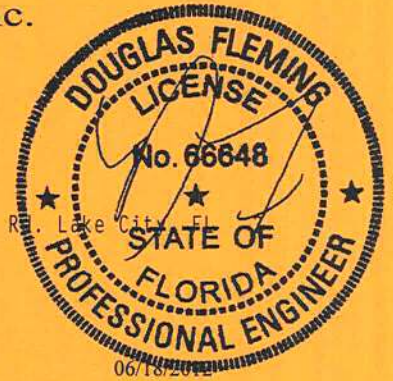


ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 2 Document ID:1UMZ487-Z0118164432



Truss Fabricator: Anderson Truss Company
Job Identification: 12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd., Lake City, FL
Truss Count: 150
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
Address:
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Douglas Fleming
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-PB120-

#	Ref	Description	Drawing#	Date
1	29318-A	69'10" Specia	12170008	06/18/12
2	29319-AV1	59'2" Stepdo	12170009	06/18/12
3	29320-AV2	65'6"8 Stepd	12170010	06/18/12
4	29321-AV3	69'10" Stepd	12170011	06/18/12
5	29322-AV4	69'10" Stepd	12170012	06/18/12
6	29323-AV5	69'10" Spec	12170013	06/18/12
7	29324-AV6	69'10" Spec	12170014	06/18/12
8	29325-AV7	69'10" Spec	12170015	06/18/12
9	29326-B1	61'2" Stepdow	12170016	06/18/12
10	29327-B2	59'2" Stepdow	12170017	06/18/12
11	29328-BP1	21'3"10 Comm	12170018	06/18/12
12	29329-BP10	30'8"10 Ste	12170019	06/18/12
13	29330-BP11	33'11"10 St	12170020	06/18/12
14	29331-BP12	33'11"10 St	12170021	06/18/12
15	29332-BP13	33'11"10 St	12170022	06/18/12
16	29333-BP14	33'11"10 Sp	12170023	06/18/12
17	29334-BP15	33'11"10 Sp	12170024	06/18/12
18	29335-BP16	33'11"10 Sp	12170025	06/18/12
19	29336-BP17	30'8"10 Spe	12170026	06/18/12
20	29337-BP18	33'11"10 Sp	12170027	06/18/12
21	29338-BP19	25'3"10 Com	12170028	06/18/12
22	29339-BP2	25'3"10 Comm	12170029	06/18/12
23	29340-BP9	33'11"10 Ste	12170030	06/18/12
24	29341-BT6	67'8" Specia	12170031	06/18/12
25	29342--CJ1V	3' Jack	12170032	06/18/12
26	29343--CJ1V	3' Jack	12170033	06/18/12
27	29344--CJ3V	5' Jack	12170034	06/18/12
28	29345--CJ3V	5' Jack	12170035	06/18/12
29	29346--CJ5V	7' Jack	12170036	06/18/12
30	29347--CJ5V	7' Jack	12170037	06/18/12
31	29348--CJ7	9' End Jack	12170038	06/18/12
32	29349-CV1	43'7" Specia	12170039	06/18/12
33	29350-CV10	43'7" Spec	12170040	06/18/12
34	29351-CV11	43'7" Spec	12170041	06/18/12
35	29352-CV12	43'7" Commo	12170042	06/18/12
36	29353-CV2	43'7" Specia	12170043	06/18/12
37	29354-CV3	43'7" Specia	12170044	06/18/12
38	29355-CV4	43'7" Specia	12170045	06/18/12

#	Ref	Description	Drawing#	Date
39	29356-CV5	43'7" Specia	12170046	06/18/12
40	29357-CV6	43'7" Specia	12170047	06/18/12
41	29358-CV7	43'7" Specia	12170048	06/18/12
42	29359-CV8	43'7" Mono H	12170049	06/18/12
43	29360-CV9	43'7" Specia	12170050	06/18/12
44	29361-CVP	7'8"10 Commo	12170051	06/18/12
45	29362-CVP1	7'8"10 Step	12170052	06/18/12
46	29363--DV	39'7" Common	12170053	06/18/12
47	29364-DV1	37'7" Common	12170054	06/18/12
48	29365--EJ7G		12170055	06/18/12
49	29367--EJ7S		12170056	06/18/12
50	29368-EJ9A	11' End Jac	12170057	06/18/12
51	29369-EJ9B	11' End Jac	12170058	06/18/12
52	29370-EJ9C	11' End Jac	12170059	06/18/12
53	29371-EV1	24'9" Common	12170060	06/18/12
54	29372-EV2	24'9" Common	12170061	06/18/12
55	29373-EV3	24'9" Common	12170062	06/18/12
56	29375--H17A		12170063	06/18/12
57	29376-H19AV	69'10" Ste	12170064	06/18/12
58	29377-H19CV	43'7" Step	12170065	06/18/12
59	29378-H20B	59'2" Stepd	12170066	06/18/12
60	29379-H21AV	69'10" Ste	12170067	06/18/12
61	29380-H23AV	65'6"8 Ste	12170068	06/18/12
62	29381-H23BP	21'3"10 St	12170069	06/18/12
63	29382-H25BP	21'3"10 St	12170070	06/18/12
64	29383-H27BP	21'3"10 St	12170071	06/18/12
65	29384-H29BP	21'3"10 St	12170072	06/18/12
66	29385--H7K		12167032	06/15/12
67	29386-H9A	69'10" Stepd	12170115	06/18/12
68	29387-H511A1	23'3" Com	12170073	06/18/12
69	29388-H513A1	23'3" Com	12170074	06/18/12
70	29389--JV7	7' End Jack	12170075	06/18/12
71	29390-JV8	7'0"8 End Ja	12170076	06/18/12
72	29391--MG1		12170077	06/18/12
73	29392--MV1	18'6" Mono	12170078	06/18/12
74	29393--MV2	18'6" Mono	12170079	06/18/12
75	29394-MV8	16'6" Mono G	12170080	06/18/12
76	29395-BT8	64'1" Specia	12170081	06/18/12

#	Ref	Description	Drawing#	Date
77	29396-BT9	64'1" Specia	12170082	06/18/12
78	29397-BT7	67'8" Specia	12170083	06/18/12
79	29398-JV8	7'0"8 Mono G	12170084	06/18/12
80	29399-H11B	61'2" Stepd	12167009	06/15/12
81	29400-ML1	16'9"12 Spec	12170085	06/18/12
82	29401-BT1	50'9"12 Spec	12170086	06/18/12
83	29402-BL1	50'9"12 Spec	12170087	06/18/12
84	29403-BL4	50'9"12 Spec	12170088	06/18/12
85	29404-BL3	50'9"12 Spec	12170089	06/18/12
86	29405-BL2	50'9"12 Spec	12170090	06/18/12
87	29406-BL5	50'9"12 Step	12170091	06/18/12
88	29407-BT3	50'9"12 Spec	12170092	06/18/12
89	29408-BT5	50'9"12 Spec	12170093	06/18/12
90	29409-BT2	50'9"12 Spec	12170094	06/18/12
91	29410-BT4	50'9"12 Spec	12170095	06/18/12
92	29411-BP8	25'3"10 Step	12170096	06/18/12
93	29412-BP3	38'3"10 Spec	12170097	06/18/12
94	29413-BP4	38'3"10 Spec	12170098	06/18/12
95	29414-BP6	25'3"10 Spec	12170099	06/18/12
96	29415-BP7	25'3"10 Spec	12170100	06/18/12
97	29416-BP5	38'3"10 Spec	12170101	06/18/12
98	29417-CVG	43'7" Common	12170004	06/18/12
99	29419--JV4	7' End Jack	12170103	06/18/12
100	29420--JV3	7' End Jack	12170104	06/18/12
101	29421--JV2	7' End Jack	12170105	06/18/12
102	29422-LG	13'5" Mono Hi	12170005	06/18/12
103	29423--JV6	7' End Jack	12170006	06/18/12
104	29424--JV5	7' End Jack	12170007	06/18/12
105	29425-H9L	13'5" Mono H	12170106	06/18/12
106	29426--JV1	7' End Jack	12170107	06/18/12
107	29428-H7L	15'6" Mono H	12170108	06/18/12
108	29429-H7G	47'4" Stepdo	12167011	06/15/12
109	29431--HS11A		12167021	06/15/12
110	29432--HS13A		12170111	06/18/12
111	29433-AV8	69'10" Spec	12167025	06/15/12
112	29434-H13B	61'2" Stepd	12167026	06/15/12
113	29435-H15B	61'2" Stepd	12167027	06/15/12
114	29436-H17B	61'2" Stepd		



Top chord 2x4 SP #1 :T2, T3 2x4 SP M-30:
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W13 2x4 SP #1:

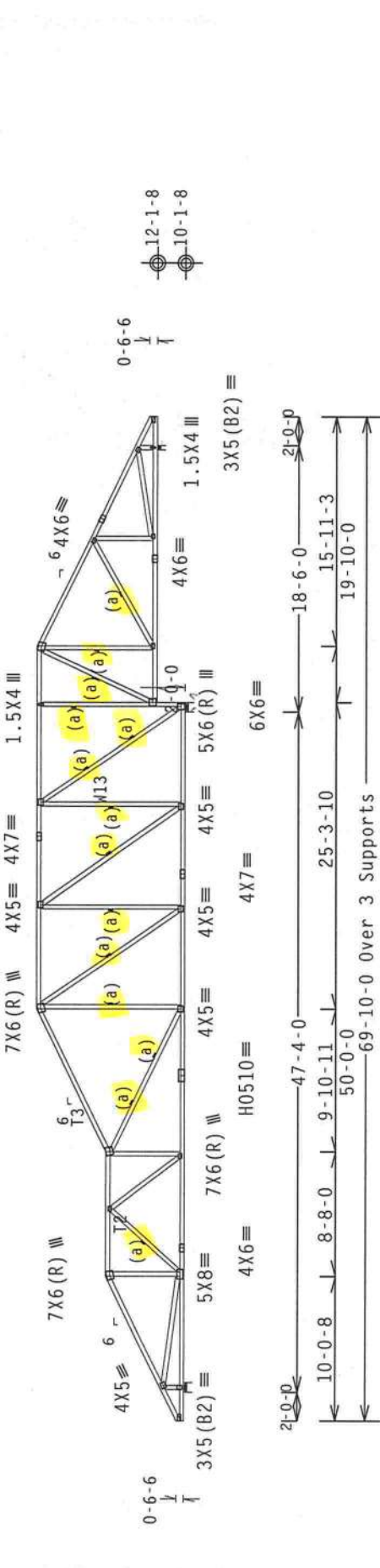
110 mph wind, 15.64 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 17.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Left and right cantilevers are exposed to wind
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 31.28 ft. from roof edge.
Wind loads and reactions based on MMFRS with additional C&C member design.
(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.



R=2127 U=0 W=8"
RL=217/-201

Note: All Plates Are 3x4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0 (0)

PLT TYP. 20 Gauge HS.Wave
R=874 U=16 W=3.5"

ALPINE		FL / - / 4 / - / R / -	Scale = .09375" / Ft.
		TC LL	20.0 PSF
		TC DL	10.0 PSF
		BC DL	10.0 PSF
		BC LL	0.0 PSF
		TOT.LD.	40.0 PSF
		DUR.FAC.	1.25
		SPACING	24.0"
		REF	R487 -- 29318
		DATE	06/18/12
		DRW	HCUSR487 12170008
		HC-ENG	DF/DF
		SEQN	8684
		JREF	1UMZ487_Z01



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x6 SP #2 :T2, T3, T4 2x4 SP #1:
Bot chord 2x6 SP #2 :B5 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W19 2x4 SP #1:

110 mph wind, 16.64 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Left end vertical not exposed to wind pressure.

Calculated horizontal deflection is 0.32" due to live load and 0.29" due to dead load.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 16.64 ft. from roof edge.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MMFRS with additional C&C member design.

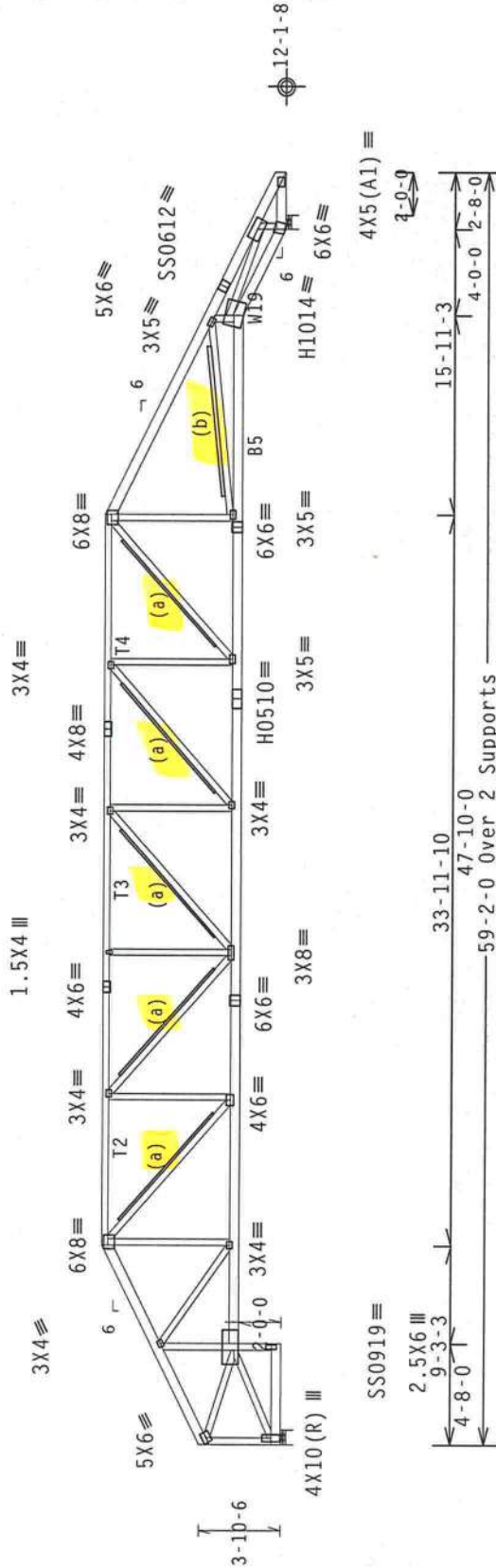
Right cantilever is exposed to wind

(a) 1x4 #3SR8 SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=2552 U=41 W=8"
RL=144/-172

PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (STD) Wave

FT/RT=10%(0%/0/0)

Scale = 1/25"/Ft.

ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET **IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WICA) for practices prior to performing these functions. Installers shall provide temporary bracing per drawings. Top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. This design is intended for use in the design of a building. It is not intended for use in the design of a structure. The responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's ICC: www.tccsa.org ITW-REG: www.itwbcg.com; TPI: www.tpinet.org; WICA: www.wicaindustry.com			
	TC LL	20.0 PSF	REF	R487 -- 29319
TC DL	10.0 PSF	DATE	06/18/12	
BC DL	10.0 PSF	DRW	HCUSR487 12170009	
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN	5748	
DUR.FAC.	1.25			
SPACING	24.0"	JREF	1UMZ487_Z01	

Top chord 2x6 SP #2 :T3, T4, T5 2x4 SP #1:

Webs 2x4 SP #3 :W24 2x4 SP #1:

Negative reaction(s) of -469# MAX. (See below) from a non-wind load case requires uplift connection.

Left and right cantilevers are exposed to wind

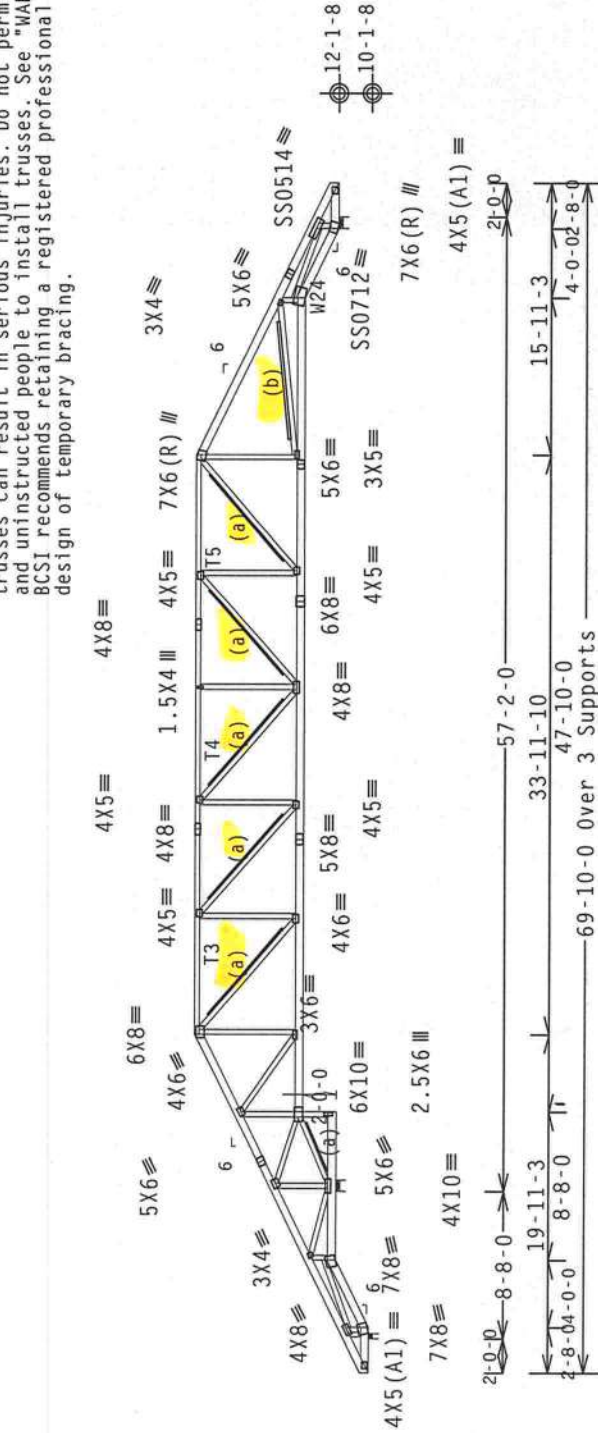
Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5" min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 31.28 ft. from roof edge.



R=-469 RW=9 U=153 W=3.5"
RI=217/-201 R=3856 U=0 W=8"

PLT TYP. 18 Gauge HS,Wave



Haines City, FL 33844
FL COA #0 278

Top chord 2x6 SP #2 :T3, T4, T5 2x4 SP M-30:

:T7 2x6 SP M-26:

Bot chord 2x6 SP #2 :B1, B3, B7 2x6 SP #1 Dense:

: B9 2x6 SP M-26:

Webs 2x4 SP #3 :W3 2x6 SP #2:

:W8, W24 2x4 SP #1:

110 mph wind, 15.64 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 17.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.18

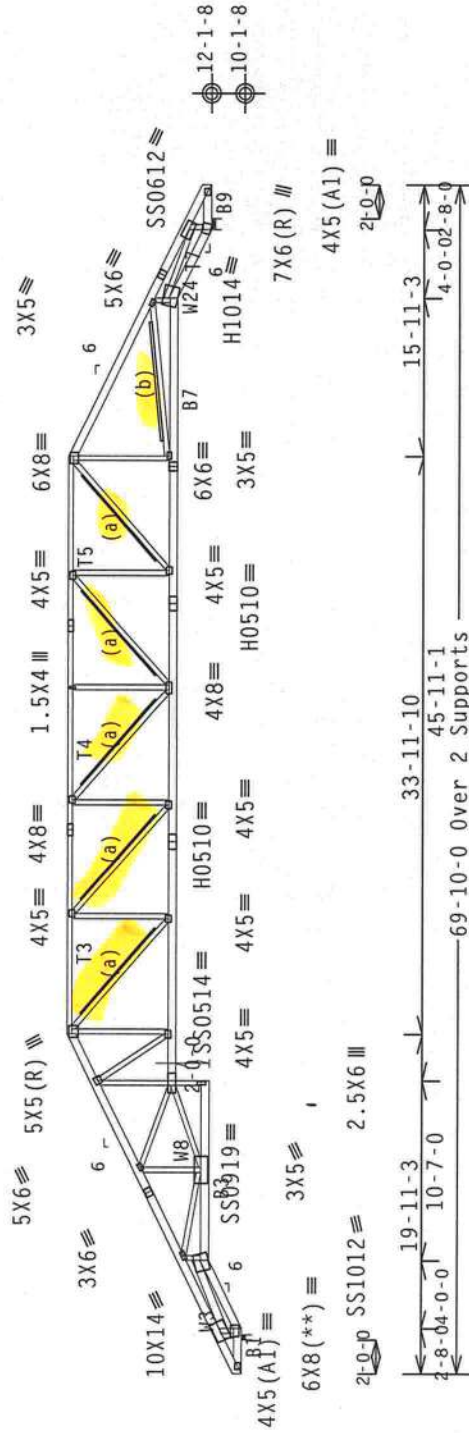
Left and right cantilevers are exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.



R=2862 U=0 W=3.5"
RL=217/-201

RL=217/-201

PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (STD)
Wave FT/RT=10% (0%) / 0 (0)

Wave

FL/-/4/-/-/R/-
Scale = .09375"/Ft.

Scale = .09375"/Ft.

TC LL	20.0	PSF	REF	R487--	29322
TC DL	10.0	PSF	DATE	06/18/12	
BC DL	10.0	PSF	DRW	HCUSR487	12170012
BC LL	0.0	PSF	HC-ENG	DF/DF	
TOT.LD.	40.0	PSF	SEQN-	5757	
DUR.FAC.	1.25				
SPACING	24.0"		JREF-	1UMZ487_Z01	

****HARNINGS** READ AND FOLLOW ALL NOTES ON THIS SHEET.**

****IMPORTANT** FURNISH THIS SET TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care fabricating, handling, shipping, installing and bracing. Read all notes carefully before commencing work. All components are made by ITW and MCA for use in conjunction with their respective products. The following practices prior to performing these functions installer shall provide:

1. Trusses shall be installed as shown on drawings. If any deviations or modifications are noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections A3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the above instructions. It is the responsibility of the contractor to ensure that all trusses are installed in accordance with the manufacturer's instructions. In the event of any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A sealant shall be applied to all joints between trusses. The manufacturer of professional engineering services shall assume full responsibility for the design shown. The ultimate acceptance of the building shall be the responsibility of the Building Designer per ANSI/TPI 1 Sec-2. For more information see: TPI general notes page; ITW-8CG; www.itwbcg.com; TPI: www.tpinet.org; MCA: www.socindustry.com; ITC: www.itccsrpae.org



ALPINE
Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(e) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x3.5".min.)nails @ 6" OC.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

(d) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3.5".min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 32.28 ft. from roof edge.

Right cantilever is exposed to wind

(H1) = (J) Hanger not calculated (2)2x6 SP #2

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

(c) 2x8 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (2) 2x6 SP #2

Note: All Plates Are 3x4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10%(0%)/0(0)

Scale = .125"/Ft.

REF R487-- 29327

DATE 06/18/12

DRW HCUSR487 12170017

HC-ENG DF/DF

SEQN- 8670

JREF- 1UMZ487_Z01

110 mph wind. 16.14 ft mean ht. ASCE 7-05. CLOSED bldg. not located within 13.00 ft from roof edge. CAT II. Exp B. Wind TC DL=5.0 psf. Wind BC DL=5.0 psf. IW=1.00 GCPI (+/-)-0.18

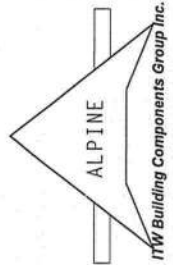
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to manufacturer publication for additional information.

Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design.

any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details. Unless noted otherwise, refer to drawings 160A-Z for standard plate positions. A seal on the

responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; NCA: www.ncaindustry.com; ICC: www.iccsafe.org



4X8=

1.5X4 III 5X8=

4X7=

5X8=

4X6=

3X6=

(e)

(d)

(c)

(b)

(a)

(b)

(a)

(b)

(a)

(b)

(a)

0-6-6

1.5X4 III

4X6=

3X5 (B2) =

4X6=

1.5X4 III

4X6=

18-6-0

15-11-3

19-10-0

3-0-0

4X6 (R) III

59-2-0 over 3 supports

4X7 25-3-10

38-8-0

17-11-3

1-6-6

12-1-8

10-1-8

0-6-6

1.5X4 III

4X6=

3X5 (B2) =

4X6=

1.5X4 III

4X6=

18-6-0

15-11-3

19-10-0

3-0-0

4X6 (R) III

59-2-0 over 3 supports

4X7 25-3-10

38-8-0

17-11-3

1-6-6

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads
-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25
TC- From 62 pif at 0.00 to 62 pif at 10.65
TC- From 62 pif at 10.65 to 62 pif at 21.30
BC- From 4 pif at 0.00 to 4 pif at 21.30

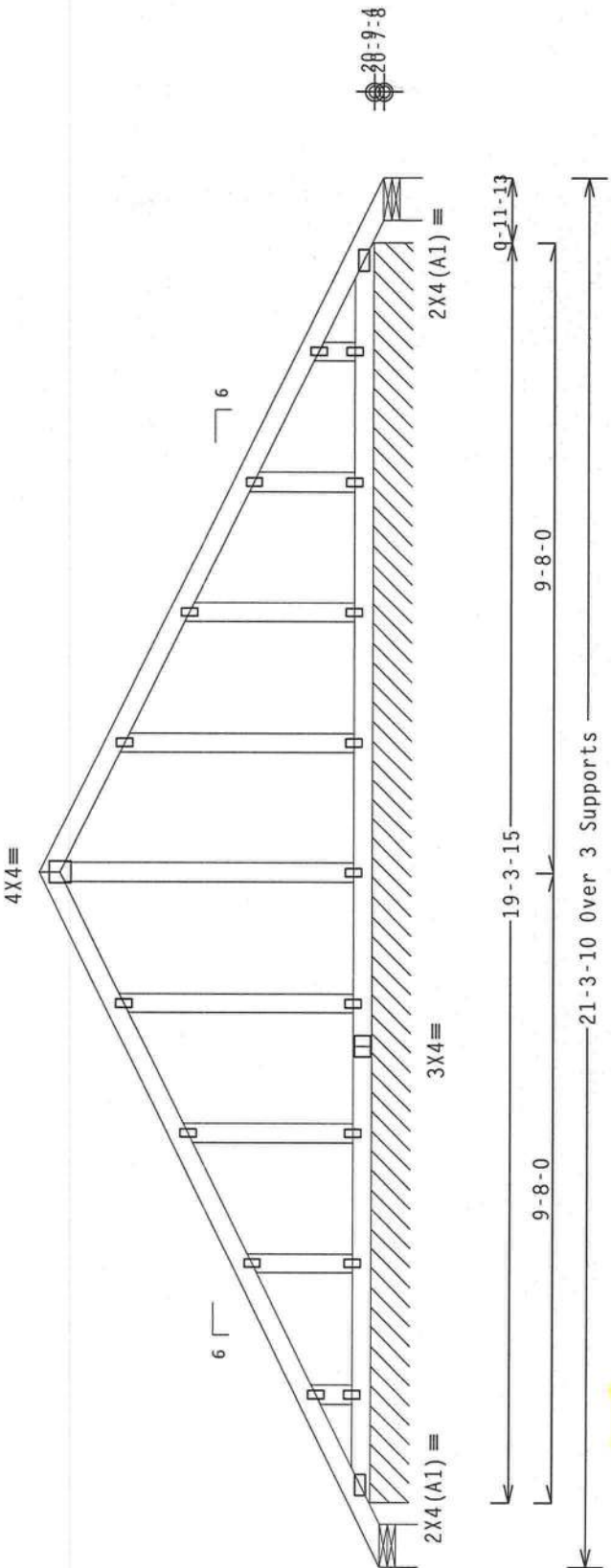
Wind loads and reactions based on MWFRS with additional C&C member design.
Deflection meets L/240 live and L/180 total load.
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 23.29 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCP(+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 23.29 ft. from roof edge.

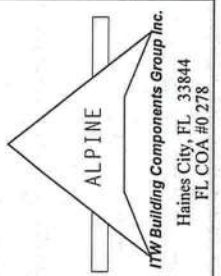


R=21 U=4 W=7.825"
RL=1099=Q99PLF U=9 PLF W=19-3-15

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave	Scale = .375"/Ft.	
ALPINE		



Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 9.07
TC- From 62 plf at 9.07 to 62 plf at 24.90
TC- From 62 plf at 24.90 to 62 plf at 33.97
BC- From 4 plf at 0.00 to 4 plf at 33.97

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

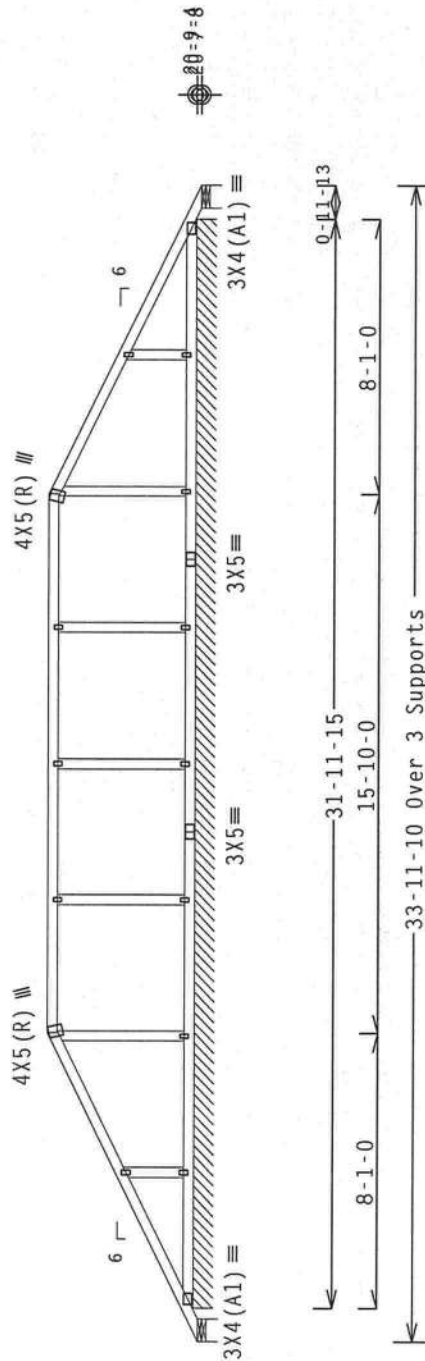
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 22.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

MWFRS loads based on trusses located at least 22.89 ft. from roof edge.



R=22 Rv=49 U=55 W=7.826"

RL=05/098LF U=13 PLF W=31-11-15

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

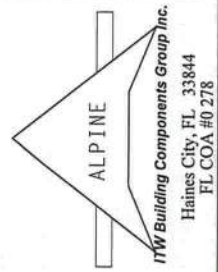
FL/-/4/-/-/R/-

Scale = .1875" / Ft.

TC LL	20.0 PSF	REF	R487--	29331
TC DL	10.0 PSF	DATE	06/18/12	
BC DL	10.0 PSF	DRW	HCUSR487	12170021
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	7797	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1UMZ487_Z01	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and MCA) for practices prior to performing these functions. Installers shall provide temporary bracing per detail unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the 3D detail. Detailing and fabrication shall be in accordance with the ITWBCG design and detail drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: general notes page: ITW-860; www.itwbcg.com; TPI: www.tpinet.org; MCA: www.sbcindustry.com; ICC: www.iccsafe.org



This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Sample	Time (sec)	Flow (ml/min)	Pressure (mm Hg)
TC-From	62 plf at 0.00 to	62 plf at 11.07	62 plf at 11.07
TC-From	62 plf at 11.07 to	62 plf at 22.90	62 plf at 22.90
TC-From	62 plf at 22.90 to	62 plf at 33.97	62 plf at 33.97
BC-From	4 plf at 0.00 to	4 plf at 33.97	4 plf at 33.97

1110 mph wind, 23.39 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, Iw=1.00 GCpi(+)=0.18

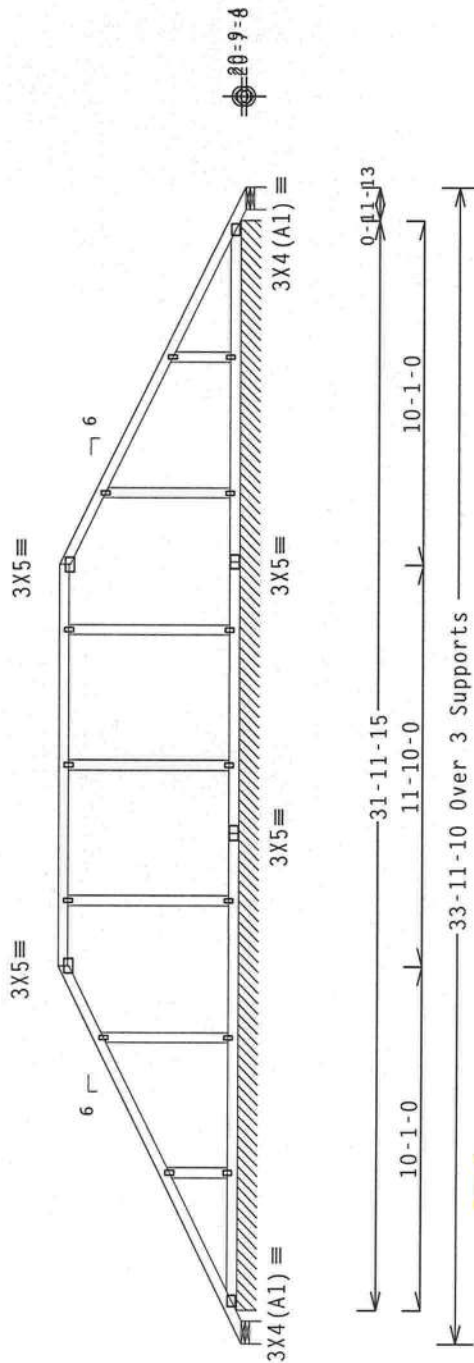
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

MMWFRS loads based on trusses located at least 23.39 ft. from roof edge.

Refer to DWG PB1200310 for piggyback details.



R = -20 R_W = 61 U = 65 W = 7.826*
 RI = 70-800 U = 11 PUF W = 31-11-15
 P = -20 R_W = 11 U = 14 W = 7.826"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

OFF R487-- 29332

DATE 06/18/12

DATE 00/18/12

RW HCUSR487 12170022

IC-ENG DF/DF

EQN- 7795

111M7407 701

*****IMPORTANT****

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, shipping, installing and bracing. After to be followed by the latest edition of RCSC's *Specification for Structural Steel Buildings*. The new specification provides a number of new design provisions, including provisions for the use of high-strength bolts, and provisions for the use of high-strength steel. The new specification also provides a number of new design provisions, including provisions for the use of high-strength bolts, and provisions for the use of high-strength steel. The new specification also provides a number of new design provisions, including provisions for the use of high-strength bolts, and provisions for the use of high-strength steel.

[illegible]

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

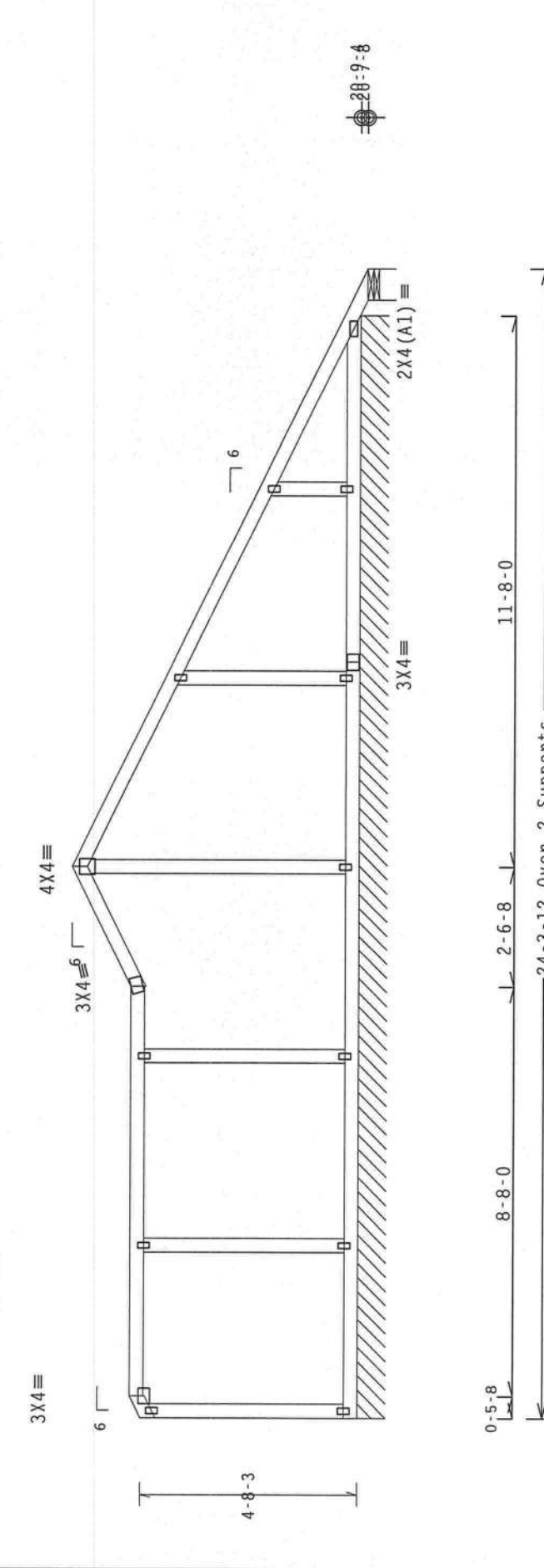
110 mph wind, 23.79 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, lw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.

Wind loads and reactions based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Refer to DWG PB1200310 for piggyback details.



0-5-8 8-8-0 2-6-8 11-8-0

24-3-12 Over 2 Supports

R=69 PLF U=15 PLF W=23-3-15

RL=2/-5 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

DOUGLAS FLEMING

FLORIDA

PROFESSIONAL ENGINEER

No. 66648

10/03/11/0200.21

FL / - / 4 / - / - / R / -		Scale = .3125" / Ft.	
TC LL	20.0 PSF	REF	R487 -- 29333
TC DL	10.0 PSF	DATE	06/18/12
BC DL	10.0 PSF	DRW	HCUSR487 12170023
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	285867
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1UMZ487_Z01

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. The fabricator shall follow the latest edition of ECSI (Building Components Suppliers Institute) Manual for the proper practices prior to performing these functions. Installers shall provide temporary bracing per ECSI Manual. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint and web bracing shall have bracing installed per ECSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. The responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MCA: www.sbcindustry.com; ICC: www.iccsafe.org

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 pif at 0.00 to 62 pif at 6.11
TC- From 62 pif at 6.11 to 62 pif at 14.77
TC- From 62 pif at 14.77 to 62 pif at 21.32
TC- From 62 pif at 21.32 to 62 pif at 33.97
BC- From 4 pif at 0.00 to 4 pif at 16.94
BC- From 4 pif at 16.94 to 4 pif at 33.97

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 23.79 ft. from roof edge.

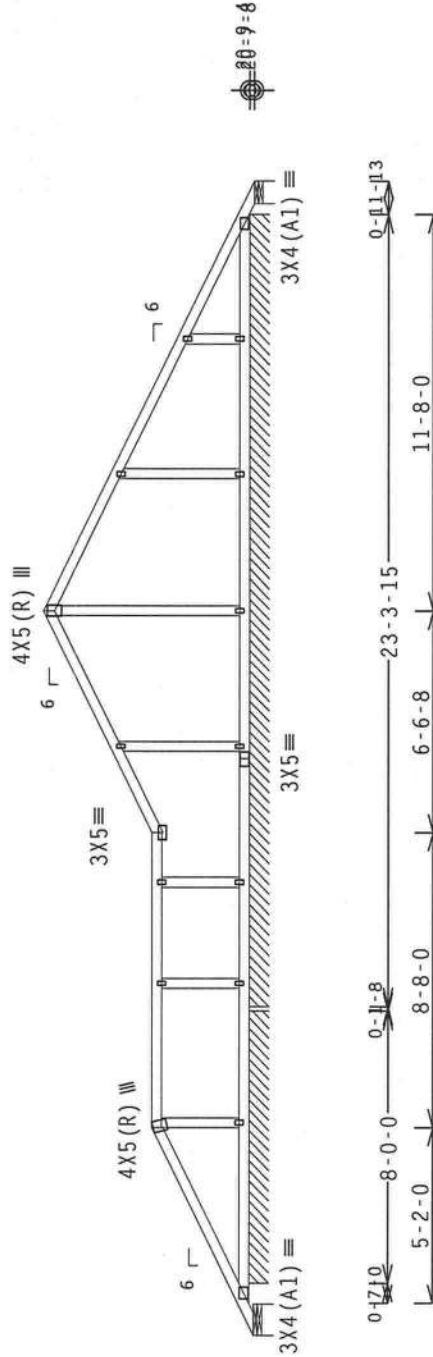
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 23.79 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. IW=1.00 GCPI(+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=125 U=9 W=11"
RL=134/53.3/F U=11 PLF W=8-0-0
R=72 PLF U=12 PLF W=23-3-15
Rw=6 U=6 W=7.827"

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%) (0 (0)

PLT TYP. Wave

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections 83, 87 or 88, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's drawings and specifications. ITW-BCG: www.itwbcg.com; TPI: www.tpinat.org; WCA: www.bcsiindustry.org; ICC: www.iccsafe.org



TY:	FL/-/4/-/-/R/-	Scale = .1875"/Ft.
TC LL	20.0 PSF	REF R487 -- 29335
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUSR487 12170025
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 7772
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UMZ487_Z01

Top chord 2x6 SP #2 :T6 2x6 SP SS:
 Bot chord 2x6 SP #2 :B6, B7, B8 2x6 SP #1 Dense:
 Webs 2x4 SP #3
 :W2, W10, W15, W17, W21 2x4 SP #1:

1110 mph wind, 15.64 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 17.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.18

Left cantilever is exposed to wind

Calculated horizontal deflection is 0.24" due to live load and 0.26" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

WFRS loads based on trusses located at least 31.28 ft. from roof edge.

Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

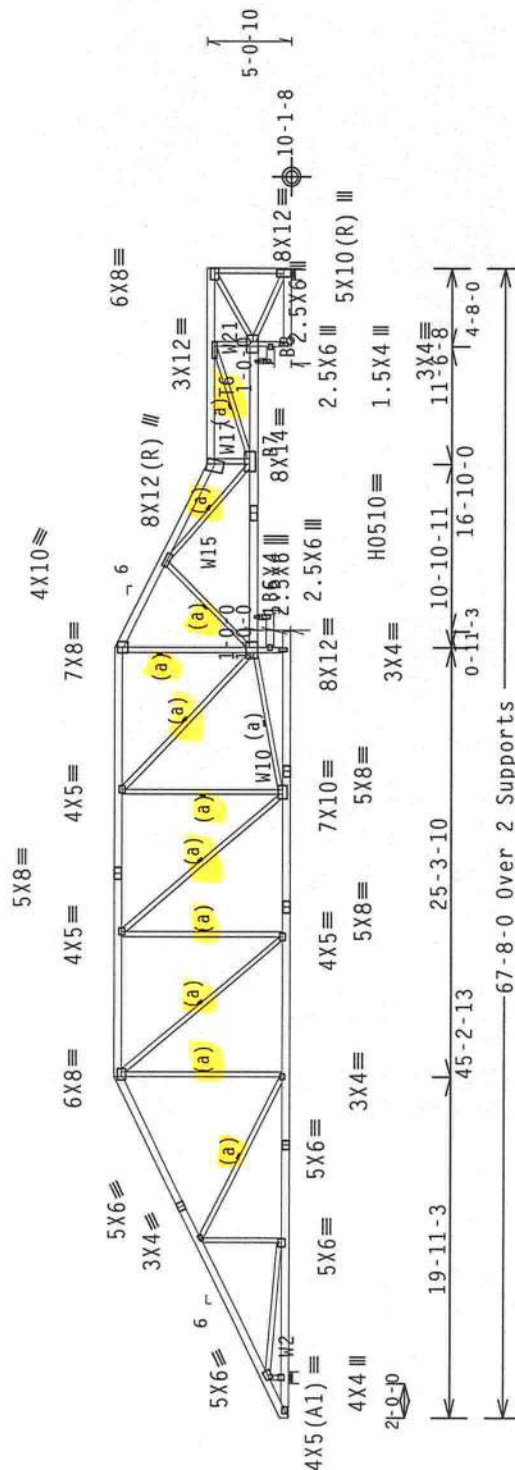
Wind Loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.



R=2886 U=0 W=8"
RL=197/-162

2687 11=0 W=8"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

$$FL1 - 141 - 1 - 1R1 - Scale = .09375" / Ft.$$

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Recommendations are given for design, detailing and construction (including Component Safety Information, by TPI and MICA) for all types of trusses. The book also contains information on various practices or techniques used in the field. Other subjects noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per RGSF sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1 or for any shipping, installation, or handling of the truss. ITWBCG shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1 or for any shipping, installation, or handling of the truss. Apply plates to each face of truss and position as shown above and on the bottom flange of the truss. Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: The general notes page: IN-86G; www.itwbcg.com; TPI: www.tpiinst.org; WTC: www.saciindust.com; ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1 or for any shipping, installation, or handling of the truss. ITWBCG shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1 or for any shipping, installation, or handling of the truss.

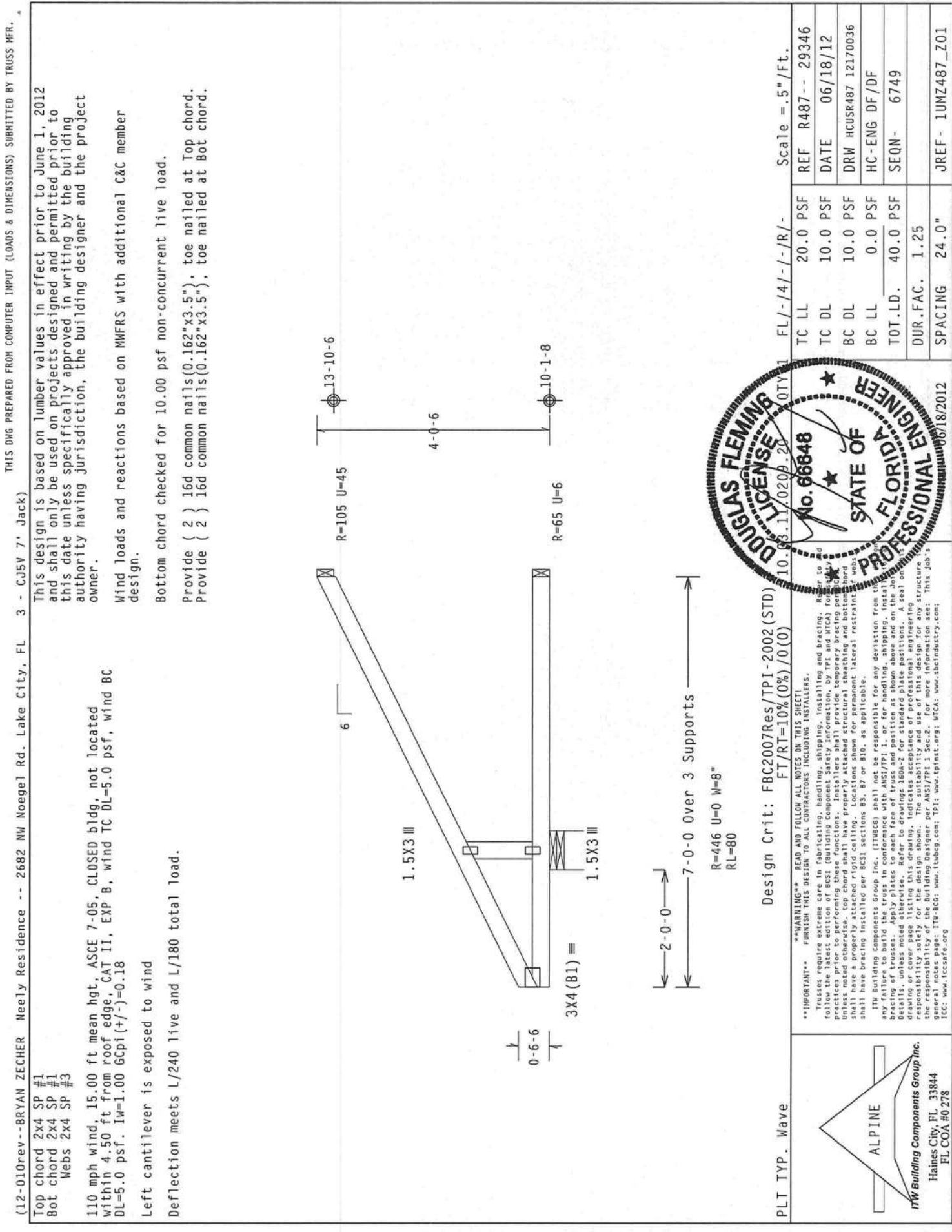
ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

The seal is circular with a double-lined border. The outer ring contains the text "DOUGLAS FLEMING" at the top, "LICENSE" on the left, "No. 66648" in the center, "STATE OF" on the right, and "FLORIDA" at the bottom. The inner ring contains the text "PROFESSIONAL ENGINEER" at the top, "10/03/11" on the left, and "EXPIRATION DATE" on the right. A signature is written across the seal. To the right of the seal, the text "06/18/2012" is visible.

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT. LD.	40.0	PSF
DUR. FAC.	1.25	
SPACING	24.0"	

REF	R487 -	29341
DATE	06/18/12	
DRW	HCUSR487	12170031
HC - ENG	DF/DF	
SEQN -	6170	
JREF -	1UMZ487	Z01



WMFRS loads based on trusses located at least 30.00 ft. from roof edge.



Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpl(+/-)=0.18

Left cantilever is exposed to wind

Deflection meets L/240 live and L/180 total load.

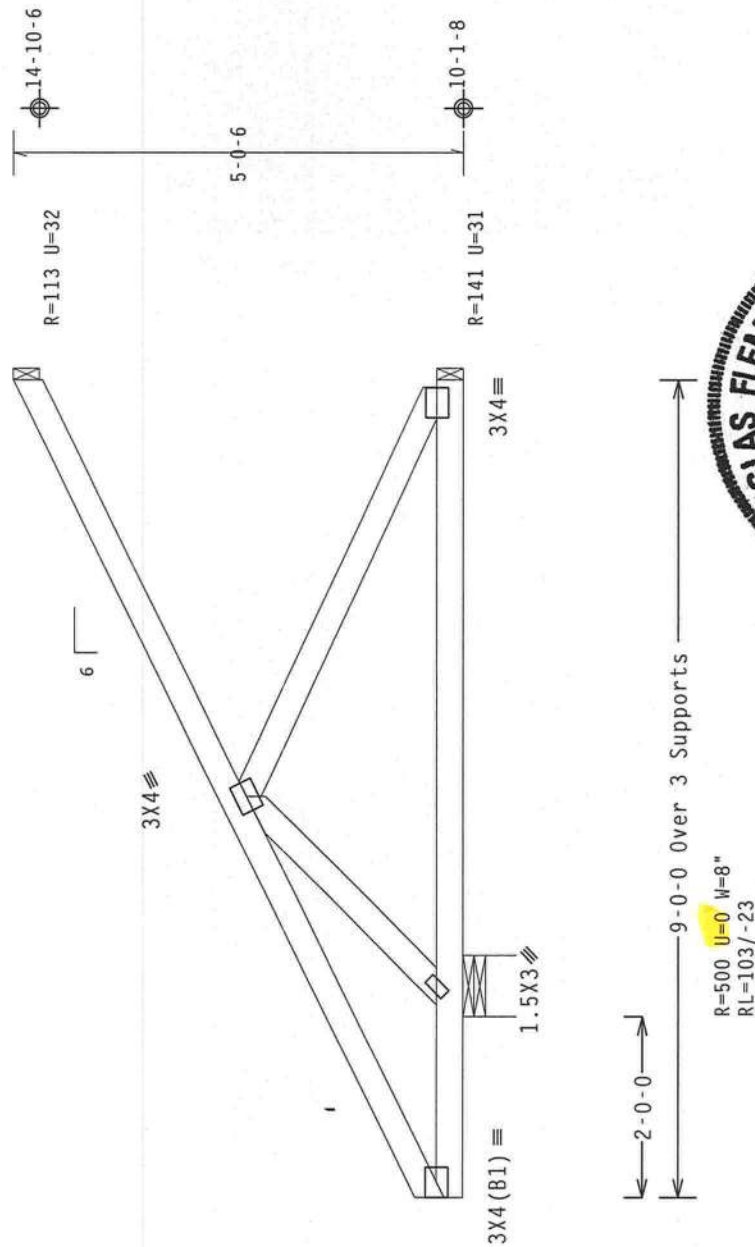
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Professional Engineer Seal for Douglas Fleming, State of Florida, License No. 68648, expires 10/8/11.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487 - - 29348
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUR487 12170038
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 4974
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UMZ487 Z01

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

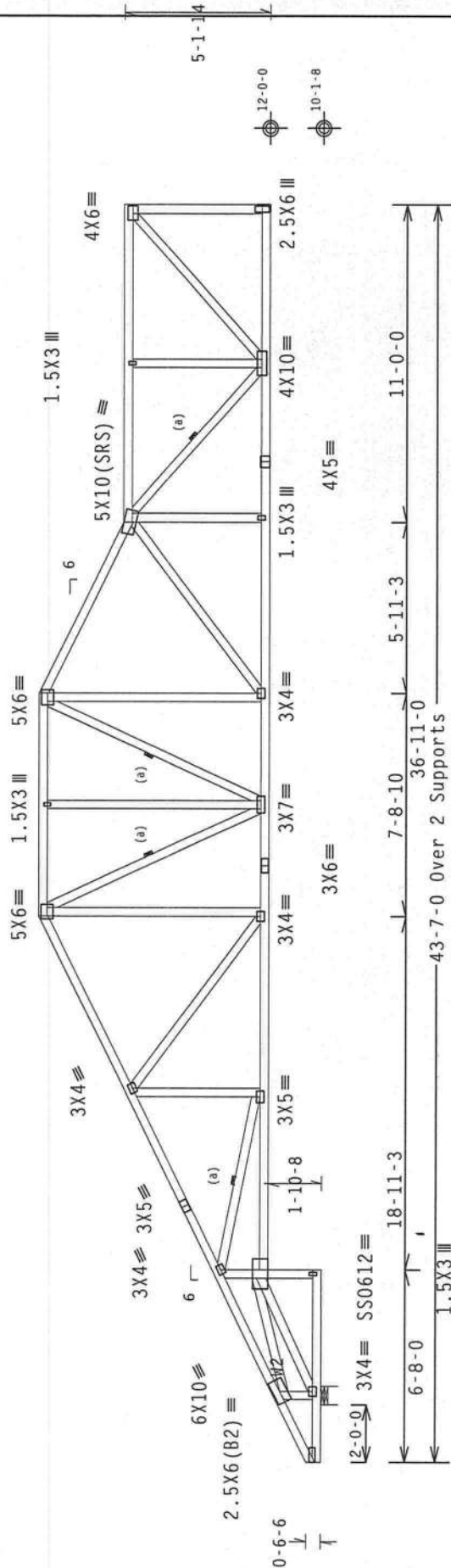
(12-010)rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - CV11 43'7" Special
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W2 2x4 SP #1:
110 mph wind, 15.39 ft mean ht, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(1/2)=0.18
Left cantilever is exposed to wind

Calculated horizontal deflection is 0.16" due to live load and 0.17" due to dead load.
These support conditions used at bearings indicated
(H1) - Simpson HUS26 w/ (3)2x6 SP H-26 supporting member.
(14) 0.148"x3" nails into supporting member.
(4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

Right end vertical not exposed to wind pressure.
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.
(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.
WFERS loads based on trusses located at least 30.78 ft. from roof edge.



R=1687 U=54 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (3) 2x6 SP #2



PLT TYP. 18 Gauge HS.Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0 (0)

Scale = 1/8" = 1'-0"

TC LL	20.0 PSF	REF	R487 -- 29351
TC DL	10.0 PSF	DATE	06/18/12
BC DL	10.0 PSF	DRW	HCUSR487 12170041
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	6699
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1UMZ487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 : #2, #15 2x4 SP #1:
110 mph wind, 15.39 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 G_{CF}(+/-)-0.18

Left cantilever is exposed to wind
Calculated horizontal deflection is 0.15" due to live load and 0.16" due to dead load.

These support conditions used at bearings indicated
(H1) 0.148"x3" nails into supporting member.
(4) 0.148"x3" nails into supporting member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

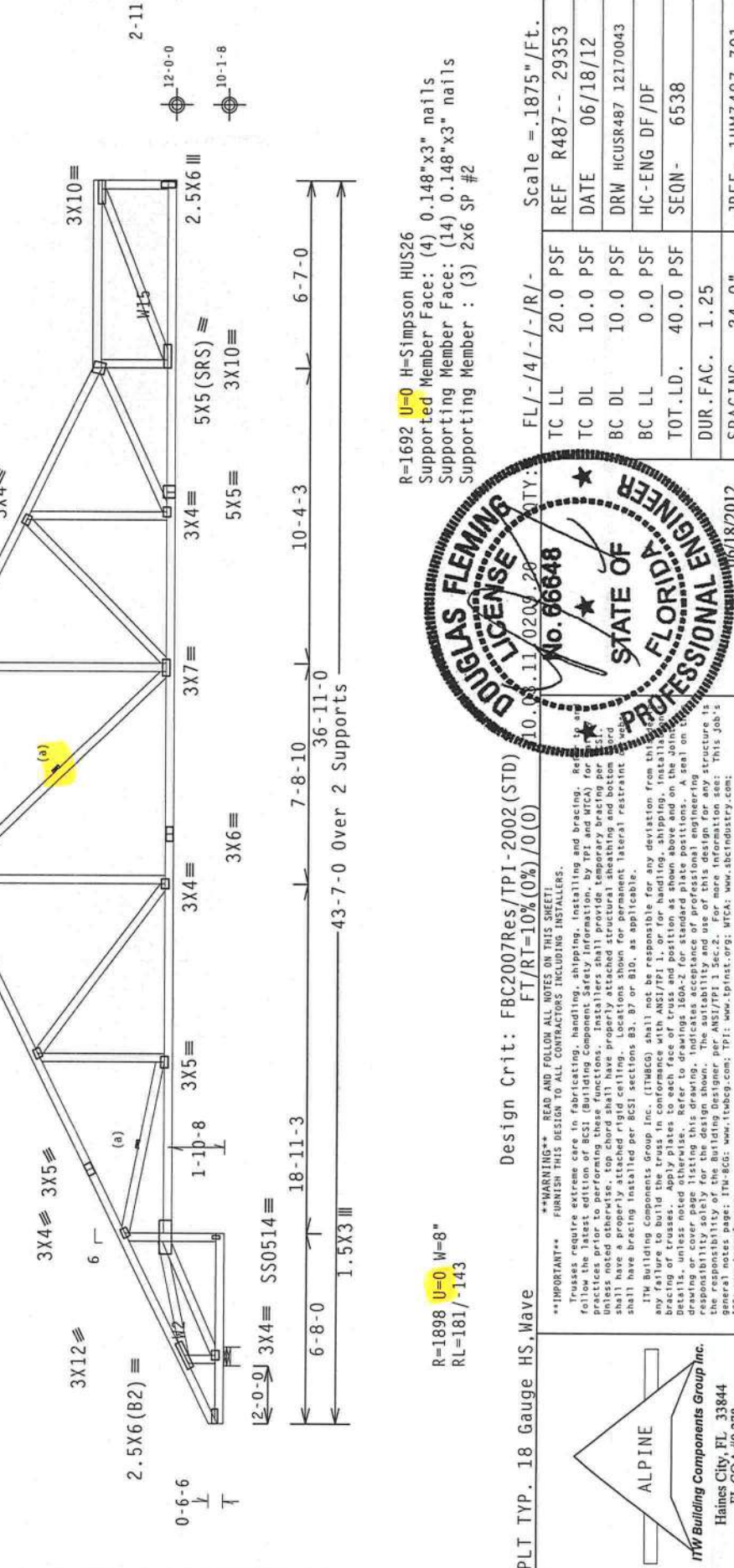
Wind loads and reactions based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.39 ft. from roof edge.



R=1692 U=0 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member: (3) 2x6 SP #2

R=1898 U=0 W=8"
RL=181/-143

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. 18 Gauge HS,Wave

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF	R487--	29353
TC DL	10.0 PSF	DATE	06/18/12	
BC DL	10.0 PSF	DRW	HCUSR487	12170043
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN	6538	
DUR.FAC.	1.25			
SPACING	24.0"	JREF	1UMZ487_Z01	

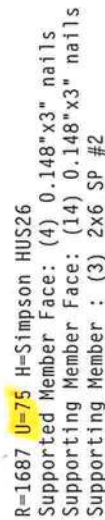


ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for practices prior to performing these functions. Installers shall provide temporary bracing per BCSI (Building Component Safety Information, by TPI and WCA) for all trusses. Trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this drawing or for any failure to build the truss in accordance with the BCSI specifications. Bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing the design shown. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. For more information see: This Job's general notes page: ITW-BCSI: www.itwbcg.com; TPI: www.tpiinc.org; WCA: www.bcsiindustry.com; ICC: www.iccsafe.org

Webs 2x4 SP #3 :W2, W6 2x4 SP #1:

Deflection meets L/240 live and L/180 total load.

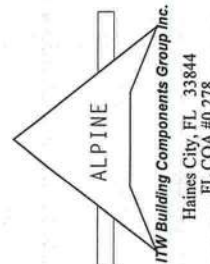


R=1903 U=0 W=8"
RL=161/-97

FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-
Scale = .1875"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

[illegible][illegible]

TC LL	20.0	PSF	REF R487 -- 29356
TC DL	10.0	PSF	DATE 06/18/12
BC DL	10.0	PSF	DRW HCUSR487 12170046
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 6601
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1UMZ487 Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MHFRS with additional C&C member design. Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MHFRS loads based on trusses located at least 30.78 ft. from roof edge.

Left cantilever is exposed to wind

Calculated horizontal deflection is 0.24" due to live load and 0.26" due to dead load.

These support conditions used at bearings indicated

(H1) - Simpson HUS26 w/ (3) 2x6 SP H-26 supporting member.

(14) 0.148"x3" nails into supporting member.

(4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #1 :T2 2x4 SP H-30:

Bot chord 2x4 SP #1 :W2, W6 2x4 SP #1:

110 mph wind, 15.39 ft mean ht. ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(t/-)=0.18

Left cantilever is exposed to wind

Calculated horizontal deflection is 0.24" due to live load and 0.26" due to dead load.

These support conditions used at bearings indicated

(H1) - Simpson HUS26 w/ (3) 2x6 SP H-26 supporting member.

(14) 0.148"x3" nails into supporting member.

(4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #1 :T2 2x4 SP H-30:

Bot chord 2x4 SP #1 :W2, W6 2x4 SP #1:

110 mph wind, 15.39 ft mean ht. ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(t/-)=0.18

Left cantilever is exposed to wind

Calculated horizontal deflection is 0.24" due to live load and 0.26" due to dead load.

These support conditions used at bearings indicated

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(4) 0.148"x3" nails into supported member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Top chord 2x4 SP #1 :T2 2x4 SP H-30:

Bot chord 2x4 SP #1 :W2, W6 2x4 SP #1:

110 mph wind, 15.39 ft mean ht. ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(t/-)=0.18

Left cantilever is exposed to wind

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

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Top chord 2x4 SP #1
Bot chord 2x4 SP #1
WBS 2x4 SP #3 :W2, W8 2x4 SP #1:
110 mph wind, 15.39 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(+/-)=0.18
Left cantilever is exposed to wind

Calculated horizontal deflection is 0.23" due to live load and 0.24" due to dead load.
These support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (3)2x6 SP W-26 supporting member.
(14) 0.148"x3" nails into supporting member,
(14) 0.148"x3" nails into supported member.

(b) 2x6 #3 or better "T" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min. nails @ 6" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
MMFRS loads based on trusses located at least 30.78 ft. from roof edge.

Right end vertical not exposed to wind pressure.
H = recommended connection based on manufacturer tested capacities and calculations.
Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min. nails @ 6" OC.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) /0 (0)

PLT TYP. 20 Gauge HS.Wave

R=1687 U=156 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (3) 2x6 SP #2

R=1903 U=14 W=8"
RL=147/-63

Scale = .1875"/Ft.

FL/-4/-/-R/-

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25
SPACING 24.0"

REF R487-- 29359
DATE 06/18/12
DRW HCUR487 12170049
HC-ENG DF/DF
SEQN- 6662
JREF- 1UMZ487_Z01

10.08.11/0208.28

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
10/18/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

110 mph wind, 15.39 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 GCP(+/-)=0.18

Left cantilever is exposed to wind

Calculated horizontal deflection is 0.23" due to live load and 0.24" due to dead load.

These support conditions used at bearings indicated
(H1) = Simpson HUS26 w/ (3)2x6 SP W-26 supporting member.
(14) 0.148"x3" nails into supporting member,
(14) 0.148"x3" nails into supported member.

(b) 2x6 #3 or better "T" brace, 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min. nails @ 6" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
MMFRS loads based on trusses located at least 30.78 ft. from roof edge.

Right end vertical not exposed to wind pressure.
H = recommended connection based on manufacturer tested capacities and calculations.
Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

(a) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min. nails @ 6" OC.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

Special loads

	Dur.Fac.=1.25 / Plate Dur.Fac.=1.25
TC- From	62 pif at -0.90 to 62 pif at 2.91
TC- From	62 pif at 2.91 to 62 pif at 6.73
BC- From	4 pif at -0.90 to 4 pif at 6.73

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

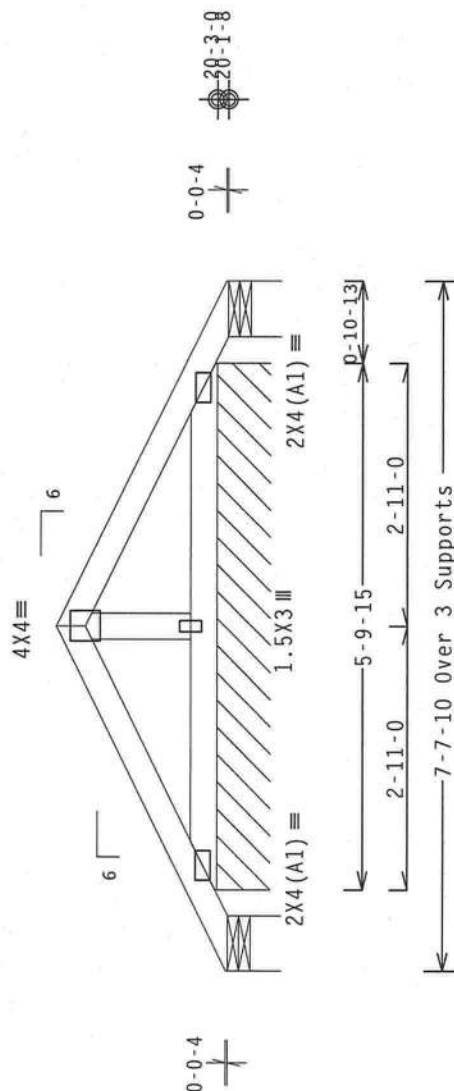
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 21.10 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, lw=1.00 GCpi (+/-)=0.18

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

WMFRS loads based on trusses located at least 21.10 ft. from roof edge.



R=-3 RW=20 U=22 W=7.326"
RL=37/-8781 PLF U=10 PLF

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

0 EI / - / 4 / - / - / R / -

Scale = 5"/Ft

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
BEFORE THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information, by TPI and BCSA for information concerning the proper handling and bracing of trusses. Installers shall provide temporary bracing and bracing to conforming these instructions. Trusses shall have a property attached rigid ceiling. Trusses shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

ITW Building Components Group Inc. (ITWBCG) with ANSIP/TPI, will not be responsible for any deviation from the design shown on this drawing. The responsibility of the Building Designer per ANSIP/TPI 1 Sec.2. For more information see: The general notes page: ITW-80G; www.itwbcg.com; TPI: www.tpinet.org; WICA: www.abctindustry.com.

ALPINE

TW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Haines City, FL 33844
FL COA #0278

Job's

REF- 1UMZ487_701

SPACING 24.0"

Job's

REF- 1UMZ487_701

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W2 2x4 SP #1:

(++) - This plate works for both joints covered.

110 mph wind, 15.61 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load.

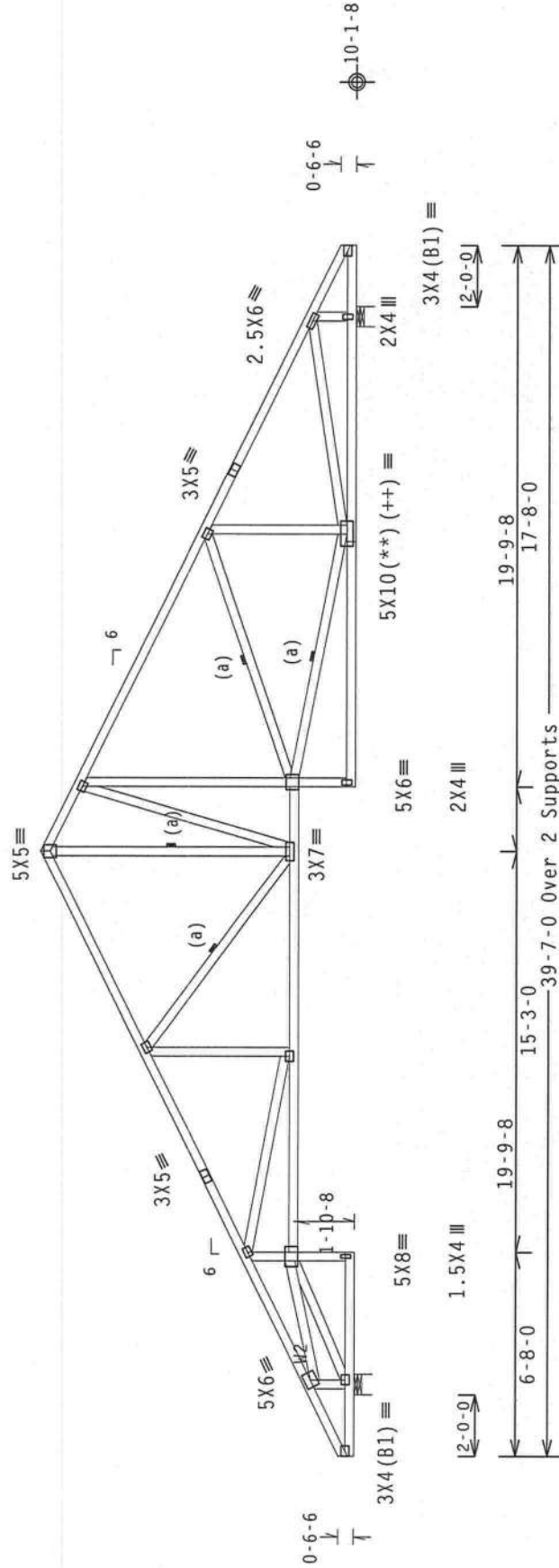
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Left and right cantilevers are exposed to wind

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.61 ft. from roof edge.



R=1637 U=0 W=8
RL=216/-216

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)

PLT TYP. Wave

FT/RT=10% (0%)/0 (0)

10'-6" 11'-0209.20

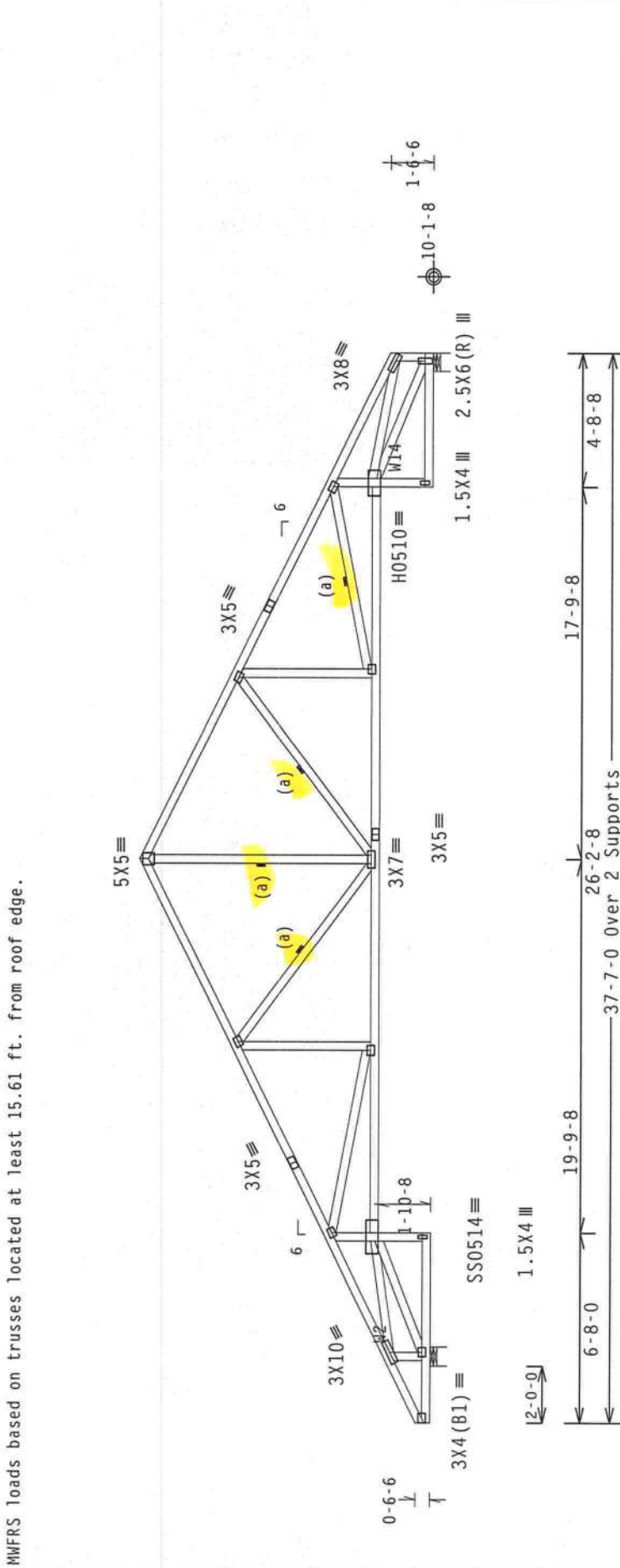
FL/-/4/-/-/R/- Scale = .1875"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.		TC LL	20.0 PSF	REF	R487--	29363
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of RCSI (Building Code of America) for details on bracing and installation. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraining webs shall have bracing installed per RCSI sections B3, B7 or B10, as applicable.		TC DL	10.0 PSF	DATE	06/18/12	
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of trusses. Apply plates to each face of truss and position as shown above and on the job. Drawing or cover page listing this drawing indicates acceptance of performance of any structure for which the building designer has accepted responsibility. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MTCA: www.sbcindustry.com; ICC: www.iccsafe.org		BC DL	10.0 PSF	DRW	HCUSR487	12170053
		BC LL	0.0 PSF	HC-ENG	DF/DF	
		TOT.LD.	40.0 PSF	SEQN-	6431	
		DUR.FAC.	1.25			
		SPACING	24.0"	JREF-	1UMZ487_Z01	



Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W2, W14 2x4 SP #1:
110 mph wind, 15.61 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge. CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18
Left cantilever is exposed to wind

(a) Continuous lateral bracing equally spaced on member.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 15.61 ft. from roof edge.
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.
Wind loads and reactions based on MWFRS with additional C&C member design.
Calculated horizontal deflection is 0.17" due to live load and 0.18" due to dead load.
Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 3X4 Except As Shown.
PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (STD)
Wave
R=1657 U=0 W=8
RL=208/-198
R=1438 U=0 W=8
Scale = .1875" / Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
10/06/18/2012

****IMPORTANT****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for best practices prior to performing these functions. Installers shall provide temporary bracing per C&C unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or any other use of the truss. Refer to drawings 160A-2 for standard plate positions and details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions and details, unless noted otherwise. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.sbcindustry.com; IBC: www.iccsafe.org

REF	DATE	DRW	HC-ENG	DF/DF
R487--	06/18/12	HCUSR487	12170054	
				6497
				1.25
				24.0"

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg. Located anywhere in roof, CAT II.
EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $W=1.00 GCP(\frac{1}{2})=0.18$

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

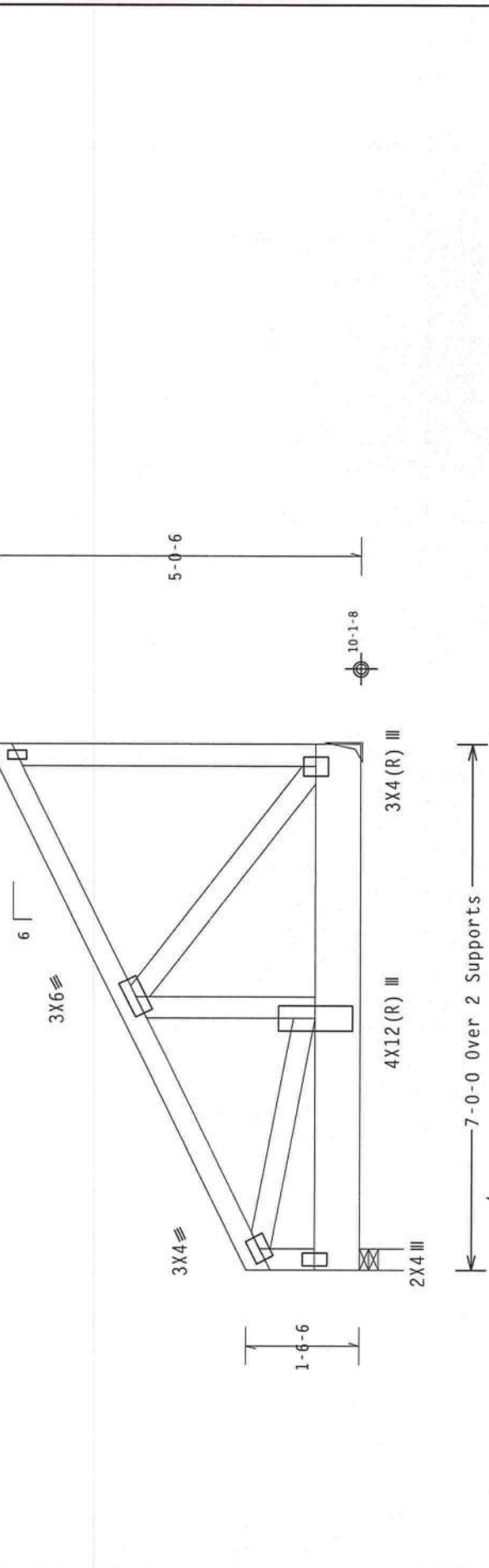
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Special loads Dur.Fac.=1.25 / Plate Dur.Fac.=1.25
TC- From 62 plf at 0.00 to 62 plf at 7.00
BC- From 10 plf at 0.00 to 10 plf at 7.00
BC- 978.22 lb Conc. Load at 1.90, 3.90, 5.90

Wind loads and reactions based on MFRS with additional C&C member design.

These support conditions used at bearings indicated
(H1) - Simpson HUS26 w/ (1)2x6 SP R-26 supporting member.
(14) 0.162"x3.5" nails into supporting member.
(4) 0.162"x3.5" nails into supported member.



R=1887 U=0 H-Simpson HUS26
Supported Member Fastener 0.162"x3.5" nails
Supporting Member Fastener 0.162"x3.5" nails
Supporting Member HUS26

R=1555 U=0 W=3.5"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5" / Ft.

ALPINE Haines City, FL 33844 FL COA #0 278	No. 66648 06/18/2012				TC LL	20.0 PSF	REF	R487 --	29365
	DATE				TC DL	10.0 PSF	DATE	06/18/12	
	DRW				BC DL	10.0 PSF	DRW	HCUSR487	12170055
	HC-ENG DF/DF				BC LL	0.0 PSF	HC-ENG	DF/DF	
	SEQN- 5021				TOT.LD.	40.0 PSF	SEQN-	5021	
DUR.FAC. 1.25				SPACING		24.0"	JREF- 1UMZ487_Z01		

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf(+/-)=0.18

Left cantilever is exposed to wind

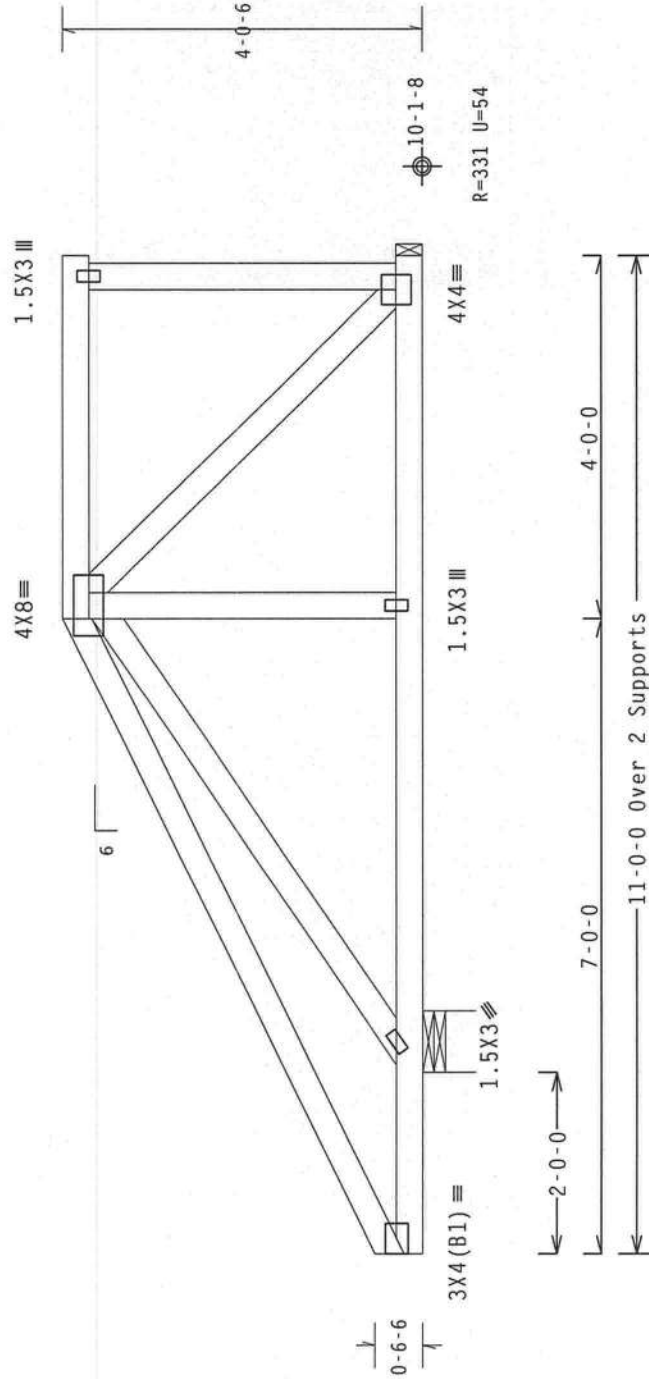
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



R=575 U=20 W=8"
RL=80

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .5"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for instructions relating to permitting conditions, bracing, and installation. Trusses and joists are designed to perform under specific loading conditions. Trusses and joists shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of the drawings, including, but not limited to, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. ITW Building Components Group Inc. 16131/177 Sec. 2, York, PA 15026. This job is not to be used for any other project. ITW-BCG: www.itwbcg.com; TPI: www.tpi.net.org; WTCA: www.wtcaindustry.com; these numbers apply. © 2004 ITW. 16131/177



TC LL	20.0 PSF	REF R487 -- 29369
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUR487 12170057
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 5697
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UMZ487_Z01

(12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - EJ9C 11' End Jack)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

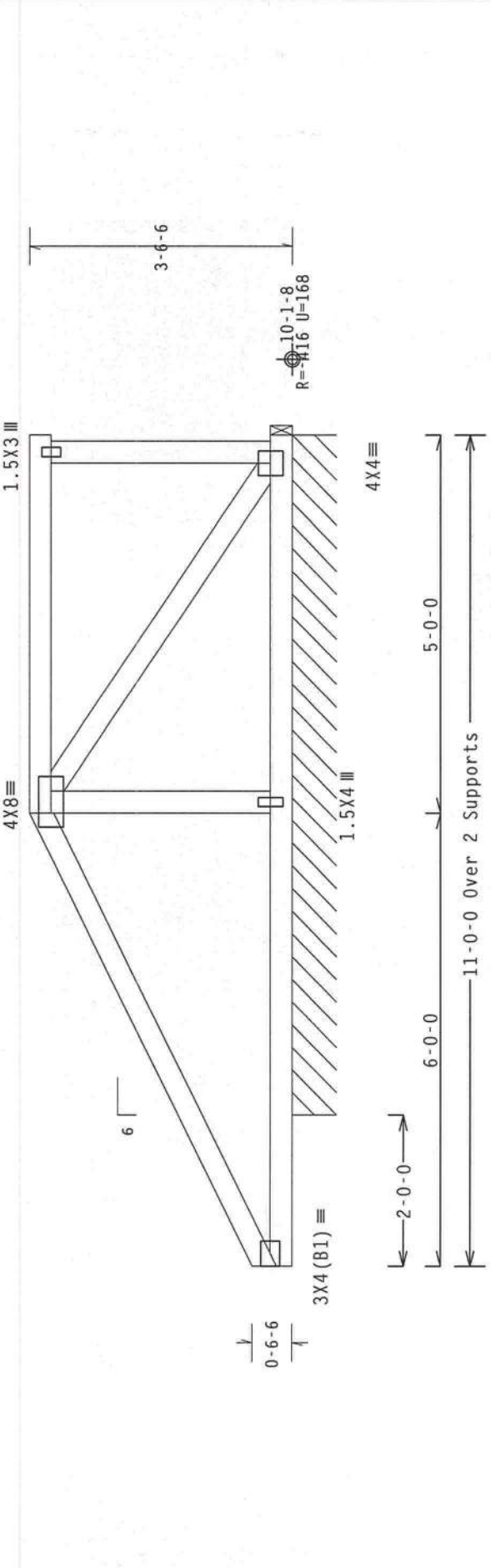
Negative reaction(s) of -416# MAX. (See below) from a non-wind load case requires uplift connection.

Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.



R-126 PLF U-3 PLF W-9-0-0
RL=8 PLF

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following notes and specifications for details. Trusses are to be installed in accordance with the following practices prior to performing these functions. Installers shall provide temporary bracing per the following notes and specifications. Trusses shall be installed in accordance with the following notes and specifications. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCS sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Trusses shall be installed in accordance with the following notes and specifications. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCS sections B3, B7 or B10, as applicable.

The responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.sbctindustry.com; ICCI: www.iccsafe.org



DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
10-03-11/0200-20

FL / - / 4 / - / - / R / -	Scale = .5" / Ft.
TC LL 20.0 PSF	REF R487 -- 29370
TC DL 10.0 PSF	DATE 06/18/12
BC DL 10.0 PSF	DRW HCUSR487 12170058
BC LL 0.0 PSF	HC-ENG DF/DF
TOT.LD. 40.0 PSF	SEQN - 5699
DUR.FAC. 1.25	
SPACING 24.0"	JREF - 1UMZ487_Z01

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MHFRS with additional C&C member design.

Right cantilever is exposed to wind

(H1) = (J) Hanger not calculated (1)2x6 SP #2 supporting member.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MHFRS loads based on trusses located at least 30.00 ft. from roof edge.

Deflection meets L/240 live and L/180 total load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

H = recommended connection based on manufacturer tested capacities and calculations.

Left end vertical not exposed to wind pressure.

110 mph wind, 15.00 ft mean ht., ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, Exp B, Wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 Gcpi(1/-)-0.18

Top chord 2x4 SP #1

Bot chord 2x4 SP #1

Web 2x4 SP #3

11-6-8

11-6-8

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11-6-8

R-913 U=90
RL=34/-101 H-Simpson LUS26
Supported Member Face: (3) 0.148"x3" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

R=1125 U=0 W=8"

Scale = .3125"/Ft.



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI Building Component Safety for details on TPI and MTCI for practices prior to performing these functions. Installers shall provide temporary bracing per unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Trusses shall be stored on a flat, level surface. Trusses shall be stored in a dry, well-ventilated area. Drawing or cover page listing the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MTCI: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

FL / - / 4 / - / - / R / -	Scale = .3125" / Ft.
TC LL 20.0 PSF	REF R487 -- 29371
TC DL 10.0 PSF	DATE 06/18/12
BC DL 10.0 PSF	DRW HCUSR487 12170059
BC LL 0.0 PSF	HC-ENG DF/DF
TOT.LD. 40.0 PSF	SEQN - 8700
DUR.FAC. 1.25	
SPACING 24.0"	JREF - 1UMZ487_Z01

(12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - EV2 24'-9" Common)
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpl(+/-)=0.18

Left end vertical not exposed to wind pressure.
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

Wind loads and reactions based on WMFRS with additional C&C member design.
Right cantilever is exposed to wind

(H1) = (J) Hanger not calculated (1)2x6 SP #2 supporting member.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WMFRS loads based on trusses located at least 30.00 ft. from roof edge.

Scale = .3125"/Ft.

PLT TYP. Wave

R=913 U=85
RL=39/-117 H=Simpson LUS26
Supported Member Face: (3) 0.148"x3" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #2

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / 0 (0)

Scale = .3125"/Ft.

PLT TYP. Wave

Scale = .3125"/Ft.

Scale = .3125"/Ft.

Scale = .3125"/Ft.

Scale = .3125"/Ft.

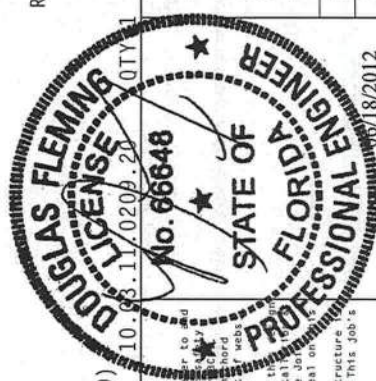
Scale = .3125"/Ft.

Scale = .3125"/Ft.

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Scale = .3125"/Ft.

Scale = .3125"/Ft.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

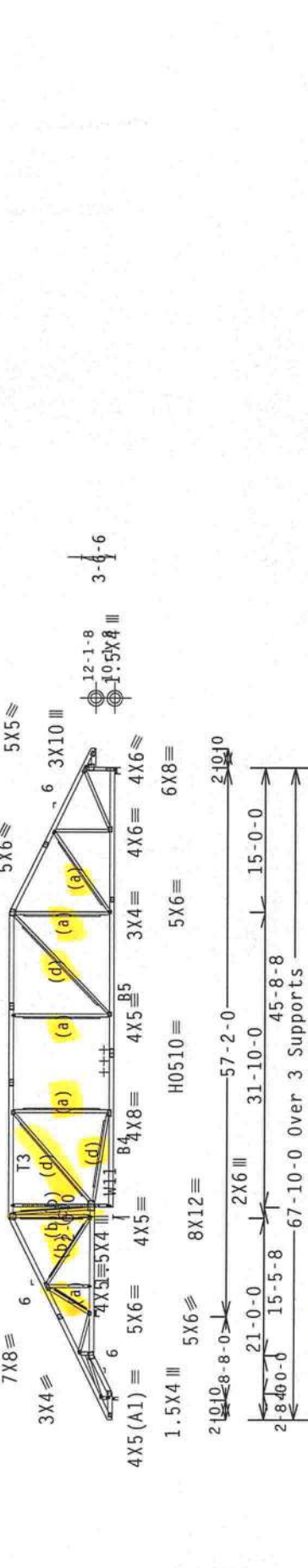
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for best practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing. Details, unless noted otherwise. Refer to drawings 180A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's General notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinat.org; WCA: www.wcaindustry.com; IBC: www.iccsafe.org

Top chord 2x6 SP #2 : T3 2x6 SP #1 Dense :
Bot chord 2x6 SP #2 : B4 2x6 SP SS :
Webs 2x4 SP #3 : W11 2x4 SP #1 :
: Rt Level Return 2x6 SP #2 : Rt Level Return 2x4 SP #3 :
Special loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 10.53
TC- From 62 plf at 10.53 to 62 plf at 21.00
TC- From 62 plf at 21.00 to 62 plf at 35.00
TC- From 62 plf at 35.00 to 62 plf at 43.00
TC- From 62 plf at 43.00 to 62 plf at 52.83
TC- From 62 plf at 52.83 to 62 plf at 59.99
TC- From 62 plf at 59.99 to 62 plf at 69.83
BC- From 20 plf at 0.00 to 20 plf at 2.67
BC- From 20 plf at 2.67 to 22 plf at 6.67
BC- From 22 plf at 6.67 to 20 plf at 22.13
BC- From 20 plf at 22.13 to 20 plf at 31.92
BC- From 60 plf at 31.92 to 60 plf at 38.13
BC- From 60 plf at 38.13 to 60 plf at 41.63
BC- From 20 plf at 41.63 to 20 plf at 54.13
BC- From 20 plf at 54.13 to 20 plf at 67.83
PLB- From 4 plf at 67.87 to 4 plf at 69.73

Wind loads and reactions based on MWFRS with additional C&C member design.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 15.91 ft. from roof edge.

WARNING: Furnish a copy of this DWG to the installation contractor.
Failure to follow provisions of BCSI in handling and installation of
trusses can result in serious injuries. Do not permit inexperienced and
uninstructed people to install trusses. See "WARNING" note below. BCSI
recommends retaining a registered professional engineer for the design of
temporary bracing.
+++ 20 PSF MAX LIVE LOAD IN OPEN PANEL+++
Trusses to be visibly stamped to indicate 20 psf MAX live load
to the top of the bottom chord.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor.
Failure to follow provisions of BCSI in handling and installation of
trusses can result in serious injuries. Do not permit inexperienced and
uninstructed people to install trusses. See "WARNING" note below. BCSI
recommends retaining a registered professional engineer for the design of
temporary bracing.
+++ 20 PSF MAX LIVE LOAD IN OPEN PANEL+++
Trusses to be visibly stamped to indicate 20 psf MAX live load
to the top of the bottom chord.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.



R=738 U=84 W=3.5"
RL=259/-2242704 U=307 W=8"

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / (0%)

PLT TYP. 20 Gauge HS.Wave

ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278		No. 66648 ★ ★ STATE OF FLORIDA PROFESSIONAL ENGINEER 06/18/2012 ***WARNING*** READ AND FOLLOW ALL NOTES ON THIS SHEET. FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Be sure to follow the latest editions of BCSI Building Components Group Inc. instructions for proper installation. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design for failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on the Jointing and Bracing details. The responsibility of the building designer for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MCA: www.sbcindustry.com; ICC: www.iccsafe.org		TC LL	20.0 PSF	REF	R487 --	29376
	TC DL	10.0 PSF	DATE	06/18/12				
	BC DL	10.0 PSF	DRW	HCU8R487	12170063			
	BC LL	0.0 PSF	HC-ENG	DF/DF				
	TOT.LD.	40.0 PSF	SEQN-	8421				
	DUR.FAC.	1.25						
	SPACING	24.0"	JREF-	1UMZ487_Z01				

(12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.
(updown Hip)

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.91 ft mean hgt., ASCE 7-05, CLOSED bldg. not located within 13.00 ft from roof edge. CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Left cantilever is exposed to wind

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Wind loads and reactions based on MWFRS with additional C&C member design.

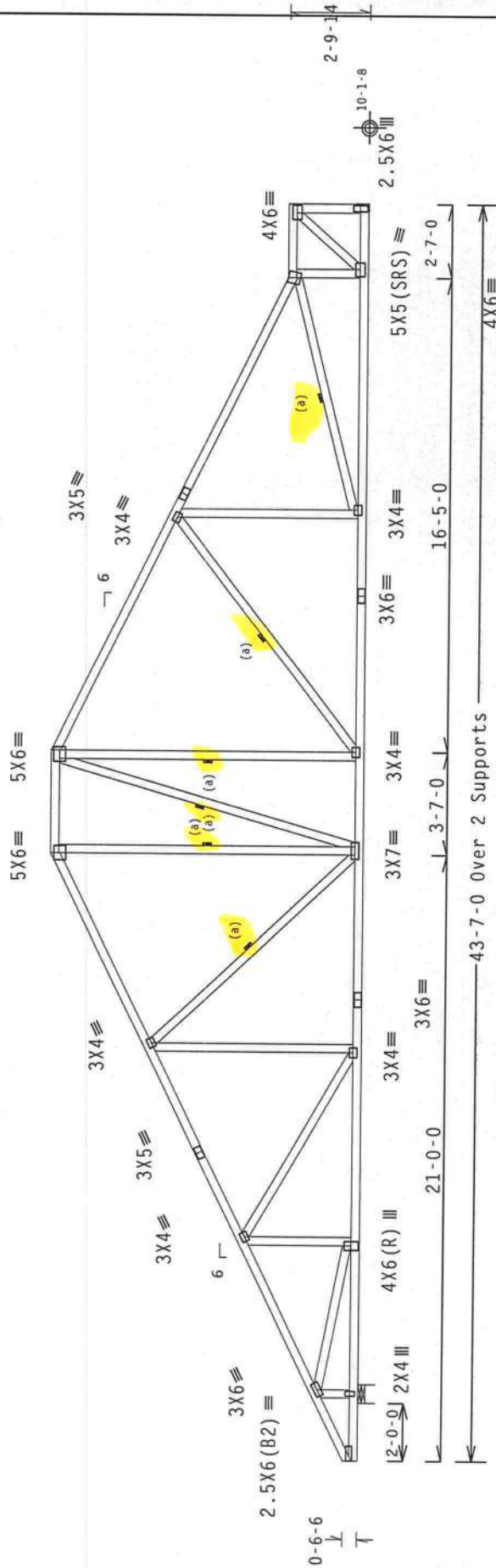
Right end vertical not exposed to wind pressure.

(H1) = (J) Hanger not calculated (3)2x6 SP M-26 supporting member.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.91 ft. from roof edge.



R=1693 U=0 H=Simpson HUS26
Supported Member Face: (4) 0.148"x3" nails
Supporting Member Face: (14) 0.148"x3" nails
Supporting Member : (3) 2x6 SP #2

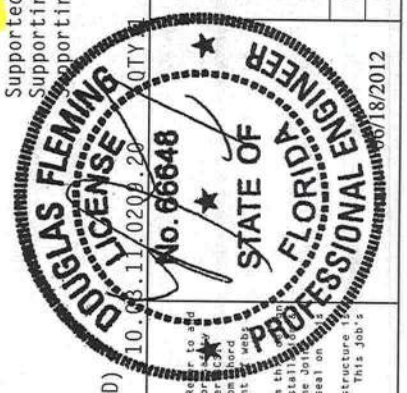
R=1896 U=0 W=8"
RL=219/-198

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

TC	LL	20.0	PSF	REF	R487--	29377
TC	DL	10.0	PSF	DATE	06/18/12	
BC	DL	10.0	PSF	DRW	HCUSR487	12170064
BC	LL	0.0	PSF	HC-ENG	DF/DF	
TOT.	I.D.	40.0	PSF	SEQN-	6502	
DUR.	FAC.	1.25				
SPACING	24.0"			JREF-	1UMZ487_Z01	



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278


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weds 2x4 Sp #3. W12 2x4 Sp #1.
:Rt Level Return 2x6 Sp #2::Rt Level Return 2x4 Sp #3:

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U=296 W=8"

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

10.6.11.0209.20 QTY 1

Refer to end
of project
for details
and third
party
instruct
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used
on
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job
and
to
be
used
on
the
structure.
This job

06/18/2012

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads

-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 62 plf at 0.00 to 62 plf at 5.07
TC- From 62 plf at 5.07 to 62 plf at 16.23
TC- From 62 plf at 16.23 to 62 plf at 21.30
BC- From 4 plf at 0.00 to 4 plf at 21.30

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load.

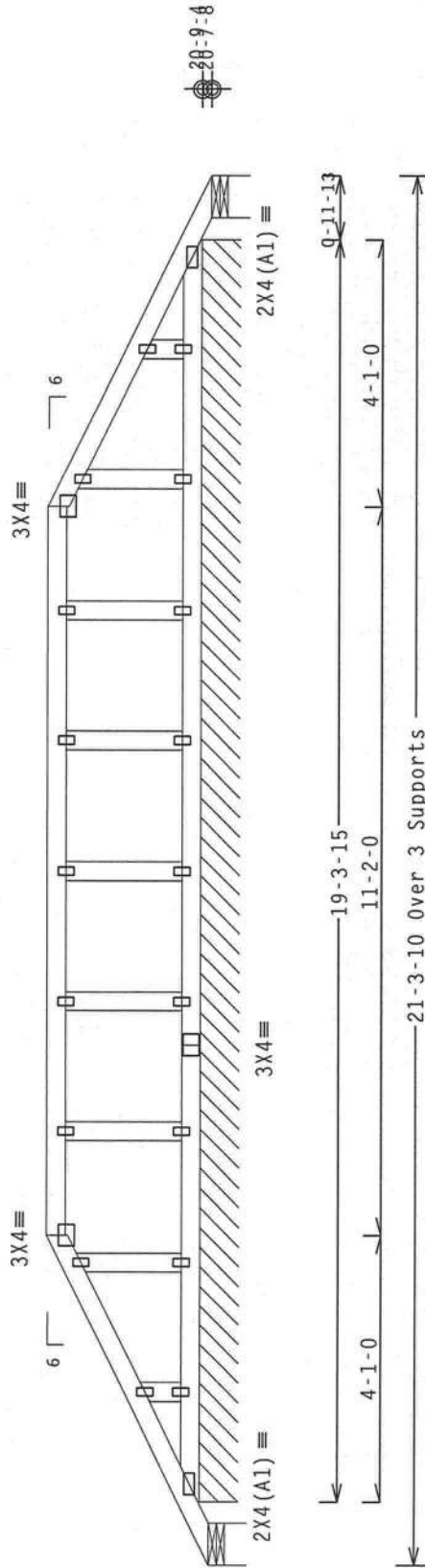
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 21.89 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. 1W=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

MWFRS loads based on trusses located at least 21.89 ft. from roof edge.



R=21 RW=22 U=20 W=7.826"
RL=50/R=509 PLF U=16 PLF W=19-3-15

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)

PLT TYP. Wave

FT/RT=10%(0%/0/0)

TY: 10.0.11/0208.28

Scale = .375"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Be sure to follow the latest edition of ACI (Building Component Safety Information, by TPI and MCA) for practices prior to performing these functions. Installers shall provide temporary bracing per details noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per ACI sections 83, 87 or 810, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing. ITWBCG shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing. ITWBCG shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing. ITWBCG shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

FL / - / 4 / - / - / R / -	TC LL	20.0 PSF	REF	R487 - -	29382
	TC DL	10.0 PSF	DATE	06/18/12	
	BC DL	10.0 PSF	DRW	HCUSR487	12170069
	BC LL	0.0 PSF	HC-ENG	DF/DF	
	TOT.LD.	40.0 PSF	SEQN-	7732	
	DUR.FAC.	1.25			
	SPACING	24.0"	JREF-	1UMZ487_Z01	

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 16.41 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpl (+/-)=0.18

Left and right cantilevers are exposed to wind

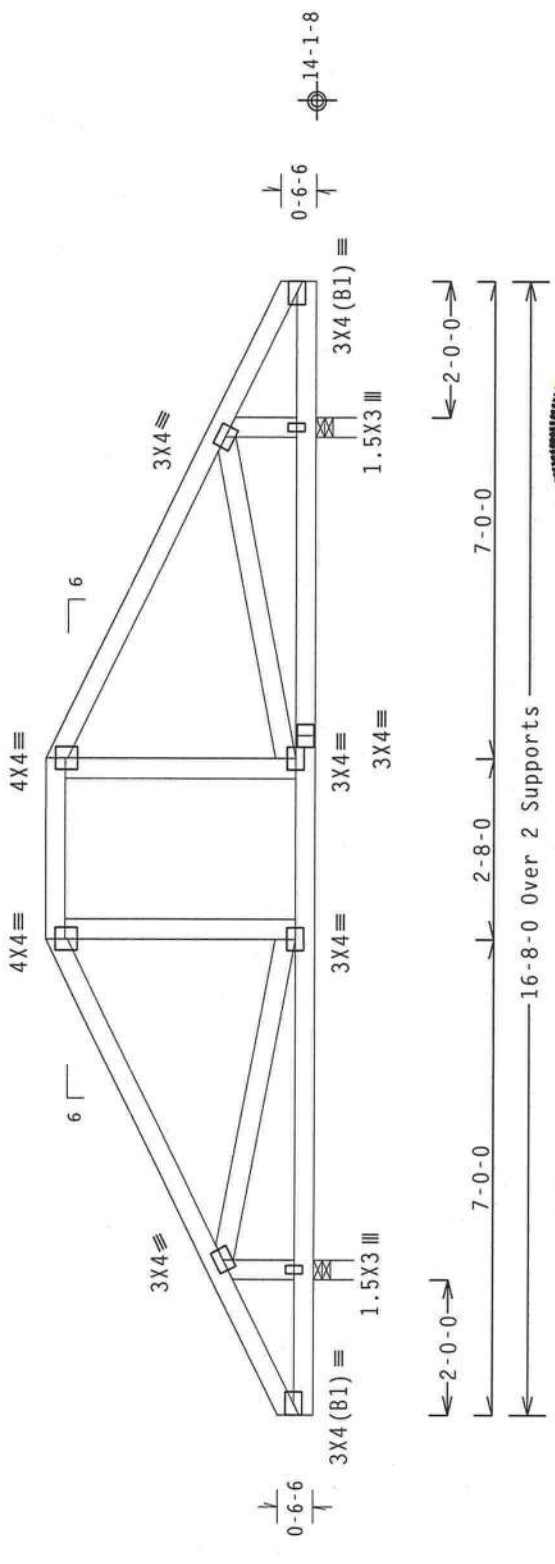
Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side Jacks have 7-0-0 setback with 2-0-0 cant and 0-0-0 overhang. End Jacks have 7-0-0 setback with 2-0-0 cant and 0-0-0 overhang. Right side Jacks have 7-0-0 setback with 2-0-0 cant and 0-0-0 overhang.

#1 hip supports 7-0-0 Jacks W/2 panel TC and no end vert.

Deflection meets L/240 live and L/180 total load.



R=919 U=102 W=3.5"

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%)/0 (0)

FL / - / 4 / - / - / R / -	Scale = .375" / Ft.
TC LL 20.0 PSF	REF R487 -- 29385
TC DL 10.0 PSF	DATE 06/15/12
BC DL 10.0 PSF	DRW HCUSR487 12167032
BC LL 0.0 PSF	HC-ENG DF/DF
TOT.LD. 40.0 PSF	SEQN- 263037
DUR.FAC. 1.25	
SPACING 24.0"	JREF- 1UMZ487_Z01



Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Institute) Manual for practices prior to performing these functions. Installers shall provide temporary bracing per BCSI Manual. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

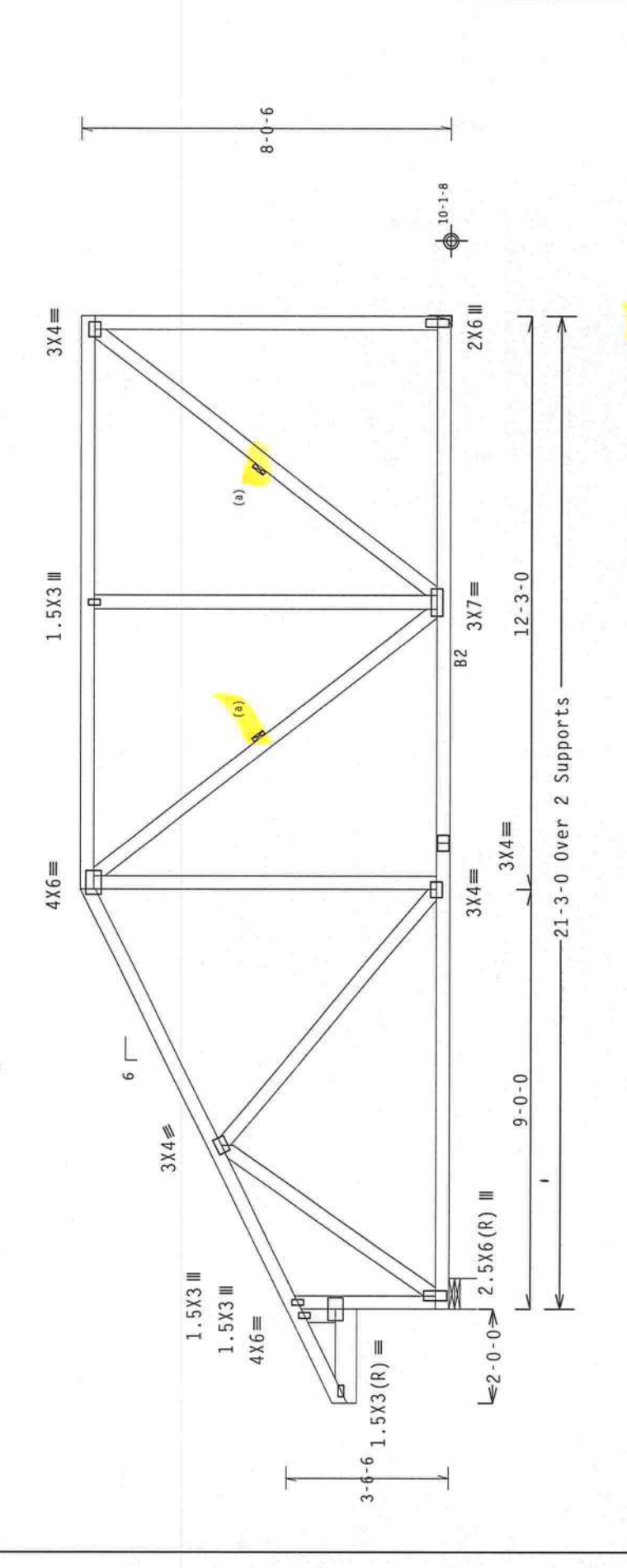
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing of trusses, apply plates to each face of truss and position as shown above and on the joint. ITWBCG shall not be responsible for any deviation from this design. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(12-010)rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - HSI3A1 23'3" Common)
Top chord 2x4 SP #1 :B2 2x4 SP M-30:
Bot chord 2x4 SP #3 :B2 2x4 SP M-30:
Webbs 2x4 SP #3 :B2 2x4 SP M-30:
:Lt Level Return 2x6 SP #2::Lt Level Return 2x4 SP #3:
110 mph wind, 15.41 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind IC DL=5.0 psf, wind BC DL=5.0 psf, 1W=1.00 GCPI(+/-)=0.18
H = recommended connection based on manufacturer tested capacities and calculations.
Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.
(a) Continuous lateral bracing equally spaced on member.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 7.70 ft. from roof edge.

Wind loads and reactions based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
(H1) = (J) Hanger not calculated (1)2x6 SP SS supporting member.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.



R=870 U=119 H=Simpson LUS24
Supported Member Face: (2)
Supporting Member Face: (4)
Supporting Member: (1) 2x6 SP SS

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%)/0 (0)

R=1000 U=77 W=8"
RL=125/-47

Scale = .3125"/Ft.

FL / - / 4 / - / - / R / -	REF	Scale
TC LL	20.0 PSF	R487 -- 29388
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUSR487 12170073
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN - 8688
DUR.FAC.	1.25	JREF - 1UMZ487_Z01
SPACING	24.0"	

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
06/18/2012

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following notes for details. These functions shall be performed by experienced truss installers. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCS sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, bracing or trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise, shall refer to drawings 100A-2 for standard plate positions. A seal on the bottom chord shall be used to indicate the location of the seal. The seal shall be used to indicate the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; NCA: www.sbcindustry.com; ICC: www.iccsafe.org

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge. CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Left cantilever is exposed to wind

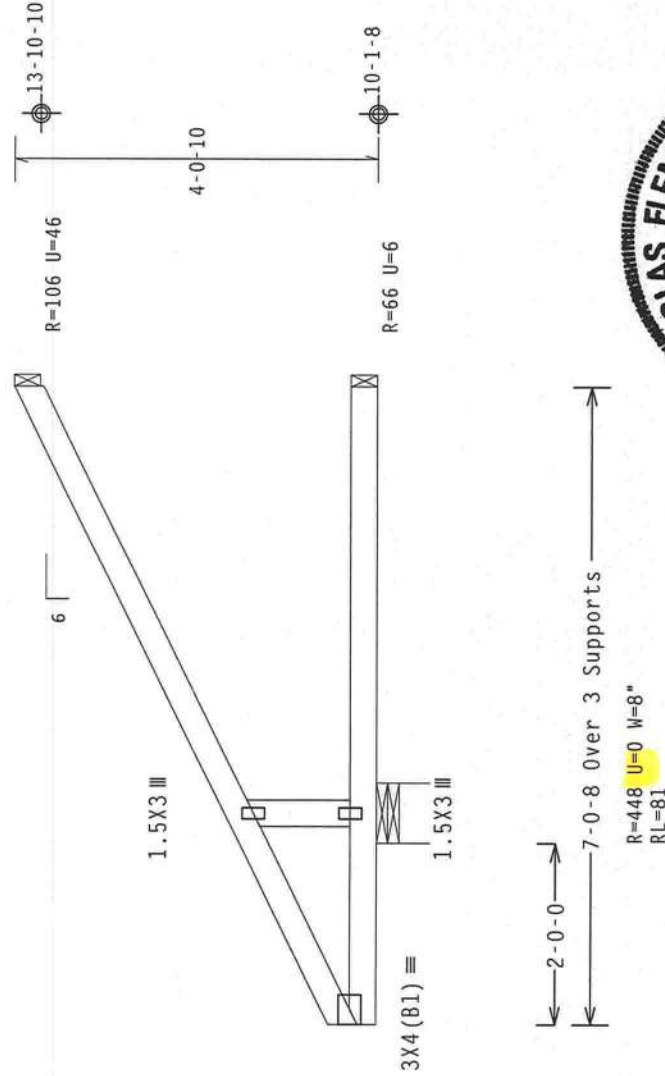
Deflection meets L/240 live and L/180 total load.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

FL/-/4/-/-/R/-
Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487-- 299
TC DL	10.0 PSF	DATE 06/18/17
BC DL	10.0 PSF	DRW HCUR487 12170
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 6024
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UNW487 Z

The seal is circular with a dashed outer border. Inside the border, the text "STATE OF FLORIDA" is written in a semi-circle at the top, and "PROFESSIONAL ENGINEER" is written in a semi-circle at the bottom. In the center, the number "No. 66648" is printed vertically on the left, and the date "06/18/2012" is printed vertically on the right. Two stars are positioned on the left and right sides of the center. A signature is written across the center of the seal.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
****IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of ANSI (Building Component Safety Information, by TPI and MTCA) for details. Trusses shall be braced in accordance with the design drawings. Trusses shall be braced prior to performing these functions. Installers shall provide temporary bracing per the design drawings. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint (unless noted otherwise) shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the manufacturer's failure to build the truss in conformance with ANSI/TPI 1, 1- or For handling, shipping, installing, bracing of trusses. Apply plates to each face of truss and position as shown above and on the Job Order. Details, unless noted otherwise, refer to drawings 160A-2 For standard plate positions. A seal on the end of the truss indicates acceptance of professional engineering design drawings. The responsibility of the design engineer shall be to provide the design drawings. The responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job Order's general notes page: TPI-8CG; www.itwbcg.com; TPI: www.tpinet.org; MTCA: www.sbcindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.78 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, W=1.00 Gcps (1/2)=0.18

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

MFERS loads based on trusses located at least 15.78 ft. from roof edge.

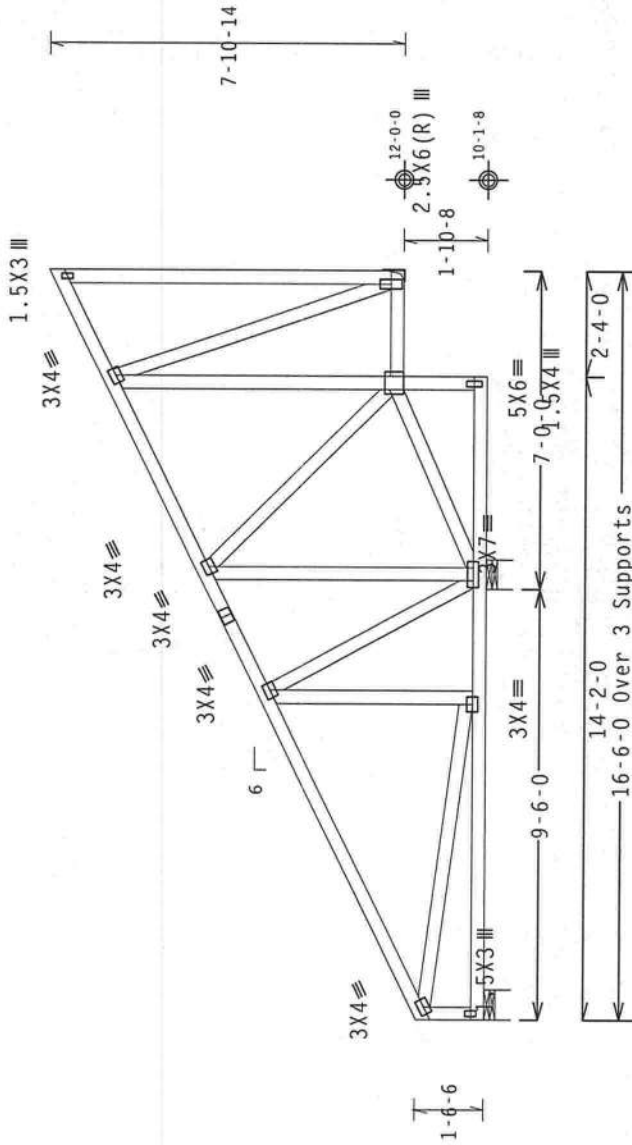
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MFERS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(H1) = (J) Hanger not calculated (1)2x6 SP #1 Dense supporting member.

Deflection meets L/240 live and L/180 total load.



R=210 U=45 H=Simpson LUS24
Supports: (2) 0.148"x3" nails
Supp: (4) 0.148"x3" nails
S: (2) 2x4 SP #1

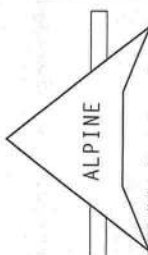
R=361 U=0 W=8"
RL=129/-41

R=788 U=0 W=8"

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .25"/Ft.

 ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET! **IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety) Information, by TPI and MTC, for proper practices prior to performing these functions. Installers shall provide temporary bracing per BCSI and MTC. The bottom chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections 83, 87 or 810, as applicable. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: general notes page: ITW-860; www.itwbcg.com; TPI: www.tpinet.org; MTC: www.sbcindustry.com; ICC: www.iccsafe.org	
	No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER 06/18/2012	
	TC LL	20.0 PSF
	TC DL	10.0 PSF
	BC DL	10.0 PSF
BC LL	0.0 PSF	
TOT.LD.	40.0 PSF	
DUR.FAC.	1.25	
SPACING	24.0"	
JREF- 1UMZ487_Z01		

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #1: T1, T2 2x6 SP #2:
2x4 SP #3: W1, W2 2x4 SP #4: W3, W4 2x4 SP #5: W5, W6 2x4 SP #6: W7, W8 2x4 SP #7: W9, W10 2x4 SP #8: W11, W12 2x4 SP #9: W13, W14 2x4 SP #10: W15, W16 2x4 SP #11: W17, W18 2x4 SP #12: W19, W20 2x4 SP #13: W21, W22 2x4 SP #14: W23, W24 2x4 SP #15: W25, W26 2x4 SP #16: W27, W28 2x4 SP #17: W29, W30 2x4 SP #18: W31, W32 2x4 SP #19: W33, W34 2x4 SP #20: W35, W36 2x4 SP #21: W37, W38 2x4 SP #22: W39, W40 2x4 SP #23: W41, W42 2x4 SP #24: W43, W44 2x4 SP #25: W45, W46 2x4 SP #26: W47, W48 2x4 SP #27: W49, W50 2x4 SP #28: W51, W52 2x4 SP #29: W53, W54 2x4 SP #30: W55, W56 2x4 SP #31: W57, W58 2x4 SP #32: W59, W60 2x4 SP #33: W61, W62 2x4 SP #34: W63, W64 2x4 SP #35: W65, W66 2x4 SP #36: W67, W68 2x4 SP #37: W69, W70 2x4 SP #38: W71, W72 2x4 SP #39: W73, W74 2x4 SP #40: W75, W76 2x4 SP #41: W77, W78 2x4 SP #42: W79, W80 2x4 SP #43: W81, W82 2x4 SP #44: W83, W84 2x4 SP #45: W85, W86 2x4 SP #46: W87, W88 2x4 SP #47: W89, W90 2x4 SP #48: W91, W92 2x4 SP #49: W93, W94 2x4 SP #50: W95, W96 2x4 SP #51: W97, W98 2x4 SP #52: W99, W100 2x4 SP #53: W101, W102 2x4 SP #54: W103, W104 2x4 SP #55: W105, W106 2x4 SP #56: W107, W108 2x4 SP #57: W109, W110 2x4 SP #58: W111, W112 2x4 SP #59: W113, W114 2x4 SP #60: W115, W116 2x4 SP #61: W117, W118 2x4 SP #62: W119, W120 2x4 SP #63: W121, W122 2x4 SP #64: W123, W124 2x4 SP #65: W125, W126 2x4 SP #66: W127, W128 2x4 SP #67: W129, W130 2x4 SP #68: W131, W132 2x4 SP #69: W133, W134 2x4 SP #70: W135, W136 2x4 SP #71: W137, W138 2x4 SP #72: W139, W140 2x4 SP #73: W141, W142 2x4 SP #74: W143, W144 2x4 SP #75: W145, W146 2x4 SP #76: W147, W148 2x4 SP #77: W149, W150 2x4 SP #78: W151, W152 2x4 SP #79: W153, W154 2x4 SP #80: W155, W156 2x4 SP #81: W157, W158 2x4 SP #82: W159, W160 2x4 SP #83: W161, W162 2x4 SP #84: W163, W164 2x4 SP #85: W165, W166 2x4 SP #86: W167, W168 2x4 SP #87: W169, W170 2x4 SP #88: W171, W172 2x4 SP #89: W173, W174 2x4 SP #90: W175, W176 2x4 SP #91: W177, W178 2x4 SP #92: W179, W180 2x4 SP #93: W181, W182 2x4 SP #94: W183, 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W434 2x4 SP #220: W435, W436 2x4 SP #221: W437, W438 2x4 SP #222: W439, W440 2x4 SP #223: W441, W442 2x4 SP #224: W443, W444 2x4 SP #225: W445, W446 2x4 SP #226: W447, W448 2x4 SP #227: W449, W450 2x4 SP #228: W451, W452 2x4 SP #229: W453, W454 2x4 SP #230: W455, W456 2x4 SP #231: W457, W458 2x4 SP #232: W459, W460 2x4 SP #233: W461, W462 2x4 SP #234: W463, W464 2x4 SP #235: W465, W466 2x4 SP #236: W467, W468 2x4 SP #237: W469, W470 2x4 SP #238: W471, W472 2x4 SP #239: W473, W474 2x4 SP #240: W475, W476 2x4 SP #241: W477, W478 2x4 SP #242: W479, W480 2x4 SP #243: W481, W482 2x4 SP #244: W483, W484 2x4 SP #245: W485, W486 2x4 SP #246: W487, W488 2x4 SP #247: W489, W490 2x4 SP #248: W491, W492 2x4 SP #249: W493, W494 2x4 SP #250: W495, W496 2x4 SP #251: W497, W498 2x4 SP #252: W499, W500 2x4 SP #253: W501, W502 2x4 SP #254: W503, W504 2x4 SP #255: W505, W506 2x4 SP #256: W507, W508 2x4 SP #257: W509, W510 2x4 SP #258: W511, W512 2x4 SP #259: W513, W514 2x4 SP #260: W515, W516 2x4 SP #261: W517, W518 2x4 SP #262: W519, W520 2x4 SP #263: W521, W522 2x4 SP #264: W523, W524 2x4 SP #265: W525, W526 2x4 SP #266: W527, W528 2x4 SP #267: W529, W530 2x4 SP #268: W531, W532 2x4 SP #269: W533, W534 2x4 SP #270: W535, W536 2x4 SP #271: W537, W538 2x4 SP #272: W539, W540 2x4 SP #273: W541, W542 2x4 SP #274: W543, W544 2x4 SP #275: W545, W546 2x4 SP #276: W547, W548 2x4 SP #277: W549, W550 2x4 SP #278: W551, W552 2x4 SP #279: W553, W554 2x4 SP #280: W555, W556 2x4 SP #281: W557, W558 2x4 SP #282: W559, W560 2x4 SP #283: W561, W562 2x4 SP #284: W563, W564 2x4 SP #285: W565, W566 2x4 SP #286: W567, W568 2x4 SP #287: W569, W570 2x4 SP #288: W571, W572 2x4 SP #289: W573, W574 2x4 SP #290: W575, W576 2x4 SP #291: W577, W578 2x4 SP #292: W579, W580 2x4 SP #293: W581, W582 2x4 SP #294: W583, W584 2x4 SP #295: W585, W586 2x4 SP #296: W587, W588 2x4 SP #297: W589, W590 2x4 SP #298: W591, W592 2x4 SP #299: W593, W594 2x4 SP #300: W595, W596 2x4 SP #301: W597, W598 2x4 SP #302: W599, W600 2x4 SP #303: W601, W602 2x4 SP #304: W603, W604 2x4 SP #305: W605, W606 2x4 SP #306: W607, W608 2x4 SP #307: W609, W610 2x4 SP #308: W611, W612 2x4 SP #309: W613, W614 2x4 SP #310: W615, W616 2x4 SP #311: W617, W618 2x4 SP #312: W619, W620 2x4 SP #313: W621, W622 2x4 SP #314: W623, W624 2x4 SP #315: W625, W626 2x4 SP #316: W627, W628 2x4 SP #317: W629, W630 2x4 SP #318: W631, W632 2x4 SP #319: W633, W634 2x4 SP #320: W635, W636 2x4 SP #321: W637, W638 2x4 SP #322: W639, W640 2x4 SP #323: W641, W642 2x4 SP #324: W643, W644 2x4 SP #325: W645, W646 2x4 SP #326: W647, W648 2x4 SP #327: W649, W650 2x4 SP #328: W651, W652 2x4 SP #329: W653, W654 2x4 SP #330: W655, W656 2x4 SP #331: W657, W658 2x4 SP #332: W659, W660 2x4 SP #333: W661, W662 2x4 SP #334: W663, W664 2x4 SP #335: W665, W666 2x4 SP #336: W667, W668 2x4 SP #337: W669, W670 2x4 SP #338: W671, W672 2x4 SP #339: W673, W674 2x4 SP #340: W675, W676 2x4 SP #341: W677, W678 2x4 SP #342: W679, W680 2x4 SP #343: W681, W682 2x4 SP #344: 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SP #386: W767, W768 2x4 SP #387: W769, W770 2x4 SP #388: W771, W772 2x4 SP #389: W773, W774 2x4 SP #390: W775, W776 2x4 SP #391: W777, W778 2x4 SP #392: W779, W780 2x4 SP #393: W781, W782 2x4 SP #394: W783, W784 2x4 SP #395: W785, W786 2x4 SP #396: W787, W788 2x4 SP #397: W789, W790 2x4 SP #398: W791, W792 2x4 SP #399: W793, W794 2x4 SP #400: W795, W796 2x4 SP #401: W797, W798 2x4 SP #402: W799, W800 2x4 SP #403: W801, W802 2x4 SP #404: W803, W804 2x4 SP #405: W805, W806 2x4 SP #406: W807, W808 2x4 SP #407: W809, W810 2x4 SP #408: W811, W812 2x4 SP #409: W813, W814 2x4 SP #410: W815, W816 2x4 SP #411: W817, W818 2x4 SP #412: W819, W820 2x4 SP #413: W821, W822 2x4 SP #414: W823, W824 2x4 SP #415: W825, W826 2x4 SP #416: W827, W828 2x4 SP #417: W829, W830 2x4 SP #418: W831, W832 2x4 SP #419: W833, W834 2x4 SP #420: W835, W836 2x4 SP #421: W837, W838 2x4 SP #422: W839, W840 2x4 SP #423: W841, W842 2x4 SP #424: W843, W844 2x4 SP #425: W845, W846 2x4 SP #426: W847, W848 2x4 SP #427: W849, W850 2x4 SP #428: W851, W852 2x4 SP #429: W853, W854 2x4 SP #430: W855, W856 2x4 SP #431: W857, W858 2x4 SP #432: W859, W860 2x4 SP #433: W861, W862 2x4 SP #434: W863, W864 2x4 SP #435: W865, W866 2x4 SP #436: W867, W868 2x4 SP #437: W869, W870 2x4 SP #438: W871, W872 2x4 SP #439: W873, W874 2x4 SP #440: W875, W876 2x4 SP #441: W877, W878 2x4 SP #442: W879, W880 2x4 SP #443: W881, W882 2x4 SP #444: W883, W884 2x4 SP #445: W885, W886 2x4 SP #446: W887, W888 2x4 SP #447: W889, W890 2x4 SP #448: W891, W892 2x4 SP #449: W893, W894 2x4 SP #450: W895, W896 2x4 SP #451: W897, W898 2x4 SP #452: W899, W900 2x4 SP #453: W901, W902 2x4 SP #454: W903, W904 2x4 SP #455: W905, W906 2x4 SP #456: W907, W908 2x4 SP #457: W909, W910 2x4 SP #458: W911, W912 2x4 SP #459: W913, W914 2x4 SP #460: W915, W916 2x4 SP #461: W917, W918 2x4 SP #462: W919, W920 2x4 SP #463: W921, W922 2x4 SP #464: W923, W924 2x4 SP #465: W925, W926 2x4 SP #466: W927, W928 2x4 SP #467: W929, W930 2x4 SP #468: W931, W932 2x4 SP #469: W933, W934 2x4 SP #470: W935, W936 2x4 SP #471: W937, W938 2x4 SP #472: W939, W940 2x4 SP #473: W941, W942 2x4 SP #474: W943, W944 2x4 SP #475: W945, W946 2x4 SP #476: W947, W948 2x4 SP #477: W949, W950 2x4 SP #478: W951, W952 2x4 SP #479: W953, W954 2x4 SP #480: W955, W956 2x4 SP #481: W957, W958 2x4 SP #482: W959, W960 2x4 SP #483: W961, W962 2x4 SP #484: W963, W964 2x4 SP #485: W965, W966 2x4 SP #486: W967, W968 2x4 SP #487: W969, W970 2x4 SP #488: W971, W972 2x4 SP #489: W973, W974 2x4 SP #490: W975, W976 2x4 SP #491: W977, W978 2x4 SP #492: W979, W980 2x4 SP #493: W981, W982 2x4 SP #494: W983, W984 2x4 SP #495: W985, W986 2x4 SP #496: W987, W988 2x4 SP #497: W989, W990 2x4 SP #498: W991, W992 2x4 SP #499: W993, W994 2x4 SP #500: W995, W996 2x4 SP #501: W997, W998 2x4 SP #502: W999, W1000 2x4 SP #503: W1001, W1002 2x4 SP #504: W1003, W1004 2x4 SP #505: W1005, W1006 2x4 SP #506: W1007, W1008 2x4 SP #507: W1009, W1010 2x4 SP #508: W1011, W1012 2x4 SP #509: W1013, W1014 2x4 SP #510: W1015, W1016 2x4 SP #511: W1017, W1018 2x4 SP #512: W1019, W1020 2x4 SP #513: W1021, W1022 2x4 SP #514: W1023, W1024 2x4 SP #515: W1025, W1026 2x4 SP #516: W1027, W1028 2x4 SP #517: W1029, W1030 2x4 SP #518: W1031, W1032 2x4 SP #519: W1033, W1034 2x4 SP #520: W1035, W1036 2x4 SP #521: W1037, W1038 2x4 SP #522: W1039, W1040 2x4 SP #523: W1041, W1042 2x4 SP #524: W1043, W1044 2x4 SP #525: W1045, W1046 2x4 SP #526: W1047, W1048 2x4 SP #527: W1049, W1050 2x4 SP #528: W1051, W1052 2x4 SP #529: W1053, W1054 2x4 SP #530: W1055, W1056 2x4 SP #531: W1057, W1058 2x4 SP #532: W1059, W1060 2x4 SP #533: W1061, W1062 2x4 SP #534: W1063, W1064 2x4 SP #535: W1065, W1066 2x4 SP #536: W1067, W1068 2x4 SP #537: W1069, W1070 2x4 SP #538: W1071, W1072 2x4 SP #539: W1073, W1074 2x4 SP #540: W1075, W1076 2x4 SP #541: W1077, W1078 2x4 SP #542: W1079, W1080 2x4 SP #543: W1081, W1082 2x4 SP #544: W1083, W1084 2x4 SP #545: W1085, W1086 2x4 SP #546: W1087, W1088 2x4 SP #547: W1089, W1090 2x4 SP #548: W1091, W1092 2x4 SP #549: W1093, W1094 2x4 SP #550: W1095, W1096 2x4 SP #551: W1097, W1098 2x4 SP #552: W1099, W1100 2x4 SP #553: W1101, W1102 2x4 SP #554: W1103, W1104 2x4 SP #555: W1105, W1106 2x4 SP #556: W1107, W1108 2x4 SP #557: W1109, W1110 2x4 SP #558: W1111, W1112 2x4 SP #559: W1113, W1114 2x4 SP #560: W1115, W1116 2x4 SP #561: W1117, W1118 2x4 SP #562: W1119, W1120 2x4 SP #563: W1121, W1122 2x4 SP #564: W1123, W1124 2x4 SP #565: W1125, W1126 2x4 SP #566: W1127, W1128 2x4 SP #567: W1129, W1130 2x4 SP #568: W1131, W1132 2x4 SP #569: W1133, W1134 2x4 SP #570: W1135, W1136 2x4 SP #571: W1137, W1138 2x4 SP #572: W1139, W1140 2x4 SP #573: W1141, W1142 2x4 SP #574: W1143, W1144 2x4 SP #575: W1145, W1146 2x4 SP #576: W1147, W1148 2x4 SP #577: W1149, W1150 2x4 SP #578: W1151, W1152 2x4 SP #579: W1153, W1154 2x4 SP #580: W1155, W1156 2x4 SP #581: W1157, W1158 2x4 SP #582: W1159, W1160 2x4 SP #583: W1161, W1162 2x4 SP #584: W1163, W1164 2x4 SP #585: W1165, W1166 2x4 SP #586: W1167, W1168 2x4 SP #587: W1169, W1170 2x4 SP #588: W1171, W1172 2x4 SP #589: W1173, W1174 2x4 SP #590: W1175, W1176 2x4 SP #591: W1177, W1178 2x4 SP #592: W1179, W1180 2x4 SP #593: W1181, W1182 2x4 SP #594: W1183, W1184 2x4 SP #595: W1185, W1186 2x4 SP #596: W1187, W1188 2x4 SP #597: W1189, W1190 2x4 SP #598: W1191, W1192 2x4 SP #599: W1193, W1194 2x4 SP #600: W1195, W1196 2x4 SP #601: W1197, W1198 2x4 SP #602: W1199, W1200 2x4 SP #603: W1201, W1202 2x4 SP #604: W1203, W1204 2x4 SP #605: W1205, W1206 2x4 SP #606: W1207, W1208 2x4 SP #607: W1209, W1210 2x4 SP #608: W1211, W1212 2x4 SP #609: W1213, W1214 2x4 SP #610: W1215, W1216 2x4 SP #611: W1217, W1218 2x4 SP #612: W1219, W1220 2x4 SP #613: W1221, W1222 2x4 SP #614: W1223, W1224 2x4 SP #615: W1225, W1226 2x4 SP #616: W1227, W1228 2x4 SP #617: W1229, W1230 2x4 SP #618: W1231, W1232 2x4 SP #619: W1233, W1234 2x4 SP #620: W1235, W1236 2x4 SP #621: W1237, W1238 2x4 SP #622: W1239, W1240 2x4 SP #623: W1241, W1242 2x4 SP #624: W1243, W1244 2x4 SP #625: W1245, W1246 2x4 SP #626: W1247, W1248 2x4 SP #627: W1249, W1250 2x4 SP #628: W1251, W1252 2x4 SP #629: W1253, W1254 2x4 SP #630: W1255, W1256 2x4 SP #631: W1257, W1258 2x4 SP #632: W1259, W1260 2x4 SP #633: W1261, W1262 2x4 SP #634: W1263, W1264 2x4 SP #635: W1265, W1266 2x4 SP #636: W1267, W1268 2x4 SP #637: W1269, W1270 2x4 SP #638: W1271, W1272 2x4 SP #639: W1273, W1274 2x4 SP #640: W1275, W1276 2x4 SP #641: W127

Top	chord	2x6	SP #2
Bot	chord	2x6	SP #2
	Webs	2x4	SP #3

110 mph wind. 15.64 ft mean hgt. ASCE 7-05, CLOSED bldg. not located within 17.00 ft from roof edge. CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 SCPI(+/-)-0.18

Left cantilever is exposed to wind
(H1) = (J) Hanger not calculated (2 supporting member).

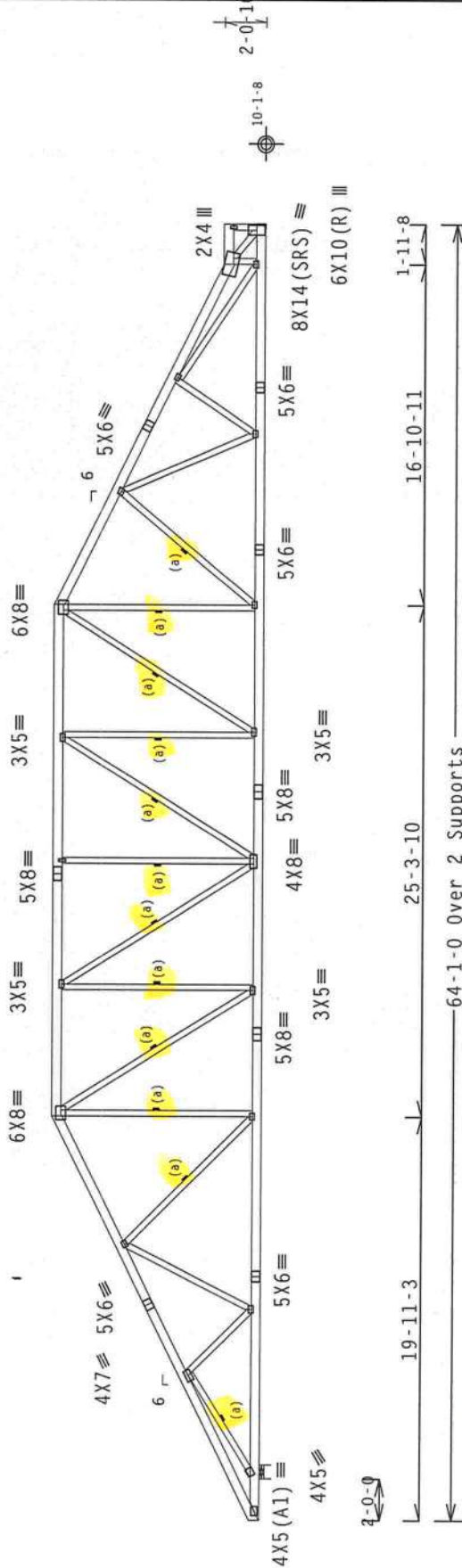
Deflection meets L/240 live and L/180 total load.

MFIRS loads based on trusses located at least 31.28 ft. from roof edge.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.



R=2739 U=0 W=8"
RL=221 / -209

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

PLT TYP. Wave

FL/-/4/-/-/R/-/-
Scale = .125"/Ft.

TC LL	20.0 PSF	REF R487 -- 29396
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUR487 12170081
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 8085
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UMZ487 Z01



****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

[illegible][illegible]

2 COMPLETE TRUSSES REQUIRED
Nail Schedule: 0.131"x3", min. nails

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special loads
----- (Lumber Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)

IC-From	62 p/f at	19.93 to	62 p/f at	31.93 to	62 p/f at	31.93 to
TC-From	62 p/f at	31.93 to	62 p/f at	45.23 to	62 p/f at	45.23 to
TC-From	62 p/f at	45.23 to	62 p/f at	58.13 to	62 p/f at	58.13 to

IC- From	62 p/f at 45.23	to	62 p/f at 58.13
TC- From	62 p/f at 58.13	to	62 p/f at 64.27
TC- From	31 p/f at 64.27	to	31 p/f at 67.67

IC- From	31 p1f at	64.27 to	31 p1f at	67.67 to
BC- From	20 p1f at	0.00 to	20 p1f at	16.00 to
BC- From	20 p1f at	16.00 to	20 p1f at	30.00 to

BC-From	20 p1f at	to	30.00
BC-From	20 p1f at	to	38.00
BC-From	20 p1f at	to	46.17

BC-From	20 pif at 46.17 to	20 pif at 53.23
BC-From	20 pif at 53.23 to	20 pif at 63.00
BC-From	10 pif at 63.00 to	10 pif at 67.67

IC- 41.30 lb Conc. Load at 64.27
TC- 106.17 lb Conc. Load at 66.27, 67.60
RC- 1823.07 lb Conc. Load at 64.27

BC- 1823.07 lb Conc. Load at 64.27
BC- 66.11 lb Conc. Load at 66.27,67.60

WARNING: Furnish a copy of this DWG to the installer. Failure to follow provisions of RCST in handling a

Failure to follow provisions of BCS1 in handling trusses can result in serious injuries. Do not permit untrained or inexperienced people to install trusses. See "WARNING" on page 10.

uninstructed people to install trusses. See "WARNING" recommentary, retaining a registered professional engineer.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

No. 66648

RECEIVED

RECEIVED

NOV 19 1999

10.03.11 0209.20 CITY

No. 66646

STATE OF FLORIDA

PROFESSIONAL ENGINEER

10/18/2012

	#1	#2	#3
Top chord	2x6	SP	SP
Bot chord	2x6	SP	SP
Webs	2x4	SP	SP

110 mph wind, 18.27 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCpf (+/-)=0.18

End verticals not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MINWFRS loads based on trusses located at least 36.53 ft. from roof edge.

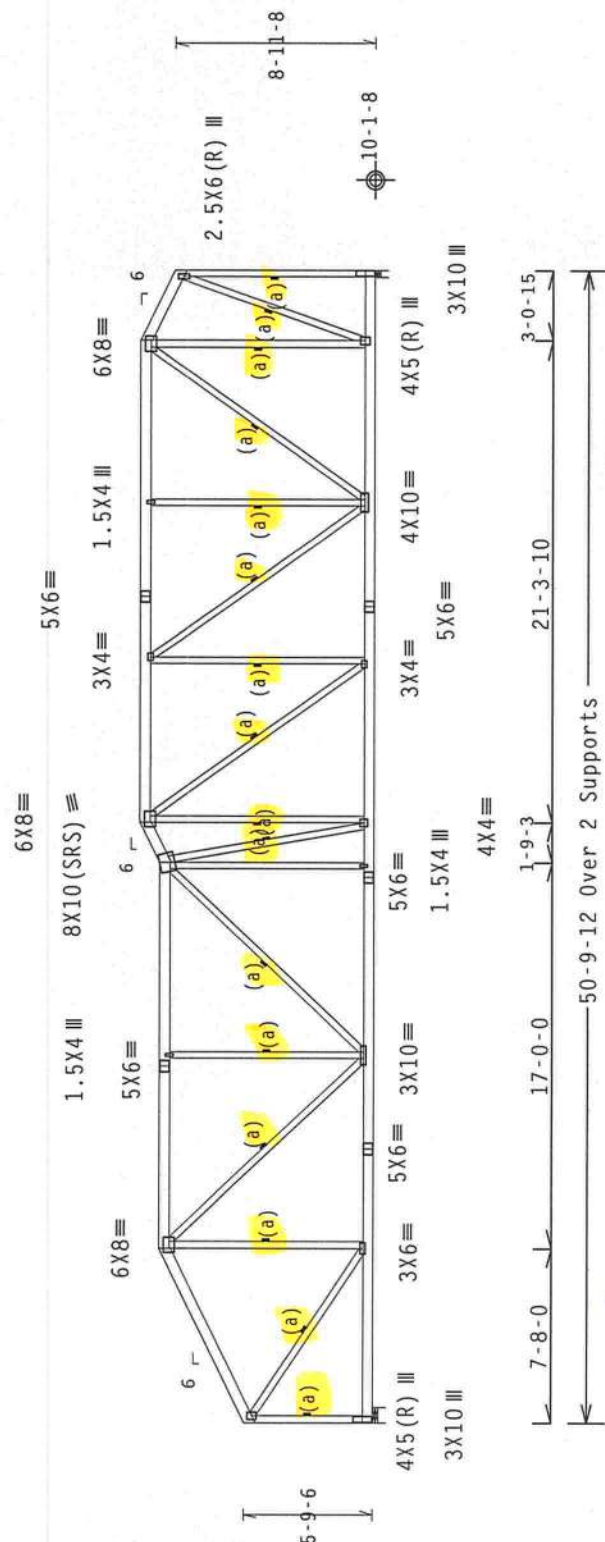
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=2092 U=46 W=8"
RL=86/-56

—50-9-12 Over 2 Supports

U=138 W=3.75"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$ FL/-/4/-/-/R/-
Scale = .125"/Ft.

TC LL	20.0	PSF	REF R487 -- 29403
TC DL	10.0	PSF	DATE 06/18/12
BC DL	10.0	PSF	DRW HCUR8487 12170087
BC LL	0.0	PSF	HC - ENG DF/DF
TOT.LD.	40.0	PSF	SEQN - 9138
DUR.FAC.	1.25		
SPACING	24.0"		JREF - 1UMZ487_Z01



****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**

****IMPORTANT** FURNISH TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Trussing shall be done in accordance with the American Institute of Steel Construction, Inc. (AISC) Manual of Steel Erection, Part 6, "Trusses." The following practices prior to performing these functions are compulsory:

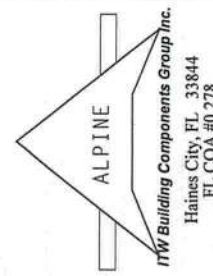
1. Trusses shall be installed as shown on drawings. No modifications shall be made without written approval from the Designer. If a modification is required, it shall have a properly attached structural steel design stamp.

2. Unless noted otherwise, top chord shall have properly attached commercial sheathing and bottom chord shall have properly attached plywood sheathing.

3. Trusses shall have permanent lateral restraints at all panel points.

4. Trusses shall have bracing installed per AISI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBGC) shall not be responsible for any deviation from these instructions. ITWBGC shall not be responsible for any damage to trusses or other components due to any failure to build the truss in conformance with AISI/TPI 1, or for handling, shipping, installing or bracing of trusses. Apply plates to each face of truss and position as shown above and on details. Unless noted otherwise, refer to Drawings 160A-2 for standard plate positions. A drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any particular application is the responsibility of the building Designer per AISI/TPI 1 sec.2. For more information see the responsibility of the building Designer per AISI/TPI 1 sec.2. For more information see: www.itwbuildingcomponents.com; itw@itwbuildingcomponents.com; 800-955-5555; 800-955-5555.



	Top chord	2x6	SP	#2
Top				
Bot				
	Web	2x4	SP	#3

110 mph wind, 18.27 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

End verticals not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MMWFRS loads based on trusses located at least 18.27 ft. from roof edge.

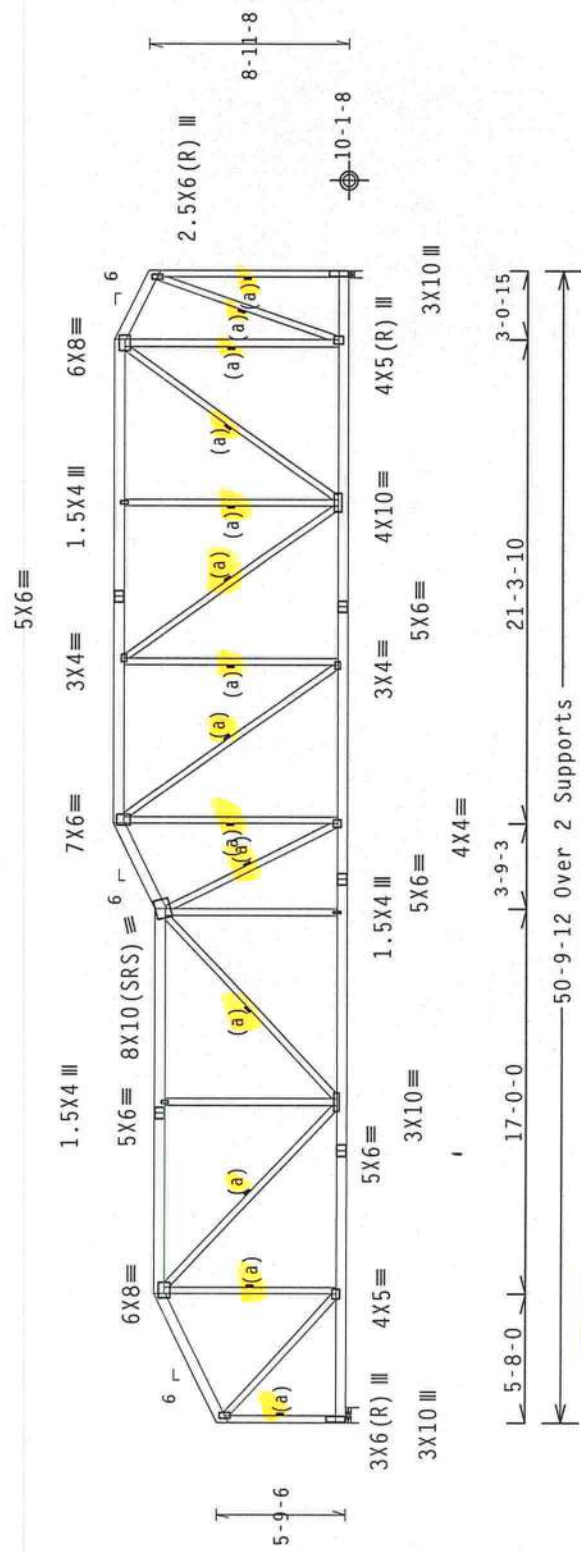
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R=2094 U=55 W=8"
RL=86 / -56

R=2092 U=139 W=3.75"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/- Scale = .125"/Ft.

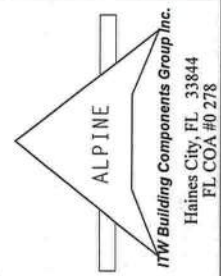
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TC DL	10.0	PSF	DATE 06/18/12
BC DL	10.0	PSF	DRW HCUSR487 12170088
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 9140
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1UMZ487_Z01

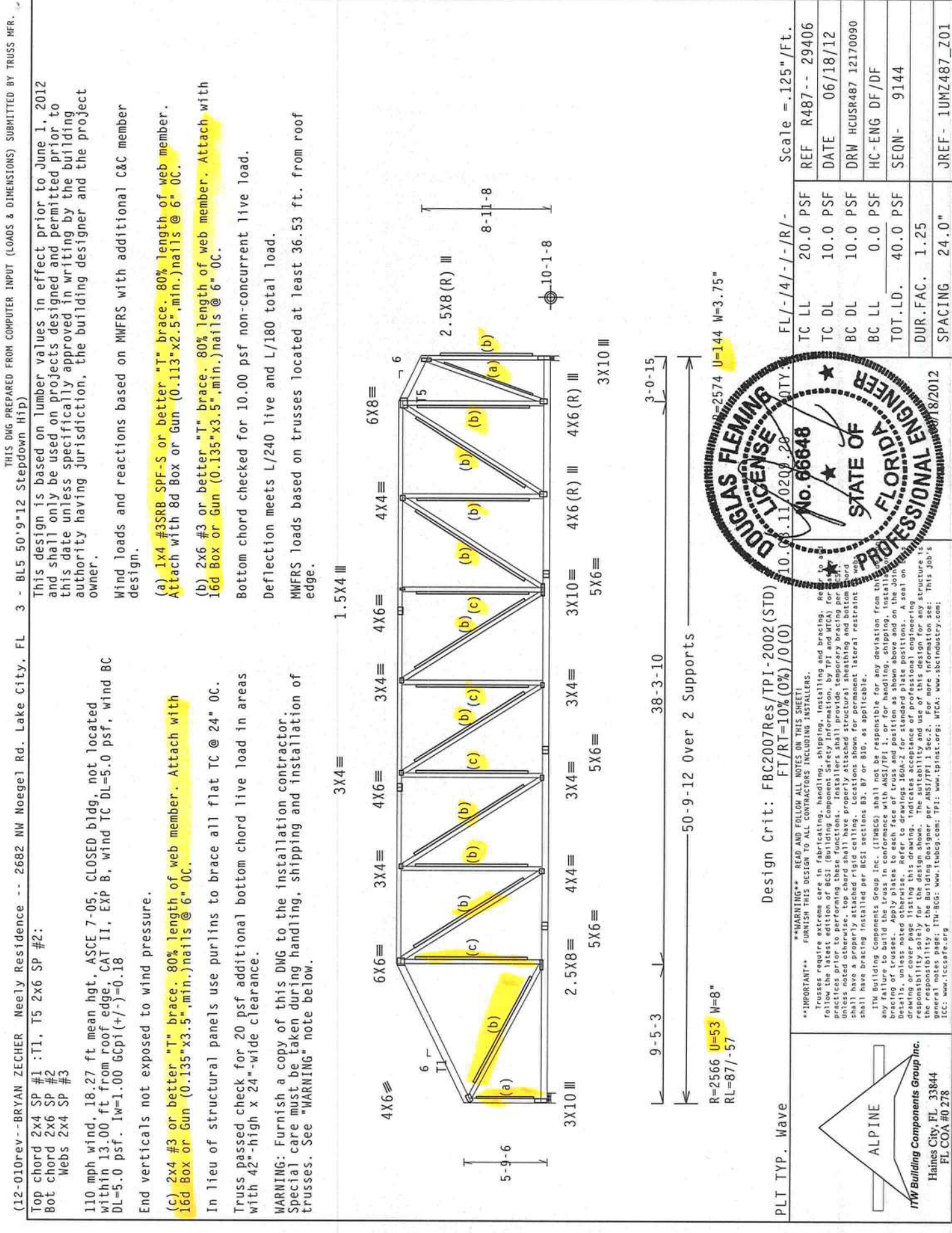


****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the following information for details. Trusses shall be installed in accordance with the design shown and the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for all practices prior to performing these functions. Installers shall provide temporary bracing per design. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Ceilings shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (INBCG) shall not be responsible for any deviation from the design shown or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing, or bracing. The designer shall be responsible for the position as shown above and on the Job Details, unless noted otherwise. Refer to the INBCG 1900-2 for details on the use of the design shown. Drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: general notes: IN-BCG: www.itwbcg.com; TPI: www.tpiinst.org; WCA: www.abcfindustry.com; ICC: www.iccsafe.org





Top chord 2x6 SP #2
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W3 2x6 SP SS:
110 mph wind, 18.27 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 13.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

End verticals not exposed to wind pressure.
(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 36.53 ft. from roof edge.
Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

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MMFRS loads based on trusses located at least 36.53 ft. from roof edge.
Laterally brace chord member above filler @ 24" O.C. or as specified, including a lateral brace at chord ends.

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads
-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
Tc- From 62 plf at 0.00 to 62 plf at 2.23
Tc- From 62 plf at 2.23 to 62 plf at 19.23
Tc- From 62 plf at 19.23 to 62 plf at 27.65
Tc- From 62 plf at 27.65 to 62 plf at 38.30
BC- From 4 plf at 0.00 to 4 plf at 38.30

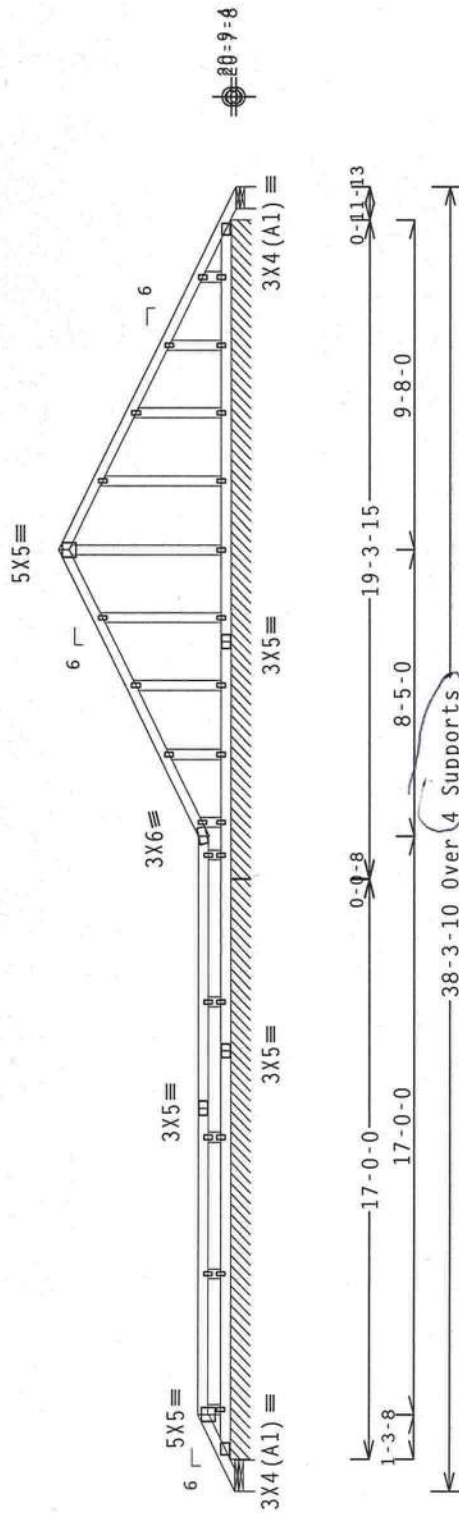
Wind loads and reactions based on MWFRS with additional C&C member design.
MWFRS loads based on trusses located at least 23.29 ft. from roof edge.
Refer to DWG PB1200310 for piggyback details.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 23.29 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. IW=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=21 U=4 W=7.825"

R=72 PLF U=8 PLF W=19.3

R=25 RW=54 U=50 W=11"

RL=467-RU6 U=17 PLF W=17-0-0

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10%(0%/0/0)

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for practices prior to performing these functions. Installers shall provide temporary bracing per details noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installing, or bracing the truss. Trusses shall be installed in accordance with the details shown on this drawing. Details, unless noted otherwise, shall refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this design, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; WCA: www.wcaindustry.com; ICC: www.iccsafe.org

Scale = .1875"/Ft.

FL/-4/-/-R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"

TY: 10.03.11 020928

No. 66848

DOUGLAS FLEMING LICENSE

STATE OF FLORIDA

PROFESSIONAL ENGINEER

06/18/2012

JREF - 1UMZ487_Z01



Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

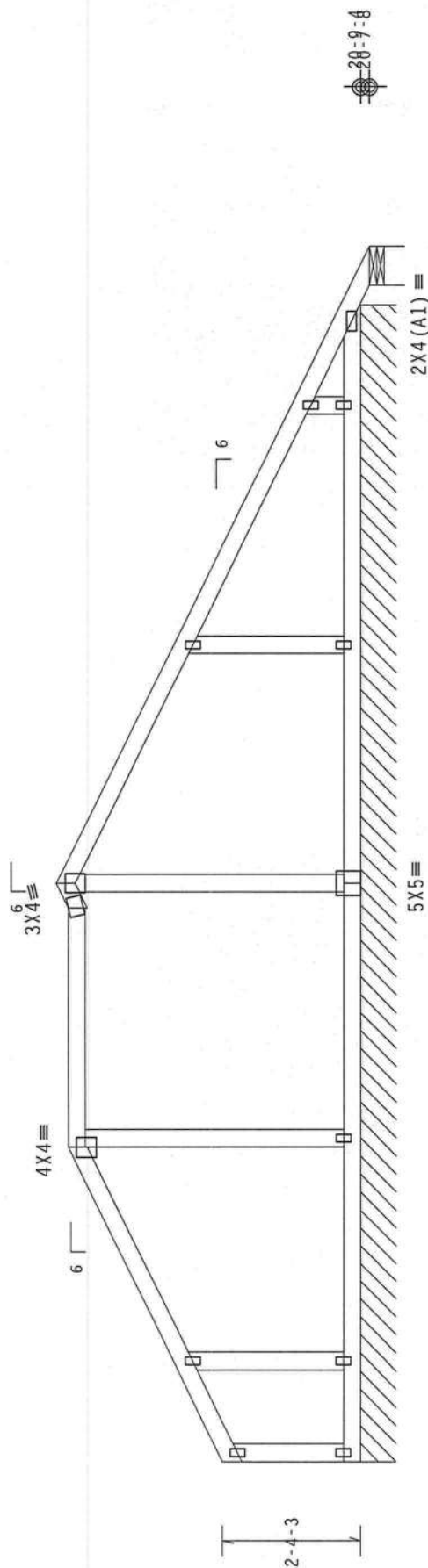
Special loads

Special	(Number	Dur.Fac.=1.25 /	Plate	Dur.Fac.=1.25)
TC-	From	62 pif at 0.00 to	62 pif at 5.25	
TC-	From	62 pif at 5.25 to	62 pif at 9.25	
TC-	From	62 pif at 9.25 to	62 pif at 13.25	
TC-	From	62 pif at 13.25 to	62 pif at 17.25	
BC-	From	4 pif at 0.00 to	4 pif at 20.31	

Wind loads and reactions based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Refer to DWG PB1200310 for piggyback details.

 $4 \times 4 =$ 

0-5-0	5-3-0	4-0-0
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-20-3-12 Over 2 Supports

R=67 PLF U=10 PLF W=19-3-15
RL=4/-6 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-

Scale = .375"/Ft.

***WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of ACI's Building Component Catalogue, by TPI and WCA for complete details. The following items should have properly attached structural sheathing and bottom chord bracing. Unless noted otherwise, top chord shall have permanent lateral restraint and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per ACI sections B3, B7 or B10, as applicable.

The Building Components Group Inc. (TUBCO) shall not be responsible for any deviation from this drawing or any failure, but it will accept responsibility for providing the correct installation details for bracing of trusses. Apply plates to each face of truss and position as shown above and on the plate. Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of Professional Engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per AISI/TC1 Section 2. For more information see: This Job's Specification. TUBCO, 11W-860, Waukegan, IL 60097; Tel: 847-933-1100; Fax: 847-933-1101; E-mail: info@tubco.com; Web: www.tubco.com.

A circular professional engineer seal for the State of Florida. The outer ring contains the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by stars. The center of the seal features a large, stylized signature. To the left of the signature, the number "No. 66648" is printed. To the right, the expiration date "06/18/2012" is printed. At the bottom of the seal, there is a small section of text that is partially cut off, which appears to read "This is a true and correct copy of the original on file with the State of Florida.".

TC LL	20.0 PSF	REF R487-- 29415
TC DL	10.0 PSF	DATE 06/18/12
BC DL	10.0 PSF	DRW HCUR487 12170099
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 9457
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UMZ487 Z01

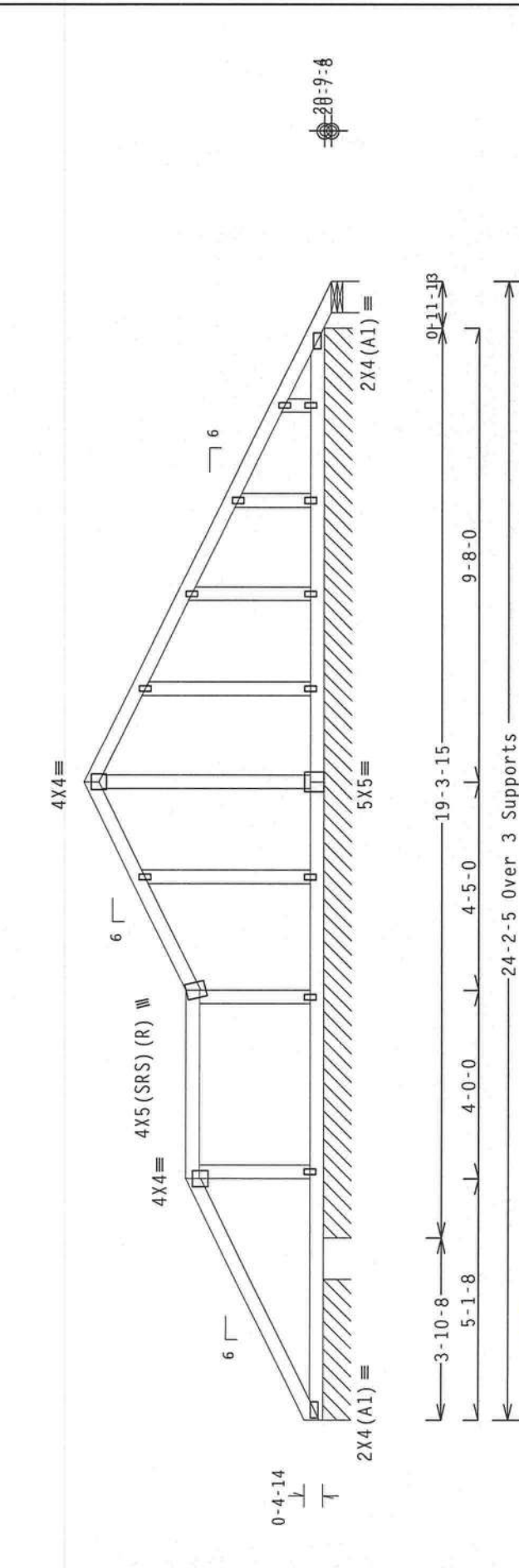
JREF- 1UMZ487_Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads
-----Lumber
Dur. Fac.=1.25 / Plate Dur. Fac.=1.25)
TC- From 62 pif at 0.00 to 62 pif at 5.13
TC- From 62 pif at 5.13 to 62 pif at 9.13
TC- From 62 pif at 9.13 to 62 pif at 13.54
TC- From 62 pif at 13.54 to 62 pif at 24.19
BC- From 4 pif at 0.00 to 4 pif at 13.54
BC- From 4 pif at 13.54 to 4 pif at 24.19

Wind loads and reactions based on MWFRS with additional C&C member design.
MWFRS loads based on trusses located at least 23.29 ft. from roof edge.

Refer to DWG PB1200310 for piggyback details.



R=70 PLF U=8 PLF W=3-0-0
RL=35/-37 PLF R=70 PLF U=10 PLF W=19-3-15

Note: All Plates Are 1.5X3 Except As Shown. Design Crit: FBC2007Res/TPI-2002 (STD) FT/RT=10%(0%/0/0)

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
06/18/2012

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per the details, unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; WCA: www.wcaindustry.com; ITC: www.itccast.org

TC LL	20.0 PSF	FL/-/4/-/-/R/-	Scale = .3125"/Ft.
TC DL	10.0 PSF		REF R487-- 29416
BC DL	10.0 PSF		DATE 06/18/12
BC LL	0.0 PSF		DRW HCUSR487 12170100
TOT.LD.	40.0 PSF		HC-ENG DF/DF
DUR.FAC.	1.25		SEQN- 9472
SPACING	24.0"		JREF- 1UMZ487_Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

110 mph wind, 15.53 ft mean ht., ASCE 7-05, CLOSED bldg. Located anywhere in roof, CAT II,
EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI(+/-)0.18

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss must be installed as shown with top chord up.

Provide { 2 } 16d common nails(0.162"x3.5"); toe nailed at Top chord.
Provide { 2 } 16d common nails(0.162"x3.5"); toe nailed at Bot chord.

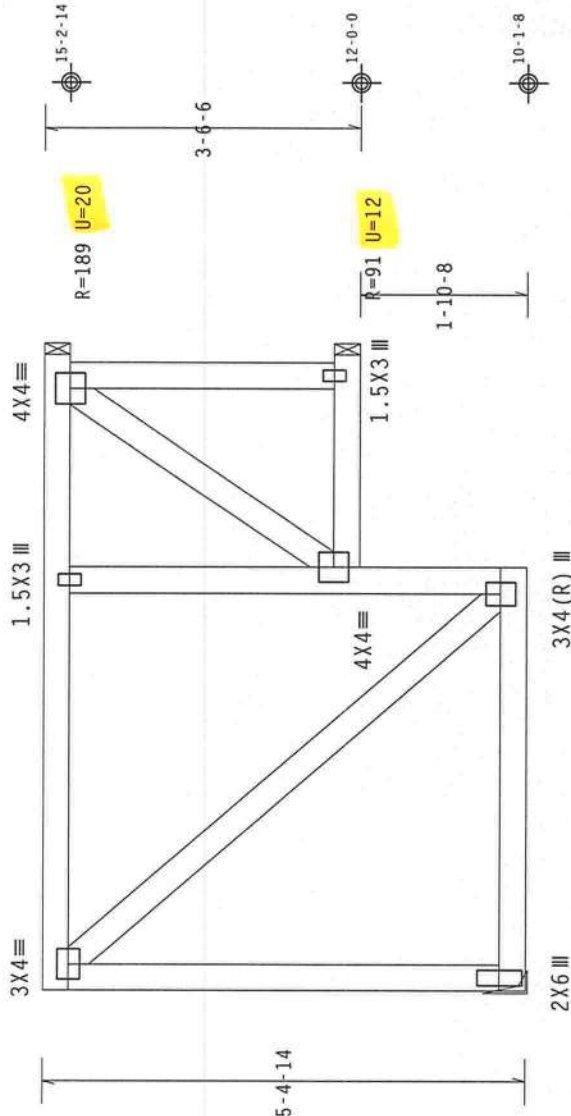
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

(H1) = (J) Hanger not calculated (1)2x6 SP #1 Dense supporting member.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 31.07 ft. from roof edge.



4-8-0
7-0-0 Over 3 Supports 2-4-0

R=280 U=32
H=H1

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%/0(0))

Scale = .5"/Ft.

FL/- /4/- /- /R/-

TY:

10.00.11.0208.20

10.00.11.0208.20

10.00.11.0208.20

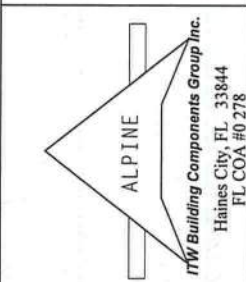
10.00.11.0208.20

10.00.11.0208.20

TC LL	20.0 PSF	REF	R487-- 29419
TC DL	10.0 PSF	DATE	06/18/12
BC DL	10.0 PSF	DRW	HCUSR487 12170004
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	9540
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UMZ487_Z01



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraints of top chord shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. It is the responsibility of the contractor to ensure that the truss is installed in accordance with the design. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's ITCI notes pages; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.bciindustry.com; ITCI: www.itcsafe.org



(12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - LG 13'5" Mono Hip Girder)

Top chord 2x6 SP #2
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 31 plf at 0.00 to 31 plf at 9.29
TC- From 62 plf at 9.29 to 62 plf at 13.42
BC- From 10 plf at 0.00 to 10 plf at 13.42
BC- 288.26 lb Conc. Load at 1.73, 3.73, 5.73, 11.73
BC- 280.00 lb Conc. Load at 7.73, 9.73

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

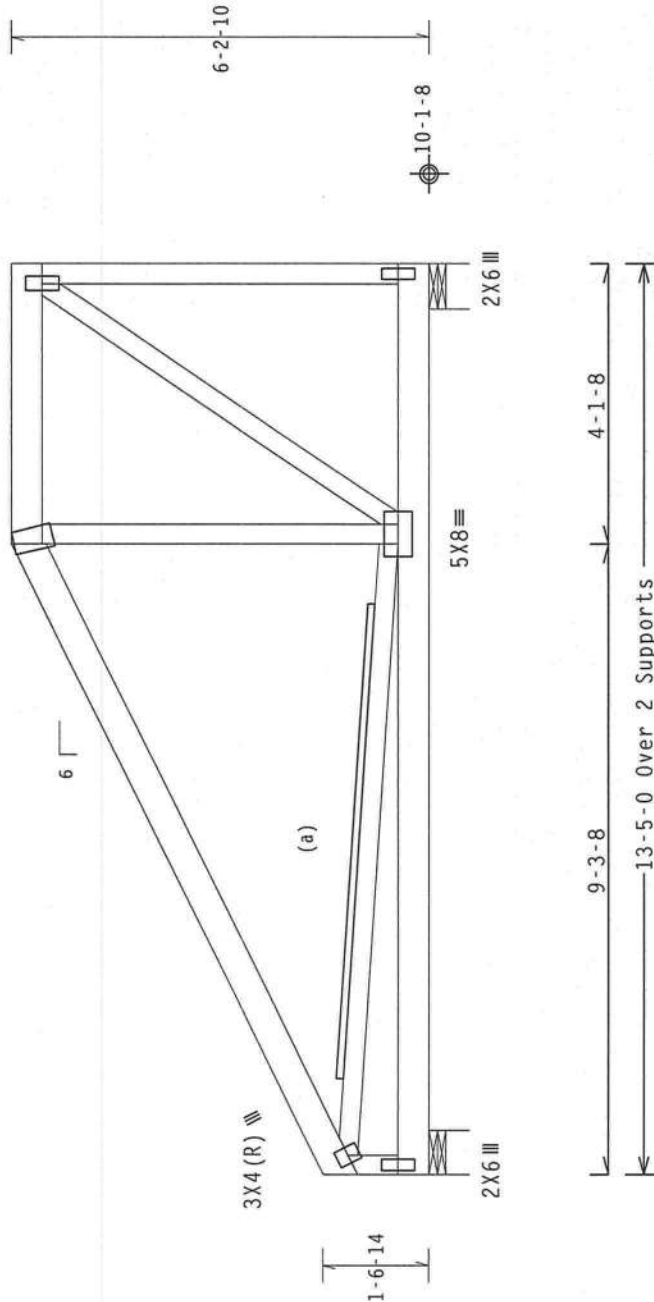
This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf(+/-)=0.18

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.
4X7(R) 2.5X6(R)



R=1152 U=77 W=8"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 - 29422
TC DL	10.0 PSF	DATE	06/18/12
BC DL	10.0 PSF	DRW	HCUSR487 12170105
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	9544
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1UM7487_Z01

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
This Job No. 12/08/2012

ALPINE
Haines City, FL 33844
FL COA #0 278

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the manufacturer's instructions for details. Do not alter the design or materials without the approval of the designer. The designer shall be responsible for the design and the manufacturer shall be responsible for the fabrication and installation. The manufacturer shall be responsible for the bracing and the designer shall be responsible for the design of the bracing. The manufacturer shall be responsible for the design of the bracing and the designer shall be responsible for the design of the bracing.

WUERS loads based on trusses located at least 16.97 ft. from roof edge.


$$\text{Scale} = 375''/E +$$

ITW Building Components Group Inc. (ITWBC) shall not be responsible for any deviation from the design shown on the drawings, or any failure to build the truss in conformance with ANSI/TPI 1 for building trusses. ITWBC shall be responsible for the design, engineering, detailing, and manufacturing of the trusses. ITWBC shall be responsible for the installation of the trusses, including the bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on the drawing or cover plate listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's drawings. ITW-BCO: www.itwbcop.com; ITW-TPO: www.tpiwtrp.com; ITW-MCA: www.sbcindustrial.com; ITW-CE: www.160series.com



TC LL	20.0	PSF	REF R487-- 29423
TC DL	10.0	PSF	DATE 06/18/12
BC DL	10.0	PSF	DRW HCUSR487 12170005
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 9545
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 11UW7487 701

06/18/2012

	Top	chord	2x6	SP	#2
	Bot	chord	2x6	SP	#2
		webs	2x4	SP	#3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

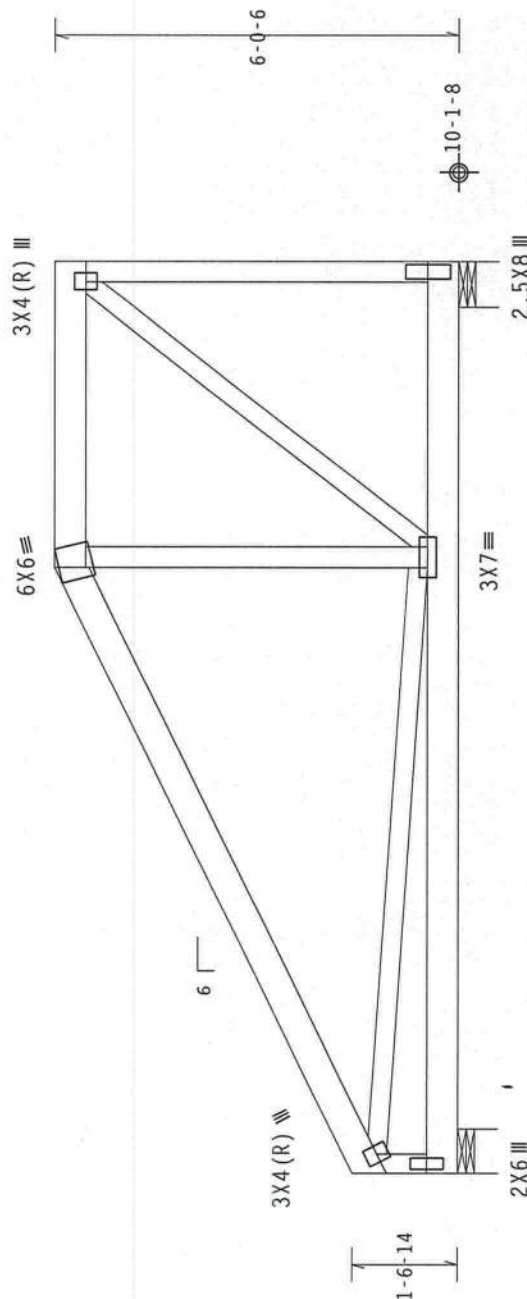
MMWFRS loads based on trusses located at least 30.00 ft. from roof edge.

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load.



8-11-0
13-5-0 Over 2 Supports
4-6-0

R=553 U=0 W=8"
RL=68/-23

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

FI 1-141-1-1R/-
Scale = 375"/ft

20 0 PSF	PFF	P497	2042E
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20:0130	DATE	06/06/10
20:0130	REF	K4875-29423

CUL	10.0 PSF	DATE	06/18/12
-----	----------	------	----------

10.0 PSF	DRW HCUSR487 12170007
----------	-----------------------

HC-ENG	DF/DF
0.0	PSF
LL	

OT.LD.	40.0 PSF	SFON-	9547
--------	----------	-------	------

1110	EAC	1	25
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No. 66648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

06/18/2012

This design is based on lumber values in effect prior to June 1, 2012 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

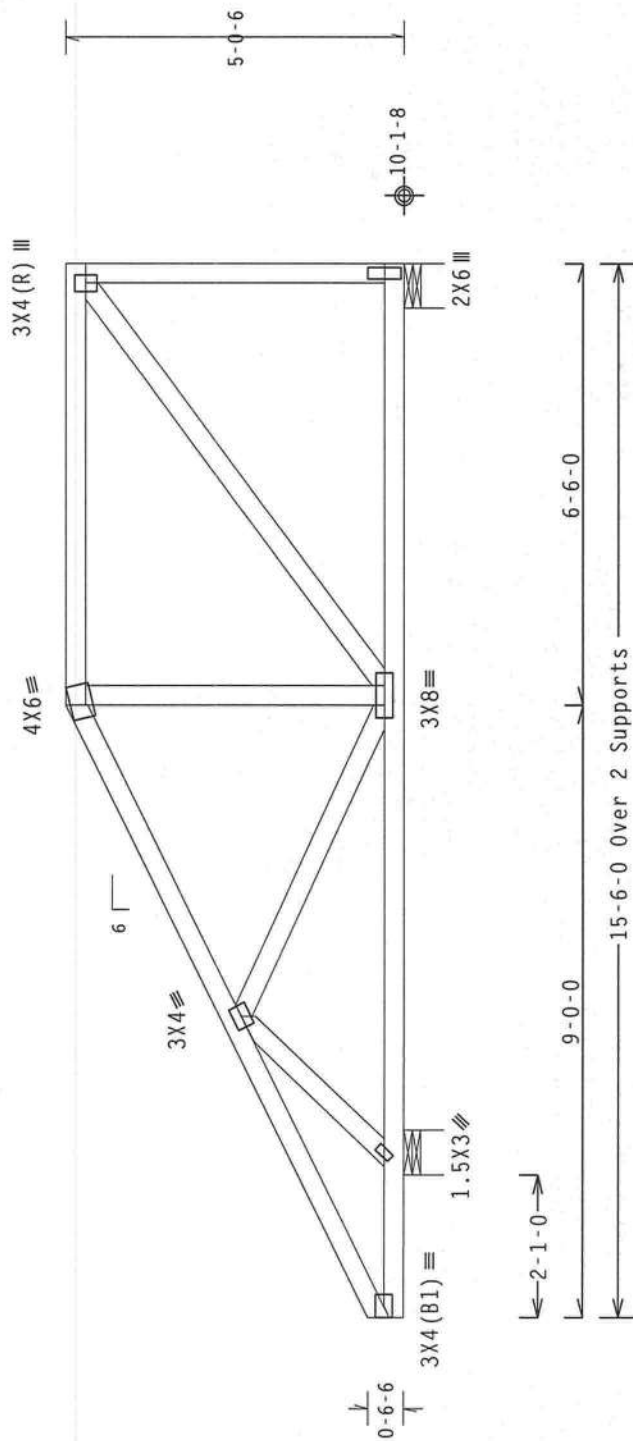
1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 9.00 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.18

Right end vertical not exposed to wind pressure.

Left cantilever is exposed to wind

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1108 H=207 W=8"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-
Scale = .375"/Ft.

REF	R487--	29428
DATE	06/18/12	
DRW	HCUSR487	12170108
HC-ENG	DF/DF	
SEGN-	9552	
JREF-	1UMZ487	Z01

The seal is circular with a dashed outer border. Inside the border, the text "STATE OF FLORIDA" is written in a semi-circle at the top, and "PROFESSIONAL ENGINEER" is written in a semi-circle at the bottom. In the center, the number "No. 66848" is printed vertically. To the right of the number, the date "11/18/2012" is printed vertically. A signature is written across the seal, and a star is visible in the upper left corner.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT****

Tusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI Building Component Safety Information by TPI and MICA for complete instructions prior to performing those functions. Installers shall provide temporary bracing per the manufacturer's instructions unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint shall have bracing installed per BCST sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (IBCG) shall not be responsible for any deviation from the design due to improper failure to build the truss in conformance with ANSI/TPI 1-1, or for Handling, shipping, installation or bracing of tusses. Apply plates to each face of truss and position as shown above and on the details, unless noted otherwise. Refer to Drawing I60A-2 for standard plate positions. A seal drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building Designer per ANSI/TPI 1 sec.2. For more information see: [http://www.itwibc.com; ITW-BGCS; w-luckey@itw.com; ITW-TPI/tipi.mrktg; MICA; www.micausa.org](#).



ALPINE
Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

Top chord 2x6 SP #2 :T1, T5 2x4 SP #1:

Bot	chord	2x6	SP #2
	webs	2x4	SP #3

Webs 2x4 SP #3

Left and right cantilevers are exposed to wind

(a) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5" min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and untrained people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

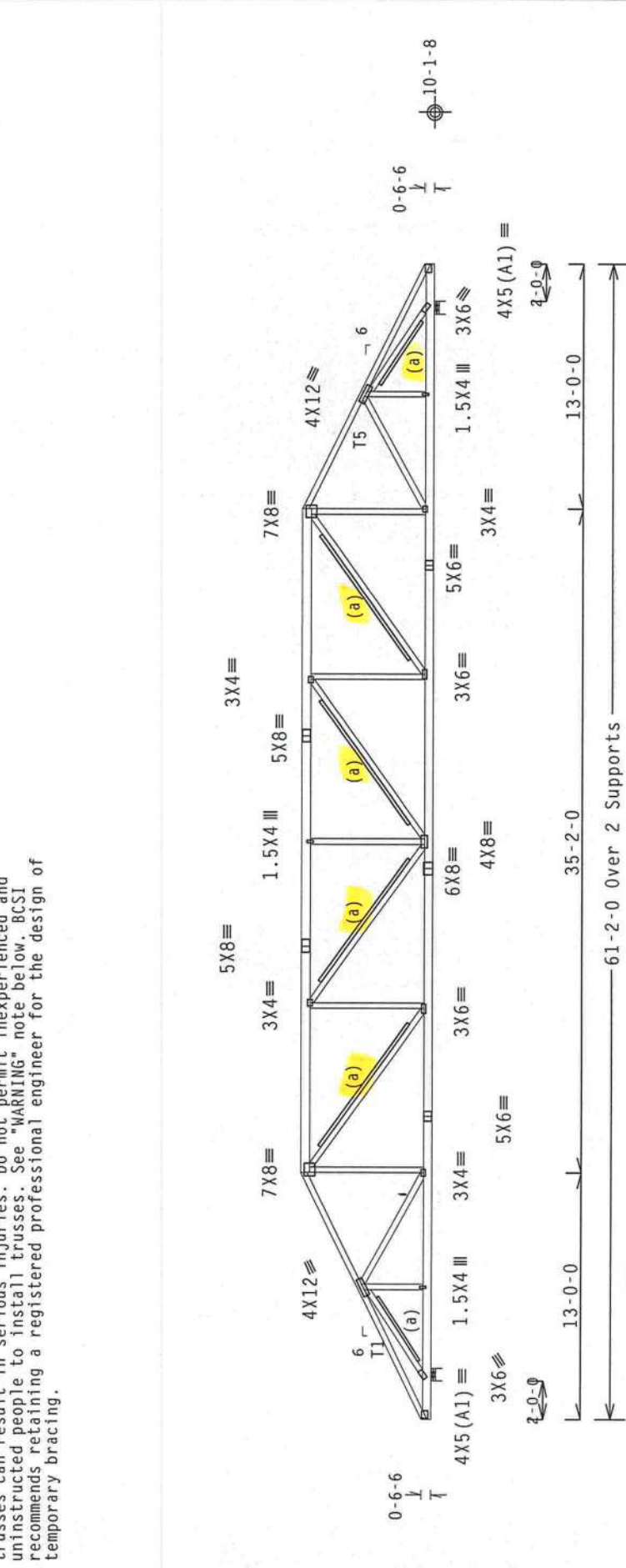
110 mph wind, 15.00 ft mean ht, ASCE 7-05, CLOSED bldg, not located within 8.50 ft from roof edge, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=2519 U=235 W=8"
RL=150/-150

-61-2-0 Over 2 Supports

R=2519 U=235 W=8"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

FL/-/4/-/-/R/-/- Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for best practices prior to performing these functions. Installers shall provide temporary bracing per the details noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint web shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.

I/TW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installing or bracing of the truss. The user of this design shall be responsible for the details of the design. Details and covers noted otherwise. Refer to drawings 100A-2 for standard plate positions. A steel on steel drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's formal notes page. ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; WCA: www.bcsiindustry.com; ITC: www.itccafe.org

No. 66648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

DATE OF EXPIRATION: 08/18/2012

TC LL	20.0	PSF	REF R487 - -	29434
TC DL	10.0	PSF	DATE	06/15/12
BC DL	10.0	PSF	DRW HCURS487	12167025
BC LL	0.0	PSF	HC-ENG DF/DF	
TOT.LD.	40.0	PSF	SEQN-	262355
DUR.FAC.	1.25			
SPACING	24.0"		JREF-	1UMZ487_Z01

ALPINE

I/TW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(12-010rev--BRYAN ZECHER Neely Residence -- 2682 NW Noegel Rd. Lake City, FL 3 - H17B 61'2" Stepped Hip)
 Top chord 2x6 SP #2 :T1, T6 2x4 SP #1:
 Bot chord 2x6 SP #2
 Webs 2x4 SP #3
 110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located
 within 17.00 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf,
 wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18
 THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Left and right cantilevers are exposed to wind

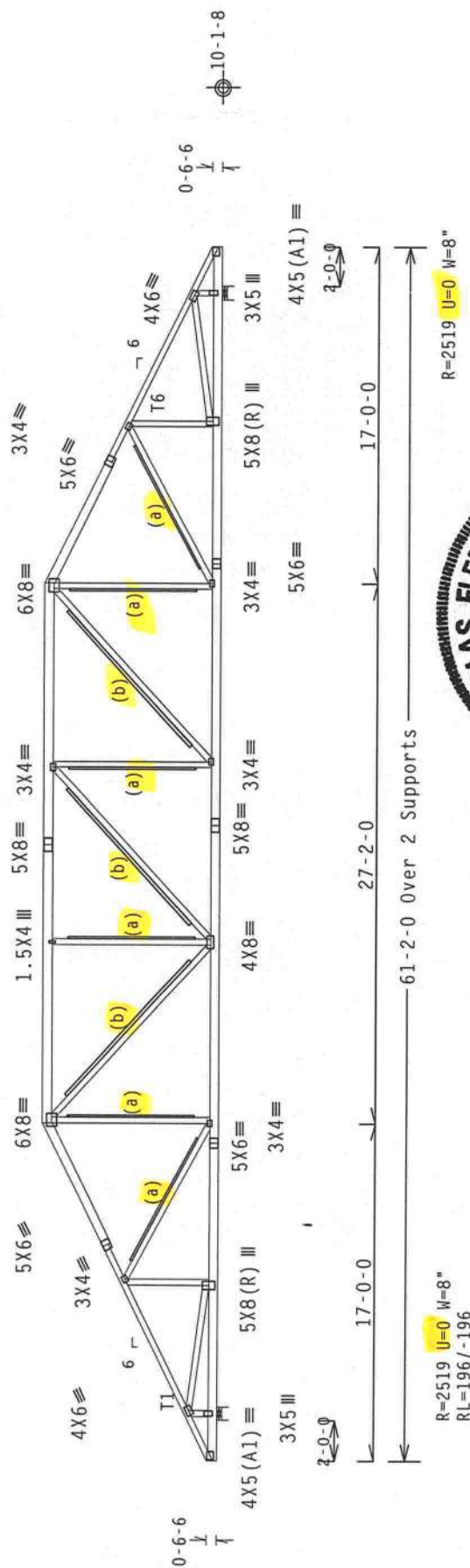
(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

WARNING: Furnish a copy of this DWG to the installation contractor. Failure to follow provisions of BCSI in handling and installation of trusses can result in serious injuries. Do not permit inexperienced and uninstructed people to install trusses. See "WARNING" note below. BCSI recommends retaining a registered professional engineer for the design of temporary bracing.

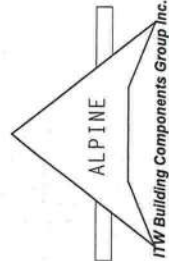
Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS EMPLOYED, INSTALLERS

[illegible]

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of the building components. ITWBCG shall not be responsible for any deviation from any manufacturing of the building components. ITWBCG shall not be responsible for any deviation from any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation, or any other details, unless noted otherwise. Refer to drawings 180A-2 for standard plate positions. A seal on the drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 sec.2. For more information see: www.itwbcg.com; ITW-800: www.1800.com; TPI: www.tpiinst.org; WCA: www.wcaintell.com; ECI: www.eciassoc.org



Haines City, FL 33844
FL COA #0278

DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
03/04/06 to 01/15/11
QTY: 1

FL/-/4/-/-/R/-		Scale = .125" /Ft.
TC LL	20.0 PSF	REF R487-- 29436
TC DL	10.0 PSF	DATE 06/15/12
BC DL	10.0 PSF	DRW HCUR487 12167027
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 262379
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UM7487_Z01