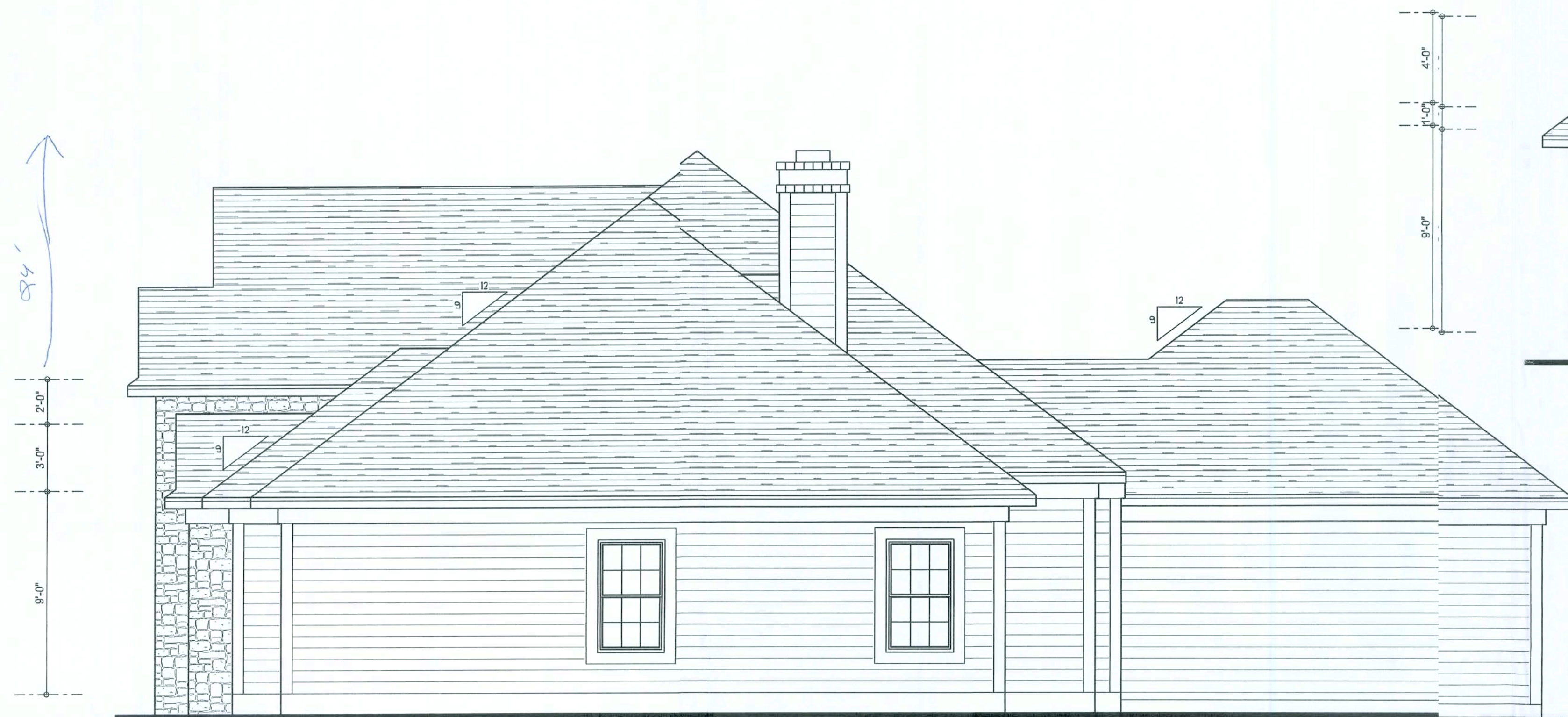


REVISIONS

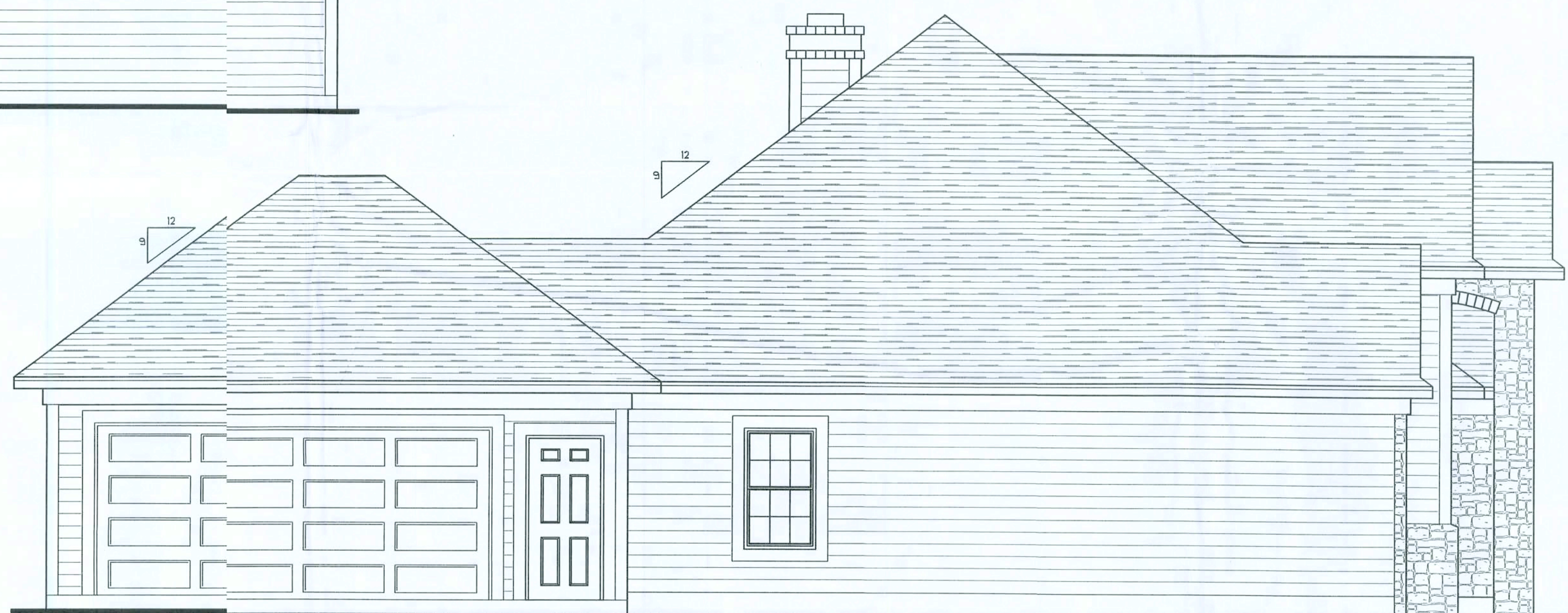
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ARCHITECTURAL DESIGN SOFTWARE



FRONT ELEVATION  
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION  
SCALE: 1/4" = 1'-0"



LEFT ELEVATION  
SCALE: 1/4" = 1'-0"



REAR ELEVATION  
SCALE: 1/4" = 1'-0"



WINDLOAD ENGINEER: Mark Discoway,  
P.E. No. 53915, PCB 66, Lake City, FL  
32056, 386-754-541

**DIMENSIONS:**  
Stated dimensions are approximate scaled  
dimensions. Refer a questions to  
Mark Discoway, P.E. for resolution.  
Do not proceed without clarification.

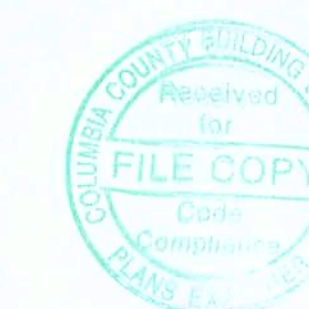
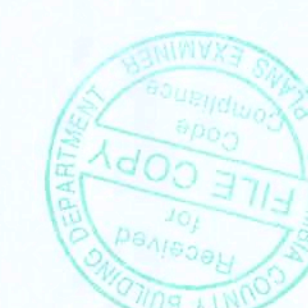
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permission and consent of Mark Discoway.

**CERTIFICATION:** I hereby certify that I have  
examined this plan, and that the applicable  
portions of the plan, relating to wind engineering  
comply with section 1301.2-1, Florida building  
code residential 200, to the best of my  
knowledge.

**LIMITATION:** This design is valid for one  
building, at specified location.

MARK DISCOWAY  
P.E. 53915

*Mark Discoway*  
24 JUL 08  
SEAL



Adam Framing  
and Construction

Adan Papka

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128 S.V. Holly Gln  
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Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
July 24, 2008

DRAWN BY: STRUCTURAL BY:  
Evan Swemley David Discoway

FINALS DATE:  
July 24, 2008

JOB NUMBER:  
802191

DRAWING NUMBER

1  
OF 1 SHEETS

**REQUIRED ROOF VENTILATION:**  
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT  
MIN. 50% TOTAL VENT AREA  
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)  
2524 S.F. / 300 x 50% = 5 S.F. RIDGE VENT AREA REQUIRED  
45 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT  
2524 S.F. / 300 x 50% = 5 S.F. SOFFIT VENT AREA REQUIRED  
167 FEET OF SOFFIT VENT REQUIRED

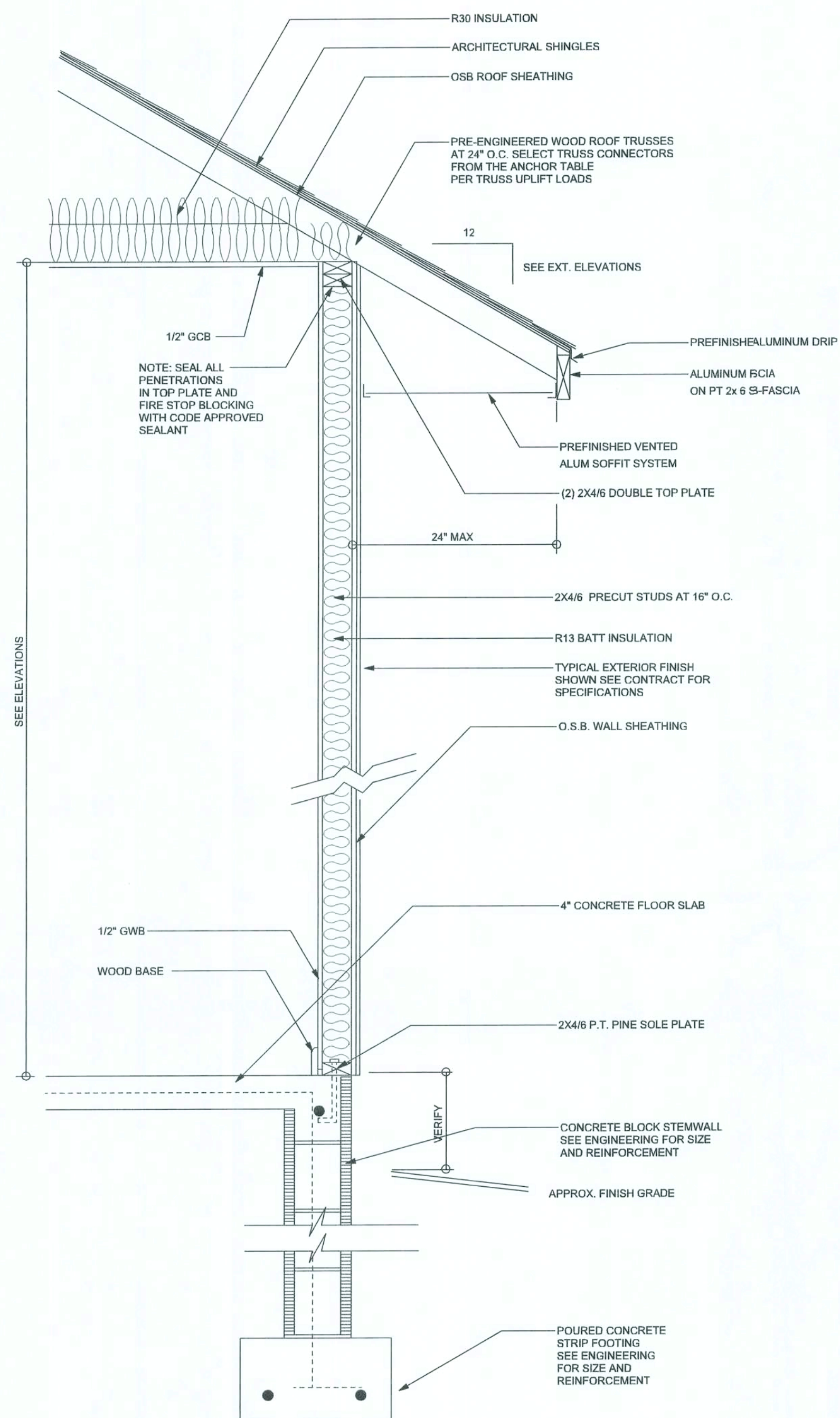
BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (11 FT2/FT)
2. OFF-RIDGE VENTS = 70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (0.3 FT2/FT)



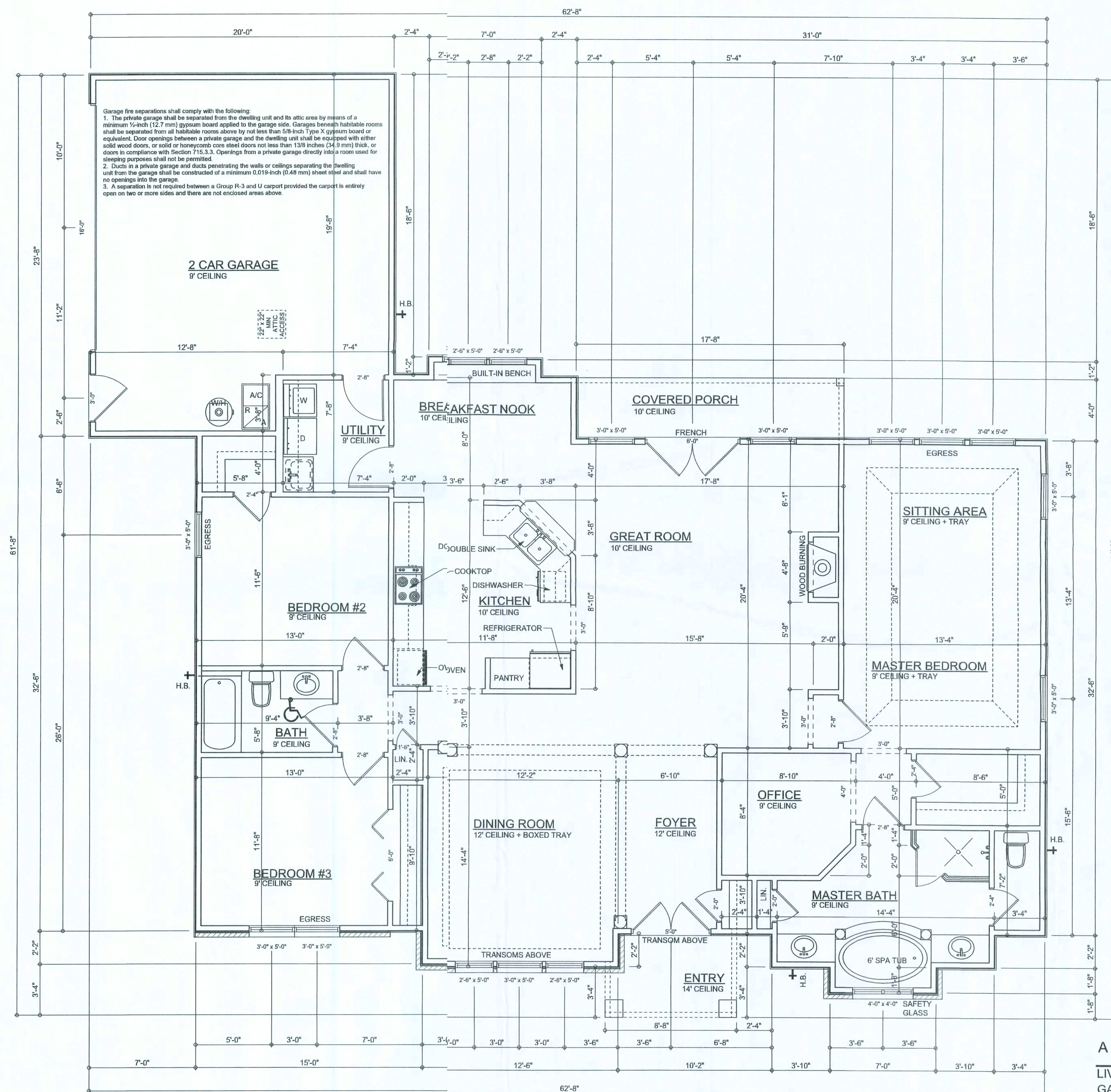
## REVISIONS

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# TYPICAL DESIGN WALL SECTION NON - STRUCTURAL DATA

SCALE: 1\"/>



1ST FLOOR PLAN  
SCALE: 1/4\"/>

## AREA SUMMARY

LIVING AREA	1962	S . F .
GARAGE AREA	446	S . F .
PORCH AREA	116	S . F .
TOTAL AREA	2524	S . F .

WINDLOAD ENGINEER: Mark Disway  
PE No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:  
Stated dimensions are to the center of the member unless otherwise noted. All dimensions are in feet and inches. Dimensions are to the center of the member unless otherwise noted. All dimensions are in feet and inches. Dimensions are to the center of the member unless otherwise noted. All dimensions are in feet and inches.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with section R01.2.1, Florida building code residential 2004 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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Ervin Beamley

STRUCTURAL BY:

David Disway

FINALS DATE:

July 24, 2008

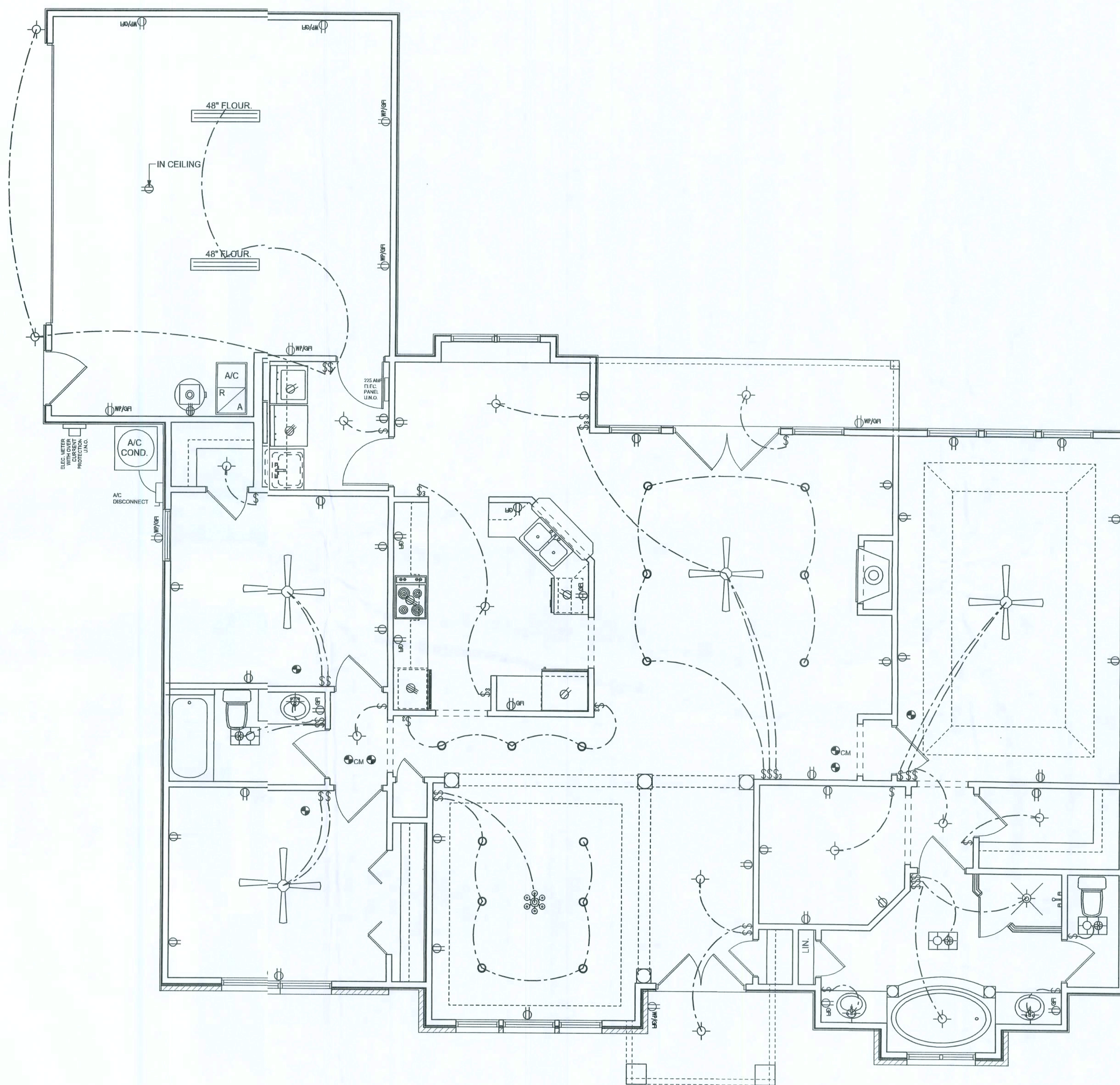
JOB NUMBER:  
802191

DRAWING NUMBER

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OF 63 SHEETS





**ELECTRICAL PLAN NOTES**

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NATL. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUIT.
- E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
- E -11 CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITH 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGSAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM

**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Disoway,  
PE No. 53915, POB 86, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. in resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R01.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
24/08  
SAL

**AdamsFraming  
and Construction**

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David Disoway

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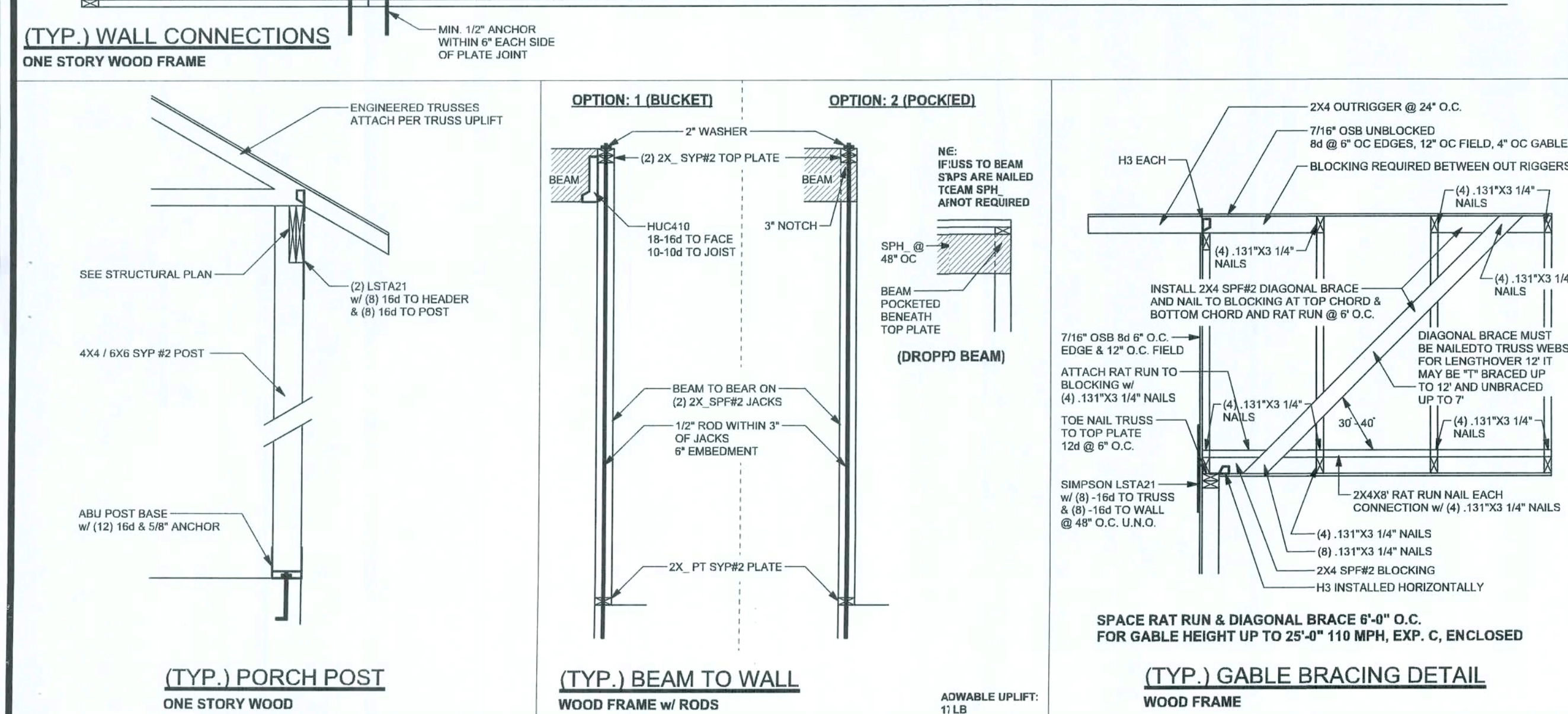
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**DRAWING NUMBER**

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OF 6 SHEETS



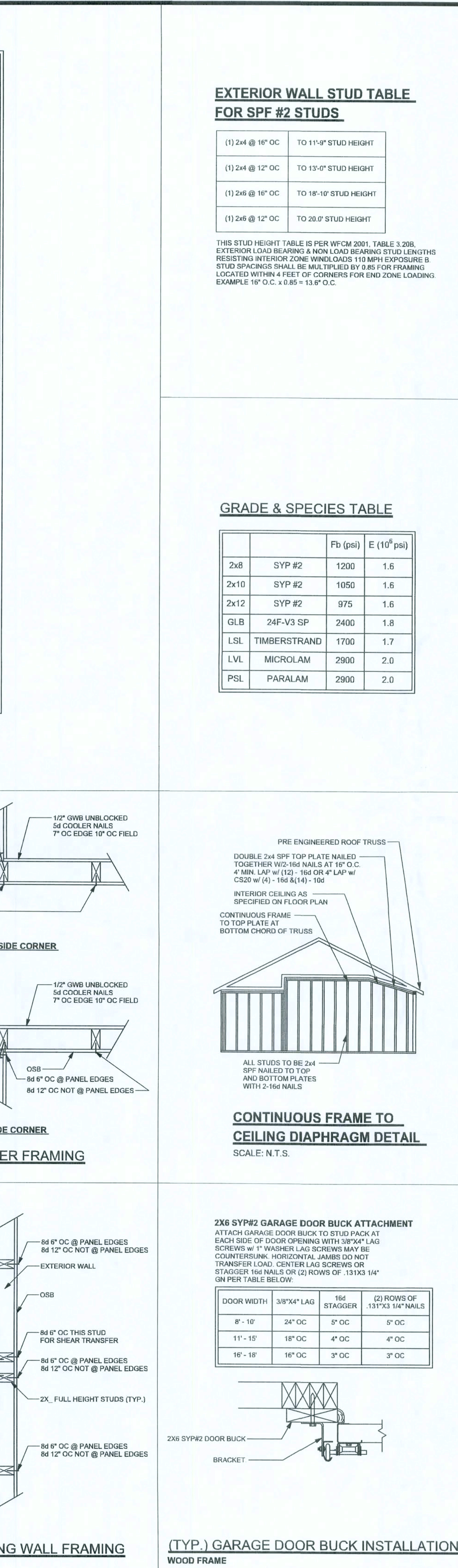
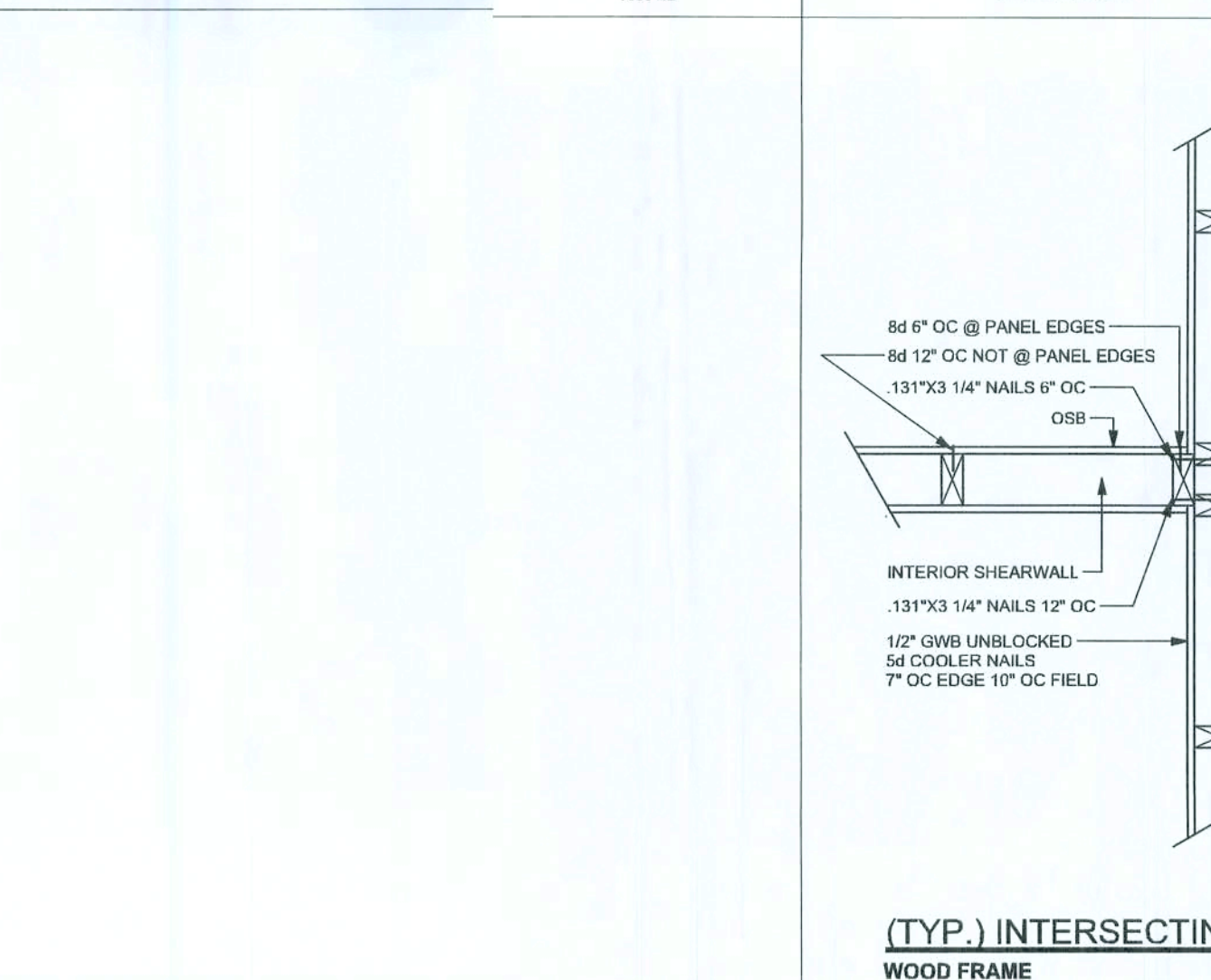
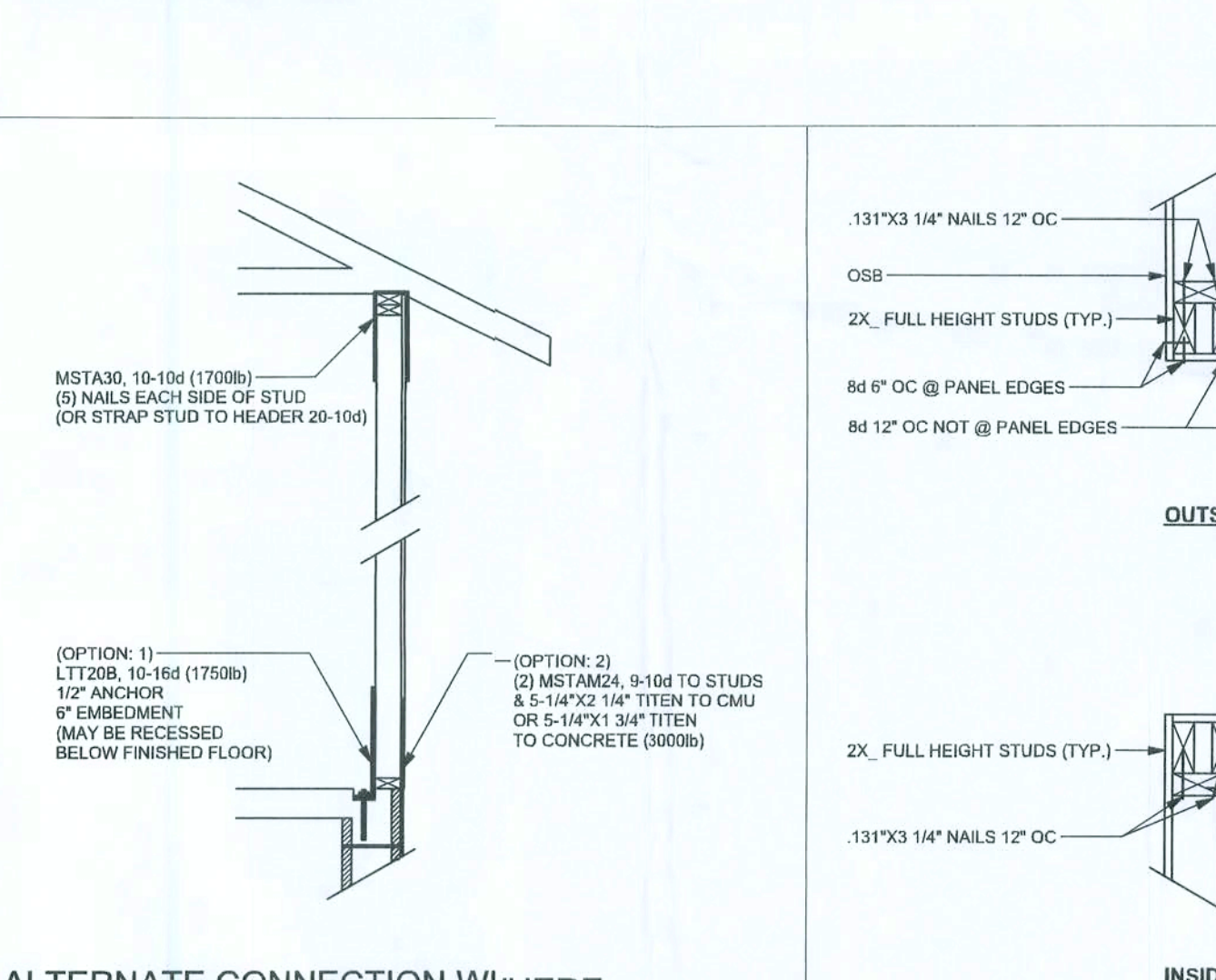
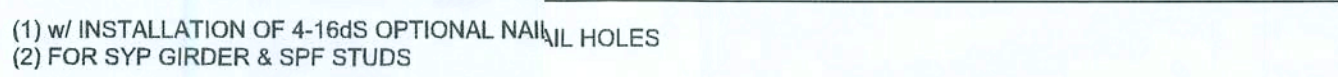


# ANCHOR TABLE

CEMENT UPLIFT REQUIREMENTS FROM TRUSS

MANUFACTURERS ENGINEERING

TRUSS CONNECTOR	UPLIFT SYF	UPLIFT SPF	F1 1 SYF	F2 SYF	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	1 115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	230	1 125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	1 150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	1 110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	620					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	5 615	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	5 615	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	5 585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	740	655	4 455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LT512- LT920	1080	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MT512- MT338	1080	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HT516- HT338	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
<b>HEAVY GIRDER REDDOWNS</b>								<b>TO FOUNDATION</b>
L012	2050	1765	7 700	170	700	170	14-16d	14-16d
LT512-S052.5	3685	2655	7 795	440	795	410	12-SDS 1/4" x 2 1/2"	26-16dS
LT512-S053	4080	3660	7 2000	675	2020	675	12-SDS 1/4" x 3"	36-16dS
M07	3965	3330					22 -10d	5/8" ANCHOR
HT3-2	10560	6485					16 -10d	2-5/8" ANCHOR
HT3-3	10530	9035					16 -10d	2-5/8" ANCHOR
HT3-4	9250	9250					16 -10d	2-5/8" ANCHOR
<b>STUD STRAP CONNECTOR</b>								<b>TO STUDS</b>
DSP DOUBLE TOP PLATE	435	435					3-10d	4 -10d
DSP SINGLE SILL PLATE	455	420					1-10d	4 -10d
DSP DOUBLE TOP PLATE	825	825					6 -10d	8 -10d
DSP SINGLE SILL PLATE	825	600					2 -10d	8 -10d
SP1	565	535					4 -10d	6 -10d
SP2	1005	605					6 -10d	6 -10d
SP4	865	760						6-10d x 1 1/2"
SP14	1240	1065						10-10d x 1 1/2"
SP6	865	760						6-10d x 1 1/2"
SP16	1240	1065						10-10d x 1 1/2"
L5TA18	1235	1110						14-10d
L5TA21	1235	1235						16-10d
C510	1030	1030						14-10d
CS26	1700	1705						16-10d
								22-10d
<b>STUD ANCHORS</b>								<b>TO FOUNDATION</b>
LT719	1350	1305					8-16d	1/2" ANCHOR
LT7151	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
H23A	2775	2570					2-5/8" 18-16d	5/8" ANCHOR
HT116	4175	3605					18-16d	5/8" ANCHOR
HT722	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR



## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2018 2024 TRUSS ENGINEERING SPECIFICATIONS. INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO COLUMN CONNECTIONS, JOINTS, JOINT BRACING, JOINT LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURERS REGISTERED PROFESSIONAL ENGINEER. THE TRUSS ENGINEER SHALL BE FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS TO BE USED. THE TRUSS ENGINEER SHALL PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING FOR EXTERIOR BEARING WALLS. PROVIDE TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6X8 PERPENDICULAR TO MAIN BEAM & 2X6X8 PERPENDICULAR TO 2X6X8 TO EACH END.

SITE PREPARATION: SITE ANALYSIS AND PARAPET IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET SEWAGE LOAD REQUIREMENTS. FOUNDATION SHALL BE DESIGNED FOR A MINIMUM VOLUME OBSERVATION OR SOILS TEST PROVES OTHERWISE.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f_c$  = 3000 PSI

WELDED WIRE REINFORCED SLAB:  $p \times p$  of W14 x 41,  $f_y$  = 60KSI, WELDED WIRE REINFORCEMENT BARS (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDSPAN. THE SLAB SHALL BE CONCRETE WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7".

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURERS RECOMMENDATIONS. CONFORM TO THE SLAB DESIGN TO PROVIDE ASTM C1119 FIBER CONCRETE TO MEET CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: JOINTS WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB ON GRADE SHALL BE PLACED IN THE MIDDLE OF THE SPAN. CONTROL JOINTS BE CUT WITH 12-HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF CONTROL JOINTS SHALL NOT EXCEED 1.0. TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER PLACEMENT OF CONTROL JOINTS. CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENFORCE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A615, GRADE 60, DEFORMED BARS,  $f_y$  = 60 KSI, ALL LAP SPICES 40" DO NOT EXCEED 12" IN ANY SPAN. REBAR SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 318-98, U.N.O.

GULLIM BARS: GULL 248-VYF, 2" x 24kt,  $f_y$  = 18000psi, UNO, SUPPLIER MAY SUPPLY, Y

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS: 7/16" OSB SHEATHING UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED. FASTENED WITH 8d COMMON NAILS (131), 6°C PANEL EDGES, 12°C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4°C. LINO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CUBE.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 7" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 7" x 2" x 9/64"; WITH 3/4" BOLTS TO BE 7" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 7" x 3" x 5/16"; LINO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL, STRUCTURAL VALUES.

## **BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND ELEVATION, HEIGHT, WIND SPEEDS AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBQR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

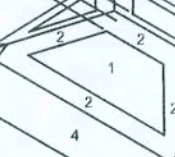
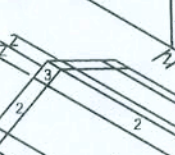
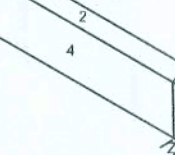
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

## ROOF SYSTEM DESIGN

R301.2.1 IS BASED ON THE TRUSS MANUFACTURER'S COMPLIANCE WITH FIBC 2004, SECTION 6.01. THE TRUSS DESIGNER'S RESPONSIBILITY FOR THE LAYOUT OF THE TRUSS SYSTEM IS BASED ON THE TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER TO COMPLETE THE ROOF SYSTEM DESIGN SUBMITTAL TO THE WIND LOAD ENGINEER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR THE WIND LOAD ENGINEER REVIEW UNDER R301.2.2. THE TRUSS DESIGNER SHALL BE RESPONSIBLE FOR THE TRUSS MEMBER SIZES, JOINTS, BRACING AND ALL SPECIAL LOADS. THE TRUSS DESIGNER SHALL REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE. THE TRUSS DESIGNER SHALL BE RESPONSIBLE TO PROVIDE THE TRUSS LAYOUT AND BRACING. THE TRUSS BRACING THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT AND BRACING. THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

## DESIGN DATA


<p><b>WIND LOADS PER FLORIDA BUILDING CODE 2004 (REVISIONS R310.1.1)</b></p> <p>ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;  MEAN ROOF HEIGHT NOT EXCEEDING EAST-HORIZONTAL DIMENSION OR 60 FT.; NOT  OR UPPER HALF OF HILL OR ESCARPMENT (SEE EXP. B, 30 FT. IN EXP. C) AND <math>\geq 10\%</math>  SURGE AND UNOBSTRUCTED WINDWAY FOR 50% HEIGHT OF 1 MILE (WHICHEVER IS LESS).  BUILDING IS NOT IN THE HIGH-VELOCITY HURRICANE REGION</p> <p>BUILDING IS NOT IN THE WIND-BORNE DEBRIS ZONE</p> <ol style="list-style-type: none"> <li>1. BASIC WIND SPEED = 110 MPH</li> <li>2. WIND EXPOSURE = B</li> <li>3. WIND IMPORTANCE FACTOR = 1.0</li> <li>4. BUILDING CATEGORY = II</li> <li>5. ROOF ANGLE = 10-45 DEGREES</li> <li>6. MEAN ROOF HEIGHT = &lt;30 FT</li> <li>7. INTERNAL PRESSURE COEFFICIENT = NA (ENCLOSED BUILDING)</li> <li>8. COMPONENTS AND CLADDING DESIGN WIND PRESSURES, <math>C_{d,F}</math> &amp; <math>p_{n1}</math> (301)</li> </ol>
---

	Zone	Effective Wind Area (ft <sup>2</sup> )	
		10	100
	1	19.9	-21.8
	2	19.9	-25.5
	2 Other		-40.8
	3 Other		-21.8
	4	21.8	-23.6
	5	21.8	-29.1

Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft)		
6x7' Garage Door	19.5	-22.9
16x7' Garage Door	18.5	-21.0

<b>REVISIONS</b>			



**SOFTMAN**  
ARCHITECTURAL, LLC (DBA) "SOFT"

**WINDLOAD ENGINEER:**  
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386/754-5419

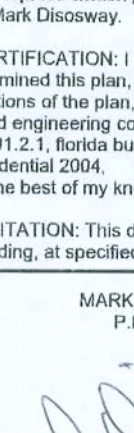
**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
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**PRINTED/DATE:**  
July 24, 2008

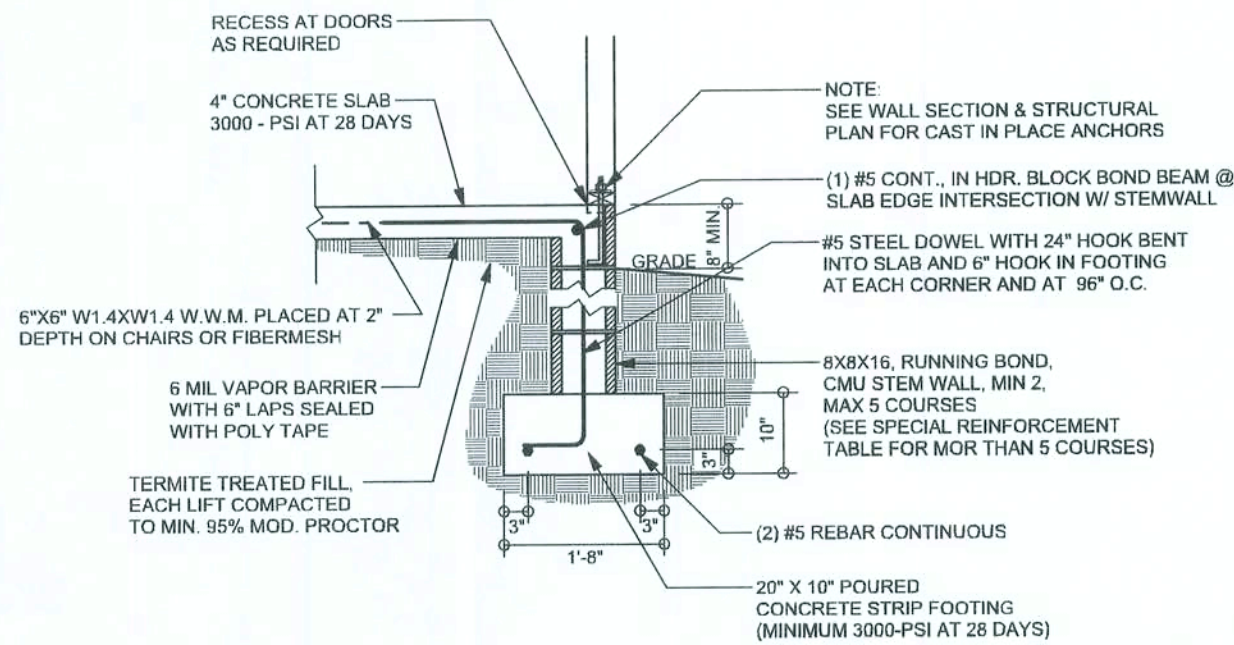
<b>DRAWN BY:</b> Evan Beamsley	<b>STRUCTURAL BY:</b> David Disoway
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<b>FINALS DATE:</b> July 24, 2008	
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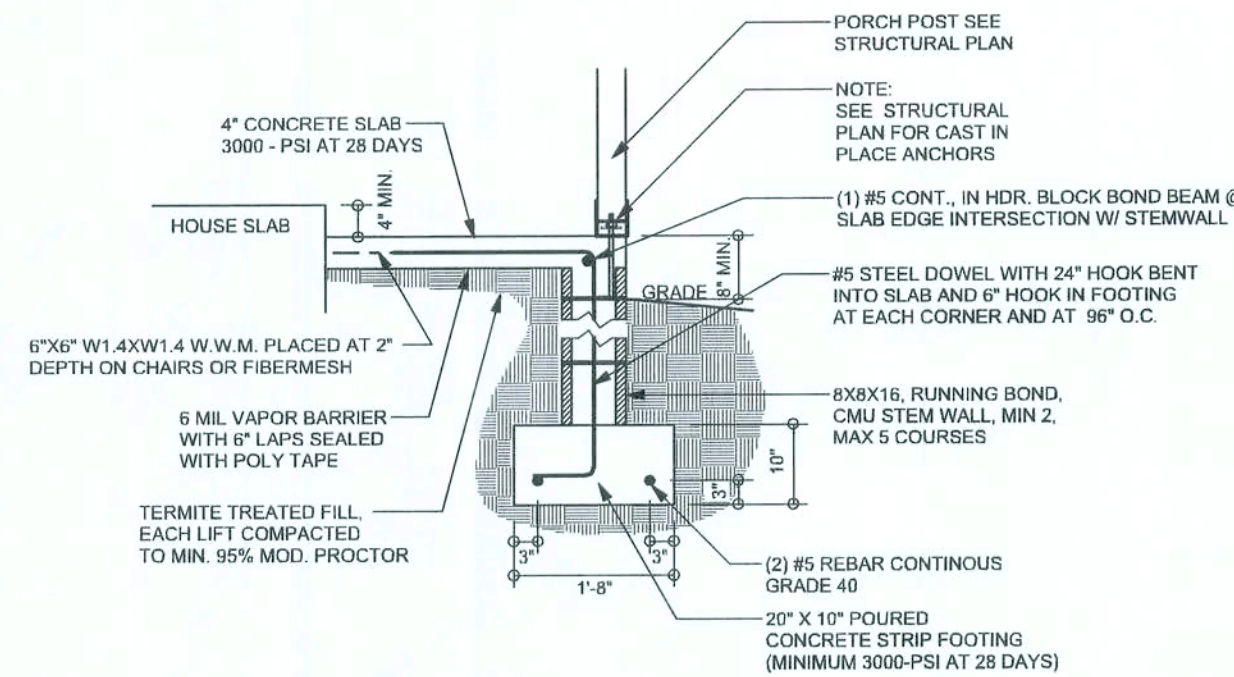
**JOB NUMBER:**  
**802-91**

**DRAWING NUMBER**  
**S-1**

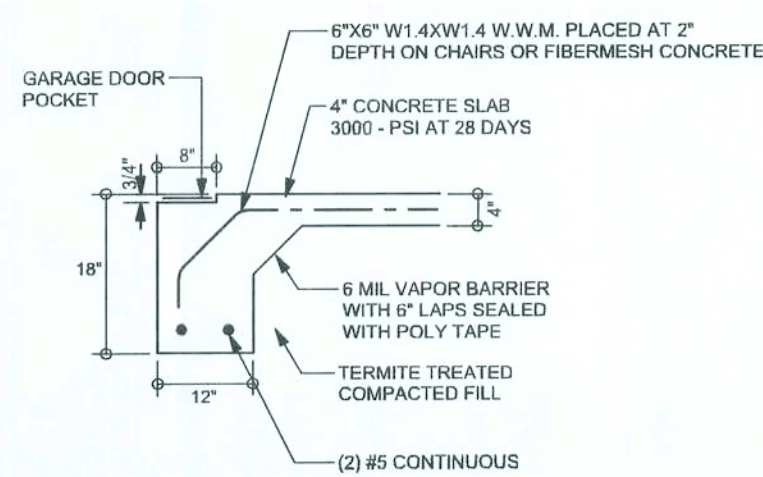




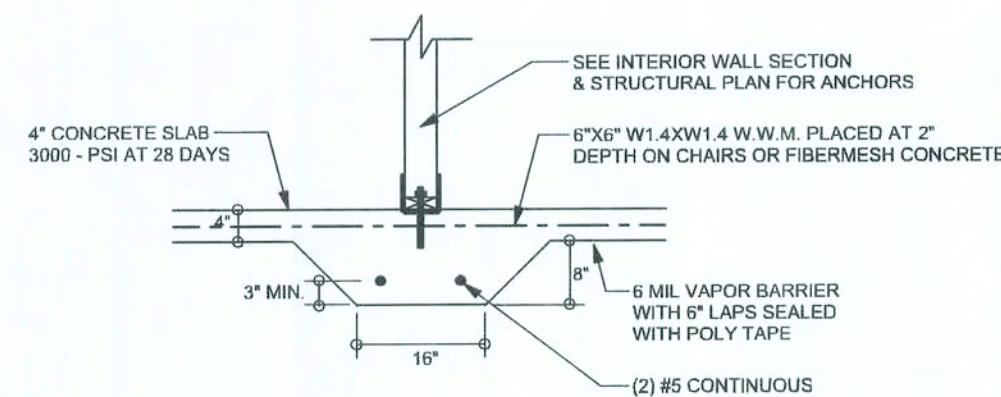
**F9 S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



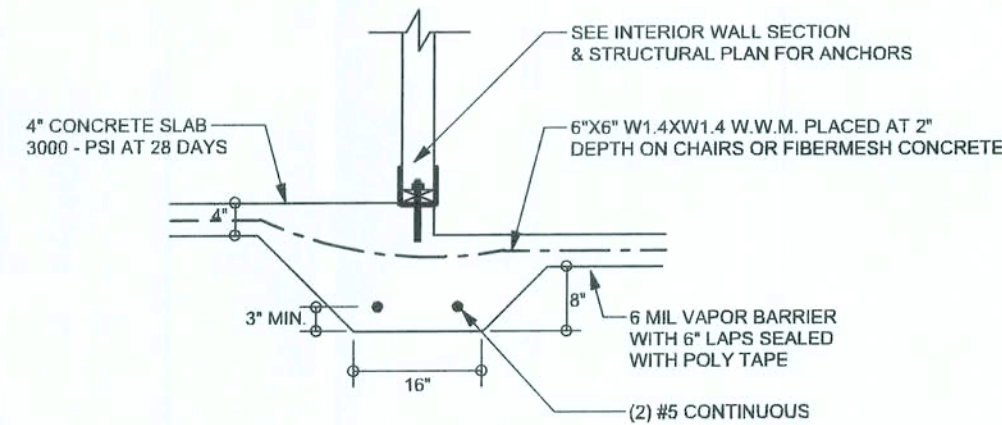
**F12 S-2** ALT. STEM WALL PORCH FOOTING  
SCALE: 1/2" = 1'-0"



**F4 S-2** GARAGE DOOR FOOTING  
SCALE: 1/2" = 1'-0"



**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"

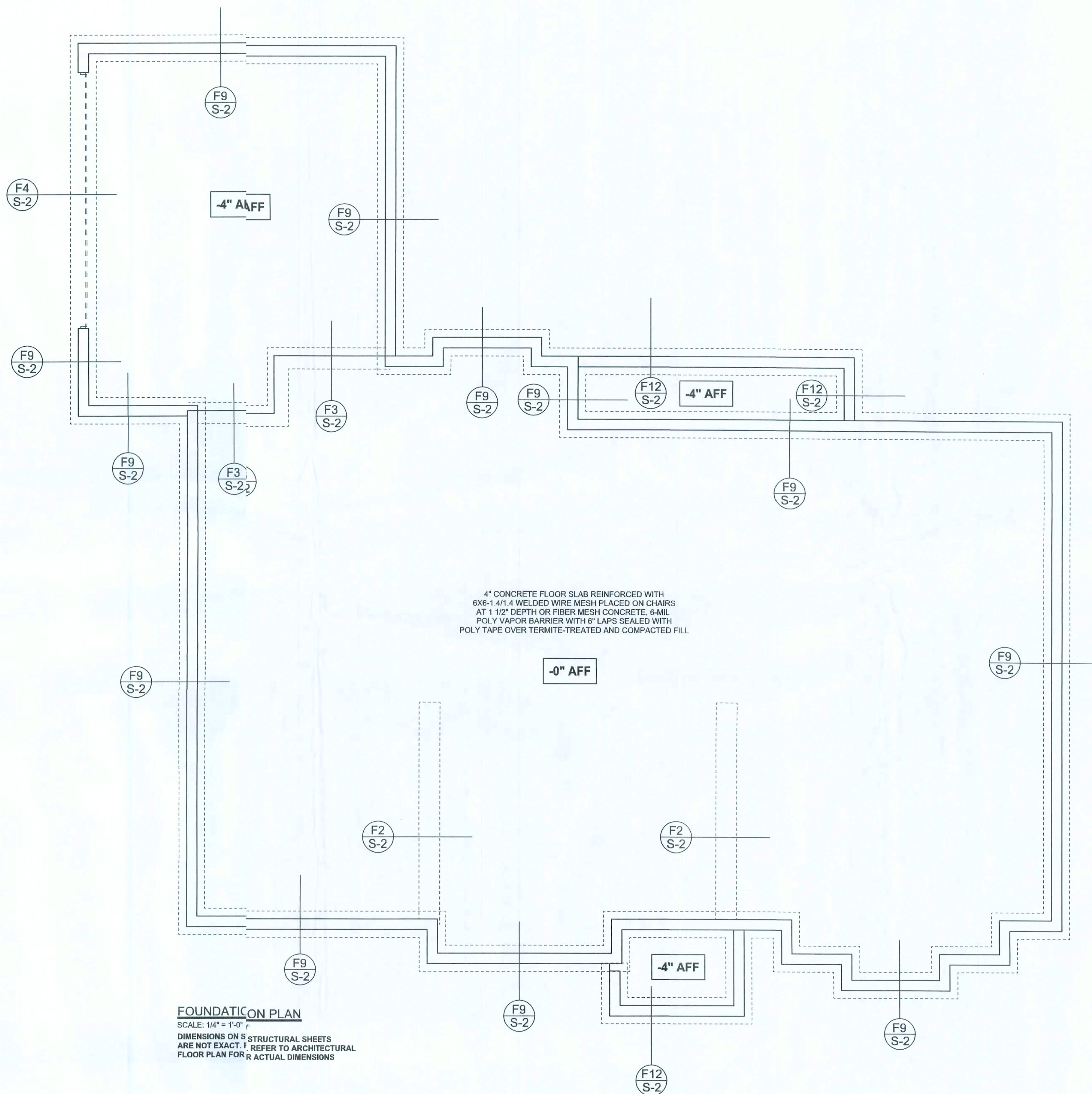


**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"

## TALL STM WALL TABLE

The table assumes all reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add wall ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 185 couplers at mid height. For higher parts of the wall 12" CMU may be used with reinforcement shown in the table below.

STEM WALL HEIGHT (FEET)	UNLANCED EXCEL HEIGHT (FEET)	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS	

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE

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**DIMENSIONS:**  
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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 6301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specification.

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FINALS DATE:  
July 24, 2008

JOB NUMBER:  
8(2191

DRAWING NUMBER  
**S-2**  
OF 1 SHEETS



