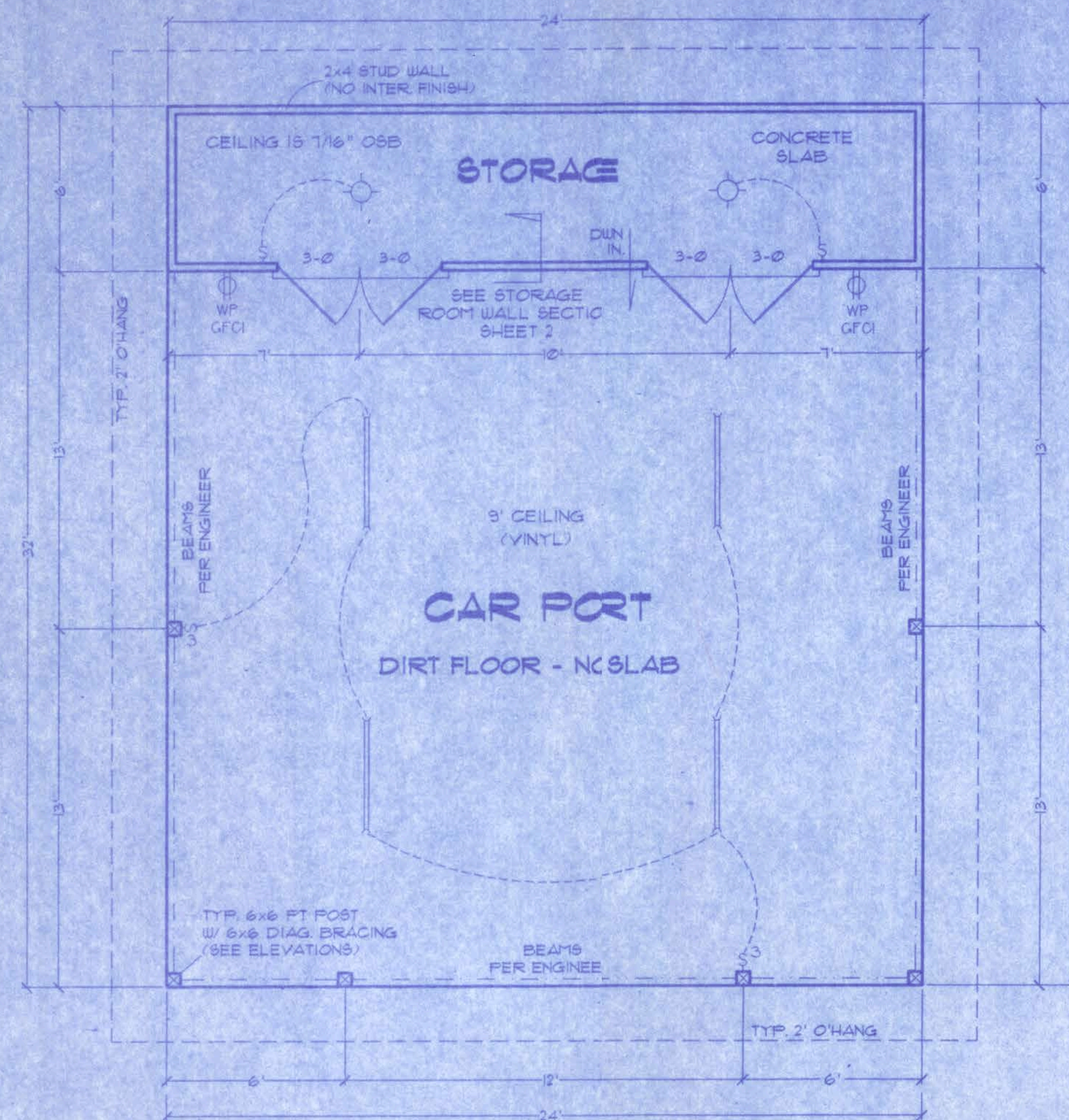
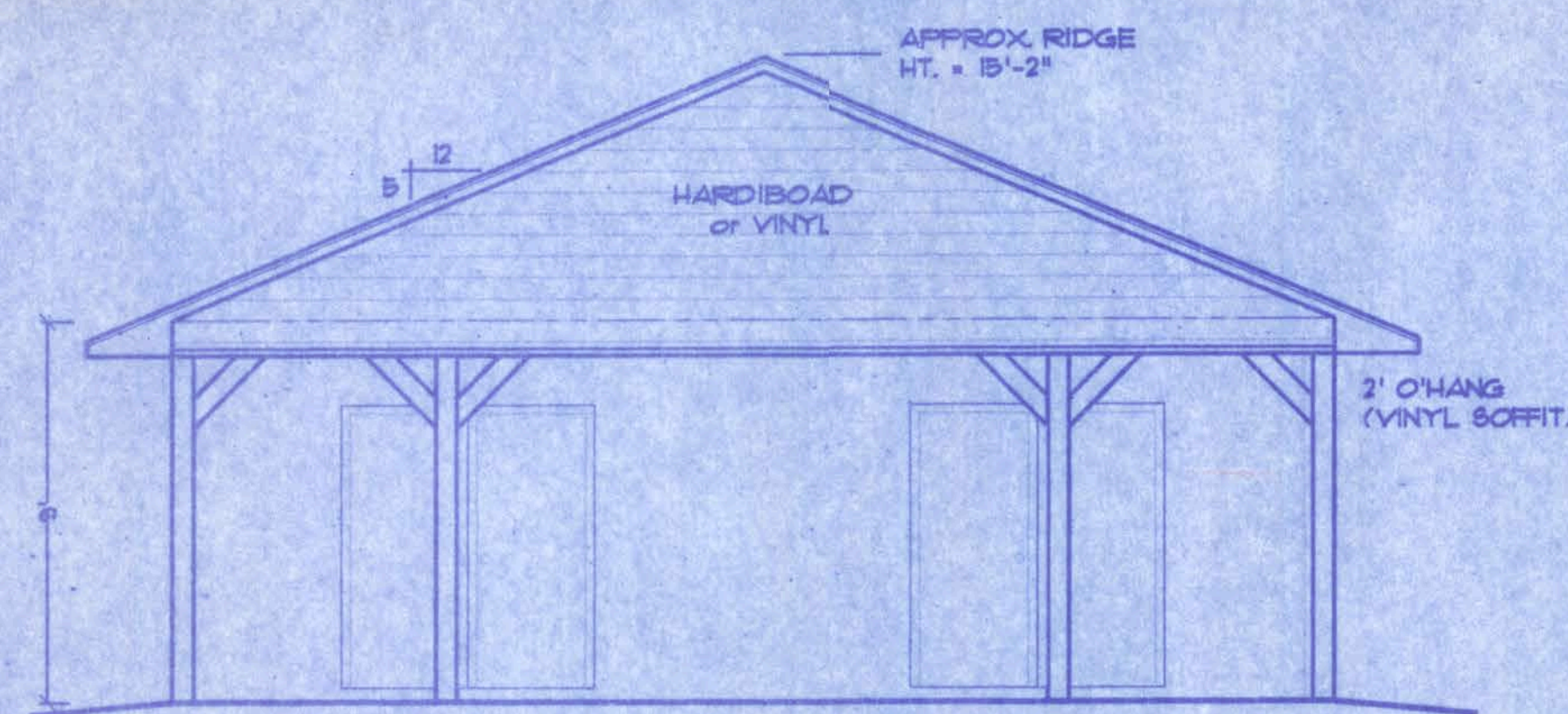


# Crews Carport



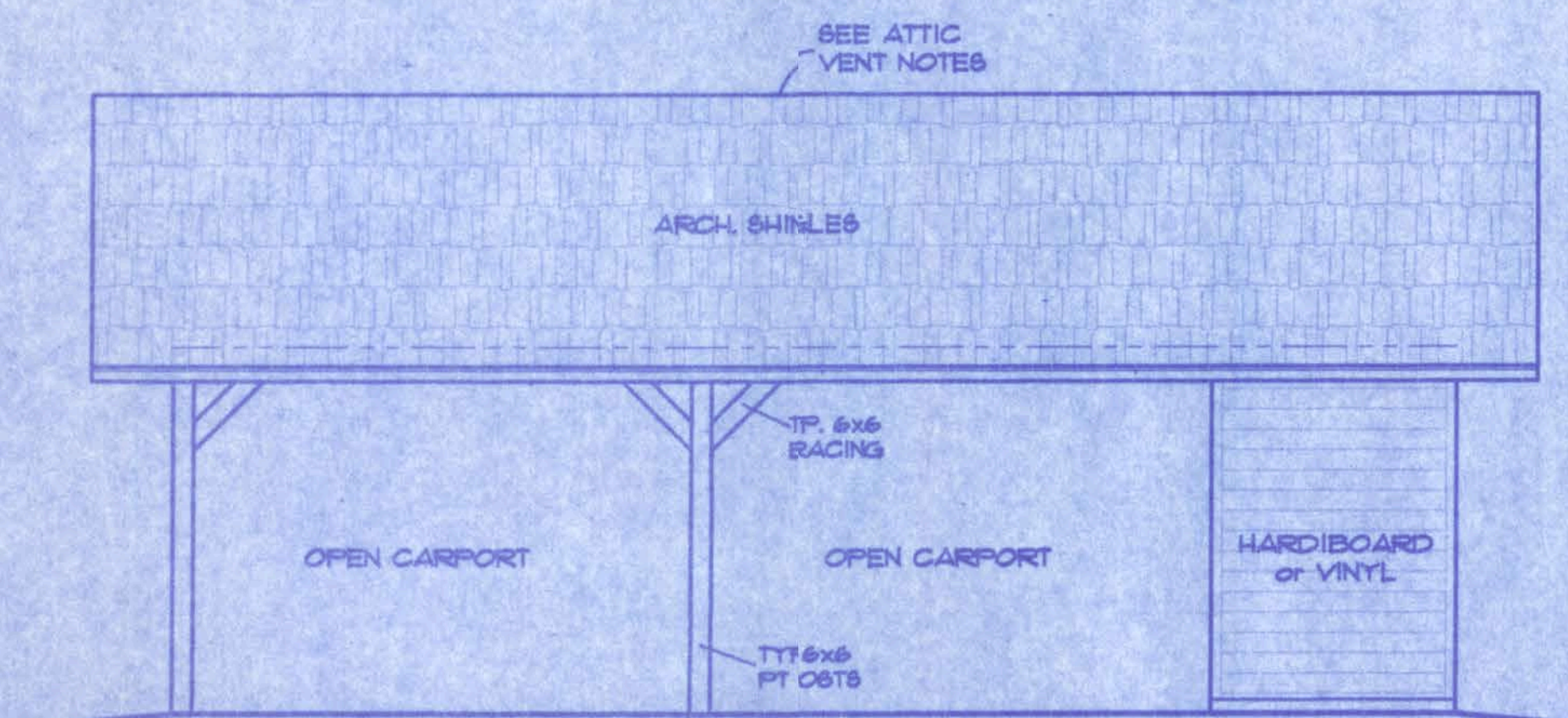
**FLOOR PLAN**

SCALE: 1/4 IN. = 1 FT.  
168 SQ. FEET



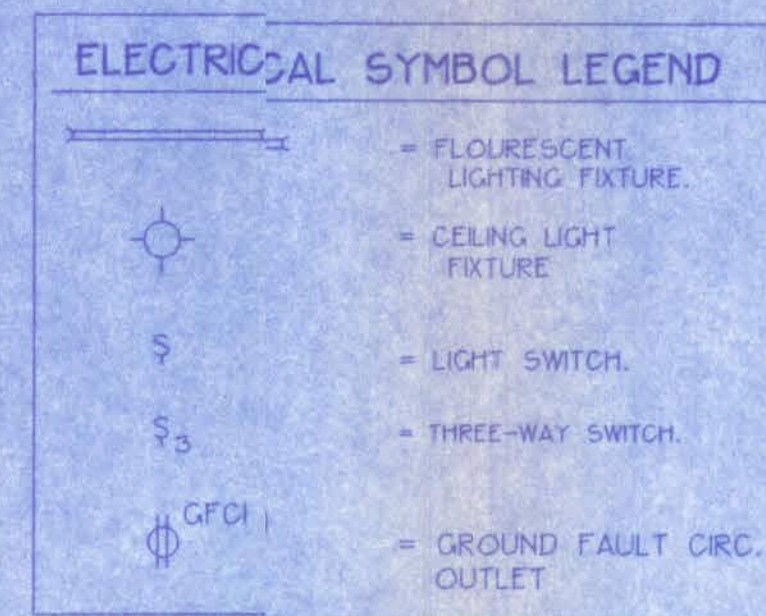
**FRONT ELEVATION**

SCALE: 1/4 IN. = 1 FT.



**RIGHT ELEVATION**

SCALE: 1/4 IN. = 1 FT.



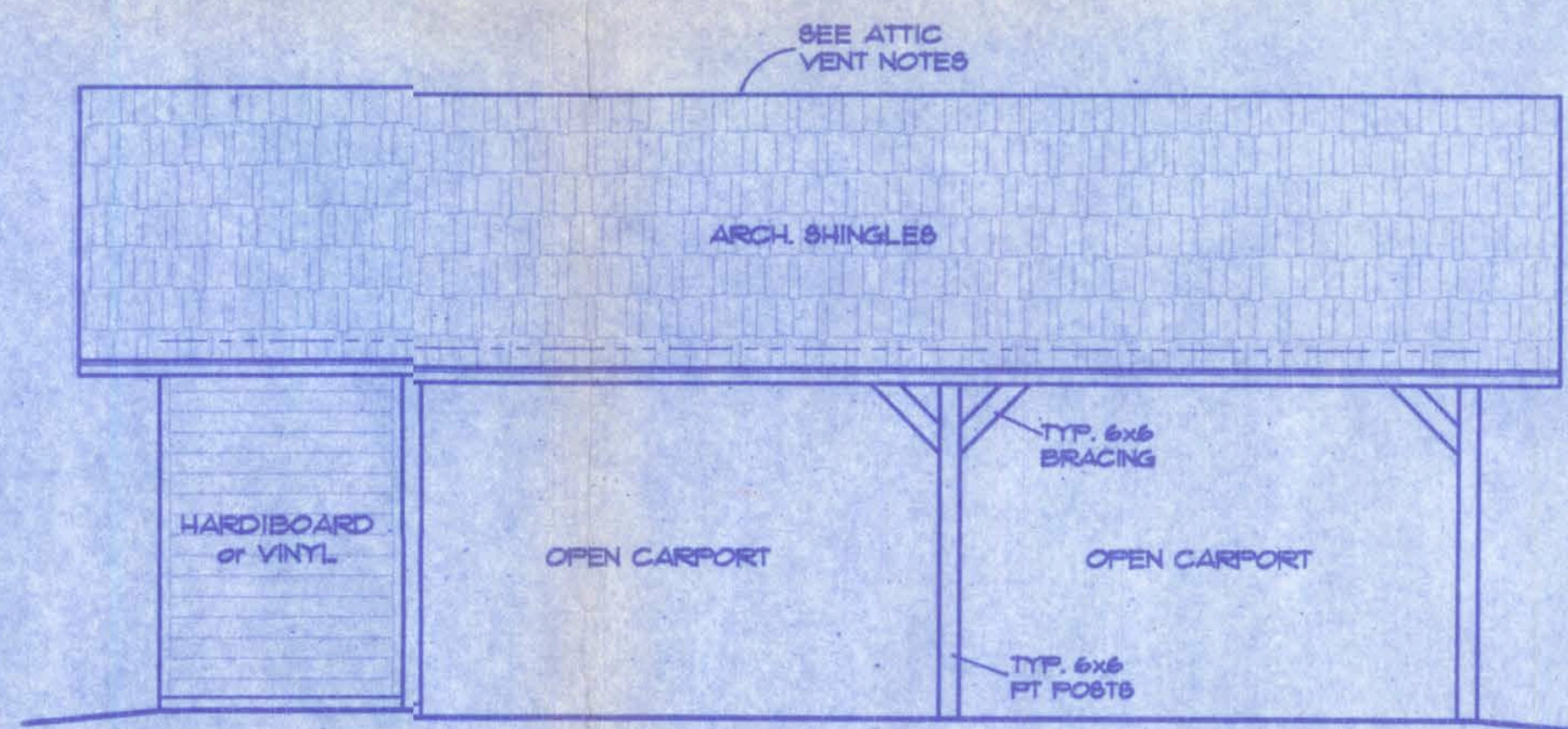
## ELECTRICAL PLAN NOTES

- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- POWER PROVIDED FROM HOUSE SERVICE.

## ATTIC VENTILATION

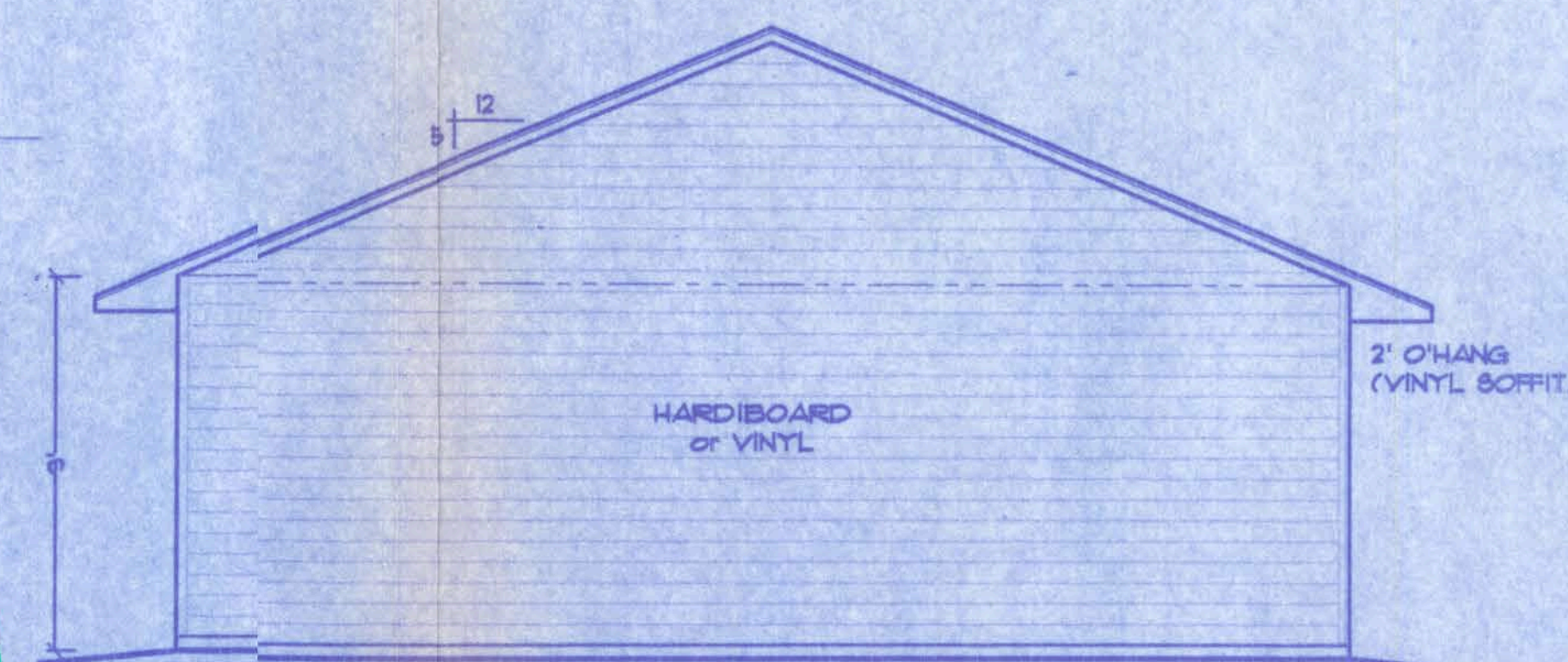
Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.



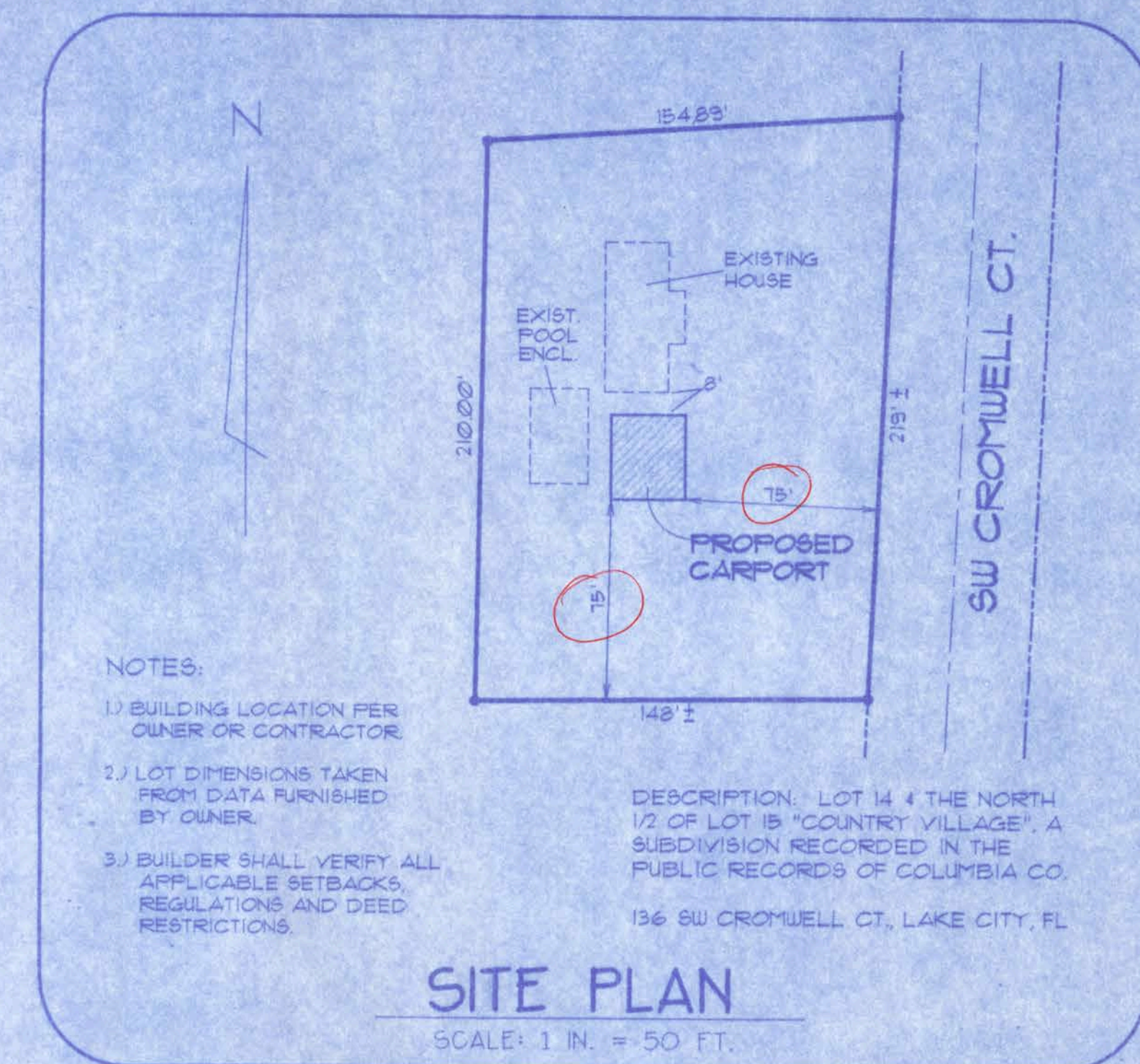
**LEFT ELEVATION**

SCALE: 1/4 IN. = 1 FT.



**REAR ELEVATION**

SCALE: 1/4 IN. = 1 FT.



## GENERAL NOTES

- See Wind Load Detail Sheet S-1 and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- All concrete used to be 2500 PSI strength or greater.
- No Air-conditioning or heating is planned for this project.
- Roof Truss design is the responsibility of the supplier.
- The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girdler locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32156, 386-754-5419

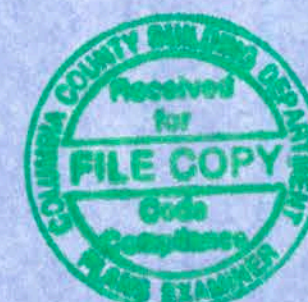
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: \_\_\_\_\_ Job No.: \_\_\_\_\_

FILE: 09-015	<b>CREWS CARPORT</b>	SHEET: 1 OF 1
DATE: 9-6-09		CAD FILE: 09015
DRAWN: T A D	PREPARED BY: TIM DELBENE	REV: 10/1/09
CHECK: T A D	Drafting + Technical Services	
	192 SW Baywood Cir., Lake City, FL 32024	
	Phone: (386) 755-5891	

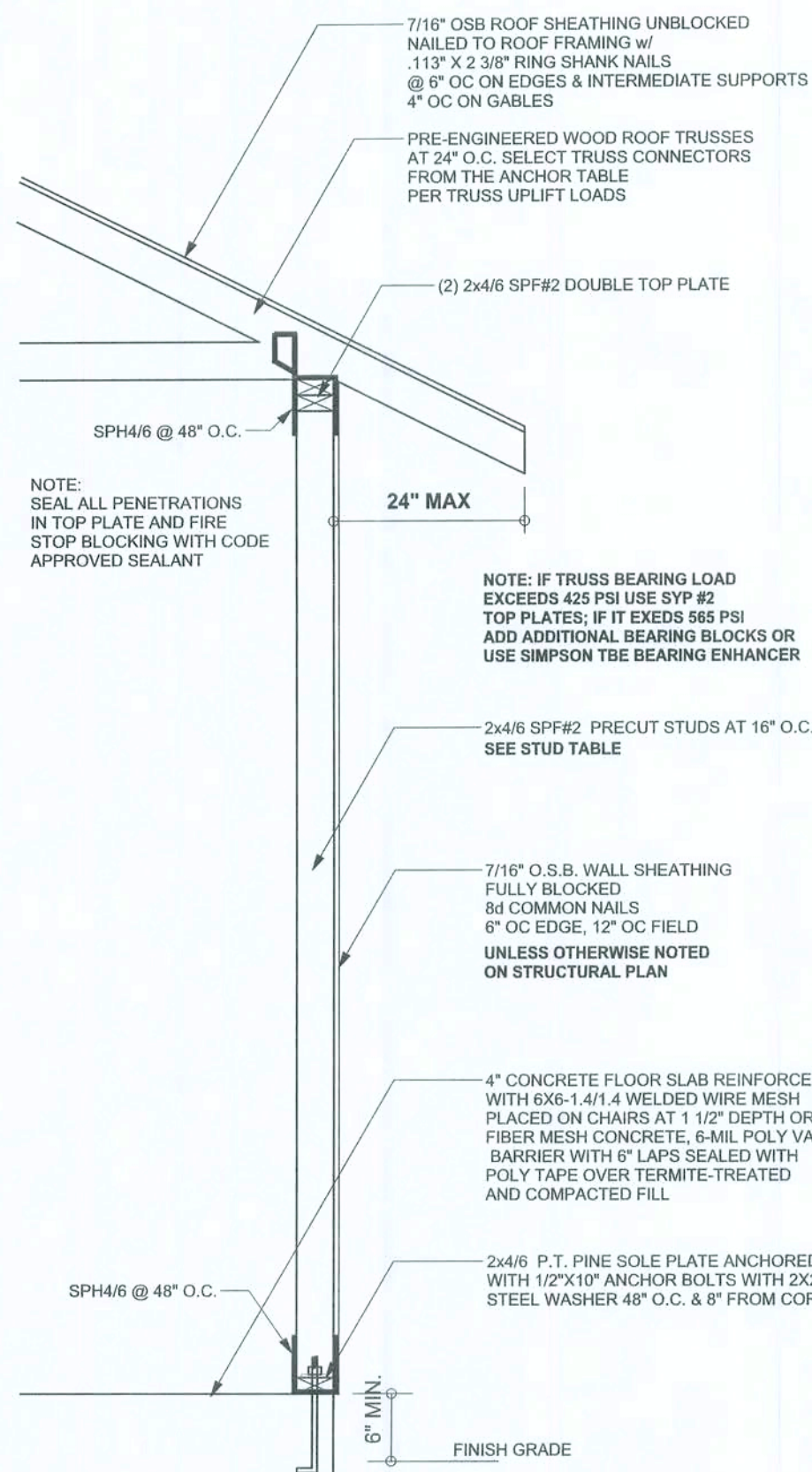
**A-1**





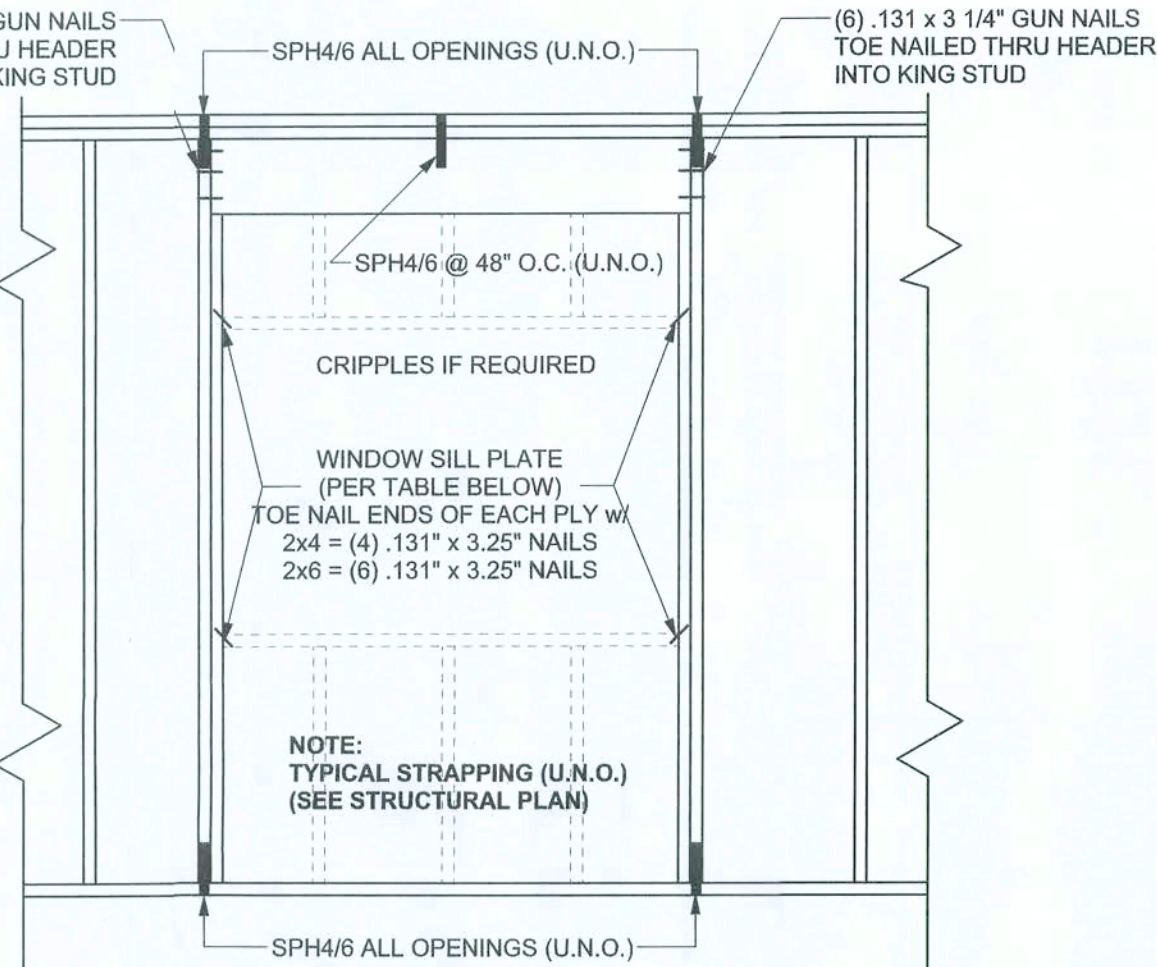
A-2





**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

NOTE:  
IF TRUSS TO ALL STRAPS ARE NAILED  
TO THE HEAD THE SPH4/6 @ 48" O.C.  
ARE NOT REQ'D



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.23B		
900 MPH	5'-3"	7'-9"	7'-8"	11'-4"
1100 MPH	4'-4"	6'-8"	6'-5"	9'-6"
1200 MPH	4'-0"	6'-0"	5'-11"	8'-6"

**TYPICAL HEADER STRAPING DETAIL**  
S.C.E. 1/2" = 1'-0"

## ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS  
MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 1/2 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5A	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H10	980	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12-LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12-MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
<b>HEAVY GIRDER TIEDOWNS</b>								
LGT2	2050	1785	750	170	700	170	14-16d	14-16d
LGT3-SDS2.5	3685	2655	755	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LGT4-SDS3	4060	3660	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MG1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
<b>STUD STRAP CONNECTOR</b>								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	450					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d	6-10d
SPH4	1240	1065					6-10d x 1 1/2"	
SP6	885	760					6-10d x 1 1/2"	
SPH6	1240	1065					6-10d x 1 1/2"	
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					16-10d	
CS30	1030	1030					14-10d	
CS16	1705	1705					22-10d	
<b>STUD ANCHORS</b>								
LT119	1355	1305					1/2" ANCHOR	
LT101	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
H22A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

- (1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES  
(2) FOR SYP GIRDER & SPF STUDS

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.

## GRADE & SPECIES TABLE

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, ALL BEARING LOCATIONS, TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS RECTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 416L EACH END 2X6 RAFTERS 10L EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (AS REQUIRED 1000 PSF BEARING CAPACITY) UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>CD</sub> = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT USE REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

GULLAM BEAMS: GLB, 24F V3SP, F<sub>y</sub> = 24ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT.

FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 8/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

## BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION; IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTION LOCATIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER, ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1				
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXPOSED EXPOSURE AND -10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)				
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION				
1.)	BASIC WIND SPEED = 110 MPH			
2.)	WIND EXPOSURE = C			
3.)	WIND IMPORTANCE FACTOR = 1.0			
4.)	BUILDING CATEGORY = II			
5.)	ROOF ANGLE = 10-45 DEGREES			
6.)	MEAN ROOF HEIGHT = <30 FT			
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)			
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))			

Zone	Effective Wind Area (ft <sup>2</sup> )	10	25.3	-25.3
1	27.8	-30.5	25.3	-25.3
2	27.8	-35.7	25.3	-30.5
2 Onlg		-56.6		-56.6
3	27.8	-35.7	25.3	-30.5
3 Onlg		-95.6		-95.6
4	30.5	-33.0	25.9	-28.5
5	30.5	-40.7	25.9	-31.6
Doors & Windows			30.5	-40.7
Worst Case (Zone 5, 10 ft <sup>2</sup> )				
8x7 Garage Door			27.3	-32.0
16x7 Garage Door			25.9	-29.4

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

## REVISIONS

NO.	DESCRIPTION
1	ISSUED FOR PERMIT

SOFTPLAN  
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER: Mark Discoway,  
P.E. No.53915, POB 868 Lake City, FL  
32066, 386-754-5419

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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Mark Discoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, stored or copied in any form or manner without the express written permission and consent of Mark Discoway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY  
P.E. 5915

Handwritten signature: Mark Discoway

Mack Robinson  
Construction

Crews Carport

ADDRESS:  
Columbia County, Florida

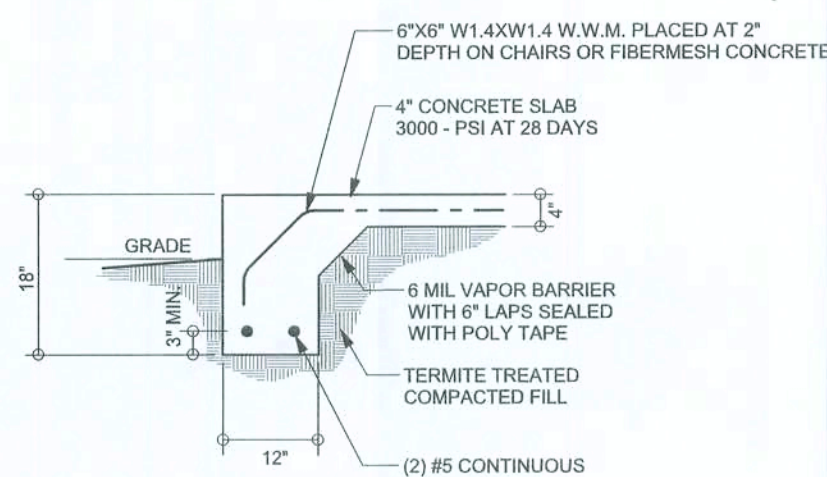
Mark Discoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
September 11, 2009  
DRAWN BY: STRUCTURAL BY:  
David Discoway

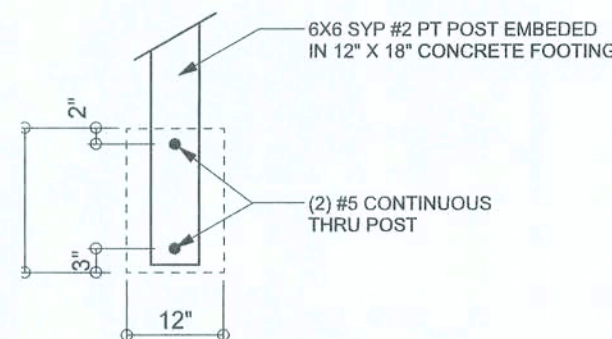
SHEET 3 OF 4  
FINAL DATE:  
11Sep09

JOB NUMBER:  
909091  
DRAWING NUMBER  
S-1  
OF 2 SHEETS

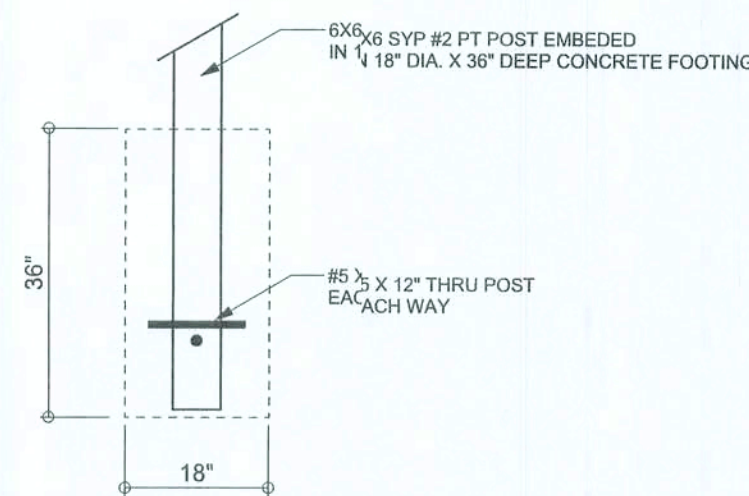




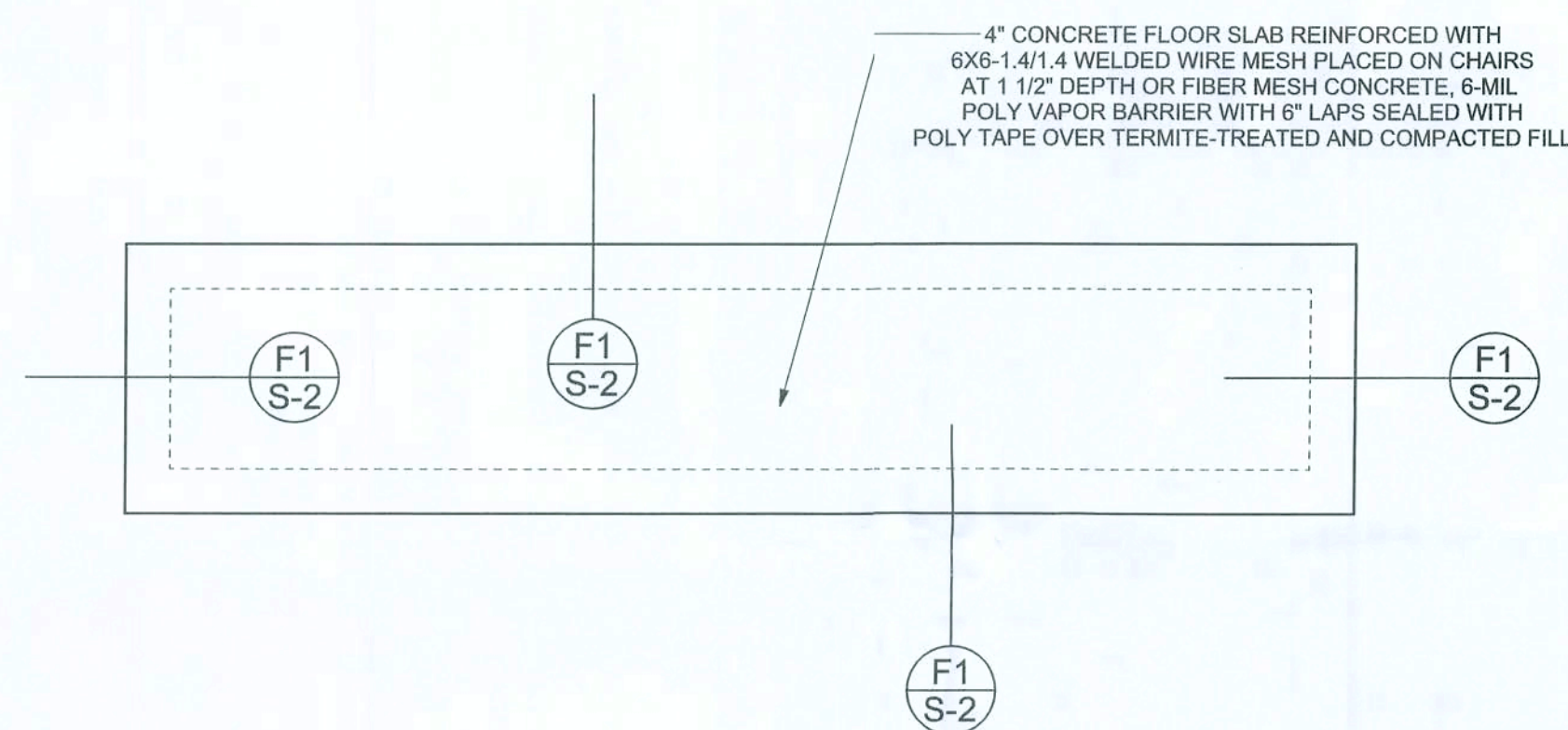
**F1**  
**S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



**F2**  
**S-2** FOOTING @ CARPORT END POSTS  
SCALE: 1/2" = 1'-0"



**F3**  
**S-2** FOOTING @ CARPORT MID POSTS  
SCALE: 1/2" = 1'-0"



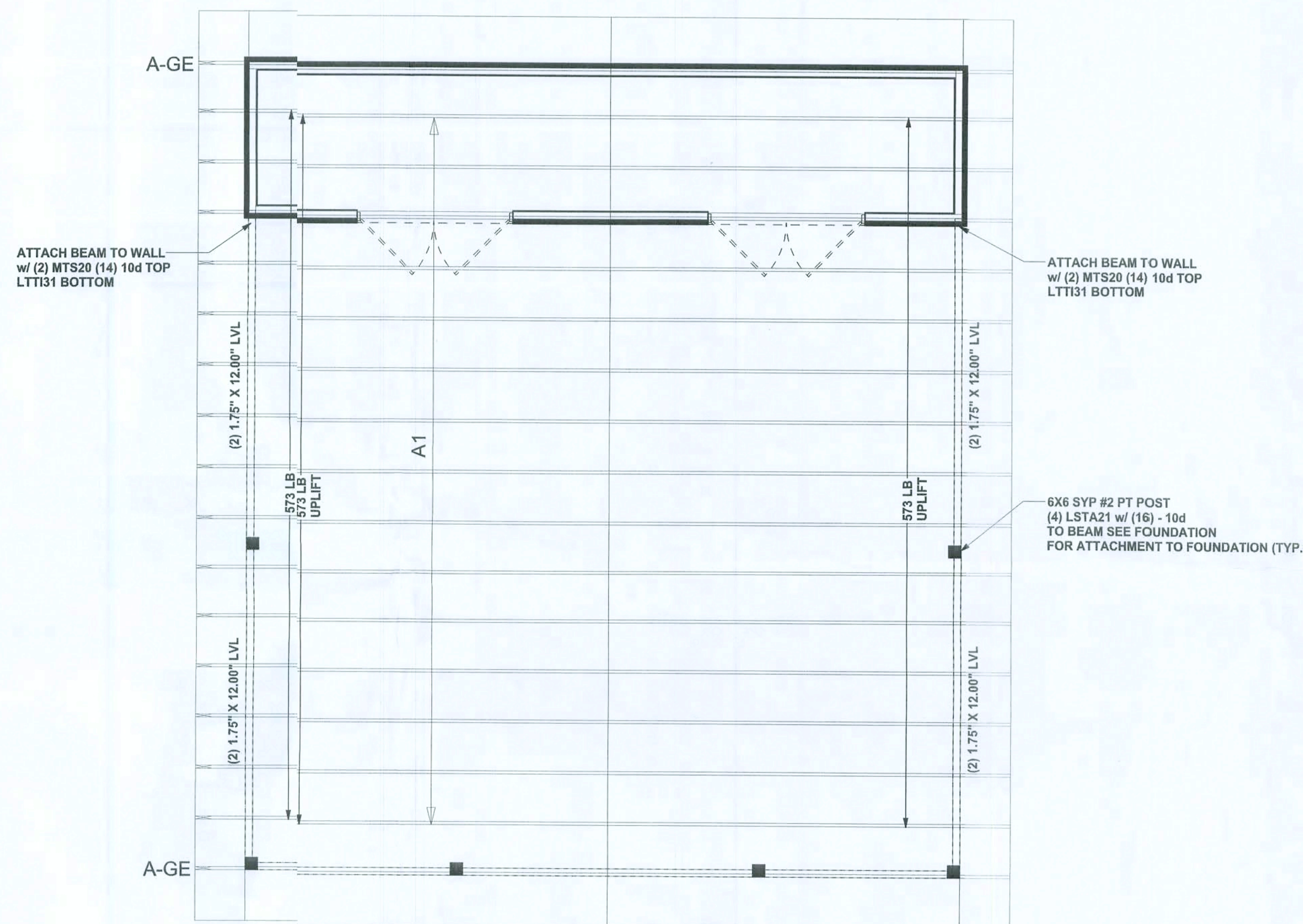
**F3**

**F3**



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

**F2**  
**S-2**



**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

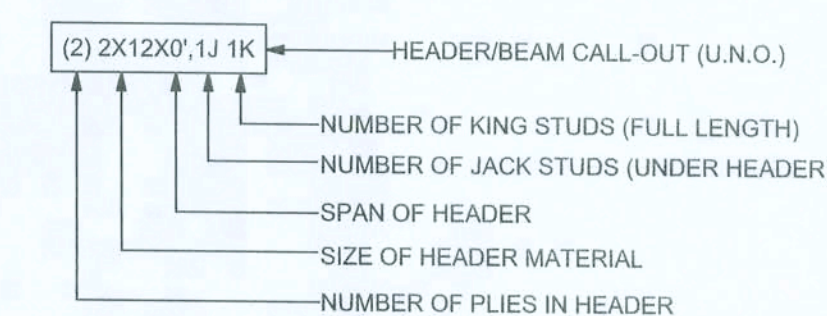
**STRUCTURAL PLAN NOTES**

- SN-1 ALL L LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL L LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**WALL LEGEND**

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

**HEADER LEGEND**



**TOTAL SHEAR WALL SEGMENTS**

	REQUIRED	ACTUAL
TRANSVERSE	20.0'	44.0'
LONGITUDINAL	10.0'	12.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS JOB # 9-181

REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway, PE No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419  
DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer a questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1907.2.1, Florida building code residential 200, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY  
P.E. 53915

*Mark Discoway*  
11SEP09  
SEAL

**Mack Robinson**  
Construction

Crew: Carport

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PRINTED DATE:  
September 11, 2009  
DRAWN BY: STRUCTURAL BY:  
David Discoway

**SHEET 1 OF 4**  
FINALS DATE:  
11Sep09

JOB NUMBER:  
909091  
DRAWING NUMBER  
**S-2**  
OF 2 SHEETS