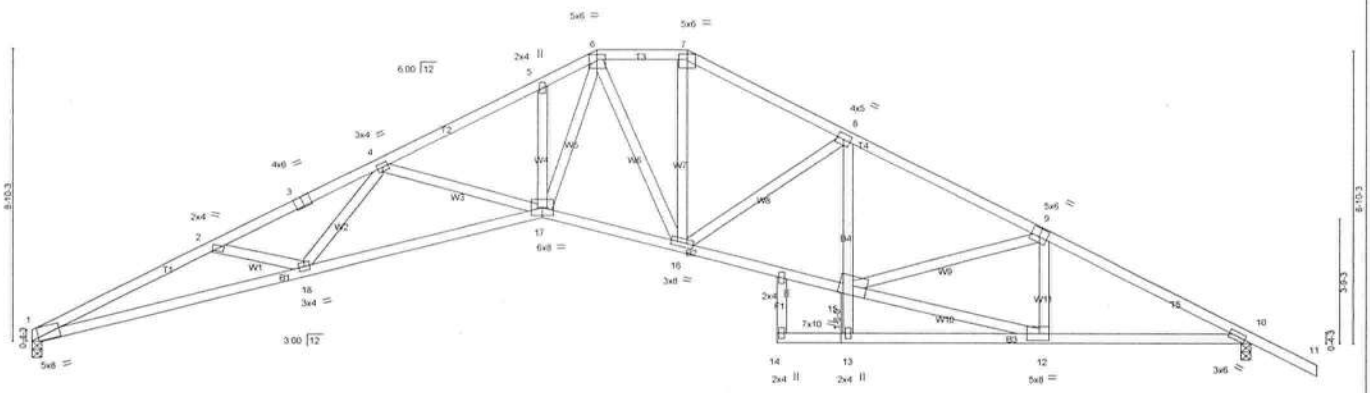
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-0MiVJJtvlSH3yny?EkQc231nYe_DoO6GbBQt5_znBk7

5-7-1	10-6-9	15-4-4	17-0-0	19-8-8	24-5-0	30-5-12	36-8-8	38-8-8
5-7-1	4-11-8	4-9-11	1-7-12	2-8-8	4-8-8	6-0-12	6-2-12	2-0-0

Scale = 1:65.7



8-2-3	15-4-4	19-8-8	22-5-0	24-5-0	30-5-12	36-8-8
8-2-3	7-2-1	4-4-4	2-8-8	2-0-0	6-0-12	6-2-12

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,Edge], [6:0-3-0,0-1-12], [7:0-3-0,0-2-0], [9:0-3-0,0-3-0], [10:0-2-9,0-1-8], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.55	17-18	>801	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.89	17-18	>495	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.50	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 211 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B4: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
10-0-0 oc bracing: 13-15
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=993/0-3-8 (min. 0-1-8), 10=1099/0-3-8 (min. 0-1-10)
Max Horz 1=-133(LC 13)
Max Uplift 1=-273(LC 12), 10=-310(LC 13)
Max Grav 1=1175(LC 2), 10=1304(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-4382/2383, 2-3=-4062/2142, 3-4=-4018/2153, 4-5=-3235/1659, 5-6=-3236/1761, 6-7=-1998/1174, 7-8=-2254/1250, 8-9=-2517/1380, 9-10=-2295/1264
BOT CHORD 1-18=-2049/4025, 17-18=-1630/3562, 16-17=-780/2240, 15-16=-966/2287, 10-12=-979/2245
WEBS 2-18=-297/334, 4-18=-156/360, 4-17=-666/527, 6-17=-983/1984, 6-16=-575/208, 7-16=-397/783, 8-16=-357/336, 12-15=-975/2031, 9-12=-468/291

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 1 and 310 lb uplift at joint 10.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

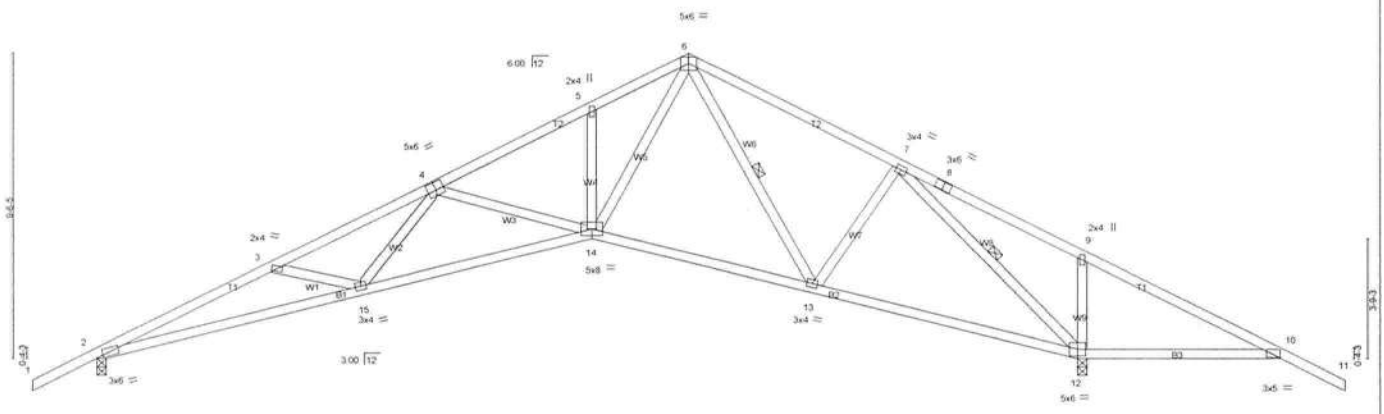
Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T14	Scissor Truss	4	1	I6380643
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 — 7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:59:05 2013 Page 1

ID:jRhrov9QzLs40H7EpCZ11VyVpE7-QxOexLvo2NfepEhavs_JghfLQr4g?IiiI9eXhJznBk4

-2-0-0	5-7-1	10-6-9	15-4-4	18-4-4	24-9-11	30-5-0	36-8-8	38-8-8
2-0-0	5-7-1	4-11-8	4-9-11	3-0-0	6-5-7	5-7-5	6-3-8	2-0-0

Scale = 1:67.7



8-2-3	15-4-4	22-3-14	30-5-0	36-8-8
8-2-3	7-2-1	6-11-11	8-1-2	6-3-8

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [10:0-5-4,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.30	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.54	14-15	>679	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.32	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 190 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-13 oc bracing.
WEBS 1 Row at midpt 6-13, 7-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=854/0-3-8 (min. 0-1-8), 12=1304/0-3-8 (min. 0-1-15)
Max Horz 2=127(LC 16)
Max Uplift 2=-279(LC 12), 12=-391(LC 13)
Max Grav 2=1015(LC 2), 12=1550(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3327/1547, 3-4=-2994/1321, 4-5=-2142/830, 5-6=-2137/933, 6-7=-1223/478, 7-8=-793/778, 8-9=-812/654, 9-10=-989/772
BOT CHORD 2-15=-1276/3058, 14-15=-871/2571, 13-14=-113/1126, 12-13=-84/885, 10-12=-670/1338
WEBS 3-15=-350/346, 4-15=-169/382, 4-14=-696/537, 6-14=-627/1559, 6-13=-307/193, 7-13=-88/326, 7-12=-1873/1218, 9-12=-342/339

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 2 and 391 lb uplift at joint 12.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

February 8,2013

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T16	Common Truss	2	1	I6380645
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:jRhrov9QzLs40H7EpCZ11VyVpE7-MJWOM0x2a_vM3Yry0H0nl6kgsfpVTnW?IT7emBznBk2

-2-0-0 2-0-0 | 8-0-0 8-0-0 | 16-0-0 8-0-0 | 18-0-0 2-0-0

Scale = 1:32.9

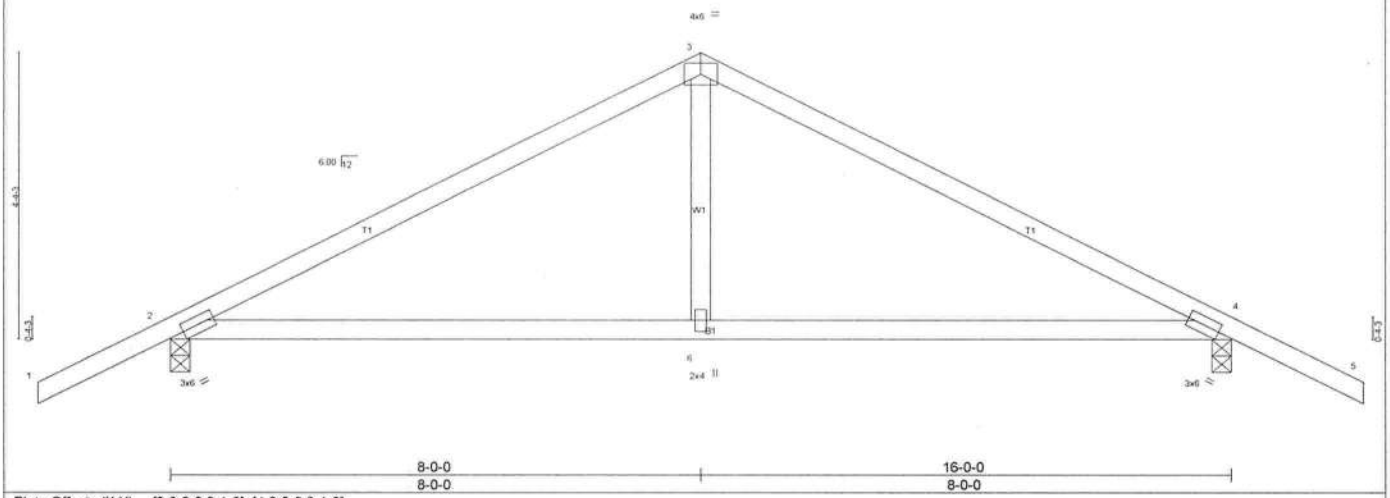

Plate Offsets (X,Y): [2:0-2-9,0-1-8], [4:0-2-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	0.21	6-12	>902	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	0.18	6-12	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.17	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 63 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-6 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=520/0-3-8 (min. 0-1-8), 4=520/0-3-8 (min. 0-1-8)
Max Horz 2=-62(LC 13)
Max Uplift2=-257(LC 9), 4=-257(LC 8)
Max Grav 2=620(LC 2), 4=620(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1346/1561, 3-4=-1356/1566
BOT CHORD 2-6=-2012/1798, 4-6=-2039/1852
WEBS 3-6=-465/240

NOTES (8-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2 and 257 lb uplift at joint 4.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T17G	Common Truss	1	1	I6380647
					Job Reference (optional)

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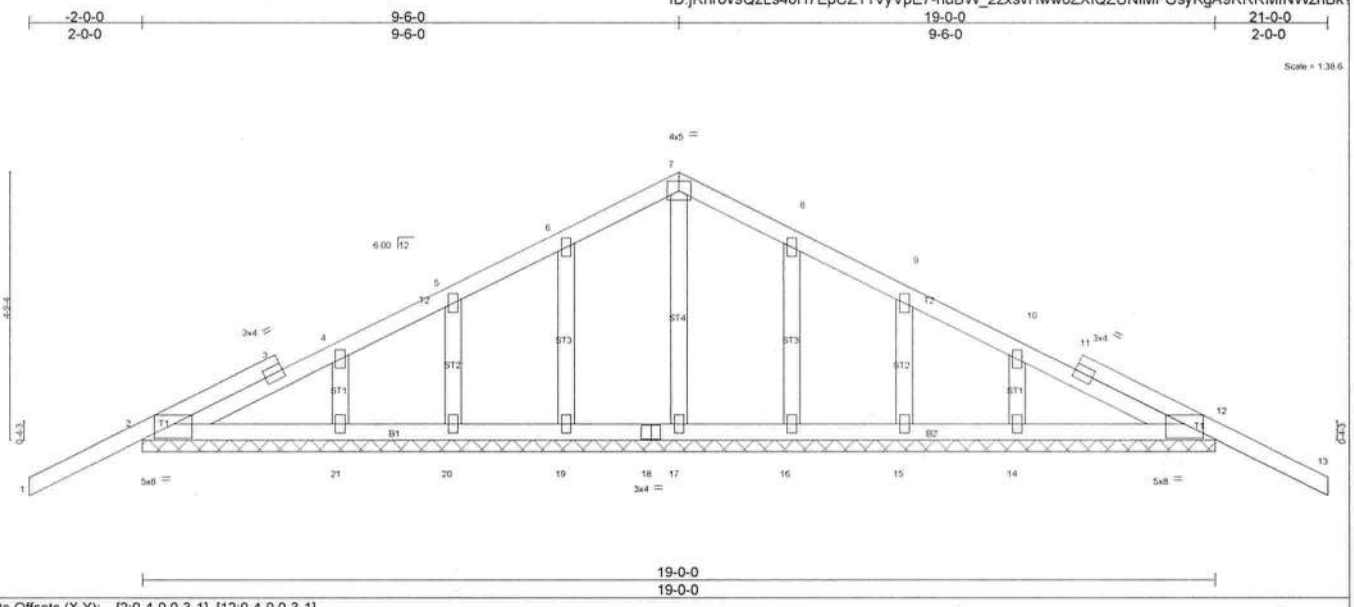

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [12:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.02	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.03	13	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix)							Weight: 99 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 19-0-0.
(lb) - Max Horz 2=114(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 19, 20, 21, 16, 15 except 2=-113(LC 12), 12=-132(LC 13), 14=-101(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 12, 17, 19, 20, 21, 16, 15, 14

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (13-15)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 20, 21, 16, 15 except (jt=lb) 2=113, 12=132, 14=101.
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

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Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T18	Common Truss	1	2	I6380648
					Job Reference (optional)

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-44, 4-6=-44, 2-6=-10

Concentrated Loads (lb)

Vert: 14=-761(B) 15=-431(B) 16=-437(B) 17=-431(B) 18=-431(B) 19=-418(B)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T19	COMMON TRUSS	1	2	I6380649
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055 7.350 s Jul 31 2012 MiTek Industries, Inc. Fri Feb 08 12:59:13 2013 Page 2

ID:jRhrov9QzLs40H7EpCZ11VyVpE7-BTtfc4?p9qgVnTI6NY7B?N_IJ4ottMlu7PayzrznBjy

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-44, 4-6=-44, 2-6=-10
   Concentrated Loads (lb)
   Vert: 14=-1909(F) 15=-981(F) 16=-1048(F)

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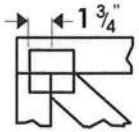
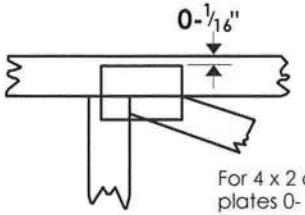
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

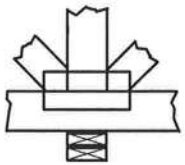
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

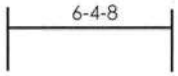


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

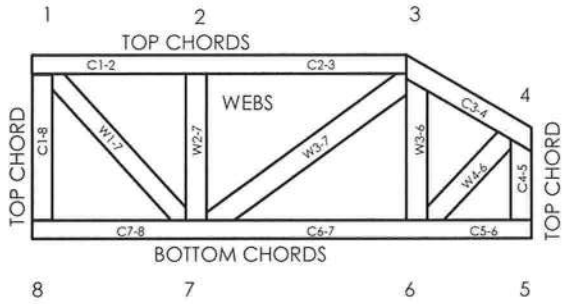
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B,
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

August 10, 2010	T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY	ST - T-BRACE 2

MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

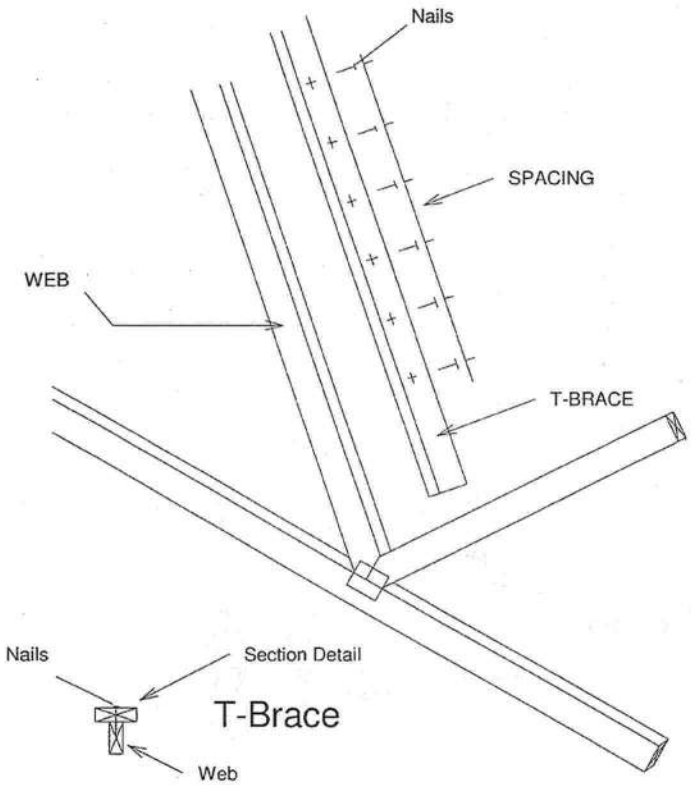
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

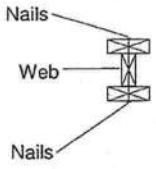
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





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JANUARY 1, 2009	LATERAL TOE-NAIL DETAIL	ST-TOENAIL_SP

MiTek Industries, Inc.

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

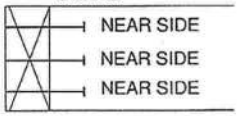
VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

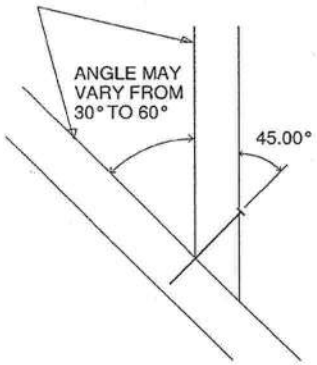
SIDE VIEW

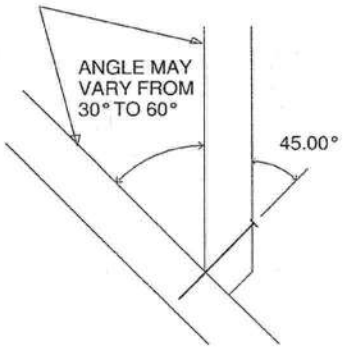
3 NAILS

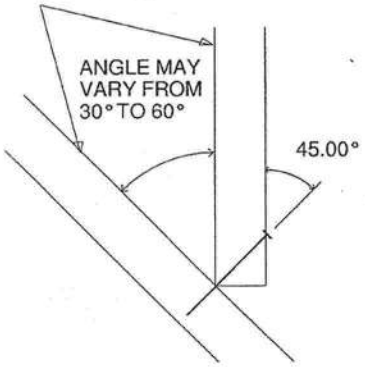


EXAMPLE:
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







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FEBRUARY 14, 2012	STANDARD PIGGYBACK TRUSS CONNECTION DETAIL	ST-PIGGY-7-10

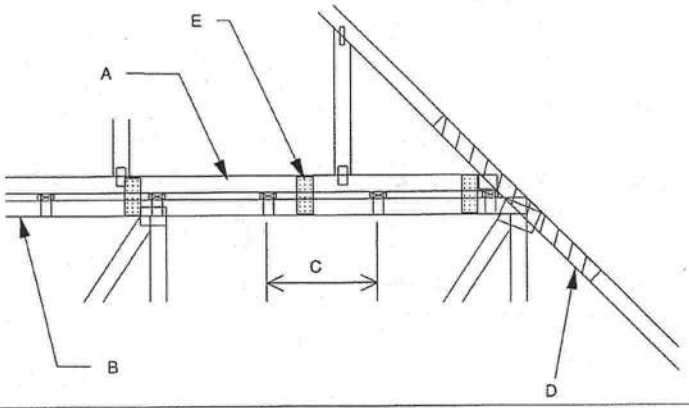
MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10 DURATION OF LOAD INCREASE : 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

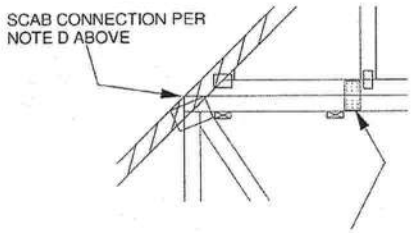
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
D - 2 X___ X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

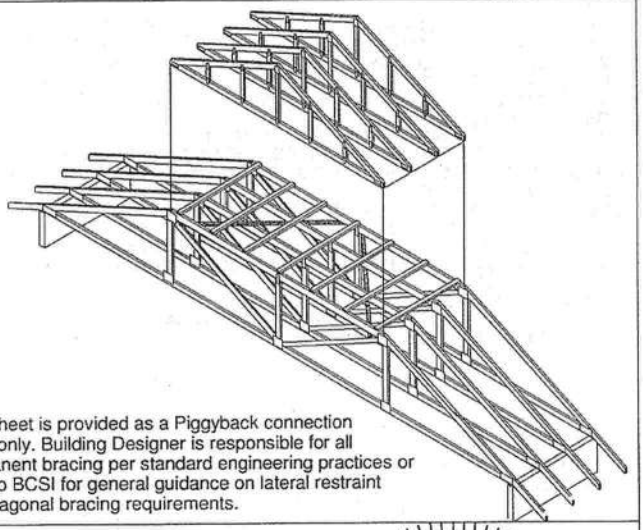


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

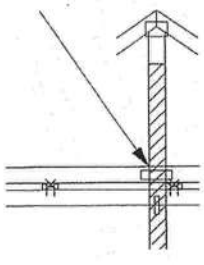


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

1109 COASTAL BAY
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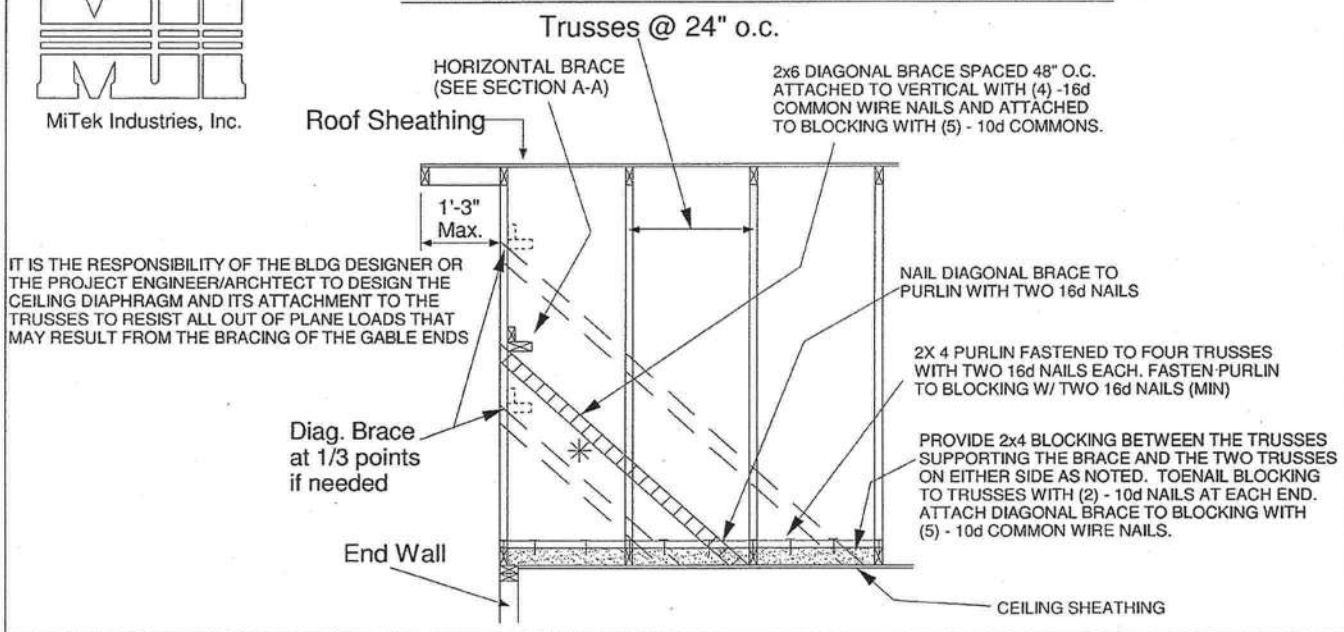
FEBRUARY 14, 2012	Standard Gable End Detail	SHEET 2

MiTek Industries, Inc.

ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

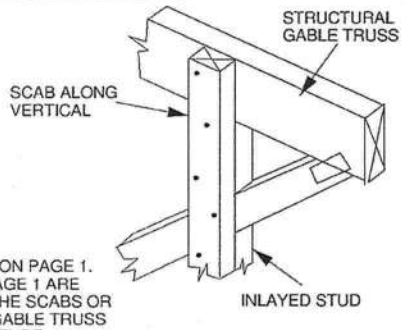
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1 : ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

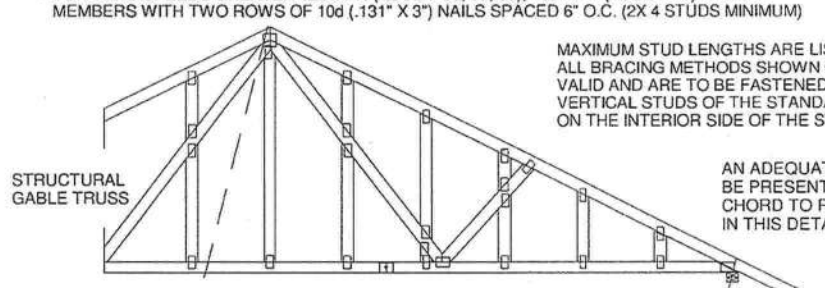
METHOD 2 : ATTACH 2X_ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

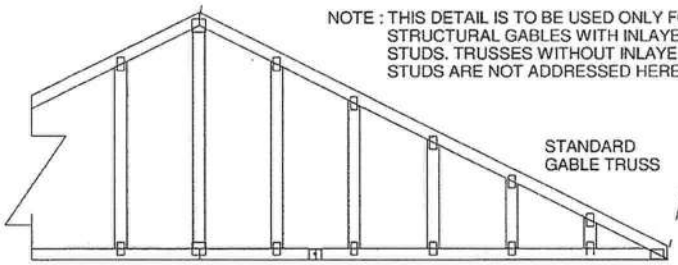


MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE : THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS. TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE.



JULIUS S.K. LEE LICENSE No 34869 3/29/12 STATE OF FLORIDA PROFESSIONAL ENGINEER

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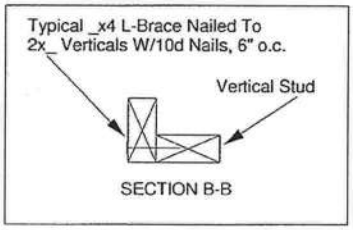
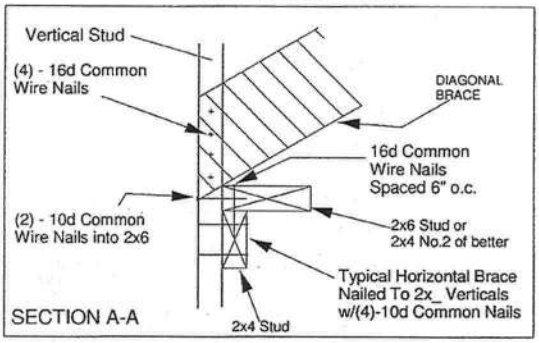
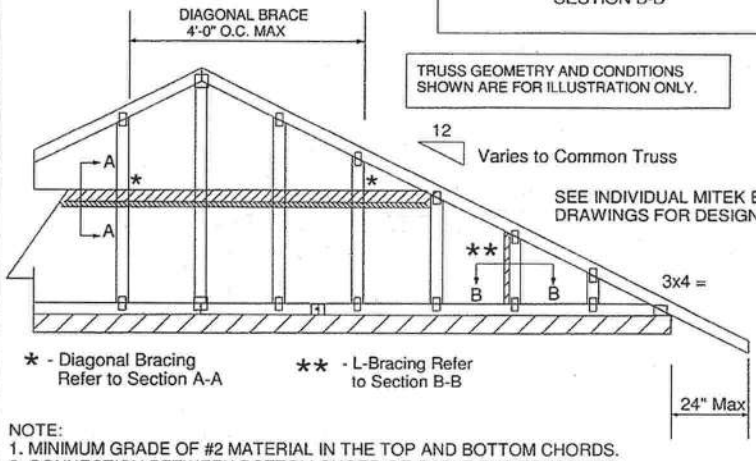
BOYNTON BC, FL 33435

FEBRUARY 14, 2012	Standard Gable End Detail	ST-GE130-001

MiTek Industries, Inc.







TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

12 Varies to Common Truss

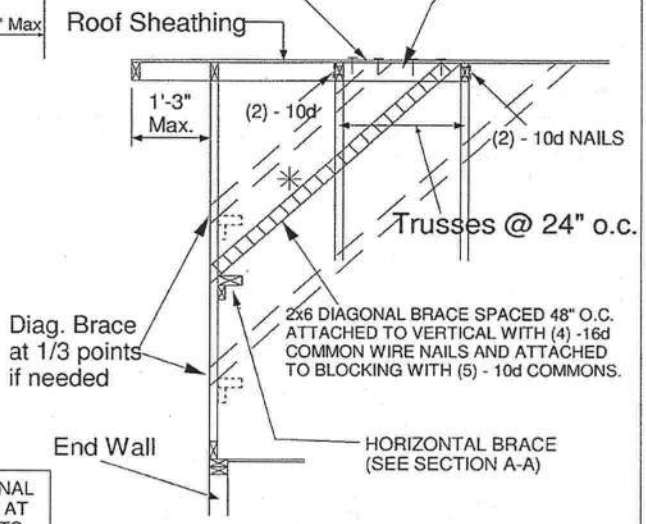
SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK



NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

✳ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 8, 2008	LATERAL BRACING RECOMMENDATIONS	ST-STRGBCK

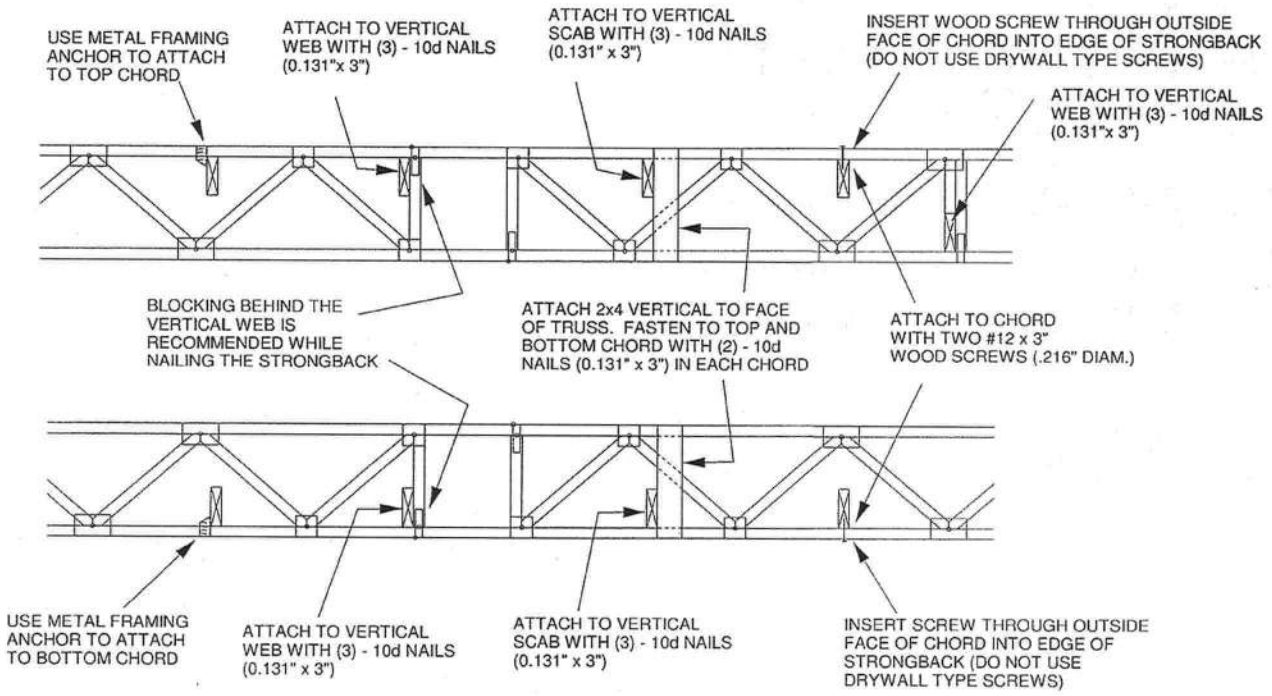
MiTek Industries, Chesterfield, MO

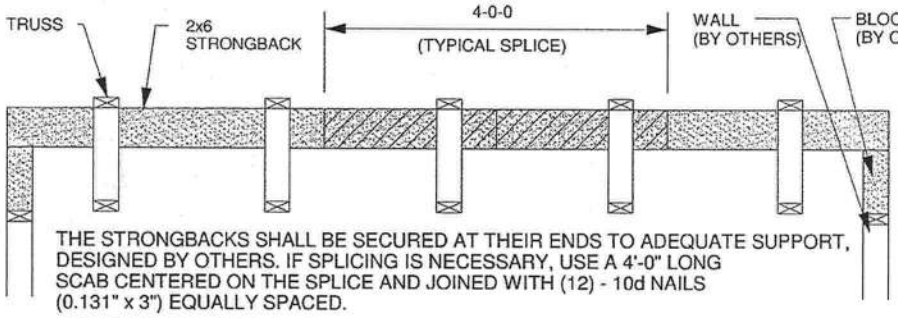
MiTek Industries, Inc.

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

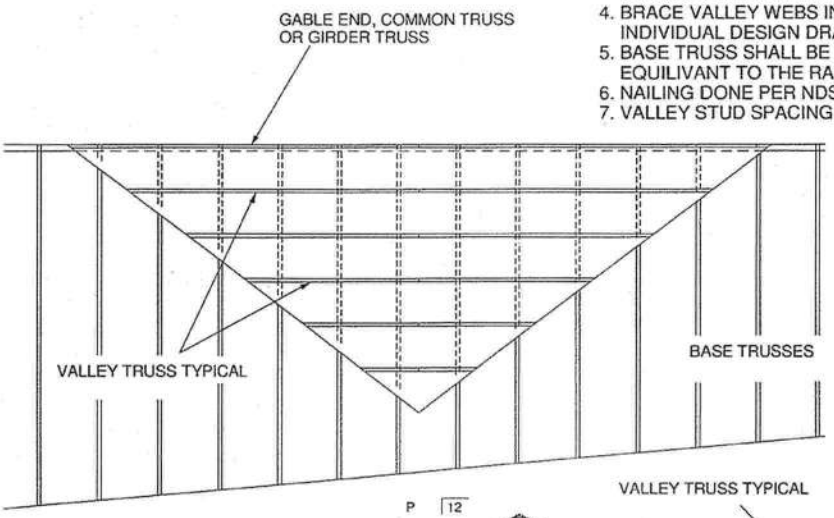
1109 COASTAL BAY
BOYNTON BC, FL 33435

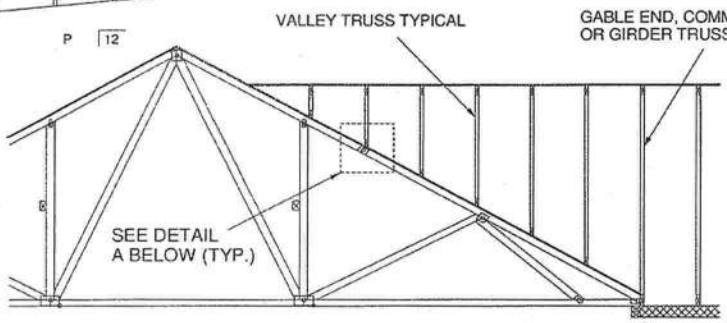
FEBRUARY 14, 2012	TRUSSED VALLEY SET DETAIL	ST-VALLEY HIGH WIND1

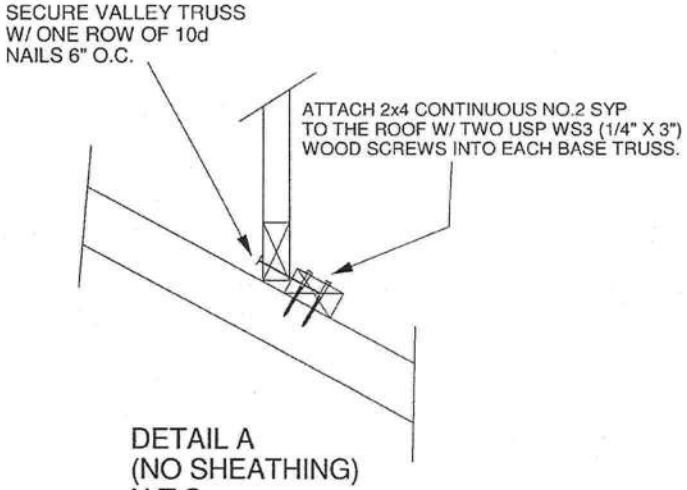
MiTek Industries, Inc.

GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
   DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
   SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
   INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
   EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.







DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES

JULIUS S.K. LEE
LICENSE
No 34869
3/29/12
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

1109 COASTAL BAY
BOYNTON BC, FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T19G	Common Truss	1	1	I6380650
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:jRhrov9QzLs40H7EpCZ11VyVpE7-fgR1qQ0Rw8oMOdtIxFeQXbXwNTJTc_A1M3KWVHznBjx

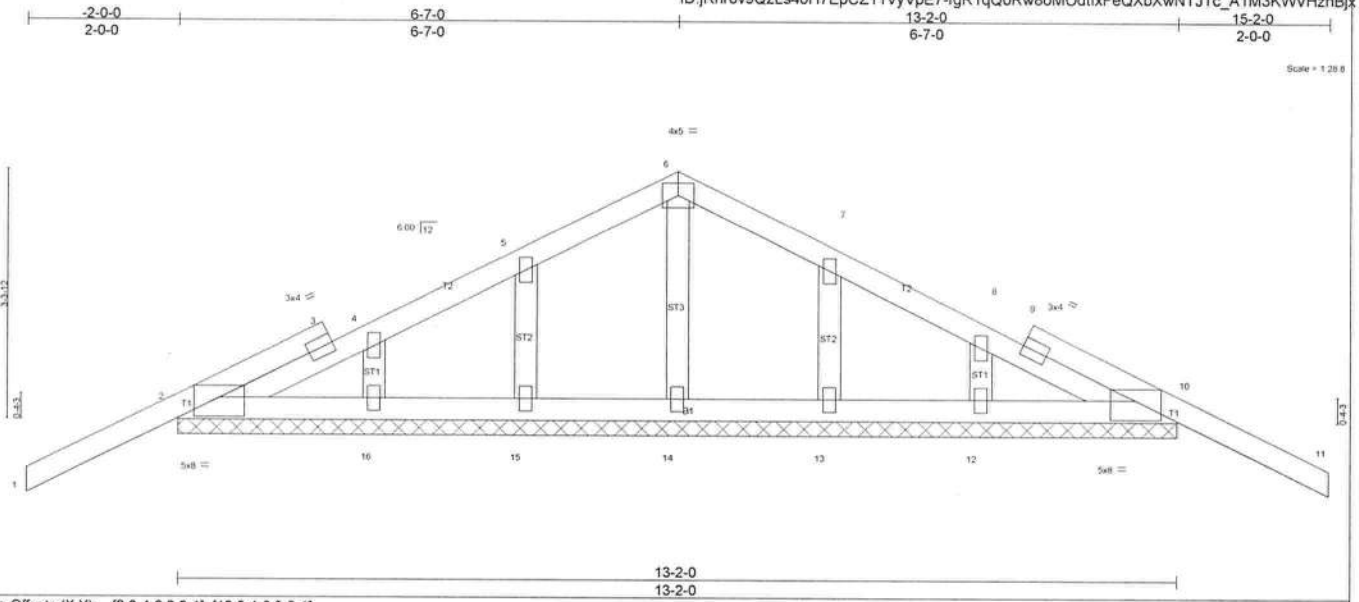

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [10:0-4-0,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.02	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.04	11	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix)							Weight: 65 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 13-2-0.
(lb) - Max Horz 2=83(LC 12)
Max Uplift All uplift 100 lb or less at joint(s) 16, 12 except 2=-124(LC 12), 10=-137(LC 13), 15=-107(LC 12), 13=-106(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 10, 14, 15, 16, 13, 12

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 12 except (jt=lb) 2=124, 10=137, 15=107, 13=106.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T19	COMMON TRUSS	1	2	I6380649

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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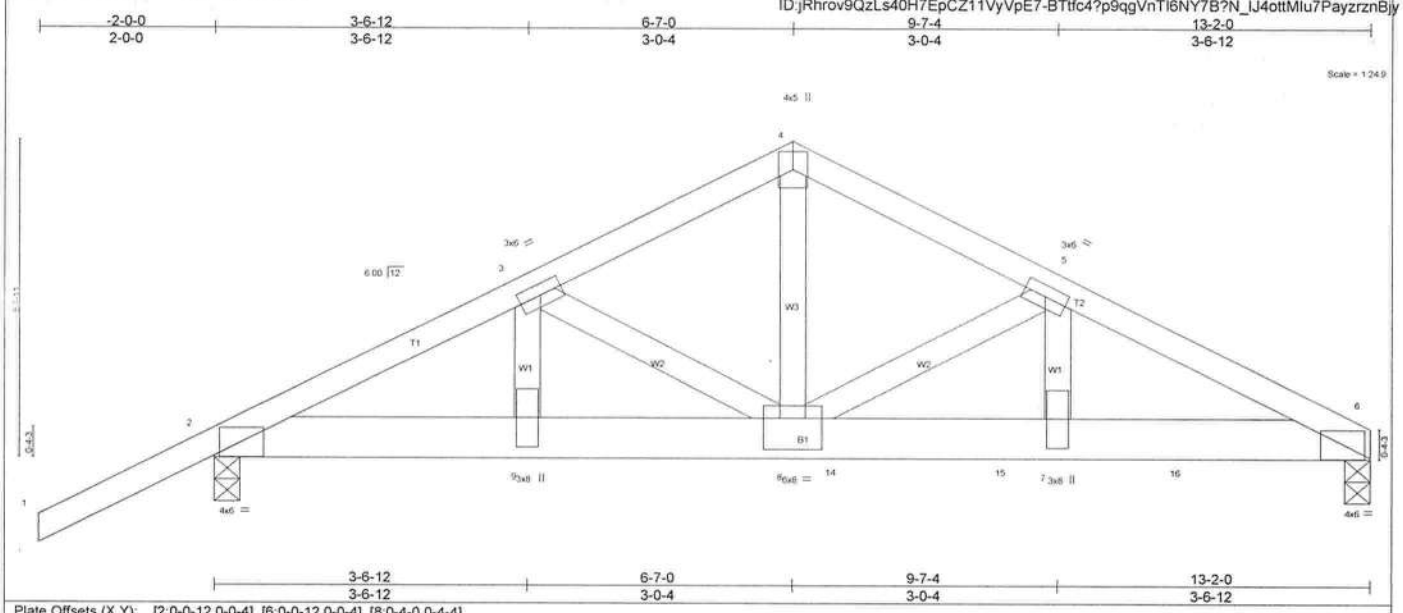

Plate Offsets (X,Y): [2:0-0-12,0-0-4], [6:0-0-12,0-0-4], [8:0-4-0,0-4-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.08	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.15	7-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 148 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=2938/0-3-8 (min. 0-2-0), 2=1799/0-3-8 (min. 0-1-8)
Max Horz 2=111(LC 8)
Max Uplift 6=-1025(LC 9), 2=-765(LC 8)
Max Grav 6=3421(LC 2), 2=2127(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4105/1358, 3-4=-4260/1435, 4-5=-4258/1427, 5-6=-6701/2040
BOT CHORD 2-9=-1227/3647, 8-9=-1227/3647, 8-14=-1772/5956, 14-15=-1772/5956, 7-15=-1772/5956, 7-16=-1772/5956, 6-16=-1772/5956
WEBS 4-8=-1174/3580, 5-8=-2505/726, 5-7=-516/2172, 3-8=-256/290, 3-9=-285/154

NOTES (11-13)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-5-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=1025, 2=765.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2268 lb down and 852 lb up at 7-1-9, and 1165 lb down and 264 lb up at 9-0-12, and 1165 lb down and 251 lb up at 11-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

Continued on page 2

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T18	Common Truss	1	2	I6380648

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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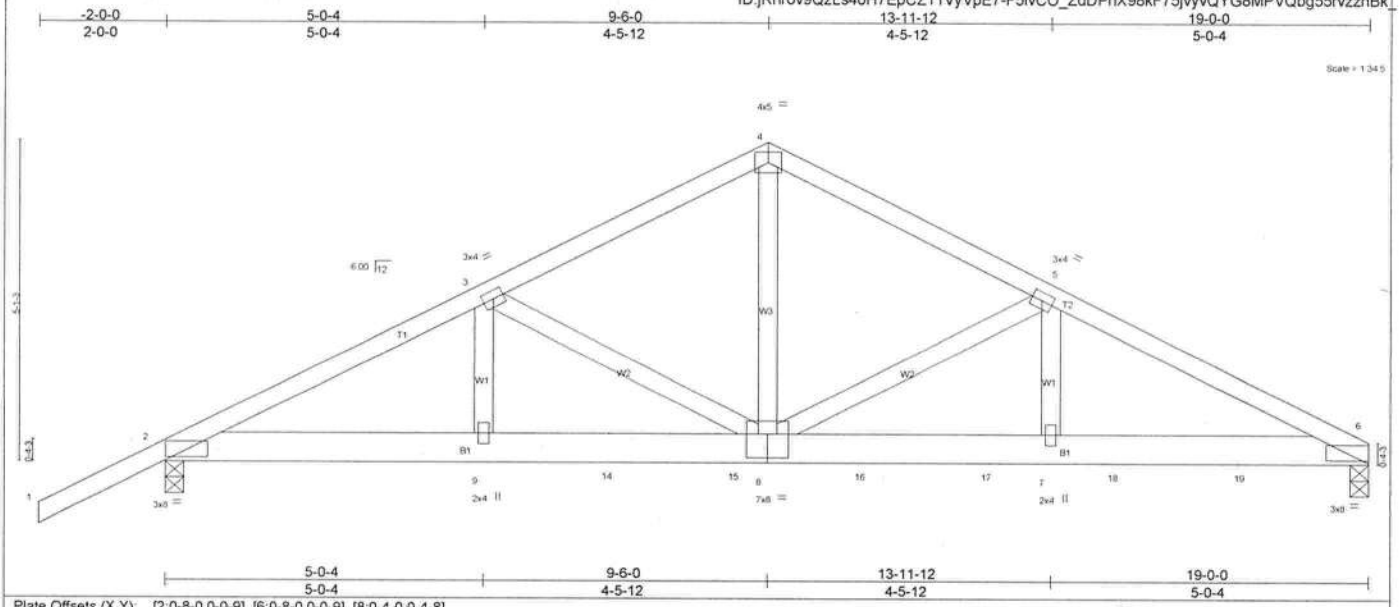

Plate Offsets (X,Y): [2:0-8-0,0-0-9], [6:0-8-0,0-0-9], [8:0-4-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.09	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.17	8-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 213 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SYP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=2264/0-3-8 (min. 0-1-9), 2=1758/0-3-8 (min. 0-1-8)
Max Horz 2=141(LC 8)
Max Uplift6=-929(LC 9), 2=-798(LC 8)
Max Grav 6=2683(LC 2), 2=2091(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-4236/1503, 3-4=-3401/1211, 4-5=-3405/1208, 5-6=-5056/1728
BOT CHORD 2-9=-1369/3744, 9-14=-1369/3744, 14-15=-1369/3744, 8-15=-1369/3744, 8-16=-1476/4487, 16-17=-1476/4487, 7-17=-1476/4487, 7-18=-1476/4487, 18-19=-1476/4487, 6-19=-1476/4487
WEBS 4-8=-941/2755, 5-8=-1712/685, 5-7=-366/1285, 3-8=-863/453, 3-9=-174/558

NOTES (11-13)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=929, 2=798.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 904 lb down and 343 lb up at 7-0-12, 514 lb down and 173 lb up at 9-0-12, 517 lb down and 157 lb up at 11-0-12, 512 lb down and 163 lb up at 13-0-12, and 510 lb down and 164 lb up at 15-0-12, and 492 lb down and 155 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T17	Common Truss	1	1	I6380646
					Job Reference (optional)

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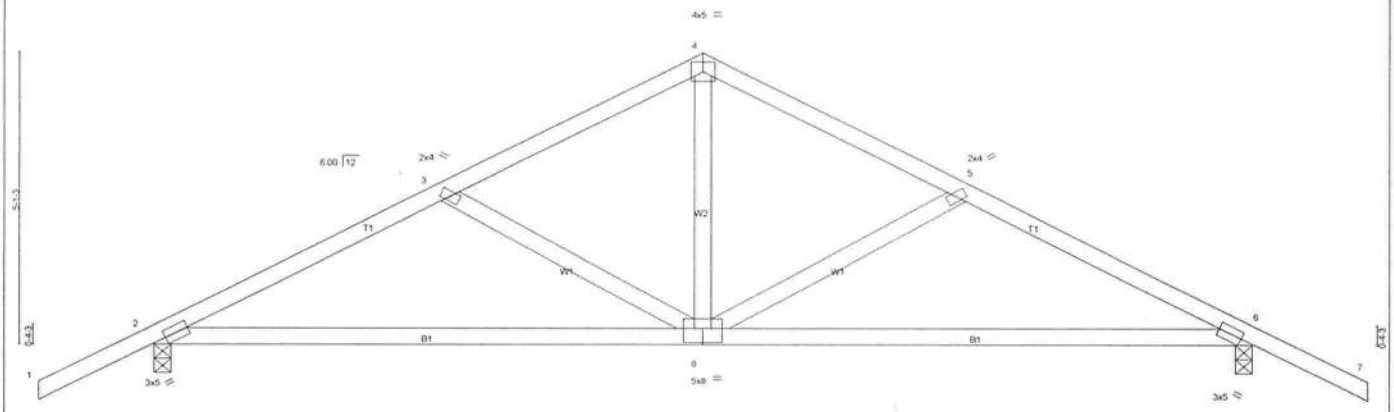
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-qW4mZMxgLI1DgiP9a?X0IKHvo384CEd9_7tBleznBk1

-2-0-0	5-1-7	9-6-0	13-10-9	19-0-0	21-0-0
2-0-0	5-1-7	4-4-9	4-4-9	5-1-7	2-0-0

Scale = 1:37.8



9-6-0	19-0-0
9-6-0	9-6-0

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-2-10,0-1-8], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.10	8-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.19	8-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 88 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-11-2 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=601/0-3-8 (min. 0-1-8), 6=601/0-3-8 (min. 0-1-8)
Max Horz 2=121(LC 12)
Max Uplift 2=-332(LC 12), 6=-332(LC 13)
Max Grav 2=716(LC 2), 6=716(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1046/625, 3-4=-769/475, 4-5=-769/475, 5-6=-1061/625
BOT CHORD 2-8=-426/1241, 6-8=-431/1314
WEBS 4-8=-225/401, 5-8=-322/295, 3-8=-323/295

NOTES (8-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 2 and 332 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T15	Hip Truss	1	1	I6380644
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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ID:jRhrov9QzLs40H7EpCZ11VyVpE7-u7y08hwQpgnVROGmTaVYCvCWyFU0kKgsWpO4ElznBk3

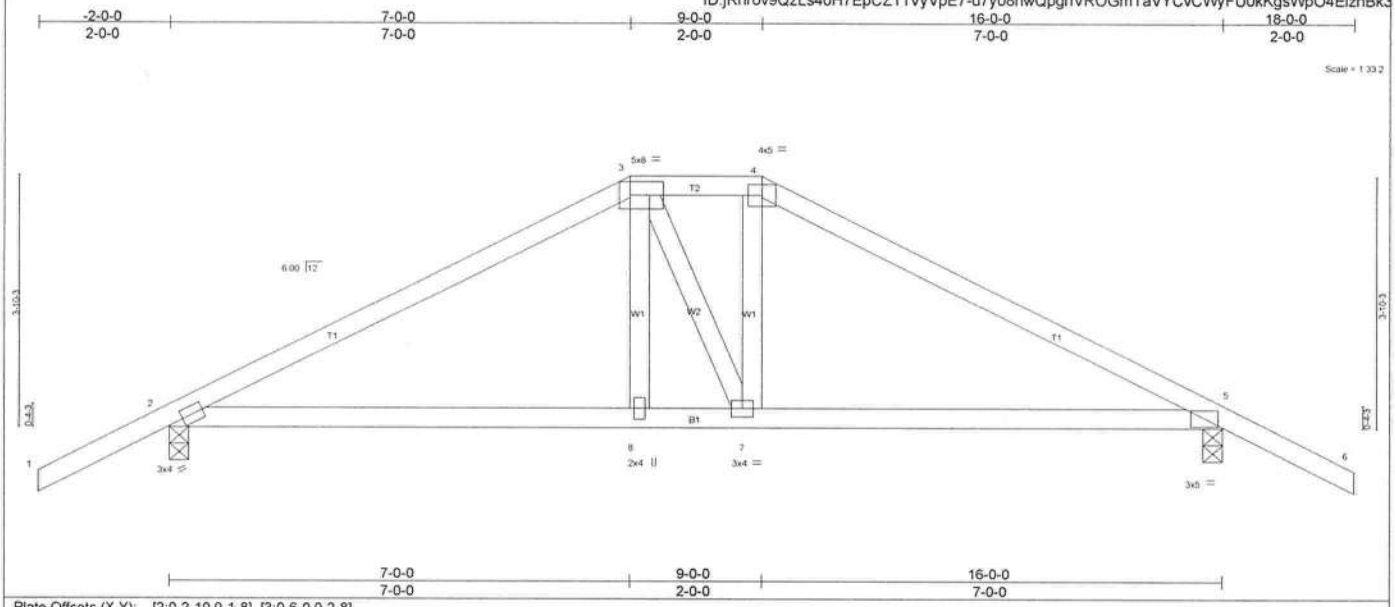

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	0.10	8-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.11	8-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 72 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-2 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=828/0-3-8 (min. 0-1-8), 5=837/0-3-8 (min. 0-1-8)
Max Horz 2=56(LC 12)
Max Uplift2=-612(LC 5), 5=-626(LC 4)
Max Grav 2=984(LC 2), 5=995(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1448/1032, 3-4=-1249/986, 4-5=-1475/1065
BOT CHORD 2-8=-883/1215, 7-8=-895/1226, 5-7=-889/1239
WEBS 3-8=-304/356, 4-7=-281/346

NOTES (11-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 612 lb uplift at joint 2 and 626 lb uplift at joint 5.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, and 178 lb down and 238 lb up at 9-0-0 on top chord, and 241 lb down and 262 lb up at 7-0-0, and 241 lb down and 262 lb up at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-44, 3-4=-44, 4-6=-44, 9-12=-10
Concentrated Loads (lb)
Vert: 3=-69(B) 4=-146(B) 8=-205(B) 7=-205(B)

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T13	Special Truss	3	1	I6380642
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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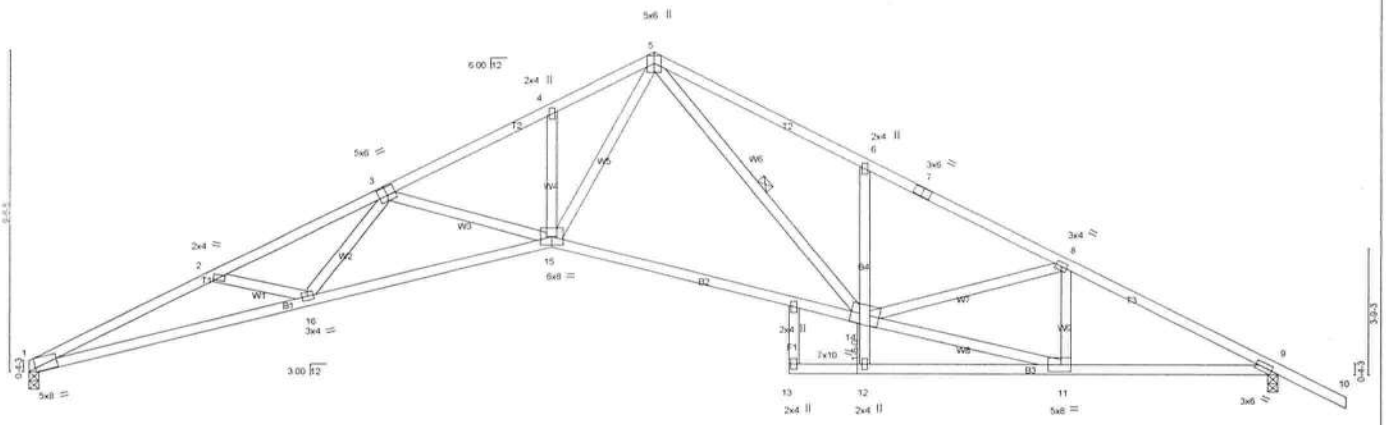
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-UYGtWftYWlPwaxXBnRxrbGaze2KPXqmPqr9QdQznBk6

5-7-1	10-6-9	15-4-4	18-4-4	24-5-0	30-5-12	36-8-8	38-8-8
5-7-1	4-11-8	4-9-11	3-0-0	6-0-12	6-0-12	6-2-12	2-0-0

Scale: 3/16"=1'



8-2-3	15-4-4	22-4-0	24-5-0	30-5-12	36-8-8
8-2-3	7-2-1	6-11-12	2-1-0	6-0-12	6-2-12

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [9:0-2-9,0-1-8], [11:0-3-8,0-2-8], [14:0-3-3,0-3-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	0.56	15-16	>793	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.98	14-15	>451	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.49	9	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 202 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B4: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
10-0-0 oc bracing: 12-14
WEBS 1 Row at midpt 5-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=993/0-3-8 (min. 0-1-8), 9=1099/0-3-8 (min. 0-1-10)
Max Horz 1=-142(LC 8)
Max Uplift 1=-279(LC 12), 9=-317(LC 13)
Max Grav 1=1175(LC 2), 9=1304(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-4392/2401, 2-3=-4077/2176, 3-4=-3262/1676, 4-5=-3256/1779, 5-6=-2646/1603, 6-7=-2461/1403, 7-8=-2565/1392, 8-9=-2304/1274
BOT CHORD 1-16=-2064/4034, 15-16=-1657/3593, 14-15=-636/1908, 6-14=-413/388, 9-11=-988/2269
WEBS 2-16=-291/327, 3-16=-152/335, 3-15=-666/534, 5-15=-994/2067, 5-14=-563/689, 11-14=-962/2051, 8-11=-486/295

NOTES (9-11)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 1 and 317 lb uplift at joint 9.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE	
466999	T11	Hip Truss	1	1	Job Reference (optional)	I6380640

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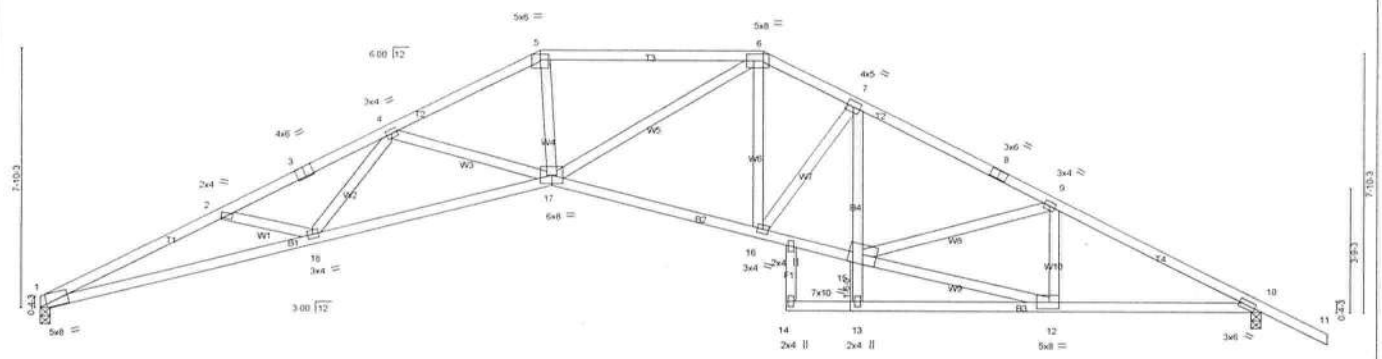
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-3zbludrfDq1LjToc6JO8zeyR9qJgKUhz8txm05znBk9

5-7-1	10-6-8	15-0-0	21-8-8	24-5-0	30-5-12	36-8-8	38-8-8
5-7-1	4-11-7	4-5-8	6-8-8	2-8-8	6-0-12	6-2-12	2-0-0

Scale = 1:65.7



8-2-3	15-4-4	21-8-8	22-5-0	24-5-0	30-5-12	36-8-8
8-2-3	7-2-1	6-4-4	0-8-8	2-0-0	6-0-12	6-2-12

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,Edge], [5:0-3-0,0-2-0], [6:0-6-0,0-2-8], [10:0-2-9,0-1-8], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.53	17-18	>826	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.87	17-18	>506	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.49	10	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 202 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except*
T3: 2x4 SYP No.1
BOT CHORD 2x4 SP No.2 *Except*
B4: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. Except:
10-0-0 oc bracing: 13-15

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=993/0-3-8 (min. 0-1-8), 10=1099/0-3-8 (min. 0-1-10)
Max Horz 1=-120(LC 13)
Max Uplift 1=-262(LC 12), 10=-299(LC 13)
Max Grav 1=1175(LC 2), 10=1303(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-4366/2358, 2-3=-4036/2111, 3-4=-3996/2121, 4-5=-3223/1632, 5-6=-2957/1553, 6-7=-2361/1321, 7-8=-2438/1357, 8-9=-2497/1337, 9-10=-2293/1257
BOT CHORD 1-18=-2026/4012, 17-18=-1594/3532, 16-17=-792/2127, 15-16=-934/2257, 10-12=-976/2194
WEBS 2-18=-311/343, 4-18=-164/359, 4-17=-638/509, 5-17=-500/1127, 6-17=-394/1000, 6-16=-203/311, 12-15=-969/2011, 9-15=-42/251, 9-12=-464/290

NOTES (10-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 1 and 299 lb uplift at joint 10.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T09	Hip Truss	1	1	I6380638
					Job Reference (optional)

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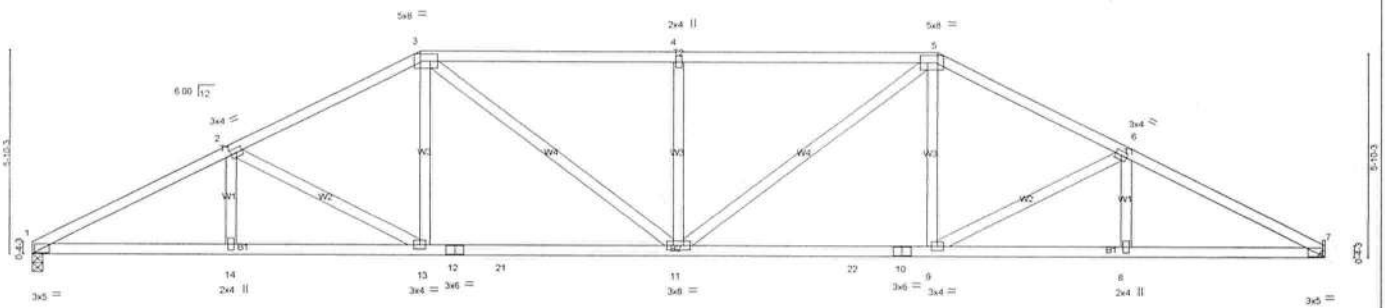
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-BCLE2Go99cWvEsVrtTJCoonneD2QOI4NDGzZtKznBkD

5-7-11	11-0-0	18-4-4	25-8-8	31-0-13	36-8-8
5-7-11	5-4-5	7-4-4	7-4-4	5-4-5	5-7-11

Scale = 1:62.1



5-7-11	11-0-0	18-4-4	25-8-8	31-0-13	36-8-8
5-7-11	5-4-5	7-4-4	7-4-4	5-4-5	5-7-11

Plate Offsets (X,Y): [3:0-6-0,0-2-8], [5:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.23	11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.34	11-13	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.12	7	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 188 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1058/0-3-8 (min. 0-1-8), 7=1058/Mechanical
Max Horz 1=75(LC 9)
Max Uplift 1=-241(LC 12), 7=-241(LC 13)
Max Grav 1=1175(LC 2), 7=1175(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2411/1274, 2-3=-2087/1132, 3-4=-2206/1269, 4-5=-2206/1269, 5-6=-2087/1132, 6-7=-2411/1274
BOT CHORD 1-14=-1048/2086, 13-14=-1048/2086, 12-13=-778/1766, 12-21=-778/1766, 11-21=-778/1766, 11-22=-778/1766, 10-22=-778/1766, 9-10=-778/1766, 8-9=-1048/2086, 7-8=-1048/2086
WEBS 2-13=-370/308, 3-13=-110/310, 3-11=-244/544, 4-11=-458/342, 5-11=-244/544, 5-9=-110/310, 6-9=-370/308

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 241 lb uplift at joint 1 and 241 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T07	HIP TRUSS	1	2	I6380636
					Job Reference (optional)

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ID:jRhrov9QzLs40H7EpCZ11VyVpE7-IR6jCule6N0UIFB4eeFGeyd8Fcd2Syfole?LkZznBkH

NOTES (12-15)

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 17-0-12, 85 lb down and 92 lb up at 19-0-12, 85 lb down and 92 lb up at 21-0-12, 85 lb down and 92 lb up at 23-0-12, 85 lb down and 92 lb up at 25-0-12, and 85 lb down and 92 lb up at 27-0-12, and 85 lb down and 92 lb up at 29-0-12 on top chord, and 241 lb down and 262 lb up at 7-0-0, 49 lb down at 9-0-12, 49 lb down at 11-0-12, 49 lb down at 13-0-12, 49 lb down at 15-0-12, 49 lb down at 17-0-12, 49 lb down at 19-0-12, 49 lb down at 21-0-12, 49 lb down at 23-0-12, 49 lb down at 25-0-12, 49 lb down at 27-0-12, 49 lb down at 29-0-12, 157 lb down and 84 lb up at 31-0-12, and 157 lb down and 84 lb up at 33-0-12, and 157 lb down and 84 lb up at 35-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

13) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

15) Use Simpson HHUS26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44, 3-8=-44, 8-9=-44, 17-20=-10

Concentrated Loads (lb)

Vert: 3=-69(B) 16=-205(B) 14=-22(B) 10=-131(B) 23=-69(B) 24=-69(B) 25=-69(B) 26=-69(B) 27=-69(B) 28=-69(B) 29=-69(B) 30=-69(B) 31=-69(B) 32=-69(B) 33=-69(B) 34=-22(B) 35=-22(B) 36=-22(B) 37=-22(B) 38=-22(B) 39=-22(B) 40=-22(B) 41=-22(B) 42=-22(B) 43=-22(B) 44=-131(B) 45=-131(B)

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T06	Hip Truss	1	1	I6380635
					Job Reference (optional)

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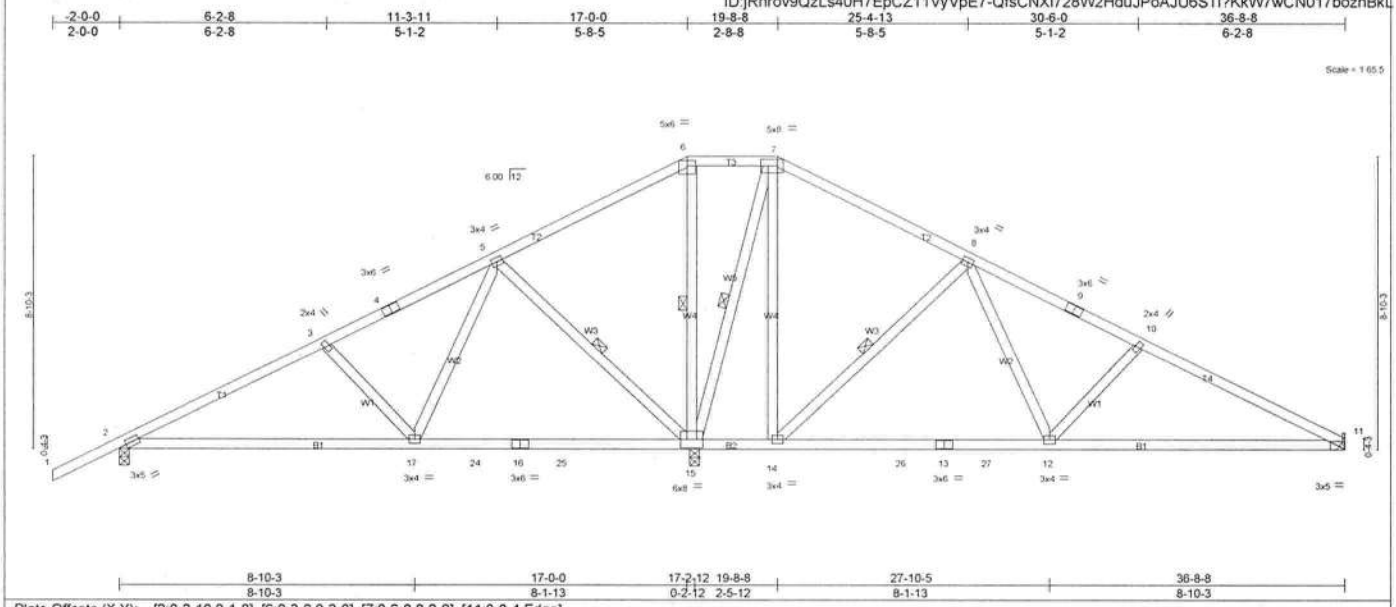

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [6:0-3-0,0-2-0], [7:0-6-0,0-2-8], [11:0-0-4,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.12	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.20	12-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 209 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 12-23.
WEBS 1 Row at midpt 5-15, 6-15, 7-15, 8-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=375/0-3-8 (min. 0-1-8), 15=1469/0-3-8 (min. 0-2-2), 11=383/Mechanical
Max Horz 2=133(LC 16)
Max Uplift2=-135(LC 12), 15=-342(LC 12), 11=-145(LC 13)
Max Grav 2=522(LC 27), 15=1565(LC 2), 11=502(LC 28)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-989/193, 3-4=-293/121, 5-6=-143/558, 6-7=-53/443, 7-8=-13/312, 8-9=-385/284, 9-10=-498/267, 10-11=-768/449
BOT CHORD 2-17=-276/1251, 14-15=-313/378, 14-26=-53/265, 13-26=-53/265, 13-27=-53/265, 12-27=-53/265, 11-12=-620/974
WEBS 3-17=-309/286, 5-17=-177/413, 5-15=-620/448, 6-15=-452/230, 7-15=-867/462, 7-14=-276/499, 8-14=-622/452, 8-12=-184/413, 10-12=-304/288

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 135 lb uplift at joint 2, 342 lb uplift at joint 15 and 145 lb uplift at joint 11.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

February 8,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T04	Hip Truss	1	1	I6380633
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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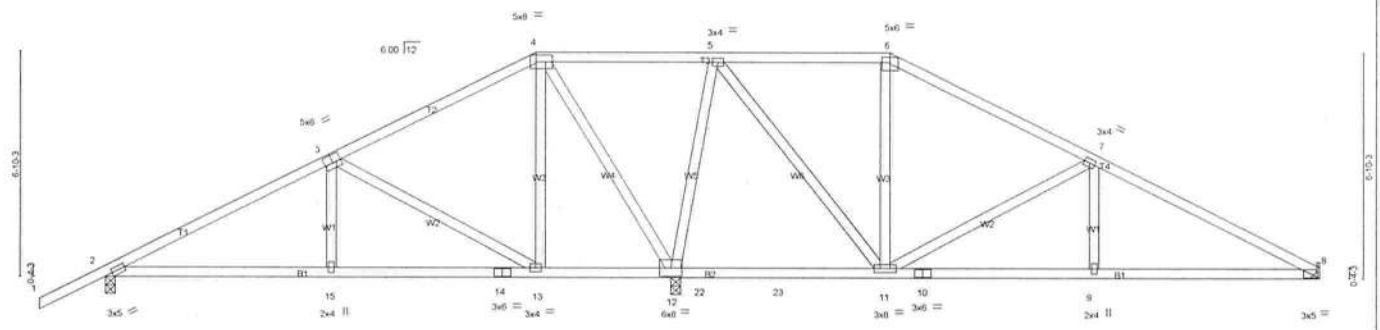
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-04B4kVgFID8TQA9kkgdcsTqyKnMsJg2mh2oT_TznBkO

-2-0-0	6-9-14	13-0-0	18-4-4	23-8-8	29-10-10	36-8-8
2-0-0	6-9-14	6-2-2	5-4-4	5-4-4	6-2-2	6-9-14

Scale = 1:66.1



6-9-14	13-0-0	17-2-12	23-8-8	29-10-10	36-8-8
6-9-14	6-2-2	4-2-12	6-5-12	6-2-2	6-9-14

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-3-0,0-3-0], [4:0-6-0,0-2-8], [6:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.04	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.07	9-18	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 198 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=410/Mechanical, 2=412/0-3-8 (min. 0-1-8), 12=1320/0-3-8 (min. 0-1-15)
Max Horz 2=108(LC 12)
Max Uplift 8=-153(LC 13), 2=-156(LC 12), 12=-258(LC 12)
Max Grav 8=522(LC 28), 2=544(LC 27), 12=1481(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-968/264, 4-5=-96/455, 6-7=-294/193, 7-8=-809/518
BOT CHORD 2-15=-379/1225, 14-15=-131/431, 13-14=-131/431, 12-22=-370/351, 22-23=-370/351, 11-23=-370/351, 10-11=-244/607, 9-10=-244/607, 8-9=-714/1053
WEBS 3-13=-552/401, 4-13=-157/301, 4-12=-730/423, 5-12=-891/538, 5-11=-375/707, 7-11=-532/408

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 8, 156 lb uplift at joint 2 and 258 lb uplift at joint 12.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T02	Half Hip Truss	1	1	I6380631
					Job Reference (optional)

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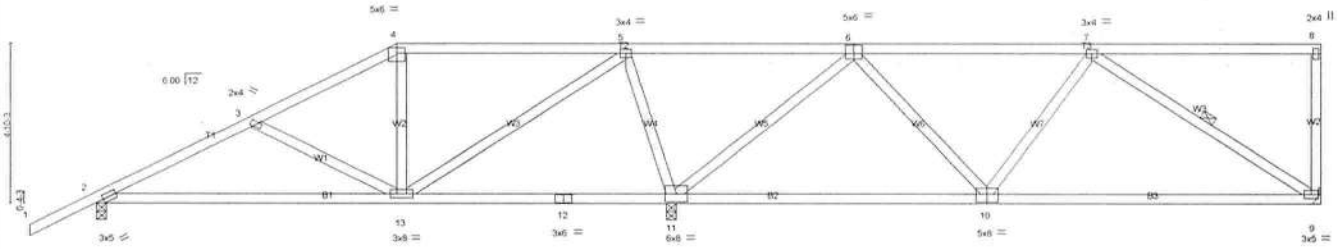
ID:jRhrov9QzLs40H7EpCZ11VyVpE7-3i3JJpe?DcumAs?LcFa8n2lZ1_a1rlLTEkJNwaznBkQ

-2-0-0	4-9-5	9-0-0	15-10-5	22-8-11	29-10-2	36-8-8
2-0-0	4-9-5	4-2-11	6-10-5	6-10-5	7-1-7	6-10-6

Scale = 1:65.6



9-0-0	17-2-12	26-8-8	36-8-8
9-0-0	8-2-12	9-5-12	10-0-1

Plate Offsets (X,Y): [2:0-2-10,0-1-8], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [10:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.23	9-10	>998	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.41	9-10	>564	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 191 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=426/Mechanical, 2=458/0-3-8 (min. 0-1-8), 11=1179/0-3-8 (min. 0-1-11)
Max Horz 2=180(LC 12)
Max Uplift 9=-163(LC 8), 2=-139(LC 12), 11=-415(LC 9)
Max Grav 9=524(LC 28), 2=545(LC 2), 11=1399(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-932/325, 3-4=-375/176, 4-5=-306/203, 5-6=-290/475, 6-7=-447/183
BOT CHORD 2-13=-458/1170, 10-11=-124/283, 9-10=-275/504
WEBS 3-13=-310/281, 5-13=-398/653, 5-11=-813/554, 6-11=-906/539, 6-10=-90/324, 7-9=-582/324

NOTES (9-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 163 lb uplift at joint 9, 139 lb uplift at joint 2 and 415 lb uplift at joint 11.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	T01	Half Hip Truss	1	1	I6380630
					Job Reference (optional)

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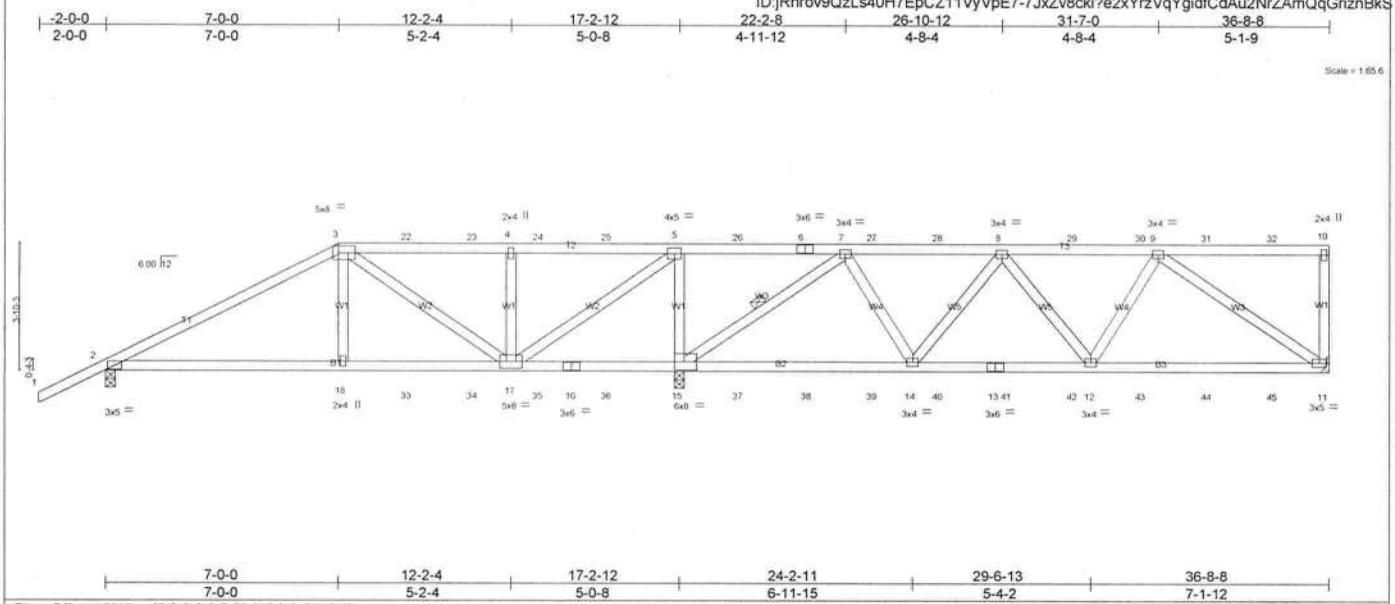

Plate Offsets (X,Y): [3:0-6-0,0-2-8], [15:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.13	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.25	11-12	>937	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.91	Horz(TL)	0.04	11	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 192 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-3-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-6 oc bracing.
WEBS 1 Row at midpt 7-15

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 11=766/Mechanical, 2=647/0-3-8 (min. 0-1-8), 15=2195/0-3-8 (min. 0-3-1)
Max Horz 2=146(LC 8)
Max Uplift 11=-333(LC 4), 2=-344(LC 8), 15=-1065(LC 5)
Max Grav 11=914(LC 22), 2=769(LC 2), 15=2598(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-996/514, 3-22=-324/196, 22-23=-324/196, 4-23=-324/196, 4-24=-324/196, 24-25=-324/196, 5-25=-324/196, 5-26=-433/1009, 6-26=-433/1009, 6-7=-433/1009, 7-27=-739/228, 27-28=-739/228, 8-28=-739/228, 8-29=-1043/352, 29-30=-1043/352, 9-30=-1043/352
BOT CHORD 2-18=-545/910, 18-33=-507/827, 33-34=-507/827, 17-34=-507/827, 17-35=-1009/433, 16-35=-1009/433, 16-36=-1009/433, 15-36=-1009/433, 15-37=-150/432, 37-38=-150/432, 38-39=-150/432, 14-39=-150/432, 14-40=-383/1054, 13-40=-383/1054, 13-41=-383/1054, 41-42=-383/1054, 12-42=-383/1054, 12-43=-371/956, 43-44=-371/956, 44-45=-371/956, 11-45=-371/956
WEBS 3-18=-211/466, 3-17=-610/378, 4-17=-523/305, 5-17=-764/1632, 5-15=-1433/722, 7-15=-1741/706, 7-14=-160/628, 8-14=-532/262, 9-12=0/311, 9-11=-1136/448

NOTES (11-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 333 lb uplift at joint 11, 344 lb uplift at joint 2 and 1065 lb uplift at joint 15.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 92 lb up at 7-0-0, 85 lb down and 92 lb up at 9-0-12, 85 lb down and 92 lb up at 11-0-12, 85 lb down and 92 lb up at 13-0-12, 85 lb down and 92 lb up at 15-0-12, 85 lb down and 92 lb up at 17-0-12, 85 lb down and 92 lb up at 19-0-12, 85 lb down and 92 lb up at 21-0-12, 85 lb down and 92 lb up at 23-0-12, 85 lb down and 92 lb up at 25-0-12, 85 lb down and 92 lb up at 27-0-12, 85 lb down and 92 lb up at 29-0-12, 85 lb down and 92 lb up at 31-0-12, and 85 lb down and 92 lb up at 33-0-12, and 85 lb down and 92 lb up at 35-0-12 on top chord, and 241 lb down and 262 lb up at 7-0-0, 49 lb down at 9-0-12, 49 lb down at 11-0-12, 49 lb down at 13-0-12, 49 lb down at 15-0-12, 49 lb down at 17-0-12, 49 lb down at 19-0-12, 49 lb down at 21-0-12, 49 lb down at 23-0-12, 49 lb down at 25-0-12, 49 lb down at 27-0-12, 49 lb down at 29-0-12, 49 lb down at 31-0-12, and 49 lb down at 33-0-12, and 49 lb down at 35-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	HJ01	Diagonal Hip Girder	4	1	I6380629
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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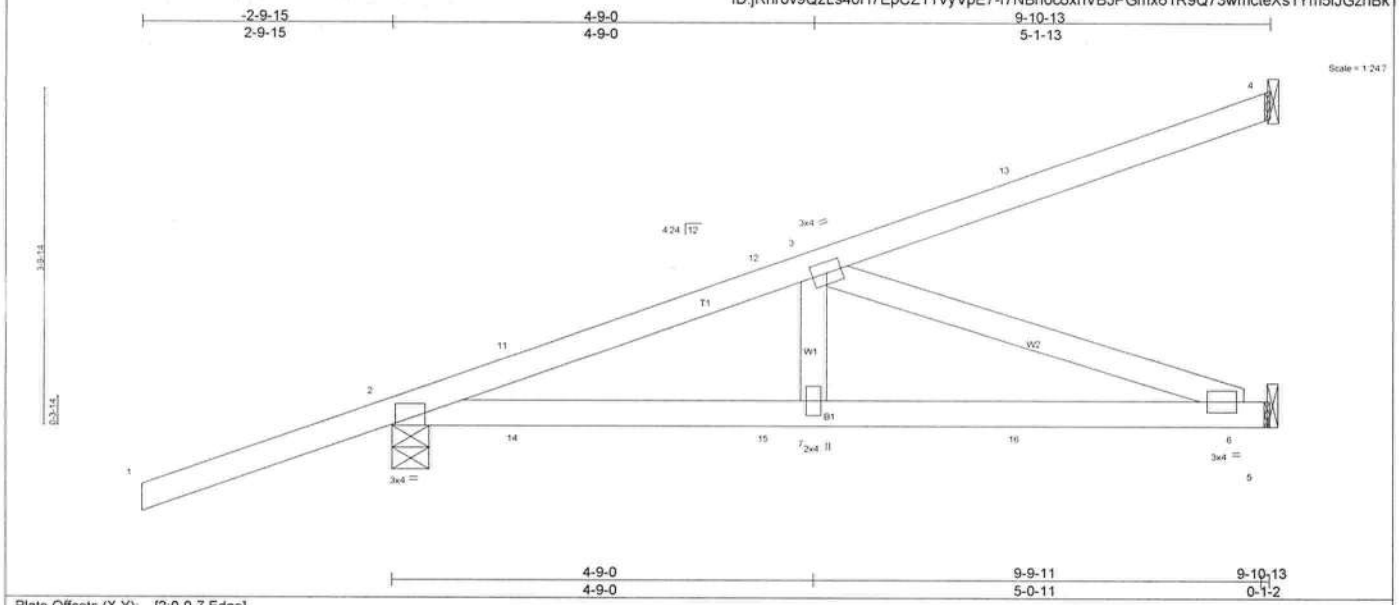

Plate Offsets (X,Y): [2:0-0-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.10	6-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.36	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 44 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-12 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=120/Mechanical, 2=370/0-4-15 (min. 0-1-8), 5=194/Mechanical
Max Horz 2=234(LC 4)
Max Uplift 4=-139(LC 4), 2=-376(LC 4), 5=-259(LC 4)
Max Grav 4=147(LC 2), 2=454(LC 2), 5=225(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-11=-622/511, 11-12=-570/533, 3-12=-571/518
BOT CHORD 2-14=-614/570, 14-15=-614/570, 7-15=-614/570, 7-16=-614/570, 6-16=-614/570
WEBS 3-6=-604/651

NOTES (9-11)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 4, 376 lb uplift at joint 2 and 259 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb down and 47 lb up at 1-5-12, 37 lb down and 47 lb up at 1-5-12, 3 lb down and 50 lb up at 4-3-11, 3 lb down and 50 lb up at 4-3-11, and 43 lb down and 93 lb up at 7-1-10, and 43 lb down and 93 lb up at 7-1-10 on top chord, and 22 lb down and 26 lb up at 1-5-12, 22 lb down and 26 lb up at 1-5-12, 3 lb down and 31 lb up at 4-3-11, 3 lb down and 31 lb up at 4-3-11, and 26 lb down and 45 lb up at 7-1-10, and 26 lb down and 45 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 5-8=-10

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	EJ01	Jack-Partial Truss	29	1	I6380627
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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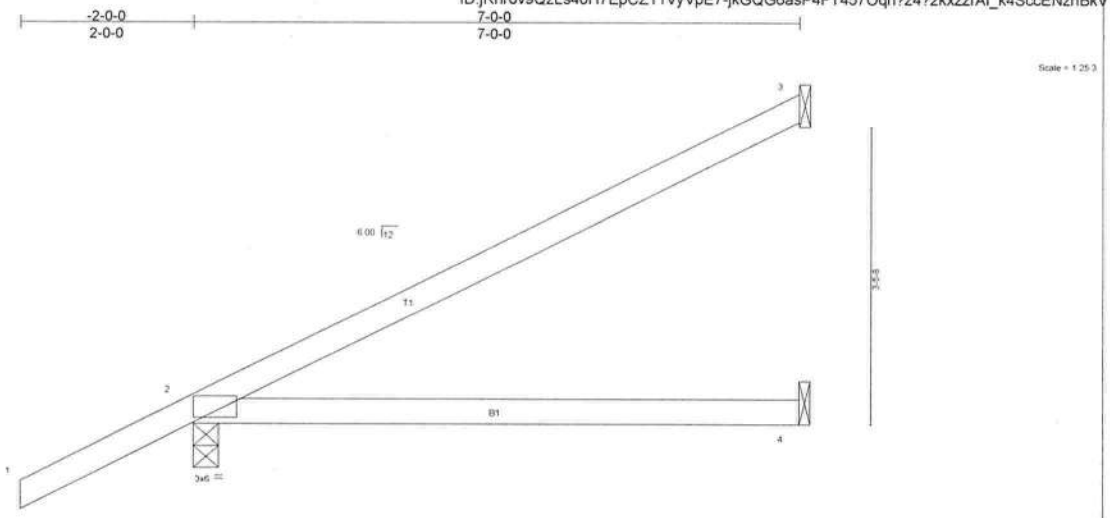

Plate Offsets (X,Y): [2:0-6-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.06	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.11	4-7	>745	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 26 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=113/Mechanical, 2=318/0-3-8 (min. 0-1-8), 4=32/Mechanical
Max Horz 2=144(LC 12)
Max Uplift3=-86(LC 12), 2=-97(LC 12)
Max Grav 3=139(LC 2), 2=380(LC 2), 4=79(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1212/532
BOT CHORD 2-4=-1041/1713

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3 and 97 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

Julius Lee PE.
1109 Coastal Bay
Boynton Beach,FL 33435

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HSE
466999	CJ02	Jack-Open Truss	8	1	I6380625
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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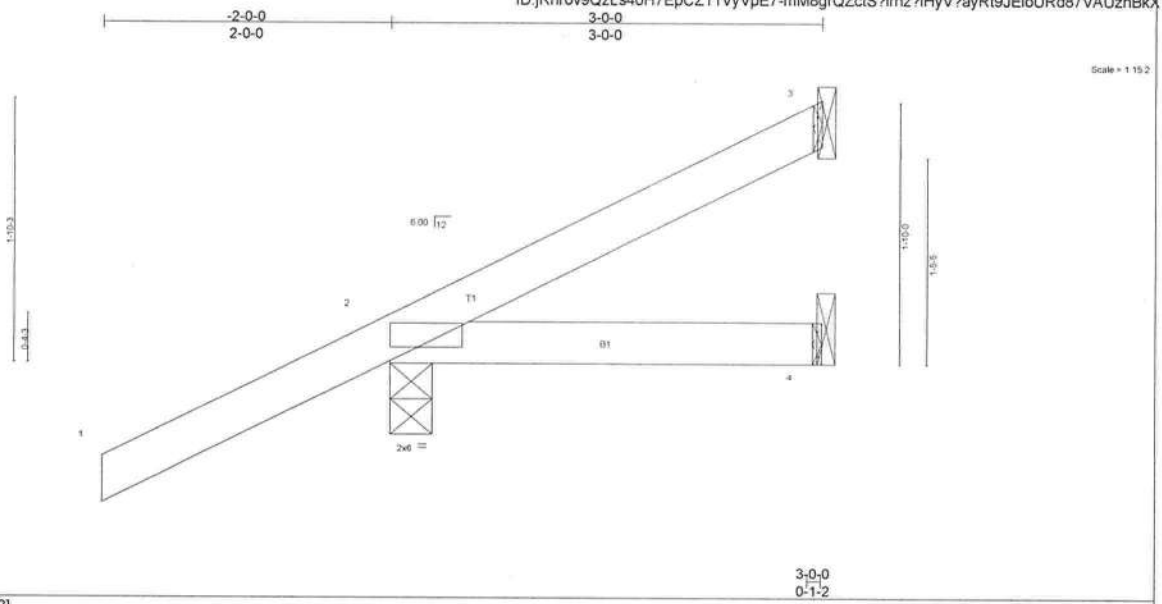

Plate Offsets (X,Y): [2:0-6-0,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.01	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.13	Vert(TL)	-0.01	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2010/TPI2007		(Matrix-M)							Weight: 13 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=42/Mechanical, 2=193/0-3-8 (min. 0-1-8), 4=11/Mechanical
Max Horz 2=113(LC 12)
Max Uplift3=-50(LC 12), 2=-121(LC 12), 4=-21(LC 9)
Max Grav 3=52(LC 2), 2=233(LC 2), 4=33(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-508/112
BOT CHORD 2-4=-106/642

NOTES (7-9)
1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 3, 121 lb uplift at joint 2 and 21 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

February 8,2013

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