

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1U22215-Z0124153512

Truss Fabricator: W.B. Howland
Job Identification: 6810-/ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL
Truss Count: 11
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD)
Engineering Software: Alpine Software, Version 9.02.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-05 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR215

Details: A1101505-GBLLETIN-BRCLBSUB-A1001505-GABRST05-PB120-

#	Ref	Description	Drawing#	Date
1	92711--A1		10144001	05/24/10
2	92712--A2		10144002	05/24/10
3	92713--A3		10144003	05/24/10
4	92714--A4		10144004	05/24/10
5	92715--A5		10144005	05/24/10
6	92716--A6		10144006	05/24/10
7	92717--A7		10144007	05/24/10
8	92718--A8		10144008	05/24/10
9	92719--A9		10144009	05/24/10
10	92720--A10		10144010	05/24/10
11	92721--A11		10144011	05/24/10



Seal Date: 05/24/2010

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

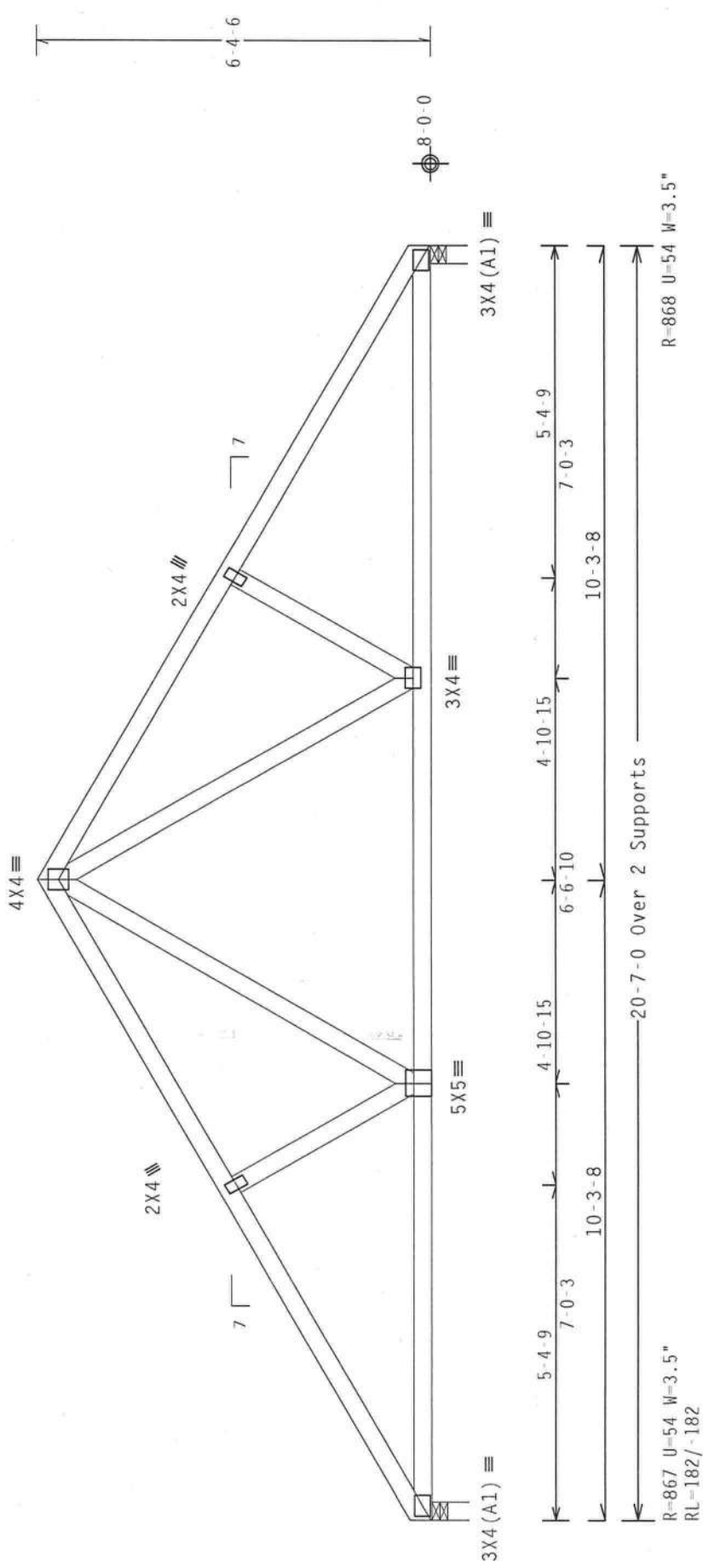


Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpl(+/-)0.18

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24" wide clearance.
Deflection meets L/240 live and L/180 total load.
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.
Bottom chord checked for 10.00 psf non-concurrent live load.
The overall height of this truss excluding overhang is 6'-4"-6".



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

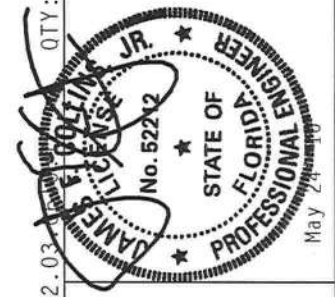
PLT TYP. Wave	QTY: 2	FL/-/5/-/-/R/-	Scale = .375"/Ft.
REF	R215--	92713	
DATE	05/24/10		
DRW	HCUSR215	10144003	
HC-ENG	SSB/AP	*	
SEQN	263871		
FROM	CDM		
JREF	1U22215_Z01		

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FLCOA #0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. TRUSS FABRICATOR SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P&A) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/1666A (H/H/SS/SS) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/P&A) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/1666A (H/H/SS/SS) ASTM A653 GRADE 40/60 (H, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 1600-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



(6810-/ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL - A4)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

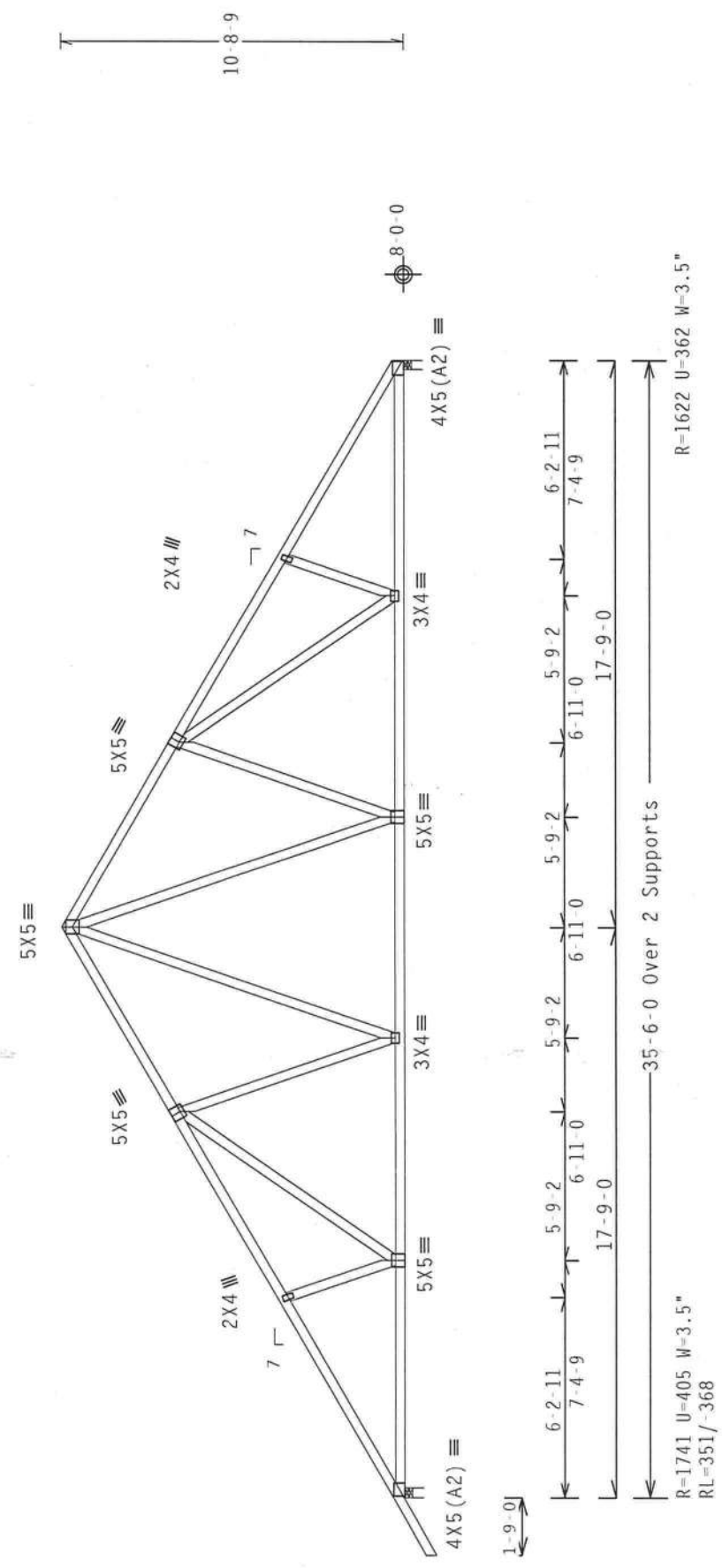
Wind reactions based on MWFRS pressures.

Truss passed check for 20 psf additional bottom chord live load in
areas with 42"-high x 24" wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 10-8-9.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

Scale = .1875"/Ft.

QTY: 7

9.02.02

PLT TYP. Wave

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	
SPACING	24.0"	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, RADFORD, VA 22379) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** PROVIDE A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. ENG. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THE DESIGN OR FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/PJA) AND TPI. THE REG. ENGINEER'S SEAL IS LOCATED ON THIS DESIGN. POSITION PER DRAWING 160A-2. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FLCOA #0278

(6810-/ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL - A5)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt. ASCE 7-05, CLOSED bldg, located anywhere in roof, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

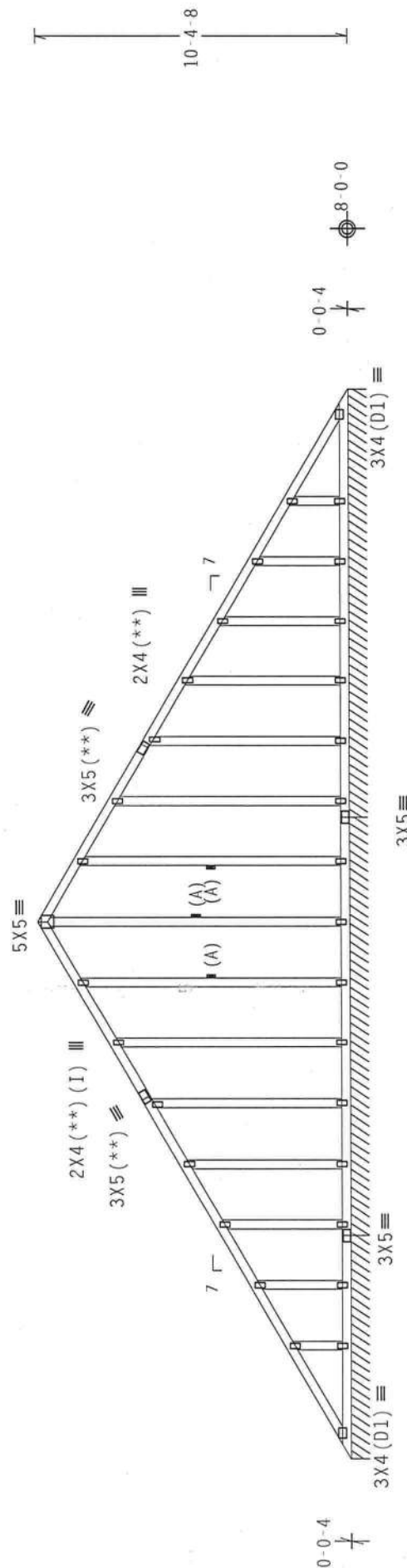
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24" wide clearance.

Deflection meets L/240 live and L/180 total load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10'-4"-8".



R=117 PLF U-33 PLF W=7-4-9
R=135 PLF U-32 PLF W=13-9-15
R=86/-86 PLF

Note: All Plates Are 2x4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

Scale = .1875"/Ft.

QTY: 2

FL/-/5/-/R/-

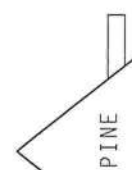
TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R215 -- 92715
DATE	05/24/10
DRW	HCUSR215 10144005
HC-ENG	SSB/AP
SEQN	264520
FROM	CDM
JREF	1U22215_Z01

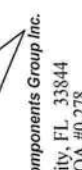
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, FOR THE LATEST EDITION OF THE TRUSS MANUFACTURING AND INSTALLATION GUIDE. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2018/166A (A572M A572) GRADE 40/60 (A572M A572) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SELECT FOR THE TRUSS COMPANY DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASHRAE/TP1 1 SEC. 2.



ALPINE



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



(6810-/ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL -- A6)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf, Iw=1.00 GCpi (+/-)-0.18

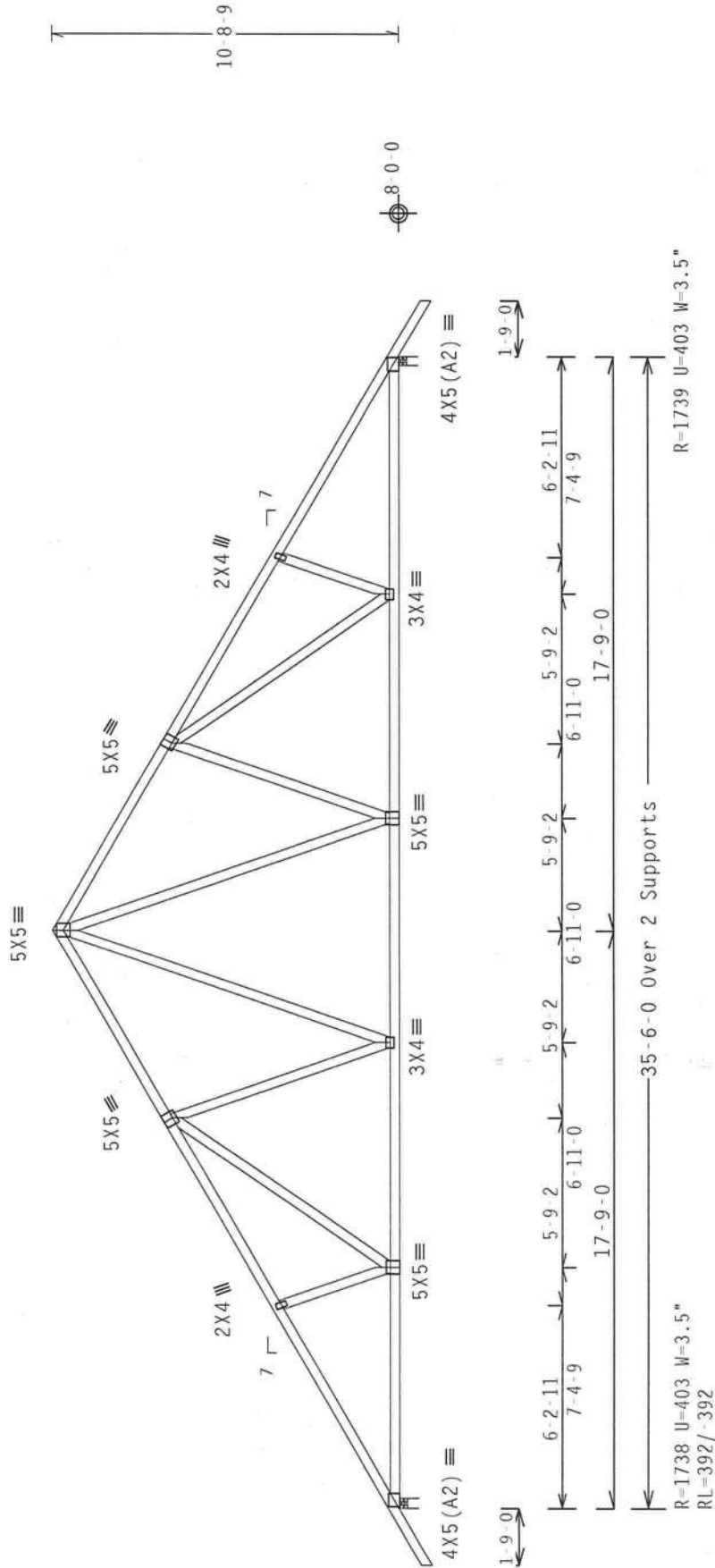
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Truss passed check for 20 psf additional bottom chord live load in
areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-8-9
Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

Scale = .1875"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R215 -- 92716
DATE	05/24/10
DRW	HCUSR215 10144006
HC-ENG	SSB/AP
SEQN	263879
FROM	CDM
JREF	1U22215_Z01

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** UNLESS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AIA (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (0.0155") ASH 6053 GRADE 40/60 (0.0155") GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/PA 3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

(6810 /ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL - A7)

Top chord 2x4 SP #2 N :13, 16 2x6 SP #2 N :
Bot chord 2x4 SP #2 N :
Webs 2x4 SP #2 N :

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

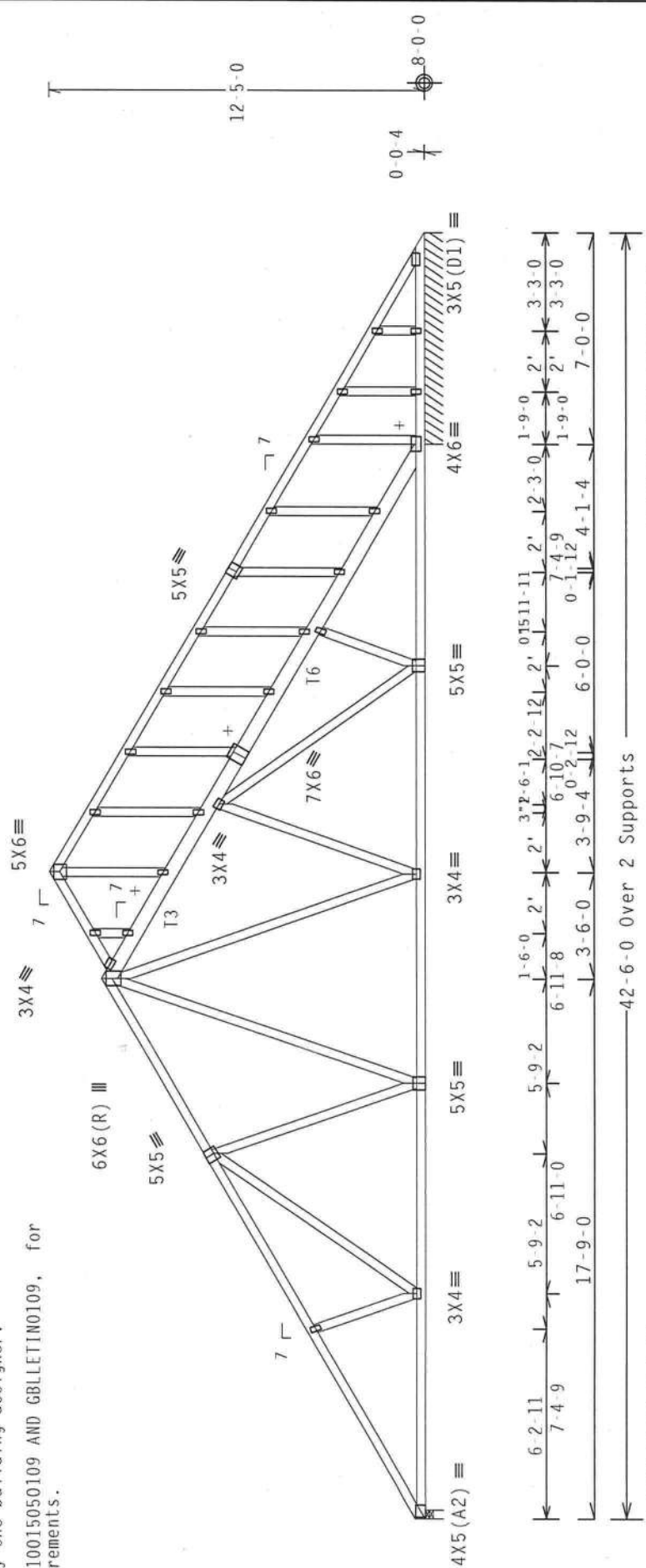
The overall height of this truss excluding overhang is 12 5-0.

Deflection meets L/240 live and L/180 total load.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the Gable end. All connections to be designed by the building designer.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

See DWGS A10015050109 AND GBLLETIN0109, for more requirements.



R=1600 U=341 W=3.5"
RL=394/-399

R=306 PLF U=73 PLF W=7-0-0

Note: All Plates Are 2x4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

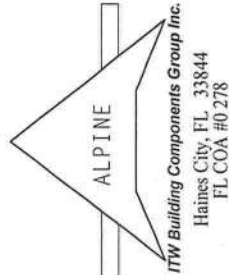
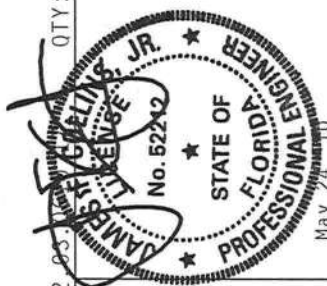
QTY: 2 FL/-/5/-/-/R/- Scale = .1875"/Ft.

PLT TYP. Wave

TC LL	20.0 PSF	REF	R215--	92717
TC DL	10.0 PSF	DATE	05/24/10	
BC DL	10.0 PSF	DRW	HCUSR215	10144007
BC LL	0.0 PSF	HC-ENG	SSB/AP	
TOT.LD.	40.0 PSF	SEQN	264525	
DUR.FAC.	1.25	FROM	CDM	
SPACING	24.0"	JREF	1U22215_Z01	

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, RADFORD, VA 53715) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. 1TH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA (0.0155/0.0155) ASTM A653 GRADE 40/60 (0.4 K/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC/A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISI/TPI 1 SEC. 2.



(6810-/ACOSTA RESIDENCE /MO PERKINS -- LAKE CITY, FL - A8)

Top chord 2x4 SP #2 N
Bot chord 2x4 SP #2 N
Webs 2x4 SP #2 N

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.16" due to live load and 0.14" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24" wide clearance.

Deflection meets L/240 live and L/180 total load.

The overall height of this truss excluding overhang is 10-0-0.

Negative reaction(s) of -245# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

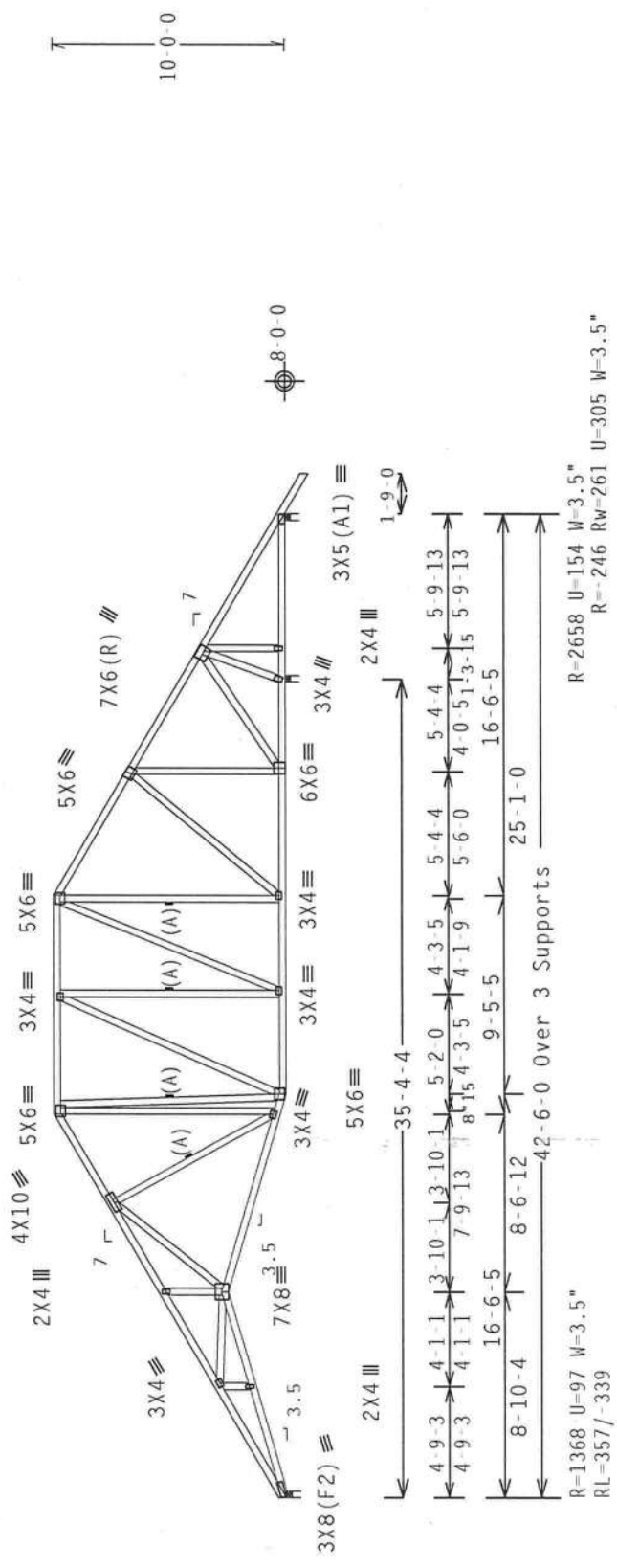
Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Shim all supports to solid bearing.

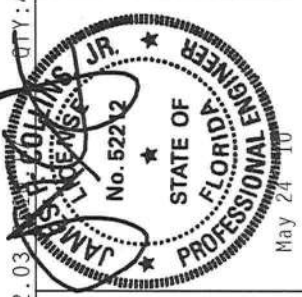
MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

TC LL	20.0 PSF	REF	R215-- 92718
TC DL	10.0 PSF	DATE	05/24/10
BC DL	10.0 PSF	DRW	HCUSR215 10144008
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT.LD.	40.0 PSF	SEQN-	263898
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1U22215_Z01



PLT TYP. Wave

Scale = .125" / Ft.

9.02.03

QTY: 4

FL / - / 5 / - / - / R / -

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	#2	N
Bot	chord	2x4	SP	#2	N
	Webs	2x4	SP	#2	N

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

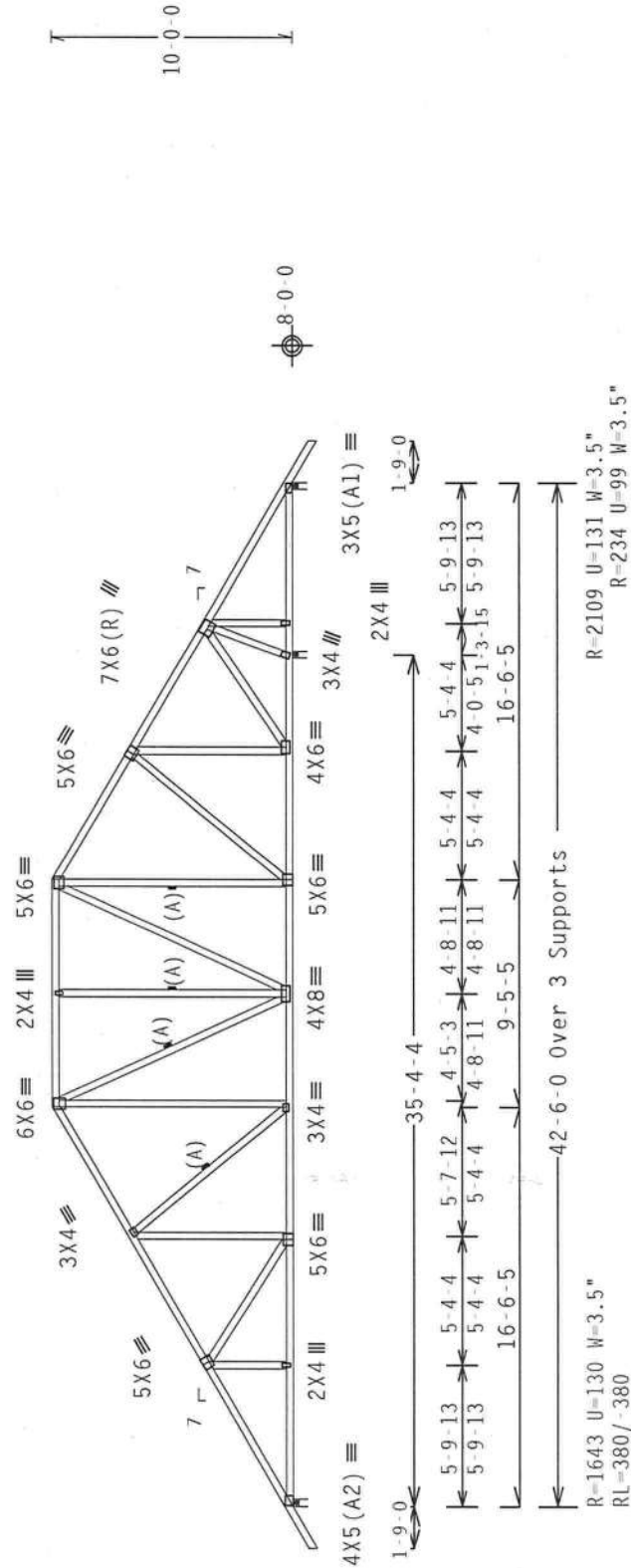
MMWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-0-0.

Design Crit: $\text{FBC2007Res/TPI-2002(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

PLT TYP. Wave

Scale = .125" / Ft.

****WARNING**** THESESS REQUIRE EXTRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING PANELS. PROPERLY ATTACHED CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

REF R215-- 92720

DATE 05/24/10

DATE	05/24/10
DR#	HCUR001F 10111010

URW HCUSRZ15 10144010

HC-ENG SSB/AP

SEQN- 263894

FROM CDM

JREF- 1U22215 Z01



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top	chord	2x4	SP	#2	N
Bot	chord	2x4	SP	#2	N
	webs	2x4	SP	#2	N

1110 mph wind, 19.39 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ $C_{Cpi}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

The overall height of this truss excluding overhang is 2.9.1.

Refer to drawing PB1200109 for piggyback detail.
Top chord of supporting truss under piggyback to be
braced @ 24" O.C., unless otherwise specified.

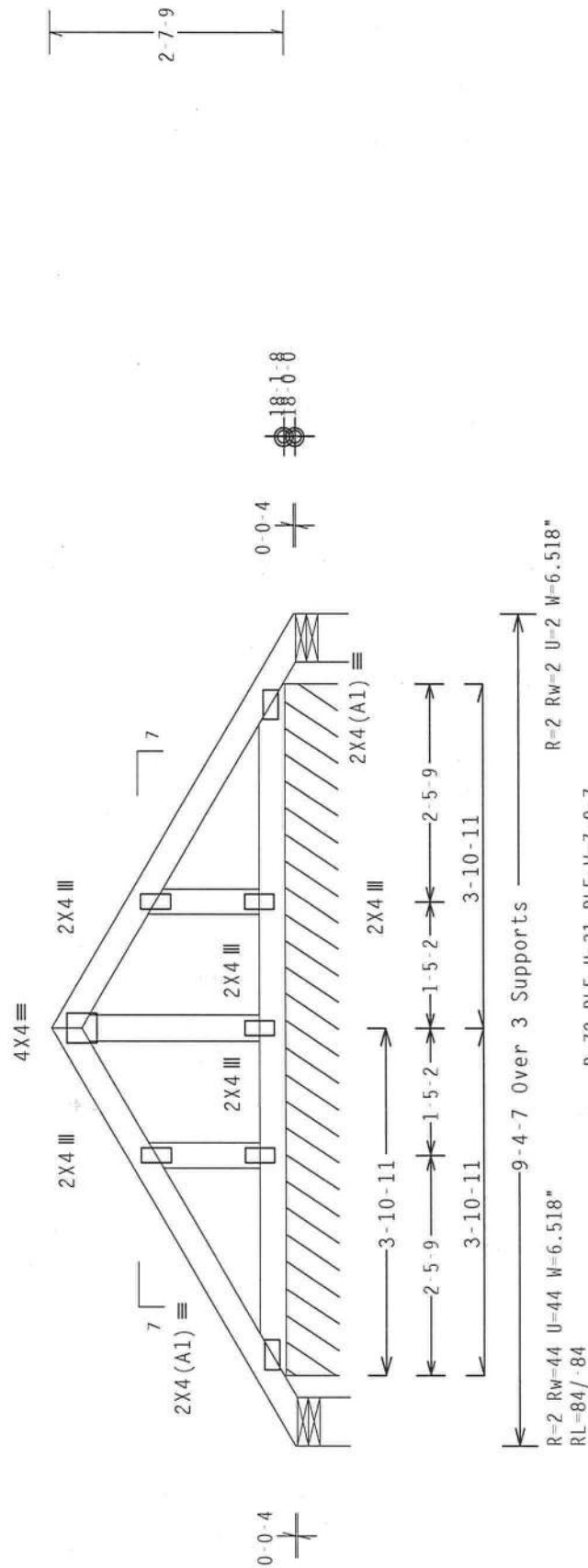
Special loads

----- (Lumber	Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25
TC- From	60 plf at -0.79 to 60 plf at 3.89
TC- From	60 plf at 3.89 to 60 plf at 8.58
BC- From	4 plf at -0.79 to 4 plf at 8.58

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

WUFRS loads based on trusses located at least 9.69 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=20\%(0\%)/10(0)$

PLT TYP. Wave

Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R215-- 92721
TC DL	10.0 PSF	DATE	05/24/10
BC DL	10.0 PSF	DRW	HCUR215 10144011
BC LL	0.0 PSF	HC-ENG	SSB/AP
TOT.LD.	40.0 PSF	SEQN-	263899
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1U22215_Z01



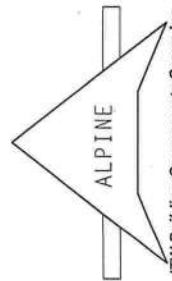
*****WARNING***** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE TRUSS PAPER INSTITUTE, 2300 HIGHT STREET, SUITE 100, HOUSTON, TEXAS 77058. (713) 572-7000. (713) 572-7001. FAX (713) 572-7002. E-MAIL: TRUSS@TRUSSPAPER.COM. OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1- OR TABRICATING, HANDLING, SHIPPING, INSTALLING & ERECTING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC., AIAA900 AND TP1. ITW BEG CONNECT PLATES ARE TO BE USED ON ALL JOINTS UNLESS OTHERWISE LOCATED IN THIS DESIGN. POSITION PER DRAWINGS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIAA 900 & TP1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROSS COMPONENT DESIGN SHOWN.

THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIAA900 TP1 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

GABLE STUD REINFORCEMENT DETAIL

2X4 GABLE VERTICAL SPACING		BRACE		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
		GRADE	NO BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
MAX GABLE VERTICAL LENGTH	24" O.C.	SPF	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"
		HF	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"
		STANDARD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"
		SP	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"
24" O.C.	24" O.C.	DFL	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"
		STANDARD	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"
		SPF	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"
		HF	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	16" O.C.	STANDARD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"
		SPF	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	14' 0"	14' 0"
		HF	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"
12" O.C.	12" O.C.	SPF	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"
		HF	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 4"	14' 0"	14' 0"
		STANDARD	4' 11"	8' 5"	8' 5"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"
		SP	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	12" O.C.	DFL	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"
		STANDARD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"
		SPF	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"
		HF	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"
12" O.C.	12" O.C.	STANDARD	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"
		SPF	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"
		HF	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"

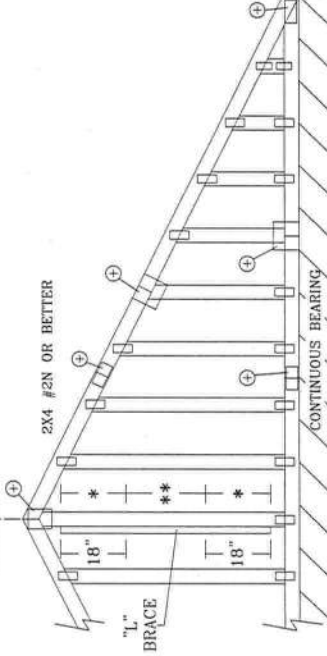
BRACING GROUP SPECIES AND GRADES:

GROUP A:		GROUP B:	
SPRUCE-PINE-FIR	HEM-FIR	SPRUCE-PINE-FIR	HEM-FIR
#1 / #2 STANDARD	#3 STUD	#1 / #2 STANDARD	#3 STUD
DOUGLAS FIR-LARCH	SOUTHERN PINE	DOUGLAS FIR-LARCH	SOUTHERN PINE
#3 STUD	#3 STUD	#1 / #2 STANDARD	#3 STUD
DOUGLAS FIR-LARCH	SOUTHERN PINE	DOUGLAS FIR-LARCH	SOUTHERN PINE
#3 STUD	#3 STUD	#1 / #2 STANDARD	#3 STUD

GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 10d NAILS. (0.126 x3" min)
- * FOR (1) "L" BRACE: SPACE NAILS AT 2' 0" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3' 0" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	

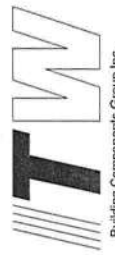


REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have properly attached structural panels. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Bracing of trusses. ITWBCG connector plates are made of 2010/1664 (W1/S/K) ASTM A563 Grade 40/46. All bracing shall be installed in accordance with the design shown on this drawing. A seal on this drawing cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

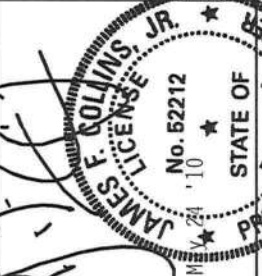
ITW - BCG: www.itwbcg.com; TPI: www.tpinet.com; WTC: www.sbcindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045

REF	ASCE7-05-GABI1015
DATE	1/1/09
DRWG	A11015050109

MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"



CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.
ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

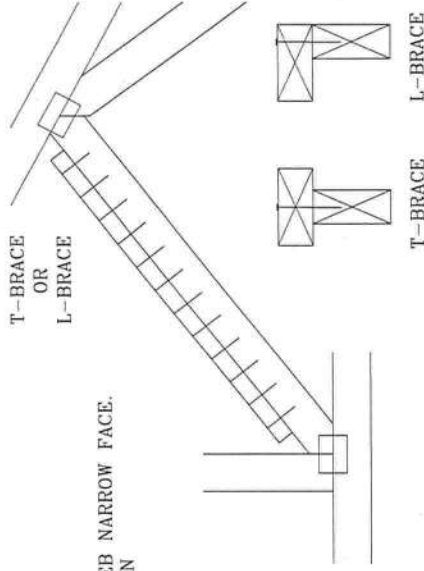
(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

T-BRACING

OR

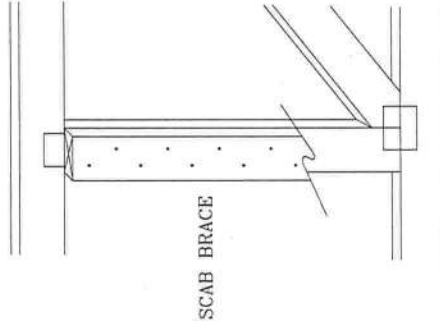
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3", MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3", MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

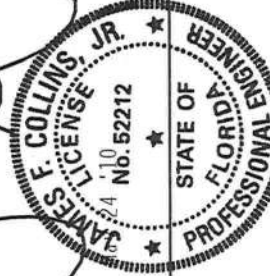
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 2010/10GA (N/H/S/K) ASTM A653 grade 37/40/80 (K/H/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint bracing. The responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCSI: www.itwbcg.com; TPI: www.tpinet.com; WTC: www.sbcindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045

REF	CLB	SUBST.
DATE	1/1/09	
DRWG	BRCLBSUB0109	
PSF	PSF	PSF
PSF	PSF	PSF
TOT. LD.		
DUR. FAC.		
SPACING		



GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 100 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

2X4 GABLE VERTICAL SPACING		BRACE		NO BRACES		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
SPACING	SPECIES	GRADE	#1 / #2	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
16" O.C.	SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 9"	10' 2"	10' 2"	10' 6"	13' 5"	13' 10"	14' 0"
	HF	#3	4' 1"	6' 9"	6' 9"	8' 7"	8' 7"	10' 2"	10' 2"	10' 2"	13' 5"	13' 5"	14' 0"
	STANDARD	#1	4' 1"	5' 9"	5' 9"	7' 8"	7' 8"	10' 2"	10' 2"	10' 2"	11' 11"	11' 11"	14' 0"
	STANDARD	#2	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 2"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
24" O.C.	SP	#1	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 2"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
	DFL	#3	4' 3"	6' 10"	6' 11"	8' 7"	9' 0"	10' 2"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
	STANDARD	#1	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 2"	10' 6"	12' 2"	14' 0"	14' 0"	14' 0"
	STANDARD	#2	4' 10"	8' 3"	8' 6"	9' 9"	10' 1"	11' 8"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	4' 8"	8' 3"	8' 3"	9' 9"	9' 9"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#3	4' 8"	8' 3"	8' 3"	9' 9"	9' 9"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#1	4' 8"	8' 3"	8' 3"	9' 9"	9' 9"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#2	5' 2"	8' 3"	8' 11"	9' 9"	10' 7"	11' 8"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SP	#1	4' 11"	8' 3"	8' 6"	9' 9"	10' 4"	11' 8"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#3	4' 10"	8' 3"	8' 5"	9' 9"	10' 4"	11' 8"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#1	4' 10"	8' 3"	8' 5"	9' 9"	10' 4"	11' 8"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#2	5' 3"	9' 1"	9' 4"	10' 9"	11' 1"	12' 10"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#3	5' 2"	9' 1"	9' 1"	10' 9"	10' 9"	12' 10"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	#1	5' 2"	9' 1"	9' 1"	10' 9"	10' 9"	12' 10"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#2	5' 9"	9' 1"	9' 10"	10' 9"	11' 7"	12' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#3	5' 8"	9' 1"	9' 10"	10' 9"	11' 4"	12' 10"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SP	#1	5' 5"	9' 1"	9' 7"	10' 9"	11' 4"	12' 10"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#3	5' 5"	9' 1"	9' 7"	10' 9"	11' 4"	12' 10"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	STANDARD	#1	5' 3"	8' 5"	8' 5"	10' 9"	11' 1"	12' 10"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	STANDARD	#2	5' 3"	8' 5"	8' 5"	10' 9"	11' 1"	12' 10"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

MAX GABLE VERTICAL LENGTH

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#3
STANDARD	STANDARD
STUD	STUD
STUD	STUD
STUD	STUD

DOUGLAS FIR-LARCH

#3	SOUTHERN PINE
STANDARD	STANDARD
STUD	STUD
STUD	STUD
STUD	STUD

GROUP B:

HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
#1	#2
STUD	STUD
STUD	STUD
STUD	STUD

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 60 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

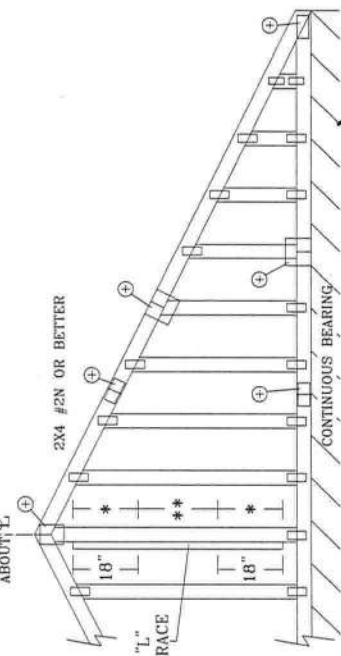
ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

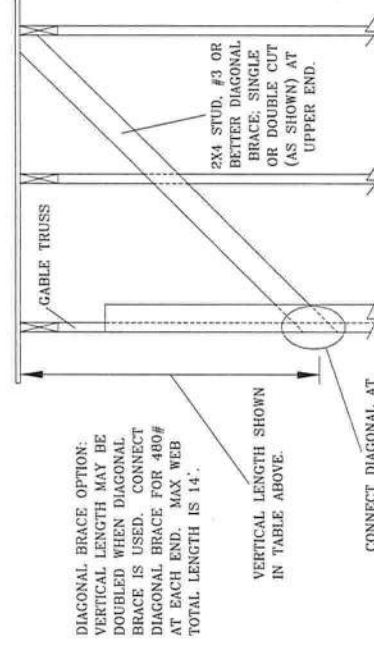
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 480# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCS (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing any work on trusses. Trusses are designed for use as shown. Trusses shall have a properly attached ridge board and ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCS sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group, Inc. (ITWBCG) shall be responsible for any deviation from this design, including but not limited to, the use of materials, fasteners, or other components not specified in the design. ITWBCG connector plates are made of 50/18/16GA (W/H/S/K) ASTM A653 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

ITW-BCC: www.itwbcg.com; TPI: www.tpiusa.com; WCA: www.wcaindustry.com; ICC: www.iccsafe.org

REF ASCE7-05-GABI0015
DATE 1/1/09
DRWG A10015050109

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"



Earth City, MO 63045

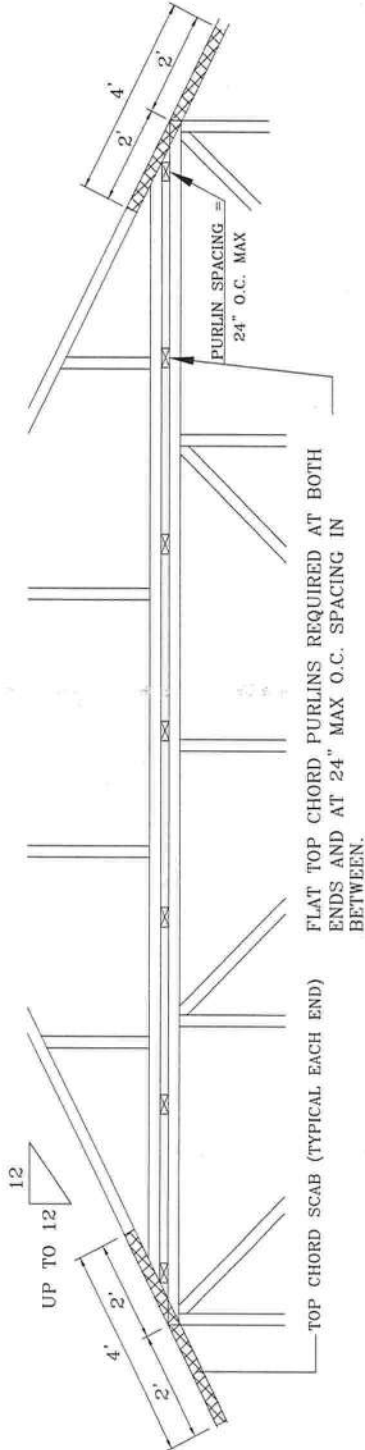
120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT. ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF (MIN). Kzt=1.0.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING OR ANY OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS, AND LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3") AT 4" O.C.

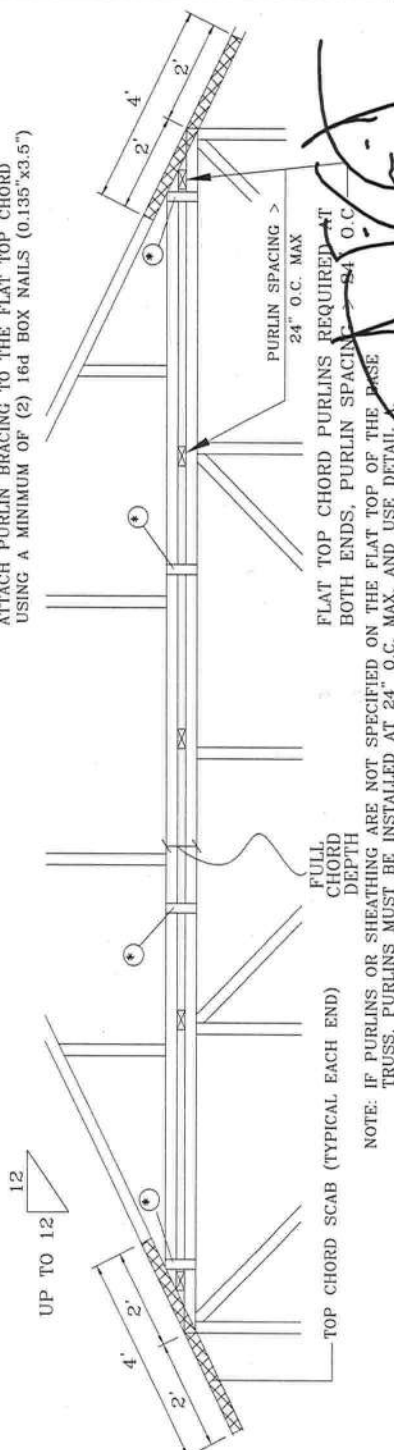
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

THE TOP CHORD #3 GRADE 2x4 SCAB MAY BE REPLACED WITH EITHER OF THE FOLLOWING: (1) 3x8 TRUSS PLATE ATTACHED WITH (8) 0.120"x1.375" NAILS, (4) INTO CAP TC & (4) INTO BASE TRUSS TC OR (1) 28PB WAVE PIGGYBACK PLATE PLATED TO THE PIGGYBACK TRUSS TC AND ATTACHED TO THE BASE TRUSS TC WITH (4) 0.120"x1.375" NAILS. NOTE: NAILING THRU HOLES OF WAVE PLATE IS ACCEPTABLE.

DETAIL B : PURLIN SPACING > 24" O.C.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING A MINIMUM OF (2) 16d BOX NAILS (0.135"x3.5")



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow ITW Building Components Group Inc. (ITWBCG) for safety practices prior to performing any work. Trusses shall be braced in accordance with the ITWBCG design and shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per ECSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Trusses shall be braced in accordance with the ITWBCG design and shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per ECSI sections B3 & B7. See this job's general notes page for more information. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSIT/TP1 1 Sec. 2. ITW-BGC: www.itwbcg.com; TP1: www.tpinat.com; WTCA: www.shcindustry.com; ICC: www.iccsafe.org



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* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRUSS USE 3x8 TRUSS PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRUSS PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRUSS PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

APA RATED GUSSET

8"x8"x7/16" (MIN) APA RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8" O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS

2x4 SPF #2, FULL CHORD DEPTH SCABS (EACH FACE). ATTACH @ 8" O.C. WITH (6) 10d BOX NAILS (0.128"x3") PER SCAB. (3) IN CAP BOTTOM CHORD AND (3) IN BASE TRUSS TOP CHORD. SCABS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

28PB WAVE PIGGYBACK PLATE

ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8" O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

REF PIGGYBACK

DATE 03/15/10

DRWG PB1200310

SPACING 24.0"

