

CHRIST CENTRAL MINISTRIES STORAGE BUILDING

LAKE CITY, FLORIDA



PLANS PREPARED FOR:

CHRIST CENTRAL MINISTRIES
CHURCH STORAGE BLDG PROJECT

CONTACT:

PASTOR LONNIE JOHNS
217 S.W. DYAL AVENUE
LAKE CITY, FL 32024
386-755-2525

PLANS PREPARED BY:



128 S.W. NASSAU STREET
LAKE CITY, FLORIDA 32025
(386) 758-4209

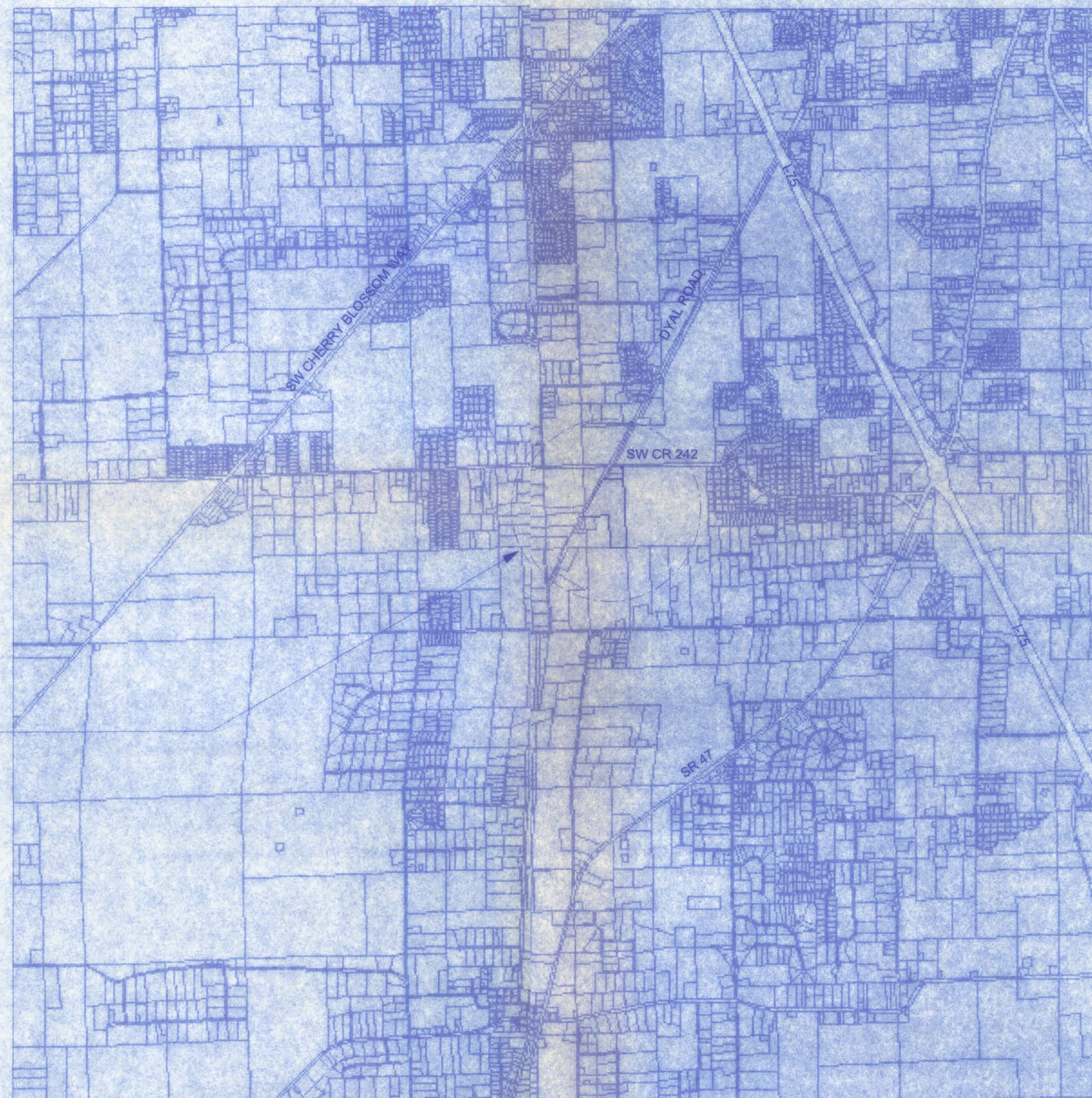
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PROJECT
LOCATION



LOCATION MAP



William H. Freeman
4/24/08
PE# 56001

GENERAL STRUCTURAL NOTES

GENERAL:

1. THE DRAWINGS ARE INTENDED TO SHOW THE GENERAL ARRANGEMENT, DESIGN AND EXTENT OF THE WORK AND ARE PARTIALLY DIAGRAMMATIC. THEY ARE NOT INTENDED TO BE SCALED FOR ROUGH-MEASUREMENTS OR TO SERVE AS SHOP DRAWINGS OR PORTIONS THEREOF.
2. ALL DETAILS AND SECTIONS SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL BE CONSTRUED TO APPLY TO ANY SIMILAR SITUATION ELSEWHERE ON THE PROJECT, EXCEPT WHERE A DIFFERENT DETAIL OR SECTION IS SHOWN.
3. PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND ALL THE SUBCONTRACTORS SHALL VERIFY ALL GRADES, LINES, LEVELS, DIMENSIONS AND COORDINATE EXISTING CONDITIONS AT THE JOB SITE WITH THE PLANS AND SPECIFICATIONS. THEY SHALL REPORT ANY INCONSISTENCIES OR ERRORS IN THE ABOVE TO THE ARCHITECT/ENGINEER BEFORE COMMENCING WORK. THE CONTRACTOR AND HIS SUBCONTRACTORS SHALL LAY OUT THEIR WORK FROM ESTABLISHED REFERENCE POINTS AND BE RESPONSIBLE FOR ALL NES, ELEVATIONS AND MEASUREMENTS IN CONNECTION WITH THEIR WORK.
4. IF ANY ERRORS OR OMISSIONS APPEAR IN THE DRAWINGS, GENERAL NOTES OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE ENGINEER IN WRITING OF SUCH OMISSION OR ERROR PRIOR TO PROCEEDING WITH ANY WORK WHICH APPEARS IN QUESTION. IN THE EVENT OF THE CONTRACTOR'S FAILING TO GIVE SUCH AN ADVANCED NOTICE, HE SHALL BE HELD RESPONSIBLE FOR THE RESULTS OF ANY SUCH ERRORS OR OMISSIONS AND THE COST OF RECTIFYING THE SAME.
5. THE CONTRACTOR SHALL USE THE STRUCTURAL DRAWINGS AND SPECIFICATIONS TOGETHER WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND OTHER TRADE DRAWINGS AND SHOP DRAWINGS, TO LOCATE DERESSED SLABS, SLOPES, DRAINS, OUTLETS, RECESSES, OPENINGS, BLT SETTING, SLEEVES, DIMENSIONS, ETC. NOTIFY ARCHITECT/ENGINEER, IN WRITING, OF ANY POTENTIAL CONFLICTS BEFORE PROCEEDING WITH THE WORK.

SHOP DRAWINGS AND DELEGATED ENGINEERING:

1. ALL SHOP DRAWINGS SHALL BE SUBMITTED FOR ENGINEER'S REVIEW ONLY AFTER THEY HAVE BEEN THOROUGHLY REVIEWED BY THE CONTRACTOR FOR CONSTRUCTION METHODS, DIMENSIONS AND OTHER TRADE REQUIREMENTS, AND STAMPED WITH THE CONTRACTOR'S APPROVAL. STILL, THE ENGINEER ASSUMES NO RESPONSIBILITY FOR DIMENSIONS, QUANTITIES, ENGINEERING DESIGN BY DELEGATED ENGINEERS, ERRORS OR OMISSIONS AS A RESULT OF REVIEWING ANY SHOP DRAWINGS. ANY ERRORS OR OMISSIONS MUST BE MADE GOOD BY THE CONTRACTOR, IRRESPECTIVE OF RECEIPT, CHECKING OR REVIEW OF DRAWINGS BY THE ENGINEER AND EVEN THOUGH WORK IS DONE IN ACCORDANCE WITH SUCH DRAWINGS.
2. BEFORE STRUCTURAL INSPECTIONS CAN BE MADE ON A PORTION OF THE STRUCTURE, ALL RELATED SHOP DRAWINGS, DELEGATED ENGINEERING, PRODUCT APPROVAL, MANUFACTURER'S DATA AND OTHER RELATED INFORMATION, MUST BE REVIEWED AND ACCEPTED BY THE ENGINEER OF RECORD AND APPROVED BY THE BUILDING DEPARTMENT.
3. SHOP DRAWINGS SHALL CONTAIN ALL INFORMATION SHOWN ON THE STRUCTURAL PLANS (RELATED TO THE DELEGATED DESIGN) INCLUDING A DESIGN LOADS, IN ADDITION TO THE INFORMATION REQUIRED BY THE DELEGATED ENGINEER'S DESIGN.
4. A/E WILL REVIEW ALL SUBMITTED SHOP DRAWINGS, PREPARED AND SIGNED AND SEALED BY THE CONTRACTOR'S DELEGATED ENGINEER, ONLY FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT, REQUIRED LOADS AND COORDINATION WITH THE STRUCTURAL DESIGN.
5. CONTRACTOR SHALL SUBMIT TO THE A/E ONLY ONE SET OF SEPIA AND/VO SETS OF BLUE PRINTS OF THE STRUCTURAL SHOP DRAWINGS FOR A/E REVIEW, BEFORE STARTING FABRICATION. THE A/E WILL RETURN THE MARKED-UP/ND STAMPED SEPIA TO THE CONTRACTOR. THESE SEPIA COPIES SHALL BE USED TO MAKE THE PRINTS REQUIRED FOR SHOP DRAWING DISTRIBUTION. SET OF BLUE PRINTS (WITHOUT SEPIA) WILL NOT BE ACCEPTED.

CONSTRUCTION MEANS AND METHODS:

1. THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCE OR PROCEDURES, SAFETY PRECAUTIONS, SHORES, RESHORES, LATERAL BRACING AND PROGRAMS IN CONNECTION WITH THE PROJECT, ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. OUR SERVICES DO NOT GUARANTEE NOR ASSURE LIABILITY FOR THE JOB SAFETY, TEMPORARY SHORING AND BRACING AND THE PERFORMANCE OF THE CONTRACTOR.
2. THE CONTRACTOR IS RESPONSIBLE AND SHALL COMPLY WITH THE SAFETY REQUIREMENTS OF THE STANDARD BUILDING CODE AND APPLICABLE LOCAL, STATE AND FEDERAL LAWS.
3. PROVIDE ALL SHORING, BRACING AND SHEETING AS REQUIRED FOR SAFETY, STRUCTURAL STABILITY AND FOR THE PROPER EXECUTION OF THE WORK. REMOVE WHEN WORK IS COMPLETED.
4. PROVIDE AND MAINTAIN GUARD LIGHTS AT ALL BARRICADES, RAILINGS, OBSTRUCTIONS IN THE STREETS, ROADS OR SIDEWALKS AND A TRENCHES OR PITS ADJACENT TO PUBLIC WALKS OR ROADS.
5. AT ALL TIMES, PROVIDE PROTECTION AGAINST WEATHER (RAIN, WIND, STORMS OR THE SUN), SO AS TO MAINTAIN ALL WORK, MATERIALS, APPARATUS AND FIXTURES FREE FROM INJURY OR DAMAGE.
6. AT THE END OF THE DAYS WORK, COVER ALL WORK LIKELY TO BE DAMAGED. ANY WORK DAMAGED BY FAILURE TO PROVIDE PROTECTION SHALL BE REMOVED AND REPLACED WITH NEW WORK AT THE CONTRACTOR'S EXPENSE.
7. THE CONTRACTOR SHALL PAY FOR ALL DAMAGES TO ADJACENT STRUCTURES, SIDEWALKS AND TO STREETS OR OTHER PUBLIC PROPRIETOR PUBLIC UTILITIES.

STRUCTURAL DESIGN CRITERIA:

1. THE DESIGN COMPLIES WITH THE REQUIREMENTS OF THE 2004 FLORIDA BUILDING CODE - WITH ALL UPDATES AND OTHER REFERENCED CODES AND SPECIFICATIONS. ALL CODES AND SPECIFICATIONS SHALL BE LATEST EDITION AT TIME OF PERMIT.
2. WIND LOAD CRITERIA:
BASED ON ANSI/ASCE 7-02, BASIC WIND VELOCITY 110 MPH.
3. ROOF DESIGN LOADS:
SUPERIMPOSED DEAD LOADS: 20 PSF
SUPERIMPOSED LIVE LOADS: 20 PSF
4. FLOOR DESIGN LOADS:
SUPERIMPOSED DEAD LOADS: 25 PSF
SUPERIMPOSED LIVE LOADS:
COMMERCIAL 50 PSF
BALCONIES 60 PSF
5. WIND NET UPLIFT: ARE AS INDICATED ON PLANS

FOUNDATIONS: (SPREAD FOOTINGS)

1. FOUNDATIONS ARE DESIGNED TO BEAR ON WELL COMPACTED GRADE OR CLEAN FILL OF AN ALLOWABLE BEARING CAPACITY OF 2,500 PSF MINIMUM. A CERTIFIED TESTING LABORATORY SHALL BE ENGAGED BY THE OWNER TO VERIFY THAT THE REQUIRED BEARING CAPACITY WAS OBTAINED. SAID SOIL CAPACITY SHALL BE CERTIFIED AND TESTED BY A FLORIDA REGISTERED FOUNDATION ENGINEER, PRIOR TO CASTING OF CONCRETE IN THE FOOTINGS.
2. NATURAL GRADE (OR FILL) BELOW FOOTINGS SHALL BE COMPACTED TO 98 % MODIFIED PROCTOR (ASTM D-1557).
3. TOP OF WALL FOOTINGS TO BE AT THE SAME ELEVATION AS TOP OF COLUMN PAD FOOTINGS. STEP WALL FOOTING FROM HIGHER COLUMN FOOTING TO THE LOWER ONE (AS DETAILED ON THE PLANS).
4. TOP OF ALL FOOTINGS TO BE A MINIMUM 1'-4" BELOW THE TOP OF CONCRETE SLAB ON GRADE (UNLESS OTHERWISE NOTED) OR MINIMUM 1'-0" BELOW FINISHED GRADE, WHICHEVER IS LOWER. IN THE EVENT THAT THE SLAB STEPS ON EACH SIDE OF THE FOOTING, THE FOOTING SHALL BE 1'-4" BELOW TOP OF THE LOWER SLAB.
5. REINFORCING IN THE CONTINUOUS WALL FOOTINGS (MONOLITHIC AND NON-MONOLITHIC) SHALL BE SPLICED 36 BAR DIAMETERS MINIMUM AND SHALL EXTEND CONTINUOUSLY THRU ALL FOOTING PADS.
6. ALL LONGITUDINAL REBARS IN THE CONTINUOUS WALL FOOTINGS, SHALL BE CONTINUED AT BENTS AND CORNERS BY BENDING THE REBARS 48 BAR DIAMETERS AROUND THE CORNERS OR ADDING MATCHING CORNER BARS, EXTENDING 48 BAR-DIAMETERS INTO FOOTING EACH SIDE OF CORNER OR BENT.
7. ALL FOOTINGS SHALL BE 12" MINIMUM THICKNESS.

CONCRETE SLABS ON GRADE:

1. ALL INTERIOR AND EXTERIOR SLABS AND WALKWAYS AS SHOWN ON THE STRUCTURAL OR ARCHITECTURAL PLANS, SHALL BE FOUR INCHES THICK MINIMUM REINFORCED WITH 6 X 6 - W1.4 X W1.4 WELDED WIRE FABRIC (UNLESS OTHERWISE NOTED).
2. ALL SLABS ON GRADE TO BE CONSTRUCTED IN ACCORDANCE WITH LATEST A.C.I. - "GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION" (A.C.I. - 302.1R).
3. JOINTS SHALL BE PROVIDED IN ALL INTERIOR SLABS ON GRADE AT COLUMN CENTER-LINES DIVIDING THE SLAB INTO SQUARE PANELS NOT TO EXCEED 20 X 20 FT. IN SIZE. CAST SLAB IN LONG ALTERNATE STRIPS. PROVIDE A CONTRACTION JOINT BETWEEN EACH STRIP. SEE PLAN FOR SAW-CUT, CONTRACTION AND ISOLATION JOINT DETAILS.
4. PROVIDE SAW-CUT JOINTS AT ALL SIDEWALKS AT A MAXIMUM SPACING OF FIVE FEET ON CENTERS AND ISOLATION JOINTS AT 20 FEET O.C. (U.O.N.).
5. FILL MATERIAL SHALL BE PLACED IN LIFTS NOT EXCEEDING 12" AND COMPACTED TO 98 % MODIFIED PROCTOR (ASTM D-1557) WITHIN A DISTANCE OF 3 FEET BEYOND ALL FOOTING EDGES. TAKE AT LEAST ONE DENSITY TEST FOR EACH 1,600 SQ.FT. OF AREA AND 12" BELOW SURFACE. SEND RESULTS OF THE TEST TO OWNER, ARCHITECT AND ENGINEER.

CONCRETE AND REINFORCING:

1. CONCRETE DESIGN AND REINFORCEMENT IN ACCORDANCE WITH "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (A.C.I. 318 - LATEST EDITION) AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" - (A.C.I. 315 - LATEST EDITION).
2. ALL CONCRETE WORK IN ACCORDANCE WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDING" (A.C.I. 301 - LATEST EDITION), PRODUCTION OF CONCRETE, DELIVERY, PLACING AND CURING TO BE IN ACCORDANCE WITH "HOT WEATHER CONCRETING" (A.C.I. 305R - LATEST EDITION).
3. ALL CONCRETE TO BE REGULAR WEIGHT WITH A DESIGN STRENGTH OF 3,000 P.S.I. AT 28 DAYS. MAXIMUM SLUMP 5".
4. ALL REINFORCING TO BE NEW BILLET STEEL CONFORMING TO THE LATEST A.S.T.M. A-615 GRADE 60, FABRICATED IN ACCORDANCE WITH C.R.S.I. MANUAL OF STANDARD PRACTICE AND PLACED IN ACCORDANCE WITH A.C.I. 315 AND C.R.S.I. MANUAL OF STANDARD PRACTICE.
5. CONCRETE COVER UNLESS OTHERWISE DETAILED ON DRAWINGS:

FOOTINGS: (BOTTOM) 3"
(TOP & SIDES) 2"

SLABS ON GRADE: CENTERED W/SLAB

COLUMNS AND BEAMS: (TO THE TIES) 1-1/2"

6. COLUMN REINFORCEMENT: DOWELS TO BE SAME SIZE AND NUMBER AS VERTICAL REBARS ABOVE. LAP 36 BAR DIAMETER OR MINIMUM OF 18 INCHES, U.O.N. PROVIDE RIGID TEMPLATES FOR DOWEL LOCATION. PROVIDE STANDARD HOOKS AT TOP OF ALL VERTICAL REINFORCEMENT AT NONCONTINUOUS COLUMNS (U.O.N.).

7. ALL DOWELS FOR COLUMNS SHALL BE SECURED IN POSITION PRIOR TO CONCRETING. PUSHING THE DOWELS INTO POSITION IN WET CONCRETE IS NOT PERMITTED.

8. BEAM REINFORCEMENT: LAPPED 36 BAR DIAMETER OR MINIMUM 18 INCHES. BOTTOM BARS SPLICED ONLY AT SUPPORTS, TOP BARS SPLICED ONLY AT MID-SPAN. ALL TOP BARS HOOKED AT NONCONTINUOUS EDGES (U.O.N.). ALL HOOKS TO BE STANDARD 90 DEGREE HOOKS AS REQUIRED (U.O.N.).

9. ADDED REINFORCEMENT: PROVIDE ADDITIONAL CORNER BARS BENT 36 INCHES MINIMUM EACH WAY AT "L" AND "T" CORNERS IN OUTER FACES OF ALL BEAMS TO MATCH ALL HORIZONTAL BAR (TOP, BOTTOM AND INTERMEDIATE REBARS).

10. SEE PLAN FOR MINIMUM SIZE CONCRETE TIE BEAM REQUIREMENTS.

REINFORCED MASONRY WALLS:

1. HOLLOW LOAD-BEARING MASONRY UNITS SHALL CONFORM TO ASTM C-90, TYPE 1, GRADE N, SQUARE END, WITH A MINIMUM AVERAGE COMPRESSIVE STRENGTH ON NET AREA OF 1700-2,000 (PSI). CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 530.1 SPECIFICATIONS.
2. SPECIAL INSPECTOR SERVICES ARE REQUIRED FOR ALL REINFORCED MASONRY CONSTRUCTION. THE SPECIAL INSPECTOR SHALL INSPECT THE PLACING OF THE REBARS IN THE CELLS, VERIFY CLEANLINESS OF THE CELLS TO BE GROUTED, AND OBSERVE THE PLACING OF THE GROUT OR CONCRETE INTO THE CELLS.
3. MORTAR SHALL CONFORM TO ASTM C-270, TYPE "M" OR "S".
4. LAY ALL MASONRY WITH FULL FACE HEAD JOINTS AND WITH FACE SHELL MORTAR BEDDING.
5. MASONRY ANCHORAGE TO SUPERSTRUCTURE SHALL BE PROVIDED IN ACCORDANCE WITH STRUCTURAL DRAWINGS AND DETAILS.
6. THE USE OF ADMIXTURES SHALL NOT BE PERMITTED WITHOUT PRIOR REVIEW OF THE ENGINEER.
7. VERTICAL REINFORCING:
 - (A) ASTM A-615 PER REINFORCING SECTION.
 - (B) WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL INCH TO SIX INCHES VERTICAL FOR ALIGNMENT, EVEN THOUGH IT IS IN A CELL ADJACENT TO THE VERTICAL WALL REINFORCING.
 - (C) VERTICAL REINFORCING STEEL SHALL BE PLACED CENTERED IN THE CELL. LAP 48 BAR-DIAMETERS. PROVIDE BAR SPACERS AS REQUIRED TO MAINTAIN REINFORCING SECURED IN POSITION.
 - (D) VERTICAL REINFORCEMENT SHALL BE PROVIDED AT EACH SIDE OF OPENINGS IN WALL, AT WALL INTERSECTIONS, CORNERS AND ENDS. THIS REINFORCING SHALL BE THE SAME SIZE AS THE SCHEDULED WALL REINFORCING FOR THE PARTICULAR WALL, BUT NEVER LESS THAN A #5 REBAR. SPECIAL CARE SHALL BE TAKEN TO INSURE THAT CELLS TO BE GROUTED LINE UP PROPERLY AND ARE CLEAN OF EXCESS MORTAR.
 - (E) ALL VERTICAL REINFORCING SHALL BE HOOKED INTO THE BOND BEAMS AT THE NON-CONTINUOUS END OF THE REBARS.
 - (F) PROVIDE INSPECTION HOLES AT THE BOTTOM OF EACH REINFORCED MASONRY CELL, AS REQUIRED FOR LIFTS HIGHER THAN 5 FT.
8. HORIZONTAL REINFORCING:
PROVIDE GALVANIZED #9 GAGE, LADDER TYPE HORIZONTAL JOINT REINFORCING EVERY SECOND BLOCK COURSE (1'-4" O.C. VERTICALLY) LAPPED 7'-1/2". PROVIDE SPECIAL HORIZONTAL REINFORCING AT "T" AND "L" INTERSECTION. ANCHOR TO COLUMNS WITH MINIMUM 4" EXTENSION INTO AREA OF POUR.
9. PROVIDE "DOVE-TAIL" ANCHORS AT 16" O.C. VERTICALLY FOR ALL MASONRY PLACED ADJACENT TO ALREADY IN PLACE COLUMNS.
10. CELL FILLING CONCRETE SHALL BE "PEA DOCK" CONCRETE MIX (6" TO 9" SLUMP) OR GROUT WITH 1700-3,500 PSI MIN. AT 28 DAYS.
11. LINTELS:
 - A. THE CONTRACTOR SHALL PROVIDE PRECAST CONCRETE OR CAST-IN-SITE LINTELS AT THE HEADS OF ALL OPENINGS IN MASONRY WALLS NOT EXCEEDING SIX (6) FEET IN WIDTH WHERE BEAMS HAVE NOT BEEN SPECIFIED. FOR OPENING ADJACENT TO CONCRETE COLUMNS - THE LINTEL SHALL BE CAST-IN-PLACE WITH THE COLUMN.
 - B. LINTEL MAY BE INTEGRAL WITH THE STRUCTURAL OR TIE BEAM WHEN HEAD OF THE OPENING IS 16 INCHES OR LESS BELOW. CONTINUE BEAM'S TYPICAL BOTTOM REBARS THROUGH AND ADD 2-#6 BOTTOM TRUSS BARS AT DROPS AND 2-#3 STIRRUPS AT 6 INCHES O.C. EACH END AT DROP.
 - C. MINIMUM BEARING FOR ALL LINTELS 8 INCHES EACH SIDE OR PROVIDE DOWELS AND POCKETS IN ADJACENT CONCRETE COLUMNS.
 - D. LINTEL TO BE MINIMUM OF 8 INCHES DEEP WITH 2-#4 TOP AND BOTTOM FOR CLEAR SPANS LESS THAN 6 FEET, 12 INCHES DEEP WITH 2-#6 TOP AND BOTTOM AND 2-#3 STIRRUPS AT 6 INCHES O.C. EACH END, FOR SPANS GREATER THAN 6 FEET (UP TO 8 FEET). CALL ENGINEER FOR SPANS LARGER THAN 8 FEET WITH NO SPECIFIED BEAMS OR LINTELS OVER.

STRUCTURAL STEEL: (SHOP DRAWINGS REQUIRED)

1. ALL STRUCTURAL STEEL TO BE DOMESTIC A.S.T.M. A-36 (Fy=36 K.S.I.) AND DESIGNED IN ACCORDANCE WITH THE LATEST A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND THE A.I.S.C. CODE OF STANDARD PRACTICE.
2. STEEL TUBES TO BE DOMESTIC STEEL CONFORMING TO A.S.T.M. A-500 GRADE B (Fy=46 K.S.I.).
TUBE AND PIPE COLUMNS TO BE CONCRETE FILLED WITH VENT HOLES TOP, MIDDLE AND BOTTOM.
3. ALL COLUMN BASE AND CAP PLATES SHALL BE 3/4" THICK (UNLESS OTHERWISE NOTED). WIDTH AND LENGTH AS REQUIRED FOR PROPER BOLTING AND AS INDICATED ON THE PLANS AND DETAILS.
4. ALL WELDING TO BE IN ACCORDANCE WITH A.W.S. LATEST "STRUCTURAL WELDING CODE - STEEL". CLEAN AND RUSTPROOF ALL FIELD WELDS WITH HEAVY DUTY RUSTPROOFING PAINT.

5. ALL CONNECTIONS TO BE FIELD AND SHOP WELDED AND TO DEVELOP MEMBER IN SHEAR.

6. SPLICE LOCATIONS TO BE REVIEWED BY ARCHITECT/ENGINEER.

7. STEEL BEARING ON STEEL TO BE WELDED THERETO.

STRUCTURAL WOOD:

1. TO CONFORM TO RULES OF THE MANUFACTURER'S ASSOCIATION UNDER WHOSE RULES THE LUMBER IS PRODUCED. (SEE SUPPLIER'S SPECIFICATIONS).
2. TO BE AIR DRIED, WELL SEASONED AND GRADE MARKED AT MILL.
3. TO BE NO. 2 SOUTHERN PINE, UTILITY GRADE DOUGLAS FIR OR WEST COAST HEMLOCK.
4. ALL STRUCTURAL WOOD TO BE SURFACED FOUR (4) SIDES (S-4-S) WITH A MINIMUM FIBER STRESS IN BENDING OF 1,200 P.S.I. AND A MAXIMUM MOISTURE CONTENT OF 18 PERCENT.
5. ALL LUMBER AND PLYWOOD IN CONTACT WITH CONCRETE, STUCCO, MASONRY OR OTHER CEMENTITIOUS MATERIALS SHALL BE TREATED TO COMPLY WITH AWPA STANDARD LP-2.
6. STORE ALL LUMBER ABOVE GRADE OR FLOOR. STACK TO ALLOW PROPER AIR CIRCULATION AND PROTECT FROM WETTING WITH SUITABLE COVER.

COLD FORMED METAL FRAMING: (SHOP DRAWINGS REQUIRED)

1. ALL COLD FORMED METAL FRAMING SHALL BE DOMESTIC A.S.T.M. A-653 (Fy= 33 K.S.I.) STEEL, AND DESIGNED IN ACCORDANCE WITH THE LATEST S.S.M.A. SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF COLD FORMED METAL FRAMING AND THE S.S.M.A. CODE OF STANDARD PRACTICE.
2. ALL CFMF COMPONENTS SHALL BE MANUFACTURED AS PER ASTM C 955 AND BE GALVANIZED WITH A MINIMUM G-60 COATING PER ASTM C 955.

ALL PRODUCTS SHALL BE FREE OF RUST, DENTS, BENDS & TWISTS AND STORED ON A FLAT PLANE PRIOR TO INCLUSION IN THE WORK.

3. ALL WELDING TO BE IN ACCORDANCE WITH A.W.S. LATEST, E1.3 & D1.3 "STRUCTURAL WELDING CODE - STEEL". CLEAN AND RUSTPROOF ALL FIELD WELDS WITH ZINK RICH RUSTPROOFING PAINT.

4. BOTTOM TRACK SHALL BE SECURED TO THE CONCRETE FOUNDATION W/ ANCHOR BOLTS AS PER THE FOUNDATION PLAN AND SHALL BE FURTHER FASTENED AT EA. FULL STUD W/ .177"-X 1" PAF, SHOT THROUGH A 1"-X 16 GA HOLELESS WASHER.

5. ALL CONNECTIONS TO BE FIELD AND SHOP WELDED AND TO FULLY DEVELOP MEMBER IN SHEAR.

6. SPLICE LOCATIONS TO BE REVIEWED BY ARCHITECT/ENGINEER.

7. STEEL BEARING ON STEEL TO BE WELDED THERETO.

TERMITE PROTECTION NOTES:

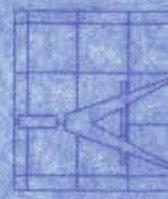
SOIL CHEMICAL BARRIER METHOD:

1. A PERMANENT SIGN WHICH IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED. THE SIGN SHALL BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL. FBC 104.2.6.
2. CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-0" AWAY FROM BUILDING SIDE WALLS. FBC 1503.4.4.
3. IRRIGATION/SPRINKLER SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN 1'-0" FROM BUILDING SIDE WALLS. FBC 1503.4.4.
4. TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION, BETWEEN WALL COVERINGS AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6". EXCEPTION: PAINT AND DECORATIVE CEMENTIOUS FINISH LESS THAN 5/8" THICK ADHERED DIRECTLY TO THE FOUNDATION WALL. FBC 1403.1.6.
5. INITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE. FBC 1816.1.1.
6. SOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLUDING SPACES BOXED OR FORMED. FBC 1816.1.2.
7. BOXED AREAS IN CONCRETE FLOOR FOR SUBSEQUENT INSTALLATION OF TRAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FORMS. PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ELIMINATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT. FBC 1816.1.3.
8. MINIMUM 6 MIL VAPOR RETARDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RETARDER PLACEMENT, RETREATMENT IS REQUIRED. FBC 1816.1.4.
9. CONCRETE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUST BE REMOVED BEFORE EXTERIOR SOIL TREATMENT. FBC 1816.1.5.
10. SOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE OR GRADE WITHIN 1'-0" OF THE STRUCTURE SIDEWALLS. FBC 1816.1.6.
11. AN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL BE RETREATED. FBC 1816.1.6.
12. ALL BUILDINGS ARE REQUIRED TO HAVE PER-CONSTRUCTION TREATMENT FBC 1816.1.7.
13. A CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPARTMENT BY A LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCUPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES". FBC 1816.1.7.
14. AFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW AND WITHIN 1'-0" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES, FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL. FBC 2303.1.3.
15. NO WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHIN 15'-0" OF ANY BUILDING OR PROPOSED BUILDING. FBC 2303.1.4.

CHRIST CENTRAL MINISTRIES
STORAGE BUILDING

128 SW NASSAU STREET
LAKE CITY, FL 32025
(386)750-4209

CERTIFICATE OF AUTHORIZATION # R0008703



Freeman
Design Group Inc.

DATE
4/24/08

DRAWN BY
J.T.D.

APPROVED
V.H.F.

REVISIONS

SHEET

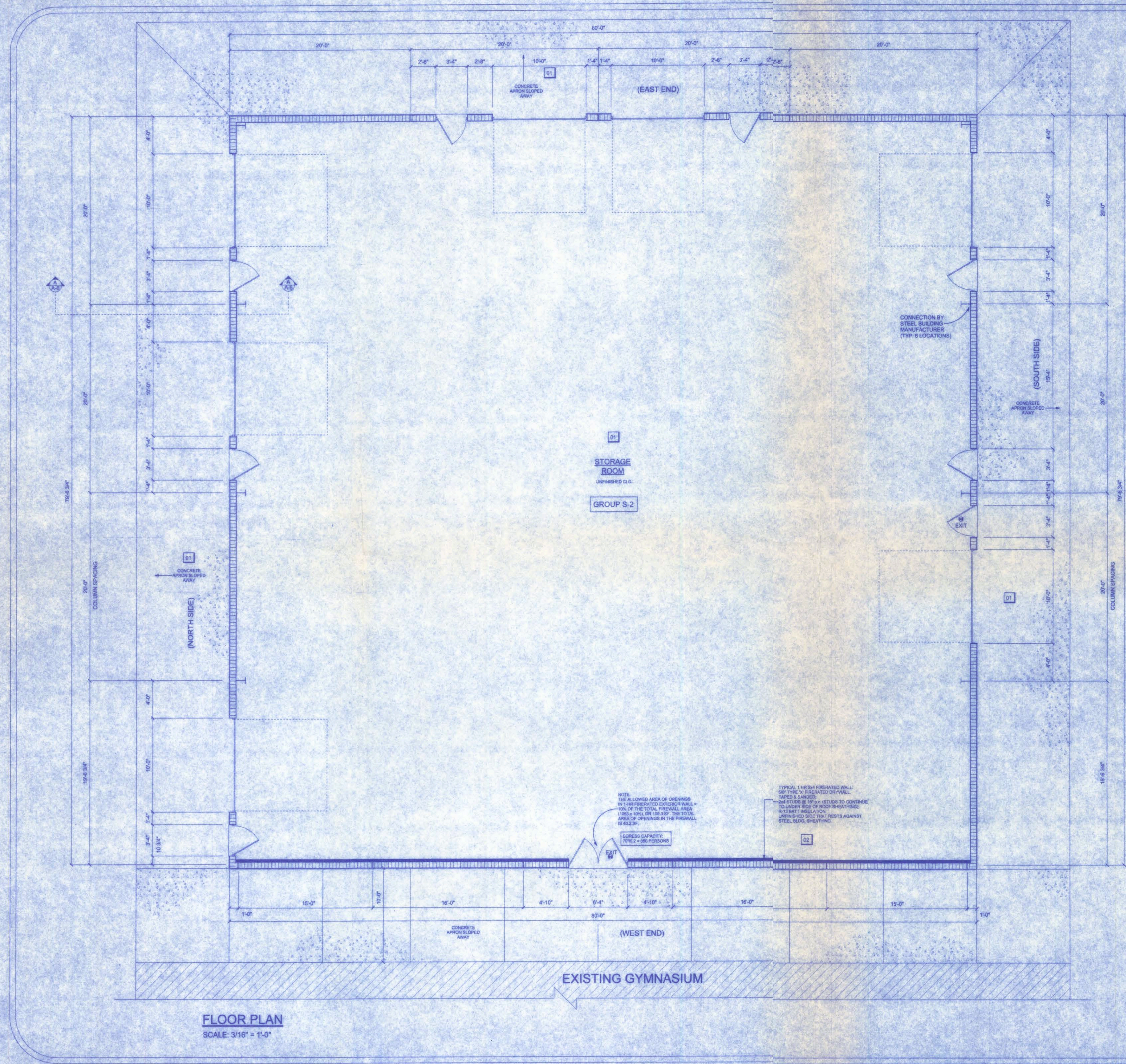
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PROJECT NO.

08.C017



FLOOR PLAN
SCALE: 3/16" = 1'-0"

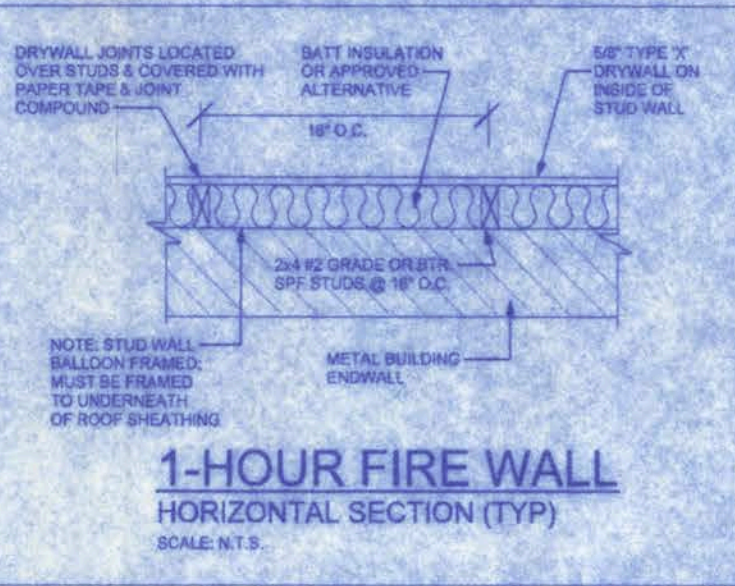
NOTE:
ALL WIND LOADS ARE IN ACCORDANCE WITH SECTION 1609, FLORIDA BUILDING CODE, 2004 EDITION w/2006 REVISIONS, AND IN ACCORDANCE WITH ASCE-7

BASIC WIND SPEED		110 MPH
IMPORTANCE FACTOR		1.0
BUILDING CATEGORY		2
EXPOSURE		B
INTERNAL PRESSURE COEFFICIENT		+/- 0.18
COMPONENT AND CLADDING PRESSURE:	WALLS	+21.8/-29.1 PSF
	ROOF	+12.5/-29.1 PSF
	OVERHANGS	-71.8 PSF
TYPE OF STRUCTURE		ENCLOSED
ROOF DEAD LOAD		10 psf
ROOF LIVE LOAD		20 psf
FLOOR DEAD LOAD		20 psf
FLOOR LIVE LOAD		40 psf

BUILDING USE, CLASSIFICATION & OCCUPANCY AS PER TABLES 503 & 1004.1.2, FLORIDA BUILDING CODE, 2004 ED.	
BUILDING GROUP/OCCUPANCY	GROUP S-2
TABLE 500 TYPE OF CONSTRUCTION	TYPE V - UNPRO.
TABLE 500 AREA/HEIGHT LIMITATIONS	9 KSF/2 STORY
OCCUPANCY	
SALES AREA: 1,800 SF GROSS 8400 SF / 500 = 12.8 OR 13	13 OCCUPANTS

NOTE:
provide a readily visible sign on or adjacent to all egress doors stating: THIS EXIT TO REMAIN UNLOCKED WHEN THIS BUILDING IS OCCUPIED. the sign shall have 1" lettering with contrasting background.

PRODUCT CODE	SIZE	COUNT
36X80 COLONIAL PLAIN	3'-0"	8
72X80 COLONIAL PLAIN	6'-0"	3
120X120 - ROLL-UP METAL	10'-0"	1



- 01 CONCRETE FLOORING, 8" THICK SLAB
- 02 1-HOUR FIRE RATED EXTERIOR WALL

AREA SUMMARY	
STORAGE (UNFINISHED).....	6,400 SF
TOTAL.....	6,400 SF

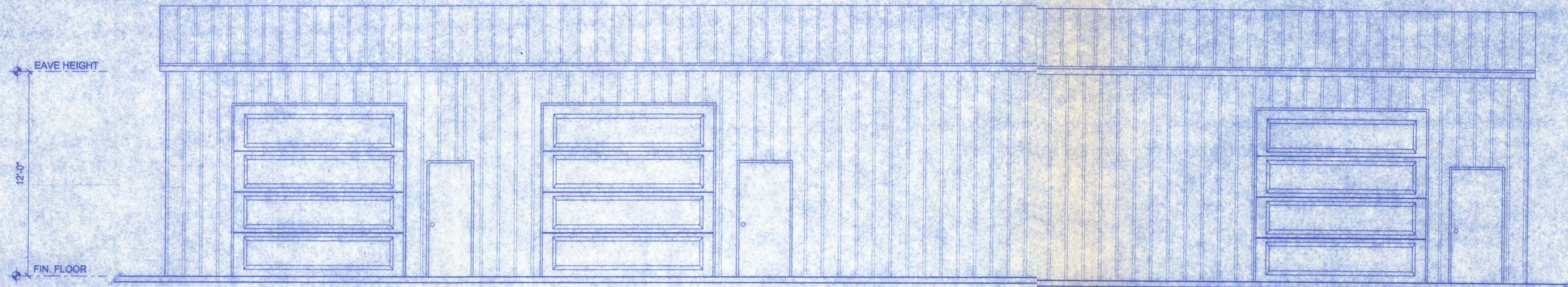
CHRIST CENTRAL MINISTRIES STORAGE BUILDING

128 SW NASSAU STREET
LAKE CITY, FL 32025
(386)758-4209

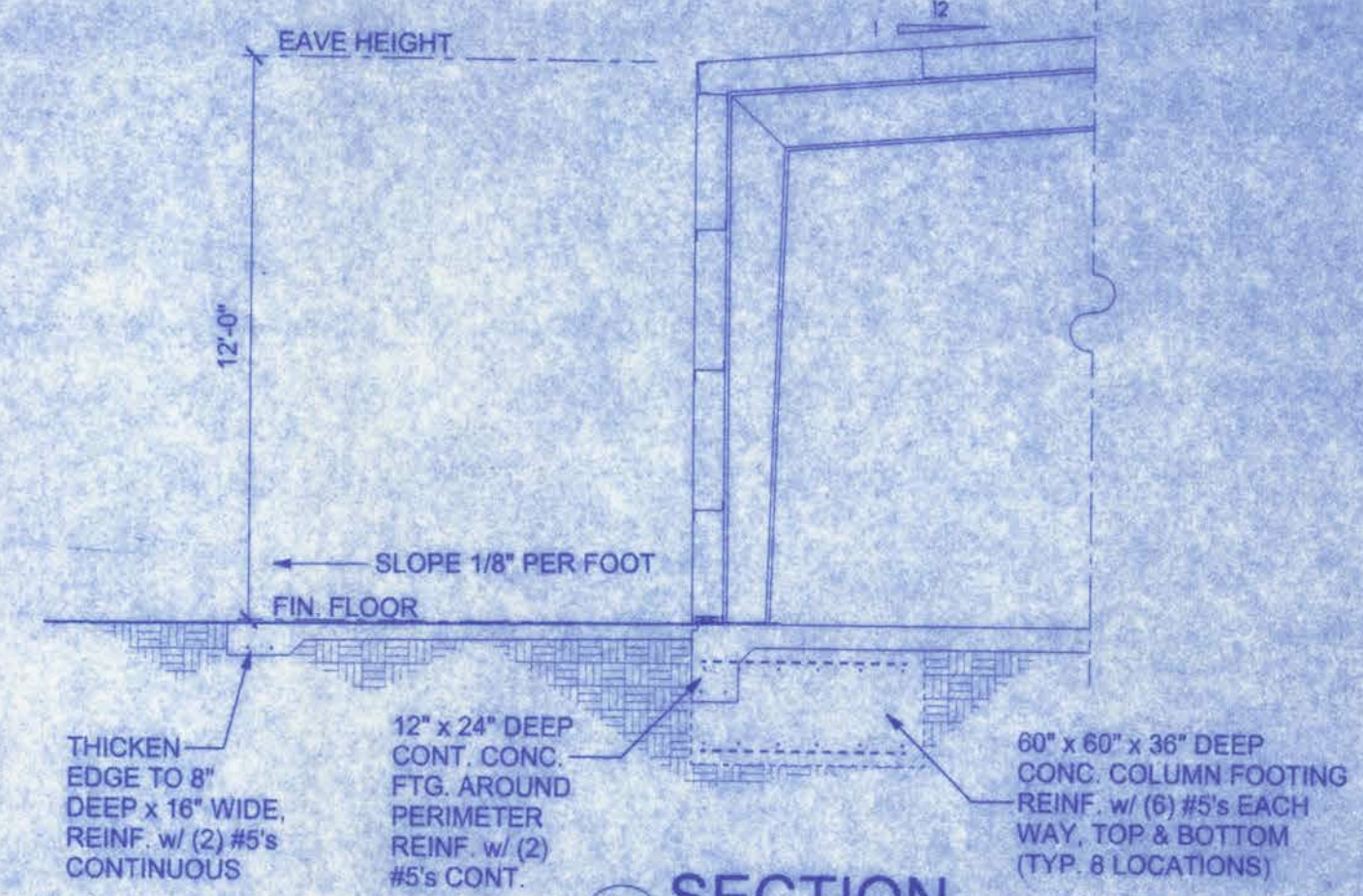
CERTIFICATE OF AUTHORIZATION # 00008701



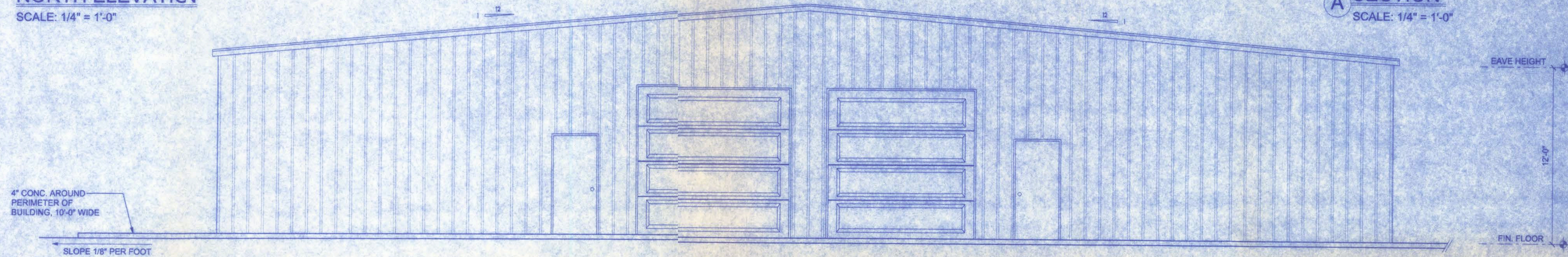
DATE 4/24/08	DRAWN BY J.T.D.
	APPROVED W.H.F.
REVISIONS	
SHEET A-2	
OF 5	
PROJECT NO. 08.C07	



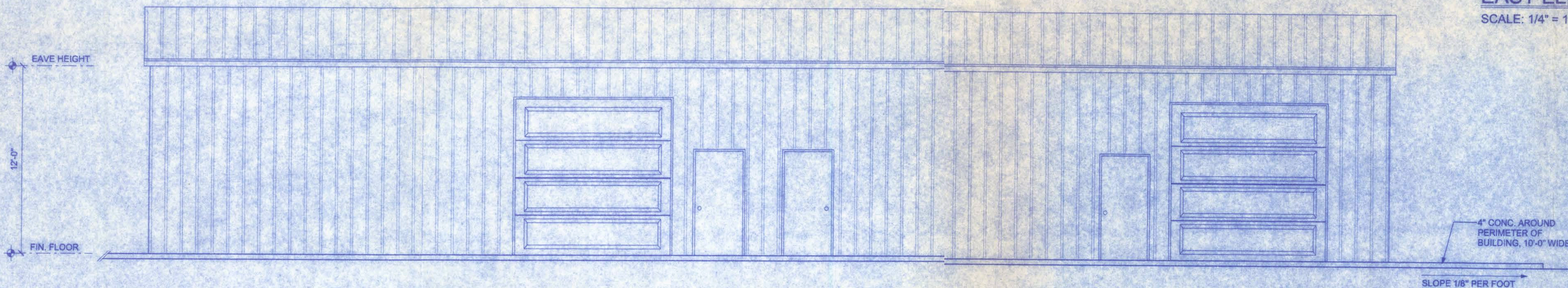
NORTH ELEVATION
SCALE: 1/4" = 1'-0"



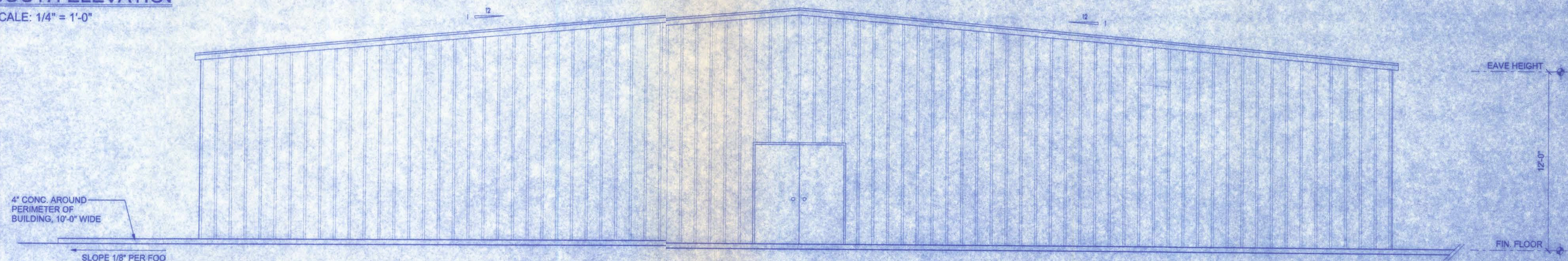
A SECTION
SCALE: 1/4" = 1'-0"



EAST ELEVATION
SCALE: 1/4" = 1'-0"



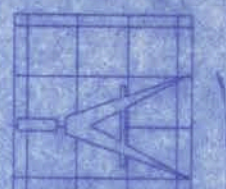
SOUTH ELEVATION
SCALE: 1/4" = 1'-0"



WEST ELEVATION
SCALE: 1/4" = 1'-0"

**CHRIST CENTRAL MINISTRIES
STORAGE BUILDING**

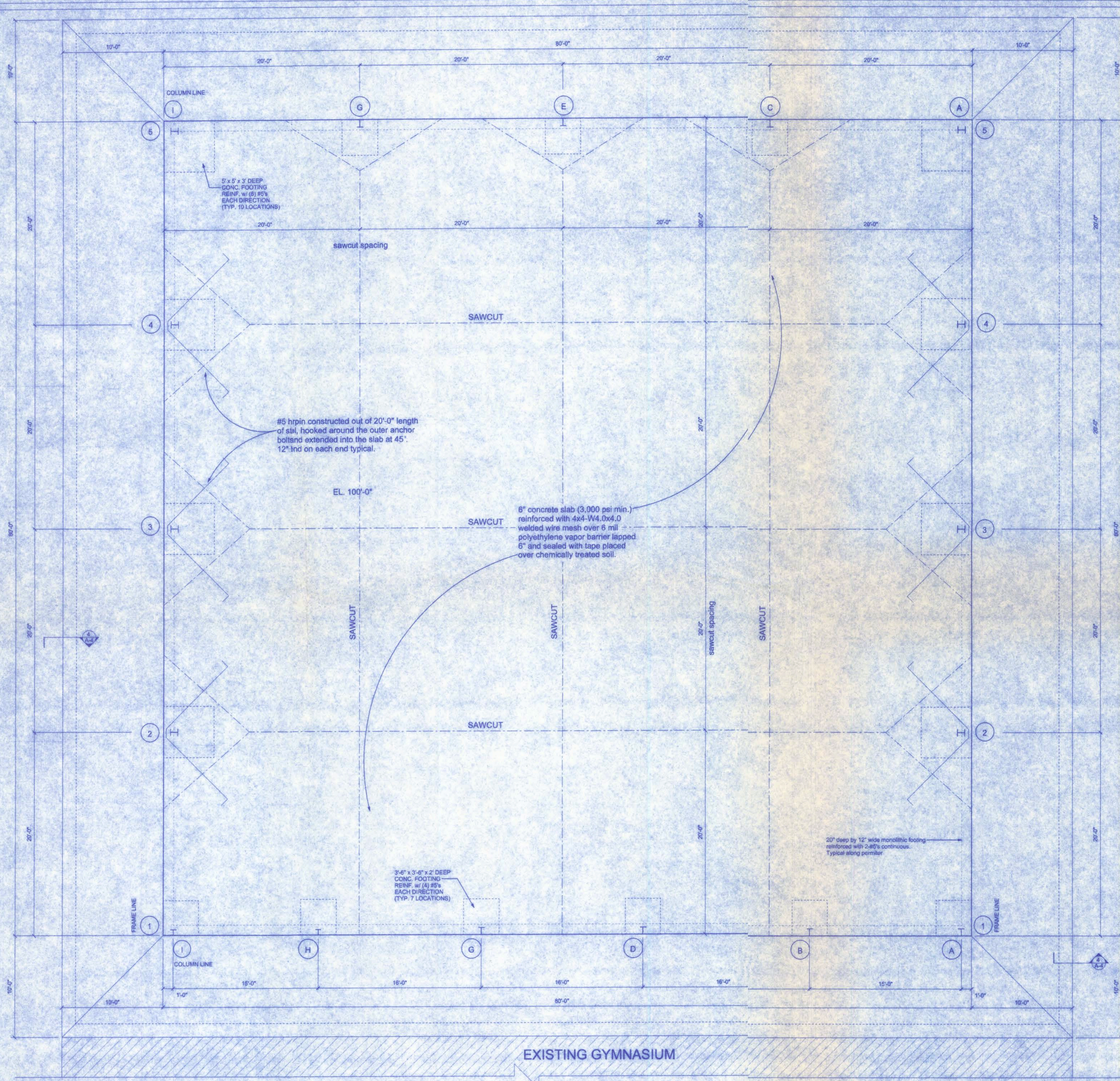
128 SW NASSAU STREET
LAKE CITY, FL 32025
(386) 788-4209



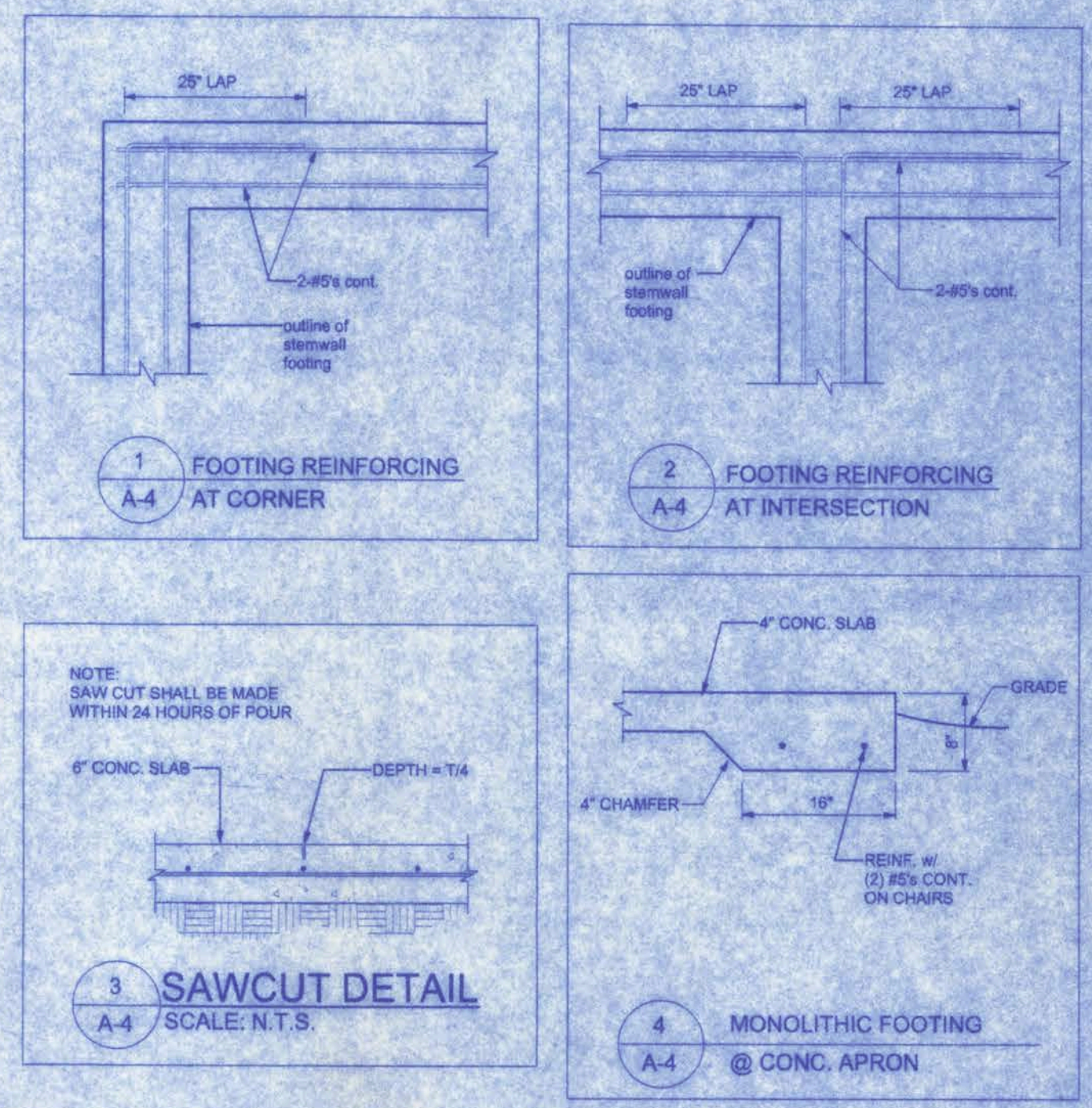
Freeman
Design Group

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	APPROVED W.H.F.
REVISIONS	
SHEET A-3	OF 5
PROJECT NO. 08.C017	

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FOUNDATION PLAN
SCALE: 3/16" = 1'-0"



FOUNDATION NOTES:

- CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.
- GALVANIZATION: METAL ACCESSORIES FOR USE IN EXTERIOR WALL CONSTRUCTION AND NOT DIRECTLY EXPOSED TO THE WEATHER SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 163, CLASS B-2. METAL PLATE CONNECTORS, SCREWS, BOLTS AND NAILS EXPOSED DIRECTLY TO THE WEATHER SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED.
- REINFORCING STEEL: THE REINFORCING STEEL SHALL BE MINIMUM GRADE 60.
- REINFORCEMENT MAY BE BENT IN THE SHOP OR THE FIELD PROVIDED:
1. ALL REINFORCEMENT IS BENT COLD.
 2. THE DIAMETER OF THE BEND MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS AND
 3. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.
- EXCEPTION: WHERE BENDING IS NECESSARY TO ALIGN DOWEL BARS WITH A VERTICAL CELL, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL BE PERMITTED TO BE BENT AT A SLOPE OF NOT MORE THAN 1 INCH OF HORIZONTAL DISPLACEMENT TO 6 INCHES OF VERTICAL BAR LENGTH.
- COVER OVER REINFORCING STEEL: FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE 3 INCHES IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH OR WEATHER AND 1 1/2 INCHES ELSEWHERE. REINFORCING BARS EMBEDDED IN GROUTED CELLS SHALL HAVE A MINIMUM CLEAR DISTANCE OF 1 1/4 INCH FOR FINE GROUT OR 1/2 INCH FOR COARSE GROUT BETWEEN REINFORCING BARS AND ANY FACE OF A CELL. REINFORCING BARS USED IN MASONRY WALLS SHALL HAVE A MASONRY COVER INCLUDING GROUT OF NOT LESS THAN 2 INCHES FOR MASONRY UNITS WITH FACE EXPOSED TO EARTH OR WEATHER 1 1/2 INCHES FOR MASONRY UNITS NOT EXPOSED TO EARTH OR WEATHER.
- BEARING CAPACITY: THE FOOTING IS DESIGNED FOR SOIL WITH AN ALLOWABLE BEARING CAPACITY OF 1,000 PSF. THE FOOTINGS SHALL REST ON UNDISTURBED OR COMPACTED SOIL OF UNIFORM DENSITY AND THICKNESS. AT THE OWNER'S REQUEST, COMPACTED SOILS SHALL BE TESTED TO A MINIMUM OF 95% OF MODIFIED PROCTOR AND COMPACTED IN LIFTS NOT TO EXCEED 12 INCHES.

FOOTING REINFORCING TABLE			
FRAME LINE	COLUMN LINE	FOOTING SIZE	REINFORCING
2, 3, 4, 5	A & I	5'-0" x 3'-0" x 3'-0"	6-#5'S EACH WAY TOP & BOTTOM
1 & 5	A, B, C, D, E, F, G, H, I	3'-6" x 3'-6" x 2'-0"	4-#5'S EACH WAY

NOTE: ALL ANCHOR BOLTS SHALL HAVE 16" MIN. CONCRETE EMBEDMENT.

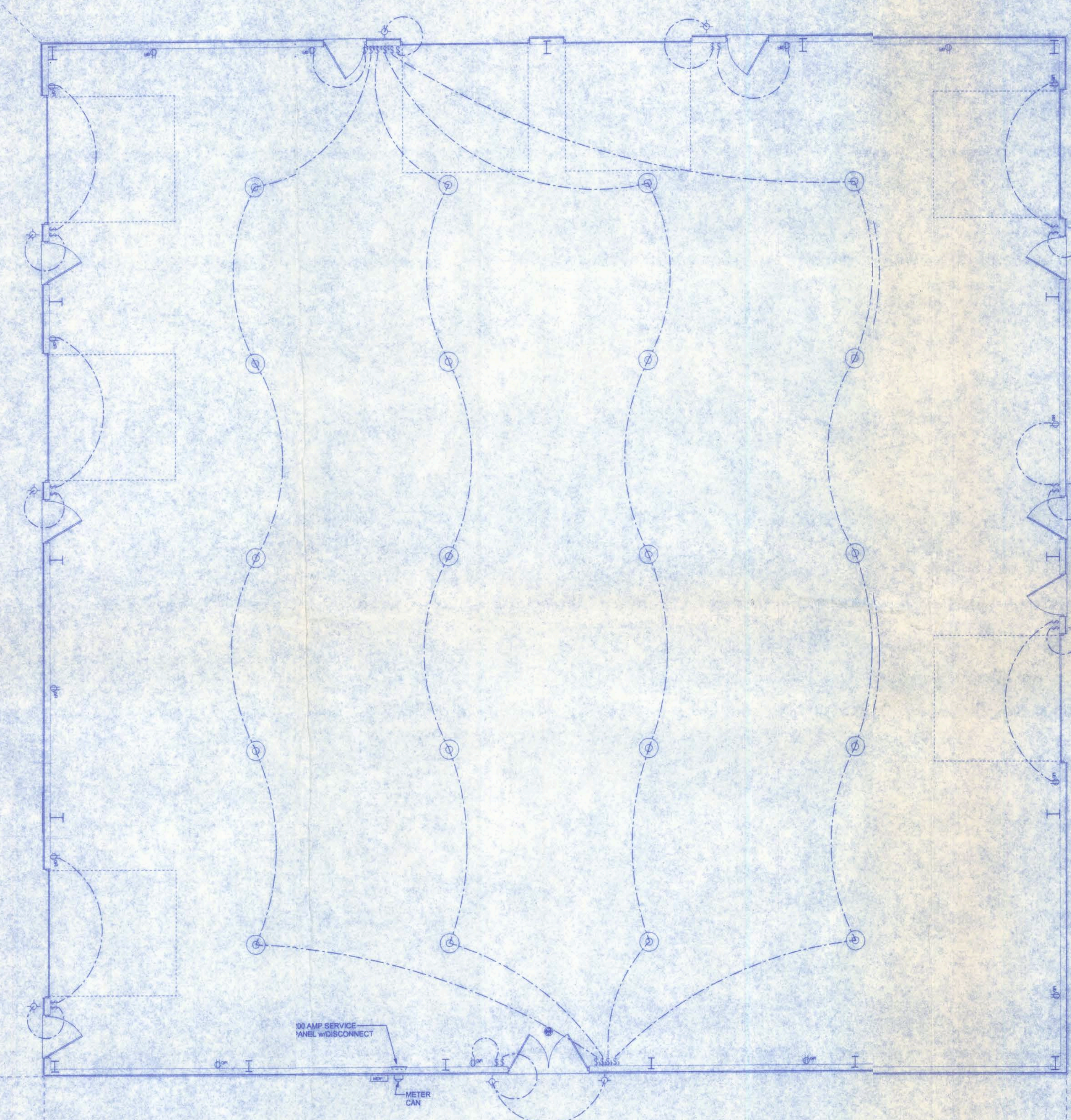
**CHRIST CENTRAL MINISTRIES
STORAGE BUILDING**

128 SW NASSAU STREET
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SHEET 4-4	
OF 5	
PROJECT NO. 08.C017	



FLOOR PLAN
SCALE: 3/16" = 1'-0"

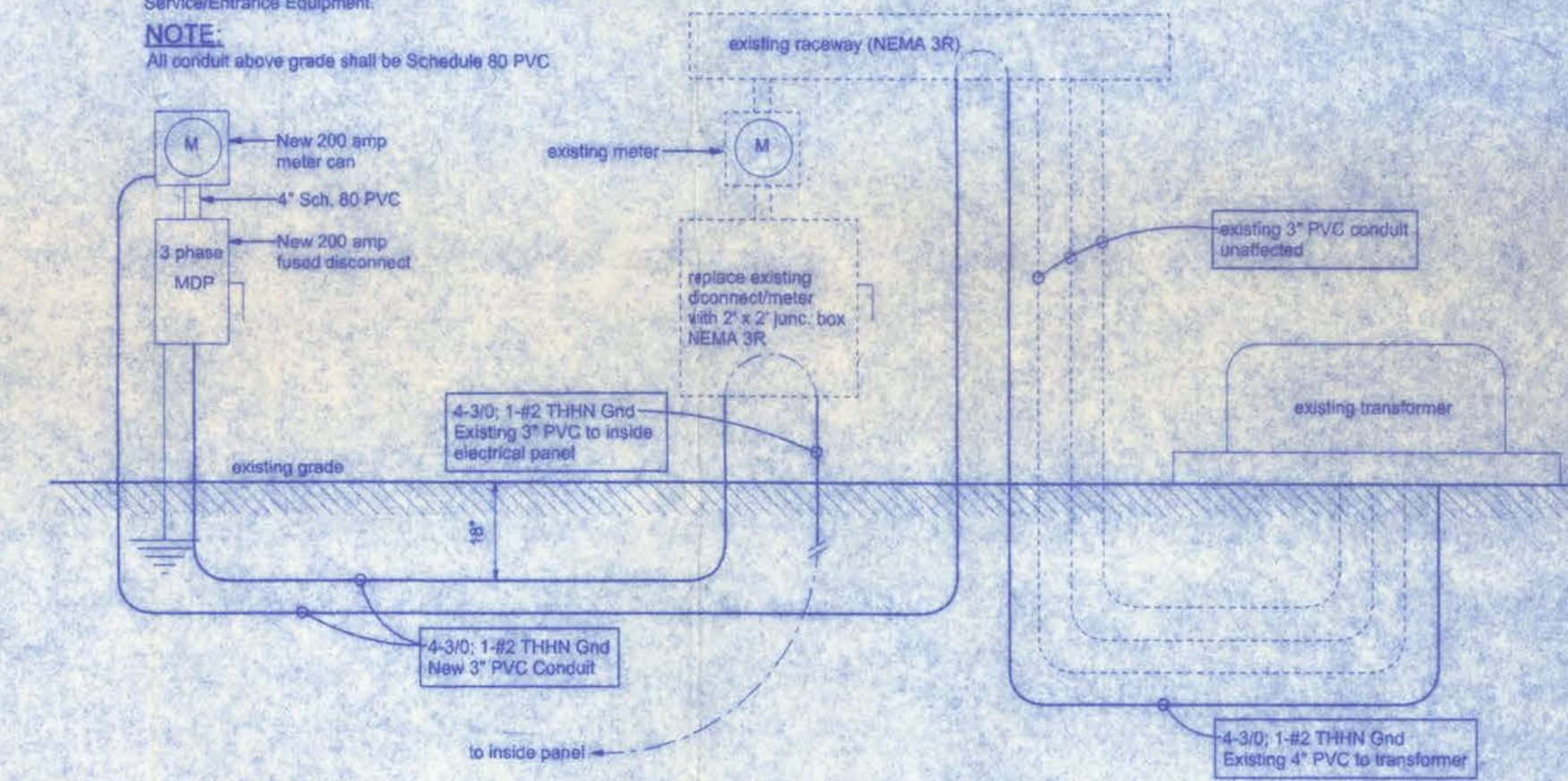
EXISTING GYMNASIUM

PANEL BOARD SCHEDULE

PANEL	NO.	TYPE	LOCATION	DESCRIPTION	WIRING	TERMINALS	WIRING	TERMINALS	WIRING	TERMINALS
1	12	1/2"	LIGHTING	1	20/1	20/1	2			
2	12	1/2"	LIGHTING	3	20/1	20/1	4			
3	12	1/2"	LIGHTING	5	20/1	20/1	6			
4	12	1/2"	RECEPT.	7	20/1	20/1	8			
5	12	1/2"	RECEPT.	9	20/1	20/1	10			
6	12	1/2"	SPACE	11	20/1	20/1	12			
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NOTE:
Service entrance ground: 3/8" diameter iron/steel rod x 6'-6" long and/or concrete encased foundation steel rod x 20'-0" long. Grounding conductor shall be bonded to each piece of Service Entrance Equipment.

NOTE:
All conduit above grade shall be Schedule 80 PVC.



RISER DIAGRAM
NTS

LOAD CALCULATIONS:			
	CONNECTED LOAD	DEMAND FACTOR	DEMAND LOAD
CONTINUOUS:			
GENERAL LIGHTING	6,000 VA	1.25	7,500 VA
EXTERIOR LIGHTING	1,200 VA	1.25	1,500 VA
NON-CONTINUOUS			
RECEPTACLES	3,000 VA	1.0	3,000 VA
TOTAL	10,200 VA		12,000 VA
@ 120/240 1 PHASE			50 AMPS
PROVIDED SERVICE			250 AMPS

Freeman
Design Group, Inc.

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**CHRIST CENTRAL MINISTRIES
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SHEET	A-5		
OF	5		
PROJECT NO.	08.C07		

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