Columbia County Building Permit Application CK# 1824

For Office Use Only Application # 0701-16 Date Received 1/5 By JW Permit # 13111 / 25469

Application Approved by - Zoning Official SLK Date 22.01.07 Plans Examiner OK TH Date 1-9-07

Flood Zone X per survey Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Special Family Lot Permit - Parents to: Daughter

NOC EH Deed or PA Site Plan State Road Info Parent Parcel # Development Permit

Name Authorized Person Signing Permit Rebecca G. Thomas Phone 386-754-8814 Fax 386-754-8881

Address 424 SW Alamo Drive Lake City FL 32025

Owners Name John & Rebecca Thomas Phone 754-8814

911 Address 547 SW Dyal Avenue Lake City FL 32024

Contractors Name Owner-builder Phone

Address —

Fee Simple Owner Name & Address —

Bonding Co. Name & Address —

Architect/Engineer Name & Address Chris Cox - Draftsman - Mark Disosway - Engineer

Mortgage Lenders Name & Address PERSONAL - CA LK

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 27-4S-16-03211-005 Estimated Cost of Construction 180,000

Subdivision Name N/A Lot ____ Block ____ Unit ____ Phase ____

Driving Directions Hwy 90 W to Sisters Welcome Rd turn Right. Continue on Sisters Welcome Rd to Caution light at Hwy 242 cross Hwy 242 to Dyal Avenue. TL 2nd PLACE Past CHRIST CENTRAL on the L.

Type of Construction New Residential Number of Existing Dwellings on Property 0

Total Acreage 1.04 Lot Size ____ Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 200' Side 20' Side 25' Rear 75'

Total Building Height 25' Number of Stories 1 Heated Floor Area 2834 SF Roof Pitch 6/12

TOTAL 3328

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Rebecca Thomas

Owner Builder or Authorized Person by Notarized Letter

Contractor Signature

Contractors License Number

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 27th day of December 2006.

Known ✓ or Produced Identification ____

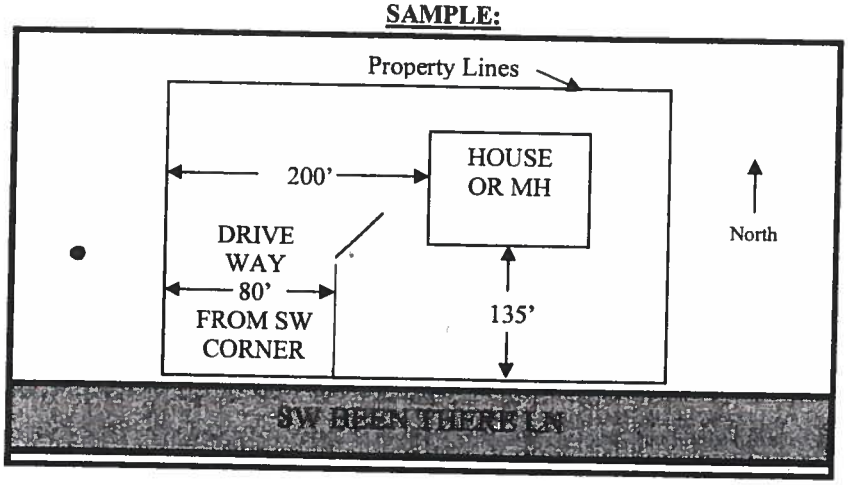
Denise L. Hingson

Notary Signature

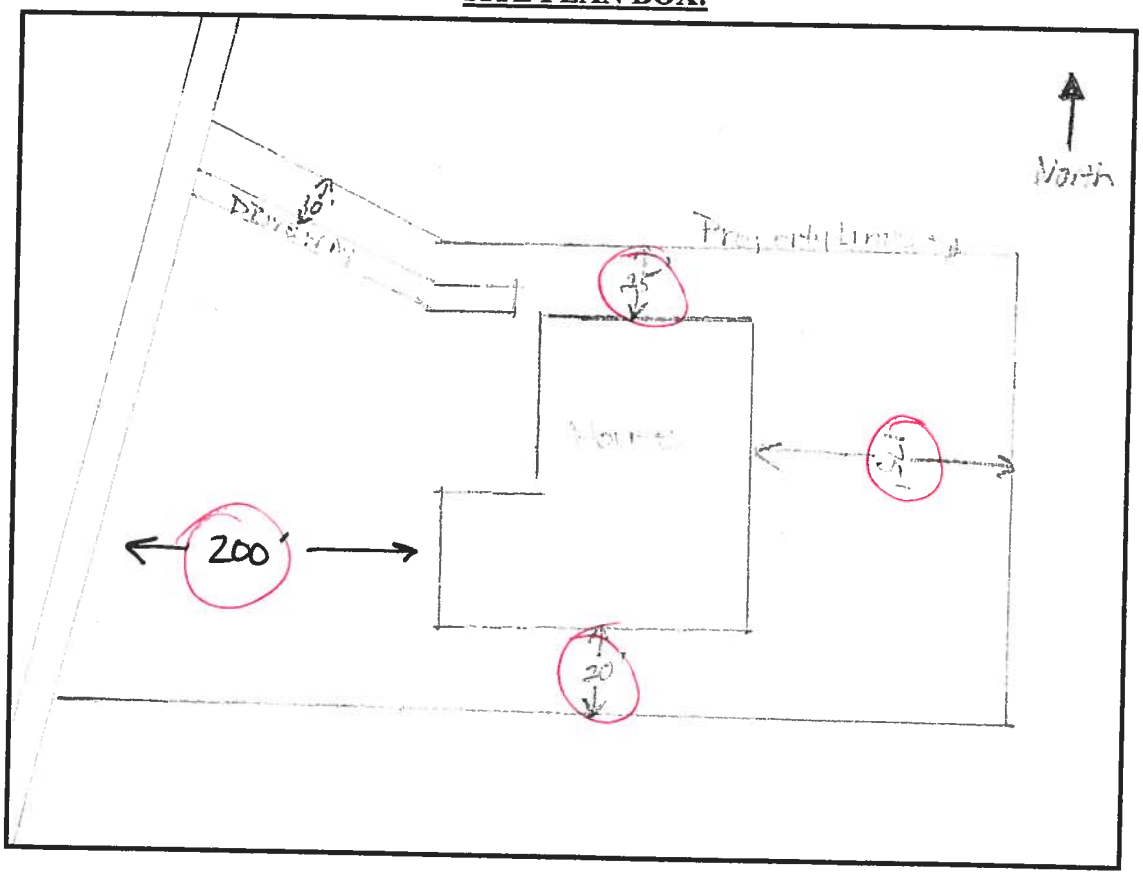
NOTARY PUBLIC-STATE OF FLORIDA
Denise L. Hingson
Commission # DD523871
Expires: MAR. 01, 2010
Bonded Thru (Revised 9/23/05) Atlantic Bonding Co., Inc.

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:



NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 27-4S-16-03211-005 **PERMIT NUMBER** _____

1. **Description of property: (legal description of the property and street address or 911 address)**
547 SW Dyal Avenue Lake City FL 32024

2. **General description of improvement:** New Residential Construction

3. **Owner Name & Address** John F Thomas Jr & Rebecca G. Thomas
424 SW Alamo Dr. Lake City FL 32025 **Interest in Property** _____

4. **Name & Address of Fee Simple Owner (if other than owner):** _____

5. **Contractor Name** Owner-builder - John F Thomas Jr. **Phone Number** (386) 754-8814
Address 424 SW Alamo Dr. Lake City FL 32025

6. **Surety Holders Name** _____ **Phone Number** _____
Address _____
Amount of Bond _____

7. **Lender Name** Columbia County Bank **Phone Number** 386-4453
Address _____

8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
Name _____ **Phone Number** _____
Address _____

9. **In addition to himself/herself the owner designates** _____ **of** _____ **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –**
(a) 7. Phone Number of the designee _____

10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)** _____

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Rebecca G Thomas
Signature of Owner

Sworn to (or affirmed) and subscribed before 24th **day of** December, **20** 06

NOTARY STAMP/SEAL

NOTARY PUBLIC-STATE OF FLORIDA
Denise L. Hingson
Commission # DD523871
Expires: MAR. 01, 2010
Bonded Thru Atlantic Bonding Co., Inc.

Denise L Hingson
Signature of Notary

Inst:2007001502 Date:01/22/2007 Time:08:48
DC,P.DeWitt Cason,Columbia County B:1108 P:1066

AFFIDAVIT OF SUBDIVIDED REAL PROPERTY
FOR USE OF IMMEDIATE FAMILY MEMBERS
FOR PRIMARY RESIDENCE

STATE OF FLORIDA
COUNTY OF COLUMBIA

BEFORE ME the undersigned Notary Public personally appeared.

Bill Gootee, the Owner of the parent tract which has been subdivided for immediate family primary residence use, hereinafter the Owner, and Rebecca THomas, the family member of the Owner, who is the owner of the family parcel which is intended for immediate family primary residence use, hereafter the Family Member, and is related to the Owner as daughter, and both individuals being first duly sworn according to law, depose and say:

1. Both the Owner and the Family Member have personal knowledge of all matters set forth in this Affidavit.

2. The Owner holds fee simple title to certain real property situated in Columbia County, and more particularly described by reference to the Columbia county Property Appraiser Tax Parcel No. 27-4S-16-03211-000.

3. The Owner has divided his parent parcel for use of immediate family members for their primary residence and the parcel divided and the remaining parent parcel are at least ½ acre in size. Immediate family is defined as grandparent, parent, step-parent, adopted parent, sibling, child, step-child, adopted child or grandchild.

4. The Family Member is a member of the Owner's immediate family, as set forth above, and holds fee simple title to certain real property divided from the Owner's parcel situated in Columbia County and more particularly described by reference to the Columbia County Property Appraiser Tax Parcel No. 27-4S-16-03211-005.

5. No person or entity other than the Owner and Family Member claims or is presently entitled to the right of possession or is in possession of the property, and there are no tenancies, leases or other occupancies that affect the Property.

6. This Affidavit is made for the specific purpose of inducing Columbia County to recognize a family division for a family member on the parcel divided in accordance with Section 14.9 of the Columbia County Land Development Regulations.

7. This Affidavit is made and given by Affiants with full knowledge that the facts contained herein are accurate and complete, and with full knowledge that the penalties under Florida law for perjury include conviction of a felony of the third degree.

We Hereby Certify that the information contained in this Affidavit are true and correct.

Owner

Family Member

Bill GooTee
Typed or Printed Name

Rebecca G Thomas
Typed or Printed Name

Subscribed and sworn to (or affirmed) before me this 22nd day of January, 2007, by Rebecca Thomas (Owner) who is personally known to me or has produced _____ as identification.

Notary Public

BEVERLY M. JACKSON
MY COMMISSION EXPIRES
April 29, 2009
#DD 390027
Bonded thru
Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA

Subscribed and sworn to (or affirmed) before me this 22 day of January, 2007, by Bill Gootee (Family Member) who is personally known to me or has produced _____ as identification.

Notary Public

BEVERLY M. JACKSON
MY COMMISSION EXPIRES
April 29, 2009
#DD 390027
Bonded thru
Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA

NOTORIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

(X) Single Family Dwelling **( ) Two-Family Residence**
( ) Farm Outbuilding **( ) Other_____**

NEW CONSTRUCTION OR IMPROVEMENT

(X) New Construction **( ) Addition, Alteration, Modification or other Improvement**

I Rebecca G. Thomas **, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____**

Rebecca Thomas 12-29-06
Owner Builder Signature **Date**

The above signer is personally known to me or produced identification _____

NOTARY PUBLIC-STATE OF FLORIDA
Denise L. Hingson
Commission # DD523871
Expires: MAR. 01, 2010
Bonded Thru Atlantic Bonding Co., Inc.

Notary Signature Denise L Hingson **Date** 12/29/06 **(Stamp / Seal)**

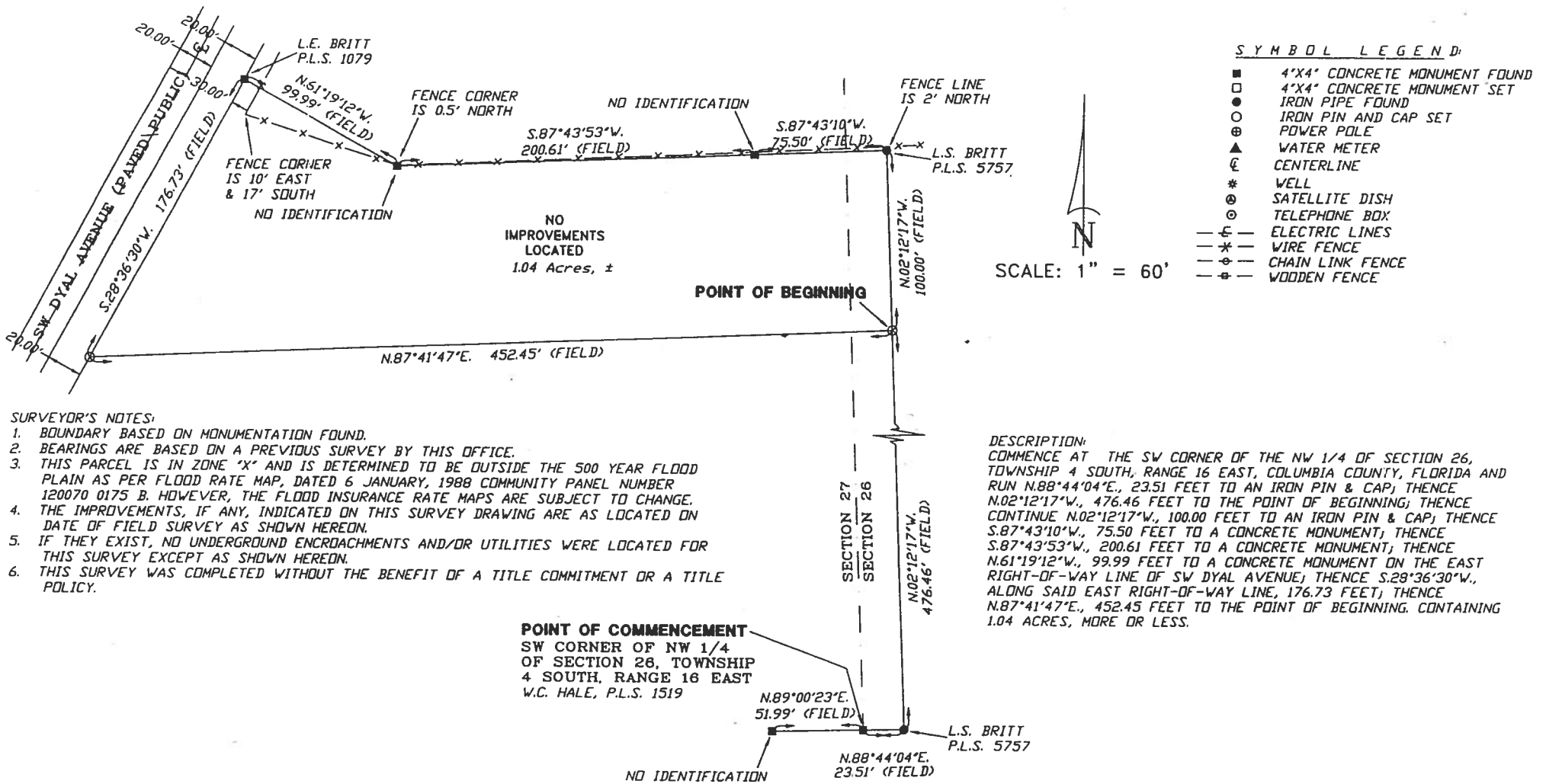
FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date_____ Building Official/Representative_____

Rebeca Thomas - 623-5079

BOUNDARY SURVEY IN SECTIONS 26 & 27, TOWNSHIP 4 SOUTH RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.



SYMBOL LEGEND:

- ■ 4"X4" CONCRETE MONUMENT FOUND
- □ 4"X4" CONCRETE MONUMENT SET
- ● IRON PIPE FOUND
- ○ IRON PIN AND CAP SET
- ⊕ POWER POLE
- ▲ WATER METER
- ℄ CENTERLINE
- ✱ WELL
- ⊛ SATELLITE DISH
- ⊙ TELEPHONE BOX
- —E— ELECTRIC LINES
- —x— WIRE FENCE
- —o— CHAIN LINK FENCE
- —⊕— WOODEN FENCE

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND.
2. BEARINGS ARE BASED ON A PREVIOUS SURVEY BY THIS OFFICE.
3. THIS PARCEL IS IN ZONE "X" AND IS DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD RATE MAP, DATED 6 JANUARY, 1988 COMMUNITY PANEL NUMBER 120070 0175 B. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.

DESCRIPTION:

COMMENCE AT THE SW CORNER OF THE NW 1/4 OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N.88°44'04"E., 23.51 FEET TO AN IRON PIN & CAP; THENCE N.02°12'17"W., 476.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N.02°12'17"W., 100.00 FEET TO AN IRON PIN & CAP; THENCE S.87°43'10"W., 75.50 FEET TO A CONCRETE MONUMENT; THENCE S.87°43'53"W., 200.61 FEET TO A CONCRETE MONUMENT; THENCE N.61°19'12"W., 99.99 FEET TO A CONCRETE MONUMENT ON THE EAST RIGHT-OF-WAY LINE OF SW DYAL AVENUE; THENCE S.28°36'30"W., ALONG SAID EAST RIGHT-OF-WAY LINE, 176.73 FEET; THENCE N.87°41'47"E., 452.45 FEET TO THE POINT OF BEGINNING. CONTAINING 1.04 ACRES, MORE OR LESS.

CERTIFIED TO:

JOHN & REBECCA THOMAS

FIELD BOOK: SEE PAGE(S): FILE

SURVEYOR'S CERTIFICATION

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

07/12/06 FIELD SURVEY DATE — 07/20/06 DRAWING DATE

L. SCOTT BRITT, P.S.M. CERTIFICATION # 5757

NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

BRITT SURVEYING
LAND SURVEYORS AND MAPPERS
830 WEST DUVAL STREET LAKE CITY, FLORIDA 32055
(386)752-7163 FAX (386)752-5573

WORK ORDER # L-17539

@.CAM112M01 CamaUSA Appraisal System Columbia County
1/05/2007 10:53 **Legal Description Maintenance** **18720 Land 001**
Year T Property Sel **AG 000**
2007 R 27-4S-16-03211-005 ... .. **Bldg 000**
Xfea 000

THOMAS JOHN F & REBECCA G **18720 TOTAL B**

1	COMM SW COR OF NW1/4, RUN E	23.51 FT, RUN N 476.46 FT FOR	2
3	POB, CONT N 100 FT, RUN W	75.50 FT TO A CONCR MONU, CONT	4
5	W 200.61 FT, RUN N 61 DG W,	99.99 FT TO E R/W LINE SW DYAL	6
7	AVE, RUN S 28 DG W ALONG R/W,	176.73 FT, E 452.45 FT TO POB.	8
9	WD 1092-296		10
11			12
13			14
15			16
17			18
19			20
21			22
23			24
25			26
27			28

Mnt **11/28/2006 WANDA**

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

Inst:2006018793 Date:08/08/2006 Time:13:16
Doc Stamp-Deed : 0.00
DC,P.DeWitt Cason,Columbia County B:1092 P:296

Above Space Reserved for Recording
[If required by your jurisdiction, list above the name & address of: 1) where to return this form; 2) preparer; 3) party requesting recording.]

Warranty Deed

Date of this Document: August 7. 2006

Reference Number of Related Documents: _____

Grantor(s):
Name William and Bonnie Gootee
Street Address 667 SW Dyal Avenue
City/State/Zip Lake City, FL 32024

Grantee(s):
Name John F. and Rebecca G. Thomas
Street Address 424 SW Alamo Drive
City/State/Zip Lake City, FL 32025

Abbreviated Legal Description (i.e., lot, block, plat, *or* section, township, range, quarter/quarter *or* unit, building and condo name): Commence at the SW Corner of the NW 1/4 of Section 26, Township 4 South, Range 16 East, Columbia County, Florida.

Assessor's Property Tax Parcel/Account Number(s): 27-4S1603211

For good consideration, William and Bonnie Gootee, of 667 SW Dyal Avenue Lake City, County of Columbia, State of Florida, hereby bargain, deed and convey to John F. and Rebecca G. Thomas of 424 SW Alamo Drive Lake City, County of Columbia, State of Florida, the following described land in Columbia County, free and clear with WARRANTY COVENANTS; to wit: COMMENCE AT THE SW CORNER OF THE NW 1/4 OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N.88°44'04"E., 23.51 FEET TO AN IRON PIN & CAP; THENCE N.02°12'17"W., 476.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N.02°12'17"W., 100.00 FEET TO AN IRON PIN & CAP; THENCE S.87°43'10"W., 75.50 FEET TO A CONCRETE MONUMENT; THENCE S.87°43'53"W., 200.61 FEET TO A CONCRETE MONUMENT; THENCE N.61°19'12"W., 99.99 FEET TO A CONCRETE MONUMENT ON THE EAST RIGHT OF WAY LINE OF SW DYAL AVENUE; THENCE S.28°36'30"W., ALONG SAID EAST RIGHT OF WAY LINE, 176.73 FEET; THENCE N.87°41'47"E., 452.45 FEET TO THE POINT OF BEGINNING. CONTAINING 1.04 ACRES, MORE OR LESS.

Grantor, for itself and its heirs, hereby covenants with Grantee, its heirs, and assigns, that Grantor is lawfully seized in fee simple of the above-described premises; that it has a good right to convey; that the premises are free from all encumbrances; that Grantor and its heirs, and all persons acquiring any interest in the property granted, through or for Grantor, will, on demand of Grantee, or its heirs or assigns, and at the expense of Grantee, its heirs or assigns, execute any instrument necessary for the further assurance of the title to the premises that may be reasonably required; and that Grantor and its heirs will forever warrant and defend all of the property so granted to Grantee, its heirs, and assigns, against every person lawfully claiming the same or any part thereof.

Being the same property conveyed to the Grantor by deed of ---------- , dated August 7, , 20 06 .

WITNESS the hands and seal of said Grantor this 7th day of August , 20 06 .

Grantor

Grantor

State of Florida)

County of Columbia)

On 08-07-06 , before me, William Gootee , personally appeared Bonnie R. Gootee , personally known to me (or proved to me on the basis of satisfactory evidence) to be the person(s) whose name(s) is/are subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their authorized capacity(ies), and that by his/her/their signature(s) on the instrument the person(s), or the entity upon behalf of which the person(s) acted, executed the instrument.

WITNESS my hand and official seal.

Signature

Affiant X Known X Unknown

ID Produced

KELLY CREWS
MY COMMISSION # DD 346937
EXPIRES: December 14, 2008
Bonded Thru Notary Public Underwriters

(Seal)

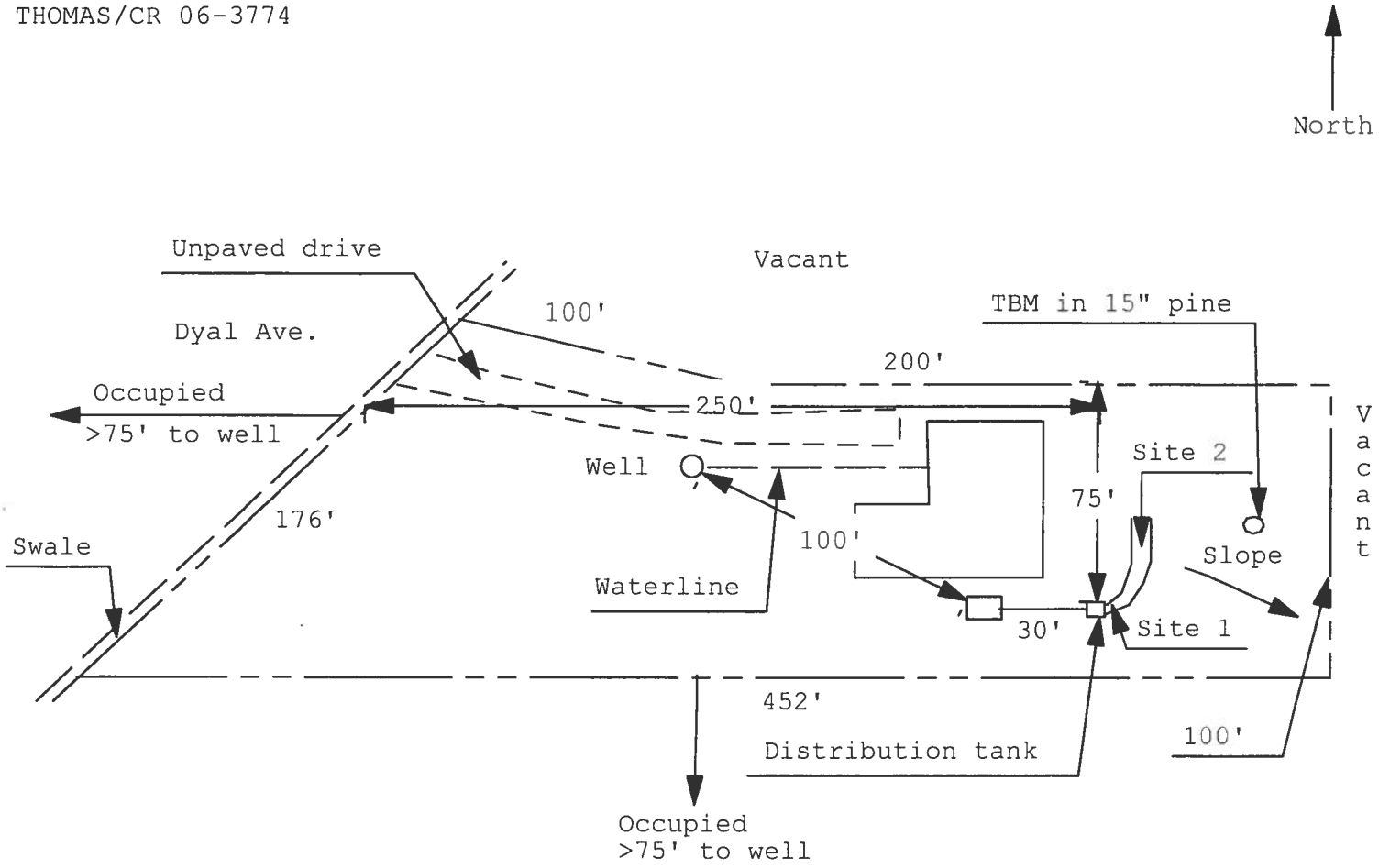
Inst:2006018793 Date:08/08/2006 Time:13:16
Doc Stamp-Deed : 0.00
DC,P.DeWitt Cason,Columbia County B:1092 P:297

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-1065N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

THOMAS/CR 06-3774



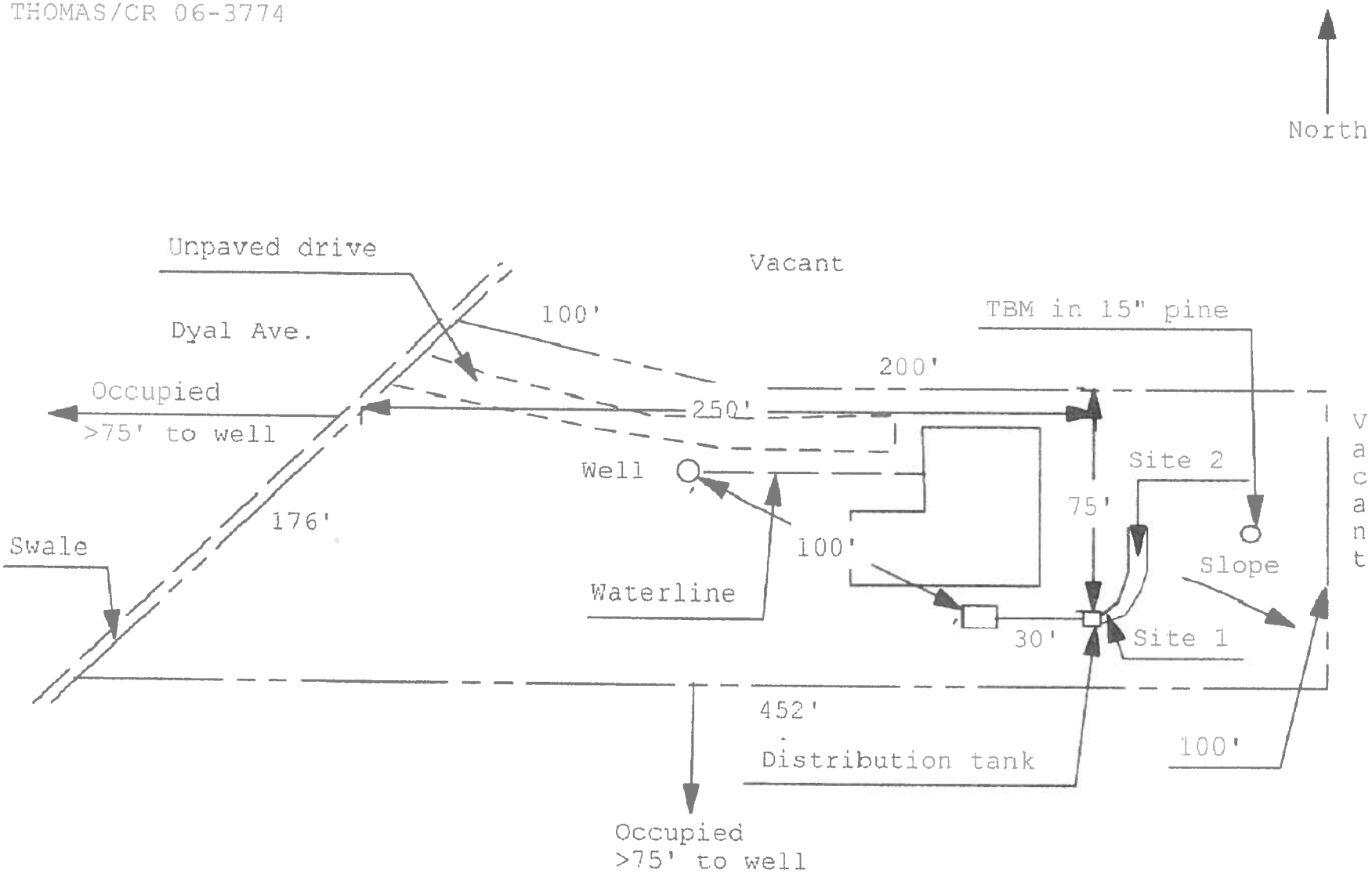
1 inch = 60 feet

Site Plan Submitted By Paul Lloyd **Date** 11/15/06

Plan Approved ✓ **Not Approved** ____ **Date** 12/7/06

By M R D Jr Columbia **CPHU**

Notes: ____



1 inch = 60 feet

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 12/5/2006 **DATE ISSUED:** 12/13/2006

ENHANCED 9-1-1 ADDRESS:

547 SW DYAL AVE

LAKE CITY FL 32024

PROPERTY APPRAISER PARCEL NUMBER:

27-4S-16-03211-005

Remarks:

Address Issued By: _____
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

517

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**609121JohnThomas**	Builder:	John Thomas
Address:		Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	25460
Owner:	**John Thomas**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	4	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	2834 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT)	7a. Description (Dble Default)	Area 287.3 ft²
b.	SHGC: (or Clear or Tint DEFAULT)	7b. (Clear)	287.3 ft²
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 227.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Exterior	R=13.0, 1702.7 ft²	
b.	Frame, Wood, Adjacent	R=13.0, 196.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 2834.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 170.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 46.0 kBtu/hr SEER: 13.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 46.0 kBtu/hr HSPF: 7.90	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.93	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

Glass/Floor Area: 0.10

Total as-built points: 32126
Total base points: 39778

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Ron Lynch

DATE: 10-25-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	**2834.0**	**20.04**	**10222.8**	Double, Clear	S	7.0	5.5	30.0	35.87	0.49	525.1
				Double, Clear	S	10.0	5.5	20.0	35.87	0.46	327.0
				Double, Clear	E	1.5	5.5	60.0	42.06	0.90	2261.9
				Double, Clear	S	1.5	5.5	45.0	35.87	0.83	1343.1
				Double, Clear	W	1.5	5.5	45.0	38.52	0.90	1554.9
				Double, Clear	W	1.5	1.5	8.0	38.52	0.53	163.5
				Double, Clear	N	8.0	6.0	30.0	19.20	0.67	385.1
				Double, Clear	N	10.0	7.0	13.3	19.20	0.66	168.6
				Double, Clear	N	1.5	5.5	20.0	19.20	0.93	356.4
				Double, Clear	E	1.5	3.5	6.0	42.06	0.78	195.7
				Double, Clear	E	1.5	5.5	10.0	42.06	0.90	377.0
				As-Built Total:				**287.3**			**7658.3**

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	196.0	0.70	137.2	Frame, Wood, Exterior	13.0	1702.7	1.50	2554.0
Exterior	1702.7	1.70	2894.6	Frame, Wood, Adjacent	13.0	196.0	0.60	117.6
Base Total:	**1898.7**		**3031.8**	**As-Built Total:**		**1898.7**		**2671.6**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	20.0	1.60	32.0	Exterior Insulated	20.0	4.10	82.0
Exterior	40.0	4.10	164.0	Adjacent Insulated	20.0	1.60	32.0
				Exterior Insulated	20.0	4.10	82.0
Base Total:	**60.0**		**196.0**	**As-Built Total:**	**60.0**		**196.0**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	2834.0	1.73	4902.8	Under Attic	30.0	2834.0	1.73 X 1.00	4902.8
Base Total:	**2834.0**		**4902.8**	**As-Built Total:**		**2834.0**		**4902.8**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	227.0(p)	-37.0	-8399.0	Slab-On-Grade Edge Insulation	0.0	227.0(p	-41.20	-9352.4
Raised	0.0	0.00	0.0					
Base Total:			**-8399.0**	**As-Built Total:**		**227.0**		**-9352.4**

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points				Area X	SPM	=	Points
	2834.0	10.21	28935.1				2834.0	10.21		28935.1
Summer Base Points: 38889.6				**Summer As-Built Points:**						**35011.5**
Total Summer Points	X System Multiplier	= Cooling Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
				(sys 1: Central Unit 46000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)						
				35012	1.00	(1.09 x 1.147 x 0.91)	0.263	1.000		10457.7
38889.6	**0.4266**	**16590.3**		**35011.5**	**1.00**	**1.138**	**0.263**	**1.000**		**10457.7**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2834.0	12.74	6498.9	Double, Clear	S	7.0	5.5	30.0	13.30	3.10	1238.1	
				Double, Clear	S	10.0	5.5	20.0	13.30	3.47	922.1	
				Double, Clear	E	1.5	5.5	60.0	18.79	1.04	1174.2	
				Double, Clear	S	1.5	5.5	45.0	13.30	1.15	686.4	
				Double, Clear	W	1.5	5.5	45.0	20.73	1.03	959.0	
				Double, Clear	W	1.5	1.5	8.0	20.73	1.17	193.5	
				Double, Clear	N	8.0	6.0	30.0	24.58	1.02	753.2	
				Double, Clear	N	10.0	7.0	13.3	24.58	1.02	334.1	
				Double, Clear	N	1.5	5.5	20.0	24.58	1.00	493.0	
				Double, Clear	E	1.5	3.5	6.0	18.79	1.09	123.3	
				Double, Clear	E	1.5	5.5	10.0	18.79	1.04	195.7	
				As-Built Total:				**287.3**			**7072.7**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	196.0	3.60	705.6	Frame, Wood, Exterior	13.0	1702.7	3.40	5789.2
Exterior	1702.7	3.70	6300.0	Frame, Wood, Adjacent	13.0	196.0	3.30	646.8
Base Total:	**1898.7**		**7005.6**	**As-Built Total:**		**1898.7**		**6436.0**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	20.0	8.00	160.0	Exterior Insulated	20.0	8.40	168.0
Exterior	40.0	8.40	336.0	Adjacent Insulated	20.0	8.00	160.0
				Exterior Insulated	20.0	8.40	168.0
Base Total:	**60.0**		**496.0**	**As-Built Total:**	**60.0**		**496.0**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM =	Points
Under Attic	2834.0	2.05	5809.7	Under Attic	30.0	2834.0	2.05 X 1.00	5809.7
Base Total:	**2834.0**		**5809.7**	**As-Built Total:**		**2834.0**		**5809.7**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	227.0(p)	8.9	2020.3	Slab-On-Grade Edge Insulation	0.0	227.0(p	18.80	4267.6
Raised	0.0	0.00	0.0					
Base Total:			**2020.3**	**As-Built Total:**		**227.0**		**4267.6**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,**	PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points		Area X	WPM	= Points			
	2834.0	-0.59	-1672.1		2834.0	-0.59	-1672.1			
Winter Base Points:			**20158.5**	**Winter As-Built Points:**						**22409.9**
Total Winter Points	X	System Multiplier	= Heating Points	Total Component (System - Points)	X	Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
				(sys 1: Electric Heat Pump 46000 btuh ,EFF(7.9) Ducts:Unc(S),Unc(R),Int(AH),R6.0						
				22409.9		1.000	(1.069 x 1.169 x 0.93)	0.432	1.000	11242.0
20158.5		**0.6274**	**12647.4**	**22409.9**		**1.00**	**1.162**	**0.432**	**1.000**	**11242.0**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
4		2635.00	10540.0	40.0	0.93	4	1.00	2606.67	1.00	10426.7	
				As-Built Total:						**10426.7**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
16590	**12647**	**10540**	**39778**	**10458**	**11242**	**10427**	**32126**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , **Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.7

The higher the score, the more efficient the home.

John Thomas, , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 46.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	4		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	2834 ft²		c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 287.3 ft²		a. Electric Heat Pump	Cap: 46.0 kBtu/hr
	b. SHGC:				HSPF: 7.90
	(or Clear or Tint DEFAULT) 7b.	(Clear) 287.3 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 227.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1702.7 ft²			EF: 0.93
	b. Frame, Wood, Adjacent	R=13.0, 196.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 2834.0 ft²	15.	HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 170.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

25469

Columbia County Property Appraiser

DB Last Updated: 4/11/2007

2007 Proposed Values

Parcel: 27-4S-16-03211-005

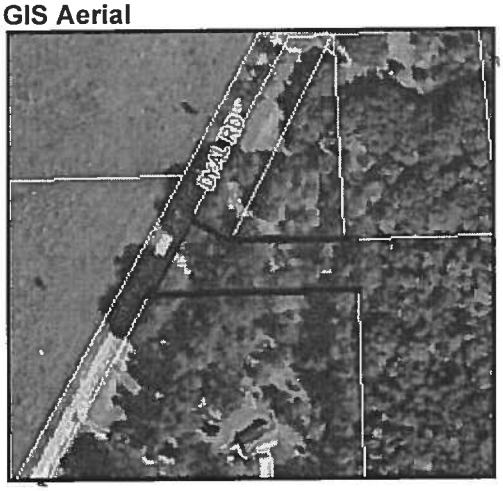
Tax Record | Property Card | Interactive GIS Map | Print

<< Prev Search Result: 6 of 8 Next >>

Owner & Property Info

Owner's Name	THOMAS JOHN F & REBECCA G		
Site Address			
Mailing Address	424 SW ALAMO DR LAKE CITY, FL 32025		
Use Desc. (code)	NO AG ACRE (009900)		
Neighborhood	27416.00	**Tax District**	3
UD Codes		**Market Area**	01
Total Land Area	1.140 ACRES		
Description	COMM SW COR OF NW1/4, RUN E 23.51 FT, RUN N 476.46 FT FOR POB, CONT N 100 FT, RUN W 75.50 FT TO A CONCR MONU, CONT W 200.61 FT, RUN N 61 DG W, 99.99 FT TO E R/W LINE SW DYAL AVE, RUN S 28 DG W ALONG R/W, 176.73 FT, E 452.45 FT TO POB & COMM SW COR OF NW1/4 OF SEC 26, E 23.51 FT, N 476.46 FT FOR POB, W 452.45 FT TO E R/W DYAL AVE, SW 11.66 FT, E 458.45 FT, N 10 FT TO POB. WD 1092-296, QC 1114-132		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	$20,520.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$20,520.00

Just Value	$20,520.00
Class Value	$0.00
Assessed Value	$20,520.00
Exempt Value	$0.00
Total Taxable Value	$20,520.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
8/7/2006	1092/296	WD	V	U	06	$0.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
009900	AC NON-AG (MKT)	1.140 AC	1.00/1.00/1.00/1.00	$18,000.00	$20,520.00

Columbia County Property Appraiser

DB Last Updated: 4/11/2007

Quitclaim Deed

This Quitclaim Deed made March 20, 2007, by
William Gootee("Transferor")
667 SW Dyal Avenue
Lake City, FL 32024

Inst:2007006341 Date:03/20/2007 Time:09:02
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1114 P:132

to:
John and Rebecca Thomas ("Transferee")
424 SW Alamo Drive
Lake City, FL 32025

Transferor, in consideration of One Dollar and other good and valuable consideration, the receipt and sufficiency of which is hereby acknowledged, remises, releases, and forever quitclaims to Transferee all of the interest of Transferor, if any, in an to that real property located in the County of Columbia, and State of Florida, and more certainly described as follows:

Parcel "A": COMMENCE AT THE SW CORNER OF THE NW 1/4 OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N.88 44'04"E., 23.51 FEET TO AN IRON PIN & CAP; THENCE N.02 12'17"W., 576.46 FEET TO AN IRON PIN & CAP; THENCE S.87 43'10"W., 75.50 FEET TO A CONCRETE MONUMENT AND TO THE POINT OF BEGINNING; THENCE S.87 43'53"W., 200.61 FEET TO A CONCRETE MONUMENT; THENCE N.61 19'12"W., 99.99 FEET TO A CONCRETE MONUMENT ON THE EAST RIGHT-OF-WAY LINE OF SW DYAL AVENUE; THENCE S.28 36'30"W., ALONG SAID EAST RIGHT-OF-WAY LINE, 176.73 FEET; THENCE N.87 41'47"E., 376.95 FEET; THENCE N.02 12'24"W., 100.03 FEET TO THE POINT OF BEGINNING. CONTAINING .87 ACRES, MORE OR LESS. ALSO Parcel "B": COMMENCE AT THE SW CORNER OF THE NW 1/4 OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N.88 44'04"E., 23.51 FEET TO AN IRON PIN & CAP; THENCE N.02 12'17"W., 476.46 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N.02 12'17"W., 100.00 FEET TO AN IRON PIN & CAP; THENCE S.87 43'10"W., 75.50 FEET TO A CONCRETE MONUMENT; THENCE S.02 12'24"E., 100.03 FEET; THENCE N.87 41'47"E., 75.50 FEET TO THE POINT OF BEGINNING. CONTAINING .17 ACRES MORE OR LESS. ALSO Parcel "C": COMMENCE AT THE SW CORNER OF THE NW 1/4 OF SECTION 26, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN N.88 44'04"E., 23.51 FEET TO AN IRON PIN & CAP; THENCE N.02 12'17"W., 476.46 FEET TO THE POINT OF BEGINNING; THENCE S.87 41'47"W., 452.45 FEET TO THE EAST RIGHT-OF-WAY LINE OF SW DYAL AVENUE; THENCE S.28 36'30"W., ALONG SAID EAST RIGHT-OF-WAY LINE. 11.66 FEET; THENCE N.87 41'47"E., 458.45 FEET; THENCE N.02 22'48"W., 10.00 FEET TO THE POINT OF

Inst:2007006341 Date:03/20/2007 Time:09:02
Doc Stamp-Deed : 0.70
DC,P.DeWitt Cason,Columbia County B:1114 P:133

BEGINNING. CONTAINING .10 ACRES MORE OR LESS. .

To have and to hold, all and singular the described property, together with the tenements, hereditaments, and appurtenances belonging to such property, or in anywise appertaining, and the rents, issues, and profits of such property to Transferee, and Transferee's heirs and assigns forever.

IN WITNESS WHEREOF, Transferor has executed this Quitclaim Deed on the date first above written.

William Gootee

Acknowledgment

State of Florida)
) ss
County of Columbia)

On this March 20, 2007, before me personally appeared William Gootee, to me know to be the person described in and who executed the foregoing Quitclaim Deed and acknowledged to me that William Gooteeexecuted the same ashis free act and deed.

Notary Public

BEVERLY M. JACKSON
MY COMMISSION EXPIRES
April 29, 2009
#DD 390027
Bonded thru
Notary Public Underwriters
NOTARY PUBLIC, STATE OF FLORIDA

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001311

DATE 01/26/2007 PARCEL ID # 27-4S-16-03211-005

APPLICANT REBECCA THOMAS PHONE 754-8814

ADDRESS 424 SW ALAMO DRIVE LAKE CITY FL 32024

OWNER JOHN & REBECCA THOMAS PHONE 754-8814

ADDRESS 547 SW DYAL AVE LAKE CITY FL 32024

CONTRACTOR OWNER BUILDER PHONE

LOCATION OF PROPERTY 90W, TL ON SISTERS WELCOME ROAD, CROSS CR 242, TL ON DYAL AVE, 2ND PLACE PAST CRIST CENTRAL MINISTRIES ON LEFT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

SIGNATURE Signature on file

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

NOTICE OF INSPECTION AND/OR TREATMENT

#25469

Date of Inspection
5/31/07

Date of Treatment

Date of Spot Treatment
Termidor

Pesticide Used
subterranean Termites

Wood-Destroying Organisms Treated

Notice
It is a violation of Florida State Law (Chap. 482.226) for anyone other than the property owner to remove this notice.

Address:
Pestmaster Services of Lake City
879 S.W. Arlington Blvd., Suite 106 • Lake City, FL 32025

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 27-4S-16-03211-005 **Building permit No.** 000025469

Use Classification SFD,UTILITY **Fire:** 70.62

Permit Holder OWNER BUILDER **Waste:** 184.25

Owner of Building JOHN & REBECCA THOMAS **Total:** 254.87

Location: 547 SW DYAL AVE, LAKE CITY, FL

Date: 11/19/2007

Wayne A. Luse

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☐	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information* must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
		Elevations including:
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☑ ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
☑ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☑ ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
☑ ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☑ ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☑ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☑ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

N/A **Floor Framing System:**

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☑ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** **If the project is to be located within the city limits of the Town of Fort White,** prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. **(386) 497-2321**

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit **($25.00)** must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver **($50.00)**. All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

_____

_____

_____ APPLICANT SIGNATURE

_____ DATE

R-1305 01-04

SHINGLES

ELK Premium Roofing

March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:

ASTM D3462
PA 100 (110 mph uplift and wind driven rain resistance)
PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA)

Raised Profile, Prestique High Definition, Prestique 25 or Prestique 30 –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.04

Prestique I 35 or Prestique I* –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection –
PA 100 = 4 nails
PA 107 = 4 nails
MD NOA# = 01-1226.03

Capstone*
PA 100 = 4 Nails
PA 107 = 4 Nails
MD NOA# = 01-0523.01

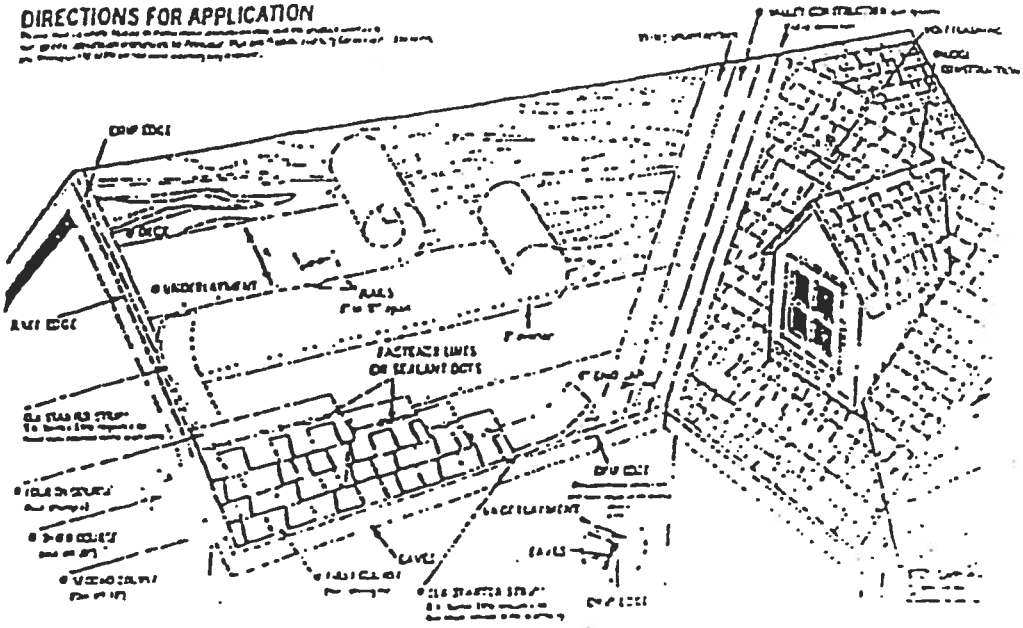
* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty

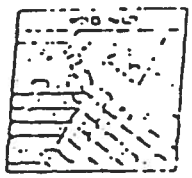
If there are any questions please contact:

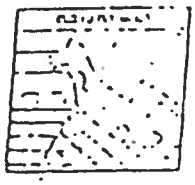
Mike Reed – Technical Manager
(205) 342-0287

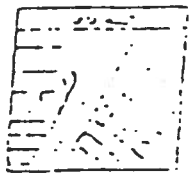
or Daniel Delemente – QA Engineer
(205) 342-0293

DIRECTIONS FOR APPLICATION









ELK

www.elkcorp.com

ROOFING PRODUCTS SPECIFICATIONS – TUSCALOOSA, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

HIP AND RIDGE SHINGLES

SOUTHEAST & ATLANTIC OFFICE:
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

PLANT LOCATION:
800.945.5545

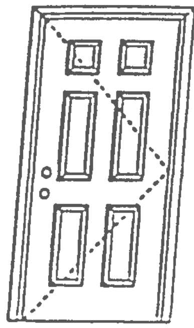
ELK
www.elkcorp.com

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Design Pressure
+66.0/-66.0

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

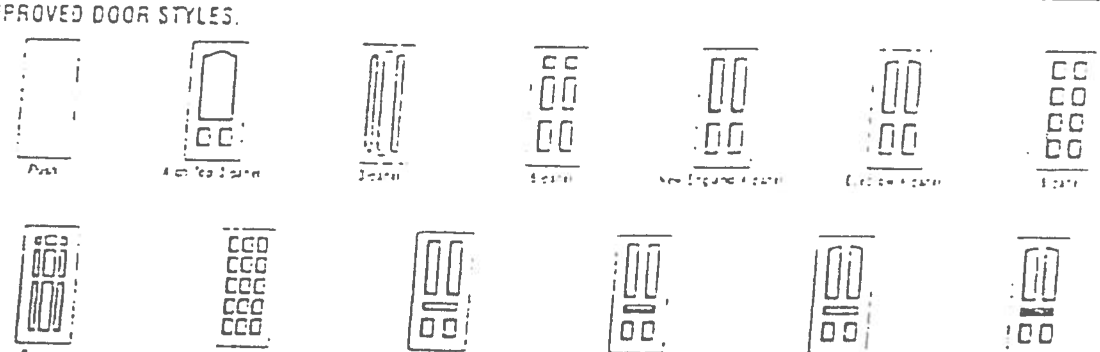
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA00[illegible]

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA00[illegible]

APPROVED DOOR STYLES.



X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #[illegible] and COP/Test Report Validation Matrix #[illegible]-001 provides additional information - available from the ITS WH website (www.[illegible].com), the Masonite website (www.masonite.com) or the Masonite technical centre.

Johnson EntrySystems

June 17, 200[illegible]
[illegible]

PREMDOOR

Exclusively from Masonite
Masonite International Corporation

FLORIDA DEPARTMENT OF
Community Affairs

Community Affairs

Product Approval

USER: Public User

FL #	FL5438
Application Type	New
Code Version	2004
Application Status	Approved
Comments	
Archived	☐
Product Manufacturer	MI Windows and Doors
Address/Phone/Email	650 W Market St Gratz, PA 17030 (717) 365-3300 ext 2101 surich@miwd.com
Authorized Signature	Steven Urich surich@miwd.com
Technical Representative Address/Phone/Email	
Quality Assurance Representative Address/Phone/Email	
Category	Windows
Subcategory	Single Hung
Compliance Method	Certification Mark or Listing
Certification Agency	American Architectural Manufacturers
Referenced Standard and Year (of	**Standard**

Standard)	ANSI/AAMA/NWWDA 101/I.S.2
Equivalence of Product Standards Certified By	
Product Approval Method	Method 1 Option A
Date Submitted	09/22/2005
Date Validated	10/14/2005
Date Pending FBC Approval	10/07/2005
Date Approved	10/17/2005

Summary of Products

Go to Page

FL #	Model, Number or Name	Description
5438.1	165 Triple with Continuous Head and Sill	106x72 Insulated SSB An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-20* DP-31.4 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** PTID_5438_I_165 SH Fla Fastener Schedule.pdf PTID_5438_I_650 SH Fla Fastener Schedule.pdf PTID_5438_I_740-744 S - Fastener Schedule.pdf PTID_5438_I_AAMA Cha Windows.pdf PTID_5438_I_Installatior BetterBilt Nail Fin Alum W PTID_5438_I_Installatior BetterBilt Nail Fin Vinyl W PTID_5438_I_Installatior Nail Fin Alum Windows.pd PTID_5438_I_Installatior Nail Fin Vinyl Windows.pd Verified By:
5438.2	165/3000 Fin Frame Oriel	47x89 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-30 DP-42.7 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:

5438.3	165/3000 Fin Frame Oriel	40x90 Insul SSB Annealed Fixed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35* DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.4	165/3000 Flange Frame Beveled Buck	53x72 Single Glazed 3/16
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.5	165/3000 Flange Frame Oriel	47x89 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-25 DP-34.7 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.6	165/3000 Flange Frame Oriel	36x88 Insulated SSB Ann Annealed Fixed
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35* DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.7	3540 Fin Frame	36x74 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-47 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.8	3540 Fin Frame	44x72 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:**		**Certification Agency Ce** **Installation Instruction**

Approved for use outside HVHZ: **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40 DP-47.2 Per manufacturers installation instructions.		Verified By:
5438.9	3540 Fin Frame Triple with Continuous Head and Sill	108x72 Insulated SSB An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35* DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.10	4340 Fin Frame	36x62 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40* DP-55 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.11	4340 Fin Frame	36x60 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-55 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.12	4340 Fin Frame	36x74 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-40* DP-47 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.13	4340 Fin Frame	36x72 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-40* DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.14	455 Fin Frame	48x84 Insulated DSB Ann

Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-50 DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.15	455 Fin Frame	54x90 Insulated DSB Ann
Limits of Use (See Other) ***Approved for use in HVHZ:*** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35 DP-50 Per manufacturers installation instructions.		**Certification Agency Ce** ***Installation Instruction*** Verified By:
5438.16	650 Fin Frame	53x90 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-30 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.17	650 Fin Oriel	48x84 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.18	650 Flange Frame	48x84 Insulated SSB Ann
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** LC-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:
5438.19	650 Flange Frame Oriel	48x84 Insulated 3/16" An
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-35 DP-47.2 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:

5438.20	740/3740 Fin Frame	52x71 Single Glazed DSB
Limits of Use (See Other) **Approved for use in HVHZ:** **Approved for use outside HVHZ:** **Impact Resistant:** **Design Pressure:** +/- **Other:** R-45 DP-45 Per manufacturers installation instructions.		**Certification Agency Ce** **Installation Instruction** Verified By:

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Back Next

DCA Administration

Department of Community Affairs
Florida Building Code Online
Codes and Standards
2555 Shumard Oak Boulevard
Tallahassee, Florida 32399-2100
(850) 487-1824, Suncom 277-1824, Fax (850) 414-8436

Product Approval Accepts:

Residential System Sizing Calculation

Summary

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

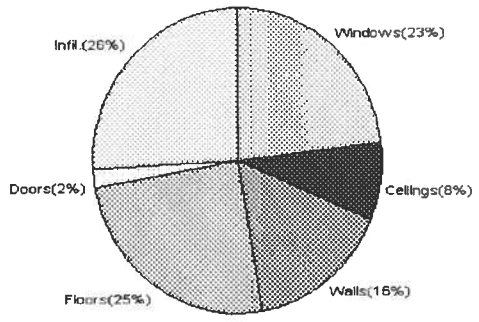
10/25/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**39613**	**Btuh**	**Total cooling load calculation**	**34439**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	116.1	46000	Sensible (SHR = 0.75)	122.6	34500
Heat Pump + Auxiliary(0.0kW)	116.1	46000	Latent	182.7	11500
			Total (Electric Heat Pump)	133.6	46000

WINTER CALCULATIONS

Winter Heating Load (for 2834 sqft)

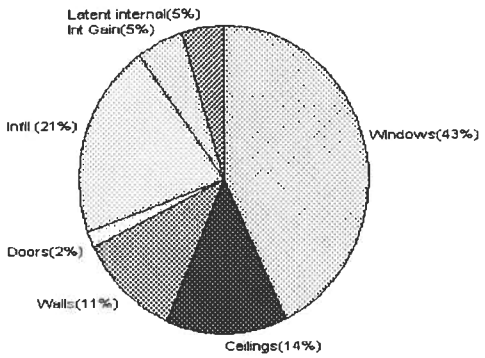
Load component			Load	
Window total	287	sqft	9248	Btuh
Wall total	1899	sqft	6235	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	2834	sqft	3339	Btuh
Floor total	227	sqft	9911	Btuh
Infiltration	249	cfm	10102	Btuh
Duct loss			0	Btuh
Subtotal			**39613**	**Btuh**
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			**39613**	**Btuh**

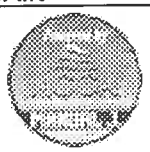


SUMMER CALCULATIONS

Summer Cooling Load (for 2834 sqft)

Load component			Load	
Window total	287	sqft	14784	Btuh
Wall total	1899	sqft	3847	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	2834	sqft	4693	Btuh
Floor total			0	Btuh
Infiltration	128	cfm	2391	Btuh
Internal gain			1840	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			**28144**	**Btuh**
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			4695	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occupants/other)			1600	Btuh
Total latent gain			**6295**	**Btuh**
TOTAL HEAT GAIN			**34439**	**Btuh**





For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 10-25-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

10/25/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	SW	60.0	32.2	1931 Btuh
4	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	NE	45.0	32.2	1449 Btuh
6	2, Clear, Metal, 0.87	NE	8.0	32.2	258 Btuh
7	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	SE	13.3	32.2	428 Btuh
9	2, Clear, Metal, 0.87	SE	20.0	32.2	644 Btuh
10	2, Clear, Metal, 0.87	SW	6.0	32.2	193 Btuh
11	2, Clear, Metal, 0.87	SW	10.0	32.2	322 Btuh
	Window Total		287(sqft)		9248 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1703	3.3	5592 Btuh
2	Frame - Wood - Adj(0.09)	13.0	196	3.3	644 Btuh
	Wall Total		1899		6235 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2834	1.2	3339 Btuh
	Ceiling Total		2834		3339Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	227.0 ft(p)	43.7	9911 Btuh
	Floor Total		227		9911 Btuh
			Zone Envelope Subtotal:		29511 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.66	22672	249.4	10102 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**39613 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

10/25/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	39613 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	39613 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

10/25/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	30.0	32.2	966 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	SW	60.0	32.2	1931 Btuh
4	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
5	2, Clear, Metal, 0.87	NE	45.0	32.2	1449 Btuh
6	2, Clear, Metal, 0.87	NE	8.0	32.2	258 Btuh
7	2, Clear, Metal, 0.87	SE	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	SE	13.3	32.2	428 Btuh
9	2, Clear, Metal, 0.87	SE	20.0	32.2	644 Btuh
10	2, Clear, Metal, 0.87	SW	6.0	32.2	193 Btuh
11	2, Clear, Metal, 0.87	SW	10.0	32.2	322 Btuh
	Window Total		287(sqft)		9248 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1703	3.3	5592 Btuh
2	Frame - Wood - Adj(0.09)	13.0	196	3.3	644 Btuh
	Wall Total		1899		6235 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	2834	1.2	3339 Btuh
	Ceiling Total		2834		3339Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	227.0 ft(p)	43.7	9911 Btuh
	Floor Total		227		9911 Btuh
			Zone Envelope Subtotal:		29511 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.66	22672	249.4	10102 Btuh
Ductload	Unsealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			**Sensible Zone Subtotal**		**39613 Btuh**

Manual J Winter Calculations

Residential Load - Component Details (continued)

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

10/25/2006

WHOLE HOUSE TOTALS

	Subtotal Sensible	39613 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	39613 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

John Thomas
Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 10/25/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	7ft.	5.5ft.	30.0	0.0	30.0	29	60	1801 Btuh
2	2, Clear, 0.87, None,N,N	NW	10ft.	5.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	60.0	24.3	35.7	29	63	2937 Btuh
4	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
5	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
6	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.5ft.	8.0	0.0	8.0	29	60	480 Btuh
7	2, Clear, 0.87, None,N,N	SE	8ft.	6ft.	30.0	30.0	0.0	29	63	869 Btuh
8	2, Clear, 0.87, None,N,N	SE	10ft.	7ft.	13.3	13.3	0.0	29	63	385 Btuh
9	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	20.0	8.1	11.9	29	63	979 Btuh
10	2, Clear, 0.87, None,N,N	SW	1.5ft.	3.5ft.	6.0	4.0	2.0	29	63	239 Btuh
11	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	10.0	4.0	6.0	29	63	489 Btuh
	Window Total				287 (sqft)					14784 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1702.7	2.1	3552 Btuh
2	Frame - Wood - Adj	13.0/0.09	196.0	1.5	296 Btuh
	Wall Total		1899 (sqft)		3847 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Adjacent	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2834.0	1.7	4693 Btuh
	Ceiling Total		2834 (sqft)		4693 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	227 (ft(p))	0.0	0 Btuh
	Floor Total		227.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		23913 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.34	22672	128.5	2391 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		8	X 230 +	0	1840 Btuh
Duct load	Unsealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		**28144 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

10/25/2006

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**28144 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**28144 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**28144 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	4695 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6295 Btuh**
	TOTAL GAIN	**34439 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

John Thomas
Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 10/25/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	7ft.	5.5ft.	30.0	0.0	30.0	29	60	1801 Btuh
2	2, Clear, 0.87, None,N,N	NW	10ft.	5.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	60.0	24.3	35.7	29	63	2937 Btuh
4	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
5	2, Clear, 0.87, None,N,N	NE	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
6	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.5ft.	8.0	0.0	8.0	29	60	480 Btuh
7	2, Clear, 0.87, None,N,N	SE	8ft.	6ft.	30.0	30.0	0.0	29	63	869 Btuh
8	2, Clear, 0.87, None,N,N	SE	10ft.	7ft.	13.3	13.3	0.0	29	63	385 Btuh
9	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	20.0	8.1	11.9	29	63	979 Btuh
10	2, Clear, 0.87, None,N,N	SW	1.5ft.	3.5ft.	6.0	4.0	2.0	29	63	239 Btuh
11	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	10.0	4.0	6.0	29	63	489 Btuh
	Window Total				287 (sqft)					14784 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1702.7	2.1	3552 Btuh
2	Frame - Wood - Adj	13.0/0.09	196.0	1.5	296 Btuh
	Wall Total		1899 (sqft)		3847 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Exterior	20.0	9.8	196 Btuh
2	Insulated - Adjacent	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	2834.0	1.7	4693 Btuh
	Ceiling Total		2834 (sqft)		4693 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	227 (ft(p))	0.0	0 Btuh
	Floor Total		227.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		23913 Btuh

Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.34	22672	128.5	2391 Btuh

Internal gain	Occupants	Btuh/occupant		Appliance	Load
	8	X 230	+	0	1840 Btuh

Duct load			
Unsealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh

	Sensible Zone Load	**28144 Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

Class 3 Rating
Registration No. 0
Climate: North

10/25/2006

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**28144 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**28144 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**28144 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	4695 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (8 people @ 200 Btuh per person)	1600 Btuh
	Latent other gain	0 Btuh
	Latent total gain	**6295 Btuh**
	TOTAL GAIN	**34439 Btuh**

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

John Thomas

Lake City, FL

Project Title:
609121JohnThomas

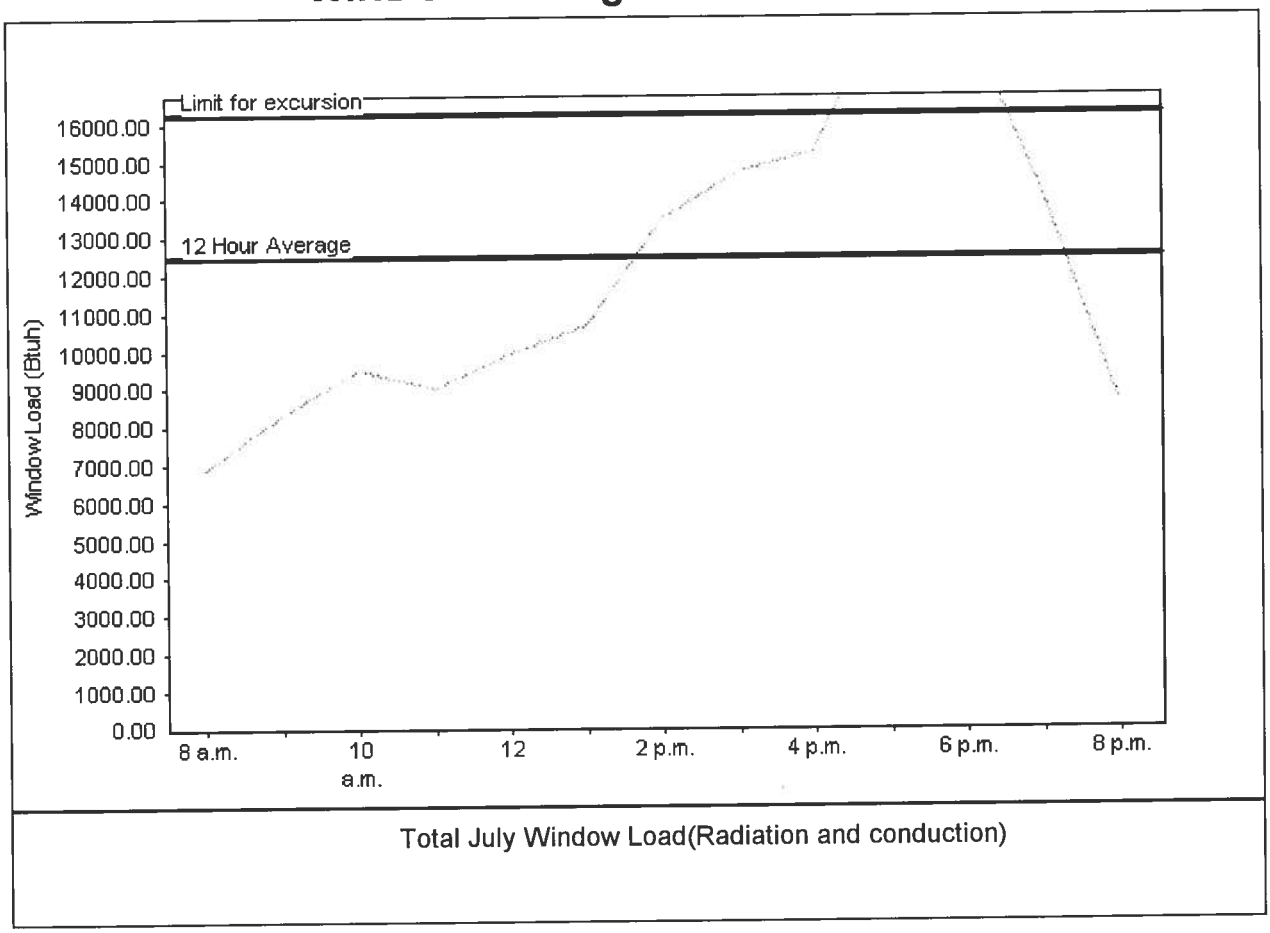
Class 3 Rating
Registration No. 0
Climate: North

10/25/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	12508 Btu
Summer setpoint	75 F	Peak window load for July	19514 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	16260 Btu
Latitude	29 North	Window excursion (July)	3254 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY: [signature]
DATE: 10-25-06

Project Information	L210709		
Builder:	CORNERSTONE	Date:	10/25/2006
Lot:	N/A	Start Number:	**1650**
Subdivision:	667 SW DYAL AVE	SEI Ref:	L210709
County or City:	COLUMBIA COUNTY		
Truss Page Count:	38		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
LAKE CITY,FLORIDA 32056 Designer: 183

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1025061650	10/25/2006				
2	CJ3	1025061651	10/25/2006				
3	CJ5	1025061652	10/25/2006				
4	EJ7	1025061653	10/25/2006				
5	EJ7A	1025061654	10/25/2006				
6	EJ7B	1025061655	10/25/2006				
7	EJ7C	1025061656	10/25/2006				
8	EJ8	1025061657	10/25/2006				
9	EJ8A	1025061658	10/25/2006				
10	EJ8G	1025061659	10/25/2006				
11	HJ9	1025061660	10/25/2006				
12	HJ9A	1025061661	10/25/2006				
13	PB01	1025061662	10/25/2006				
14	PB02	1025061663	10/25/2006				
15	T01	1025061664	10/25/2006				
16	T02	1025061665	10/25/2006				
17	T03	1025061666	10/25/2006				
18	T04	1025061667	10/25/2006				
19	T05	1025061668	10/25/2006				
20	T06	1025061669	10/25/2006				
21	T07	1025061670	10/25/2006				
22	T08	1025061671	10/25/2006				
23	T10	1025061672	10/25/2006				
24	T12	1025061673	10/25/2006				
25	T13	1025061674	10/25/2006				
26	T14	1025061675	10/25/2006				
27	T15	1025061676	10/25/2006				
28	T16	1025061677	10/25/2006				
29	T17	1025061678	10/25/2006				
30	T18	1025061679	10/25/2006				
31	T19	1025061680	10/25/2006				
32	T20	1025061681	10/25/2006				
33	T21	1025061682	10/25/2006				
34	T21G	1025061683	10/25/2006				
35	T23	1025061684	10/25/2006				
36	T24	1025061685	10/25/2006				
37	T25	1025061686	10/25/2006				
38	T26	1025061687	10/25/2006				

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10:00:13 AM

Licensee Details

Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN** (Primary Name) **BRYAN ZECHER CONSTRUCTION INC** (DBA
Main Address:	**P O BOX 815 LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST LAKE CITY, FL 32055 United States Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

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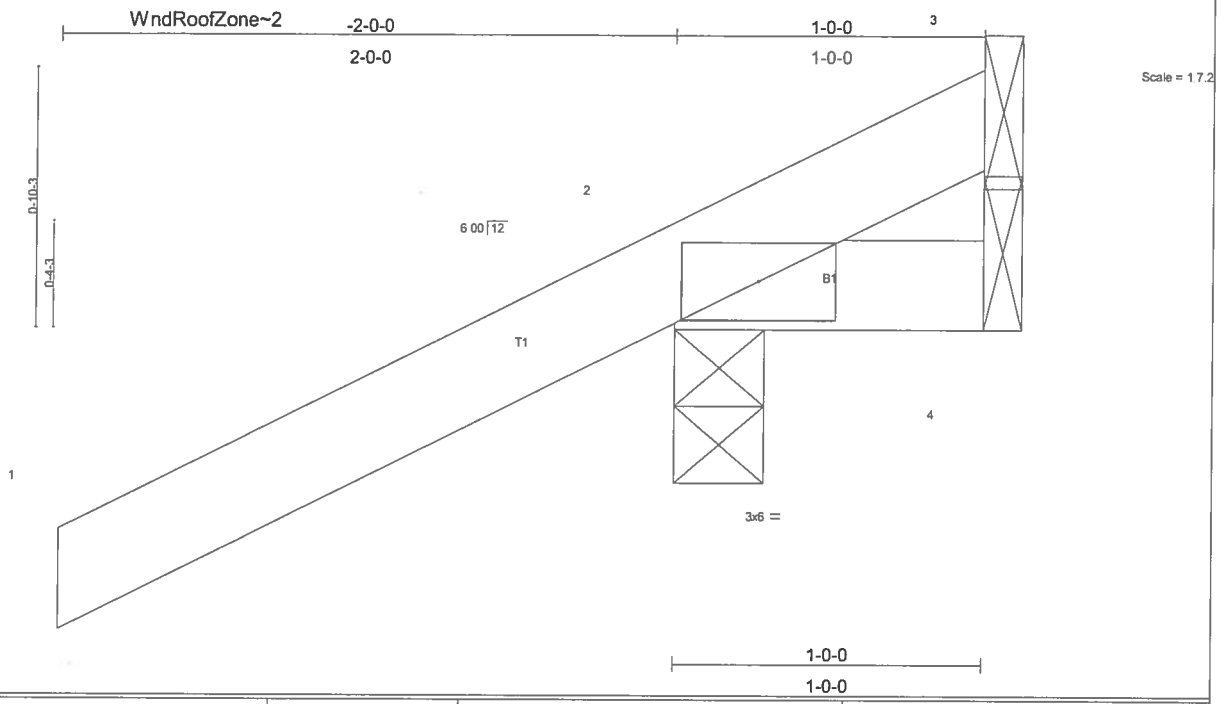
OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061650

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	CJ1	JACK	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Sep 27 14:43:48 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/84
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

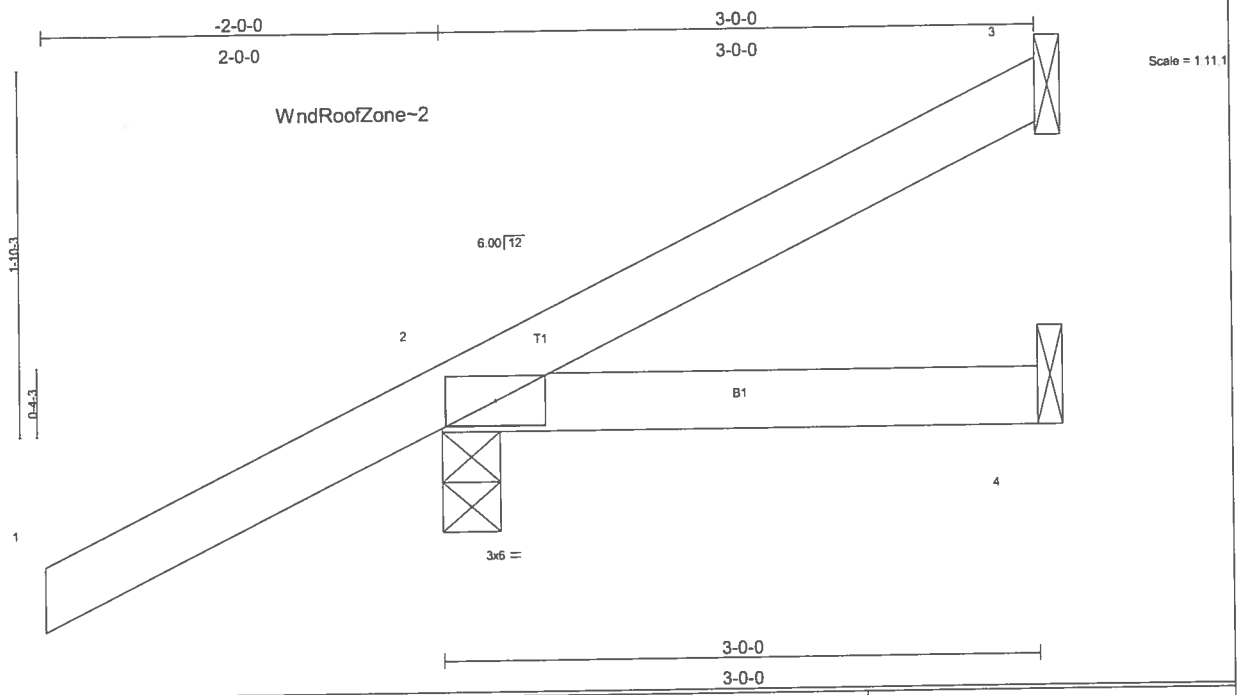
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061651

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	CJ3	JACK	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Sep 27 14:43:39 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

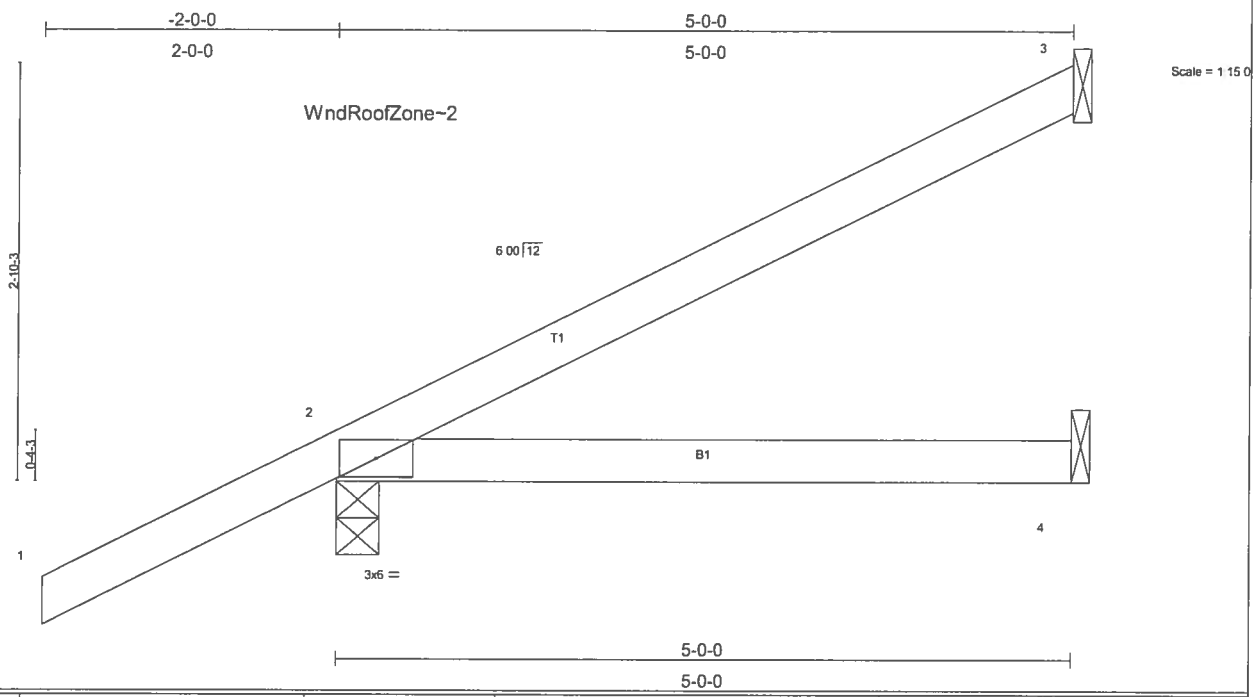
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061652

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	CJ5	JACK	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Sep 27 14:52:10 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-72/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061653

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ7	MONO TRUSS	42	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 15:46:17 2006 Page 1

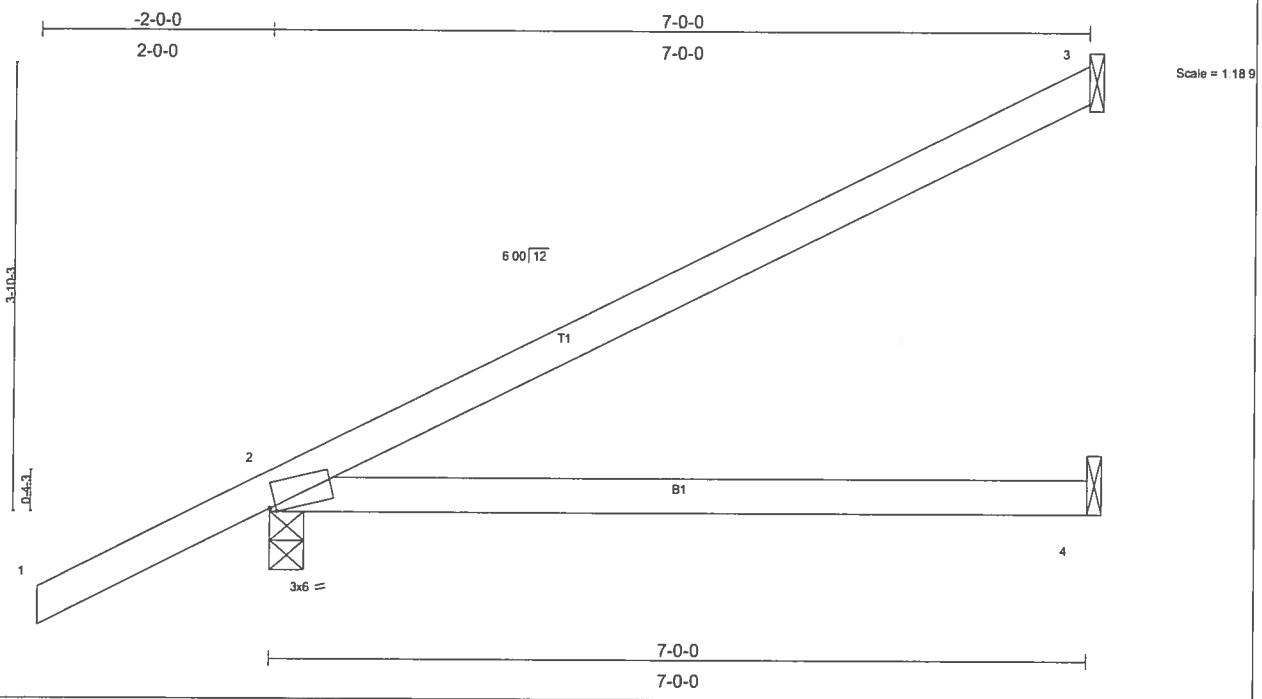


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.27	2-4	>305	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.22	2-4	>374	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-295(load case 5), 4=-68(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.75

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 295 lb uplift at joint 2 and 68 lb uplift at joint 4.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061654

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ7A	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 08:02:05 2006 Page 1

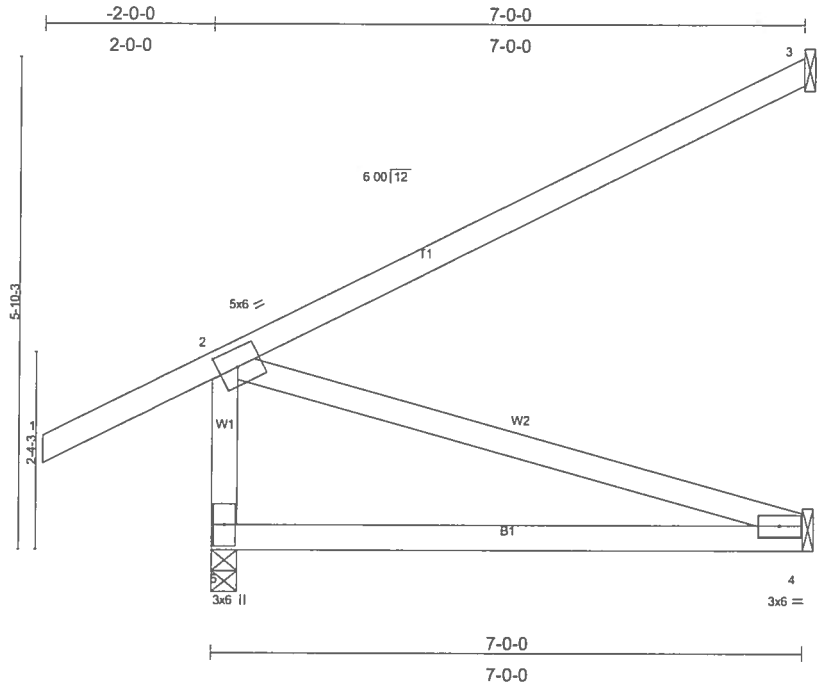


Plate Offsets (X,Y): [2:0-2-11,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.06	4-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.10	4-5	>802	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=142/Mechanical, 5=421/0-3-8, 4=126/Mechanical
Max Horz 5=226(load case 5)
Max Uplift3=-131(load case 5), 5=-144(load case 5), 4=-61(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-101/47, 2-5=-296/195
BOT CHORD 4-5=-256/145
WEBS 2-4=-152/268

JOINT STRESS INDEX
2 = 0.77, 4 = 0.08 and 5 = 0.45

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 3, 144 lb uplift at joint 5 and 61 lb uplift at joint 4.

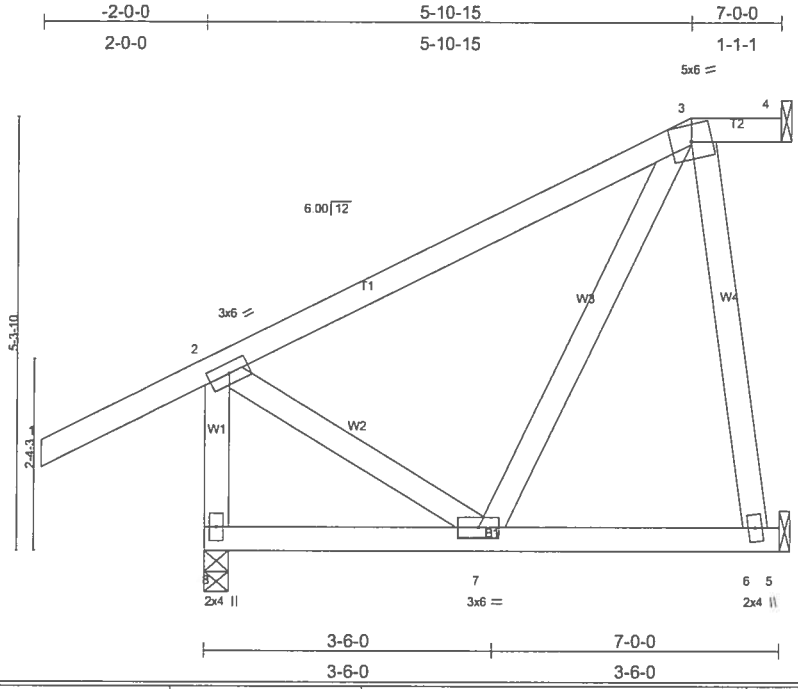
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061655

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
TEMP	EJ7B	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 08:04:33 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=-91/Mechanical, 8=408/0-3-8, 6=371/Mechanical
Max Horz 8=203(load case 5)
Max Uplift4=-91(load case 1), 8=-152(load case 5), 6=-262(load case 5)
Max Grav 4=92(load case 5), 8=408(load case 1), 6=371(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-170/20, 3-4=0/0, 2-8=-361/193
BOT CHORD 7-8=-193/26, 6-7=-44/53, 5-6=0/0
WEBS 3-7=-62/90, 3-6=-327/274, 2-7=0/145

JOINT STRESS INDEX
2 = 0.79, 3 = 0.53, 6 = 0.16, 7 = 0.07 and 8 = 0.26

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 4, 152 lb uplift at joint 8 and 262 lb uplift at joint 6.

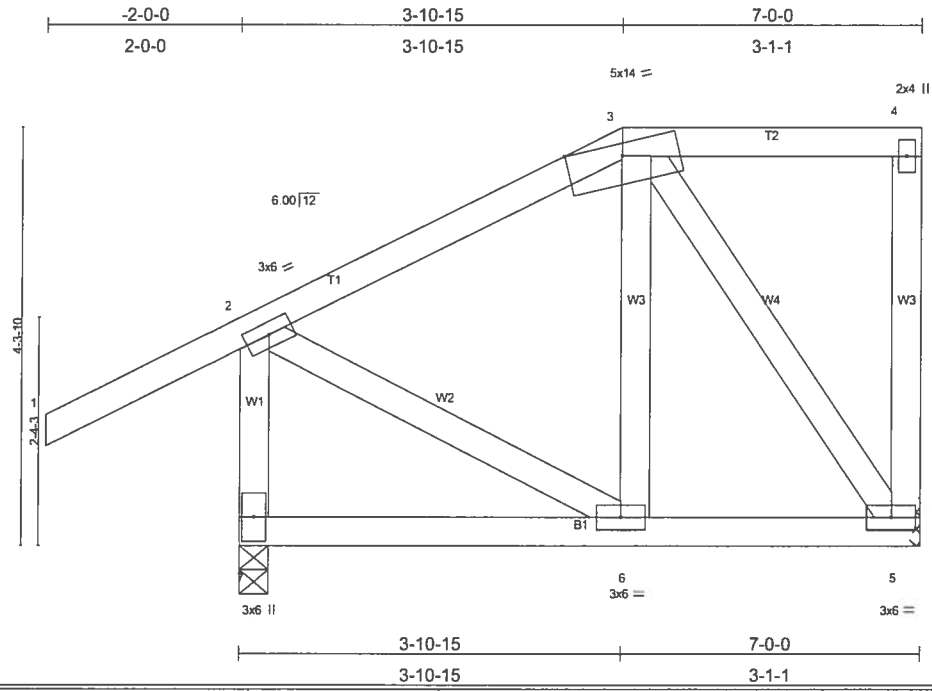
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061656

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ7C	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 08:05:36 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.03	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.05	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=671/Mechanical, 7=824/0-3-8
Max Horz 7=157(load case 4)
Max Uplift5=-267(load case 3), 7=-335(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/52, 2-3=-410/126, 3-4=-18/8, 4-5=-79/59, 2-7=-539/264
BOT CHORD 6-7=-140/58, 5-6=-147/333
WEBS 3-6=-153/429, 3-5=-532/239, 2-6=-53/294

JOINT STRESS INDEX
2 = 0.69, 3 = 0.16, 4 = 0.32, 5 = 0.23, 6 = 0.28 and 7 = 0.39

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 5 and 335 lb uplift at joint 7.
6) Girder carries tie-in span(s): 7-10-15 from 0-0-0 to 7-0-0
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-54, 3-4=-54, 5-7=-152(F=-122)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061657

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ8	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 08:06:26 2006 Page 1

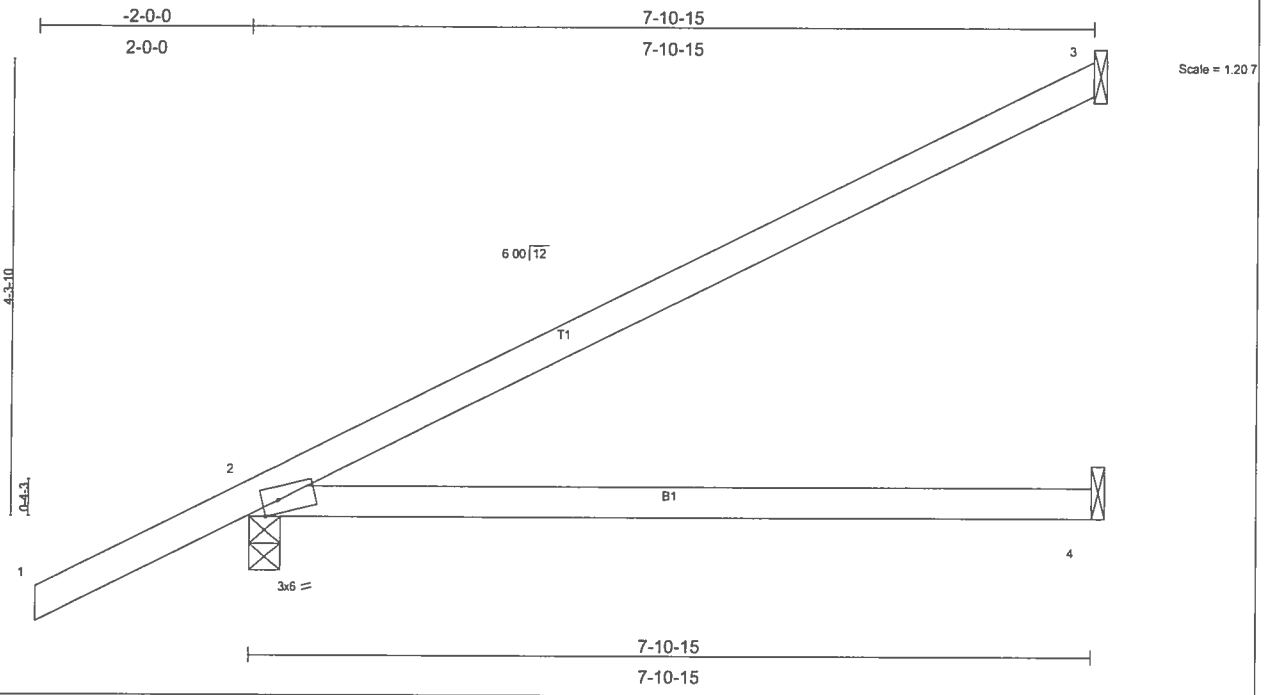


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.21	2-4	>433	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.36	2-4	>260	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=186/Mechanical, 2=456/0-3-8, 4=121/Mechanical
Max Horz 2=245(load case 5)
Max Uplift3=-154(load case 5), 2=-216(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-105/66
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.80

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 154 lb uplift at joint 3 and 216 lb uplift at joint 2.

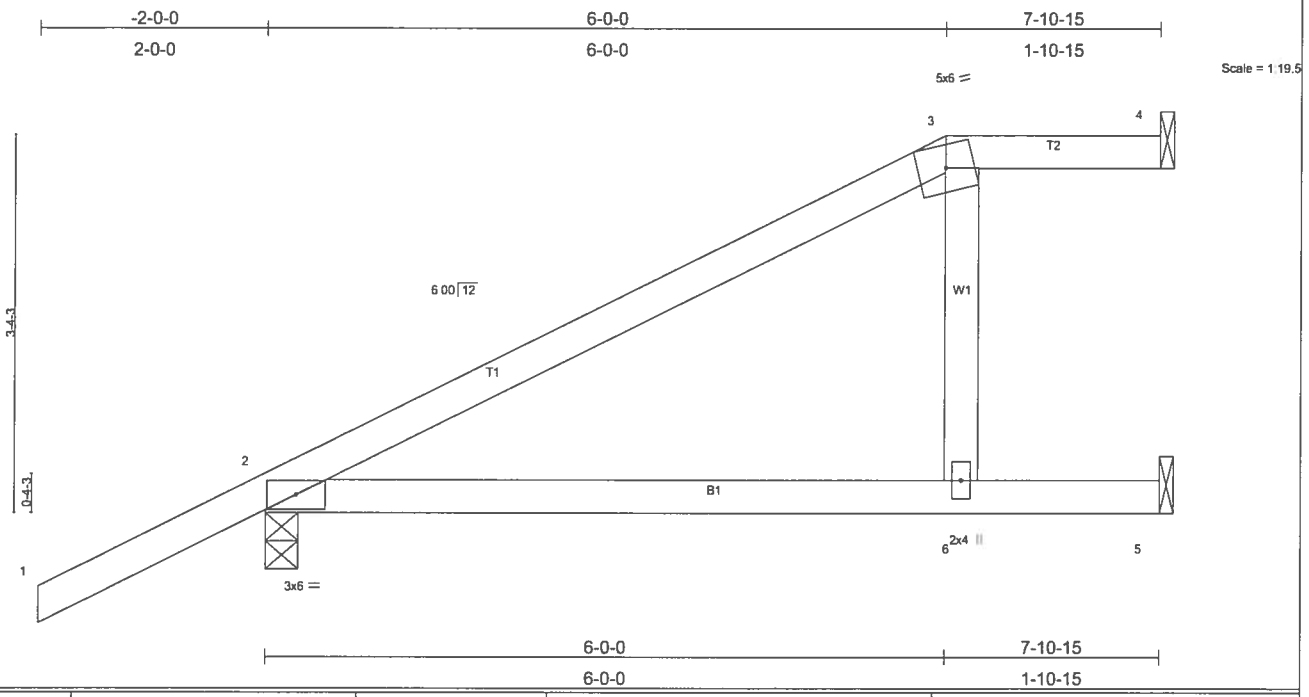
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061658

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ8A	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 08:07:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.25	2-6	>368	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.41	2-6	>227	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.14	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=130/Mechanical, 2=456/0-3-8, 5=177/Mechanical
Max Horz 2=203(load case 5)
Max Uplift4=-40(load case 4), 2=-232(load case 5), 5=-82(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-97/35, 3-4=-0/1
BOT CHORD 2-6=-6/4, 5-6=0/0
WEBS 3-6=-84/134

JOINT STRESS INDEX
2 = 0.35, 3 = 0.65 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 4, 232 lb uplift at joint 2 and 82 lb uplift at joint 5.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061659

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	EJ8G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industnes, Inc. Thu Sep 28 08:07:55 2006 Page 1

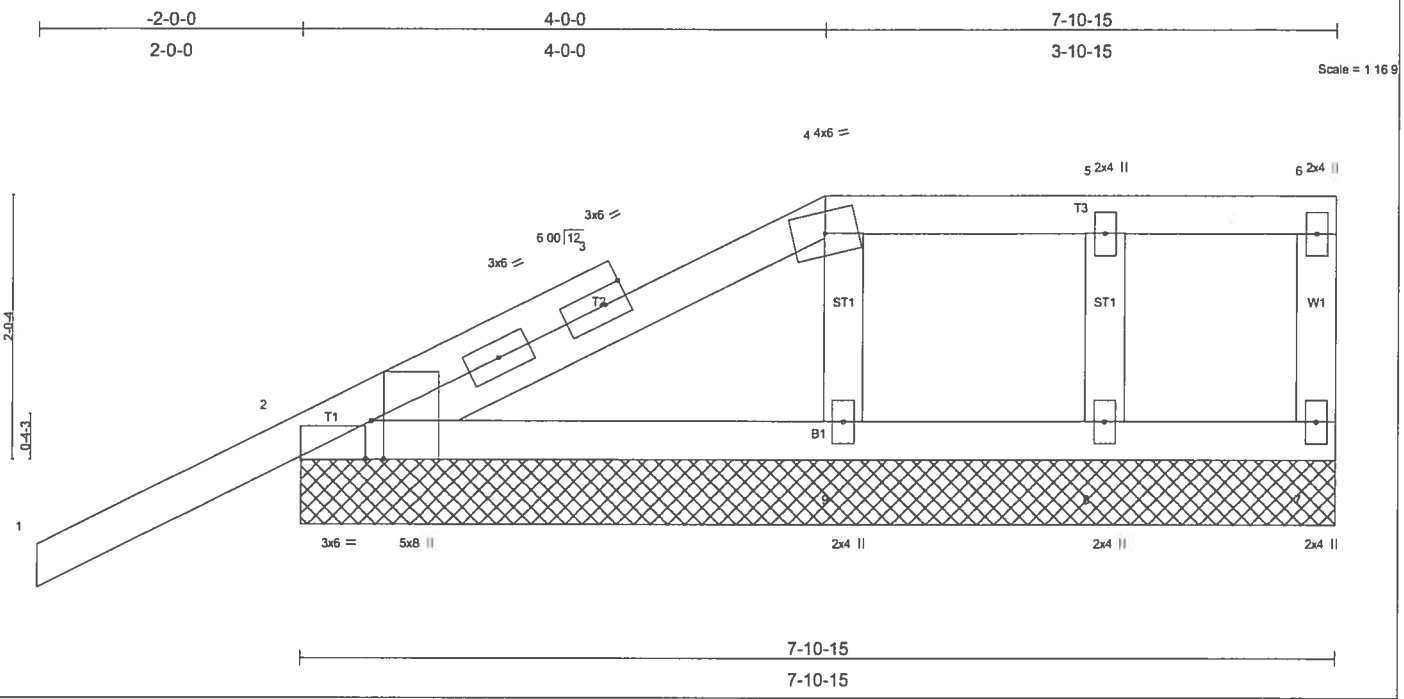


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.05	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.06	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=560/7-10-15, 7=107/7-10-15, 9=447/7-10-15, 8=232/7-10-15
Max Horz 2=142(load case 5)
Max Uplift2=-296(load case 5), 7=-37(load case 4), 9=-129(load case 5), 8=-120(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/99, 2-3=-131/16, 3-4=-29/52, 4-5=-8/3, 5-6=-8/3, 6-7=-82/42
BOT CHORD 2-9=-15/36, 8-9=-3/8, 7-8=-3/8
WEBS 4-9=-340/157, 5-8=-195/125

JOINT STRESS INDEX
2 = 0.67, 2 = 0.00, 3 = 0.00, 3 = 0.40, 3 = 0.40, 4 = 0.44, 5 = 0.07, 6 = 0.10, 7 = 0.05, 8 = 0.07 and 9 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) Provide adequate drainage to prevent water ponding.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 37 lb uplift at joint 7, 129 lb uplift at joint 9 and 120 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-6=-114(F=-60), 2-7=-30

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061660

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	HJ9	MONO TRUSS	2	1	Job Reference (optional)

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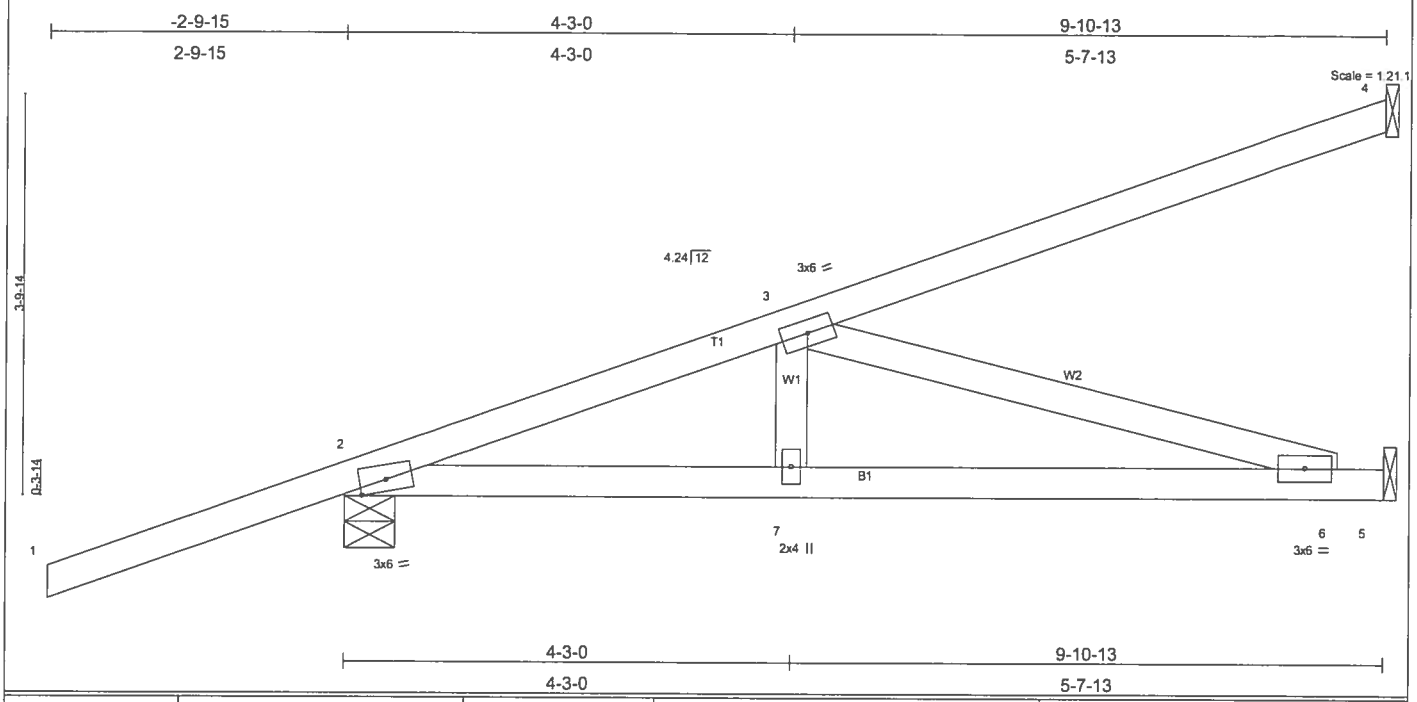

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.19	6-7	>623	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-281(load case 2), 5=-62(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-883/119, 3-4=-105/66
BOT CHORD 2-7=-308/816, 6-7=-308/816, 5-6=0/0
WEBS 3-7=0/188, 3-6=-851/321

JOINT STRESS INDEX
2 = 0.75, 3 = 0.22, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061661

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	HJ9A	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Sep 27 16:13:51 2006 Page 1

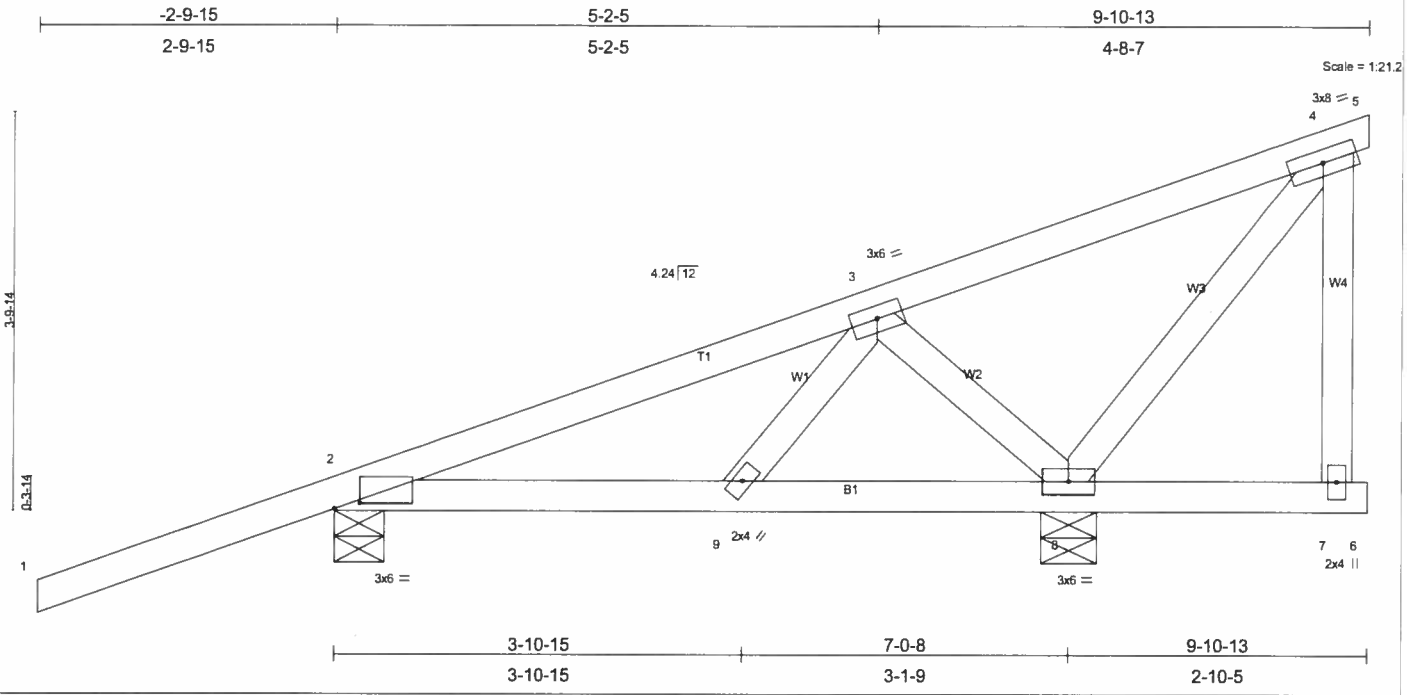

Plate Offsets (X,Y): [2:0-2-15,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.02	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	0.02	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.20	Horz(TL)	0.00	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=269/0-5-11, 8=904/0-6-7
Max Horz 2=270(load case 2)
Max Uplift2=-158(load case 2), 8=-417(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-326/90, 3-4=-305/414, 4-5=-9/0, 4-7=0/97
BOT CHORD 2-9=-45/127, 8-9=-65/70, 7-8=-33/42, 6-7=0/0
WEBS 3-9=0/115, 3-8=-355/201, 4-8=-594/368

JOINT STRESS INDEX
2 = 0.79, 3 = 0.12, 4 = 0.81, 7 = 0.45, 8 = 0.21 and 9 = 0.06

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 2 and 417 lb uplift at joint 8.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-130(F=-38, B=-38), 4=-90(F=-38, B=-38)-to-5=-94(F=-40, B=-40), 2=0(F=15, B=15)-to-6=-74(F=-22, B=-22)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061662

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	PB01	HIP PIGGYBACK	2	1	Job Reference (optional)

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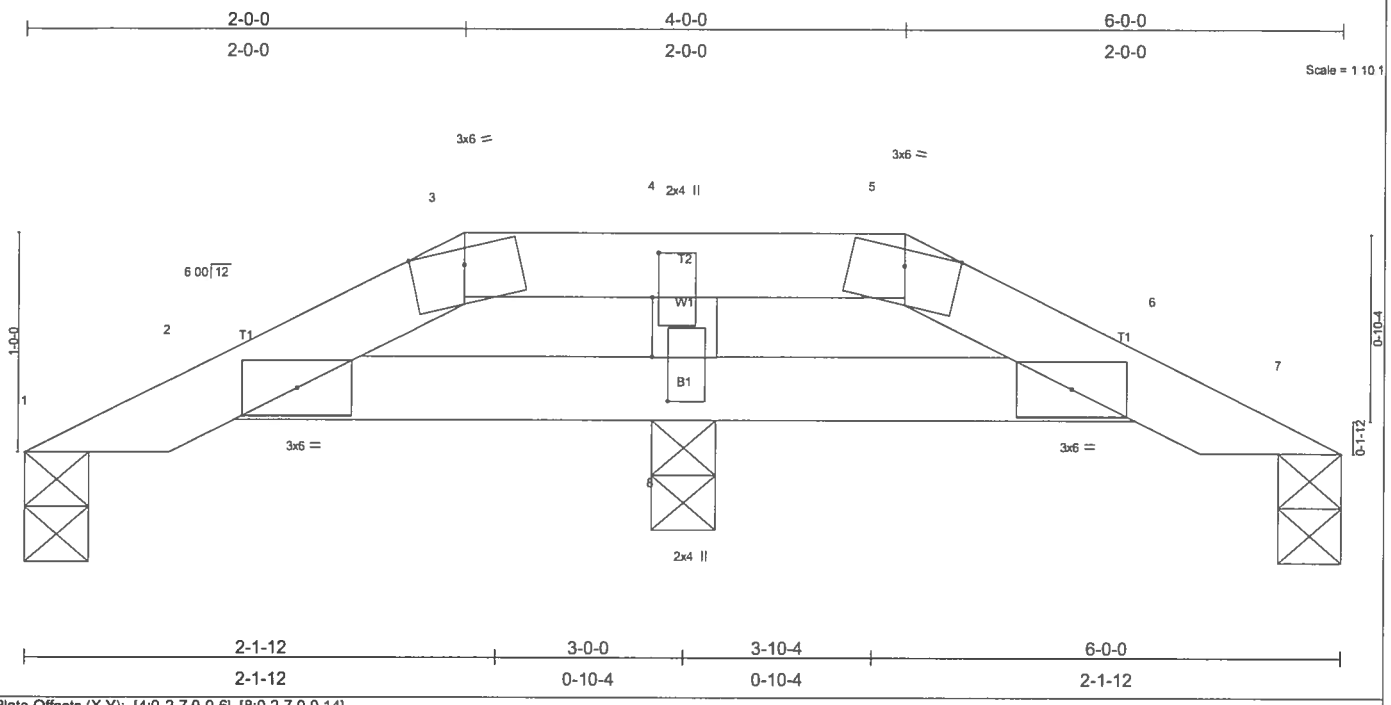

Plate Offsets (X,Y): [4:0-2-7,0-0-6], [8:0-2-7,0-0-14]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=68/0-3-8, 7=68/0-3-8, 8=313/0-3-8
Max Horz 1=14(load case 4)
Max Uplift1=-32(load case 5), 7=-36(load case 6), 8=-77(load case 5)
Max Grav 1=75(load case 9), 7=75(load case 10), 8=313(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-30/26, 2-3=-4/49, 3-4=0/37, 4-5=0/37, 5-6=-1/49, 6-7=-30/21
BOT CHORD 2-8=-37/29, 6-8=-37/29
WEBS 4-8=-189/87

JOINT STRESS INDEX
2 = 0.19, 3 = 0.08, 4 = 0.06, 5 = 0.08, 6 = 0.19 and 8 = 0.06

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1, 36 lb uplift at joint 7 and 77 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
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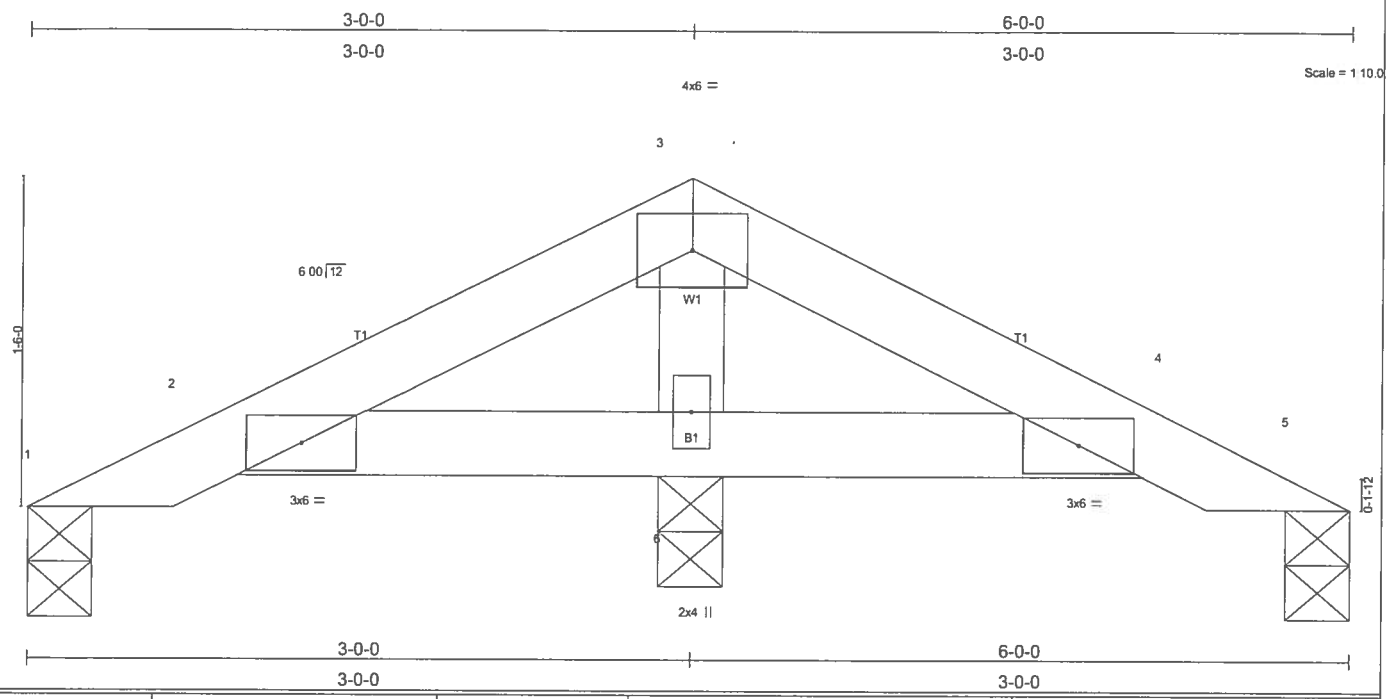
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061663

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	PB02	PIGGYBACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Wed Sep 27 16:16:40 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	-0.00	4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.00	4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 17 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=45/0-3-8, 5=45/0-3-8, 6=358/0-3-8
Max Horz 1=-21(load case 3)
Max Uplift1=-22(load case 5), 5=-30(load case 6), 6=-110(load case 5)
Max Grav 1=61(load case 9), 5=61(load case 10), 6=358(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-24/30, 2-3=-44/153, 3-4=-35/153, 4-5=-24/18
BOT CHORD 2-6=-107/62, 4-6=-107/62
WEBS 3-6=-252/105

JOINT STRESS INDEX
2 = 0.16, 3 = 0.17, 4 = 0.16 and 6 = 0.09

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1, 30 lb uplift at joint 5 and 110 lb uplift at joint 6.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061664

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T01	HIP	1	2	Job Reference (optional)

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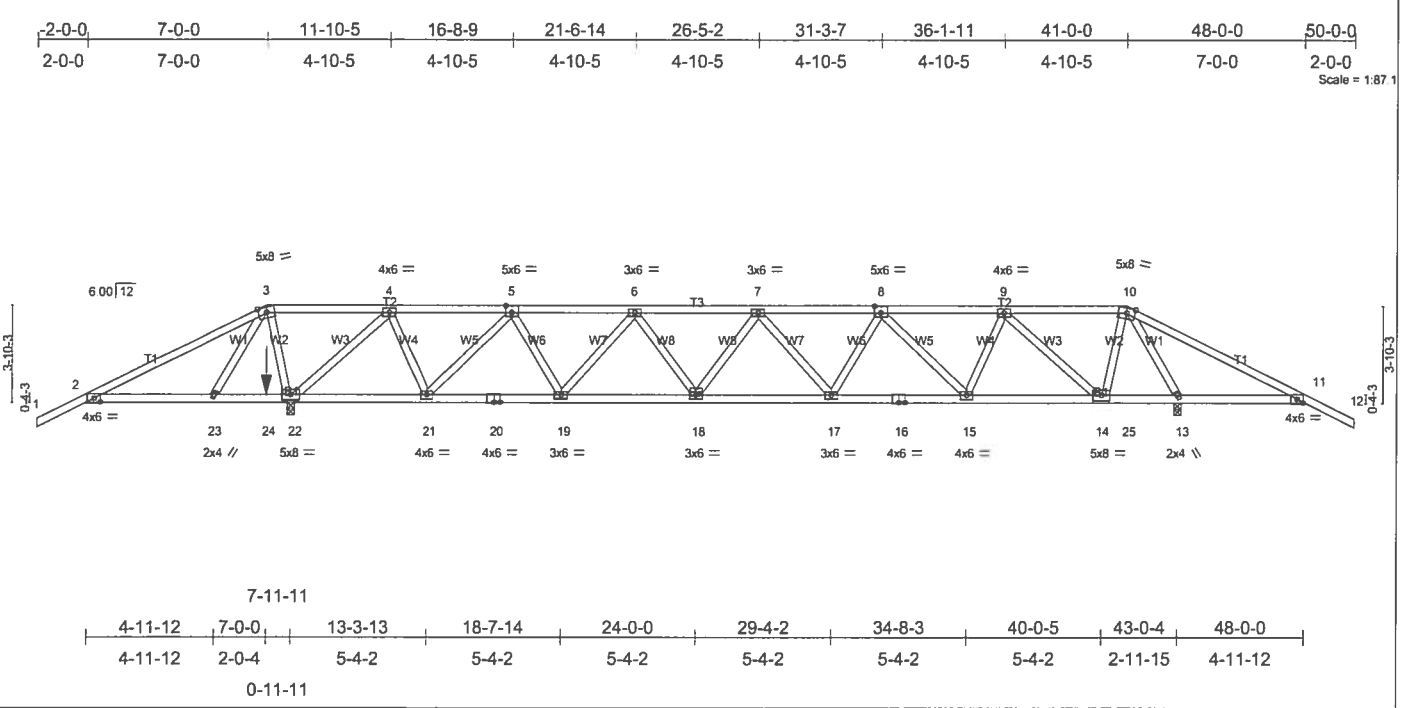


Plate Offsets (X,Y): [5:0-3-0,0-3-0], [8:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.32	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.51	17-18	>833	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.52	Horz(TL)	0.14	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 500 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 22=4631/0-3-8, 13=3388/0-3-8
Max Horz 22=87(load case 4)
Max Uplift22=-2125(load case 4), 13=-1489(load case 5)
Max Grav 22=4655(load case 8), 13=3445(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-589/791, 3-4=-791/1137, 4-5=-3335/1520, 5-6=-5922/2682, 6-7=-7076/3198, 7-8=-6805/3075, 8-9=-5163/2352, 9-10=-2171/1008, 10-11=-600/799, 11-12=0/47
BOT CHORD 2-23=-623/656, 23-24=-807/779, 22-24=-807/779, 21-22=-1094/2420, 20-21=-2336/5171, 19-20=-2336/5171, 18-19=-3053/6760, 17-18=-3232/7160, 16-17=-2875/6383, 15-16=-2875/6383, 14-15=-1996/4507, 14-25=-602/1463, 13-25=-602/1463, 11-13=-631/666
WEBS 3-23=-242/361, 3-22=-1168/843, 4-22=-4206/1978, 4-21=-937/2505, 5-21=-2780/1332, 5-19=-591/1655, 6-19=-1426/749, 6-18=-172/660, 7-18=-249/258, 7-17=-561/373, 8-17=-251/878, 8-15=-1844/923, 9-15=-647/1827, 9-14=-3275/1579, 10-14=-1047/2551, 10-13=-3793/1621

JOINT STRESS INDEX
2 = 0.19, 3 = 0.80, 4 = 0.76, 5 = 0.45, 6 = 0.41, 7 = 0.41, 8 = 0.45, 9 = 0.76, 10 = 0.80, 11 = 0.19, 13 = 0.85, 14 = 0.65, 15 = 0.76, 16 = 0.98, 17 = 0.66, 18 = 0.41, 19 = 0.66, 20 = 0.98, 21 = 0.76, 22 = 0.65 and 23 = 0.85

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2125 lb uplift at joint 22 and 1489 lb uplift at joint 13.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-10=-115(F=-61), 10-12=-54, 2-24=-30, 24-25=-64(F=-34), 11-25=-30
Concentrated Loads (lb)
Vert: 24=-539(F)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061665

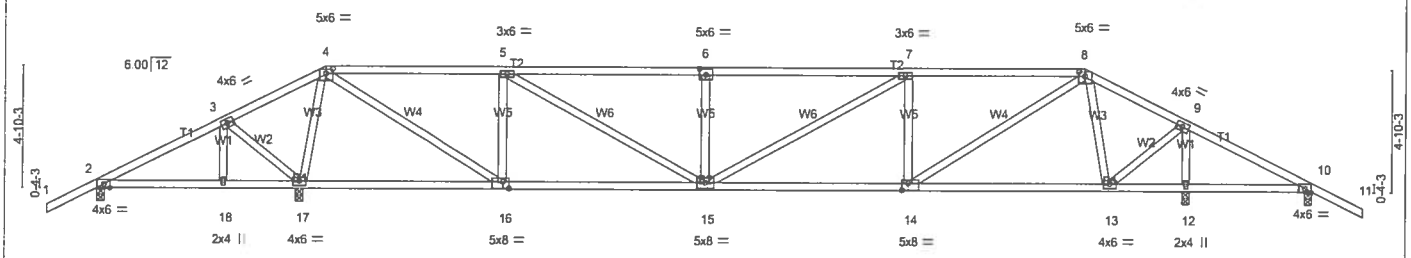
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
TEMP	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:21:38 2006 Page 1

-2-0-0	4-11-12	9-0-0	15-11-14	24-0-0	32-0-2	39-0-0	43-0-4	48-0-0	50-0-0
2-0-0	4-11-12	4-0-4	6-11-14	8-0-2	8-0-2	6-11-14	4-0-4	4-11-12	2-0-0

Scale = 1:87.1

* Uplift in gravity load case.



4-11-12	7-11-12	15-11-14	24-0-0	32-0-2	40-0-4	43-0-4	48-0-0
4-11-12	3-0-0	8-0-2	8-0-2	8-0-2	8-0-2	3-0-0	4-11-12

Plate Offsets (X,Y): [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [8:0-3-0,0-2-0], [14:0-3-0,0-3-0], [16:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.21	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.35	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.02	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.

REACTIONS (lb/size) 2=-180/0-3-8, 17=2489/0-3-8, 12=2185/0-3-8, 10=-254/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-414(load case 10), 17=-950(load case 4), 12=-767(load case 3), 10=-438(load case 9)
Max Grav 2=62(load case 4), 17=2489(load case 1), 12=2191(load case 10), 10=86(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-385/1188, 3-4=-496/1395, 4-5=-1109/482, 5-6=-2022/824, 6-7=-2022/824, 7-8=-1756/714, 8-9=-363/177, 9-10=-435/1280, 10-11=0/47
BOT CHORD 2-18=-1037/430, 17-18=-1037/430, 16-17=-742/365, 15-16=-352/1106, 14-15=-579/1755, 13-14=-102/496, 12-13=-1081/433, 10-12=-1081/433
WEBS 3-18=-146/91, 3-17=-320/294, 4-17=-2137/845, 4-16=-803/2177, 5-16=-943/490, 5-15=-394/1064, 6-15=-439/305, 7-15=-131/314, 7-14=-578/360, 8-14=-570/1516, 8-13=-991/429, 9-13=-564/1779, 9-12=-2076/695

JOINT STRESS INDEX
2 = 0.38, 3 = 0.84, 4 = 0.78, 5 = 0.62, 6 = 0.66, 7 = 0.62, 8 = 0.78, 9 = 0.84, 10 = 0.38, 12 = 0.76, 13 = 0.75, 14 = 0.89, 15 = 0.49, 16 = 0.89, 17 = 0.75 and 18 = 0.76

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 414 lb uplift at joint 2, 950 lb uplift at joint 17, 767 lb uplift at joint 12 and 438 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

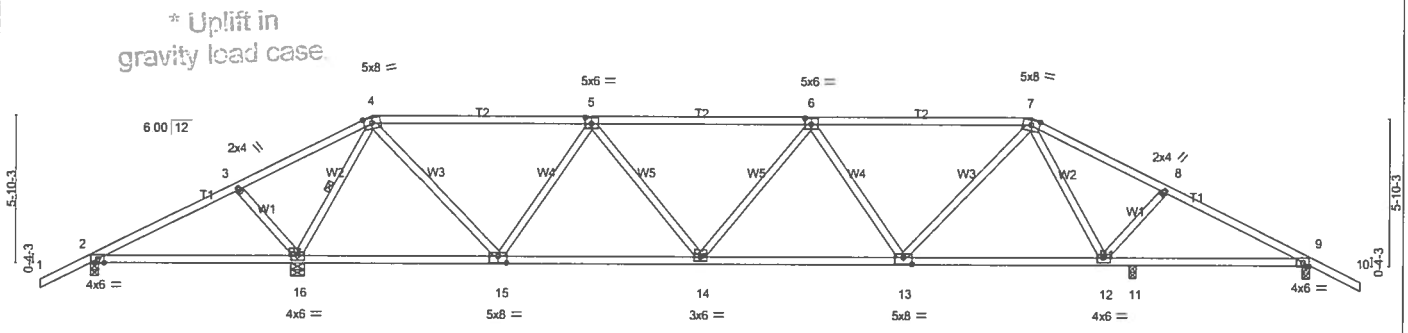
Dwg.#1025061666

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T03	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:25:08 2006 Page 1

-2-0-0	5-9-4	11-0-0	19-8-0	28-4-0	37-0-0	42-2-12	48-0-0	50-0-0
2-0-0	5-9-4	5-2-12	8-8-0	8-8-0	8-8-0	5-2-12	5-9-4	2-0-0

Scale = 1:87.1



8-1-4	16-0-10	24-0-0	31-11-6	39-10-12	41-0-4	48-0-0
8-1-4	7-11-6	7-11-6	7-11-6	7-11-6	1-1-8	6-11-12

Plate Offsets (X,Y): [5:0-3-0,0-3-0], [6:0-3-0,0-3-0], [13:0-4-0,0-3-0], [15:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.29	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	0.13	2-16	>746	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 243 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-16

REACTIONS (lb/size) 2=-144/0-3-8, 16=2586/0-6-7, 9=1034/0-3-8, 11=763/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-377(load case 10), 16=-879(load case 4), 9=-482(load case 6), 11=-361(load case 3)
Max Grav 2=14(load case 4), 16=2586(load case 1), 9=1035(load case 10), 11=763(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-356/1186, 3-4=-367/1345, 4-5=-1015/414, 5-6=-1956/738, 6-7=-1953/691, 7-8=-1470/429, 8-9=-1651/462, 9-10=0/47
BOT CHORD 2-16=-1031/417, 15-16=-39/190, 14-15=-539/1616, 13-14=-660/2122, 12-13=-348/1442, 11-12=-296/1394, 9-11=-296/1394
WEBS 3-16=-283/260, 4-16=-2378/797, 4-15=-471/1564, 5-15=-1102/491, 5-14=-91/556, 6-14=-272/152, 6-13=-377/283, 7-13=-271/821, 7-12=-437/293, 8-12=-191/232

JOINT STRESS INDEX
2 = 0.64, 3 = 0.34, 4 = 0.57, 5 = 0.80, 6 = 0.80, 7 = 0.57, 8 = 0.34, 9 = 0.64, 12 = 0.67, 13 = 0.75, 14 = 0.40, 15 = 0.75 and 16 = 0.67

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 377 lb uplift at joint 2, 879 lb uplift at joint 16, 482 lb uplift at joint 9 and 361 lb uplift at joint 11.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

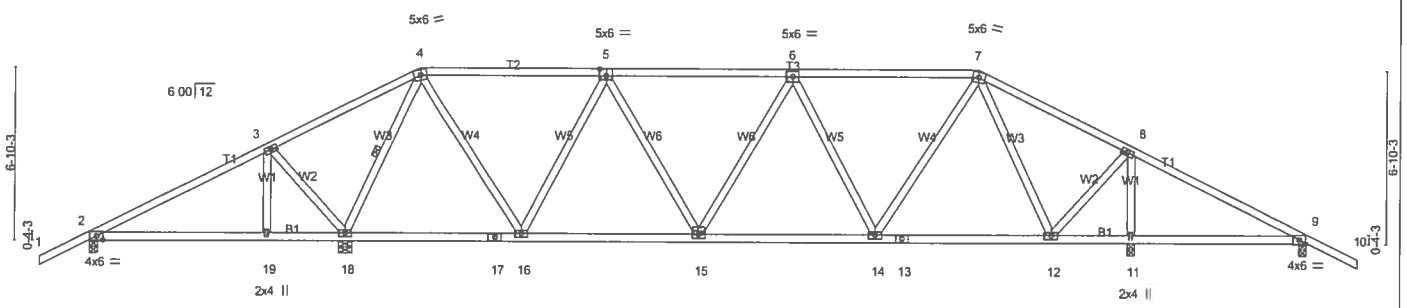
Dwg.#1025061667

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:34:11 2006 Page 1

2-0-0	6-11-12	13-0-0	20-4-0	27-8-0	35-0-0	41-0-4	48-0-0	50-0-0
2-0-0	6-11-12	6-0-4	7-4-0	7-4-0	7-4-0	6-0-4	6-11-12	2-0-0

Scale = 1:87.1



6-11-12	10-1-4	17-0-10	24-0-0	30-11-6	37-10-12	41-0-4	48-0-0
6-11-12	3-1-8	6-11-6	6-11-6	6-11-6	6-11-6	3-1-8	6-11-12

Plate Offsets (X,Y): [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.09	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.14	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 263 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-18

REACTIONS (lb/size) 2=276/0-3-8, 18=2014/0-6-7, 11=1747/0-3-8, 9=202/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-315(load case 5), 18=-741(load case 4), 11=-578(load case 3), 9=-291(load case 6)
Max Grav 2=284(load case 9), 18=2014(load case 1), 11=1751(load case 10), 9=204(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-82/350, 3-4=-194/653, 4-5=-712/321, 5-6=-1200/503, 6-7=-1066/443, 7-8=-530/249, 8-9=-123/457, 9-10=0/47
BOT CHORD 2-19=-288/167, 18-19=-288/167, 17-18=0/193, 16-17=0/193, 15-16=-319/1052, 14-15=-372/1235, 13-14=-151/719, 12-13=-151/719, 11-12=-337/181, 9-11=-337/181
WEBS 3-19=-218/172, 3-18=-499/436, 4-18=-1642/591, 4-16=-329/1052, 5-16=-759/362, 5-15=-40/312, 6-15=-79/88, 6-14=-408/246, 7-14=-211/688, 7-12=-771/295, 8-12=-278/1101, 8-11=-1560/471

JOINT STRESS INDEX
2 = 0.31, 3 = 0.86, 4 = 0.63, 5 = 0.58, 6 = 0.24, 7 = 0.63, 8 = 0.86, 9 = 0.31, 11 = 0.57, 12 = 0.73, 13 = 0.30, 14 = 0.81, 15 = 0.44, 16 = 0.81, 17 = 0.30, 18 = 0.73 and 19 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2, 741 lb uplift at joint 18, 578 lb uplift at joint 11 and 291 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

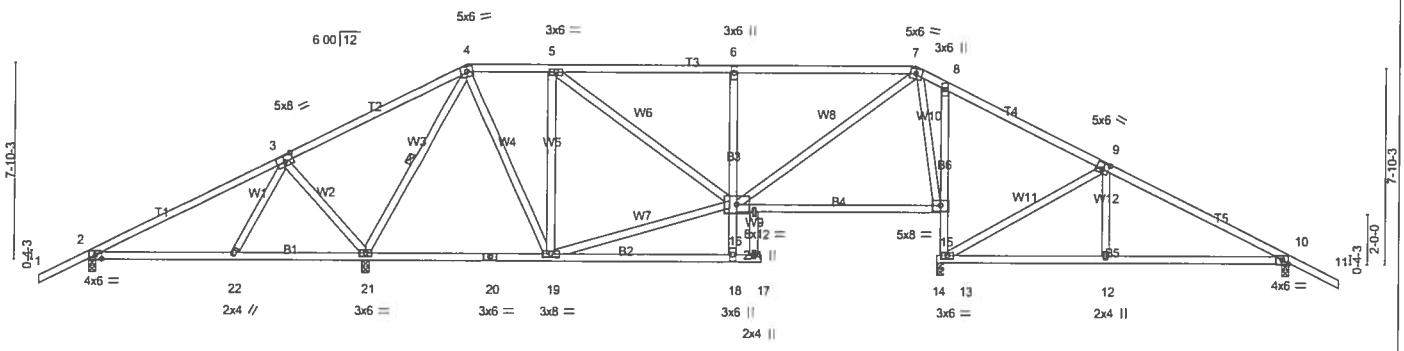
Dwg.#1025061668

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:42:28 2006 Page 1

-2-0-0	7-9-4	15-0-0	18-5-2	25-10-0	33-0-0	33-10-8	40-8-0	48-0-0	50-0-0
2-0-0	7-9-4	7-2-12	3-5-2	7-4-14	7-2-0	0-10-8	6-9-8	7-4-0	2-0-0

Scale = 1:88.3



5-9-6	11-0-4	18-5-2	25-10-0	26-10-0	33-10-8	34-0-4	40-8-0	48-0-0
5-9-6	5-2-14	7-4-14	7-4-14	1-0-0	7-0-8	0-1-12	6-7-12	7-4-0

Plate Offsets (X,Y): [3:0-4-0,0-3-0], [9:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.11	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.03	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 289 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-21

REACTIONS (lb/size) 2=413/0-3-8, 13=1570/0-3-8, 10=649/0-3-8, 21=1650/0-3-8
Max Horz 2=194(load case 5)
Max Uplift2=-340(load case 5), 13=-493(load case 3), 10=-606(load case 6), 21=-711(load case 5)
Max Grav 2=414(load case 9), 13=1572(load case 10), 10=649(load case 10), 21=1657(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-197/210, 3-4=-152/457, 4-5=-510/288, 5-6=-851/443, 6-7=-865/440, 7-8=0/382, 8-9=0/325, 9-10=-670/669, 10-11=0/47
BOT CHORD 2-22=-141/99, 21-22=-105/98, 20-21=-56/226, 19-20=-56/226, 18-19=-13/8, 17-18=0/0, 16-18=0/137, 6-16=-423/293, 15-16=-103/135, 13-15=-1143/254, 8-15=-145/161, 13-14=0/0, 12-13=-434/516, 10-12=-437/522
WEBS 3-22=-240/206, 3-21=-527/453, 4-21=-1206/461, 4-19=-285/718, 5-19=-561/338, 16-19=-151/521, 5-16=-199/431, 7-16=-264/933, 7-15=-904/350, 9-13=-687/557, 9-12=-265/252

JOINT STRESS INDEX
2 = 0.25, 3 = 0.71, 4 = 0.52, 5 = 0.35, 6 = 0.16, 7 = 0.60, 8 = 0.20, 9 = 0.81, 10 = 0.31, 12 = 0.34, 13 = 0.49, 15 = 0.63, 16 = 0.27, 18 = 0.30, 19 = 0.83, 20 = 0.18, 21 = 0.49, 22 = 0.34, 23 = 0.34 and 24 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 340 lb uplift at joint 2, 493 lb uplift at joint 13, 606 lb uplift at joint 10 and 711 lb uplift at joint 21.

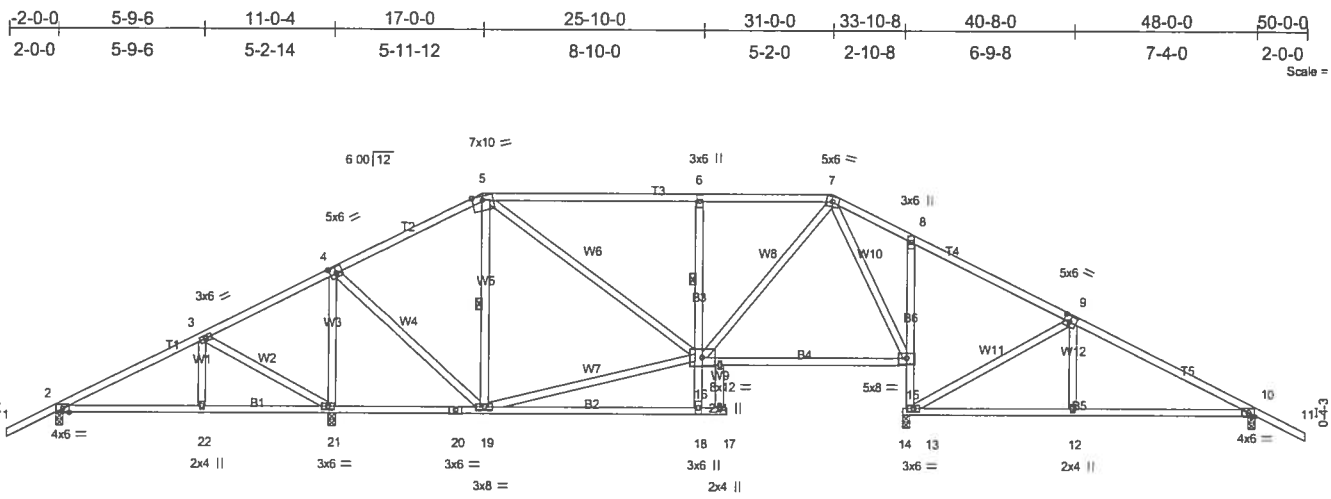
LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061669

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:45:10 2006 Page 1



5-9-6 11-0-4 17-0-0 25-10-0 26-10-0 33-10-8 34-0-4 40-8-0 48-0-0
5-9-6 5-2-14 5-11-12 8-10-0 1-0-0 7-0-8 0-1-12 6-7-12 7-4-0

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.12	18-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.20	18-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.03	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 293 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 6-16
WEBS 1 Row at midpt 5-19

REACTIONS (lb/size) 2=464/0-3-8, 13=1590/0-3-8, 10=651/0-3-8, 21=1577/0-3-8
Max Horz 2=208(load case 5)
Max Uplift2=-351(load case 5), 13=-457(load case 6), 10=-606(load case 6), 21=-712(load case 5)
Max Grav 2=467(load case 9), 13=1590(load case 1), 10=652(load case 10), 21=1577(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-353/274, 3-4=-132/290, 4-5=-551/284, 5-6=-750/410, 6-7=-748/399, 7-8=0/421, 8-9=-6/324, 9-10=-676/669, 10-11=0/47
BOT CHORD 2-22=-218/255, 21-22=-218/255, 20-21=-188/225, 19-20=-188/225, 18-19=0/26, 17-18=0/0, 16-18=0/162, 6-16=-431/299, 15-16=-122/295, 13-15=-1159/216, 8-15=-268/230, 13-14=0/0, 12-13=-433/522, 10-12=-437/528
WEBS 3-22=-197/173, 3-21=-521/426, 4-21=-1168/477, 4-19=-270/852, 5-19=-440/265, 16-19=-119/418, 5-16=-208/424, 7-16=-196/718, 7-15=-841/202, 9-13=-687/556, 9-12=-265/251

JOINT STRESS INDEX
2 = 0.31, 3 = 0.41, 4 = 0.42, 5 = 0.87, 6 = 0.24, 7 = 0.30, 8 = 0.20, 9 = 0.68, 10 = 0.31, 12 = 0.34, 13 = 0.50, 15 = 0.65, 16 = 0.28, 18 = 0.50, 19 = 0.84, 20 = 0.20, 21 = 0.38, 22 = 0.34, 23 = 0.34 and 24 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 351 lb uplift at joint 2, 457 lb uplift at joint 13, 606 lb uplift at joint 10 and 712 lb uplift at joint 21.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

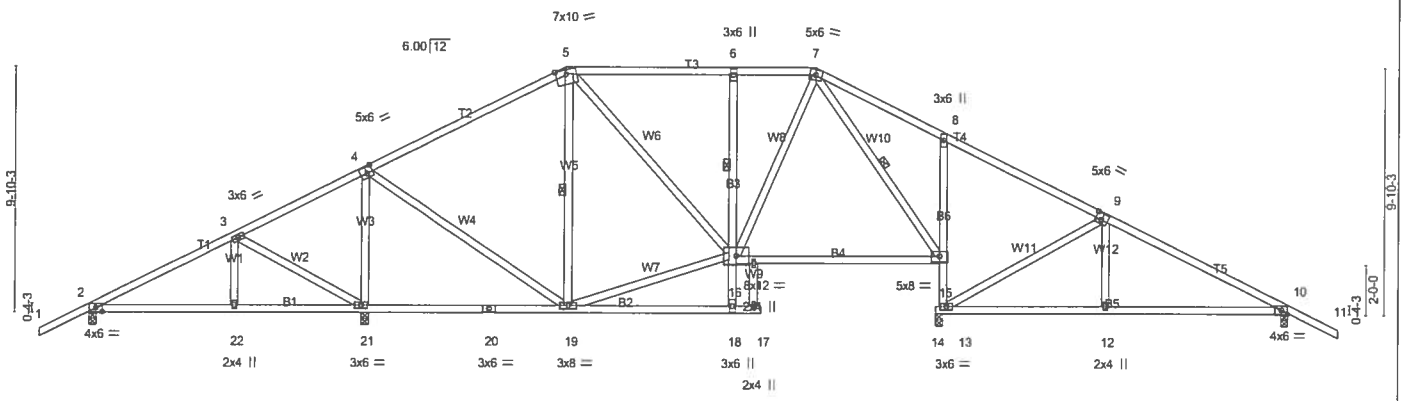
Dwg.#1025061670

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 09:47:35 2006 Page 1

-2-0-0	5-9-6	11-0-4	19-0-0	25-10-0	29-0-0	33-10-8	40-8-0	48-0-0	50-0-0
2-0-0	5-9-6	5-2-14	7-11-12	6-10-0	3-2-0	4-10-8	6-9-8	7-4-0	2-0-0

Scale = 1:88.3



5-9-6	11-0-4	19-0-0	25-10-0	26-10-0	33-10-8	34-0-4	40-8-0	48-0-0
5-9-6	5-2-14	7-11-12	6-10-0	1-0-0	7-0-8	0-1-12	6-7-12	7-4-0

Plate Offsets (X,Y): [4:0-2-12,0-3-0], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.11	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.17	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.03	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 297 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3, B6 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
1 Row at midpt 6-16
WEBS 1 Row at midpt 5-19, 7-15

REACTIONS (lb/size) 2=482/0-3-8, 13=1589/0-3-8, 10=657/0-3-8, 21=1554/0-3-8
Max Horz 2=222(load case 5)
Max Uplift2=-356(load case 5), 13=-481(load case 6), 10=-606(load case 6), 21=-718(load case 5)
Max Grav 2=485(load case 9), 13=1589(load case 1), 10=658(load case 10), 21=1554(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-379/304, 3-4=-116/222, 4-5=-648/333, 5-6=-642/381, 6-7=-645/377, 7-8=-23/459, 8-9=-23/325, 9-10=-690/669, 10-11=0/47
BOT CHORD 2-22=-236/276, 21-22=-236/276, 20-21=-135/204, 19-20=-135/204, 18-19=-33/0, 17-18=0/0, 16-18=0/118, 6-16=-294/208, 15-16=-113/435, 13-15=-1164/175, 8-15=-329/284, 13-14=0/0, 12-13=-440/541, 10-12=-440/541
WEBS 3-22=-205/157, 3-21=-462/406, 4-21=-1116/511, 4-19=-206/763, 5-19=-352/200, 16-19=-106/542, 5-16=-128/291, 7-16=-128/561, 7-15=-852/128, 9-13=-686/558, 9-12=-264/248

JOINT STRESS INDEX
2 = 0.29, 3 = 0.41, 4 = 0.76, 5 = 0.89, 6 = 0.16, 7 = 0.30, 8 = 0.16, 9 = 0.67, 10 = 0.31, 12 = 0.34, 13 = 0.50, 15 = 0.64, 16 = 0.31, 18 = 0.17, 19 = 0.73, 20 = 0.32, 21 = 0.36, 22 = 0.34, 23 = 0.34 and 24 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 356 lb uplift at joint 2, 481 lb uplift at joint 13, 606 lb uplift at joint 10 and 718 lb uplift at joint 21.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

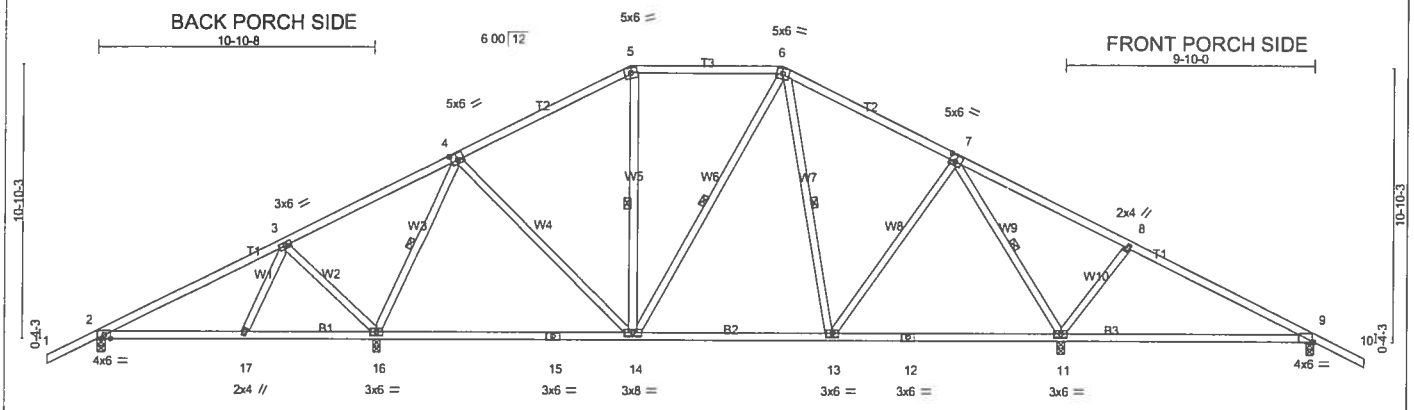
Dwg.#1025061671

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
TEMP	T08	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 15:44:54 2006 Page 1

-2-0-0	7-4-5	14-2-3	21-0-0	27-0-0	33-9-13	40-7-11	48-0-0	50-0-0
2-0-0	7-4-5	6-9-13	6-9-13	6-0-0	6-9-13	6-9-13	7-4-5	2-0-0

Scale = 1:87.1



5-9-6	11-0-4	21-0-0	29-0-4	38-0-4	48-0-0
5-9-6	5-2-14	9-11-12	8-0-4	9-0-0	9-11-12

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-0-12,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.45	9-11	>260	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.37	9-11	>319	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 275 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-16, 5-14, 6-14, 6-13, 7-11

REACTIONS (lb/size) 2=453/0-3-8, 16=1725/0-3-8, 11=1625/0-3-8, 9=436/0-3-8
Max Horz 2=-186(load case 6)
Max Uplift2=-339(load case 5), 16=-749(load case 5), 11=-660(load case 6), 9=-364(load case 6)
Max Grav 2=463(load case 9), 16=1725(load case 1), 11=1625(load case 1), 9=442(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-298/255, 3-4=-126/342, 4-5=-752/340, 5-6=-596/346, 6-7=-754/362, 7-8=0/258, 8-9=-142/87, 9-10=0/47
BOT CHORD 2-17=-142/190, 16-17=-78/123, 15-16=0/256, 14-15=0/256, 13-14=-11/604, 12-13=0/421, 11-12=0/421, 9-11=-10/77
WEBS 3-17=-243/166, 3-16=-481/431, 4-16=-1228/479, 4-14=-60/483, 5-14=-101/115, 6-14=-107/95, 6-13=-47/90, 7-13=-24/334, 7-11=-1156/352, 8-11=-382/339

JOINT STRESS INDEX
2 = 0.21, 3 = 0.41, 4 = 0.75, 5 = 0.54, 6 = 0.45, 7 = 0.72, 8 = 0.34, 9 = 0.71, 11 = 0.45, 12 = 0.33, 13 = 0.42, 14 = 0.60, 15 = 0.41, 16 = 0.53 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 2, 749 lb uplift at joint 16, 660 lb uplift at joint 11 and 364 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

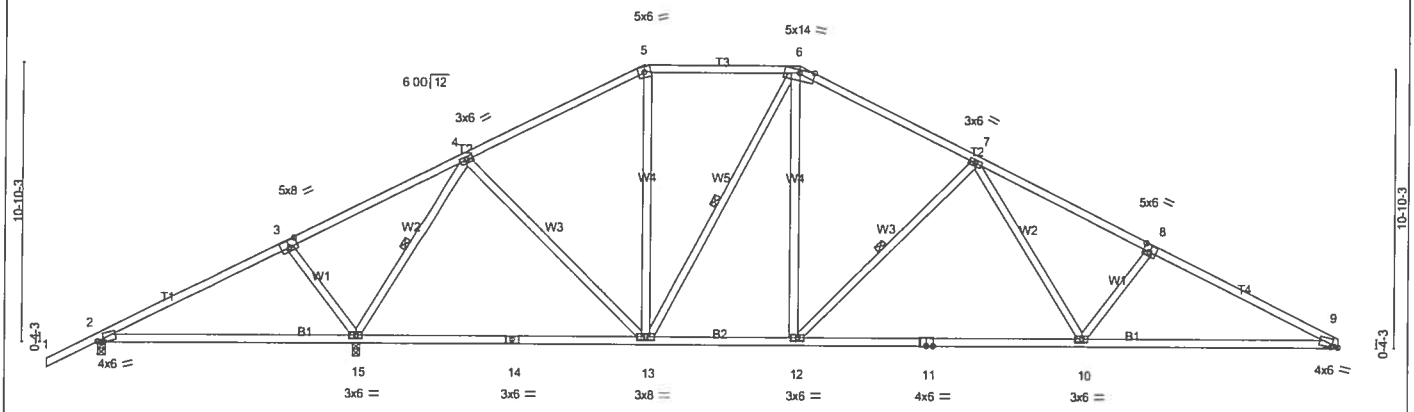
Dwg.#1025061672

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
TEMP	T10	HIP	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 10:16:46 2006 Page 1

-2-0-0	7-4-5	14-2-3	21-0-0	27-0-0	33-9-13	40-7-11	48-0-0
2-0-0	7-4-5	6-9-13	6-9-13	6-0-0	6-9-13	6-9-13	7-4-5

Scale = 1:85.5



9-11-12	21-0-0	27-0-0	38-0-4	48-0-0
9-11-12	11-0-4	6-0-0	11-0-4	9-11-12

Plate Offsets (X,Y): [2:0-2-13,Edge], [3:0-4-0,0-3-0], [8:0-3-0,0-3-0], [9:0-3-1,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.69	Vert(LL)	0.47	2-15	>253	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	0.40	2-15	>293	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	-0.09	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 267 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-15, 6-13, 7-12

REACTIONS (lb/size) 2=159/0-3-8, 15=2473/0-3-8, 9=1491/Mechanical
Max Horz 9=210(load case 5)
Max Uplift2=-313(load case 5), 15=-839(load case 5), 9=-491(load case 6)
Max Grav 2=263(load case 9), 15=2473(load case 1), 9=1491(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-180/620, 3-4=-193/848, 4-5=-1166/448, 5-6=-970/465, 6-7=-1474/561, 7-8=-2508/870, 8-9=-2723/895
BOT CHORD 2-15=-513/222, 14-15=0/372, 13-14=0/372, 12-13=-206/1254, 11-12=-521/1782, 10-11=-521/1782, 9-10=-827/2372
WEBS 3-15=-389/344, 4-15=-2089/635, 4-13=-162/853, 5-13=-51/213, 6-13=-664/273, 6-12=-285/791, 7-12=-771/457, 7-10=-233/756, 8-10=-356/318

JOINT STRESS INDEX
2 = 0.86, 3 = 0.70, 4 = 0.71, 5 = 0.57, 6 = 0.99, 7 = 0.51, 8 = 0.71, 9 = 0.87, 10 = 0.59, 11 = 0.87, 12 = 0.52, 13 = 0.84, 14 = 0.72 and 15 = 0.82

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 839 lb uplift at joint 15 and 491 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061673

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T12	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 10:21:56 2006 Page 1

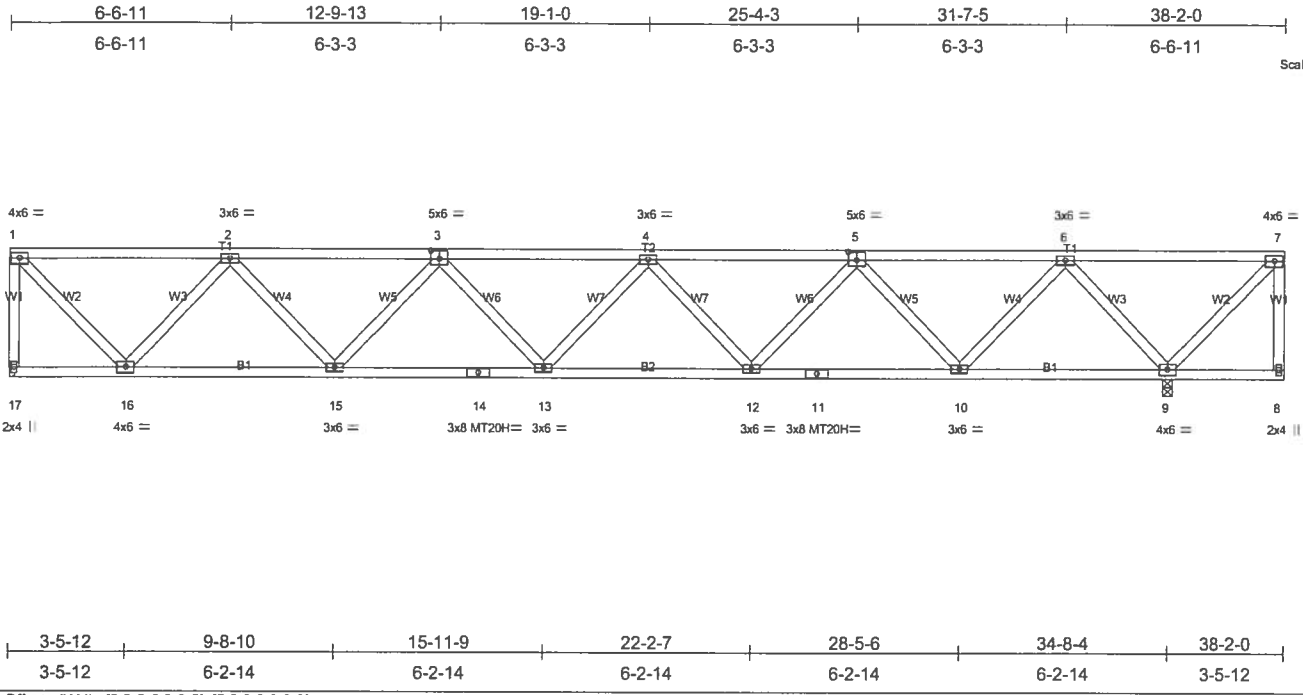


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.32	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.51	12-13	>810	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.13	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 408 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-4-7 oc bracing.

REACTIONS (lb/size) 17=2994/Mechanical, 9=3634/0-3-8
Max Uplift17=-1130(load case 2), 9=-1372(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-17=-2884/1102, 1-2=-2502/921, 2-3=-5813/2171, 3-4=-7155/2678, 4-5=-6595/2466, 5-6=-4131/1536, 6-7=-132/287, 7-8=-16/75
BOT CHORD 16-17=-53/106, 15-16=-1706/4446, 14-15=-2579/6771, 13-14=-2579/6771, 12-13=-2733/7174, 11-12=-2154/5644, 10-11=-2154/5644, 9-10=-861/2208, 8-9=-45/84
WEBS 1-16=-1269/3502, 2-16=-2970/1199, 2-15=-700/2056, 3-15=-1458/622, 3-13=-148/580, 4-13=-29/84, 4-12=-878/404, 5-12=-472/1436, 5-10=-2303/941, 6-10=-1015/2892, 6-9=-3813/1517, 7-9=-542/258

JOINT STRESS INDEX
1 = 0.78, 2 = 0.94, 3 = 0.53, 4 = 0.38, 5 = 0.53, 6 = 0.94, 7 = 0.78, 8 = 0.74, 9 = 0.78, 10 = 0.94, 11 = 0.86, 12 = 0.47, 13 = 0.47, 14 = 0.86, 15 = 0.94, 16 = 0.78 and 17 = 0.74

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1130 lb uplift at joint 17 and 1372 lb uplift at joint 9.
9) Girder carries tie-in span(s): 4-0-2 from 0-0-0 to 38-2-0; 4-9-14 from 0-0-0 to 38-2-0

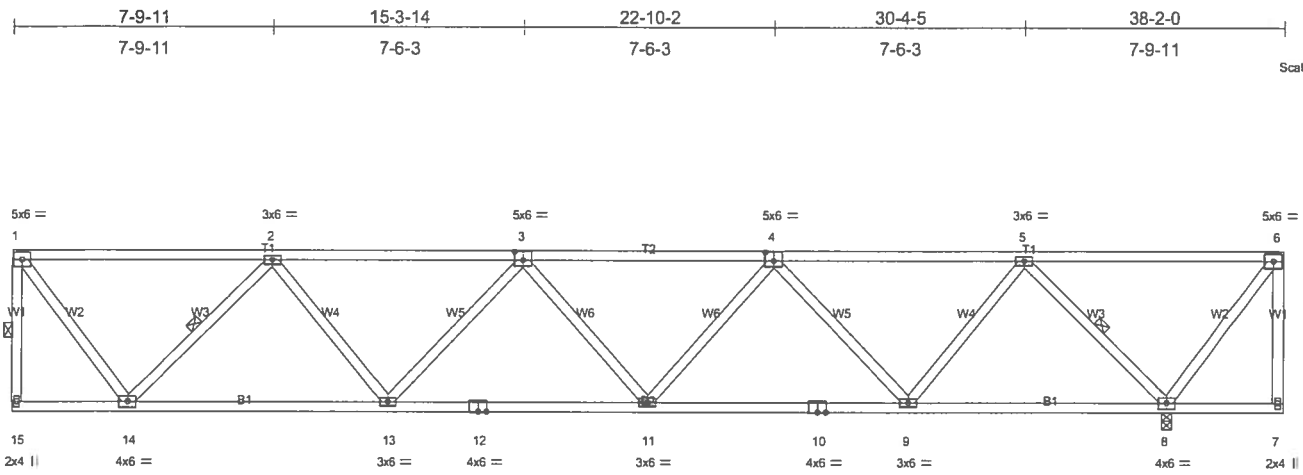
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-108(F=-54), 8-17=-67(F=-37)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061674

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 10:24:25 2006 Page 1



3-5-12	11-3-6	19-1-0	26-10-10	34-8-4	38-2-0
3-5-12	7-9-10	7-9-10	7-9-10	7-9-10	3-5-12

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.23	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.37	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-10 oc bracing.
WEBS 1 Row at midpt 1-15, 2-14, 5-8

REACTIONS (lb/size) 15=1437/Mechanical, 8=1744/0-3-8
Max Uplift15=-543(load case 3), 8=-659(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-15=-1397/530, 1-2=-932/325, 2-3=-2378/870, 3-4=-2667/979, 4-5=-1852/671, 5-6=-71/114, 6-7=-8/26
BOT CHORD 14-15=-35/52, 13-14=-754/1916, 12-13=-1046/2699, 11-12=-1046/2699, 10-11=-950/2445, 9-10=-950/2445, 8-9=-468/1160, 7-8=-33/45
WEBS 1-14=-490/1492, 2-14=-1427/623, 2-13=-191/762, 3-13=-484/266, 3-11=-51/106, 4-11=-45/348, 4-9=-893/420, 5-9=-334/1142, 5-8=-1849/782, 6-8=-270/175

JOINT STRESS INDEX
1 = 0.75, 2 = 0.80, 3 = 0.61, 4 = 0.61, 5 = 0.80, 6 = 0.75, 7 = 0.77, 8 = 0.74, 9 = 0.80, 10 = 0.86, 11 = 0.39, 12 = 0.86, 13 = 0.80, 14 = 0.74 and 15 = 0.77

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 543 lb uplift at joint 15 and 659 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061675

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 10:25:13 2006 Page 1

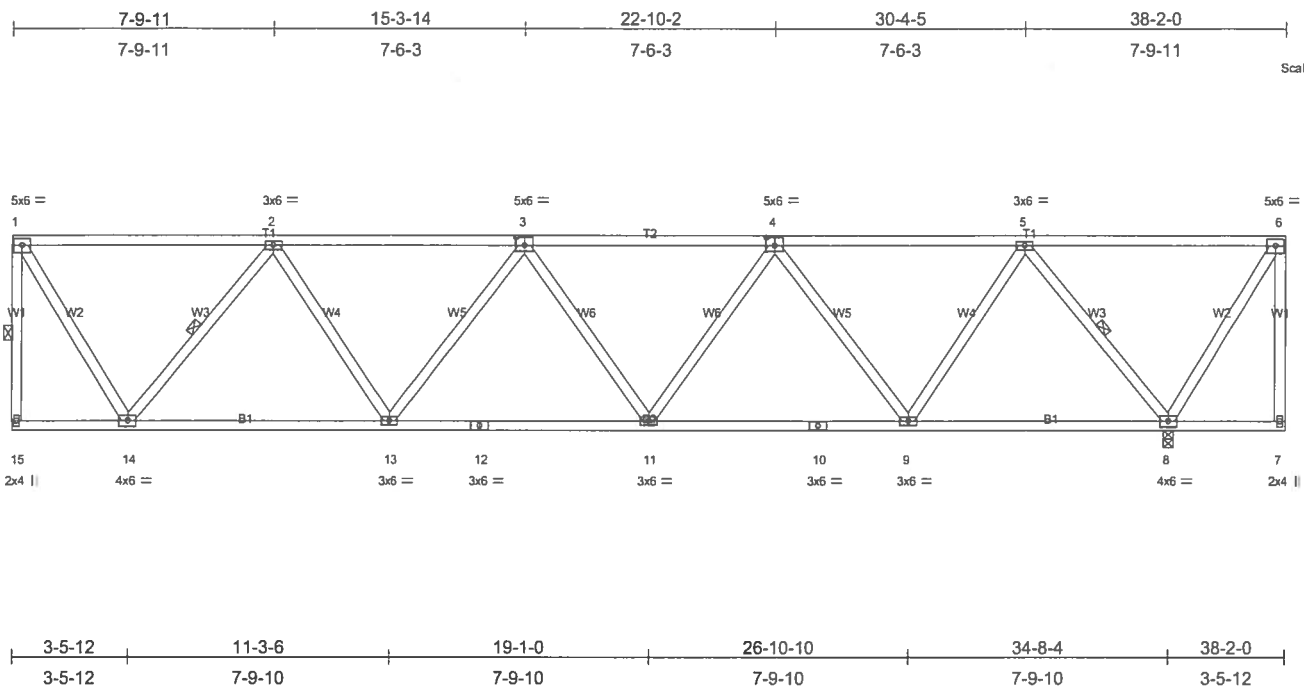


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.18	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.29	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 225 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-5 oc bracing.
WEBS 1 Row at midpt 1-15, 2-14, 5-8

REACTIONS (lb/size) 15=1437/Mechanical, 8=1744/0-3-8
Max Uplift15=-543(load case 3), 8=-659(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-15=-1397/530, 1-2=-764/266, 2-3=-1951/714, 3-4=-2188/803, 4-5=-1520/551, 5-6=-59/95, 6-7=-8/27
BOT CHORD 14-15=-27/40, 13-14=-619/1572, 12-13=-858/2214, 11-12=-858/2214, 10-11=-779/2006, 9-10=-779/2006, 8-9=-385/952, 7-8=-25/35
WEBS 1-14=-463/1406, 2-14=-1315/575, 2-13=-179/715, 3-13=-447/245, 3-11=-48/98, 4-11=-42/324, 4-9=-826/388, 5-9=-313/1071, 5-8=-1703/721, 6-8=-251/163

JOINT STRESS INDEX
1 = 0.69, 2 = 0.82, 3 = 0.58, 4 = 0.58, 5 = 0.82, 6 = 0.69, 7 = 0.77, 8 = 0.76, 9 = 0.82, 10 = 0.84, 11 = 0.42, 12 = 0.84, 13 = 0.82, 14 = 0.76 and 15 = 0.77

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 543 lb uplift at joint 15 and 659 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061676

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
TEMP	T15	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 10:30:49 2006 Page 1

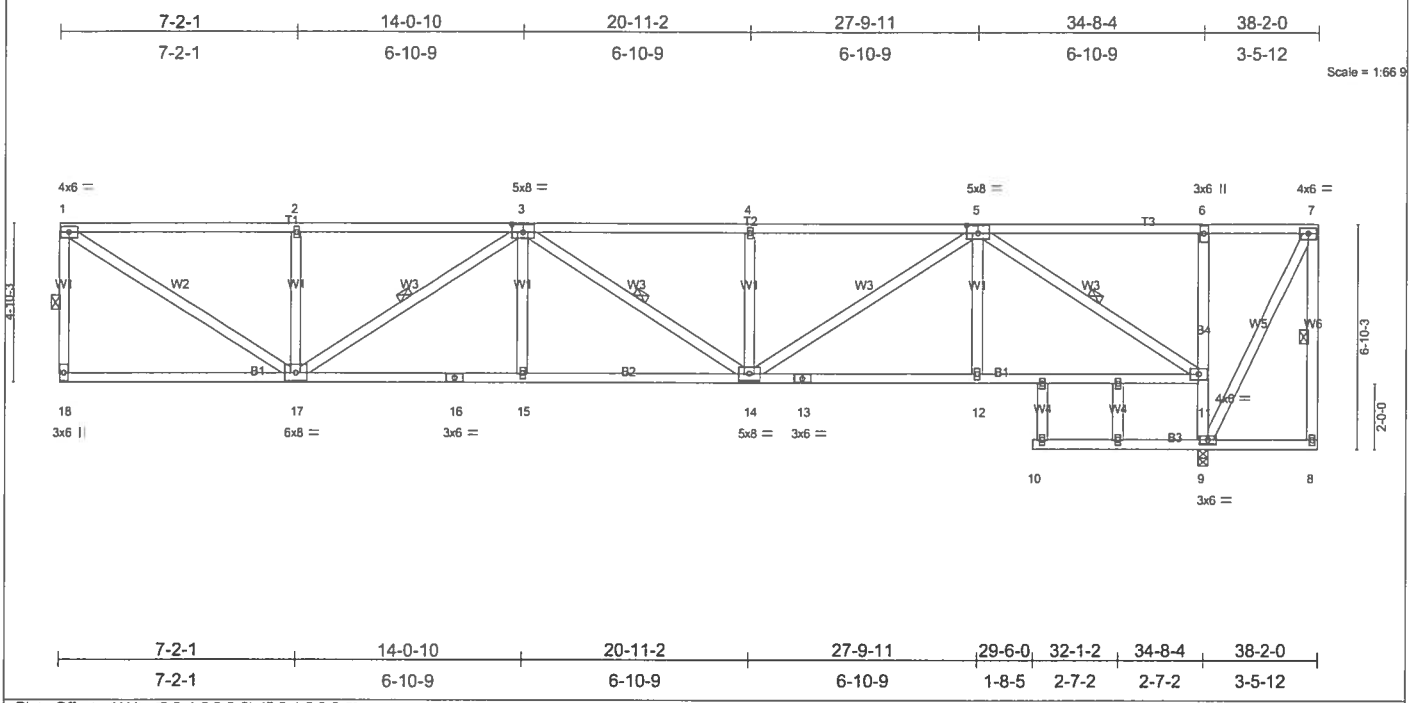


Plate Offsets (X,Y): [3:0-4-0,0-3-0], [5:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.68	10	>613	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-1.18	10	>352	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.26	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 245 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 1-18, 7-8, 3-17, 3-14, 5-11
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 18=1449/Mechanical, 9=1888/0-3-8
Max Uplift18=-540(load case 3), 9=-630(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-18=-1342/554, 1-2=-1853/689, 2-3=-1853/689, 3-4=-2695/987, 4-5=-2695/987, 5-6=-44/0, 6-7=-22/14, 7-8=-50/0
BOT CHORD 17-18=-24/62, 16-17=-1002/2707, 15-16=-1002/2707, 14-15=-1002/2707, 13-14=-642/1816, 12-13=-642/1816, 11-12=-642/1816, 9-11=-1564/652, 6-11=-294/224, 9-10=0/0, 8-9=-4/0
WEBS 1-17=-793/2134, 2-17=-387/276, 3-17=-1025/375, 3-15=0/206, 3-14=-14/17, 4-14=-370/259, 5-14=-414/1054, 5-12=0/216, 5-11=-2125/799, 7-9=-21/53

JOINT STRESS INDEX
1 = 0.88, 2 = 0.34, 3 = 0.41, 4 = 0.34, 5 = 0.61, 6 = 0.38, 7 = 0.32, 8 = 0.34, 9 = 0.83, 11 = 0.84, 12 = 0.34, 13 = 0.63, 14 = 0.49, 15 = 0.34, 16 = 0.97, 17 = 0.83, 18 = 0.41, 19 = 0.34, 20 = 0.34, 21 = 0.34 and 22 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 2x4 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 540 lb uplift at joint 18 and 630 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061677

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Sep 28 11:22:41 2006 Page 1

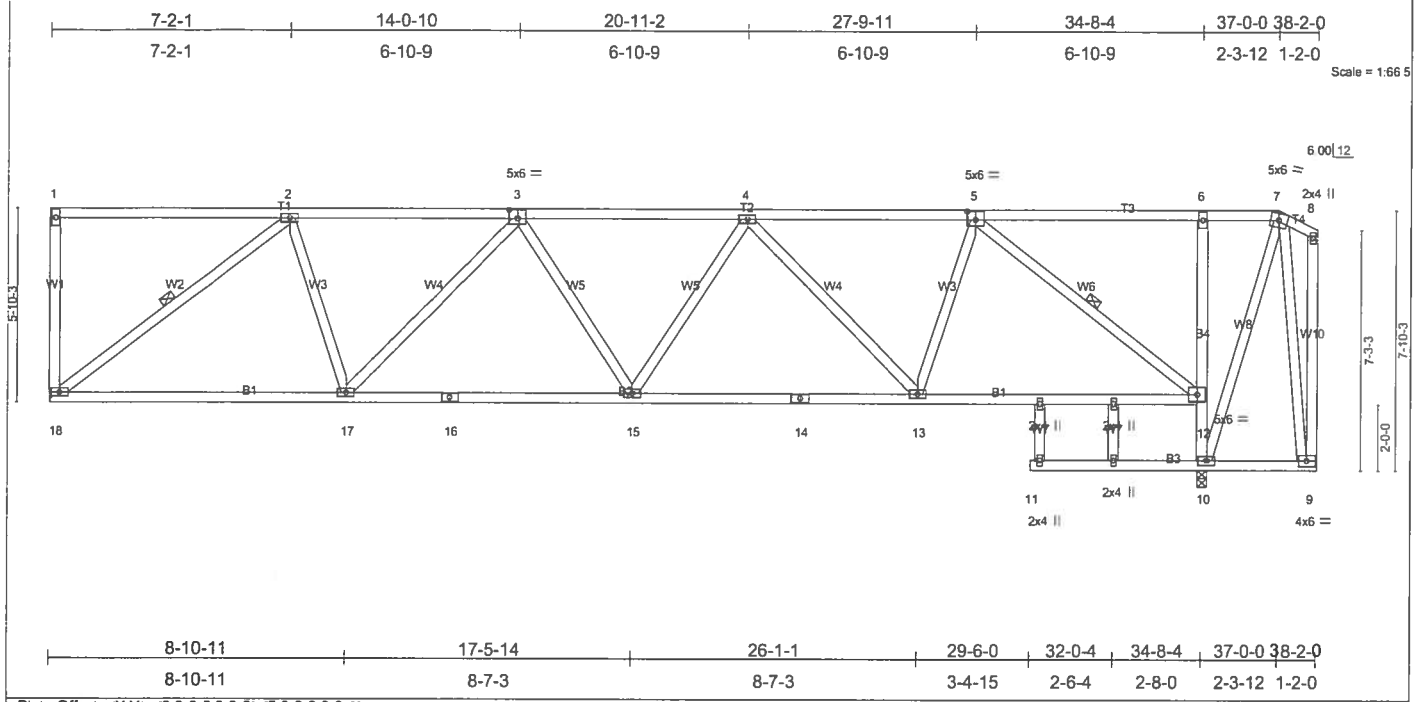


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.67	11	>623	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-1.16	11	>356	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.27	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-13 oc bracing. Except:
1 Row at midpt 12-13
WEBS 1 Row at midpt 2-18, 5-12
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 18=1449/Mechanical, 10=1888/0-3-8
Max Horz 18=-24(load case 6)
Max Uplift18=-544(load case 4), 10=-612(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-18=-180/120, 1-2=-55/15, 2-3=-1734/628, 3-4=-2259/820, 4-5=-1708/608, 5-6=-41/0, 6-7=0/13, 7-8=0/39, 8-9=0/39
BOT CHORD 17-18=-570/1501, 16-17=-830/2185, 15-16=-830/2185, 14-15=-824/2176, 13-14=-824/2176, 12-13=-544/1469, 10-12=-1525/651, 6-12=-253/226, 10-11=0/0, 9-10=0/6
WEBS 2-18=-1843/717, 2-17=-173/789, 3-17=-663/308, 3-15=0/140, 4-15=0/157, 4-13=-688/329, 5-13=-191/811, 5-12=-1835/709, 7-10=-50/46, 7-9=-74/0

JOINT STRESS INDEX
1 = 0.45, 2 = 0.74, 3 = 0.51, 4 = 0.43, 5 = 0.53, 6 = 0.42, 7 = 0.31, 8 = 0.34, 9 = 0.26, 10 = 0.72, 12 = 0.76, 13 = 0.76, 14 = 0.89, 15 = 0.43, 16 = 0.87, 17 = 0.74, 18 = 0.75, 19 = 0.34, 20 = 0.34, 21 = 0.34 and 22 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All plates are 3x6 MT20 unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 544 lb uplift at joint 18 and 612 lb uplift at joint 10.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

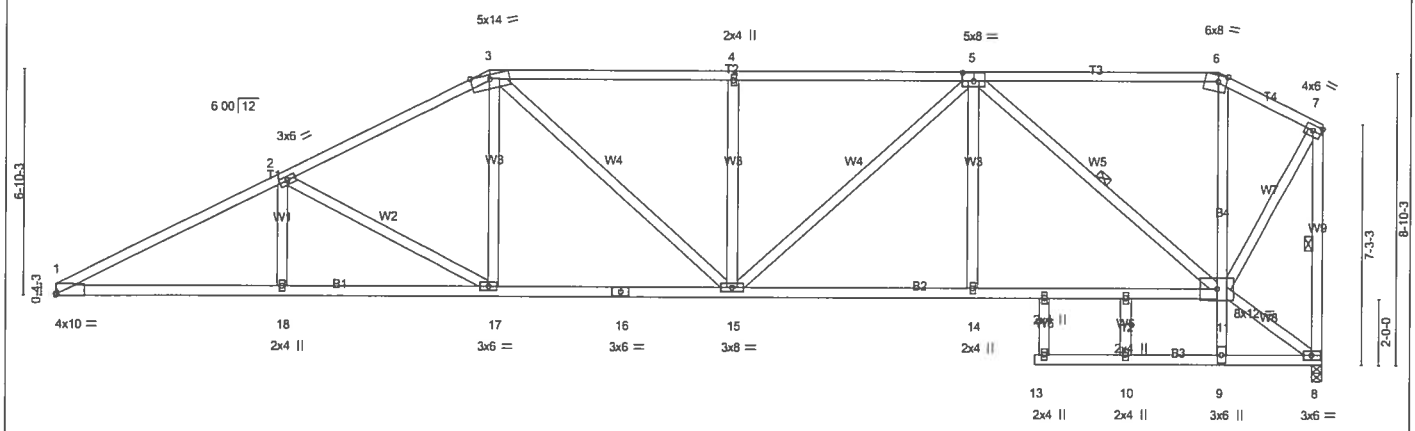
Dwg.#1025061678

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 29 07:56:55 2006 Page 1

6-9-4	13-0-0	20-4-9	27-7-7	35-0-0	38-2-0
6-9-4	6-2-12	7-4-9	7-2-13	7-4-9	3-2-0

Scale = 1:66.7



6-9-4	13-0-0	20-4-9	27-7-7	29-6-0	32-3-0	35-0-0	38-2-0
6-9-4	6-2-12	7-4-9	7-2-13	1-10-9	2-9-0	2-9-0	3-2-0

Plate Offsets (X,Y): [1:0-0-0,0-0-4], [5:0-4-0,0-3-0], [6:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.36	13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.61	13	>748	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.75	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 250 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-11, 7-8
JOINTS 1 Brace at Jt(s): 11, 12

REACTIONS (lb/size) 1=1610/Mechanical, 8=1661/0-3-8
Max Horz 1=253(load case 5)
Max Uplift1=-465(load case 5), 8=-468(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3095/856, 2-3=-2476/744, 3-4=-2399/814, 4-5=-2399/814, 5-6=-761/249, 6-7=-842/233, 7-8=-1692/447
BOT CHORD 1-18=-930/2692, 17-18=-930/2692, 16-17=-681/2158, 15-16=-681/2158, 14-15=-640/1963, 12-14=-640/1963, 11-12=-640/1963, 9-11=0/101, 6-11=0/100, 10-13=0/0, 9-10=0/0, 8-9=-90/0
WEBS 2-18=0/223, 2-17=-618/344, 3-17=-118/495, 3-15=-224/468, 4-15=-409/286, 5-15=-208/594, 5-14=0/256, 5-11=-1603/560, 8-11=0/113, 10-12=0/124, 7-11=-418/1476

JOINT STRESS INDEX
1 = 0.75, 2 = 0.41, 3 = 1.00, 4 = 0.34, 5 = 0.48, 6 = 0.65, 7 = 0.84, 8 = 0.55, 9 = 0.29, 10 = 0.34, 11 = 0.48, 12 = 0.34, 14 = 0.34, 15 = 0.58, 16 = 0.88, 17 = 0.35, 18 = 0.34, 19 = 0.34 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 465 lb uplift at joint 1 and 468 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061679

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 29 07:56:56 2006 Page 1

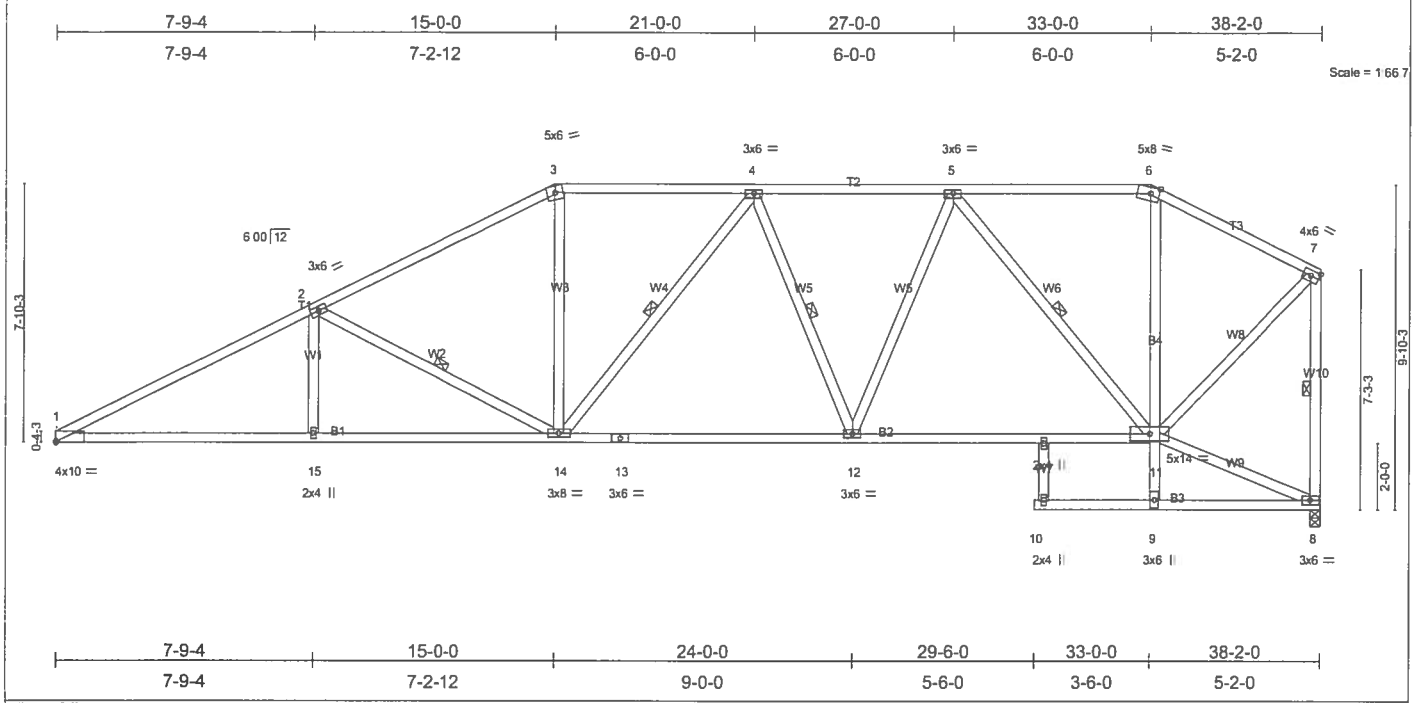


Plate Offsets (X,Y): [1:0-0-0,0-0-4], [6:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.25	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.42	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.17	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 246 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 2-14, 4-14, 4-12, 5-11, 7-8

REACTIONS (lb/size) 1=1614/Mechanical, 8=1684/0-3-8
Max Horz 1=267(load case 5)
Max Uplift1=-479(load case 5), 8=-403(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3054/868, 2-3=-2317/681, 3-4=-2001/666, 4-5=-1943/614, 5-6=-1031/336, 6-7=-1199/328, 7-8=-1675/449
BOT CHORD 1-15=-944/2649, 14-15=-944/2649, 13-14=-634/2044, 12-13=-634/2044, 11-12=-528/1730, 9-11=0/198, 6-11=-1/276, 9-10=0/0, 8-9=-117/0
WEBS 2-15=0/259, 2-14=-749/409, 3-14=-104/642, 4-14=-244/172, 4-12=-287/191, 5-12=-131/589, 5-11=-1135/436, 8-11=0/140, 7-11=-359/1452

JOINT STRESS INDEX
1 = 0.75, 2 = 0.41, 3 = 0.64, 4 = 0.48, 5 = 0.53, 6 = 0.58, 7 = 0.74, 8 = 0.55, 9 = 0.42, 11 = 0.76, 12 = 0.53, 13 = 0.73, 14 = 0.57, 15 = 0.34, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 479 lb uplift at joint 1 and 403 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061680

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 29 07:56:58 2006 Page 1

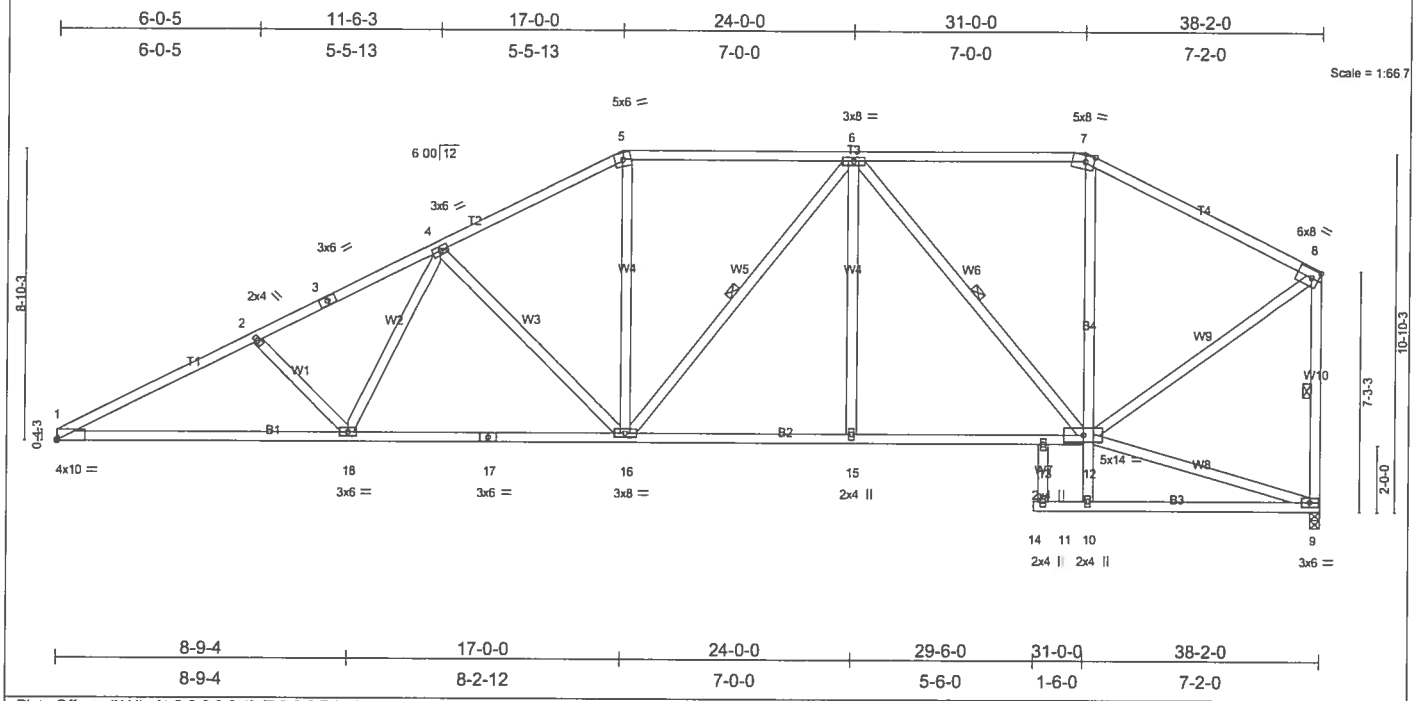


Plate Offsets (X,Y): [1:0-0-0,0-0-4], [7:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.24	1-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.39	1-18	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.15	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-16, 6-12, 8-9
JOINTS 1 Brace at Jt(s): 12

REACTIONS (lb/size) 1=1596/Mechanical, 9=1601/0-3-8
Max Horz 1=281(load case 5)
Max Uplift1=-496(load case 5), 9=-426(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3033/944, 2-3=-2816/869, 3-4=-2743/888, 4-5=-2053/670, 5-6=-1792/655, 6-7=-1106/423, 7-8=-1316/411, 8-9=-1514/474
BOT CHORD 1-18=-1046/2663, 17-18=-792/2214, 16-17=-792/2214, 15-16=-489/1691, 13-15=-489/1691, 12-13=-489/1691, 10-12=0/221, 7-12=0/258, 11-14=0/0, 10-11=0/0, 9-10=-10/11
WEBS 2-18=-305/267, 4-18=-164/559, 4-16=-621/362, 5-16=-106/543, 6-16=-136/190, 6-15=0/200, 6-12=-962/325, 8-12=-353/1333, 11-13=-60/0, 9-12=-30/43

JOINT STRESS INDEX
1 = 0.74, 2 = 0.34, 3 = 0.39, 4 = 0.41, 5 = 0.62, 6 = 0.57, 7 = 0.73, 8 = 0.75, 9 = 0.75, 10 = 0.34, 11 = 0.34, 12 = 0.56, 13 = 0.34, 15 = 0.34, 16 = 0.57, 17 = 0.92 and 18 = 0.47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 496 lb uplift at joint 1 and 426 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

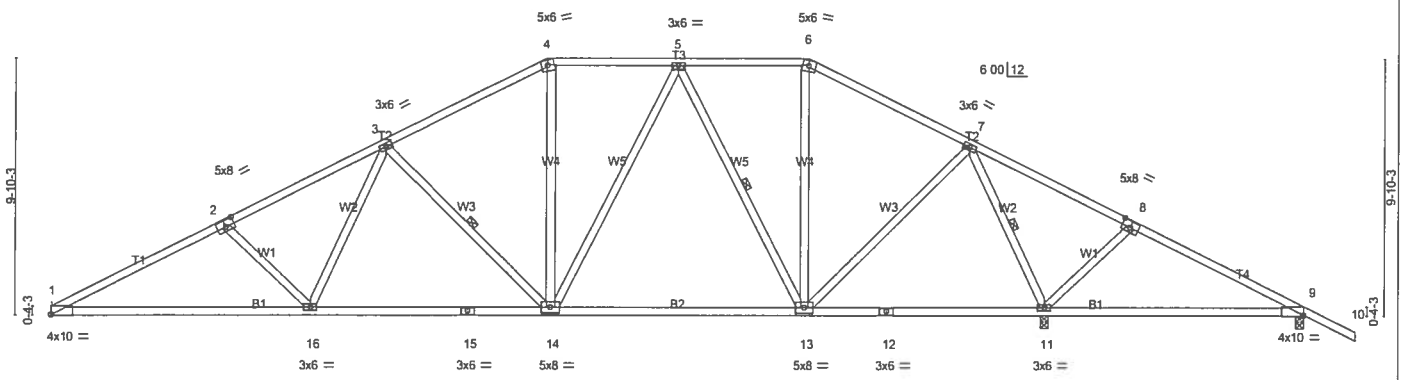
Dwg.#1025061681

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 29 07:57:00 2006 Page 1

6-8-5	12-10-3	19-0-0	24-0-0	29-0-0	35-1-13	41-3-11	48-0-0	50-0-0
6-8-5	6-1-13	6-1-13	5-0-0	5-0-0	6-1-13	6-1-13	6-8-5	2-0-0

Scale = 1:84.5



9-11-12	19-0-0	29-0-0	38-0-4	48-0-0
9-11-12	9-0-4	10-0-0	9-0-4	9-11-12

Plate Offsets (X,Y): [1:0-0-0,0-0-4], [2:0-4-0,0-3-0], [8:0-4-0,0-3-0], [9:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.33	1-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.55	1-16	>824	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.09	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 271 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-11, 5-13, 3-14

REACTIONS (lb/size) 1=1485/Mechanical, 11=2527/0-3-8, 9=119/0-3-8
Max Horz 1=-196(load case 6)
Max Uplift1=-485(load case 5), 11=-686(load case 6), 9=-201(load case 6)
Max Grav 1=1485(load case 1), 11=2527(load case 1), 9=220(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 6-7=-1043/427, 7-8=-223/948, 8-9=-188/700, 9-10=0/47, 4-5=-1403/591, 5-6=-867/428, 1-2=-2733/907, 2-3=-2478/837, 3-4=-1636/595
BOT CHORD 1-16=-833/2395, 15-16=-546/1895, 14-15=-546/1895, 13-14=-241/1195, 12-13=0/143, 11-12=0/143, 9-11=-590/234
WEBS 8-11=-378/290, 7-11=-2136/632, 7-13=-236/1123, 6-13=-24/198, 5-13=-779/280, 5-14=-184/487, 4-14=-86/382, 3-14=-715/412, 3-16=-181/615, 2-16=-355/302

JOINT STRESS INDEX
1 = 0.67, 2 = 0.62, 3 = 0.85, 4 = 0.50, 5 = 0.46, 6 = 0.50, 7 = 0.85, 8 = 0.62, 9 = 0.67, 11 = 0.92, 12 = 0.74, 13 = 0.54, 14 = 0.54, 15 = 0.74 and 16 = 0.92

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 485 lb uplift at joint 1, 686 lb uplift at joint 11 and 201 lb uplift at joint 9.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061682

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T21	COMMON	11	1	Job Reference (optional)

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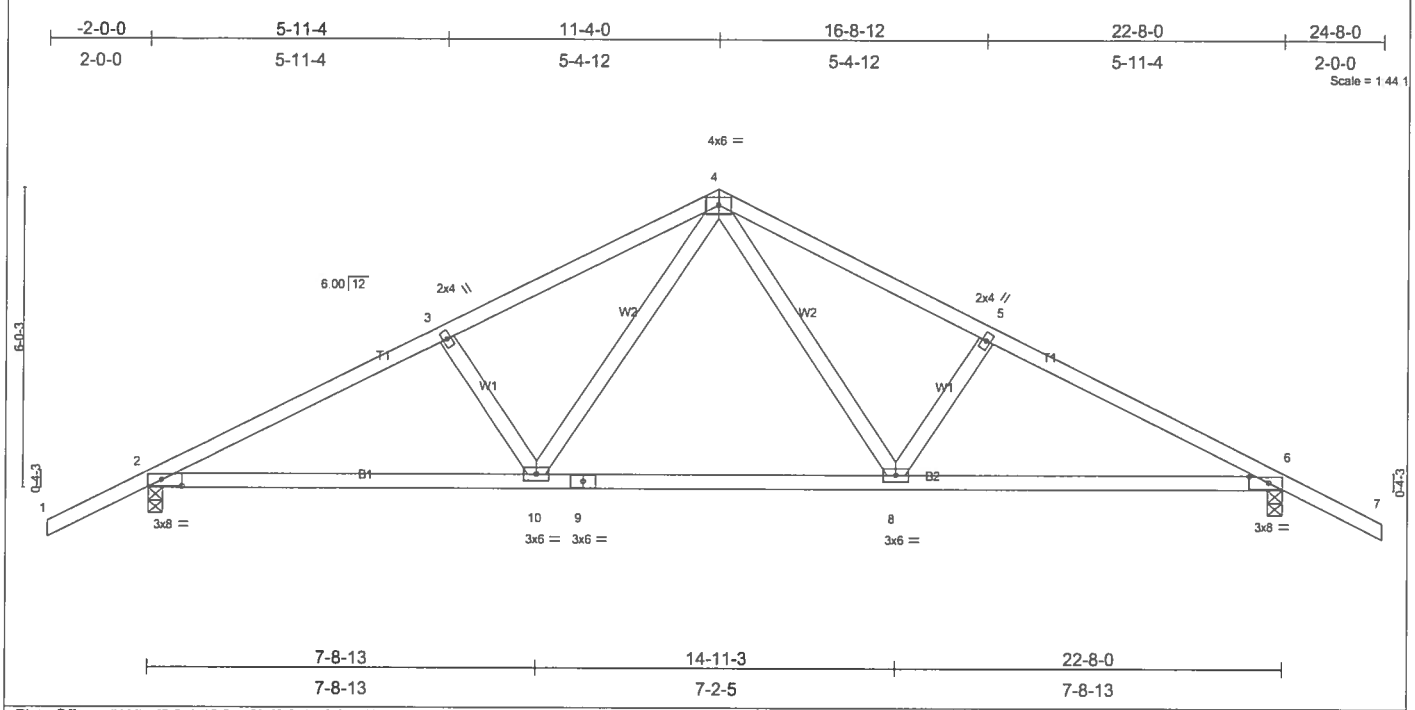

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.24	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.38	8-10	>707	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.06	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-2 oc bracing.

REACTIONS (lb/size) 2=1235/0-3-8, 6=1235/0-3-8
Max Horz 2=118(load case 5)
Max Uplift2=-499(load case 5), 6=-499(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1999/637, 3-4=-1834/636, 4-5=-1834/636, 5-6=-1999/637, 6-7=0/47
BOT CHORD 2-10=-538/1713, 9-10=-276/1177, 8-9=-276/1177, 6-8=-446/1713
WEBS 3-10=-263/230, 4-10=-261/775, 4-8=-261/775, 5-8=-263/230

JOINT STRESS INDEX
2 = 0.75, 3 = 0.34, 4 = 0.62, 5 = 0.34, 6 = 0.75, 8 = 0.59, 9 = 0.47 and 10 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 499 lb uplift at joint 2 and 499 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061683

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T21G	GABLE	1	1	Job Reference (optional)

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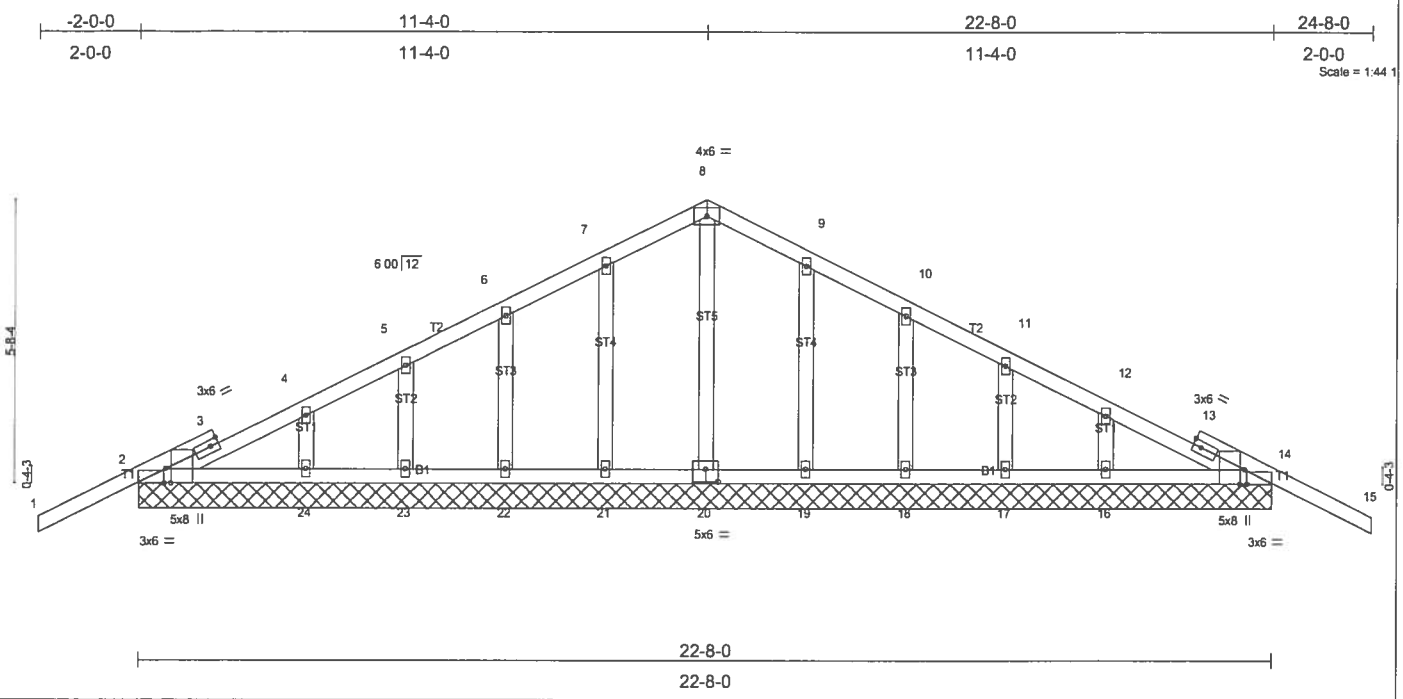

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [14:0-3-8,Edge], [14:0-0-8,Edge], [20:0-3-0,0-3-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	15	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.06	15	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.00	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 120 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=515/22-8-0, 14=515/22-8-0, 20=283/22-8-0, 21=287/22-8-0, 22=293/22-8-0, 23=266/22-8-0, 24=357/22-8-0, 19=287/22-8-0, 18=293/22-8-0, 17=266/22-8-0, 16=357/22-8-0
Max Horz 2=113(load case 5)
Max Uplift2=-271(load case 5), 14=-288(load case 6), 21=-128(load case 5), 22=-131(load case 5), 23=-143(load case 5), 24=-112(load case 6), 19=-126(load case 6), 18=-132(load case 6), 17=-141(load case 6), 16=-109(load case 6)
Max Grav 2=515(load case 1), 14=515(load case 1), 20=283(load case 1), 21=292(load case 9), 22=293(load case 1), 23=266(load case 9), 24=357(load case 1), 19=292(load case 10), 18=293(load case 1), 17=266(load case 10), 16=357(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-12/99, 2-3=-76/46, 3-4=-77/63, 4-5=-61/89, 5-6=-55/121, 6-7=-56/160, 7-8=-56/191, 8-9=-56/184, 9-10=-56/143, 10-11=-55/103, 11-12=-61/58, 12-13=-32/30, 13-14=-76/25, 14-15=-12/99
BOT CHORD 2-24=0/126, 23-24=0/126, 22-23=0/126, 21-22=0/126, 20-21=0/126, 19-20=0/126, 18-19=0/126, 17-18=0/126, 16-17=0/126, 14-16=0/126
WEBS 8-20=-222/0, 7-21=-232/140, 6-22=-231/145, 5-23=-214/147, 4-24=-275/134, 9-19=-232/137, 10-18=-231/146, 11-17=-214/145, 12-16=-275/139

JOINT STRESS INDEX
2 = 0.66, 2 = 0.18, 3 = 0.00, 3 = 0.32, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.34, 8 = 0.25, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.00, 13 = 0.32, 14 = 0.66, 14 = 0.18, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.20, 21 = 0.34, 22 = 0.34, 23 = 0.34 and 24 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 2, 288 lb uplift at joint 14, 128 lb uplift at joint 21, 131 lb uplift at joint 22, 143 lb uplift at joint 23, 112 lb uplift at joint 24, 126 lb uplift at joint 19, 132 lb uplift at joint 18, 141 lb uplift at joint 17 and 109 lb uplift at joint 16.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-8=-114(F=-60), 8-15=-114(F=-60), 2-14=-30

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061684

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T23	SPECIAL	1	1	Job Reference (optional)

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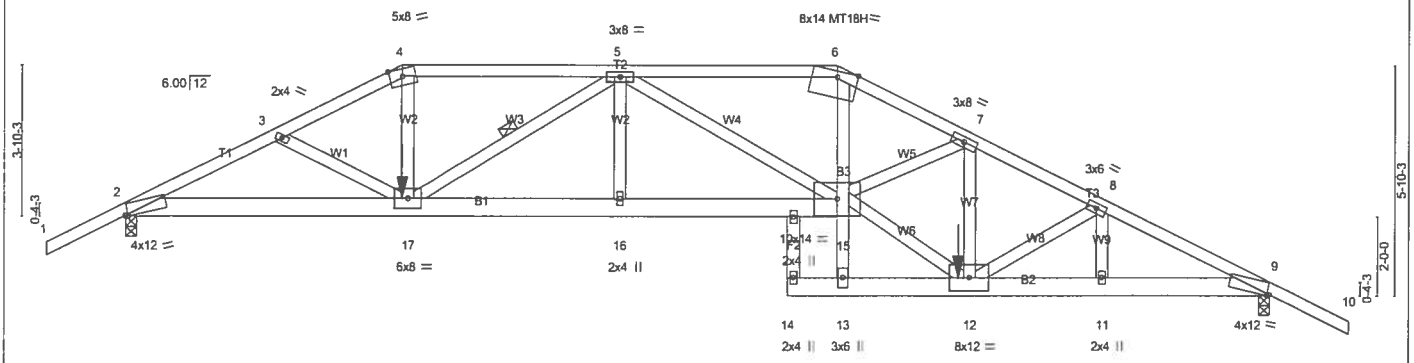
-2-0-0	3-11-4	7-0-0	12-6-0	18-0-0	21-4-8	24-9-0	29-0-0	31-0-0
2-0-0	3-11-4	3-0-12	5-6-0	5-6-0	3-4-8	3-4-8	4-3-0	2-0-0

Scale = 1:56.0



7-0-0	12-6-0	16-9-0	18-0-0	21-1-1	21-4-8	24-9-0	29-0-0
7-0-0	5-6-0	4-3-0	1-3-0	3-1-1	0-3-7	3-4-8	4-3-0

Plate Offsets (X,Y): [2:0-0-13,Edge], [6:0-6-3,Edge], [9:0-0-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	-0.42	14	>824	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.67	14	>513	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(TL)	0.23	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B3 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W6 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-1 oc bracing.
WEBS 1 Row at midpt 5-17

REACTIONS (lb/size) 2=2662/0-3-8, 9=2698/0-3-8
Max Horz 2=-297(load case 5)
Max Uplift2=-1110(load case 4), 9=-1098(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5139/1973, 3-4=-5030/1968, 4-5=-4569/1824, 5-6=-5851/2269, 6-7=-6667/2450, 7-8=-5036/1922, 8-9=-5205/1897, 9-10=0/51
BOT CHORD 2-17=-1571/4524, 16-17=-2137/6050, 15-16=-2137/6050, 13-15=0/284, 6-15=-691/2404, 13-14=0/0, 12-13=-99/212, 11-12=-1564/4593, 9-11=-1564/4593
WEBS 3-17=-84/112, 4-17=-576/1773, 5-17=-1818/736, 5-16=0/429, 5-15=-380/265, 12-15=-1673/5008, 7-15=-559/1620, 7-12=-1763/667, 8-12=-147/58, 8-11=-17/95

JOINT STRESS INDEX
2 = 0.77, 3 = 0.34, 4 = 0.98, 5 = 0.87, 6 = 0.93, 7 = 0.95, 8 = 0.41, 9 = 0.78, 11 = 0.34, 12 = 0.87, 13 = 0.72, 14 = 0.34, 15 = 0.80, 16 = 0.34, 17 = 0.44 and 18 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1110 lb uplift at joint 2 and 1098 lb uplift at joint 9.
7) Girder carries hip end with 7-10-15 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0, and 671 lb down and 253 lb up at 21-1-1 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-118(F=-64), 6-7=-118(F=-64), 7-10=-54, 2-17=-30, 15-17=-65(F=-35), 13-14=-65(F=-35), 12-13=-65(F=-35), 9-12=-30
Concentrated Loads (lb)
Vert: 17=-539(F) 12=-671(F)

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061685

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T24	SPECIAL	1	1	Job Reference (optional)

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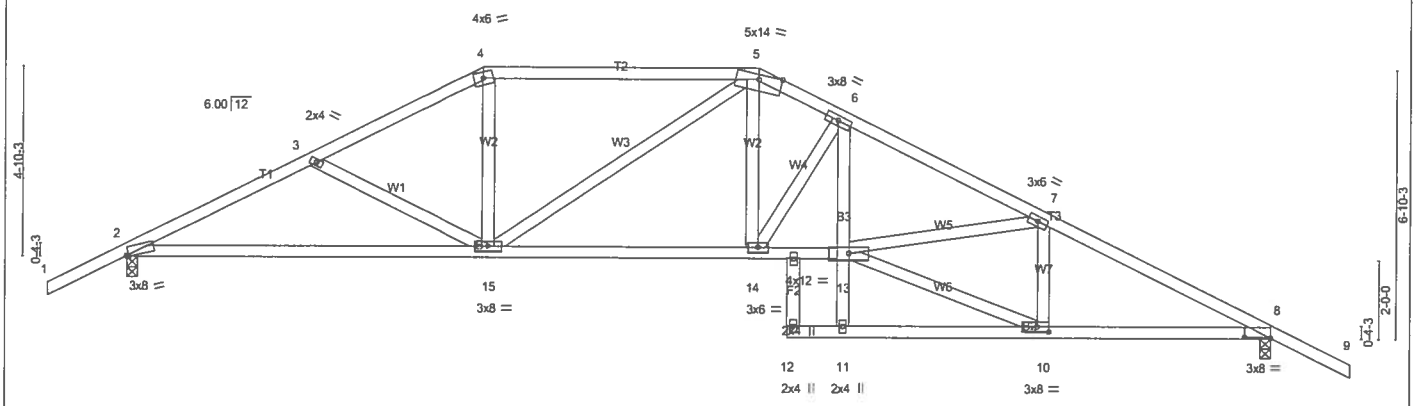
-2-0-0 | 4-9-4 | 9-0-0 | 16-0-0 | 18-0-0 | 23-2-12 | 29-0-0 | 31-0-0

2-0-0 | 4-9-4 | 4-2-12 | 7-0-0 | 2-0-0 | 5-2-12 | 5-9-4 | 2-0-0

Scale = 1:56.0



18-0-0

9-0-0 | 16-0-0 | 16-9-0 | 23-2-12 | 29-0-0

9-0-0 | 7-0-0 | 0-9-0 | 5-2-12 | 5-9-4

1-3-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-8-0,0-0-6], [10:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.21	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.35	2-15	>998	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-6-15 oc bracing.
JOINTS 1 Brace at Jt(s): 13

REACTIONS (lb/size) 2=1338/0-3-8, 8=1347/0-3-8
Max Horz 2=-194(load case 6)
Max Uplift2=-471(load case 5), 8=-516(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2164/585, 3-4=-1943/492, 4-5=-1716/490, 5-6=-2163/609, 6-7=-2817/727, 7-8=-2249/649, 8-9=0/47
BOT CHORD 2-15=-421/1884, 14-15=-271/1974, 13-14=-369/2436, 11-13=0/126, 6-13=-172/906, 11-12=0/0, 10-11=-11/79, 8-10=-434/1936
WEBS 3-15=-215/188, 4-15=-71/522, 5-15=-410/127, 5-14=-202/805, 6-14=-884/283, 10-13=-458/2013, 7-13=-96/528, 7-10=-619/222

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.79, 5 = 0.96, 6 = 0.92, 7 = 0.41, 8 = 0.71, 10 = 0.80, 11 = 0.79, 12 = 0.34, 13 = 0.91, 14 = 0.53, 15 = 0.57 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 471 lb uplift at joint 2 and 516 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

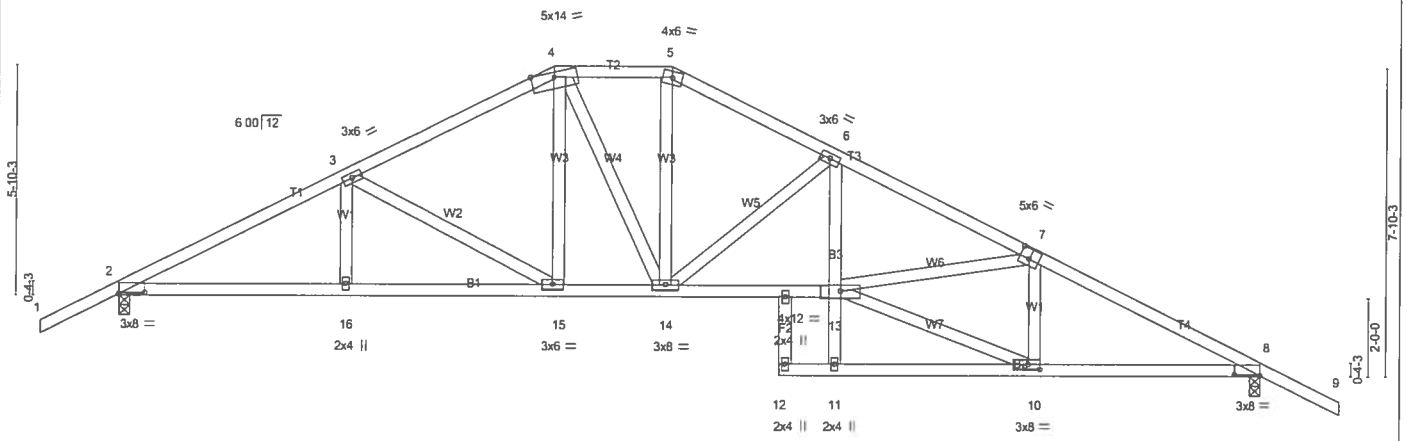
Dwg.#1025061686

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industnes, Inc. Fri Sep 29 07:57:08 2006 Page 1

-2-0-0 | 5-9-4 | 11-0-0 | 14-0-0 | 18-0-0 | 23-2-12 | 29-0-0 | 31-0-0
2-0-0 | 5-9-4 | 5-2-12 | 3-0-0 | 4-0-0 | 5-2-12 | 5-9-4 | 2-0-0

Scale = 1:56.0



5-9-4 | 11-0-0 | 14-0-0 | 16-9-0 | 18-0-0 | 23-2-12 | 29-0-0
5-9-4 | 5-2-12 | 3-0-0 | 2-9-0 | 1-3-0 | 5-2-12 | 5-9-4

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [7:0-3-0,0-3-0], [8:0-8-0,0-0-6], [10:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.20	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.33	12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-4-10 oc bracing.

REACTIONS (lb/size) 2=1338/0-3-8, 8=1347/0-3-8
Max Horz 2=-208(load case 6)
Max Uplift2=-487(load case 5), 8=-528(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2234/588, 3-4=-1737/509, 4-5=-1595/530, 5-6=-1818/547, 6-7=-2832/771, 7-8=-2245/671, 8-9=0/47
BOT CHORD 2-16=-431/1923, 15-16=-431/1923, 14-15=-225/1502, 13-14=-421/2489, 11-13=0/124, 6-13=-168/941, 11-12=0/0, 10-11=-16/63, 8-10=-452/1932
WEBS 3-16=0/178, 3-15=-497/238, 4-15=-92/351, 4-14=-94/315, 5-14=-163/610, 6-14=-1167/392, 10-13=-479/2044, 7-13=-43/533, 7-10=-629/229

JOINT STRESS INDEX
2 = 0.71, 3 = 0.41, 4 = 0.44, 5 = 0.39, 6 = 0.70, 7 = 0.43, 8 = 0.71, 10 = 0.81, 11 = 0.72, 12 = 0.34, 13 = 0.71, 14 = 0.63, 15 = 0.35, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 487 lb uplift at joint 2 and 528 lb uplift at joint 8.

LOAD CASE(S) Standard

OCTOBER 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1025061687

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE- JOHN THOMAS
L210709	T26	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 29 07:57:09 2006 Page 1

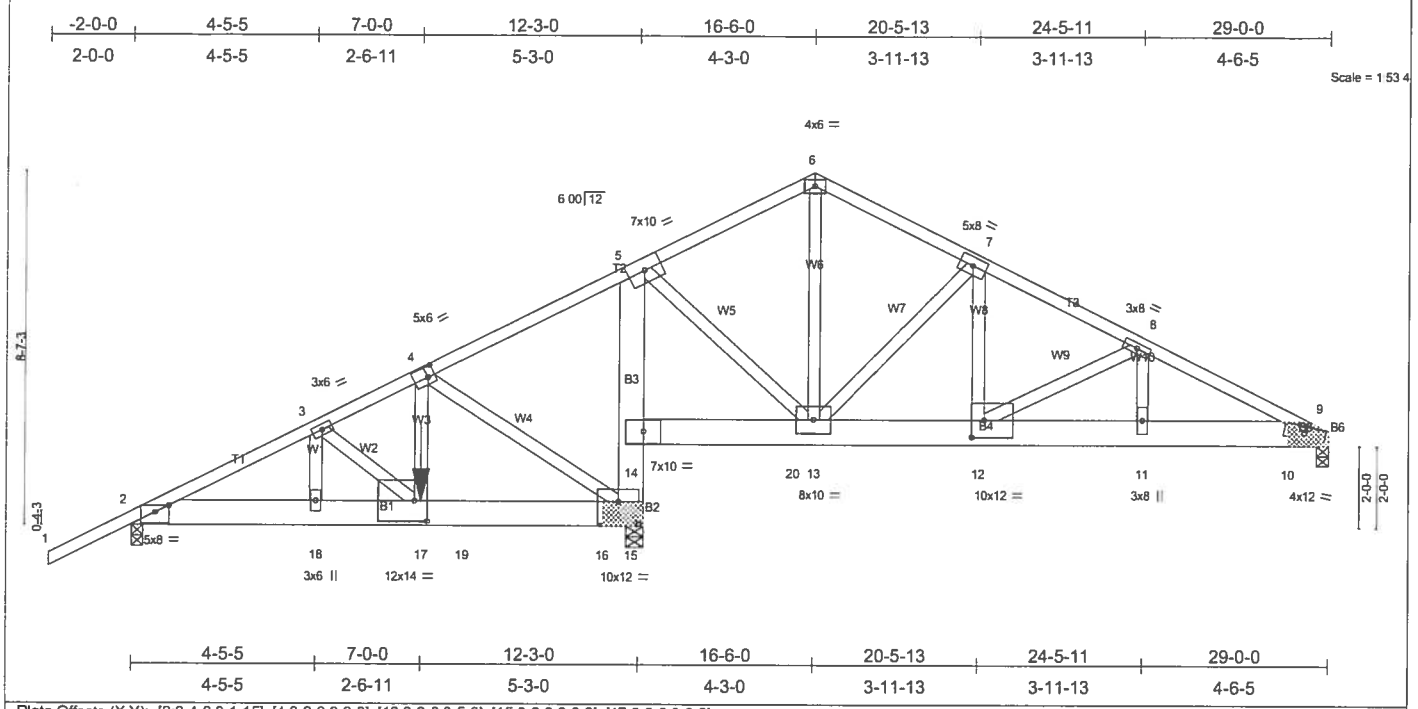


Plate Offsets (X,Y): [2:0-4-0,0-1-15], [4:0-2-0,0-3-0], [12:0-3-8,0-5-0], [15:0-6-0,0-6-8], [17:0-3-8,0-6-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.44	Vert(LL) -0.13 11-12 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.58	Vert(TL) -0.21 11-12 >966 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.94	Horz(TL) -0.02 9 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 459 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 14-15,13-14.

REACTIONS (lb/size) 9=6371/0-3-12 (0-3-8 + bearing block), 2=1499/0-3-8, 15=12754/0-7-8 (0-5-0 + bearing block)
Max Horz 2=2451(load case 4)
Max Uplift9=-2304(load case 5), 15=-5526(load case 4)
Max Grav 9=6371(load case 1), 2=1525(load case 8), 15=12754(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-2605/0, 3-4=-2737/0, 4-5=-2116/1947, 5-6=-2879/991, 6-7=-2867/974, 7-8=-6718/2354, 8-9=-10360/3698
BOT CHORD 2-18=-1746/2287, 17-18=-1746/2287, 17-19=-1687/2314, 16-19=-1687/2314, 15-16=-1687/2314, 14-15=-8396/3844, 5-14=-6307/3250, 14-20=-1887/1318, 13-20=-1887/1318, 12-13=-1956/5981, 11-12=-3265/9271, 10-11=-3265/9271, 9-10=-3265/9271
WEBS 3-18=-368/99, 3-17=-82/272, 4-17=-1484/4553, 4-15=-4770/1704, 5-13=-2594/5891, 6-13=-801/2366, 7-13=-4987/1919, 7-12=-1840/5095, 8-12=-3737/1486, 8-11=-1124/3105

JOINT STRESS INDEX
2 = 0.30, 3 = 0.41, 4 = 0.86, 5 = 0.65, 6 = 0.57, 7 = 0.84, 8 = 0.92, 9 = 0.76, 9 = 0.00, 9 = 0.00, 10 = 0.00, 10 = 0.00, 10 = 0.00, 11 = 0.51, 12 = 0.51, 13 = 0.69, 14 = 0.69, 15 = 0.60, 15 = 0.00, 15 = 0.00, 16 = 0.00, 16 = 0.00, 16 = 0.00, 17 = 0.43 and 18 = 0.16

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 4-17 2 X 4 - 1 row at 0-8-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 15 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 9 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
5) Unbalanced roof live loads have been considered for this design.
6) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2304 lb uplift at joint 9 and 5526 lb uplift at joint 15.
9) Girder carries tie-in span(s): 34-6-8 from 8-0-0 to 16-0-0; 38-2-0 from 16-0-0 to 29-0-0
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2994 lb down and 1130 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-54, 6-9=-54, 2-19=-30, 15-19=-707(F=-677), 14-20=-707(F=-677), 9-20=-783(F=-753)
Concentrated Loads (lb)
Vert: 17=-2994(F)

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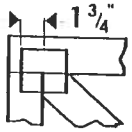
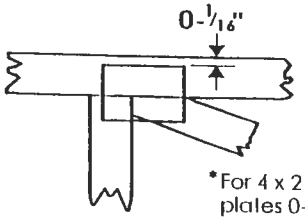
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

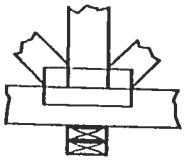
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

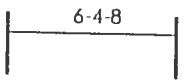


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

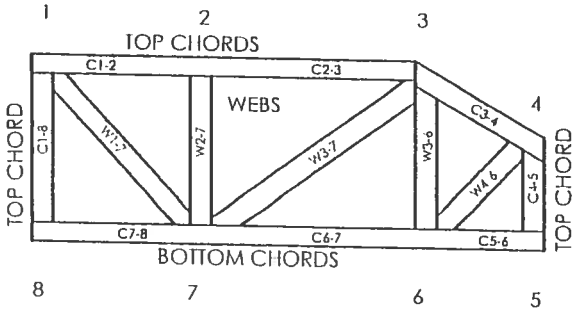
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.