ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
 Document ID:1TAK487-Z0311145557

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-244R-Gary Sandlin
Truss Count:	6
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 32.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 09/11/2007

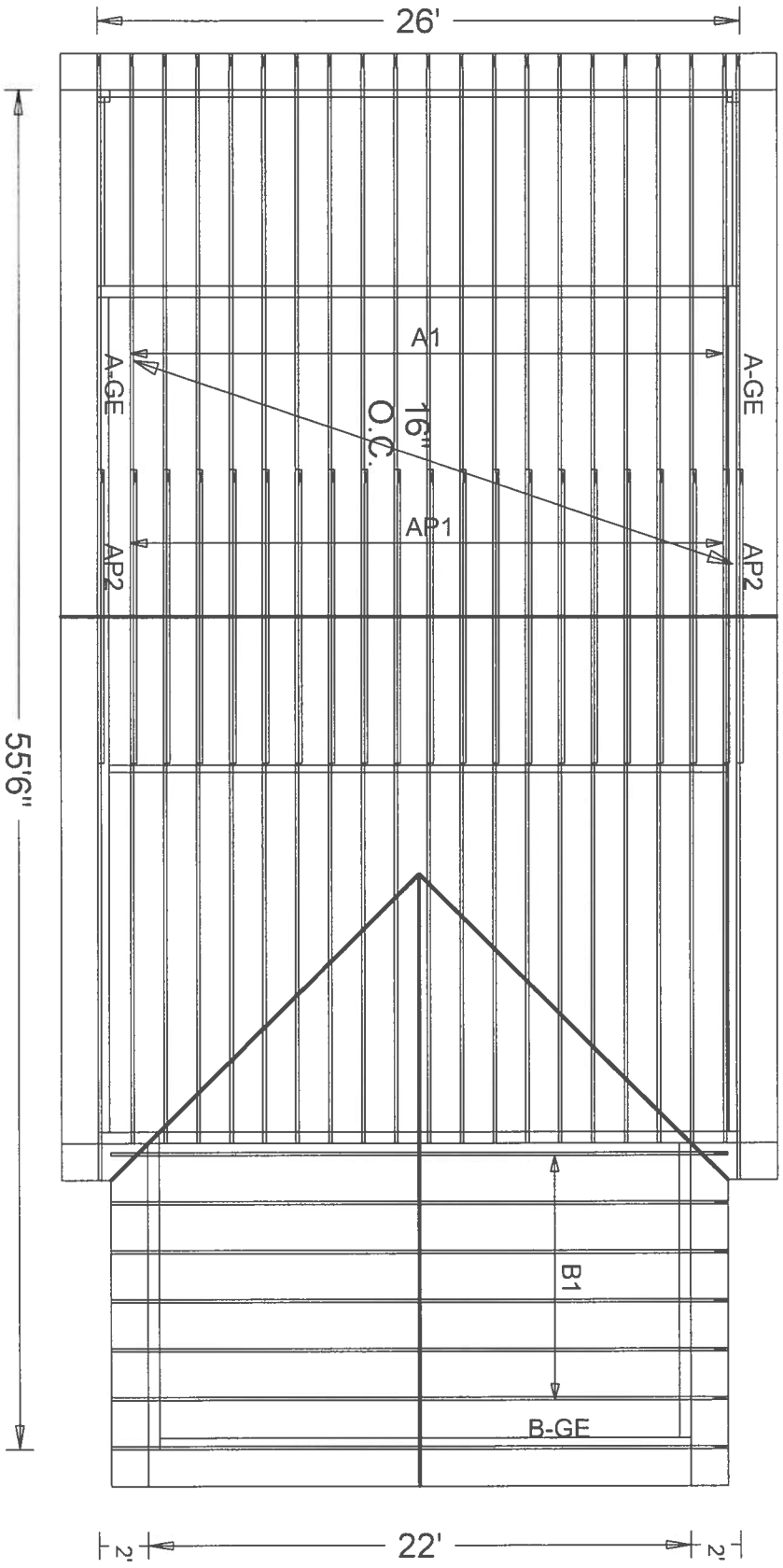
-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: A11015EE-GBLLETIN-140PB-

#	Ref	Description	Drawing#	Date
1	40690	A1	07250011	09/07/07
2	40691	A-GE	07250012	09/07/07
3	40692	B1	07250013	09/07/07
4	40693	B-GE	07250014	09/07/07
5	40694	AP2	07250015	09/07/07
6	40695	AP1	07250016	09/07/07



#7-244R
GARY SANDLIN-
WILSON SPRINGS

JOB DESCRIPTION:: OWNER BUILDER
/: Gary Sandlin-Wilson Sprin

JOB NO:
7-244R

PAGE NO:
1 OF 1

(7-244R Gary Sandlin - A1)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #2:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W4, W31 2x4 SP #2 Dense:

Trusses to be spaced at 16.0" OC maximum.

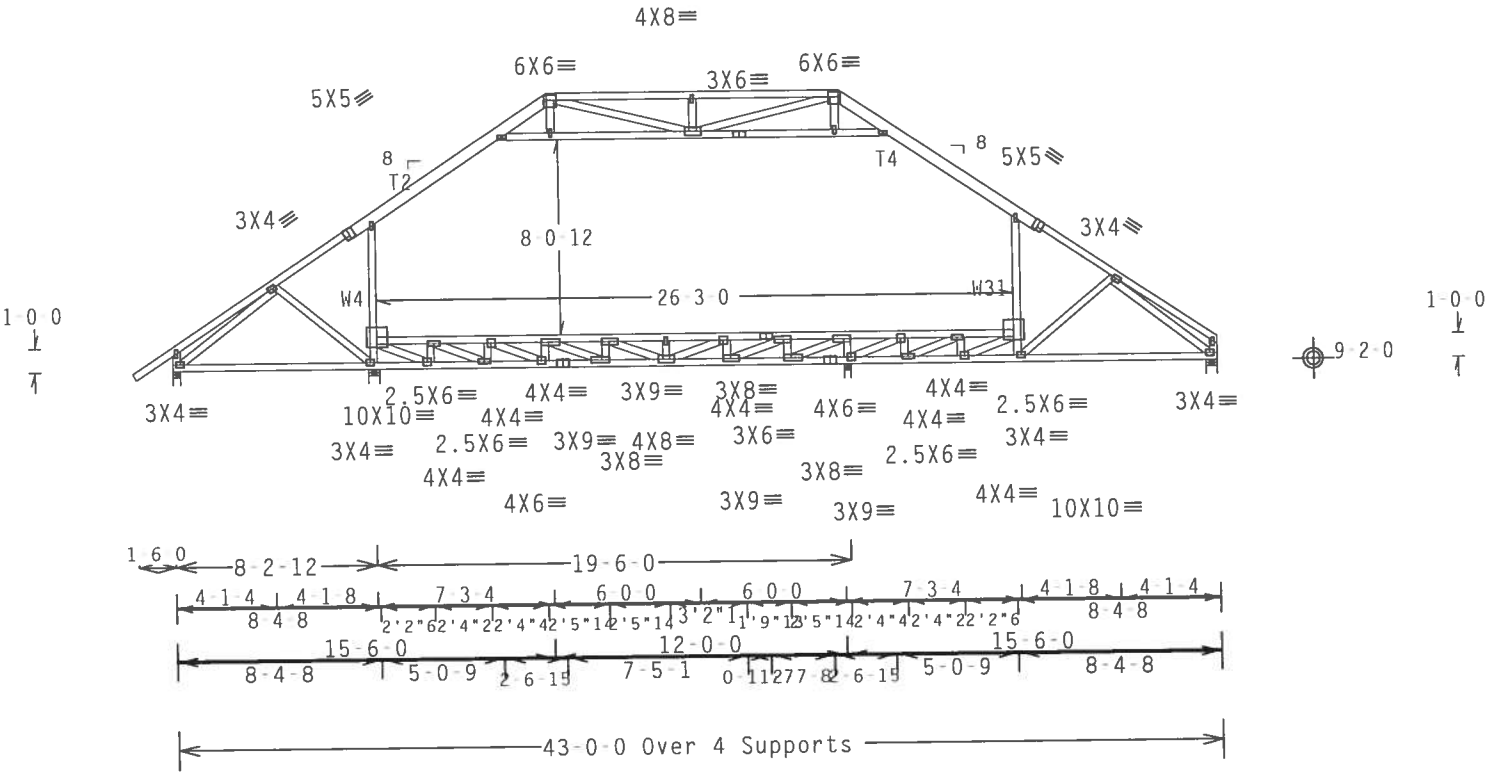
Collar tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-4-8 to 34-7-8.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230.17

QTY:19 FL/-/4/-/E/-/- Scale =.125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,H/SS/K) ASTM A653 GRADE 40/60 (H, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 62212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 1[illegible]

TC LL	20.0 PSF	REF R487-- 40690
TC DL	10.0 PSF	DATE 09/07/07
BC DL	10.0 PSF	DRW HCUSR487 07250011
BC LL	0.0 PSF	HC-ENG CC/AP
TOT.LD.	40.0 PSF	SEQN- 166956
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JRFF- 1TAK487 Z03

(7 244R Gary Sandlin - A GE)

Top chord 2x4 SP #2 Dense :T3, T5 2x6 SP #2:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W33 2x4 SP #2 Dense:
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

Trusses to be spaced at 16.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

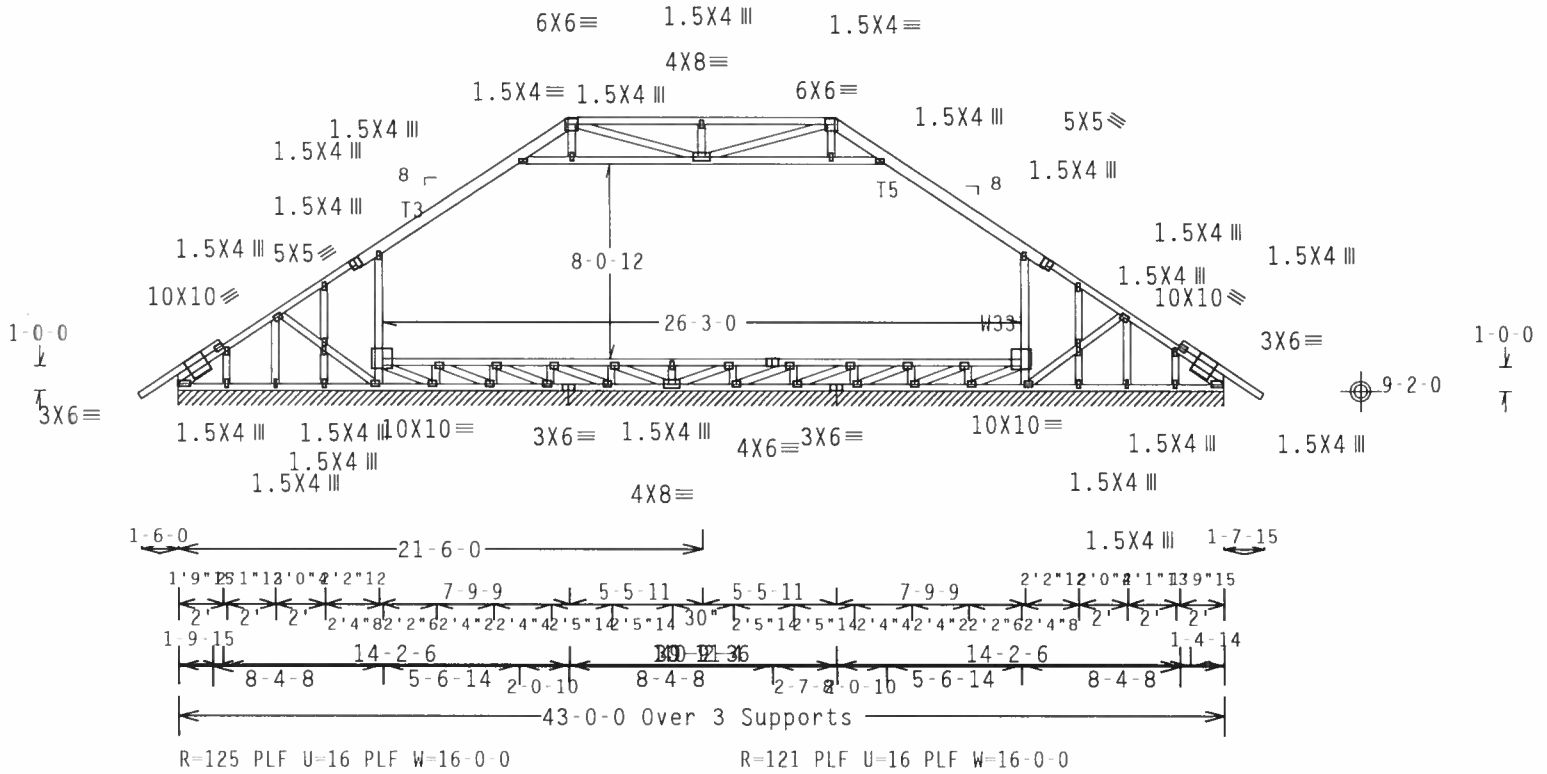
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 8-4-8 to 34-7-8.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave | Cq/RT=1.00(1.25)/10(0) 7.24.1230 | QTY:2 | FL/-/4/-/E/-/- | Scale =.125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No.52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 1

TC LL	20.0 PSF	REF	R487-- 40691
TC DL	10.0 PSF	DATE	09/07/07
BC DL	10.0 PSF	DRW	HCUSR487 07250012
BC LL	0.0 PSF	HC-ENG	CC/AP
TOT.LD.	40.0 PSF	SEQN-	42150 REV
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TAK487 Z03

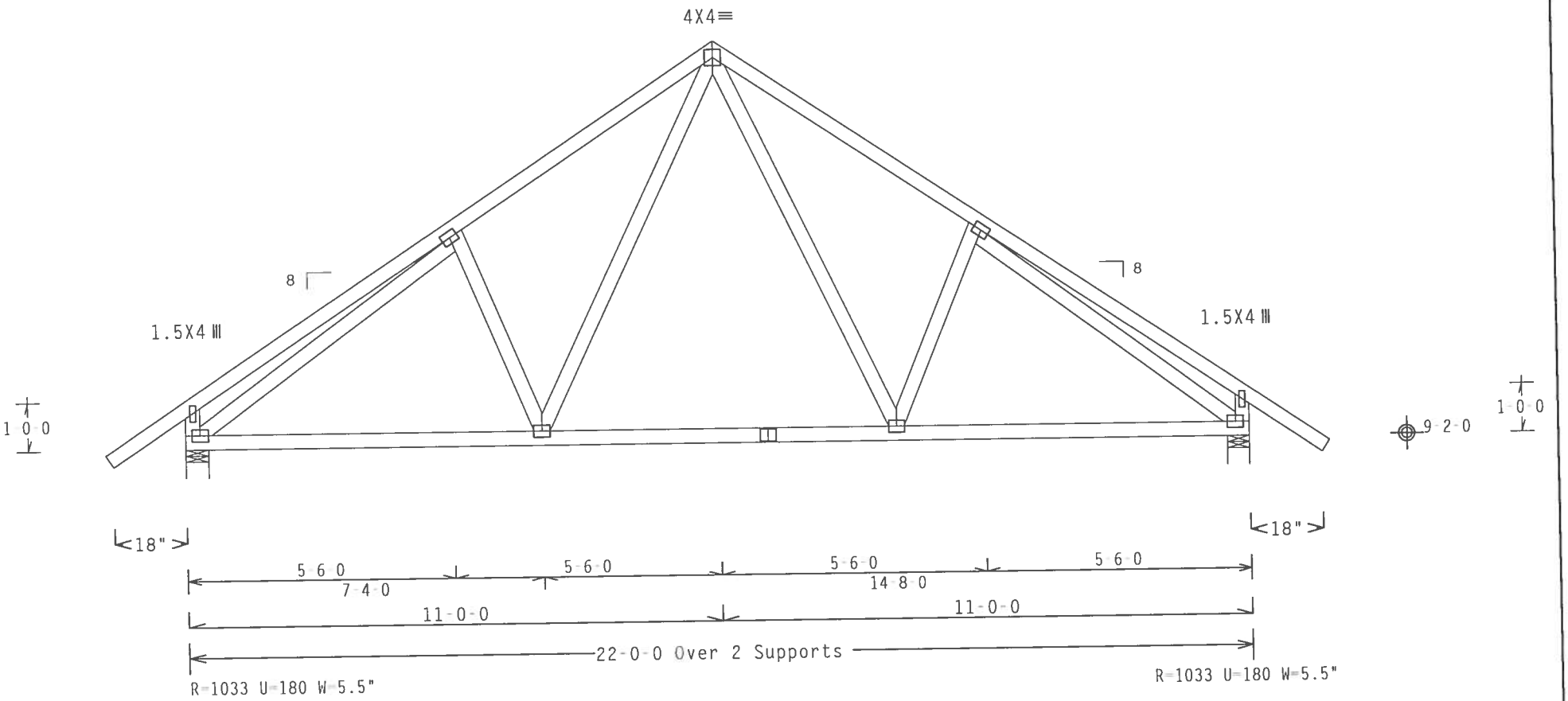
(7-244R Gary Sandlin - B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.24.1230

PLT TYP. Wave

QTY:6 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 1[illegible] 07

TC LL	20.0 PSF	REF R487-- 40692
TC DL	10.0 PSF	DATE 09/07/07
BC DL	10.0 PSF	DRW HCUSR487 07250013
BC LL	0.0 PSF	HC-ENG CC/AP
TOT.LD.	40.0 PSF	SEQN- 41244
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TAK487 Z03

(7 244R Gary Sandlin B GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

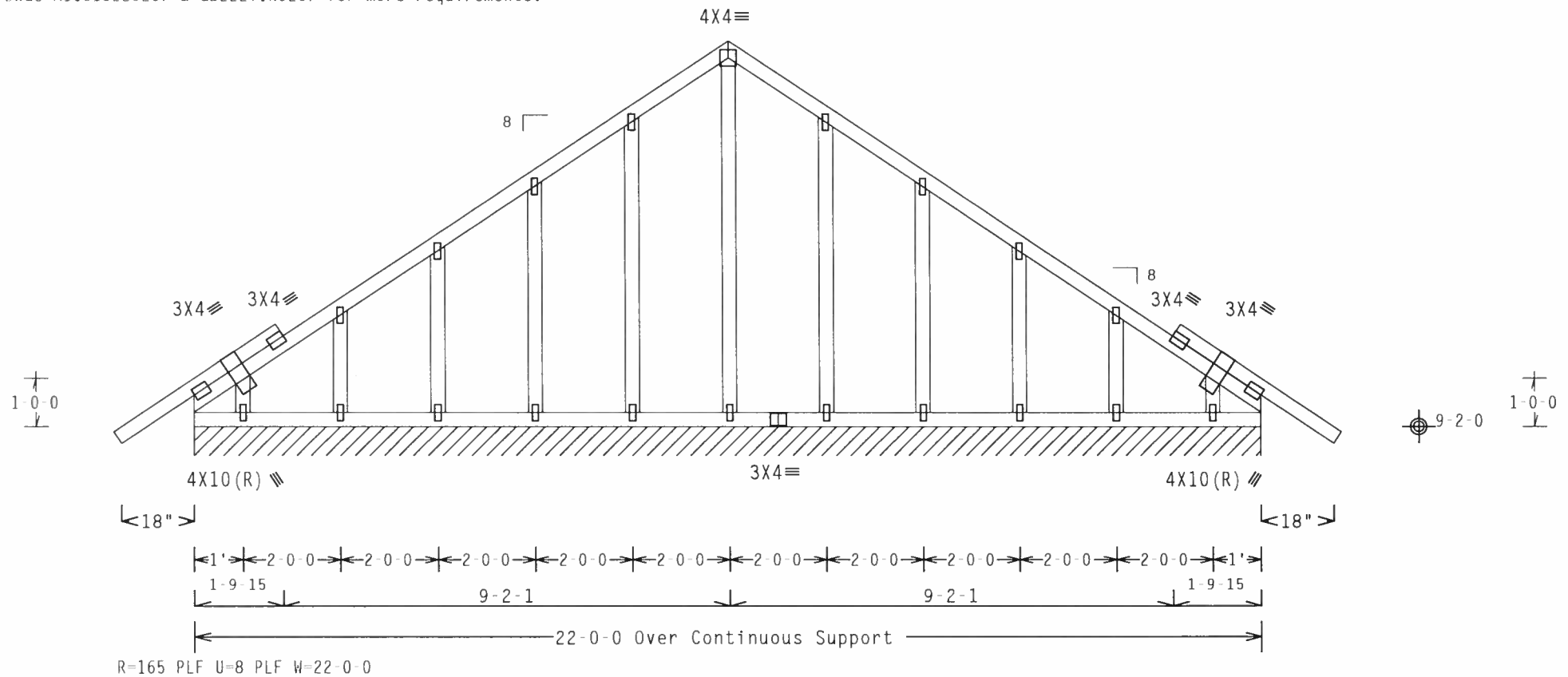
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Fasten rated sheathing to one face of this frame.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/10(0) 7.24.1230 QTY:1 FL/-/4/-/E/-/- Scale =.3125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Sep 11 '07

TC LL	20.0 PSF	REF	R487-- 40693
TC DL	10.0 PSF	DATE	09/07/07
BC DL	10.0 PSF	DRW	HCUSR487 07250014
BC LL	0.0 PSF		HC-ENG CC/AP
TOT.LD.	40.0 PSF	SEQN-	42104
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TAK487 Z03

(7-244R-Gary Sandlin - AP2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

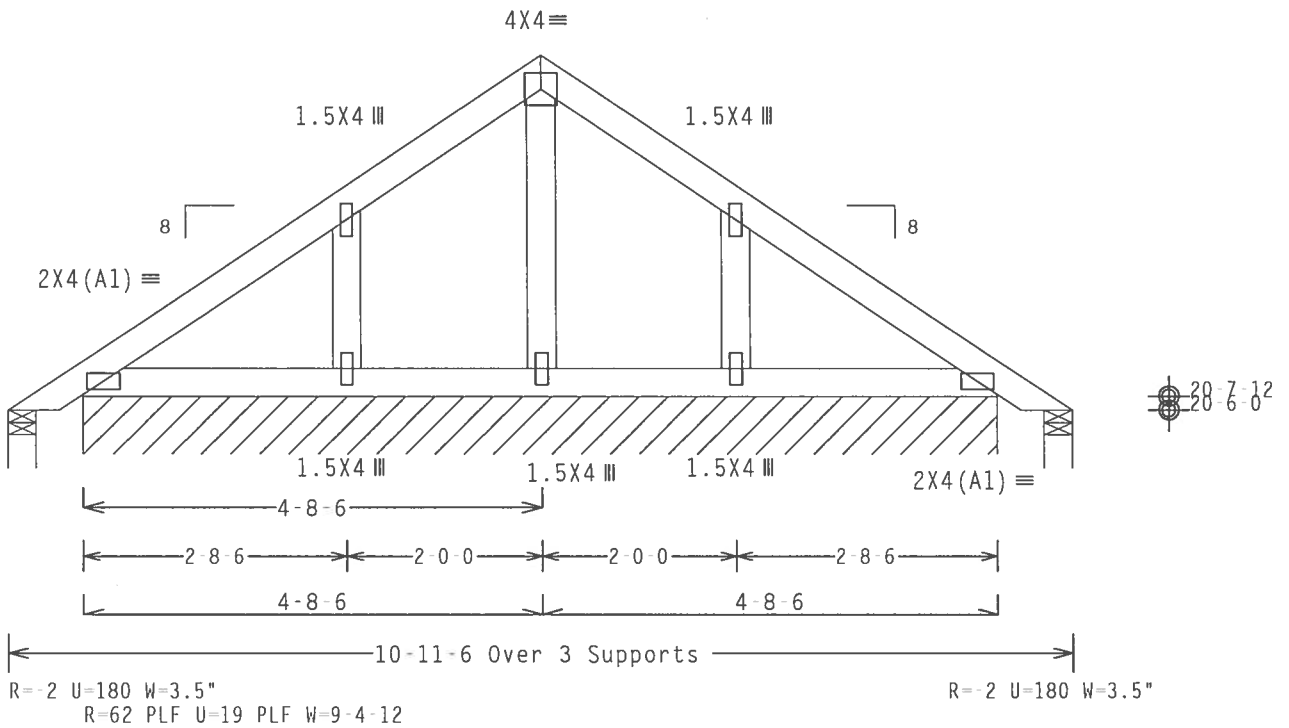
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

See DRW HCUSR001 02086006 for piggyback details. Top chord of supporting truss under piggyback to be laterally braced at 24" oc, unless specified otherwise.

110 mph wind, 22.32 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:2 FL/-/4/-/E/-/- Scale =.5"/Ft.

ALPINE

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JAMES F. COLLINS - LICENSE - No. 52212 - STATE OF FLORIDA - PROFESSIONAL ENGINEER

Sep 11 07

TC LL	20.0 PSF	REF	R487-- 40694
TC DL	10.0 PSF	DATE	09/07/07
BC DL	10.0 PSF	DRW	HCUSR487 07250015
BC LL	0.0 PSF		HC-ENG CC/AP
TOT.LD.	40.0 PSF	SEQN-	166556
DUR.FAC.	1.25	FROM	AH
SPACING	16.0"	JREF-	1TAK487 Z03

(7 244R Gary Sandlin AP1)

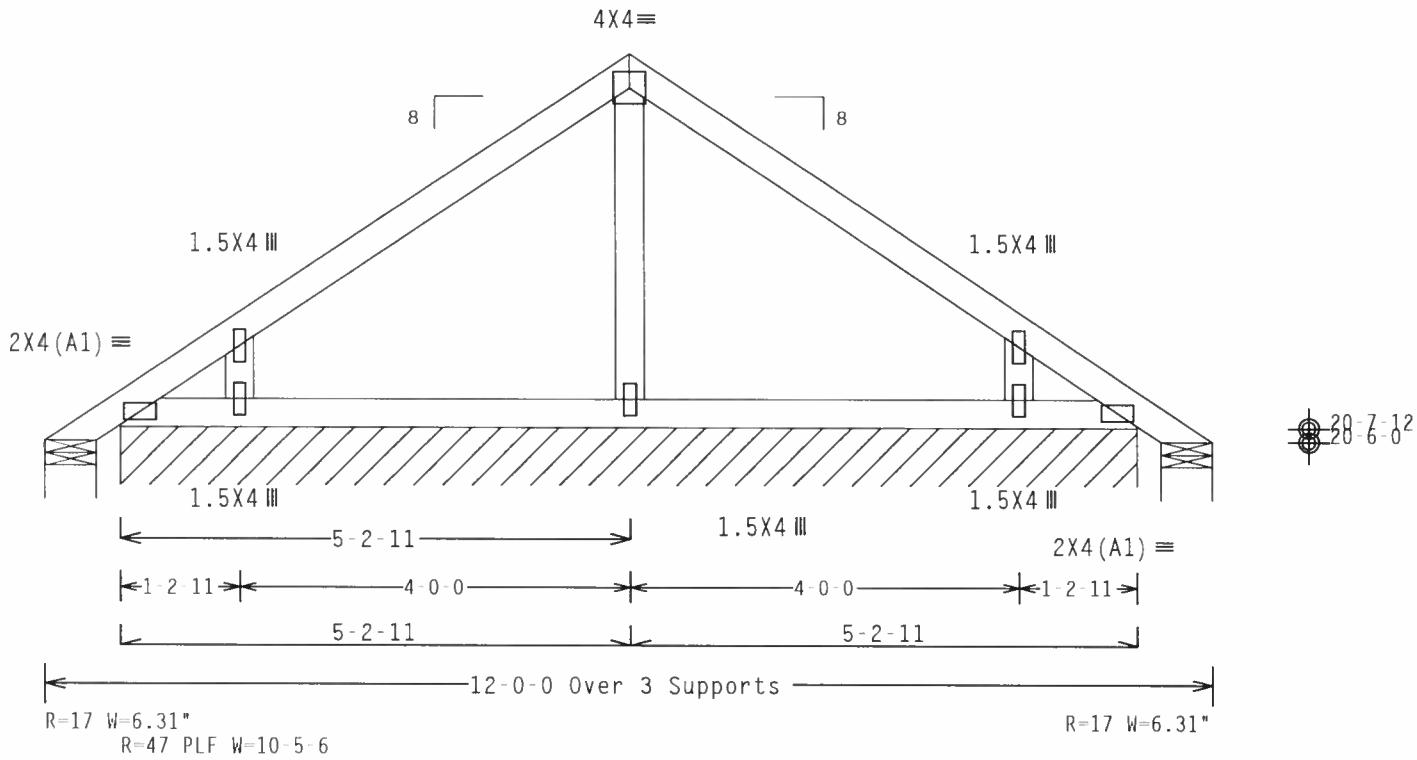
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

See DRW HCUSR001 02086006 for piggyback details. Top chord of supporting truss under piggyback to be laterally braced at 24" oc, unless specified otherwise.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Trusses to be spaced at 16.0" OC maximum.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/10(0) 7.24.1230

QTY:19 FL/-/4/-/E/-/-

Scale =.5"/Ft.

ALPINE

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 11 07

TC LL	20.0 PSF	REF R487-- 40695
TC DL	10.0 PSF	DATE 09/07/07
BC DL	2.0 PSF	DRW HCUSR487 07250016
BC LL	0.0 PSF	HC-ENG CC/AP
TOT.LD.	32.0 PSF	SEQN- 41237
DUR.FAC.	1.25	FROM AH
SPACING	16.0"	JREF- 1TAK487 Z03

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

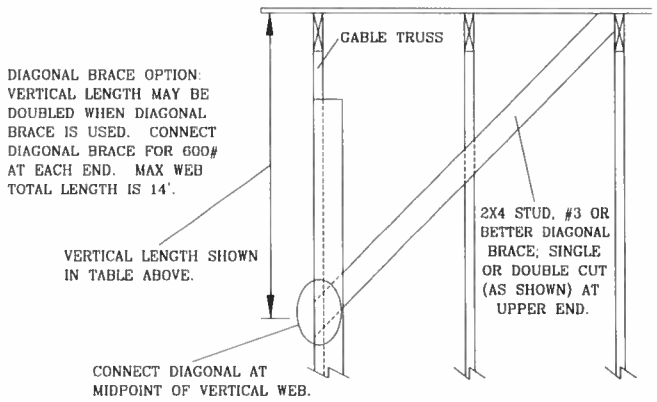
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

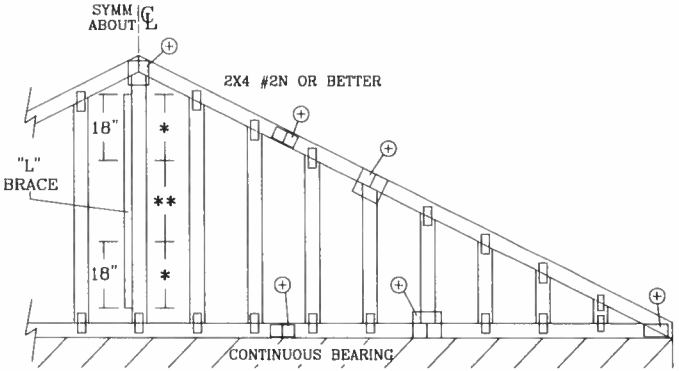
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR: #1 & BTR, #1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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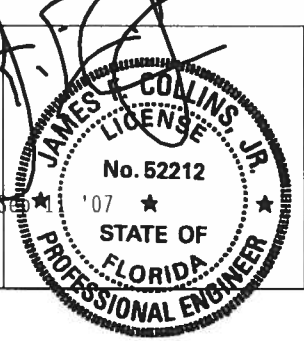
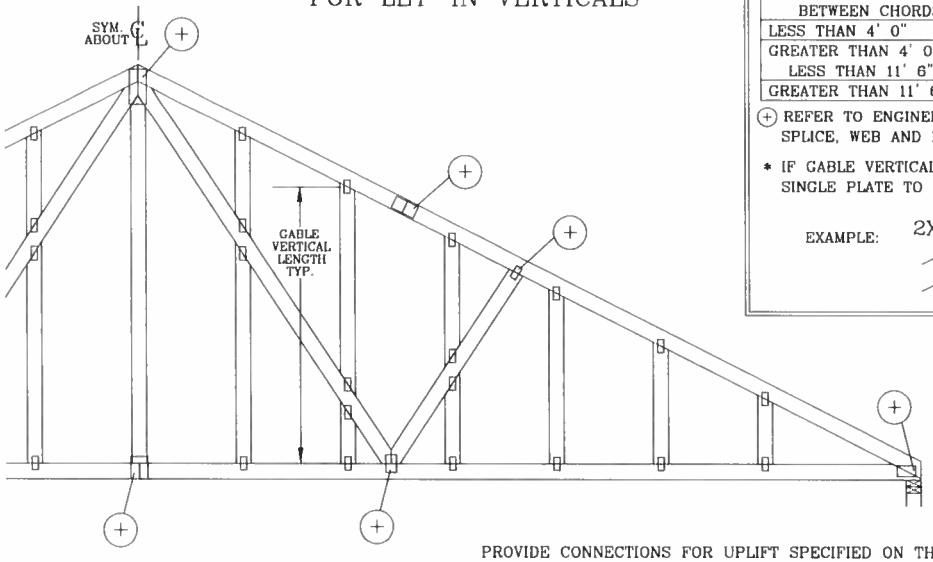


MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS



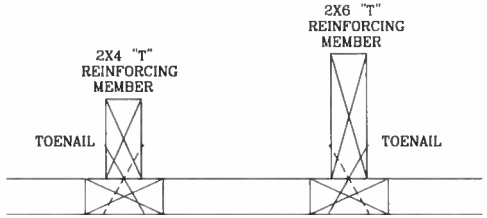
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 2X4 2X4 2X8



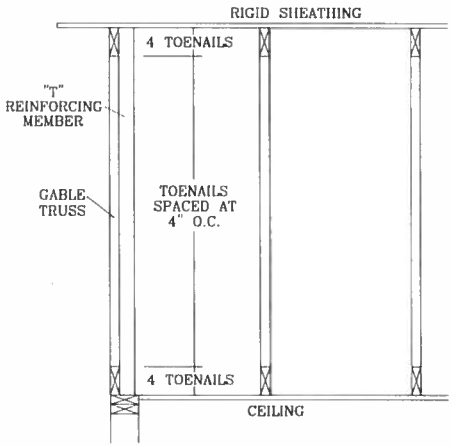
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep [illegible] 07

MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

(PIGGYBACK -)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

TOP CHORD 2x4 SP SP #2N
BOT CHORD 2x4 SP #3 OR #2N
WEBS 2x4 SP #3

*,E (4) 0.131"X1.375" SCOTCH NAILS OR EQUAL IN EACH MEMBER. TRULOX PLATE TO BE APPLIED TO EACH FACE AT 2'0" O.C. MAXIMUM SPACING. REFER TO DRAWING 142 FOR TRULOX INFORMATION. PLATES ON THE FRONT FACE OF TRUSS MAY BE OFFSET FROM THE PLATES ON THE BACK FACE AS LONG AS PLATES ARE SPACED 4'0" OC. MAX.

NOTE: PIGGYBACK VERTICALS TO BE SPACED AT 4'0" O.C. MAXIMUM.

** MAXIMUM SIZE OF 2X12, #2 HEM-FIR OR BETTER.

E - 4X6 ALPINE, 3X6 TRULOX AT 2'0" OC. MAX.

++ PIGGYBACY BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH W1.5X3 ALPINE.

* - 3X8 TRULOX PLATE OR ALPINE PIGGYBACK SPECIAL PLATE (SEE DRWG. 847,847)

TRUSSES BUILT PER THIS DETAIL DESIGNED TO BE USED FOR THE FOLLOWING:
140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC.BLDG, CAT II, EXP C.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC.BLDG, CAT II, EXP C.

NOTE: THIS DETAIL MAY ALSO BE USED FOR A MONO OR HIP-MONO PIGGYBACK USING A TYPE-C PLATE AT THE HIGH END. AND END VERTICAL WHICH IS GREATER THAN 6-0-0 IN LENGTH AND EXPOSED TO WIND MUST BE VERIFIED BY ALPINE ENGINEERED PRODUCTS.

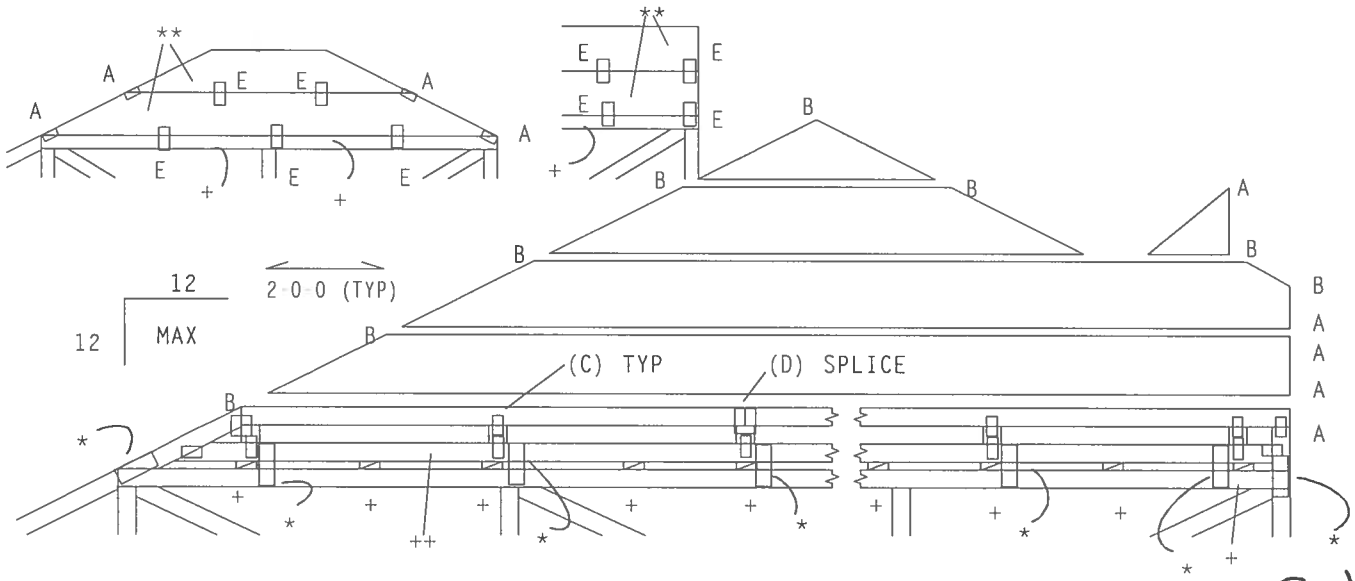
NOTE: TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" OC. MAX SPACING. ATTACH TO TOP SIDE OF SUPPORTED TRUSS TOP CHORD WITH 2-16D NAILS IN EACH TRUSS.
OR
1X4 CONTINUOUS LATERAL BRACING AT 24" OC. MAX. SPACING. ATTACH TO BOTTOM SIDE OF SUPPORTED TRUSS TOP CHORD WITH 2-16D NAILS IN EACH TRUSS. BOTTOM CHORD OF PIGGYBACK SHOULD REST DIRECTLY ON THE TOP CHORD OF THE SUPPORTED TRUSS.

NOTE: BRACING MATERIAL IS TO BE ATTACHED TO A SUITABLE SUPPORT AT EACH END, AND MUST BE #3 HEM-FIR OR BETTER.

JOINT TYPE	SPANS UP TO 30'0"	34'0"	38'0"	42'0"
A	W2X4	W2X5	W3X4	W3X5
B	W5X4	W5X5	W5X5	W5X5
C	W1X3	W1X3	W1.5X3	W1.5X3
D	W5X4	W5X4	W5X5	W5X5

WEB BRACING
UP TO 7'9" NO BRACING
7'9" TO 12'3"-1X4 "T" BRACE, SAME GRADE AND LENGTH AS WEB, ATTACH WITH 8D NAILS AT 6" OC.
12'3" TO 14'0"-2X4 "T" BRACE, SAME GRADE AND LENGTH AS WEB, ATTACH WITH 16D NAILS AT 4" OC.



42-0-0 MAXIMUM PIGGYBACK SPAN

ALTERNATE LOADING:

TCLL	20	30 PSF
TCDL	20	OR 15 PSF
BCDL	10	10 PSF
TOTL	50	55 PSF
	1.25	1.33

R1: REVISED FOR ASCE 7-02.
DLJ 09/30/2005

DETAIL: 140PB

PLT TYP. High Strength,Wave TPI-95 Design Criteria: TPI(STD) QTY 1 HI/-/1/-/-/R/-

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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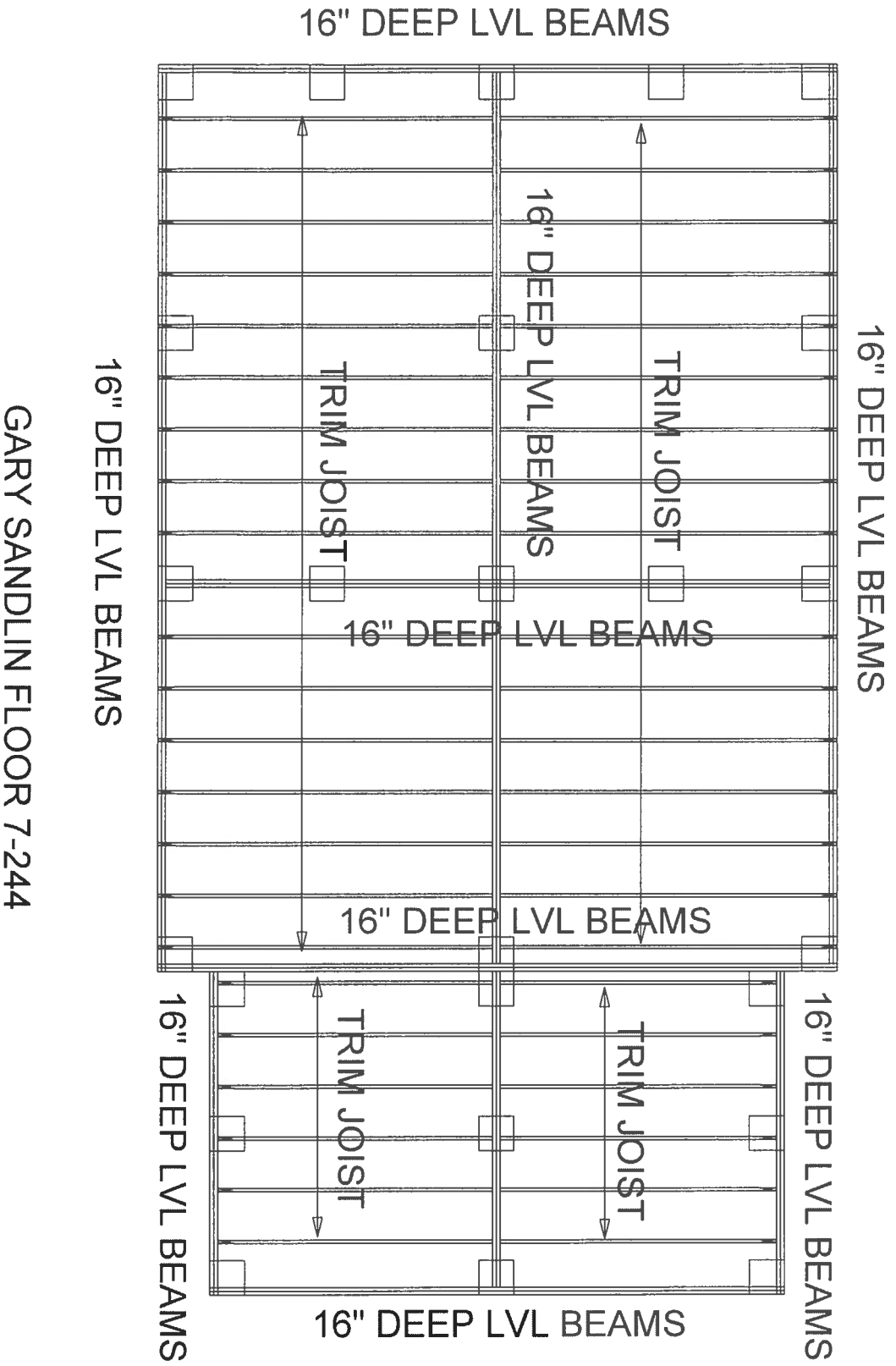
JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER Sep 11 '07

TC LL	30.0 PSF	REF R001-- 0
TC DL	7.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCUSR001 02086006
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	47.0 PSF	SEQN - 24938
DUR.FAC.	1.33	
SPACING	24.0"	JRFF- 1SQV001 R38



JOB DESCRIPTION:: Fill in later
/: GARY SANDLIN

JOB NO
7-244

PAGE NO
1 OF 1