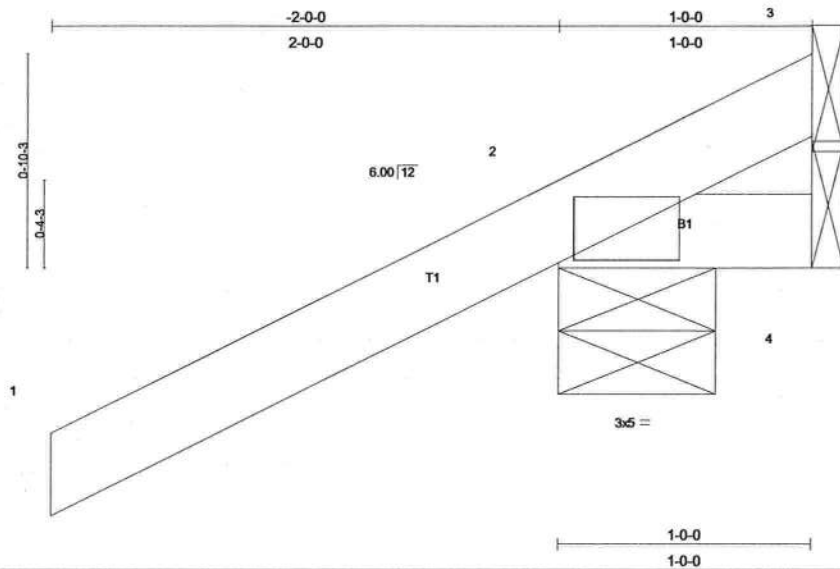


Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - WILLIAMS RES.
298148	CJ1	JACK	8	1	298148001
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.300 s Apr 19 2006 MITek Industries, Inc. Fri Feb 20 14:03:24 2009 Page 1



Scale = 1/8"

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	2'-0"	TC 0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

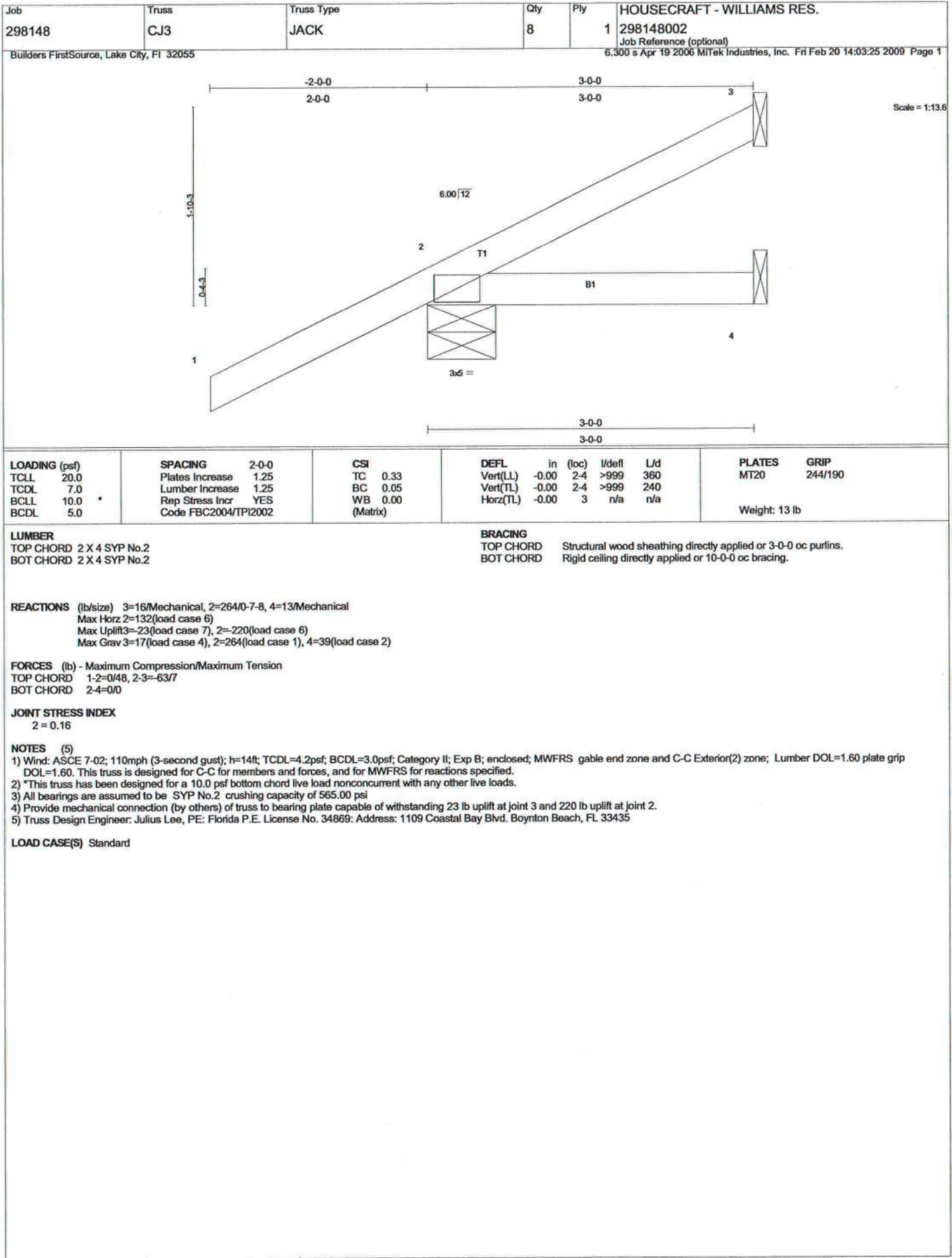
REACTIONS (lb/size) 2=265/0-7-8, 4=5/Mechanical, 3=99/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=285(load case 6), 3=99(load case 1)
Max Grav 2=265(load case 1), 4=14(load case 2), 3=137(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=74/84
BOT CHORD 2-4=0/0

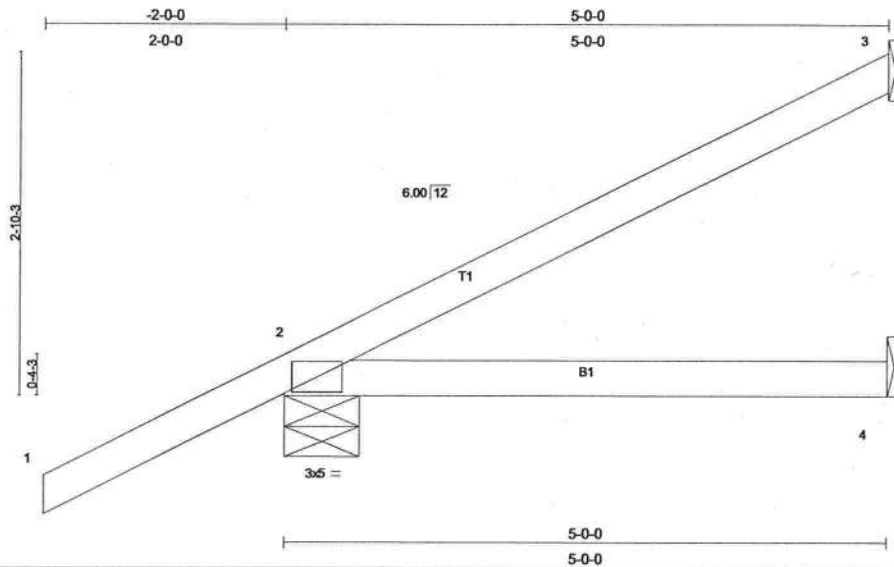
JOINT STRESS INDEX
2 = 0.18

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip
DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 2 and 99 lb uplift at joint 3.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job 298148	Truss CJ5	Truss Type JACK	Qty 8	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148003 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MITek Industries, Inc. Fri Feb 20 14:03:25 2009 Page 1		



Scale = 1:18.4

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(LL)	-0.02	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	-0.04	2-4	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 19 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

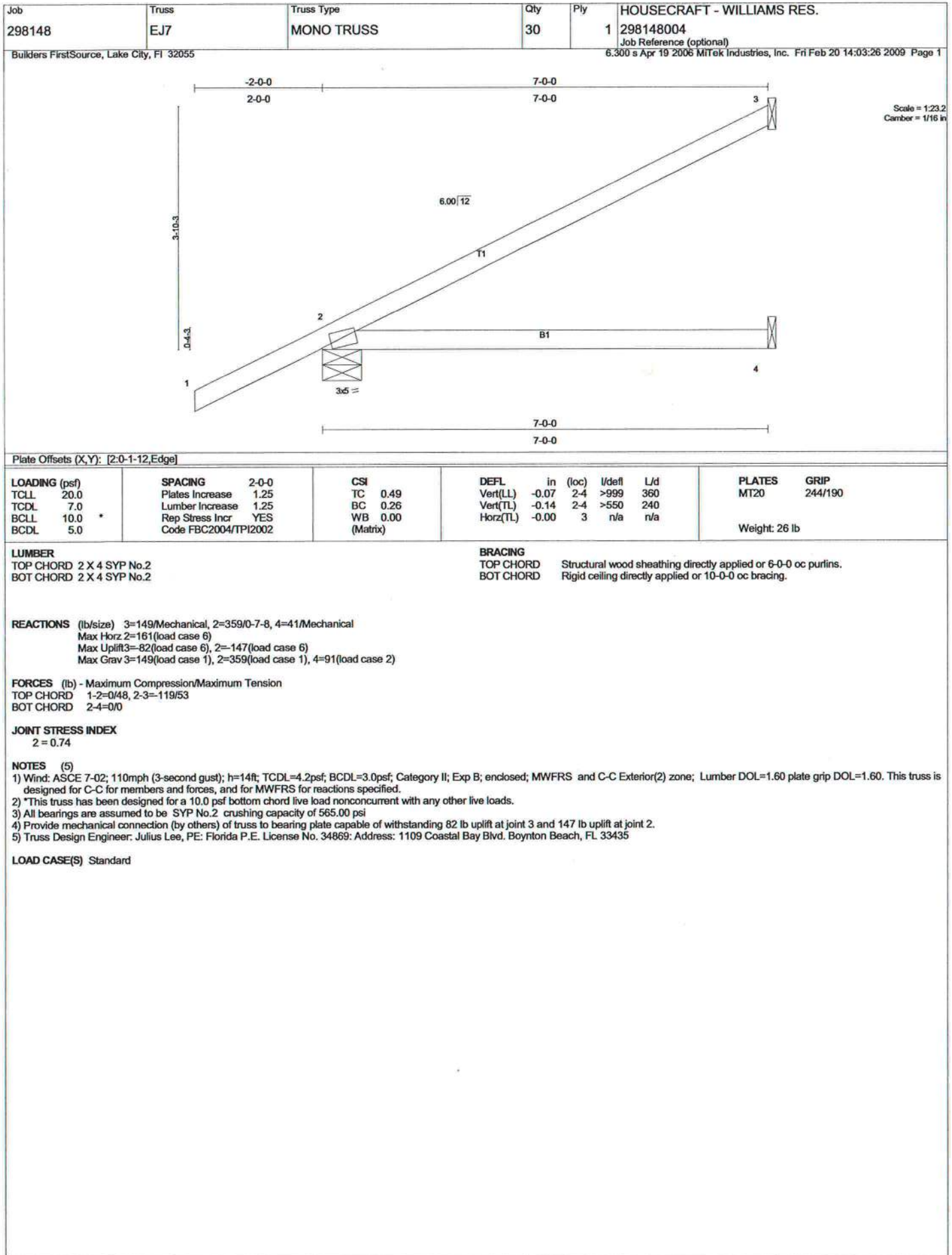
REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-7-8, 4=23/Mechanical
Max Horz 2=178(load case 6)
Max Uplift 3=80(load case 6), 2=211(load case 6)
Max Grav 3=94(load case 1), 2=304(load case 1), 4=69(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=84/33
BOT CHORD 2-4=0/0

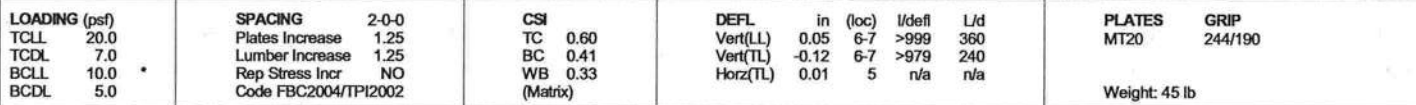
JOINT STRESS INDEX
2 = 0.18

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip
DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 80 lb uplift at joint 3 and 211 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Builders FirstSource, Lake City, FL 32055



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=449/0-11-6, 5=215/Mechanical
Max Horiz 2=265(load case 3)
Max Uplift4=231(load case 3), 2=274(load case 3), 5=59(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-8=0/45, 2-8=0/45, 2-3=635/108, 3-4=105/65
BOT CHORD 2-7=296/585, 6-7=296/585, 5-6=0/0
WEBS 3-7=0/190, 3-6=610/309

JOINT STRESS INDEX
2 = 0.84, 3 = 0.18, 6 = 0.21 and 7 = 0.14

NOTES (6)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 2) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 274 lb uplift at joint 2 and 59 lb uplift at joint 5.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=54
Trapezoidal Loads (plf)
Vert: 8-0(F=27, B=27)-to-4=134(F=40, B=40), 2=1(F=5, B=5)-to-5=25(F=7, B=7)

Job 298148	Truss T02	Truss Type HIP	Qty 2	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148007 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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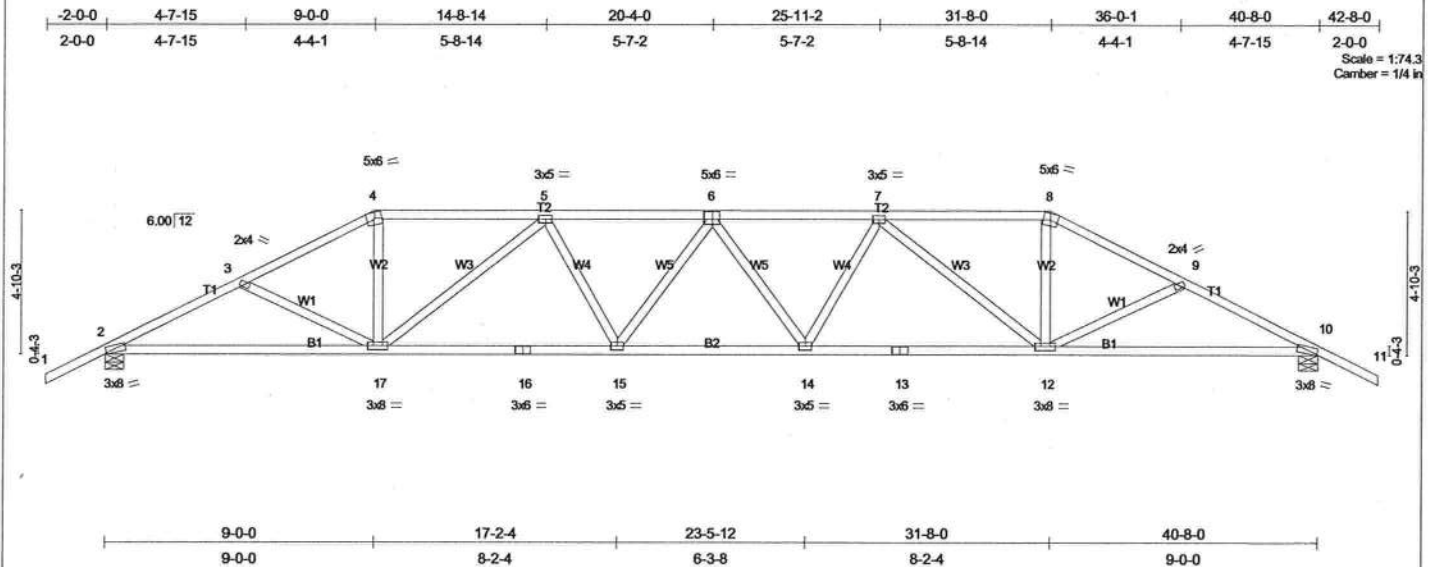


Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0], [10:0-0-10,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.37	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.57	Vert(LL) 0.28 14-15 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.83	Vert(TL) -0.47 14-15 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.16 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 209 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-8-8 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-8-3 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=1406/0-7-8, 10=1406/0-7-8
Max Horz 2=89(load case 6)
Max Uplift 2=307(load case 6), 10=307(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=2379/1229, 3-4=2198/1155, 4-5=1945/1099, 5-6=2705/1487, 6-7=2705/1487, 7-8=1945/1099, 8-9=2198/1155, 9-10=2379/1229, 10-11=0/48
BOT CHORD 2-17=918/2035, 16-17=1139/2587, 15-16=1139/2587, 14-15=1252/2808, 13-14=1139/2587, 12-13=1139/2587, 10-12=918/2035
WEBS 3-17=128/153, 4-17=288/665, 5-17=895/451, 5-15=80/284, 6-15=223/132, 6-14=223/132, 7-14=80/284, 7-12=895/451, 8-12=288/665, 9-12=128/153

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.50, 5 = 0.45, 6 = 0.43, 7 = 0.45, 8 = 0.50, 9 = 0.34, 10 = 0.81, 12 = 0.57, 13 = 0.93, 14 = 0.45, 15 = 0.45, 16 = 0.93 and 17 = 0.57

NOTES (7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2 and 307 lb uplift at joint 10.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Weight: 207 lb

LOAD CASE(S) Standard

Job 298148	Truss T04	Truss Type SPECIAL	Qty 1	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148009 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 Mitek Industries, Inc. Fri Feb 20 14:03:32 2009 Page 1		

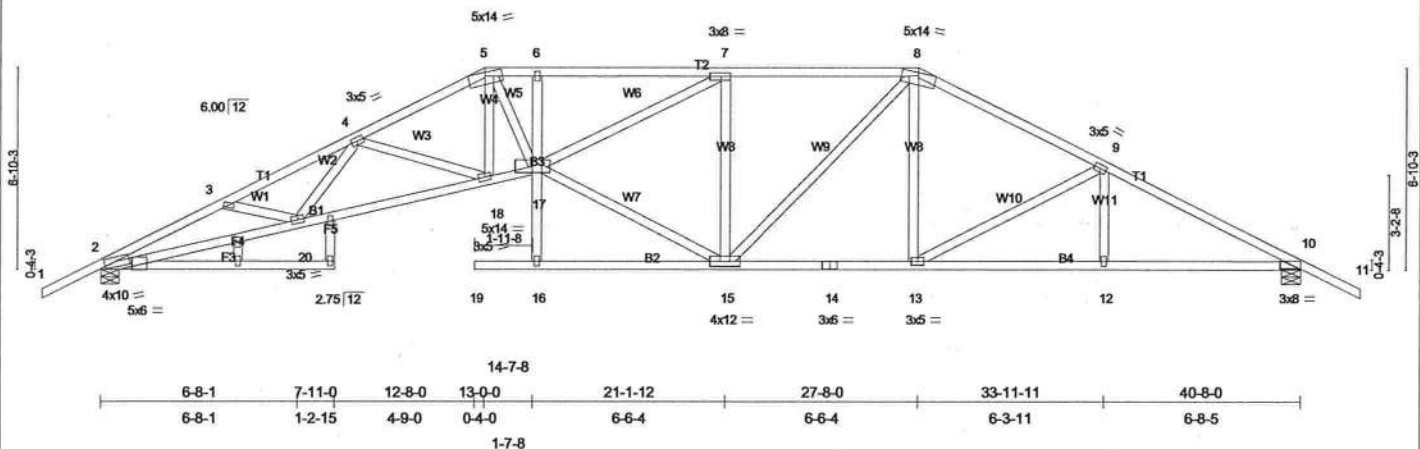


Plate Offsets (X,Y): [2:0-2-0,Edge], [7:0-3-8,0-1-8], [10:0-8-0,0-0-6]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) 0.46	19 >999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.75	Vert(TL) -0.80	19 >597	240		
BCLL 10.0 *	Rep Stress Incr YES	WB 0.91	Horz(TL) 0.41	10 n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					
						Weight: 252 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-5 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 5-2-2 oc bracing. Except:
B3 2 X 4 SYP No.3	1 Row at midpt 2-20
WEBS 2 X 4 SYP No.3	JOINTS 1 Brace at Jt(s): 17

REACTIONS (lb/size) 2=1420/0-7-8, 10=1413/0-7-8
Max Horz 2=-112(load case 7)
Max Uplift 2=329(load case 6), 10=334(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-3852/1867, 3-4=-3796/1822, 4-5=-3439/1644, 5-6=-3535/1728, 6-7=-3555/1745, 7-8=-1986/1182, 8-9=-1996/1107, 9-10=-2407/1227, 10-11=0/48
BOT CHORD 2-20=-1505/3379, 18-20=-1452/3456, 17-18=-1128/3086, 16-17=0/162, 6-17=-218/137, 16-19=0/0, 15-16=-16/49, 14-15=-649/1723, 13-14=-649/1723, 12-13=-906/2056, 10-12=-906/2056
WEBS 3-20=-21/174, 4-20=-43/167, 4-18=-365/325, 5-18=-212/406, 5-17=-441/1056, 15-17=-845/2168, 7-17=-635/1772, 7-15=-1186/546, 8-15=-172/502, 8-13=-104/324, 9-13=-389/293, 9-12=0/201

JOINT STRESS INDEX
2 = 0.66, 3 = 0.34, 4 = 0.48, 5 = 0.80, 6 = 0.59, 7 = 0.73, 8 = 0.90, 9 = 0.48, 10 = 0.76, 12 = 0.34, 13 = 0.40, 14 = 0.61, 15 = 0.84, 16 = 0.66, 17 = 0.99, 18 = 0.44, 20 = 0.44, 21 = 0.12, 22 = 0.34, 23 = 0.34, 24 = 0.34 and 25 = 0.34

- NOTES** (9)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02: 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) All plates are 2x4 MT20 unless otherwise indicated.
 - 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 2 and 334 lb uplift at joint 10.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job

298148

Truss

T05

Truss Type

SPECIAL

Qty

1

Ply

1

HOUSECRAFT - WILLIAMS RES.

298148010

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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-2-0-0

3-6-1

7-1-2

11-1-15

15-0-0

18-3-9

22-4-7

25-8-0

29-6-1

33-6-14

37-1-15

40-8-0

42-8-0

2-0-0

3-6-1

3-7-1

4-0-13

3-10-1

3-3-9

4-0-15

3-3-9

3-10-1

4-0-13

3-7-1

3-6-1

2-0-0

Scale = 1:75.3

Camber = 1/2 in

5-2-6

7-11-0

9-9-3

15-0-0

20-4-0

25-8-0

27-8-8

30-10-13

35-5-10

40-8-0

5-2-6

2-8-10

1-10-3

5-2-13

5-4-0

5-4-0

2-0-8

3-2-5

4-6-12

5-2-6

Plate Offsets (X,Y): [2:0-2-0,Edge], [4:0-3-0,0-3-0], [11:0-3-0,0-3-0], [13:0-2-0,Edge], [16:0-3-0,0-3-0], [20:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	0.59	18	>819	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.76	Vert(TL)	-1.02	18	>471	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.62	Horz(TL)	0.71	13	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 257 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-5 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-5-1 oc bracing. Except:

1 Row at midpt 2-20

2 Rows at 1/3 pts 13-17

REACTIONS (lb/size)

2=1406/0-7-8, 13=1406/0-7-8

Max Horz 2=123(load case 6)

Max Uplift 2=351(load case 6), 13=351(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=3635/1810, 3-4=3694/1850, 4-5=3643/1869, 5-6=3158/1591, 6-7=2825/1483, 7-8=4145/1988, 8-9=2825/1483, 9-10=3158/1591, 10-11=3643/1869, 11-12=3694/1850, 12-13=3635/1810, 13-14=0/45

BOT CHORD 2-21=1446/3155, 20-21=1570/3470, 19-20=1340/3218, 18-19=1372/3677, 17-18=1372/3677, 16-17=1340/3218, 15-16=1570/3470, 13-15=1446/3155

WEBS 3-21=39/220, 4-21=153/128, 4-20=202/197, 5-20=137/254, 5-19=410/345, 6-19=553/1177, 7-19=1267/522, 7-18=347/1009, 8-18=347/1009, 8-17=1267/522, 9-17=553/1177, 10-17=410/345, 10-16=137/254, 11-16=202/197, 11-15=153/128, 12-15=39/220

JOINT STRESS INDEX

2 = 0.63, 3 = 0.34, 4 = 0.44, 5 = 0.48, 6 = 0.58, 7 = 0.75, 8 = 0.75, 9 = 0.58, 10 = 0.48, 11 = 0.44, 12 = 0.34, 13 = 0.63, 15 = 0.44, 16 = 0.83, 17 = 0.68, 18 = 0.85, 19 = 0.68, 20 = 0.83, 21 = 0.44, 22 = 0.12, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 28 = 0.12, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34 and 33 = 0.34

NOTES (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

7) Bearing at joint(s) 2, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 2 and 351 lb uplift at joint 13.

9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job 298148	Truss T06	Truss Type SPECIAL	Qty 1	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148011 Job Reference (optional)
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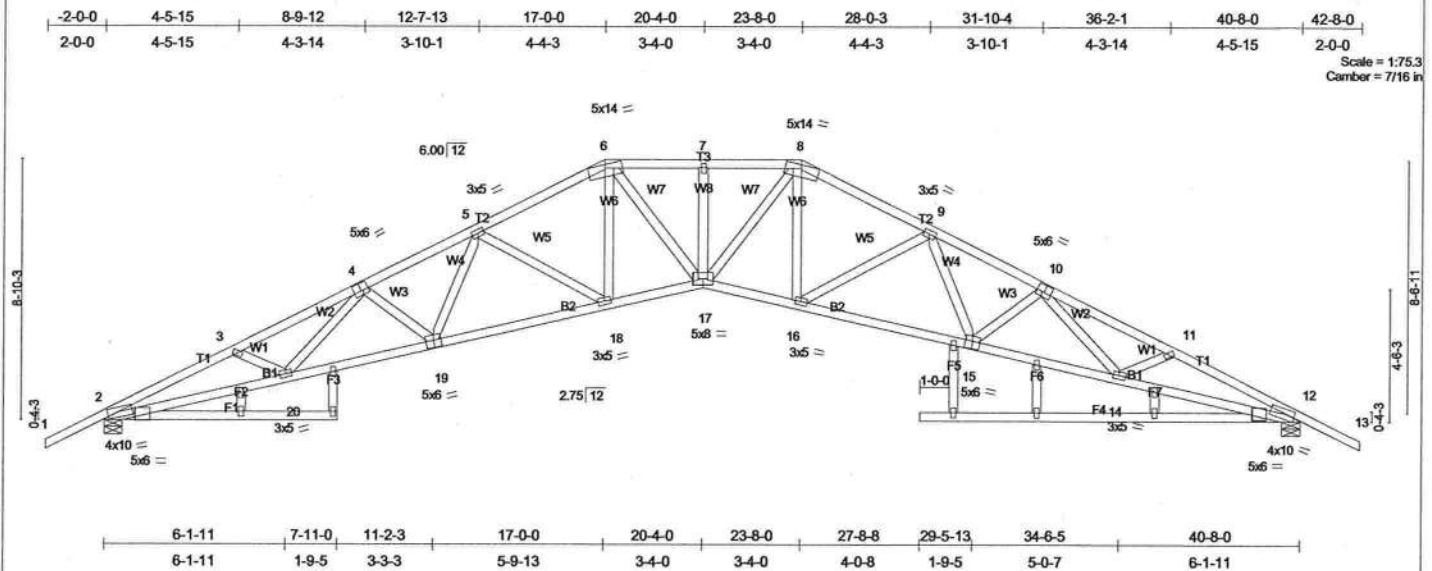


Plate Offsets (X,Y): [2:0-2:0,Edge], [4:0-3:0,0-3:0], [10:0-3:0,0-3:0], [12:0-5:3,0-0-8], [15:0-3:0,0-3:0], [19:0-3:0,0-3:0]	
LOADING (psf)	SPACING 2-0-0
TCLL 20.0	Plates Increase 1.25
TCDL 7.0	Lumber Increase 1.25
BCLL 10.0	Rep Stress Incr YES
BCDL 5.0	Code FBC2004/TPI2002
CSI	DEFL in (loc) l/defl L/d
TC 0.43	Vert(LL) 0.52 17 >927 360
BC 0.77	Vert(TL) -0.88 17 >546 240
WB 0.27	Horz(TL) 0.63 12 n/a n/a
(Matrix)	
	PLATES GRIP
	MT20 244/190
	Weight: 260 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-0-12 oc bracing. Except:
WEBS 2 X 4 SYP No.3	1 Row at midpt 2-20, 12-15

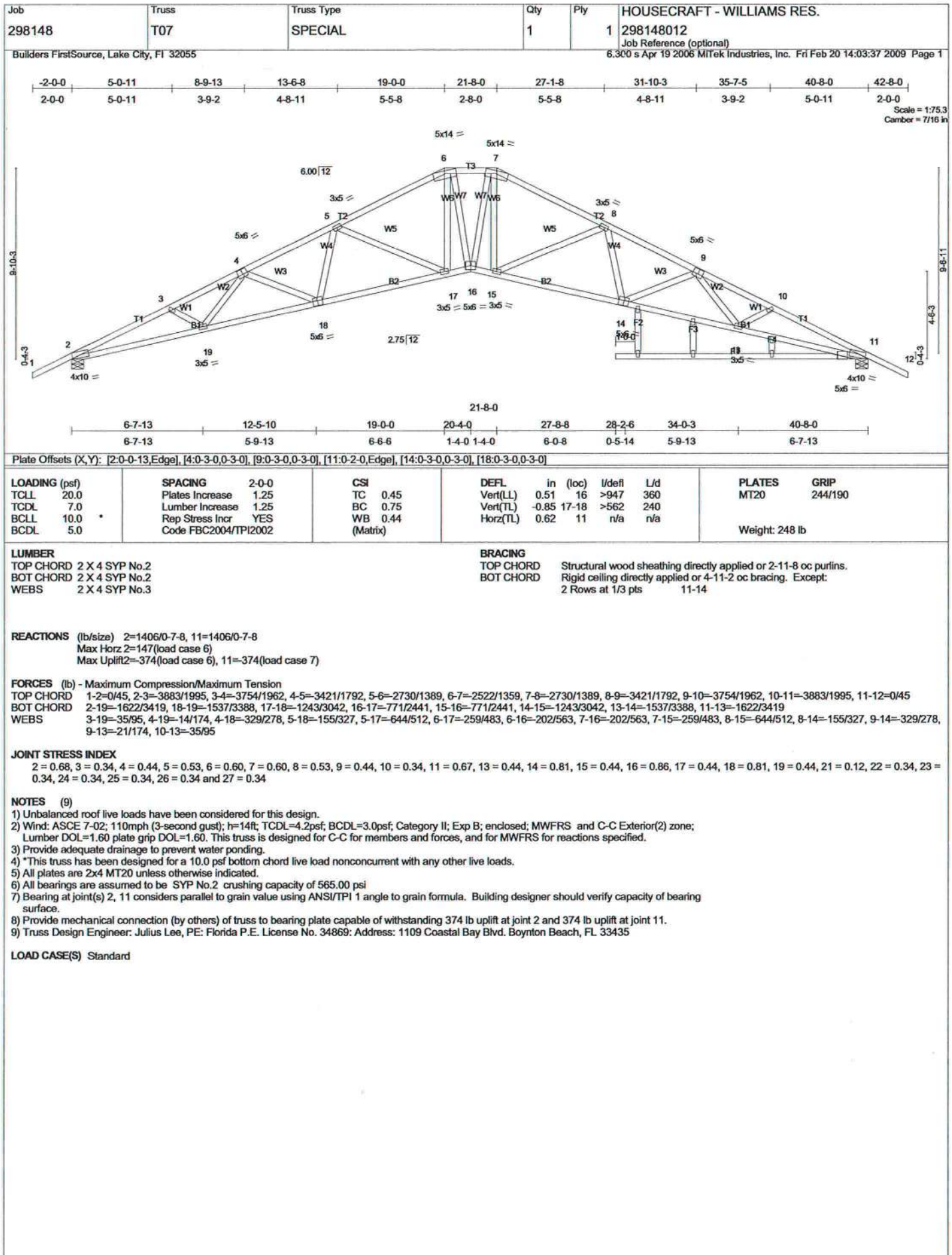
REACTIONS (lb/size) 2=1406/0-7-8, 12=1406/0-7-8
 Max Horz 2=135(load case 6)
 Max Uplift 2=363(load case 6), 12=363(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/45, 2-3=3834/1952, 3-4=3735/1918, 4-5=3532/1849, 5-6=2941/1496, 6-7=3137/1586, 7-8=3137/1586, 8-9=2941/1496, 9-10=3532/1849, 10-11=3735/1918, 11-12=3834/1952, 12-13=0/45
 BOT CHORD 2-20=1584/3366, 19-20=1518/3396, 18-19=1260/3090, 17-18=908/2665, 16-17=908/2664, 15-16=1260/3090, 14-15=1518/3396, 12-14=1584/3366
 WEBS 3-20=1/111, 4-20=41/130, 4-19=260/237, 5-19=201/327, 5-18=497/402, 6-18=248/403, 6-17=302/849, 7-17=140/75, 8-17=302/849, 8-16=248/403, 9-16=497/402, 9-15=201/327, 10-15=260/237, 10-14=41/130, 11-14=4/111

JOINT STRESS INDEX
 2 = 0.66, 3 = 0.34, 4 = 0.45, 5 = 0.48, 6 = 0.67, 7 = 0.34, 8 = 0.67, 9 = 0.48, 10 = 0.45, 11 = 0.34, 12 = 0.63, 12 = 0.39, 14 = 0.44, 15 = 0.82, 16 = 0.44, 17 = 0.81, 18 = 0.44, 19 = 0.82, 20 = 0.44, 21 = 0.12, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 27 = 0.00, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34 and 33 = 0.34

- NOTES** (9)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02: 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) All plates are 2x4 MT20 unless otherwise indicated.
 - 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 7) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 2 and 363 lb uplift at joint 12.
 - 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job 298148	Truss T08	Truss Type HIP	Qty 1	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148013 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MITek Industries, Inc. Fri Feb 20 14:03:38 2009 Page 1

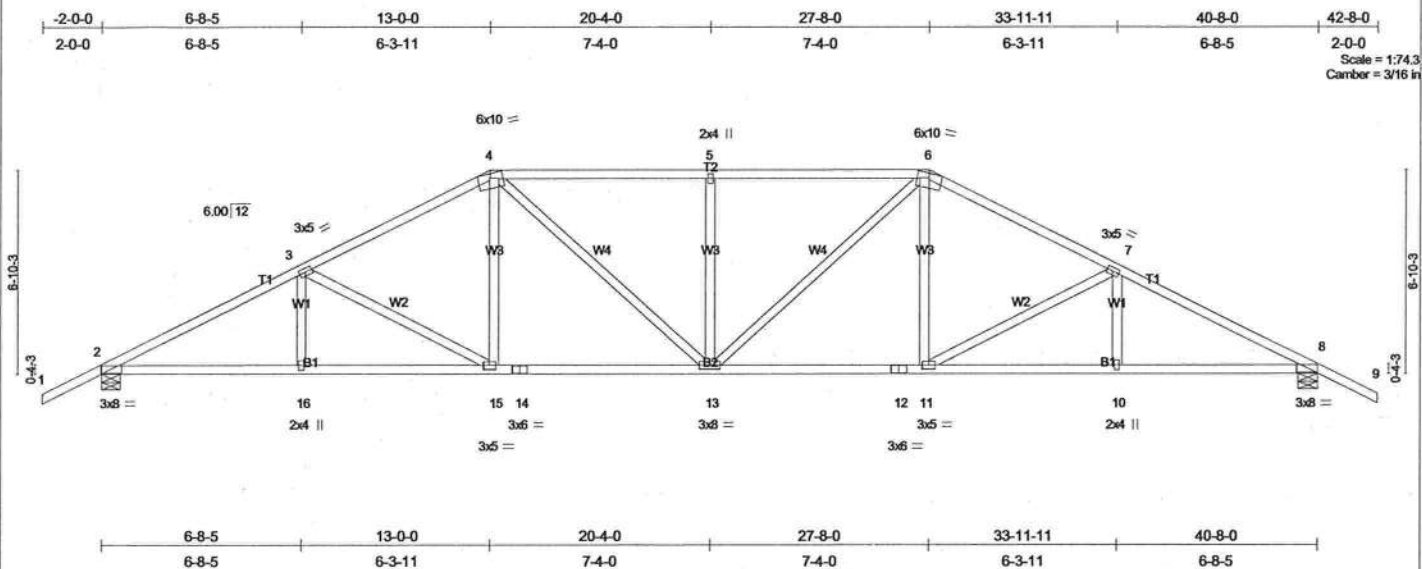


Plate Offsets (X,Y): [2-0-0,0-0-0-6], [8-0-8,0-0-0-6]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.38	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.43	Vert(LL) 0.20 13 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.35	Vert(TL) -0.33 13-15 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.13 8 n/a n/a		
	Code FBC2004/TPI2002			Weight: 218 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-7-3 oc bracing.
WEBS 2 X 4 SYP No.3	

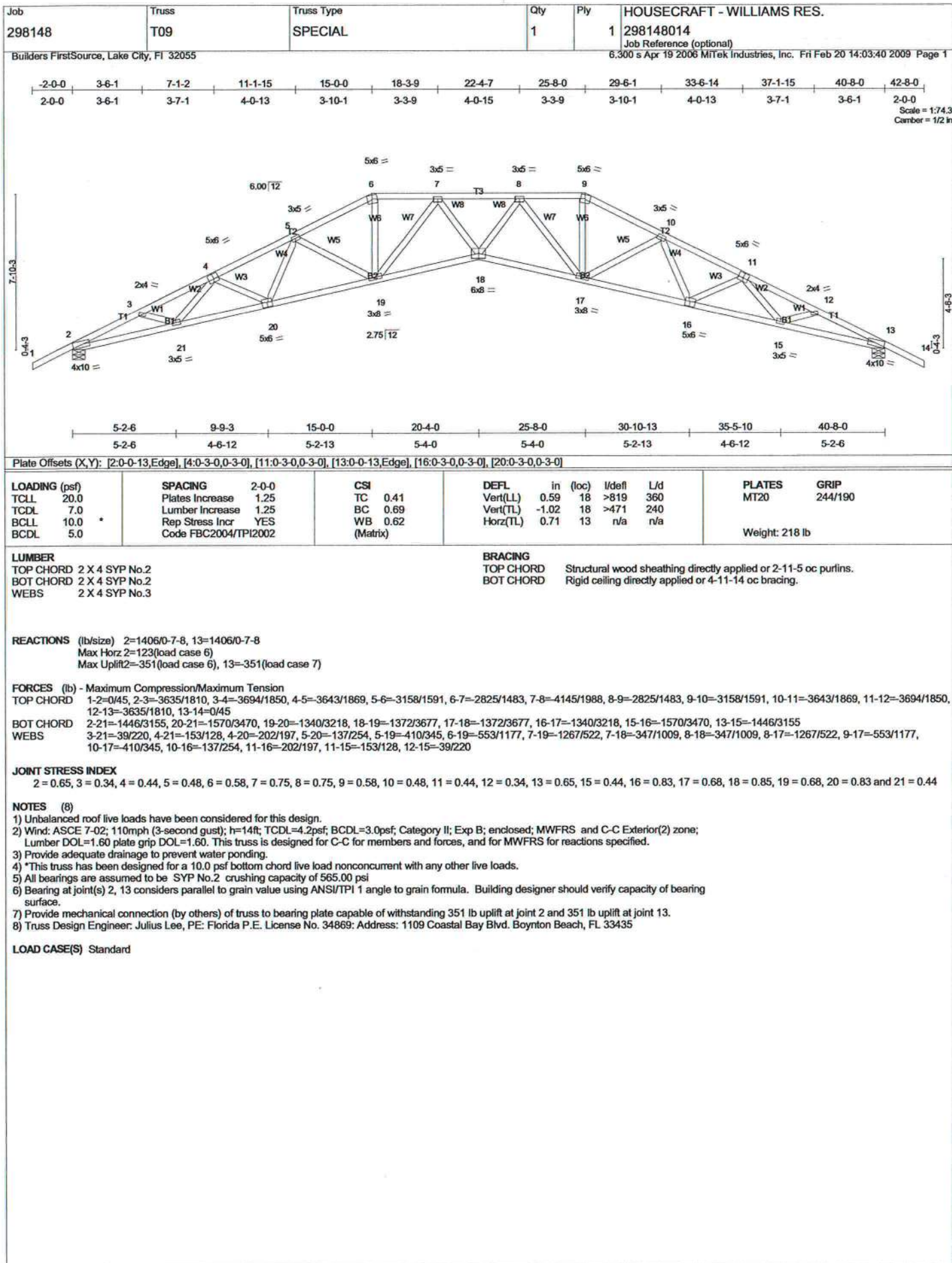
REACTIONS (lb/size) 2=1406/0-7-8, 8=1406/0-7-8
Max Horz 2=113(load case 6)
Max Uplift 2=338(load case 6), 8=338(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=2390/1236, 3-4=1983/1117, 4-5=1978/1200, 5-6=1978/1200, 6-7=1983/1117, 7-8=2390/1236, 8-9=0/48
BOT CHORD 2-16=913/2041, 15-16=913/2041, 14-15=660/1714, 13-14=660/1714, 12-13=660/1714, 11-12=660/1714, 10-11=913/2041, 8-10=913/2041
WEBS 3-16=0/198, 3-15=383/288, 4-15=98/334, 4-13=172/488, 5-13=416/276, 6-13=172/488, 6-11=98/334, 7-11=383/288, 7-10=0/198

JOINT STRESS INDEX
2 = 0.76, 3 = 0.48, 4 = 0.67, 5 = 0.34, 6 = 0.67, 7 = 0.48, 8 = 0.76, 10 = 0.34, 11 = 0.40, 12 = 0.59, 13 = 0.57, 14 = 0.59, 15 = 0.40 and 16 = 0.34

- NOTES** (7)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 2 and 338 lb uplift at joint 8.
 - 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job 298148	Truss T10	Truss Type SPECIAL	Qty 1	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148015 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Feb 20 14:03:42 2009 Page 1

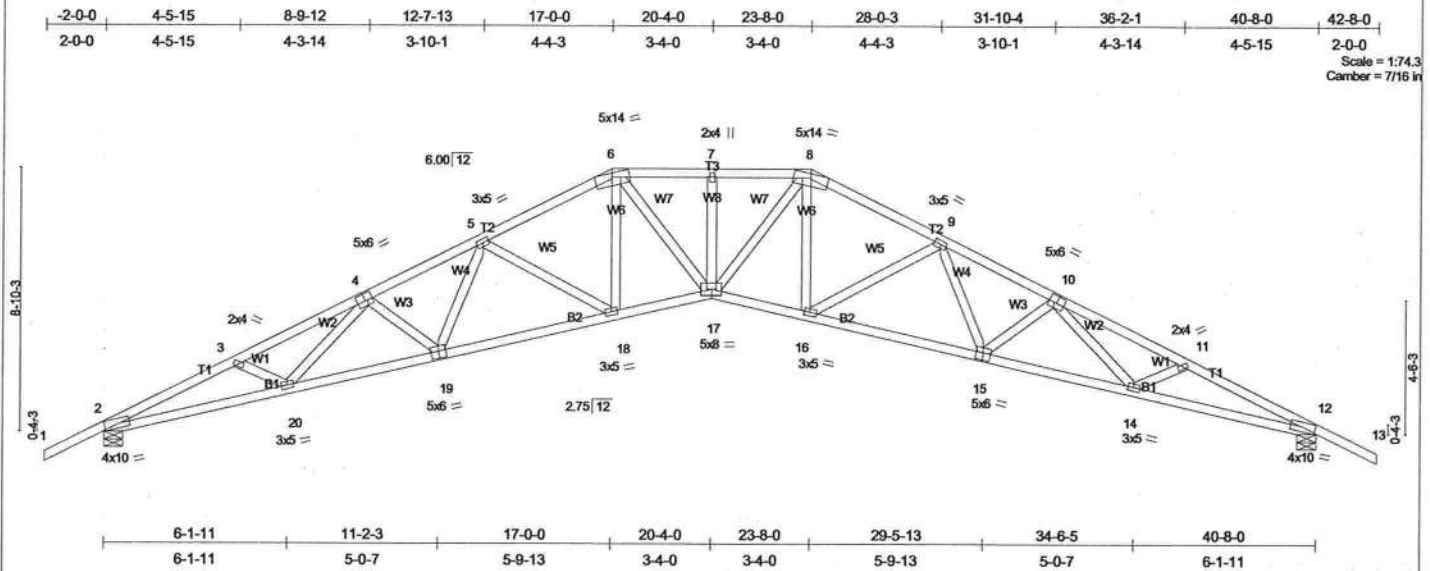


Plate Offsets (X,Y): [2:0-0-13,Edge], [4:0-3-0,0-3-0], [10:0-3-0,0-3-0], [12:0-0-13,Edge], [15:0-3-0,0-3-0], [19:0-3-0,0-3-0]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.67	Vert(LL) 0.52 17 >927 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.27	Vert(TL) -0.88 17 >546 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.63 12 n/a n/a		
	Code FBC2004/TPI2002			Weight: 221 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-11-13 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 4-11-11 oc bracing.
WEBS 2 X 4 SYP No.3	

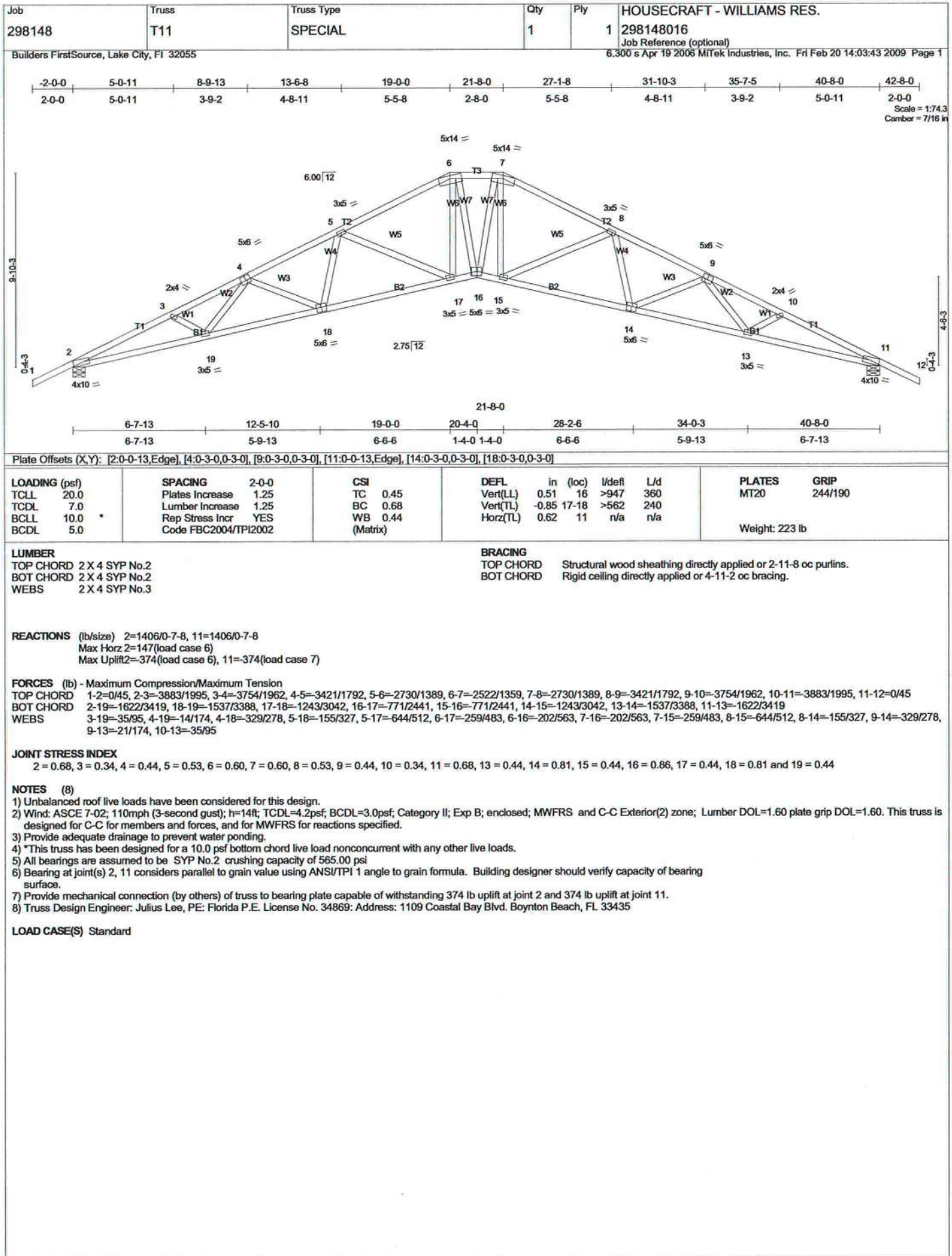
REACTIONS (lb/size) 2=1406/0-7-8, 12=1406/0-7-8
Max Horz 2=135(load case 6)
Max Uplift 2=363(load case 6), 12=363(load case 7)

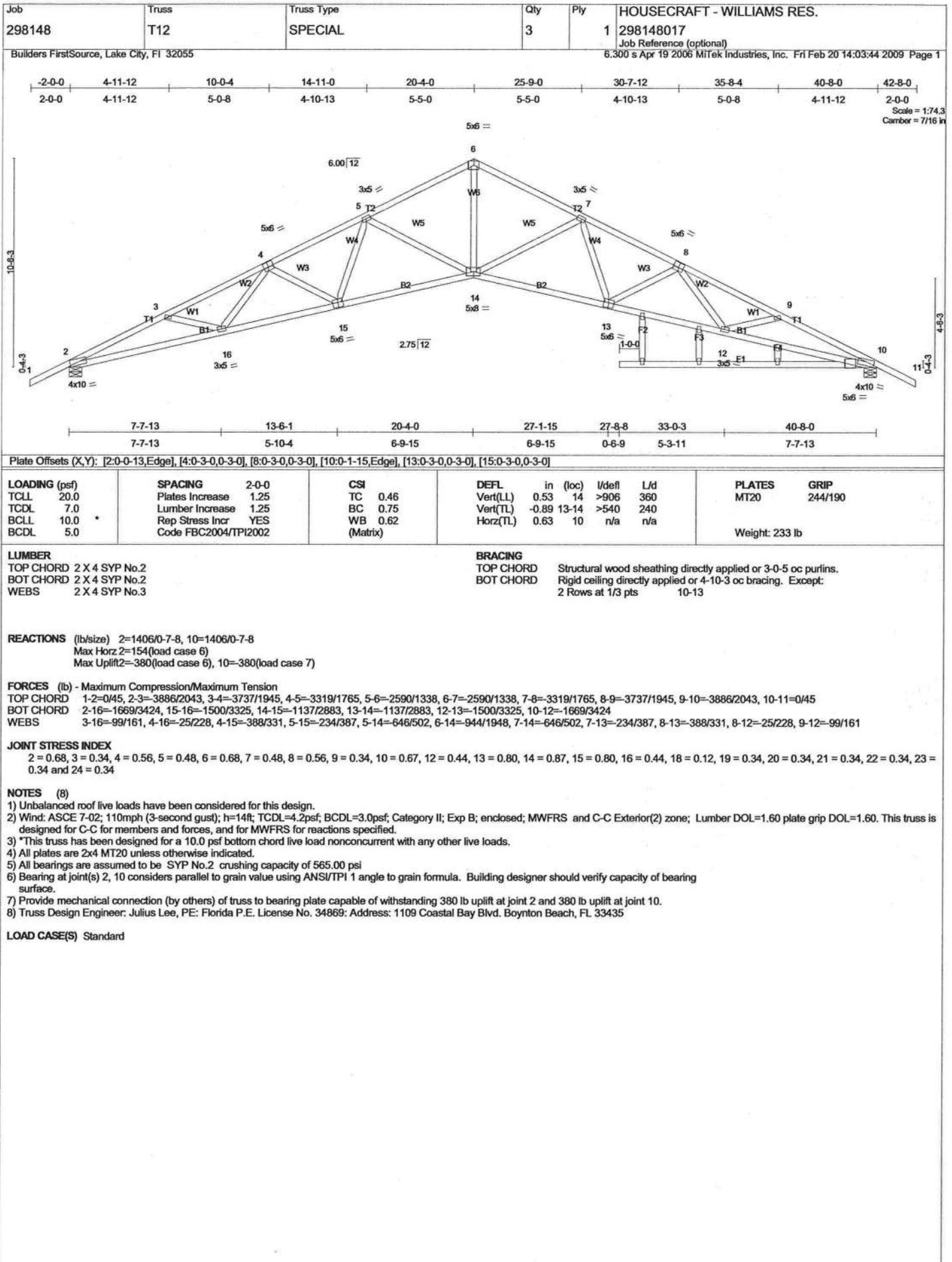
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=3834/1952, 3-4=3735/1918, 4-5=3532/1849, 5-6=2941/1496, 6-7=3137/1586, 7-8=3137/1586, 8-9=2941/1496, 9-10=3532/1849, 10-11=3735/1918, 11-12=3834/1952, 12-13=0/45
BOT CHORD 2-20=1584/3366, 19-20=1518/3396, 18-19=1260/3090, 17-18=908/2665, 16-17=908/2664, 15-16=1260/3090, 14-15=1518/3396, 12-14=1584/3366
WEBS 3-20=1/111, 4-20=41/130, 4-19=260/237, 5-19=201/327, 5-18=497/402, 6-18=248/403, 6-17=302/849, 7-17=140/75, 8-17=302/849, 8-16=248/403, 9-16=497/402, 9-15=201/327, 10-15=260/237, 10-14=41/130, 11-14=4/111

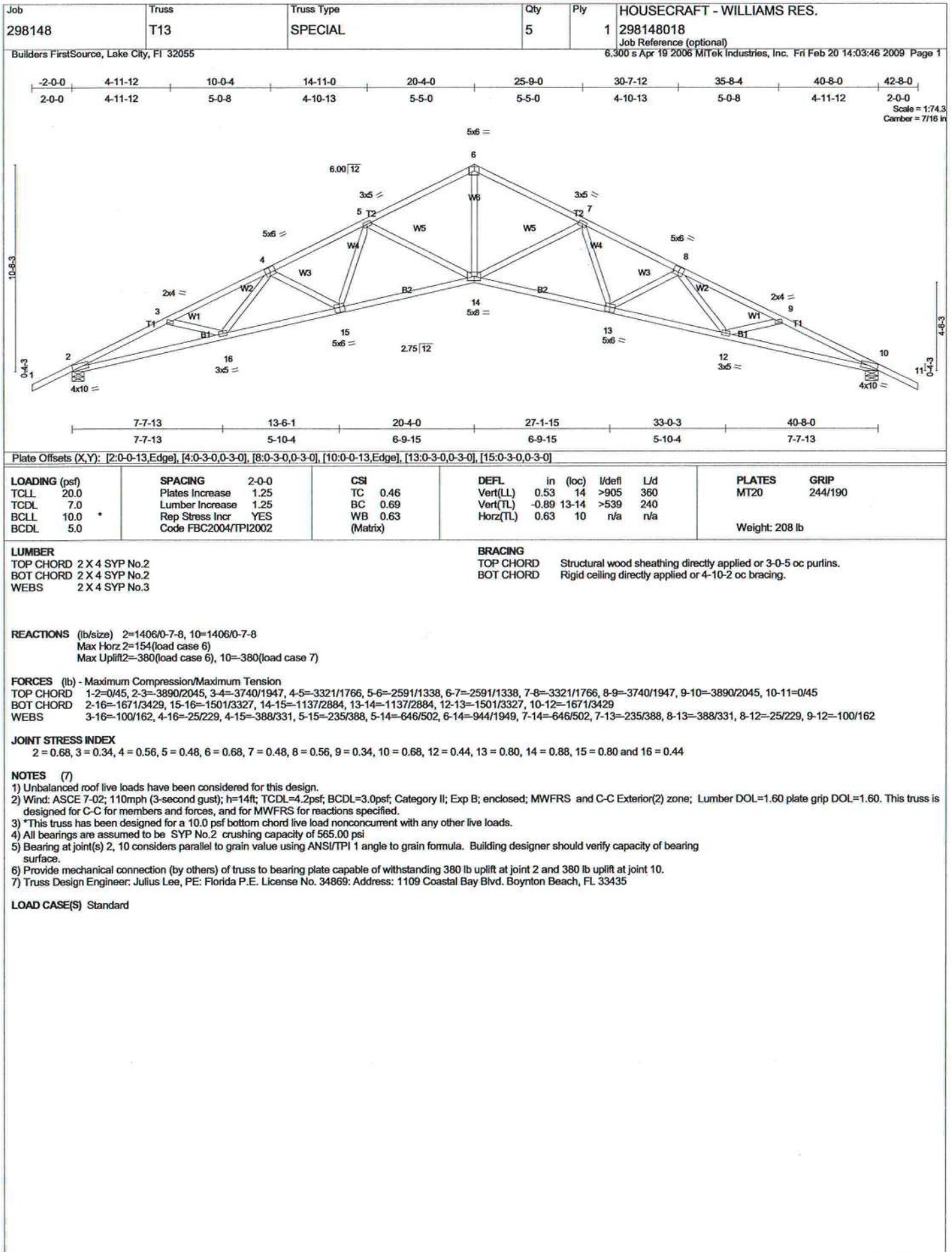
JOINT STRESS INDEX
2 = 0.68, 3 = 0.34, 4 = 0.45, 5 = 0.48, 6 = 0.67, 7 = 0.34, 8 = 0.67, 9 = 0.48, 10 = 0.45, 11 = 0.34, 12 = 0.68, 14 = 0.44, 15 = 0.82, 16 = 0.44, 17 = 0.81, 18 = 0.44, 19 = 0.82 and 20 = 0.44

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone;
Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 2 and 363 lb uplift at joint 12.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

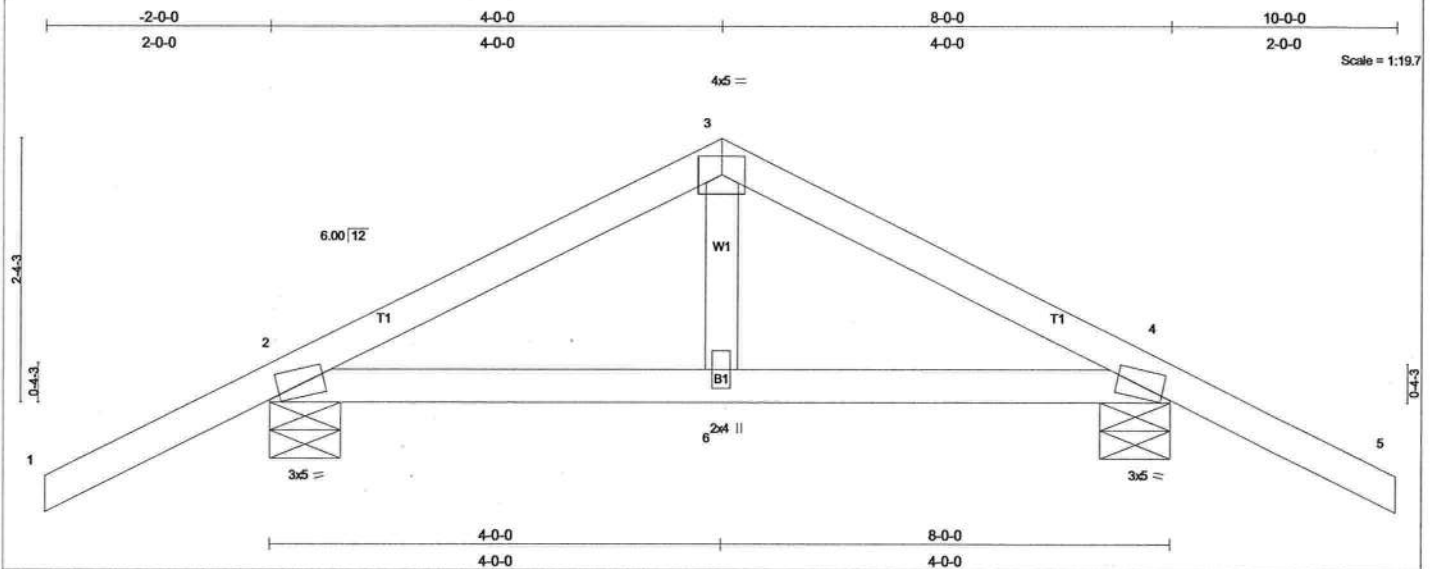
LOAD CASE(S) Standard







Job 298148	Truss T14	Truss Type COMMON	Qty 4	Ply 1	HOUSECRAFT - WILLIAMS RES. 298148019 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MITek Industries, Inc. Fri Feb 20 14:03:46 2009 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.33	Vert(LL) 0.01 2-6 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL) -0.01 2-6 >999 240		
BCLL 10.0 *	Rep Stress Incr YES	WB 0.04	Horz(TL) -0.00 4 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			
				Weight: 35 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

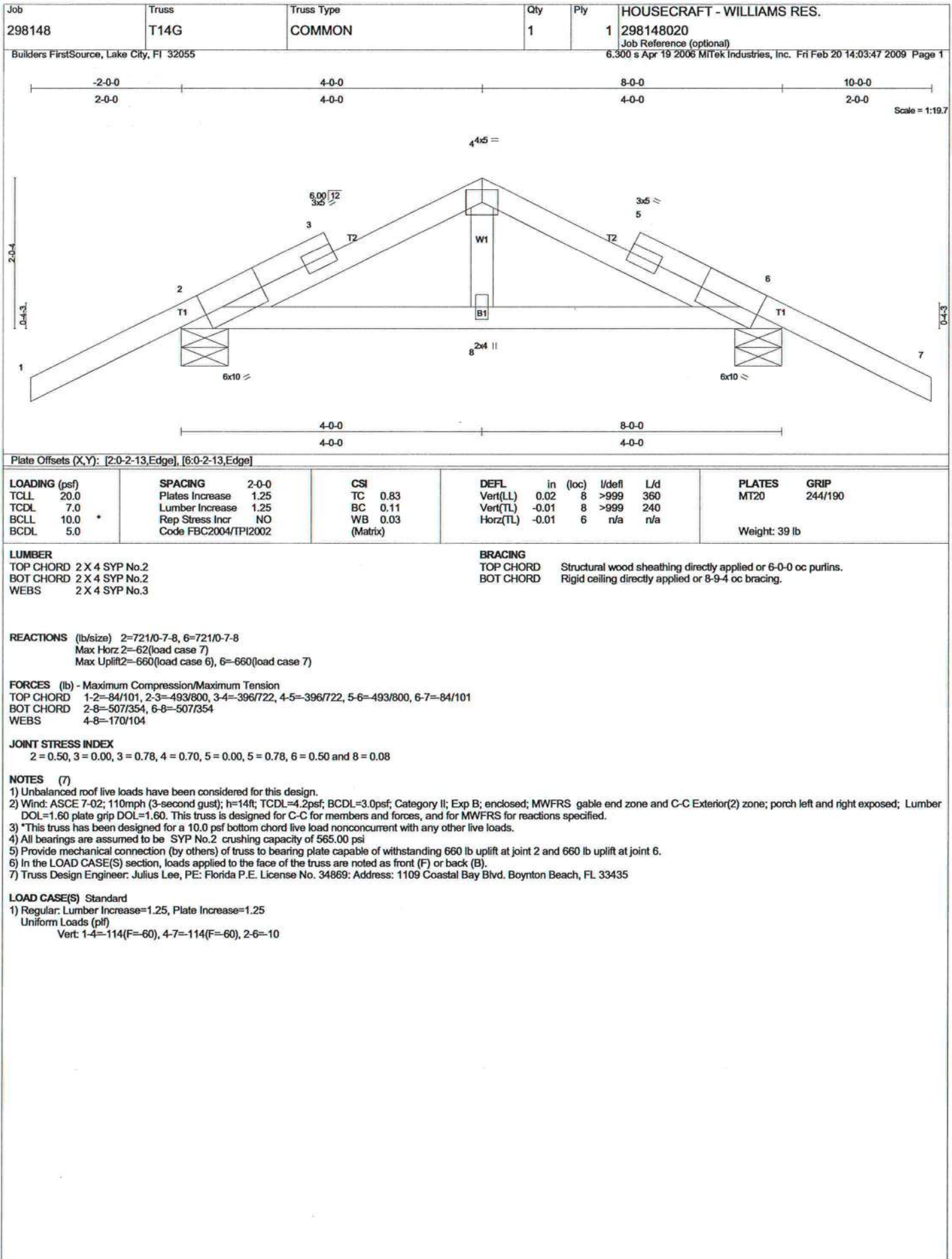
REACTIONS (lb/size) 2=361/0-7-8, 4=361/0-7-8
Max Horz 2=60(load case 7)
Max Uplift 2=257(load case 6), 4=257(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=234/358, 3-4=234/358, 4-5=0/48
BOT CHORD 2-6=157/157, 4-6=157/157
WEBS 3-6=163/113

JOINT STRESS INDEX
2 = 0.63, 3 = 0.35, 4 = 0.63 and 6 = 0.08

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 257 lb uplift at joint 2 and 257 lb uplift at joint 4.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



120 MPH MAX

Setback 7' or Less

UPLIFT: 400# or Less

2' TYP.
MAX
or Less
CJ's
2' TYP.
MAX

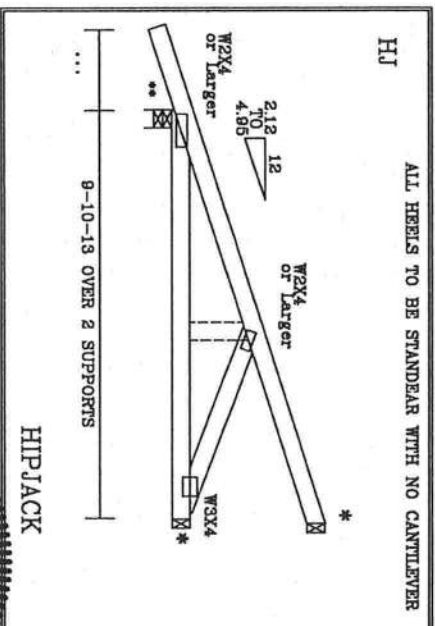
UPLIFT: 400# or Less

The diagram illustrates a truss structure with the following components:

- Members:**
 - Cj:** Vertical members connecting the top and bottom chords.
 - Ej:** Horizontal members connecting the vertical members.
 - HJ:** A diagonal member connecting the bottom left corner to the top right corner.
- Sections:**
 - #2 HIP OR COMMON TRUSS:** The upper section of the truss.
 - #1 HIP TRUSS:** The lower section of the truss.

UPPLIFT: 400# or Less

2' TYP.
MAX



HIPJACK

7'0" MAX

REVISIONS: THE TRUSSES REQUIRING EXTENSIVE CARE IN FABRICATING, HANDING, SHIPPING, INSTALLING AND BRACING, REFER TO BCST-1-04E BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CRUSS PLATE INSTITUTE, 268 DOWNTOWN PARK, SUITE 200, MIDDLETOWN, VA 52749 AND VTCO CYCLOD TRUSS CONSULT. OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

PRODUCTS: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR APPLICABLE, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN & CODE COMMENTARY. ALPINE CONTRACTOR PLATES ARE MADE OF 20/18/16GA C/V/H/S ASH 6653 GRADE/DEN 40/60 C/V/H/S DGLV STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE INDICATED, ALL TRUSSES IN THIS DESIGN, POSITION PER DRAWINGS BELOW. THE INSTALLATION OF UNFINISHED ACCEPTED OF PROFESSIONAL ENGINEERING RESPONSIBILITY, A STEEL, FOR THE TRUSS COMPONENT DESIGN SHOWN, THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1, SEC. 2.

COM. KENNEDY, P. J.
 1486 ST. ALB. AVENUE
 BETHLEHEM, PA. 18015
 TEL. 610-748-6541

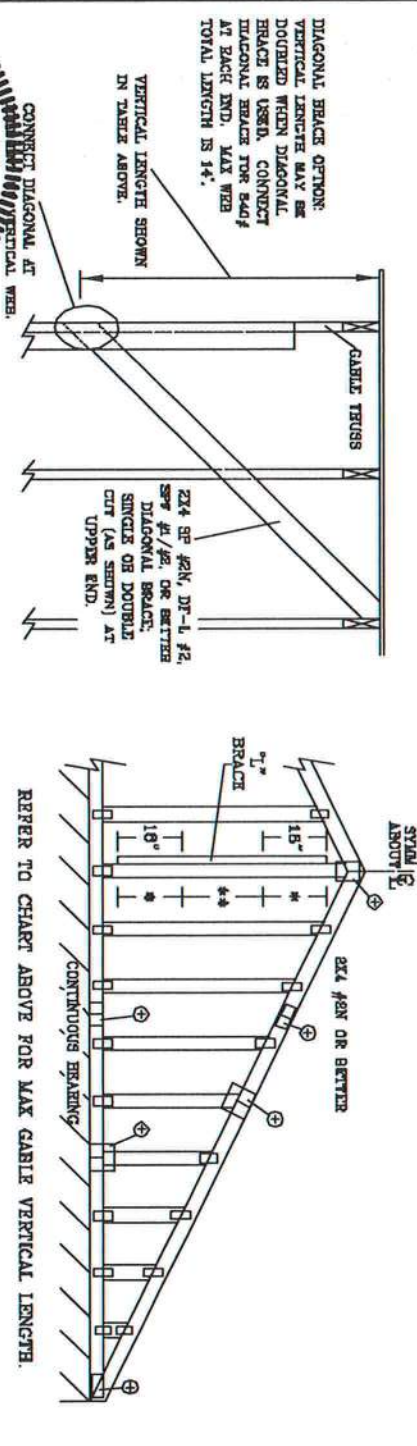
SPACING		2' MAX	
DUR.	FAC.	1.25	

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
 By Julius Lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 16' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *	
GABLE VERTICAL SPACING	SPECIES GRADE	BRACE	SPECIES	GRADE	NO	GROUP A		GROUP B		GROUP A		GROUP B		GROUP A	
						#1 / #2	#3	#1 / #2	#3	#1 / #2	#3	#1 / #2	#3	#1 / #2	#3
12" O.C.	SPF	DFL	SPF	DFL	SPF	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
16" O.C.	SPF	DFL	SPF	DFL	SPF	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
24" O.C.	SPF	DFL	SPF	DFL	SPF	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
	SPF	DFL	SPF	DFL	SPF	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"



BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPF-PIR-YB	
#1 / #2	STANDARD
STUD	

RED-PIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH

#2	
STUD	
STANDARD	

SOUTHERN PINE

#2	
STUD	
STANDARD	

GROUP B:

RED-PIR	
#1 & BIR	
#1	

SOUTHERN PINE

#1	
#2	

DOUGLAS FIR-LARCH

#1	
#2	

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4" O" OUTLEAKERS WITH 8" O" OVERHANG, OR 12" PLTWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONE

** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C.

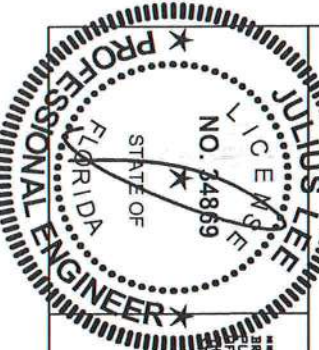
IN 18" END ZONES AND 6" O.C. BETWEEN ZONE

"L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SERVICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT	2X4
LESS THAN 11' 6"	
GREATER THAN 11' 6"	2.5X4

+ REFER TO COLUMN TRUSS DESIGN FOR PEAK, SERVICE, AND DEAD PLATES.



REVIEWED By Julius Lee at 12:00 pm, Jun 11, 2008

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE 7-02-CAB13015

DATE 11/26/03

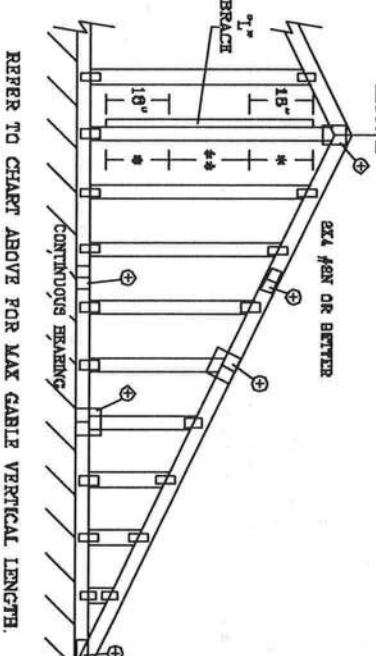
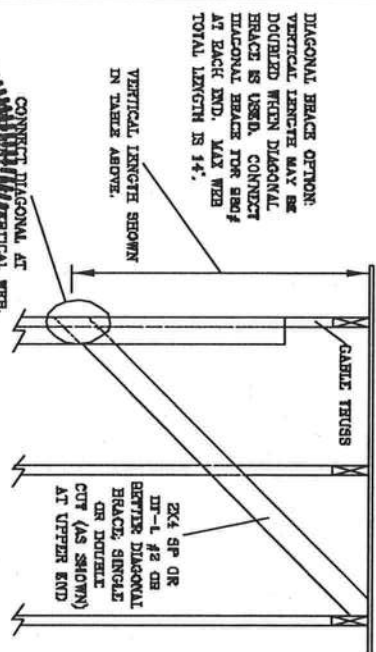
DRWG MTRK STD GABLE 15 E HT

-ENG

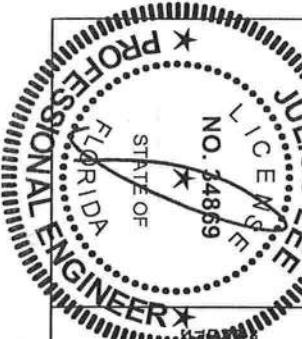
JULIUS LEE'S CONSULTING ENGINEERS P.A. 1455 ST. ARNOLD DRIVE, MIAMI BEACH, FL 33141-2161

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2X4		BRACE		NO		(1) 1X4 T ¹ BRACE *		(1) 2X4 T ¹ BRACE *		(2) 2X4 T ¹ BRACE **		(1) 2X6 T ¹ BRACE *		(2) 2X6 T ¹ BRACE *	
SPACING	CABLE VERTICAL SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
	HF	STUD	3' 1"	4' 5"	4' 5"	6' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
	SP	STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 0"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	12' 3"	12' 3"	12' 3"	12' 3"
	DFL	#1	3' 6"	5' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
16" O.C.	SPF	STUD	3' 3"	4' 6"	4' 6"	4' 6"	5' 11"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
	HF	STUD	3' 0"	4' 3"	4' 3"	3' 10"	3' 10"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"
	SP	#1 / #2	3' 7"	5' 5"	5' 5"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	STUD	3' 7"	5' 5"	5' 5"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	STUD	3' 7"	5' 5"	5' 5"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	3' 7"	5' 5"	5' 5"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 0"	6' 4"	6' 4"	6' 10"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	STUD	4' 0"	6' 4"	6' 4"	6' 10"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 150 PSF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINE WITH 2' 0" OVERHANG, OR 12" PLUMB OVERHANG.	
ATTACH EACH T ¹ BRACE WITH 104 NAILS.	
* FOR (1) T ¹ BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) T ¹ BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
T ¹ BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	



CONNECTIONS: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCES 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 3833 DUNSTON DR., SUITE 200, WASHINGTON, VA 22191 AND VITA (WOOD TRUSS COUNCIL) 10000 WOODBRIDGE BLVD., SUITE 100, WOODBRIDGE, VA 22191 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE INSTALLATION. UNLESS OTHERWISE INDICATED, THE CHART SHALL HAVE PRELIMINARY ATTACHED TO THE CHART. THE CHART SHALL HAVE A PROPERLY ATTACHED TO THE CHART.

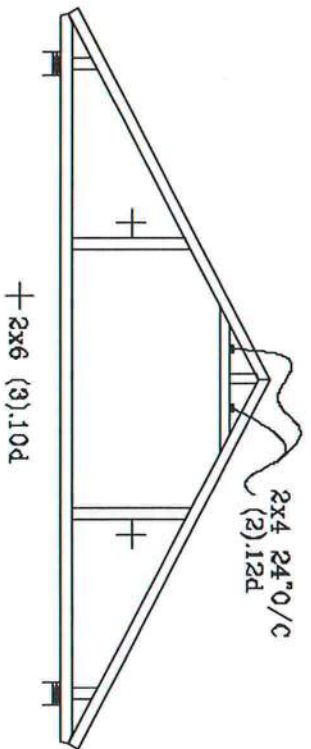
JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DEERFIELD BEACH, FL 33444-4161

REF ASCE7-02-CAB10380
DATE 11/26/03
DWG WEEK STD GABLE 30' x 17'
-ENG

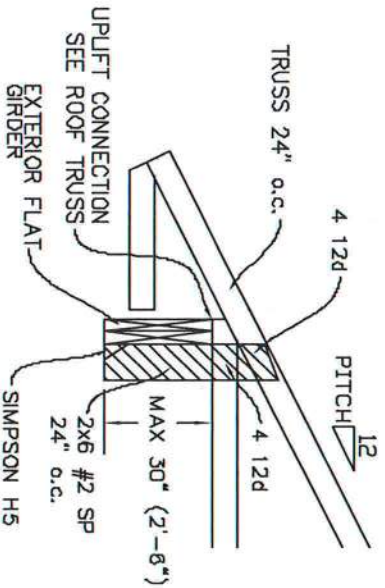
MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

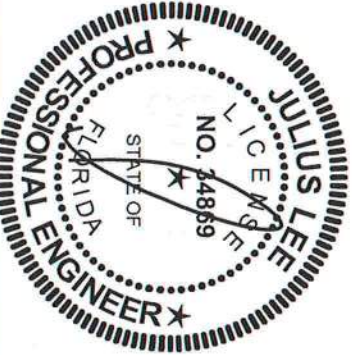
TYPICAL ATTIC TRUSS BRACING



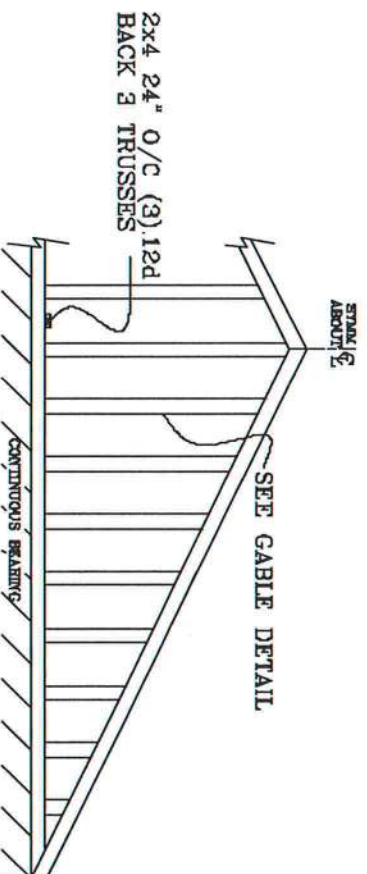
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

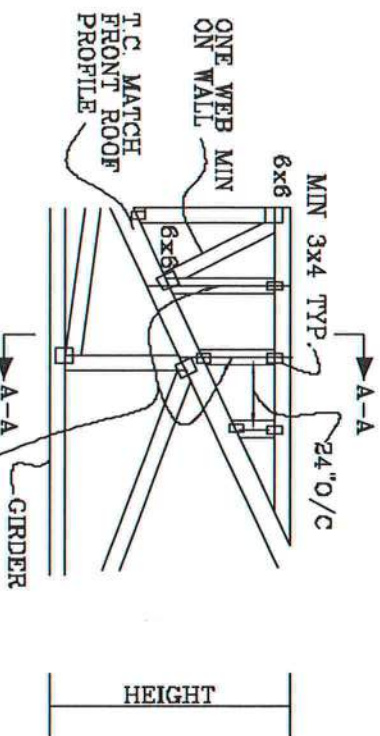


GABLE END TRUSS DETAIL



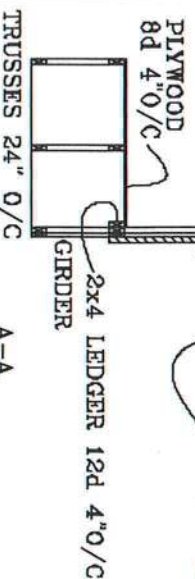
MINIMUM BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" o/c

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 43rd AVENUE
MIAMI BEACH, FL 33144-2661

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
 BOT CHORD 2X4 #2 OR BETTER
 WEBS 2X4 #3 OR BETTER

PICGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.
 SPACE PICGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PICGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PICGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

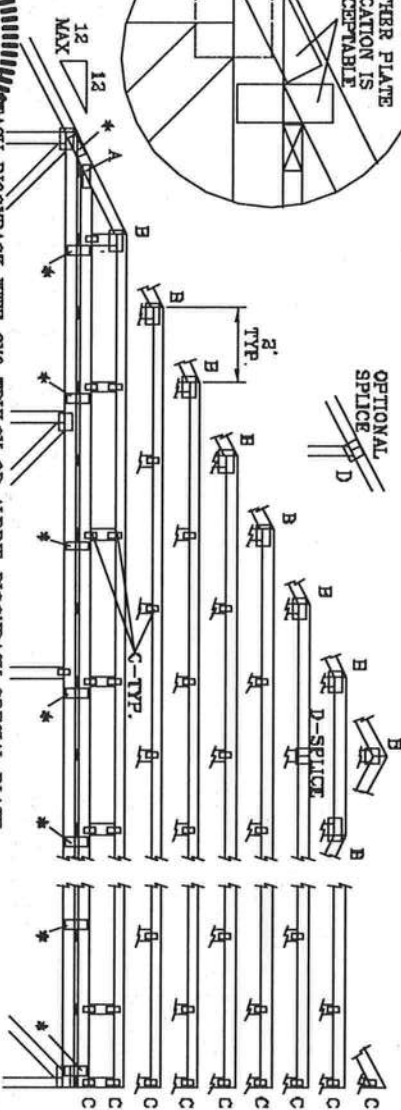
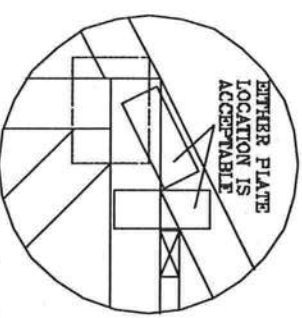
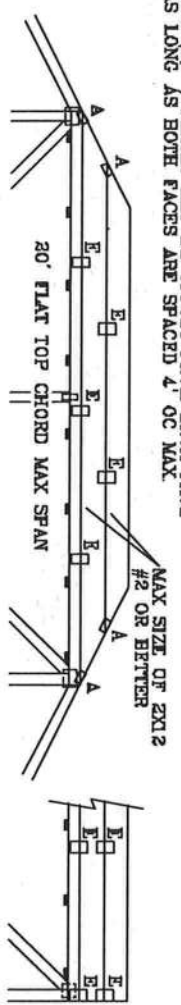
110 MPH WIND, 30' MEAN HGT, FBC

ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE

PLATES AS LONG AS BOTH FACES ARE SPACED 2' OC MAX

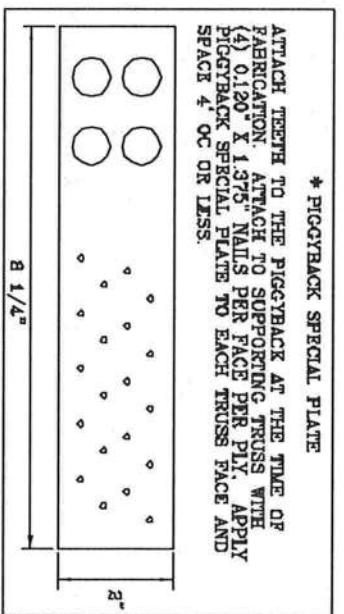


130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

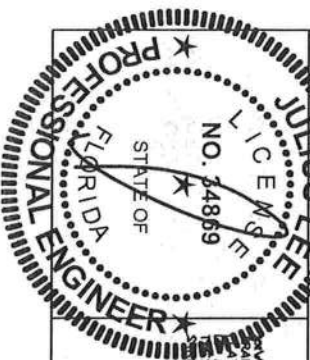
JOINT TYPE	SPANS UP TO		
	30'	34'	62'
A	2X4	2.6X4	3X6
B	4X6	6X6	6X8
C	1.5X3	1.5X4	1.5X4
D	5X4	6X6	6X8
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4" OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICE BUILDING COMPONENT SAFETY INFORMATION, PROVIDED BY THE TRUSS MANUFACTURER, FOR PROPER BRACING, AND TO THE TRUSS MANUFACTURER'S INSTRUCTIONS FOR TRUSS BRACING. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



REVIEWED
 By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
 CONS. ENGINEERS P.A.

1406 SW 4th AVENUE
 DEERBAY BEACH, FL 33441-2661

No. 34869
 STATE OF FLORIDA

MAX LOADING		REF	
55 PSF AT	1.33 DUR. FAC.	DATE	09/12/07
60 PSF AT	1.25 DUR. FAC.	DRWG/ITEK STD	PICGY
47 PSF AT	1.15 DUR. FAC.	—ENG	JL
SPACING	24.0"		

THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X6 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004. 110 MPH. ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC D1=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "I"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.13" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

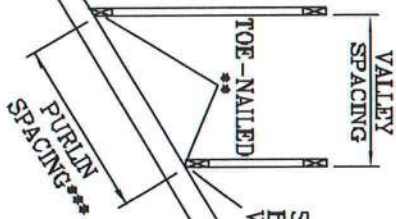
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

+++ NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

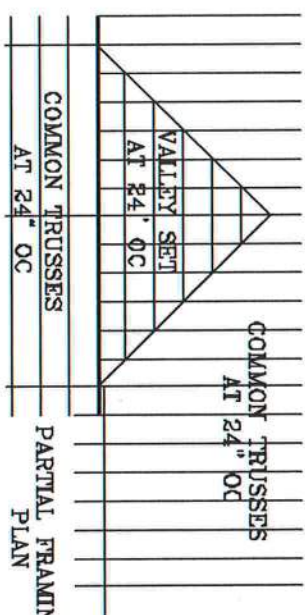
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



**SQUARE CUT
BOTTOM CHORD
VALLEY**

OPTIONAL, STUD
END DETAIL

OPTIONAL HIP JOINT DETAIL



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING PLAN

STRENGTHENING TRUSSES AT 24" OC MAXIMUM SPACING

THIS DRAWING REPLACES DRAWING A105

[illegible]

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING	24"				

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/APA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

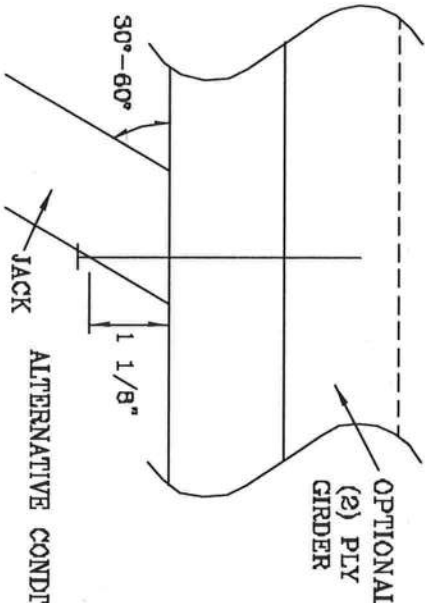
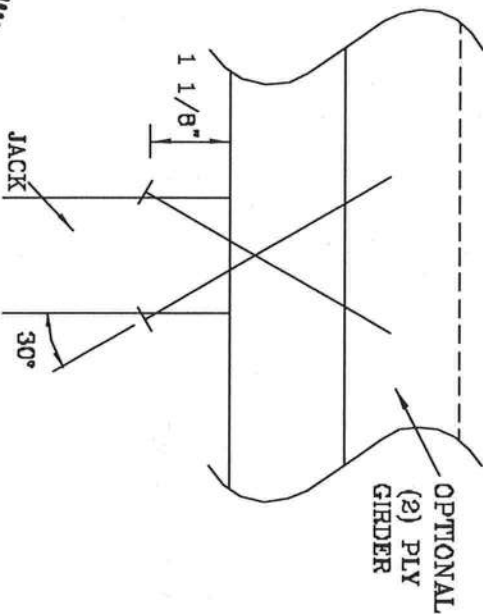
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	187#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.

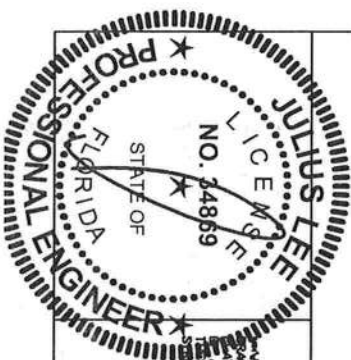


THIS DRAWING REPLACES DRAWING 784040

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO BEST PRACTICES FOR TRUSS CONSTRUCTION, PUBLISHED BY THE TRUSS ASSOCIATION OF AMERICA, 6800 ENTERPRISE LN, WASHINGTON, DC 20015 AND VITA WOOD TRUSS DETAILING PRACTICES. UNLESS OTHERWISE INDICATED, TRUSS CHORDS SHALL HAVE PROPERLY ATTACHED MEMBERS. UNLESS OTHERWISE INDICATED, TRUSS CHORDS SHALL HAVE A PROPERLY ATTACHED RIGID JOINT.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
DELMAR BEACH, FL 33444-2161

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			



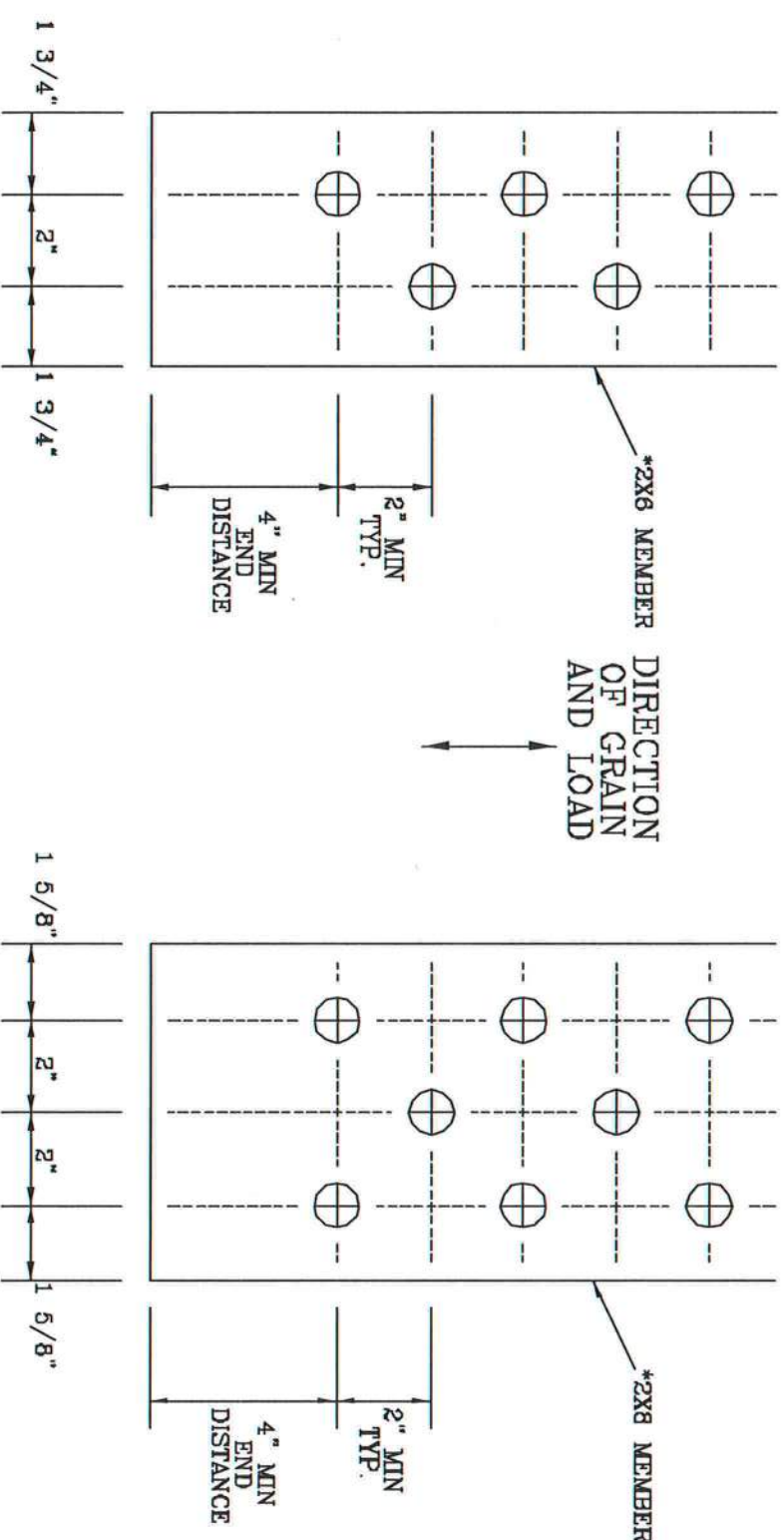
REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT

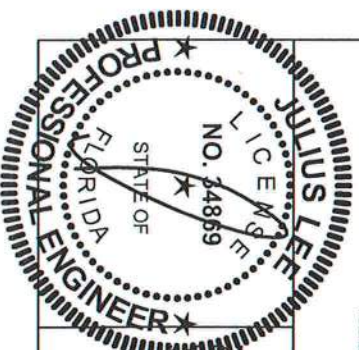


2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2015 IBC BUILDING CODEBOOK SAFETY INFORMATION, PUBLISHED BY THE TRUSS LATE INSTITUTE, 365 CONVENT DR., SUITE 200, MADISON, VA 22719 AND AIAA CODE TRUSS COUNCIL, 1450 BY 4th AVENUE, DELRAY BEACH, FL 33444-2461. THESE FUNCTIONS, CONCEPTS OTHERWISE INDICATED TOP CROSS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND EXTERIOR CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



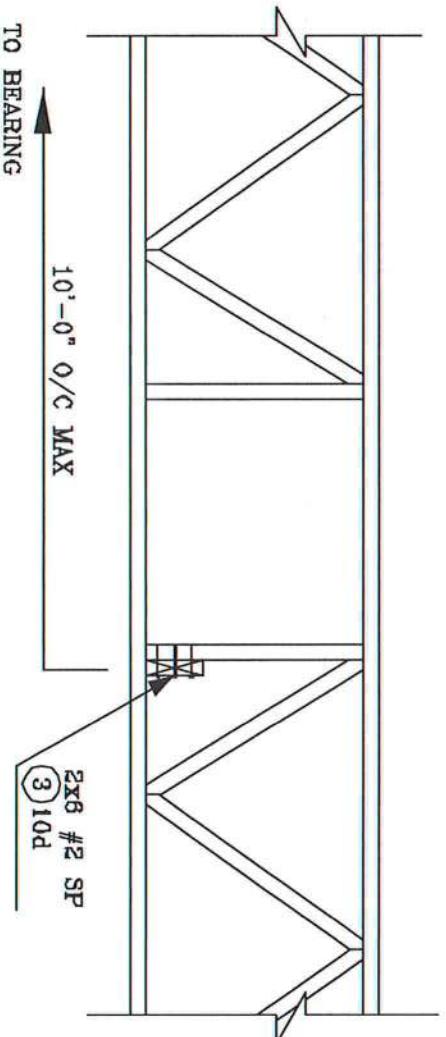
REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1450 BY 4th AVENUE
DELRAY BEACH, FL 33444-2461

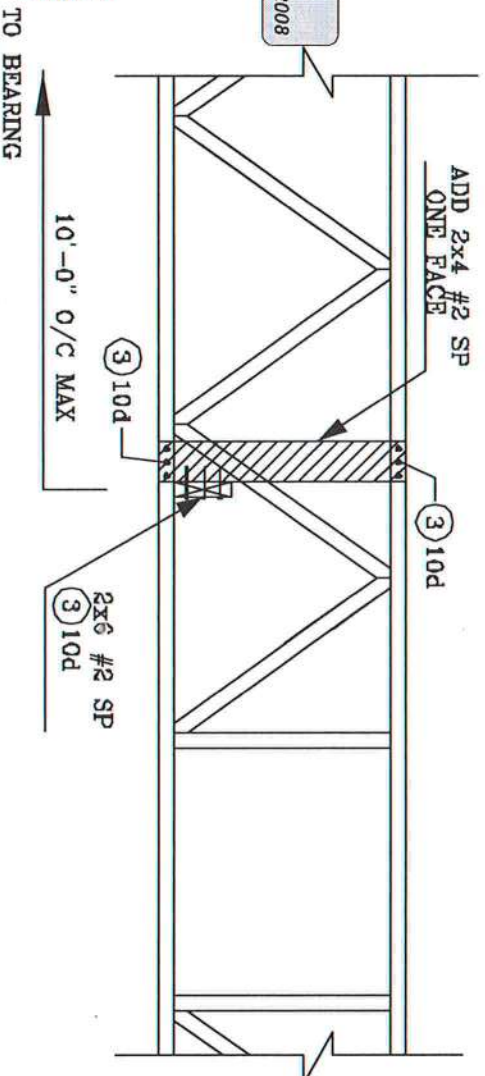
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

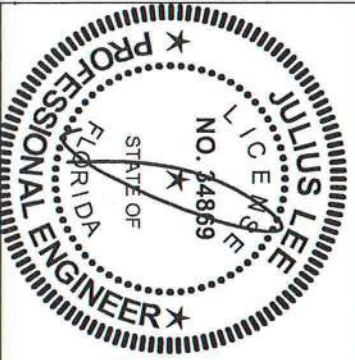


ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008



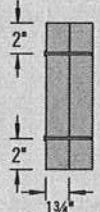
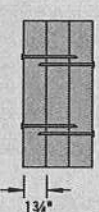
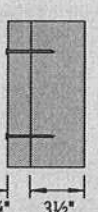
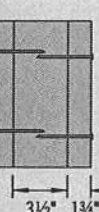
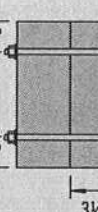
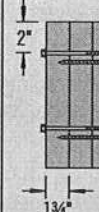
JULIUS LEE'S
CONS. ENGINEERS P.A.

1425 SW 4th AVENUE
DIERKE BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 3/16" maximum.

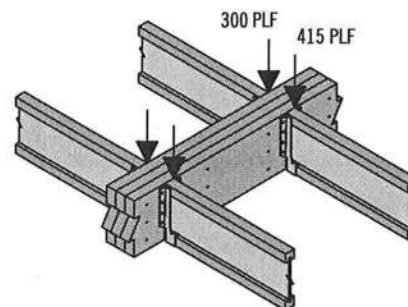
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

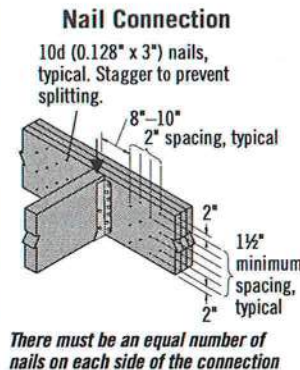
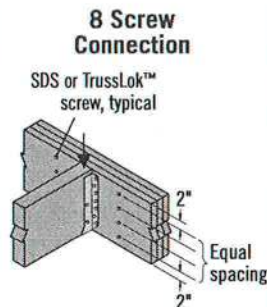
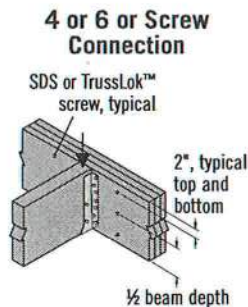
See General Notes on page 38

(2) 6" long screws required.

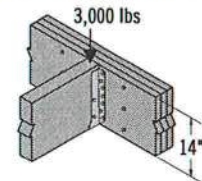
(3) 5" long screws required.

(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections



Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

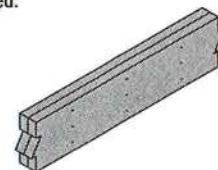
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

BEARING HEIGHT SCHEDULE

	8'-0"
	q'-0"

6/12 PITCH
2'0" O/H

NOTES:

- 1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES INCLUDING TRUSSES UNDER VALLEY FRAMING MUST BE COMPLETED DECIDED OR REFER TO DETAIL V05 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 7' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL 820F TRUSS HANGERS TO BE SAMPSON H1026 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SAMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) BEAUMIERE/DELINTEL (HDB) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND V055 ALL PERIODS ARCHITECTURAL OR OTHER T055 LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Engineer's Stamp Date: _____

Approved By: _____ Date: _____



Bunnell

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PHONE: 904-772-6100 FAX: 904-772-9973

PHONE: 386-793-6894 FAX: 386-793-7973
Lake City

Sanford

PHONE: 407-322-0094 FAX: 407-322-5553

HOUSECRAFT

WILLIAMS RES.

CUSTOM

DATE: 2-9-09
DRAWN BY: K.L.H.
SCALE: NTS
JOB #: 298148

