DATE 04/19/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025730

APPLICANT JOHN NORRIS PHONE 758-3663

ADDRESS 351 NW CORWIN GLEN LAKE CITY FL 32055

OWNER IMAGE DEVELOPMENT PHONE 758-3663

ADDRESS 341 SW THISTLEWOOD LANE FT. WHITE FL 32038

CONTRACTOR JOHN NORRIS PHONE 758-3663

LOCATION OF PROPERTY 47S, TL ON 27, TL ON CR 18, TR ON GREENWOOD TERR, TL
THISTLEWOOD, LAST LOT ON RIGHT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 89400.00

HEATED FLOOR AREA 1788.00 TOTAL AREA 2630.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING FT. WHITE MAX. HEIGHT 17

Minimum Set Back Requirments: STREET-FRONT REAR SIDE

NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO.

PARCEL ID 34-6S-16-04056-120 SUBDIVISION THORNWOOD

LOT 20 BLOCK PHASE UNIT TOTAL ACRES

		RG0066597	John D Norris	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
FT WHITE	07-305	BK	JH	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FT. WHITE LETTER ON FILE, NOC ON FILE

Check # or Cash 3989

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 450.00 CERTIFICATION FEE $ 13.15 SURCHARGE FEE $ 13.15

MISC. FEES $ 0.00 ZONING CERT. FEE $ FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ CULVERT FEE $ **TOTAL FEE** 476.30

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Kirkland Title
723 E Wade St
Trenton FL 32693

Inst:2007007274 Date:03/30/2007 Time:10:06
DC,P.DeWitt Cason,Columbia County B:1116P:424

NOTICE OF COMMENCEMENT

To Whom It May Concern:

The undersigned hereby informs you that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is stated in this NOTICE OF COMMENCEMENT.

Description of Property: Lot 20 of THORNWOOD, a subdivision, according to the Plat thereof as recorded in Plat Book 7, Page(s) 202, of the Public Records of Columbia County, Florida.

General Description of Improvements: CONSTRUCTION

Owner and Address: Image Development Group, LLC (Richard C Parker)
341 SW Thistlewood Lane
Fort White, FL 32038

Owner's Interest in Site of the Improvement: Fee Simple

Contractor and Address: Image Development Group, LLC
20074 NW 258th Drive
High Springs, FL 32643

John Norris Corporation, Inc.
Lake City, FL 32055

Surety (if any): NA

Address: _____ Amount of Bond $_____

Name and address of person within the State of Florida designated by owner upon whom notices or other documents may be served:

In addition to himself, owner designated the following person to receive a copy of Lienor's Notice as provided in Section 713.06 (2) (b) Florida Statutes:

Name and Address: Ameris Bank
530 E Wade Street
Trenton, FL 32693

Richard C Parker, MGRM

State of Florida
County of Alachua

I hereby certify that on this day, before me, an officer duly authorized to administer oaths and take acknowledgements, personally appeared Richard C Parker known to me to be the person(s) described in and who executed the foregoing instrument, who acknowledged before me that they executed the same.

Witness my hand and official seal in the County and State aforesaid this 30th day of March 2007.

DEVON WEAVER
MY COMMISSION # DD 380533
EXPIRES: December 19, 2008
Bonded Thru Notary Public Underwriters

Notary Public

Columbia County Building Permit Application

For Office Use Only Application # 0704-26 Date Received 4/12/07 By CP Permit # 25730

Application Approved by - Zoning Official _____ Date _____ Plans Examiner OK JTH Date 4-17-07

Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____

Comments See PAGE 1 OF PLAN FOR SITE PLAN TOWN OF FORT WHITE

NOC need EH □ Deed or PA □ Site Plan □ State Road Info □ Parent Parcel # □ Development Permit

Name Authorized Person Signing Permit John Norris Fax 758-9530 Phone 758-3663

Address 351 NW Corwin Glen L.C. 32055

Owners Name Image Development Phone _____

911 Address 341 SW Thistlewood Lane Ft White Fl. 32038

Contractors Name John Norris Phone _____

Address 351 NW Corwin Gln., Lake City, FL 32055

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Freeman

Mortgage Lenders Name & Address _____

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 04056-120 34-6S-16 Estimated Cost of Construction $ 125,000

Subdivision Name Thornwood Subdivision Lot 20 Block ____ Unit ____ Phase ____

Driving Directions ~~Single family dwellings~~

47 to Ft. White. Left on 27 left on 18 1/4 miles on left side IR on Greenwood Terr, TL Thistlewood, last lot on right.

Type of Construction Residential Number of Existing Dwellings on Property 1

Total Acreage 1.02ac Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 37' Side 50' Side 50' Rear _____

Total Building Height 17' Number of Stories 1 Heated Floor Area 1788 TOTAL 2630 Roof Pitch 6'12"

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter

Contractor Signature

Contractors License Number RG0066597

Competency Card Number _____

STATE OF FLORIDA
COUNTY OF COLUMBIA

GALE TEDDER
MY COMMISSION # DD 439586
EXPIRES: June 28, 2008
Bonded Thru Notary Public Und.

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 17th day of April 20 07.

Personally known ____ or Produced Identification ____

Notary Signature

(Revised Sept. 2006)

TOWN OF FORT WHITE

Home of the Ichetucknee River

Post Office Box 129 Fort White, FL 32038
Email: townofftwhite@alltel.net Web: townoffortwhitefl.com
Tel: (386) 497-2321/(386) 497-3345 Fax: (386) 497-4946
Office Hours: Monday through Friday 9:00 a.m. to 1:00 p.m

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

OWNER'S NAME: IMAGE DEVELOPMENT GROUP

ADDRESS: P.O. Box 305 Newberry, FL 32669

PROPERTY DESCRIPTION w/ parcel number: Lot #20 parcel # 4056-120
1.02 ac Thornwood Subdivision

DEVELOPMENT: Single Family Dwelling

You are hereby authorized to issue the appropriate permits

04-02-07
DATE

Janice Revels (Ka)
LDR ADMINISTRATOR
Town of Fort White

Mayor	*District 1*	*District 2*	*District 3*	*District 4*
Truett George	Donald Cook	Henry Maini	Warren Barnes	Demetric Jackson
497-4741	497-1086	497-2992	497-3112	497-2078

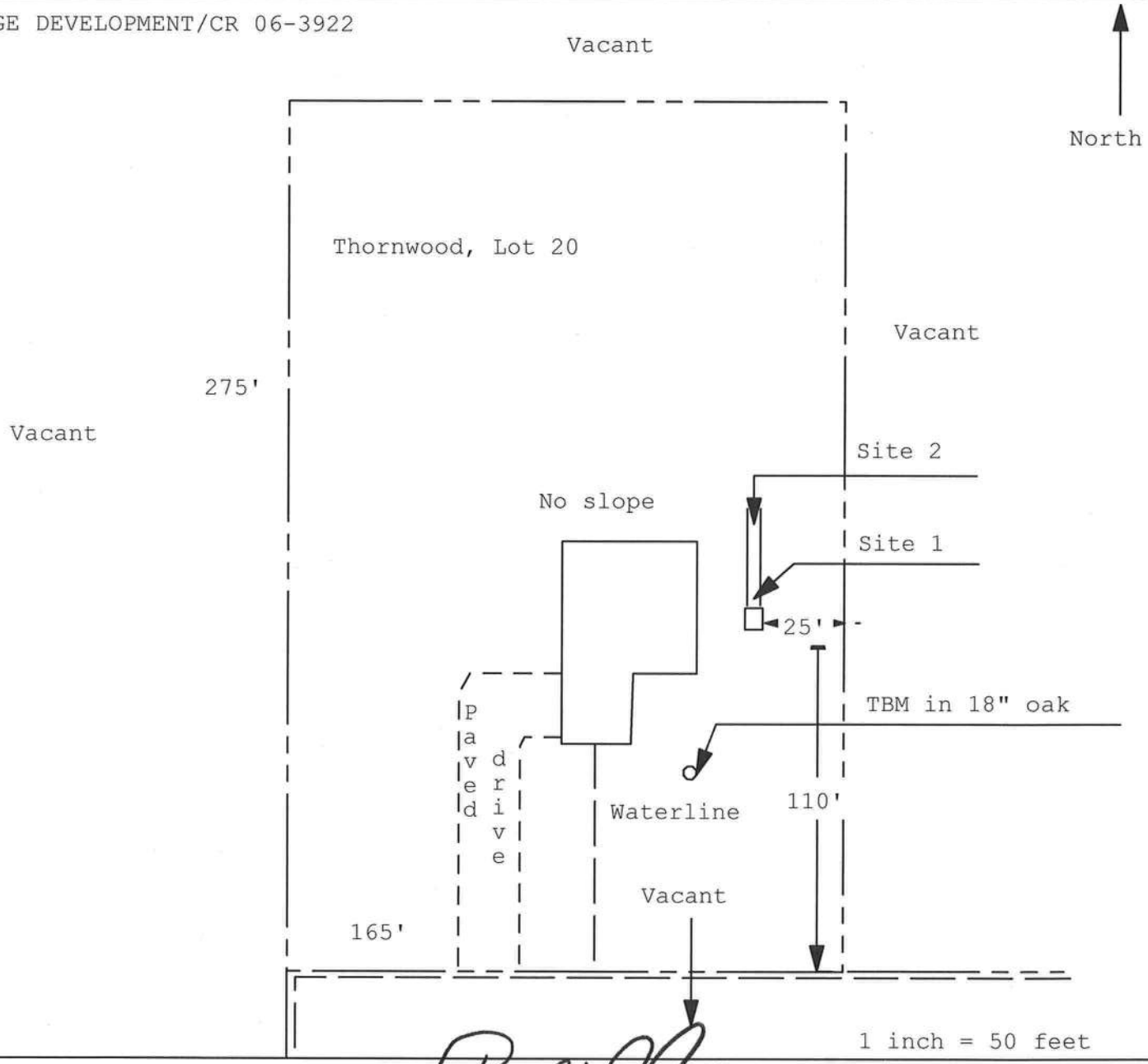
```
@ CAM112M01     S  CamaUSA Appraisal System                    Columbia County
 4/12/2007  8:57  Legal Description Maintenance      40500  Land   001
Year T Property                              Sel              AG     000
2007 R 34-6S-16-04056-120 . . . . . . . . . . . . . . . . .   Bldg   000
                                                             Xfea   000
       IMAGE DEVELOPMENT GROUP LLC                   40500  TOTAL       B*

  1  LOT 20 THORNWOOD S/D . . . . . WD 1030-2804 . . . . . . . . . . . .  2
  3  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . .  4
  5  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . .  6
  7  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . .  8
  9  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 10
 11  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 12
 13  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 14
 15  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 16
 17  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 18
 19  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 20
 21  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 22
 23  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 24
 25  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 26
 27  . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 28
                                          Mnt 12/03/2004 LARRY
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
```

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 07-305

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

IMAGE DEVELOPMENT/CR 06-3922



Site Plan Submitted By Paul Lloyd **Date** 3/19/07

Plan Approved ✓ **Not Approved** ____ **Date** 4/17/07

By M. A. Jr. Columbia **CPHU**

Notes: _____

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Thornwood Lot 20**	Builder:	John Norris
Address:	**Lot: 20, Sub: Thronwood, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL 32055-**	Permit Number:	25730
Owner:	**Norris**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1788 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	209.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 232.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1856.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1966.8 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 64.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 36.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 36.0 kBtu/hr	
	HSPF: 8.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

Glass/Floor Area: 0.12

Total as-built points: 22532

Total base points: 28016

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 3/12/07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1788.0	20.04	6449.7	Double, Clear	E	1.5	5.0	32.0	42.06	0.87	1177.2	
				Double, Clear	E	1.5	6.0	50.0	42.06	0.91	1919.6	
				Double, Clear	E	1.5	6.0	15.0	42.06	0.91	575.9	
				Double, Clear	W	1.5	7.0	48.0	38.52	0.94	1736.2	
				Double, Clear	W	1.5	6.0	30.0	38.52	0.91	1055.6	
				Double, Clear	W	1.5	6.0	25.0	38.52	0.91	879.7	
				Double, Clear	N	1.5	4.0	9.0	19.20	0.88	152.3	
				As-Built Total:				**209.0**			**7496.4**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1856.0	1.50		2784.0	
Exterior	1856.0	1.70	3155.2									
Base Total:	**1856.0**		**3155.2**	**As-Built Total:**				**1856.0**			**2784.0**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Wood				40.8	6.10		248.9	
Exterior	40.8	6.10	248.9									
Base Total:	**40.8**		**248.9**	**As-Built Total:**				**40.8**			**248.9**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1788.0	1.73	3093.2	Under Attic			30.0	1966.8	1.73 X 1.00		3402.6	
Base Total:	**1788.0**		**3093.2**	**As-Built Total:**				**1966.8**			**3402.6**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	232.0(p)	-37.0	-8584.0	Slab-On-Grade Edge Insulation			0.0	232.0(p	-41.20		-9558.4	
Raised	0.0	0.00	0.0									
Base Total:			**-8584.0**	**As-Built Total:**				**232.0**			**-9558.4**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1788.0	10.21	18255.5					1788.0	10.21		18255.5	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**22618.5**	**Summer As-Built Points:**					**22629.0**
Total Summer Points	System Multiplier	Cooling Points	Total Component	Cap Ratio	Duct Multiplier (DM x DSM x AHU)	System Multiplier	Credit Multiplier	Cooling Points
22618.5	**0.4266**	**9649.0**	22629.0 **22629.0**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.857 **0.857**	5795.1 **5795.1**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1788.0	12.74	4100.2	Double, Clear	E	1.5	5.0	32.0	18.79	1.05	631.4
				Double, Clear	E	1.5	6.0	50.0	18.79	1.04	973.0
				Double, Clear	E	1.5	6.0	15.0	18.79	1.04	291.9
				Double, Clear	W	1.5	7.0	48.0	20.73	1.02	1011.3
				Double, Clear	W	1.5	6.0	30.0	20.73	1.02	636.4
				Double, Clear	W	1.5	6.0	25.0	20.73	1.02	530.4
				Double, Clear	N	1.5	4.0	9.0	24.58	1.01	222.5
				As-Built Total:				**209.0**			**4296.9**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1856.0	3.40		6310.4
Exterior	1856.0	3.70	6867.2								
Base Total:	**1856.0**		**6867.2**	**As-Built Total:**				**1856.0**			**6310.4**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Wood				40.8	12.30		501.8
Exterior	40.8	12.30	501.8								
Base Total:	**40.8**		**501.8**	**As-Built Total:**				**40.8**			**501.8**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1788.0	2.05	3665.4	Under Attic			30.0	1966.8	2.05 X 1.00		4031.9
Base Total:	**1788.0**		**3665.4**	**As-Built Total:**				**1966.8**			**4031.9**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	232.0(p)	8.9	2064.8	Slab-On-Grade Edge Insulation			0.0	232.0(p	18.80		4361.6
Raised	0.0	0.00	0.0								
Base Total:			**2064.8**	**As-Built Total:**				**232.0**			**4361.6**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1788.0	-0.59	-1054.9					1788.0	-0.59		-1054.9

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**16144.6**	**Winter As-Built Points:**					**18447.8**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
16144.6	**0.6274**	**10129.1**	18447.8 **18447.8**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.426 **0.426**	0.950 **0.950**	8681.8 **8681.8**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-**	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2746.00	8238.0	50.0	0.90	3		1.00	2684.98	1.00	8054.9
				As-Built Total:							**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
9649	**10129**	**8238**	**28016**	**5795**	**8682**	**8055**	**22532**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-**	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.9

The higher the score, the more efficient the home.

Norris, Lot: 20, Sub: Thronwood, Plat: , Lake City, FL, 32055-

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1788 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear - single pane	0.0 ft²	209.0 ft²	
b. Clear - double pane	0.0 ft²	0.0 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	0.0 ft²	
d. Tint/other SHGC - double pane			
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 232.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1856.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1966.8 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 64.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit		Cap: 36.0 kBtu/hr
		SEER: 13.00
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump		Cap: 36.0 kBtu/hr
		HSPF: 8.00
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance		Cap: 50.0 gallons
		EF: 0.90
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		MZ-C, PT, CF,

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

Residential System Sizing Calculation

Summary

Norris

Lake City, FL 32055-

Project Title:
Thornwood Lot 20

Code Only
Professional Version
Climate: North

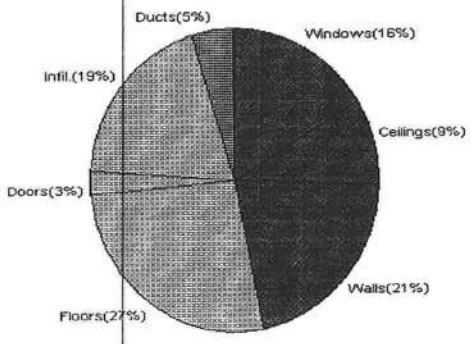
3/12/2007

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	98	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	23	F
Total heating load calculation	**27291**	**Btuh**	**Total cooling load calculation**	**25480**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	131.9	36000	Sensible (SHR = 0.5)	87.9	18000
Heat Pump + Auxiliary(0.0kW)	131.9	36000	Latent	359.7	18000
			Total (Electric Heat Pump)	141.3	36000

WINTER CALCULATIONS

Winter Heating Load (for 1788 sqft)

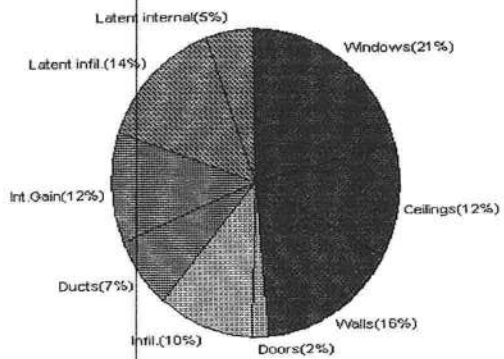
Load component			Load	
Window total	209	sqft	4494	Btuh
Wall total	1856	sqft	5754	Btuh
Door total	41	sqft	732	Btuh
Ceiling total	1967	sqft	2557	Btuh
Floor total	232	ft	7331	Btuh
Infiltration	119	cfm	5124	Btuh
Subtotal			**25991**	**Btuh**
Duct loss			1300	Btuh
TOTAL HEAT LOSS			**27291**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1788 sqft)

Load component			Load	
Window total	209	sqft	5429	Btuh
Wall total	1856	sqft	3972	Btuh
Door total	41	sqft	501	Btuh
Ceiling total	1967	sqft	3068	Btuh
Floor total			0	Btuh
Infiltration	105	cfm	2644	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**18614**	**Btuh**
Duct gain			1861	Btuh
Total sensible gain			**20476**	**Btuh**
Latent gain(infiltration)			3624	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**5004**	**Btuh**
TOTAL HEAT GAIN			**25480**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: _____

DATE: 3/12/07

System Sizing Calculations - Winter

Residential Load - Component Details

Norris

Project Title:
Thornwood Lot 20

Code Only
Professional Version
Climate: North

Lake City, FL 32055-

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

3/12/2007

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	32.0	21.5	688 Btuh
2	2, Clear, Wood, DEF	N	50.0	21.5	1075 Btuh
3	2, Clear, Wood, DEF	N	15.0	21.5	322 Btuh
4	2, Clear, Wood, DEF	S	48.0	21.5	1032 Btuh
5	2, Clear, Wood, DEF	S	30.0	21.5	645 Btuh
6	2, Clear, Wood, DEF	S	25.0	21.5	538 Btuh
7	2, Clear, Wood, DEF	W	9.0	21.5	194 Btuh
	Window Total		209		4494 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1856	3.1	5754 Btuh
	Wall Total		1856		5754 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		41	17.9	732 Btuh
	Door Total		41		732Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1967	1.3	2557 Btuh
	Ceiling Total		1967		2557Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	232.0 ft(p)	31.6	7331 Btuh
	Floor Total		232		7331 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	17880(sqft)	119	5124 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			119	5124 Btuh

Totals for Heating	**Subtotal**	**25991 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1300 Btuh**
	Total Btuh Loss	**27291 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Norris

Lake City, FL 32055-

Project Title:
Thornwood Lot 20

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Summer Temperature Difference: 23.0 F 3/12/2007

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N	N	1.5	5	32.0	0.0	32.0	24	24	768 Btuh
2	2, Clear, DEF, N, N	N	1.5	6	50.0	0.0	50.0	24	24	1200 Btuh
3	2, Clear, DEF, N, N	N	1.5	6	15.0	0.0	15.0	24	24	360 Btuh
4	2, Clear, DEF, N, N	S	1.5	7	48.0	48.0	0.0	24	39	1152 Btuh
5	2, Clear, DEF, N, N	S	1.5	6	30.0	30.0	0.0	24	39	720 Btuh
6	2, Clear, DEF, N, N	S	1.5	6	25.0	25.0	0.0	24	39	600 Btuh
7	2, Clear, DEF, N, N	W	1.5	4	9.0	0.7	8.3	24	74	629 Btuh
	Window Total				209					5429 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Exterior	13.0	1856.0	2.1	3972 Btuh
	Wall Total		1856.0		3972 Btuh

Doors	Type	Area	HTM	Load
1	Wood - Exter	40.8	12.3	501 Btuh
	Door Total	40.8		501 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1966.8	1.6	3068 Btuh
	Ceiling Total		1966.8		3068 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	232.0 ft(p)	0.0	0 Btuh
	Floor Total		232.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	17880	104.5	2644 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			105	2644 Btuh

Internal gain	Occupants	Btuh/occupant			Appliance	Load
	6	X	300	+	1200	3000 Btuh

Totals for Cooling		
	Subtotal	**18614 Btuh**
	Duct gain(using duct multiplier of 0.10)	**1861 Btuh**
	Total sensible gain	**20476 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**3624 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**25480 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise.

#25730

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109576 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: John Norris Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 341 S.W. Thistlewood Terr. Ft. White, FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 12 Inside 24 Type of Fill Dirt

Section 4: Treatment Information

Date(s) of Treatment(s) 5-7-07
Brand Name of Product(s) Used Biflex
EPA Registration No. 53883-184
Approximate Final Mix Solution % .06
Approximate Size of Treatment Area: Sq. ft. 2630 Linear ft. 254 Linear ft. of Masonry Voids 254
Approximate Total Gallons of Solution Applied 577
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No Upon completion

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) Steve Brannon Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Date 5-7-07

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Project Information for:	L232942		
Builder:	JOHN NORRIS	Date:	3/23/2007
Lot:	LOT 20	Start Number:	**1178**
Subdivision:	THORNWOOD	SEI Ref:	L232942
County or City:	COLUMBIA COUNTY		
Truss Page Count:	38		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **131**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

MAR 23 2007

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0323071178	3/23/2007				
2	CJ3	0323071179	3/23/2007				
3	CJ3T	0323071180	3/23/2007				
4	CJ5	0323071181	3/23/2007				
5	CJ5T	0323071182	3/23/2007				
6	EJ7	0323071183	3/23/2007				
7	EJ7A	0323071184	3/23/2007				
8	EJ7T	0323071185	3/23/2007				
9	HJ9	0323071186	3/23/2007				
10	HJ9T	0323071187	3/23/2007				
11	T01	0323071188	3/23/2007				
12	T01A	0323071189	3/23/2007				
13	T01G	0323071190	3/23/2007				
14	T02	0323071191	3/23/2007				
15	T03	0323071192	3/23/2007				
16	T04	0323071193	3/23/2007				
17	T04A	0323071194	3/23/2007				
18	T04G	0323071195	3/23/2007				
19	T05	0323071196	3/23/2007				
20	T06	0323071197	3/23/2007				
21	T07	0323071198	3/23/2007				
22	T08	0323071199	3/23/2007				
23	T09	0323071200	3/23/2007				
24	T10	0323071201	3/23/2007				
25	T11	0323071202	3/23/2007				
26	T12	0323071203	3/23/2007				
27	T13	0323071204	3/23/2007				
28	T14	0323071205	3/23/2007				
29	T15	0323071206	3/23/2007				
30	T17	0323071207	3/23/2007				
31	T18	0323071208	3/23/2007				
32	T19	0323071209	3/23/2007				
33	T20	0323071210	3/23/2007				
34	T21	0323071211	3/23/2007				
35	T22	0323071212	3/23/2007				
36	T23	0323071213	3/23/2007				
37	T24	0323071214	3/23/2007				
38	T25	0323071215	3/23/2007				

Structural Engineering and Inspections. Inc. 16105 N. Florida Ave. Ste B Lutz, FL 33549 Thomas Miller. PE 56877, EB 9196

LATERAL TOE-NAIL DETAIL

ST-TOENAIL

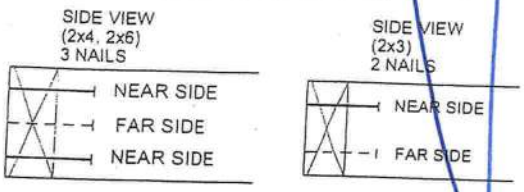
NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 30 DEGREES WITH THE MEMBER AND STARTED 1/3 THE LENGTH OF THE NAIL FROM THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE BOTTOM CHORD SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

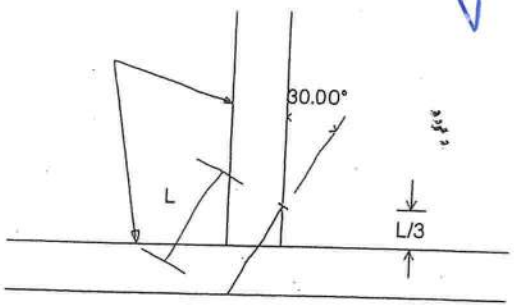
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP
3.5" LONG	.131	83.3
	.135	89.6
	.162	118.3
3.25" LONG	.128	80.5
	.131	83.3
	.148	102.1
3.0" LONG	.120	70.5
	.128	80.5
	.131	83.3
	.148	102.1

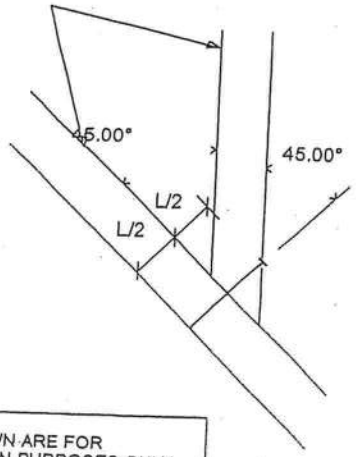
VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

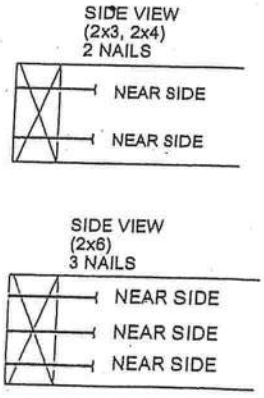
SQUARE CUT





45 DEGREE ANGLE BEVEL CUT





VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

This detail may only be applied to Pre-engineered truss drawings signed and sealed by Structural Engineering and Inspections Inc.

MAR 23 2007

Public Services
Search for a Licensee
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AB&T Delinquent Invoice & Activity List Search
User Services
Renew a License
Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary
Online Help

Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID (Primary Name)** **INDIVIDUAL (DBA Name)**
Main Address:	**351 NW CORWIN GLN** **LAKE CITY Florida 32055**
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**WOODGLEN DRIVE** **LAKE CITY FL 32055**
County:	**COLUMBIA**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current,Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2007**

Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
No Qualified Business License Required	**02/20/2004**

View Related License Information

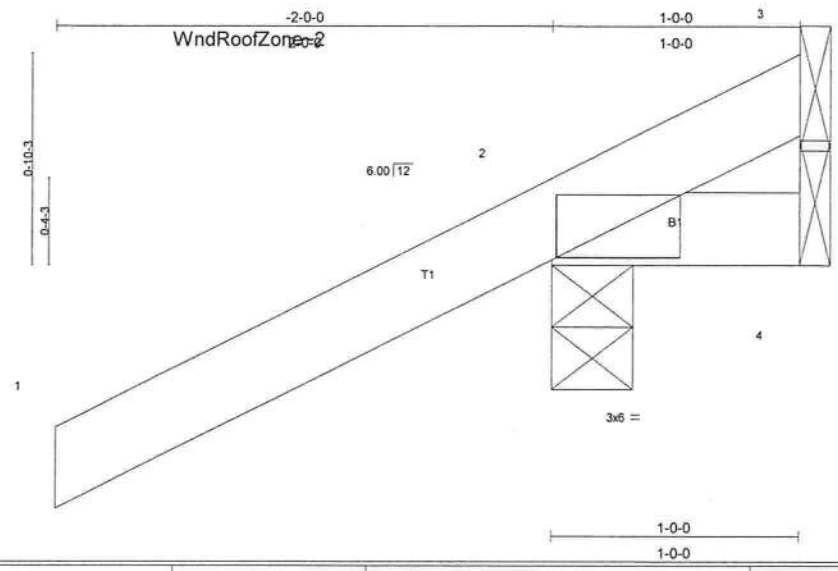
View License Complaint

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
SEALED: Dwg. #0323071177
ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071178

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	CJ1	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:30 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-287(load case 5), 4=-9(load case 3), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

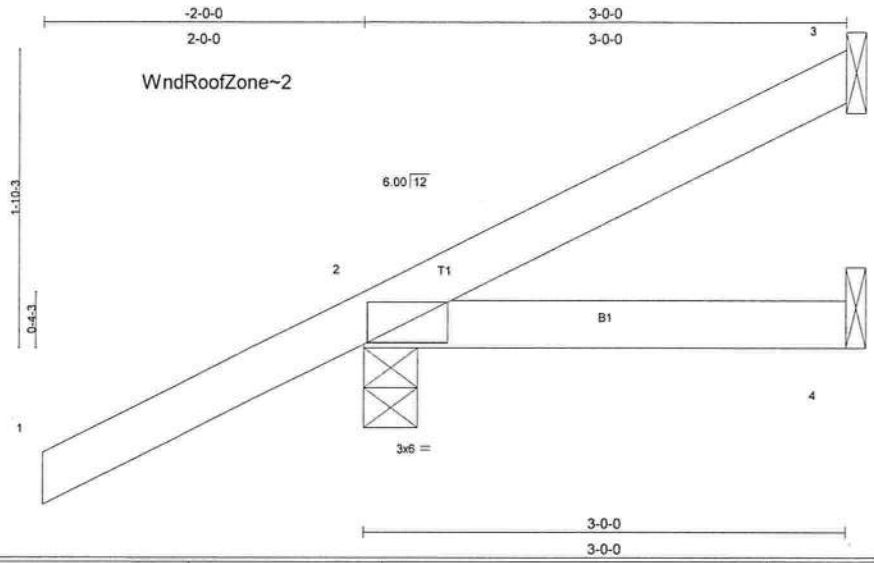
LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071179

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:30 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-240(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

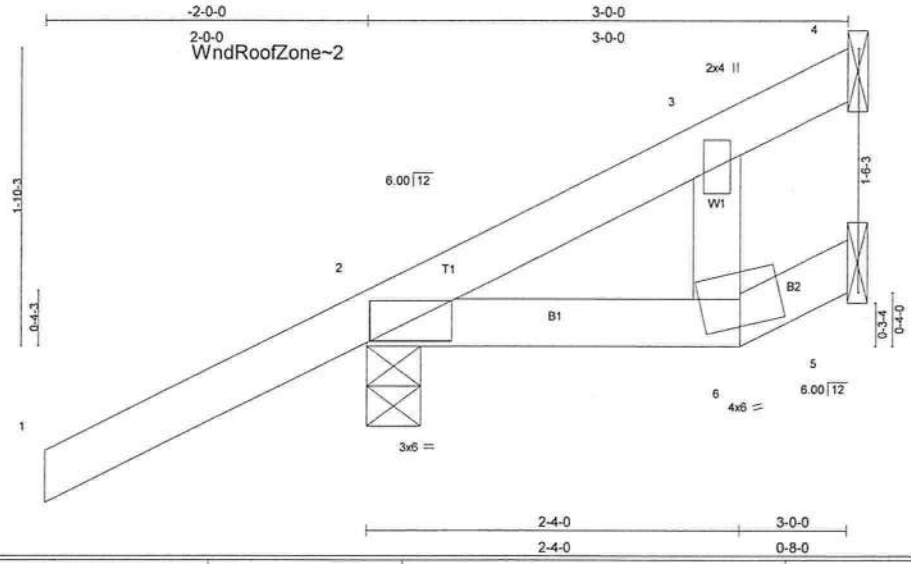
LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071180

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	CJ3T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:31 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=62/Mechanical, 2=279/0-4-0, 5=9/Mechanical
Max Horz 2=132(load case 5)
Max Uplift4=-21(load case 6), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-59/3, 3-4=-16/26
BOT CHORD 2-6=-5/0, 5-6=-3/3
WEBS 3-6=0/42

JOINT STRESS INDEX
2 = 0.13, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071181

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	CJ5	MONO TRUSS	10	1	Job Reference (optional)

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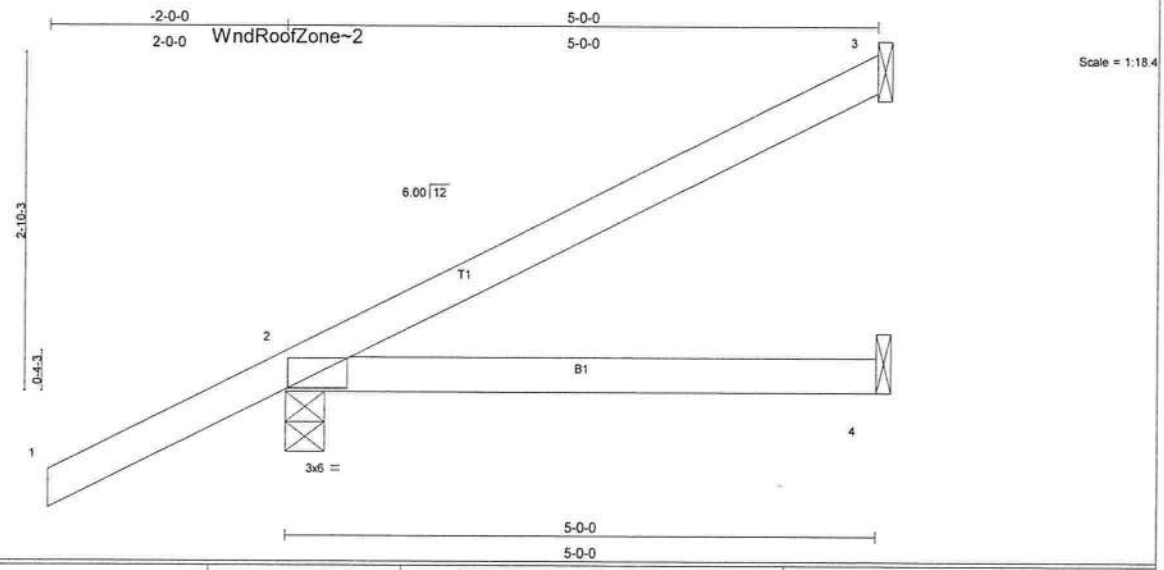

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-86(load case 5), 2=-261(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071182

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	CJ5T	SPECIAL	2	1	Job Reference (optional)

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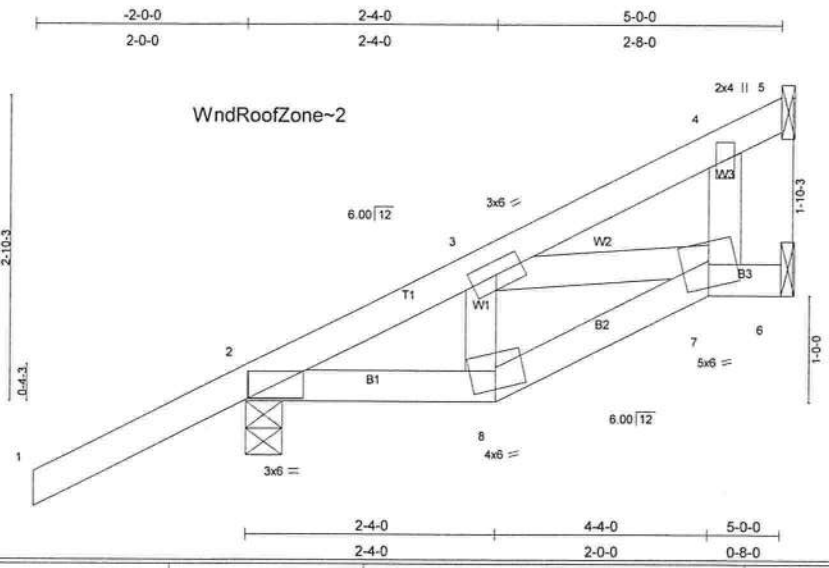


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=165/Mechanical, 2=344/0-4-0, 6=9/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-74(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-227/3, 3-4=-60/0, 4-5=-62/73
BOT CHORD 2-8=-92/151, 7-8=-97/178, 6-7=-0/0
WEBS 3-8=-12/55, 4-7=-39/122, 3-7=-136/80

JOINT STRESS INDEX
2 = 0.16, 3 = 0.03, 4 = 0.08, 7 = 0.06 and 8 = 0.05

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 5 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071183

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	EJ7	MONO TRUSS	24	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:33 2007 Page 1

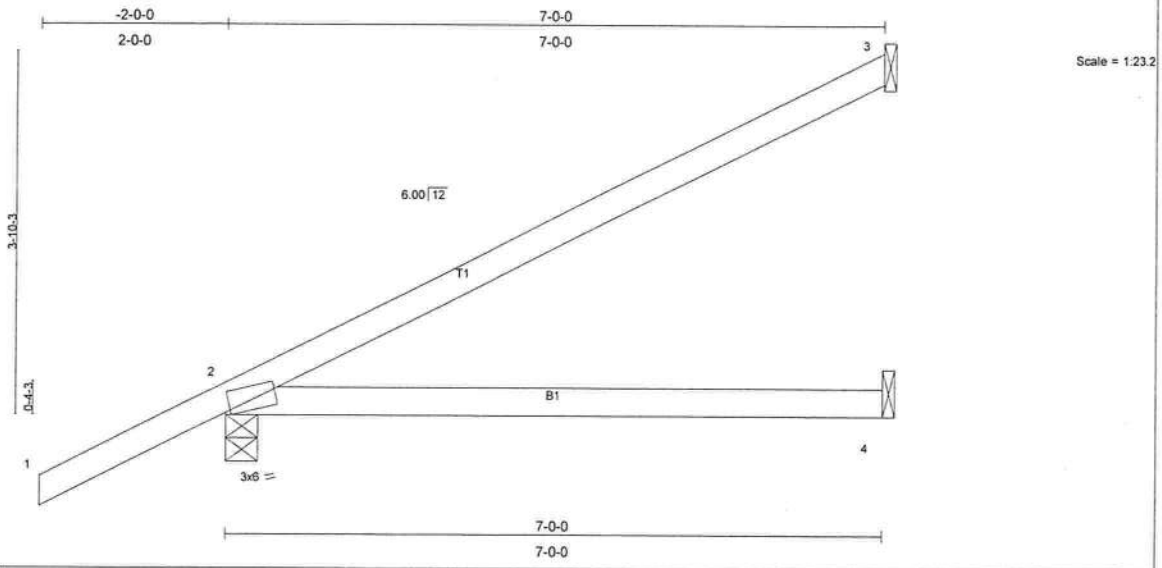


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.26	2-4	>309	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.21	2-4	>379	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-296(load case 5), 4=-67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.76

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 296 lb uplift at joint 2 and 67 lb uplift at joint 4.

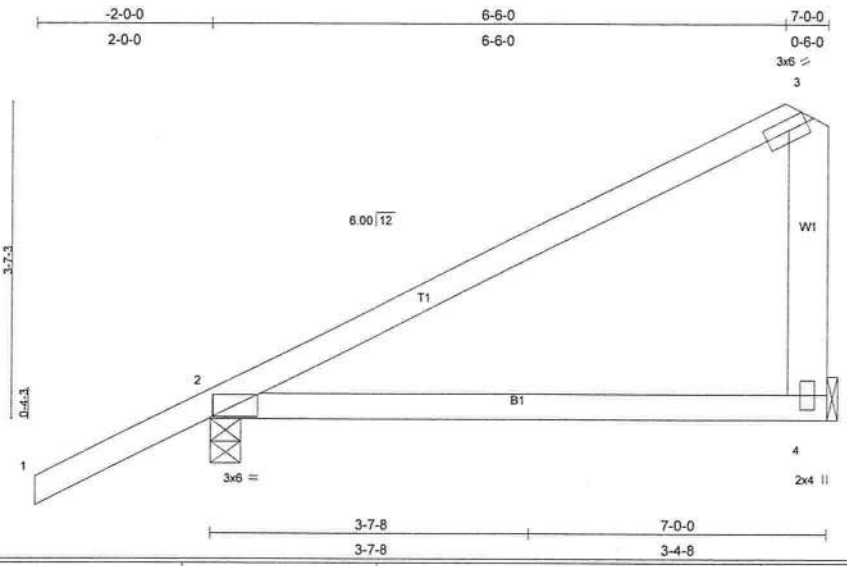
LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071184

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	EJ7A	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:33 2007 Page 1



Scale = 1:24.8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.11	2-4	>737	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.18	2-4	>440	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=414/0-4-0, 4=258/Mechanical, 4=258/Mechanical
Max Horz 2=220(load case 5)
Max Uplift2=-210(load case 5), 4=-121(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-92/55
BOT CHORD 2-4=-0/0
WEBS 3-4=-157/129

JOINT STRESS INDEX
2 = 0.46, 3 = 0.06 and 4 = 0.07

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 210 lb uplift at joint 2 and 121 lb uplift at joint 4.

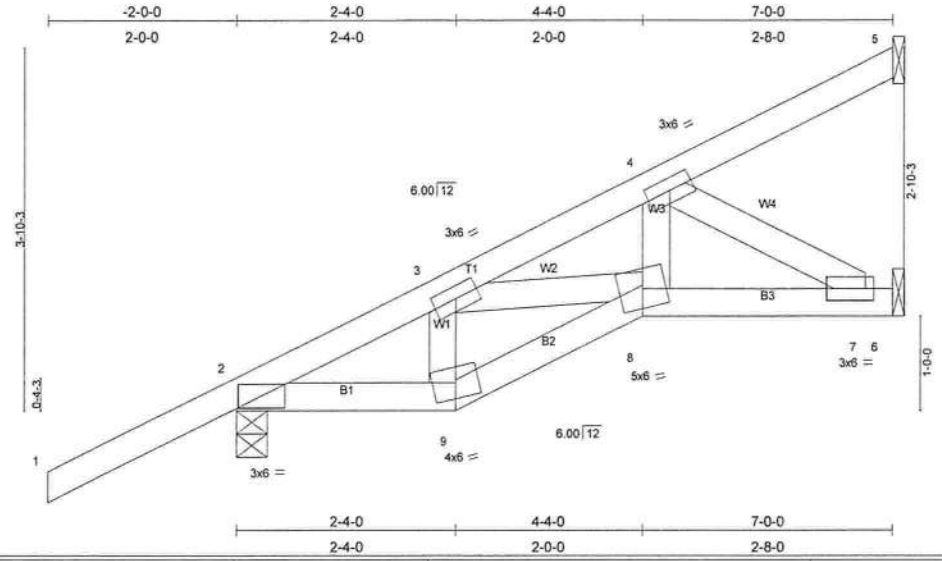
LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071185

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	EJ7T	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:34 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.01	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=56/Mechanical, 2=420/0-4-0, 6=209/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-48(load case 5), 2=-211(load case 5), 6=-77(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-353/1, 3-4=-429/92, 4-5=-34/19
BOT CHORD 2-9=-85/272, 8-9=-90/307, 7-8=-163/341, 6-7=0/0
WEBS 3-9=-114/102, 3-8=-115/111, 4-8=-35/232, 4-7=-393/188

JOINT STRESS INDEX
2 = 0.57, 3 = 0.06, 4 = 0.17, 7 = 0.11, 8 = 0.16 and 9 = 0.11

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 5, 211 lb uplift at joint 2 and 77 lb uplift at joint 6.

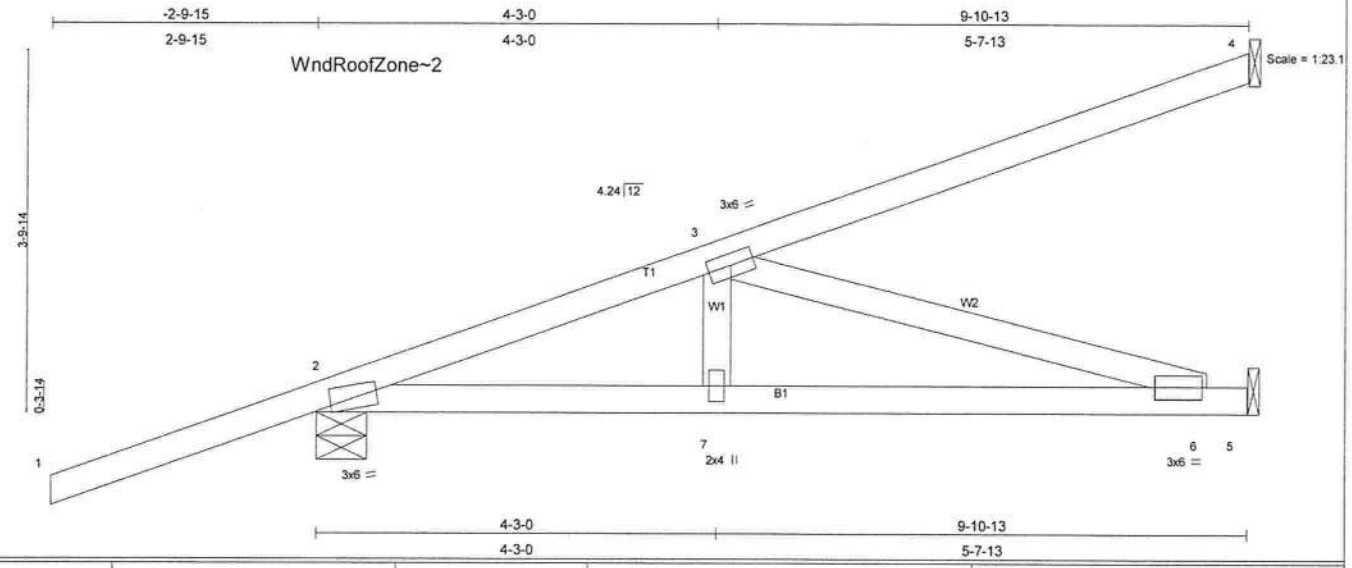
LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071186

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	HJ9	MONO TRUSS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:35 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-7, 5=372/Mechanical
Max Horz 2=270(load case 2)
Max Uplift4=-233(load case 2), 2=-404(load case 2), 5=-180(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-877/359, 3-4=-105/66
BOT CHORD 2-7=-530/810, 6-7=-530/810, 5-6=0/0
WEBS 3-7=-94/187, 3-6=-844/553

JOINT STRESS INDEX
2 = 0.78, 3 = 0.23, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071187

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	HJ9T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:36 2007 Page 1

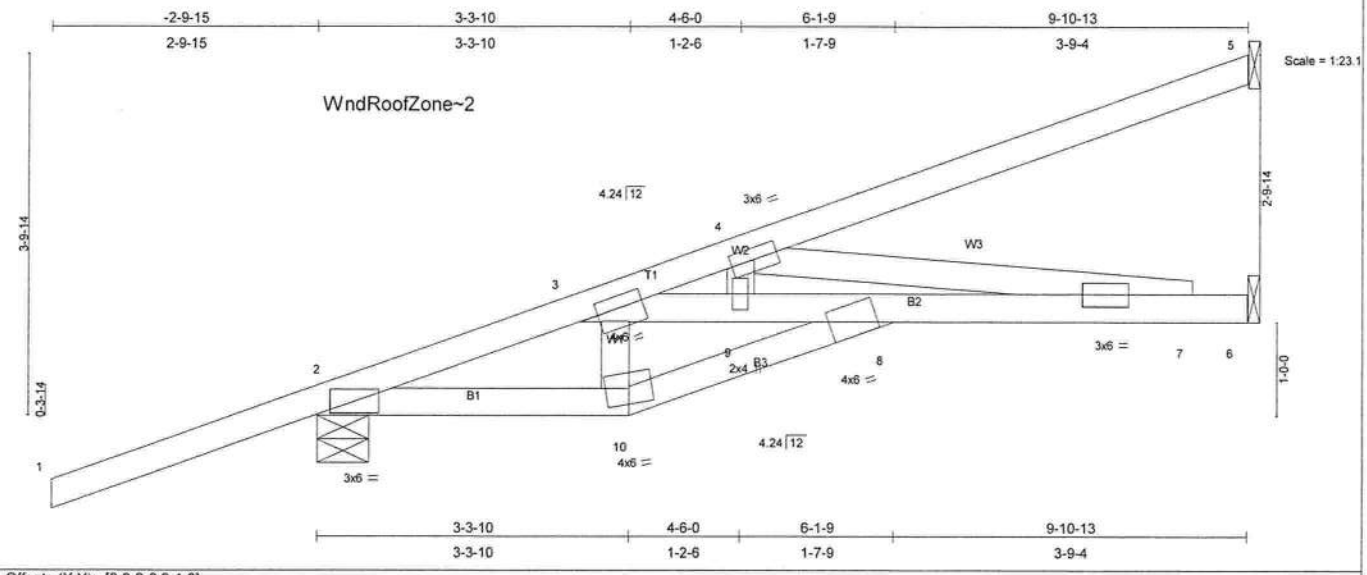


Plate Offsets (X,Y): [3:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.19	7-8	>603	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.31	7-8	>366	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.78	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-3-6 oc bracing: 7-8.

REACTIONS (lb/size) 5=267/Mechanical, 2=539/0-6-6, 6=381/Mechanical
Max Horz 2=268(load case 2)
Max Uplift5=-219(load case 2), 2=-283(load case 2), 6=-73(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-572/10, 3-4=-1803/534, 4-5=-100/66
BOT CHORD 2-10=-156/482, 8-10=-130/463, 3-9=-646/1296, 8-9=-646/1296, 7-8=-727/1722, 6-7=0/0
WEBS 3-10=-118/93, 4-7=-1736/733, 4-9=0/421

JOINT STRESS INDEX
2 = 0.63, 3 = 0.58, 4 = 0.49, 7 = 0.50, 8 = 0.10, 9 = 0.31 and 10 = 0.16

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 5, 283 lb uplift at joint 2 and 73 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

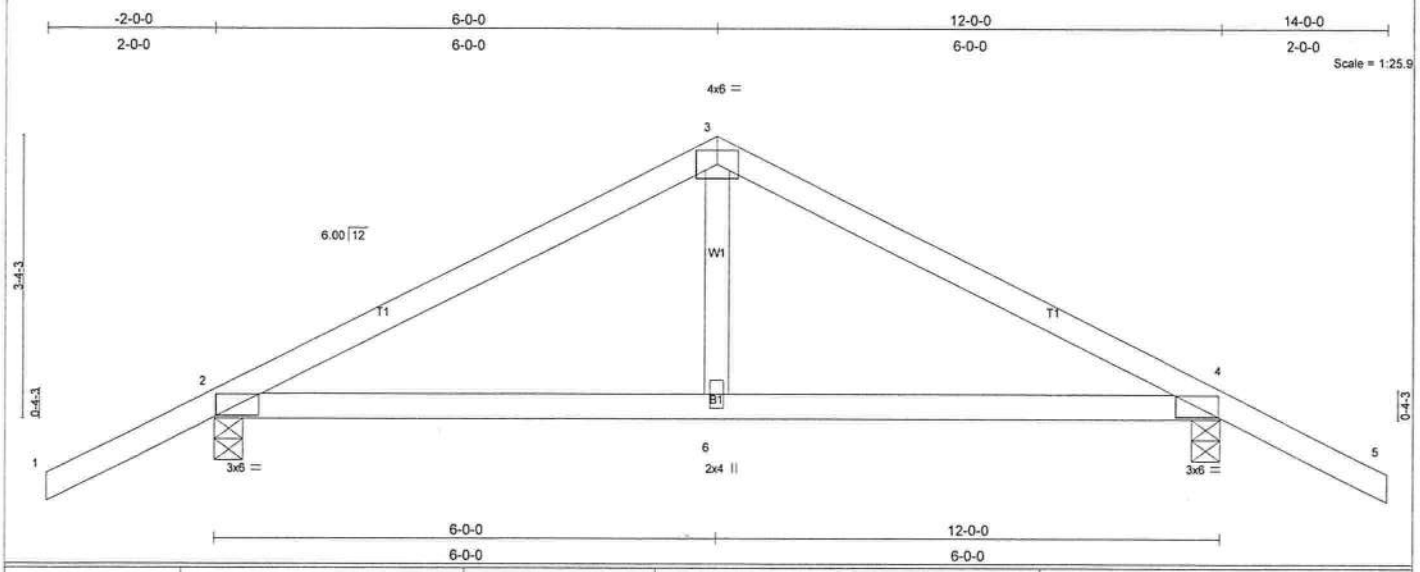
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-5=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-10=-24(F=3, B=3), 10=-24(F=3, B=3)-to-8=-45(F=-8, B=-8), 3=11(F=5, B=5)-to-8=-15(F=-8, B=-8), 8=-45(F=-8, B=-8)-to-6=-74(F=-22, B=-22)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071188

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T01	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:37 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=607/0-4-0, 4=607/0-4-0
Max Horz 2=80(load case 5)
Max Uplift2=-290(load case 5), 4=-290(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-653/172, 3-4=-653/171, 4-5=0/47
BOT CHORD 2-6=-71/520, 4-6=-71/520
WEBS 3-6=0/201

JOINT STRESS INDEX
2 = 0.42, 3 = 0.48, 4 = 0.42 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 2 and 290 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071189

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T01A	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:39 2007 Page 1

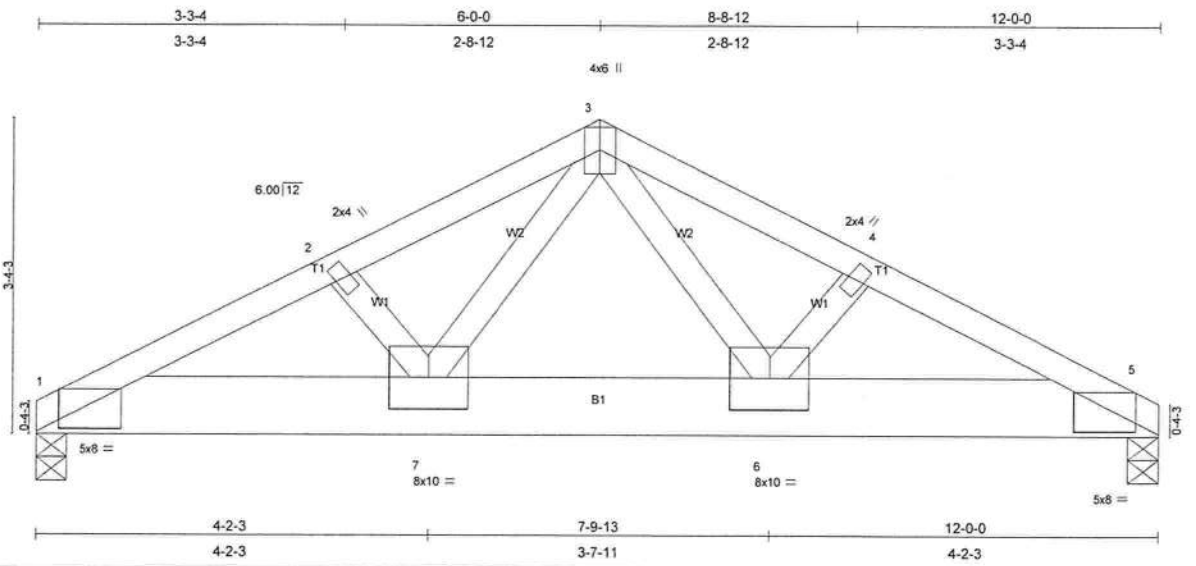

Plate Offsets (X,Y): [1:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.45	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3675/0-4-0, 5=3675/0-4-0
Max Horz 1=-42(load case 2)
Max Uplift1=-1362(load case 4), 5=-1362(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5652/2090, 2-3=-5564/2084, 3-4=-5564/2085, 4-5=-5652/2091
BOT CHORD 1-7=-1878/5046, 6-7=-1250/3493, 5-6=-1838/5046
WEBS 2-7=-130/124, 3-7=-1045/2781, 3-6=-1045/2781, 4-6=-130/124

JOINT STRESS INDEX
1 = 0.63, 2 = 0.03, 3 = 0.68, 4 = 0.03, 5 = 0.63, 6 = 0.27 and 7 = 0.27

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1362 lb uplift at joint 1 and 1362 lb uplift at joint 5.
7) Girder carries tie-in span(s): 28-0-0 from 0-0-0 to 12-0-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-576(F=-546)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071190

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T01G	COMMON	1	1	Job Reference (optional)

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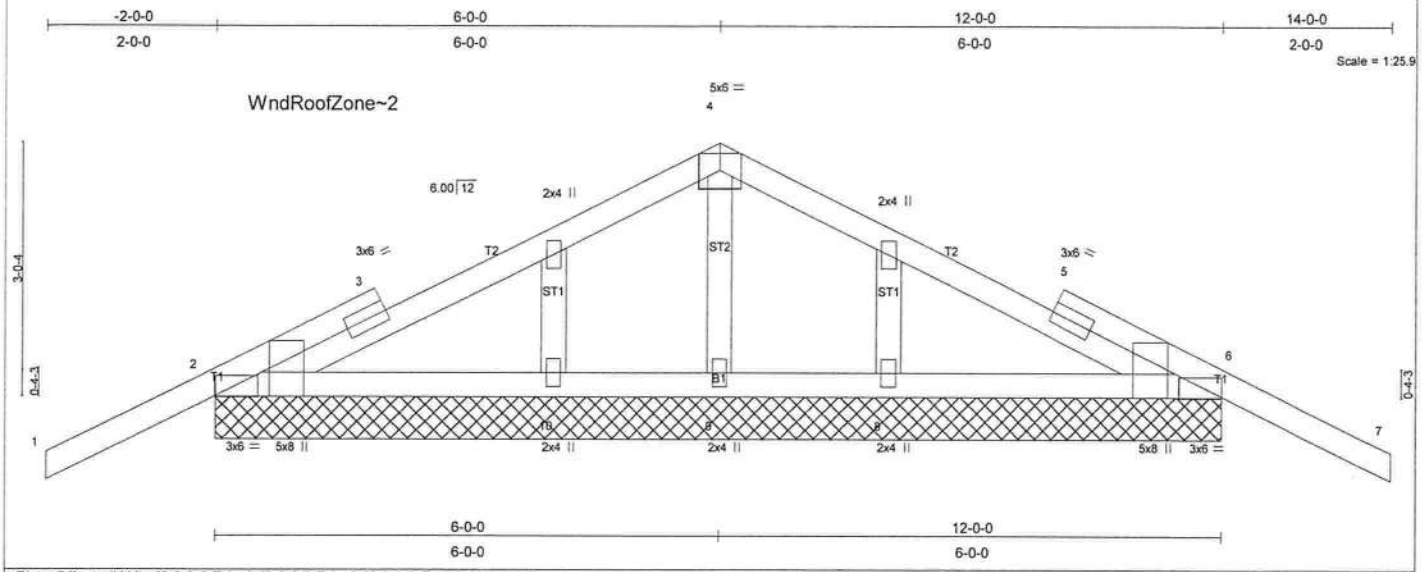

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.04	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	0.06	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 57 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=900/12-0-0, 6=900/12-0-0, 9=-92/12-0-0, 10=238/12-0-0, 8=238/12-0-0
Max Horz 2=76(load case 5)
Max Uplift2=-457(load case 5), 6=-458(load case 6), 9=-92(load case 1), 10=-39(load case 5), 8=-38(load case 6)
Max Grav 2=900(load case 1), 6=900(load case 1), 9=57(load case 5), 10=239(load case 9), 8=239(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-19/99, 2-3=-934/543, 3-4=-834/531, 4-5=-834/531, 5-6=-934/543, 6-7=-19/99
BOT CHORD 2-10=-329/746, 9-10=-329/746, 8-9=-329/746, 6-8=-329/746

JOINT STRESS INDEX
2 = 0.90, 2 = 0.00, 3 = 0.00, 3 = 0.65, 4 = 0.83, 5 = 0.00, 5 = 0.65, 6 = 0.90, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 2, 458 lb uplift at joint 6, 92 lb uplift at joint 9, 39 lb uplift at joint 10 and 38 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071191

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T02	HIP	1	1	Job Reference (optional)

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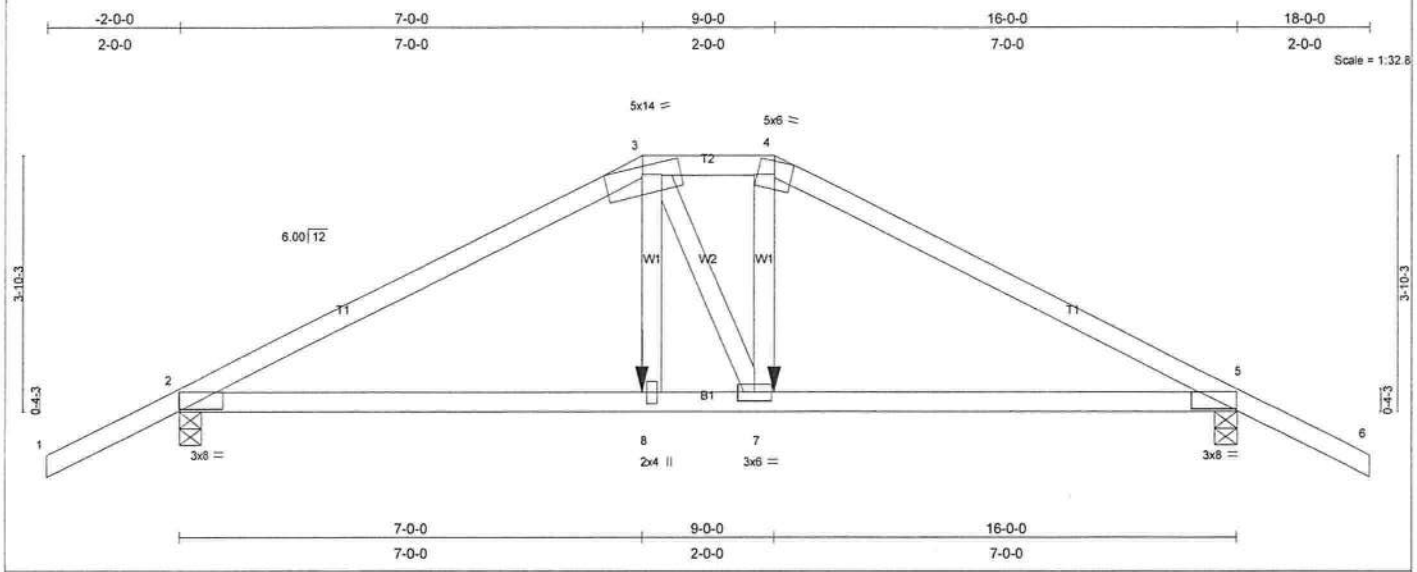

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.13	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.20	2-8	>955	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 72 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-3 oc bracing.

REACTIONS (lb/size) 2=1407/0-4-0, 5=1407/0-4-0
Max Horz 2=-87(load case 5)
Max Uplift2=-862(load case 4), 5=-862(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2301/1215, 3-4=-2004/1164, 4-5=-2306/1217, 5-6=0/47
BOT CHORD 2-8=-1010/1971, 7-8=-1025/2000, 5-7=-990/1975
WEBS 3-8=-375/716, 4-7=-422/829, 3-7=-141/161

JOINT STRESS INDEX
2 = 0.73, 3 = 0.91, 4 = 0.64, 5 = 0.73, 7 = 0.54 and 8 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 862 lb uplift at joint 2 and 862 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-117(F=-63), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071192

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T03	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:41 2007 Page 1

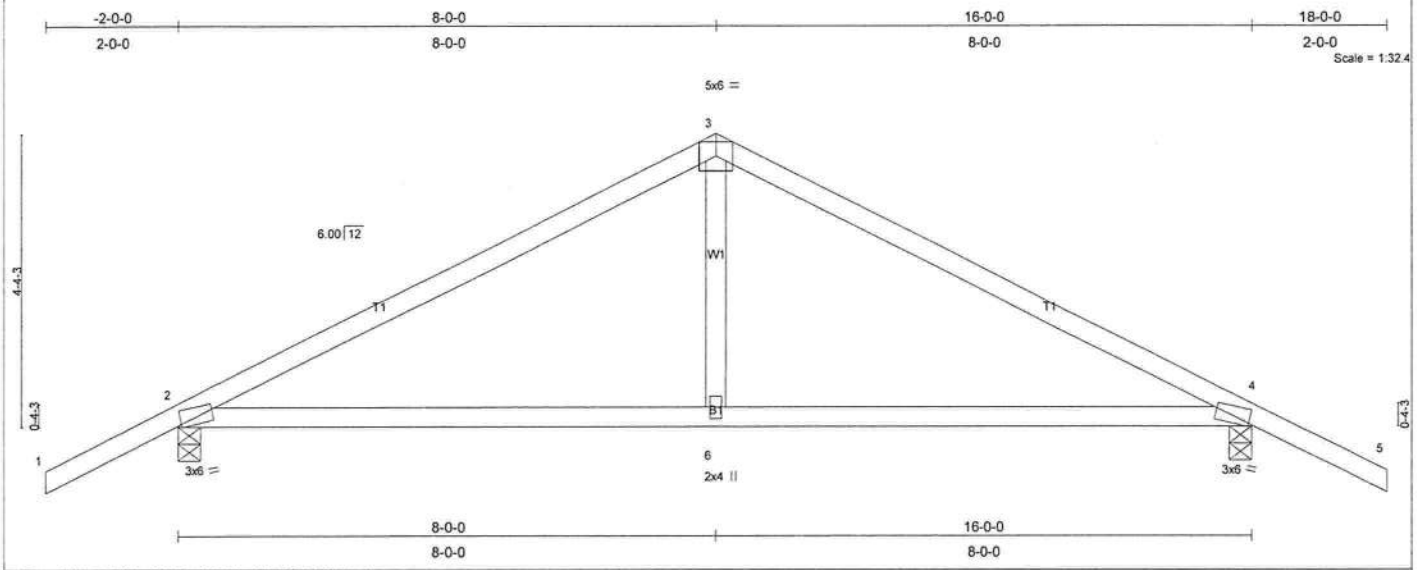

Plate Offsets (X,Y): [2:0-0-10,Edge], [4:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.21	4-6	>890	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	4-6	>937	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-2 oc bracing.

REACTIONS (lb/size) 2=775/0-4-0, 4=775/0-4-0
Max Horz 2=-94(load case 6)
Max Uplift2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-924/803, 3-4=-924/803, 4-5=0/47
BOT CHORD 2-6=-558/746, 4-6=-558/746
WEBS 3-6=-466/293

JOINT STRESS INDEX
2 = 0.76, 3 = 0.75, 4 = 0.76 and 6 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

LOAD CASE(S) Standard

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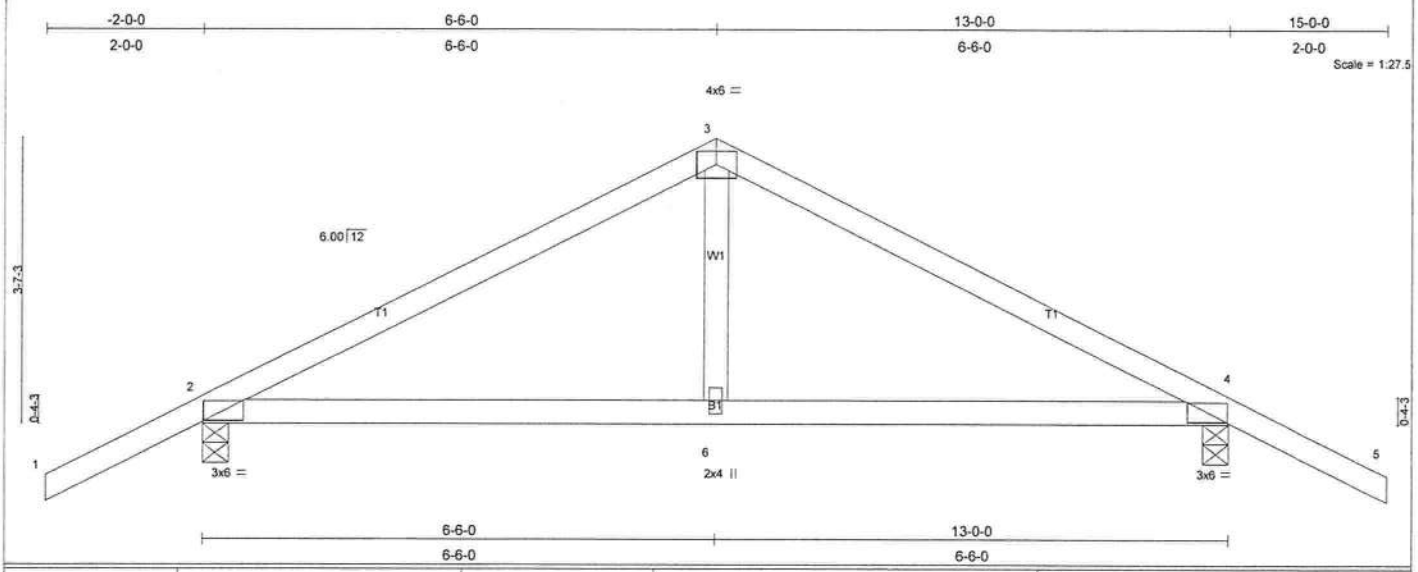
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071193

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T04	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:42 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.08	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=649/0-4-0, 4=649/0-4-0
Max Horz 2=-84(load case 6)
Max Uplift2=-303(load case 5), 4=-303(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-722/196, 3-4=-722/195, 4-5=0/47
BOT CHORD 2-6=-89/578, 4-6=-89/578
WEBS 3-6=0/225

JOINT STRESS INDEX
2 = 0.47, 3 = 0.58, 4 = 0.47 and 6 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 303 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071194

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T04A	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:43 2007 Page 1

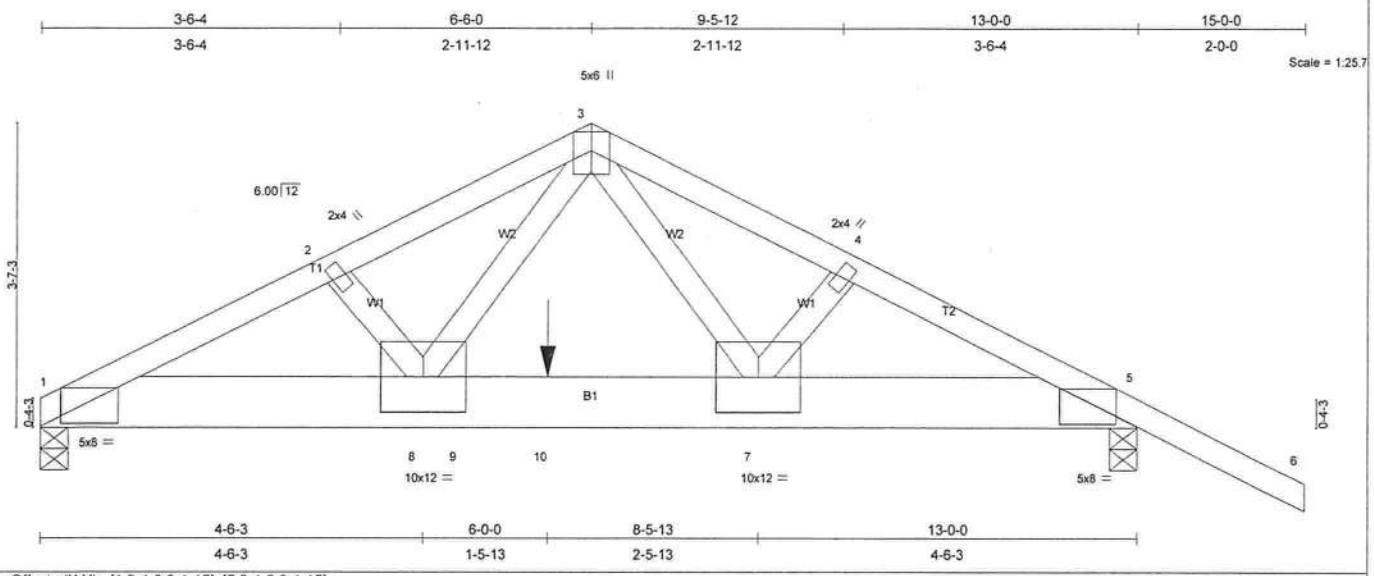


Plate Offsets (X,Y): [1:0-4-0,0-1-15], [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.09	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.14	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.70	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4174/0-4-0, 5=2457/0-4-0
Max Horz 1=-116(load case 5)
Max Uplift1=-1548(load case 4), 5=-987(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6796/2517, 2-3=-6702/2512, 3-4=-4925/1817, 4-5=-5022/1812, 5-6=0/54
BOT CHORD 1-8=-2231/6064, 8-9=-1271/3644, 9-10=-1271/3644, 7-10=-1271/3644, 5-7=-1560/4452
WEBS 2-8=-132/131, 3-8=-1653/4388, 3-7=-483/1420, 4-7=-95/112

JOINT STRESS INDEX
1 = 0.75, 2 = 0.03, 3 = 0.75, 4 = 0.03, 5 = 0.75, 7 = 0.38 and 8 = 0.38

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1548 lb uplift at joint 1 and 987 lb uplift at joint 5.
7) Girder carries tie-in span(s): 28-0-0 from 0-0-0 to 5-0-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2811 lb down and 1061 lb up at 6-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-576(F=-546), 5-9=-30
Concentrated Loads (lb)
Vert: 10=-2811(F)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071195

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T04G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:44 2007 Page 1

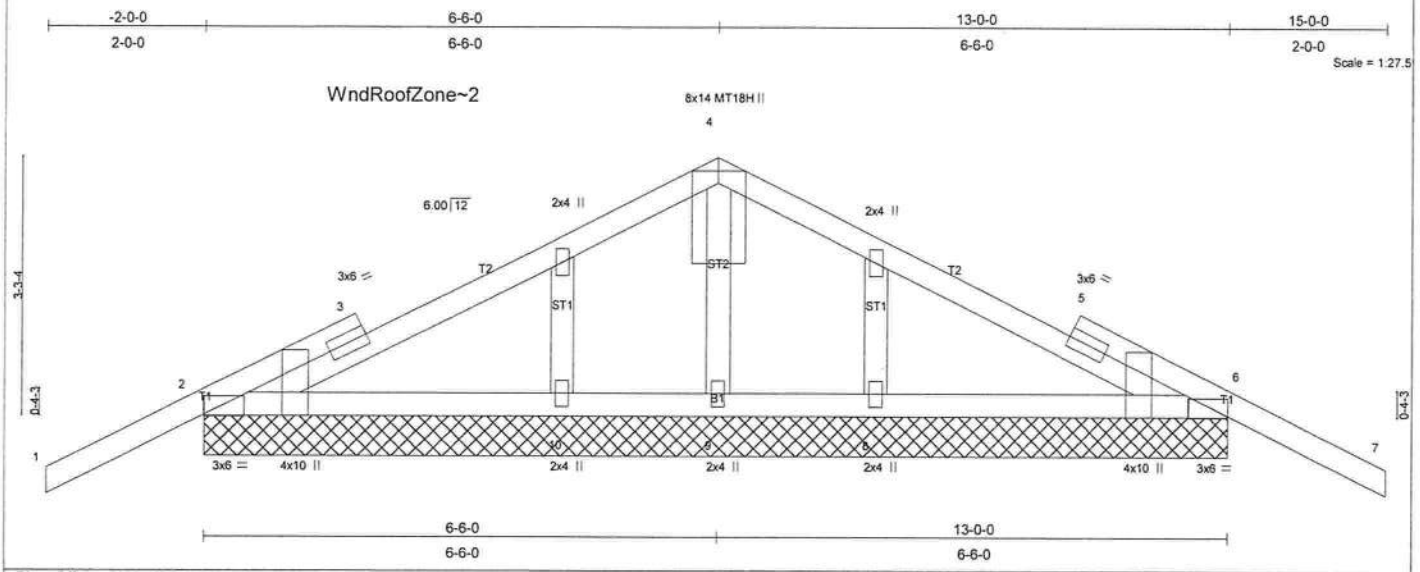


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	0.07	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	0.10	7	n/r	90	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 62 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-7 oc bracing.

REACTIONS (lb/size) 2=956/13-0-0, 6=956/13-0-0, 9=-138/13-0-0, 10=277/13-0-0, 8=277/13-0-0
Max Horz 2=79(load case 5)
Max Uplift2=-478(load case 5), 6=-479(load case 6), 9=-138(load case 1), 10=-50(load case 5), 8=-50(load case 6)
Max Grav 2=956(load case 1), 6=956(load case 1), 9=70(load case 5), 10=278(load case 9), 8=278(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-18/99, 2-3=-1018/577, 3-4=-911/566, 4-5=-911/566, 5-6=-1018/577, 6-7=-18/99
BOT CHORD 2-10=-356/815, 9-10=-356/815, 8-9=-356/815, 6-8=-356/815

JOINT STRESS INDEX
2 = 0.84, 2 = 0.00, 3 = 0.00, 3 = 0.70, 4 = 0.92, 5 = 0.00, 5 = 0.70, 6 = 0.84, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 478 lb uplift at joint 2, 479 lb uplift at joint 6, 138 lb uplift at joint 9 , 50 lb uplift at joint 10 and 50 lb uplift at joint 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071196

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T05	SPECIAL	1	2	Job Reference (optional)

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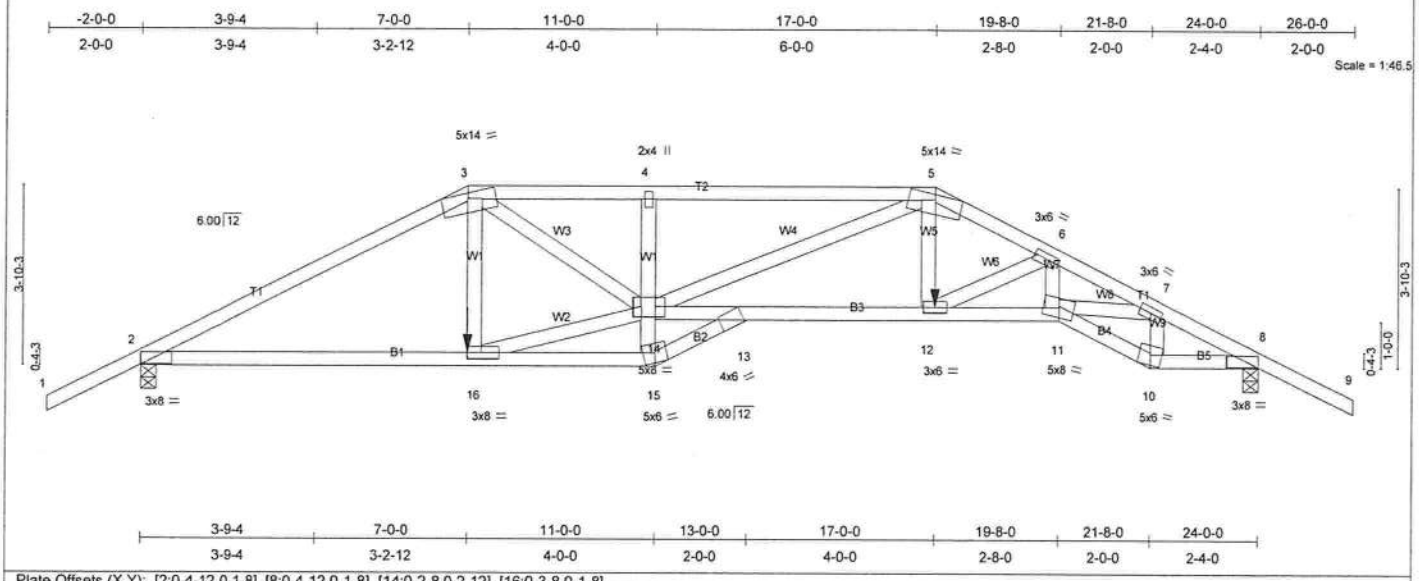

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [8:0-4-12,0-1-8], [14:0-2-8,0-2-12], [16:0-3-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.26	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.41	12-13	>687	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.55	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
6-0-0 oc bracing: 13-15
9-7-12 oc bracing: 11-12.

REACTIONS (lb/size) 2=2208/0-4-0, 8=2202/0-4-0
Max Horz 2=136(load case 4)
Max Uplift2=-912(load case 4), 8=-867(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-4041/1508, 3-4=-5830/2202, 4-5=-5874/2208, 5-6=-5491/1959, 6-7=-6984/2370, 7-8=-4006/1347, 8-9=0/47
BOT CHORD 2-16=-1348/3518, 15-16=-157/175, 13-15=-158/43, 13-14=-1652/5021, 12-13=-1697/5028, 11-12=-1999/6068, 10-11=-1170/3600, 8-10=-1128/3447
WEBS 3-16=-117/99, 3-14=-1046/2800, 14-15=-71/477, 4-14=-385/319, 5-14=-487/1022, 5-12=-546/1720, 6-12=-1262/429, 6-11=-358/1224, 7-11=-979/2954, 7-10=-1461/508, 14-16=-1227/3450

JOINT STRESS INDEX
2 = 0.74, 3 = 0.50, 4 = 0.34, 5 = 0.75, 6 = 0.45, 7 = 0.73, 8 = 0.73, 10 = 0.47, 11 = 0.87, 12 = 0.56, 13 = 0.19, 14 = 0.86, 15 = 0.41 and 16 = 0.67

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 912 lb uplift at joint 2 and 867 lb uplift at joint 8.
8) Girder carries tie-in span(s): 2-3-9 from 11-0-0 to 17-0-0; 6-6-2 from 11-0-0 to 17-0-0
9) Girder carries hip end with 13-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0, and 539 lb down and 204 lb up at 17-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-117(F=-63), 4-5=-60(F=-6), 5-9=-54, 2-16=-30, 15-16=-65(F=-35), 13-15=-119(F=-89), 13-14=-89(F), 12-13=-119(F=-89), 11-12=-30, 10-11=-30, 8-10=-30
Concentrated Loads (lb)
Vert: 16=-539(F) 12=-539(F)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071197

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T06	SPECIAL	1	1	Job Reference (optional)

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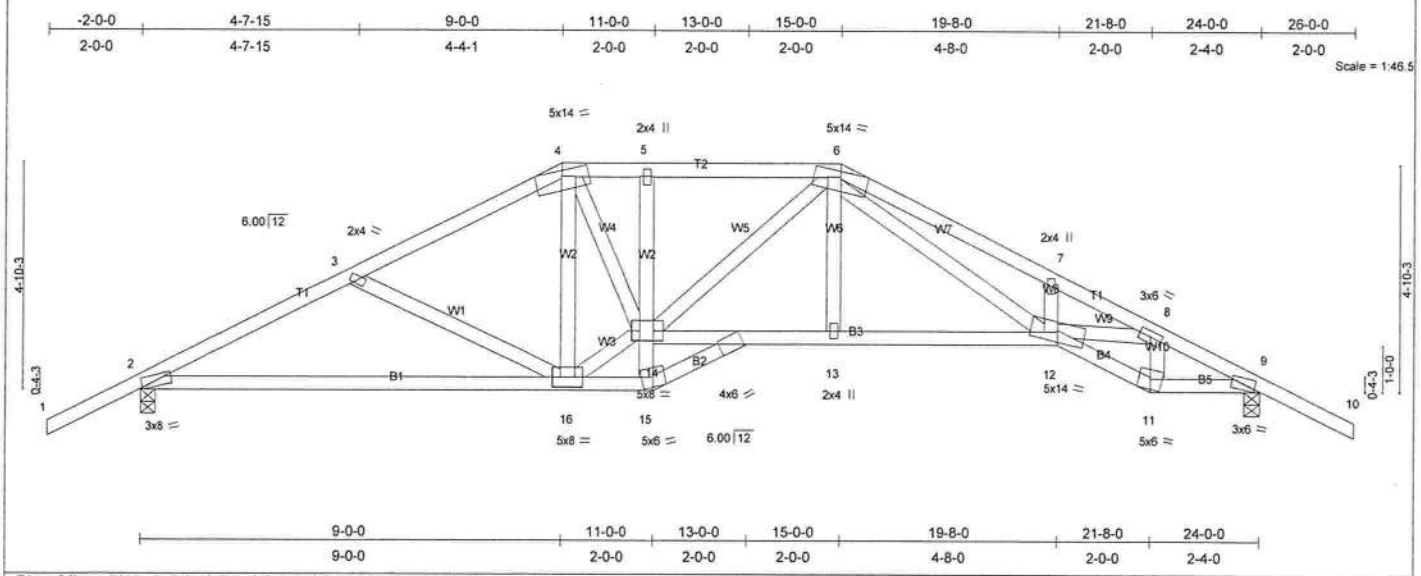

Plate Offsets (X,Y): [2:0-0-10,Edge], [9:0-1-8,0-0-7], [14:0-2-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.18	2-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.30	2-16	>944	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-8-11 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 9=1109/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-431(load case 5), 9=-431(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1682/500, 3-4=-1428/396, 4-5=-1595/465, 5-6=-1599/466, 6-7=-2907/800, 7-8=-2898/699, 8-9=-1758/413, 9-10=0/47
BOT CHORD 2-16=-411/1460, 15-16=-10/18, 13-14=-264/1571, 12-13=-265/1563, 11-12=-302/1561, 9-11=-295/1493
WEBS 3-16=-267/202, 4-16=-259/109, 14-15=-84/0, 5-14=-134/109, 6-13=0/180, 6-12=-392/1282, 7-12=-161/187, 8-12=-264/1157, 8-11=-598/150, 4-14=-206/769, 14-16=-246/1362, 6-14=-93/159

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.40, 5 = 0.34, 6 = 0.65, 7 = 0.34, 8 = 0.57, 9 = 0.78, 11 = 0.40, 12 = 0.51, 13 = 0.34, 14 = 0.71, 15 = 0.20, 16 = 0.63 and 17 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 2 and 431 lb uplift at joint 9.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071198

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:47 2007 Page 1

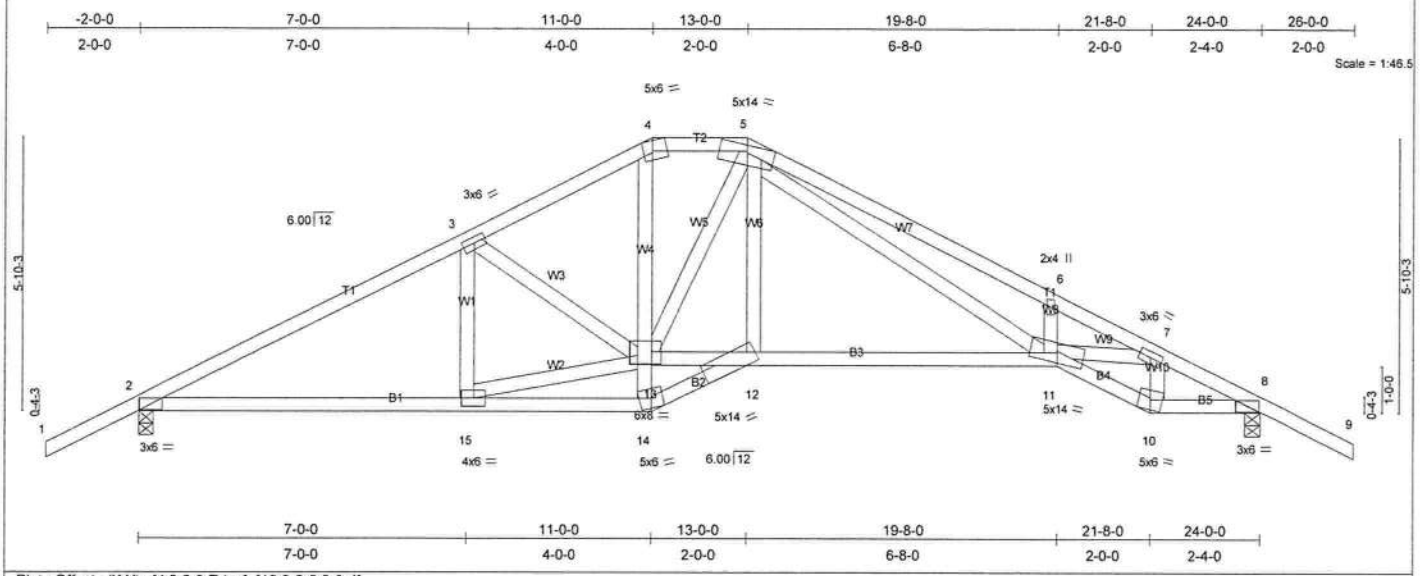

Plate Offsets (X,Y): [4:0-2-3,Edge], [13:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.22	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.35	11-12	>800	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=-115(load case 6)
Max Uplift2=-444(load case 5), 8=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1667/473, 3-4=-1460/457, 4-5=-1248/436, 5-6=-3035/924, 6-7=-2968/776, 7-8=-1740/433, 8-9=0/47
BOT CHORD 2-15=-377/1408, 14-15=-7/33, 12-13=-238/1321, 11-12=-237/1328, 10-11=-319/1543, 8-10=-314/1472
WEBS 3-15=-139/126, 13-14=0/34, 4-13=-224/615, 5-12=0/216, 5-11=-548/1614, 6-11=-266/276, 7-11=-335/1259, 7-10=-601/137, 3-13=-242/172, 13-15=-382/1418, 5-13=-289/137

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.52, 5 = 0.81, 6 = 0.34, 7 = 0.62, 8 = 0.77, 10 = 0.40, 11 = 0.52, 12 = 0.38, 13 = 0.85, 14 = 0.20 and 15 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2 and 444 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071199

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:48 2007 Page 1

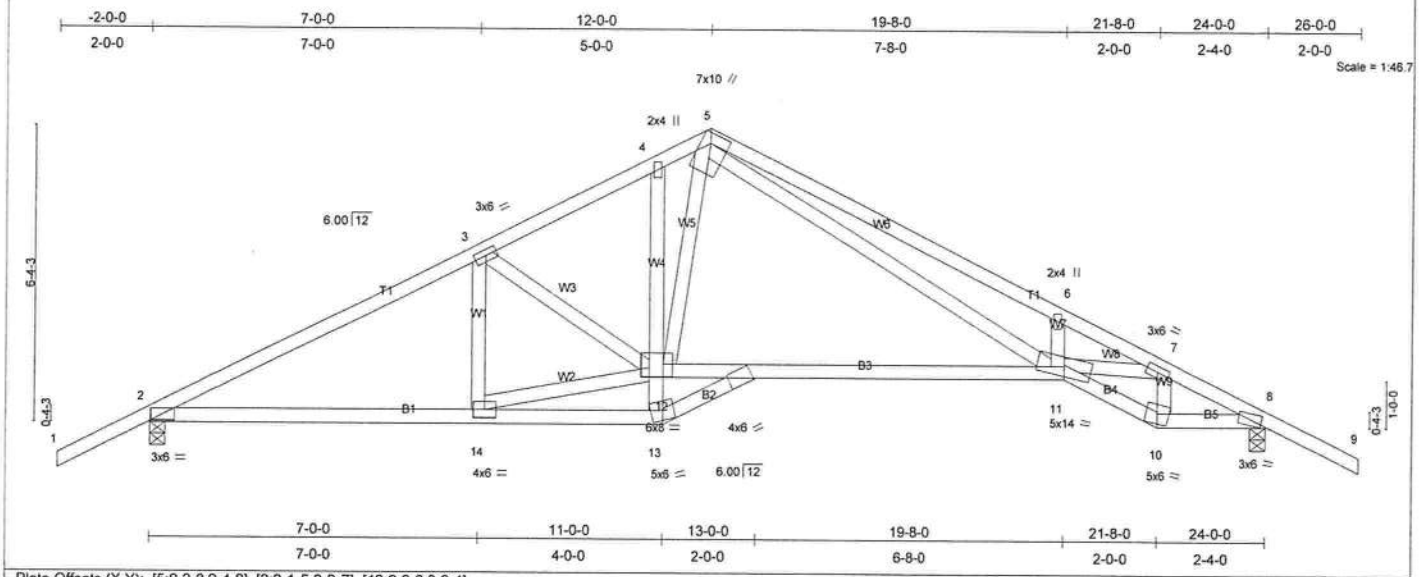

Plate Offsets (X,Y): [5:0-2-8,0-4-8], [8:0-1-5,0-0-7], [12:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.38	11-12	>745	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.63	11-12	>449	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-12 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-450(load case 5), 8=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1668/485, 3-4=-1459/473, 4-5=-1278/455, 5-6=-3207/977, 6-7=-3129/806, 7-8=-1701/444, 8-9=0/47
BOT CHORD 2-14=-395/1411, 13-14=-7/32, 11-12=-237/1180, 10-11=-331/1534, 8-10=-327/1439
WEBS 3-14=-140/131, 3-12=-259/167, 5-11=-608/1949, 6-11=-298/318, 7-11=-364/1452, 7-10=-694/123, 12-13=0/34, 4-12=-172/282, 5-12=-103/541, 12-14=-400/1421

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.34, 5 = 0.90, 6 = 0.34, 7 = 0.71, 8 = 0.78, 10 = 0.44, 11 = 0.63, 12 = 0.85, 13 = 0.20, 14 = 0.65 and 15 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071200

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T09	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:49 2007 Page 1

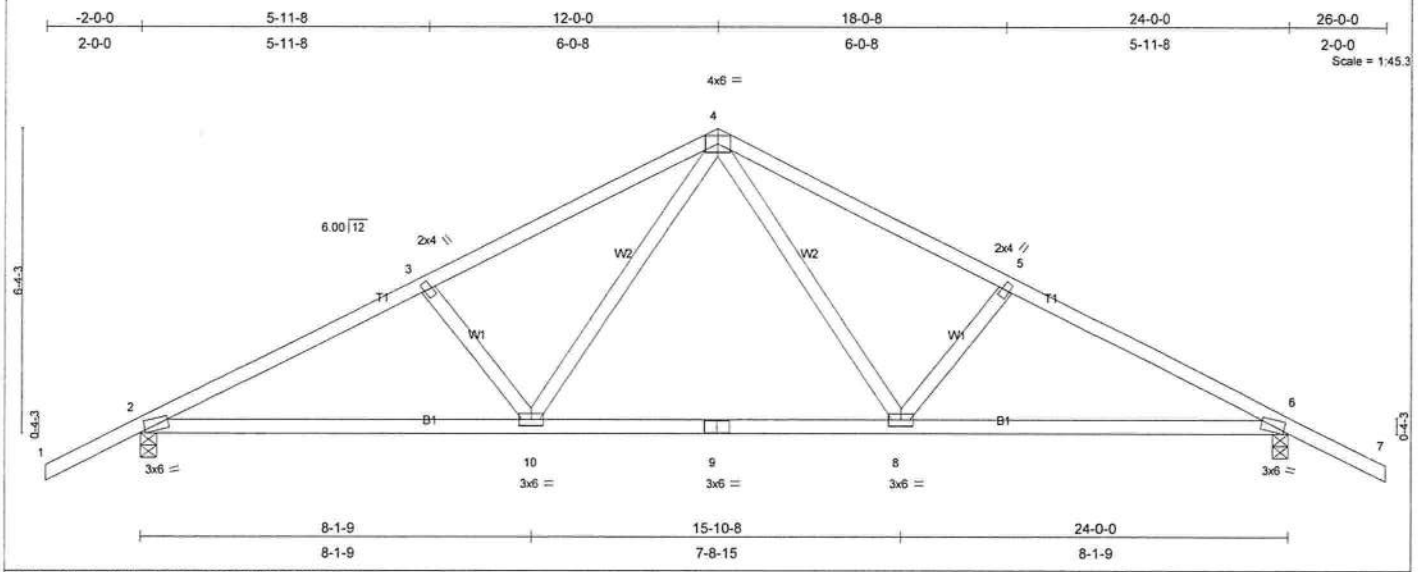

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [6:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.12	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.21	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-5-14 oc bracing.

REACTIONS (lb/size) 2=1111/0-4-0, 6=1111/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-449(load case 5), 6=-449(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1687/520, 3-4=-1495/498, 4-5=-1495/498, 5-6=-1687/520, 6-7=0/47
BOT CHORD 2-10=-441/1441, 9-10=-193/971, 8-9=-193/971, 6-8=-336/1441
WEBS 3-10=-302/253, 4-10=-176/575, 4-8=-176/575, 5-8=-302/254

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.56, 5 = 0.34, 6 = 0.79, 8 = 0.44, 9 = 0.46 and 10 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 449 lb uplift at joint 6.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071201

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T10	COMMON	1	2	Job Reference (optional)

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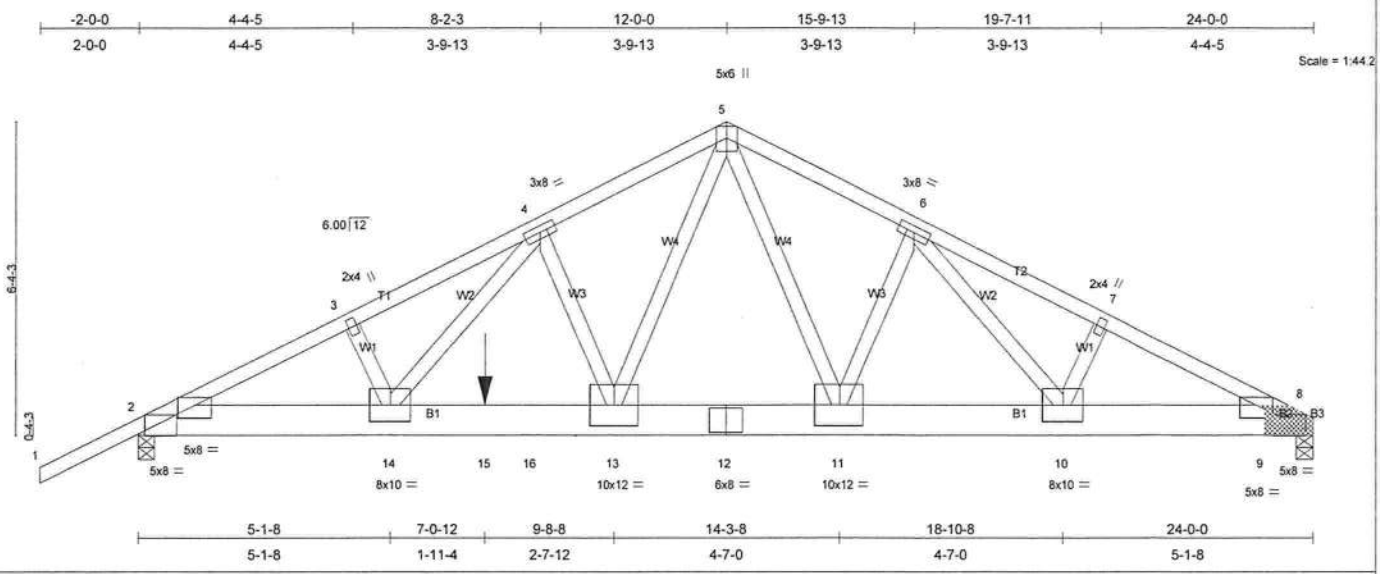

Plate Offsets (X,Y): [2:0-10-0,0-3-15], [2:0-1-11,Edge], [8:0-1-11,Edge], [8:0-10-0,0-3-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.26	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.42	13-14	>682	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 345 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=7493/0-4-7 (0-4-0 + bearing block), 2=5835/0-4-0
Max Horz 2=155(load case 4)
Max Uplift8=-2777(load case 5), 2=-2234(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-12195/4441, 3-4=-12103/4470, 4-5=-10552/3962, 5-6=-10358/3905, 6-7=-13204/4941, 7-8=-13283/4911
BOT CHORD 2-14=-4007/10881, 14-15=-3557/9747, 15-16=-3557/9747, 13-16=-3557/9747, 12-13=-2693/7557, 11-12=-2693/7557, 10-11=-3520/9810, 9-10=-4353/11883, 8-9=-4353/11883
WEBS 3-14=-208/177, 4-14=-629/1746, 4-13=-959/449, 5-13=-1937/5104, 5-11=-1768/4626, 6-11=-1618/717, 6-10=-1245/3256, 7-10=-257/199

JOINT STRESS INDEX
2 = 0.81, 2 = 0.66, 3 = 0.34, 4 = 0.85, 5 = 0.96, 6 = 0.85, 7 = 0.34, 8 = 0.81, 8 = 0.00, 8 = 0.00, 8 = 0.66, 9 = 0.00, 9 = 0.00, 9 = 0.00, 10 = 0.29, 11 = 0.47, 12 = 0.67, 13 = 0.47 and 14 = 0.29

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) 2 X 8 SYP 2400F 2.0E bearing block 12" long at jt. 8 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block. Bearing is assumed to be SYP.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2777 lb uplift at joint 8 and 2234 lb uplift at joint 2.
8) Girder carries tie-in span(s): 28-0-0 from 8-0-0 to 24-0-0
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2578 lb down and 973 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-16=-30, 8-16=-576(F=-546)
Concentrated Loads (lb)
Vert: 15=-2578(F)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071202

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T11	MONO HIP	1	1	Job Reference (optional)

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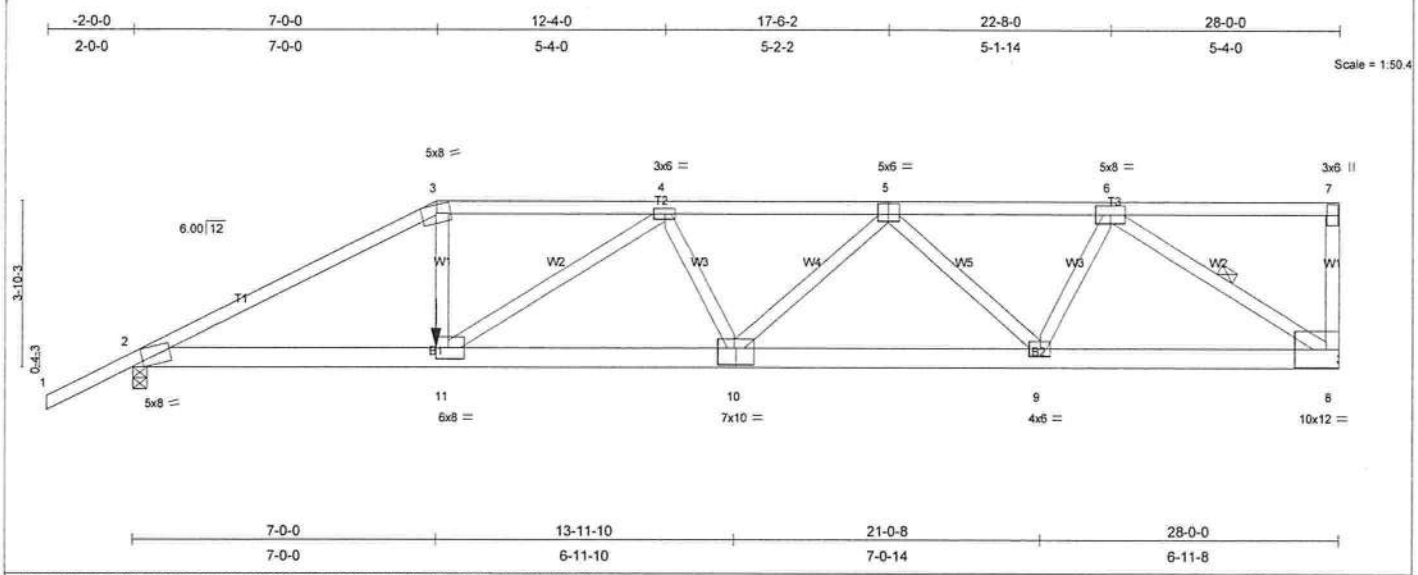

Plate Offsets (X,Y): [2:0-2-15,0-0-14], [5:0-3-0,0-3-0], [10:0-5-0,0-4-8], [11:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.28	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.46	10-11	>728	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-8 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=2578/Mechanical, 2=2460/0-4-0
Max Horz 2=228(load case 4)
Max Uplift8=-1161(load case 3), 2=-1055(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4685/1980, 3-4=-4176/1832, 4-5=-5263/2306, 5-6=-3865/1673, 6-7=-114/57, 7-8=-297/232
BOT CHORD 2-11=-1809/4109, 10-11=-2379/5224, 9-10=-2235/4869, 8-9=-1472/3190
WEBS 3-11=-578/1572, 4-11=-1262/714, 4-10=0/180, 5-10=-113/552, 5-9=-1408/788, 6-9=-475/1590, 6-8=-3702/1702

JOINT STRESS INDEX
2 = 0.91, 3 = 0.88, 4 = 0.46, 5 = 0.80, 6 = 0.91, 7 = 0.62, 8 = 0.64, 9 = 0.91, 10 = 0.86 and 11 = 0.43

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1161 lb uplift at joint 8 and 1055 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-11=-30, 8-11=-65(F=-35)
Concentrated Loads (lb)
Vert: 11=-539(F)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071203

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T12	MONO HIP	1	1	Job Reference (optional)

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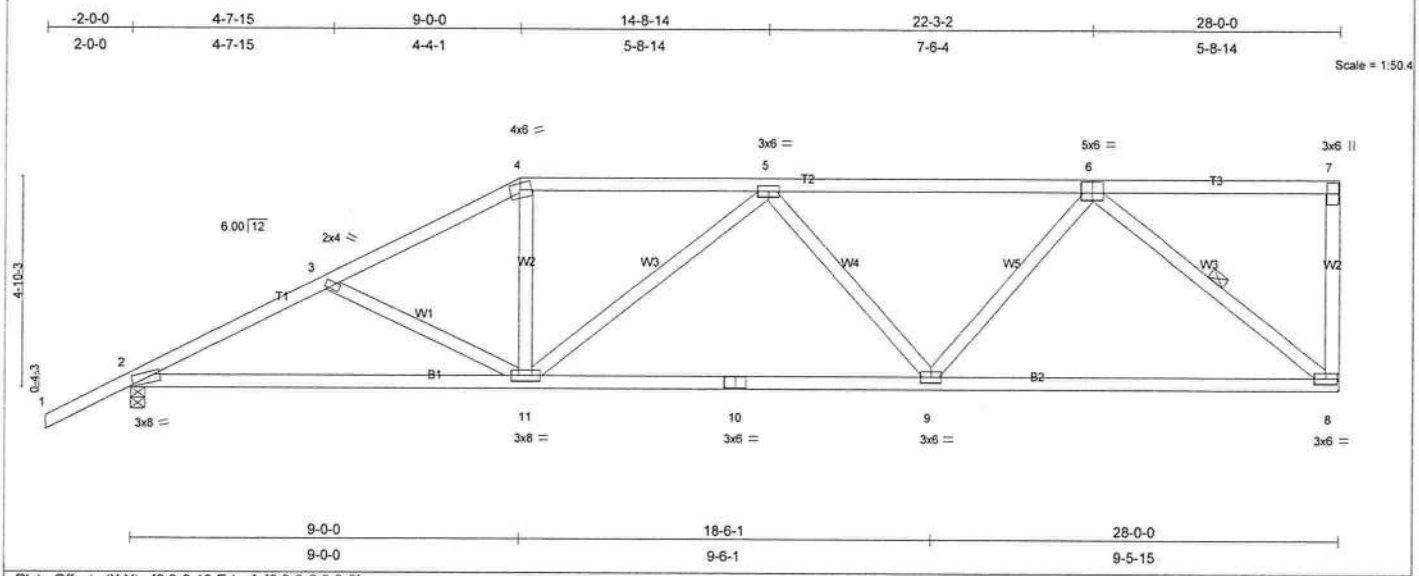

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.18	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.29	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-9 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=272(load case 5)
Max Uplift8=-423(load case 4), 2=-459(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2057/588, 3-4=-1821/548, 4-5=-1598/521, 5-6=-1585/528, 6-7=-56/3, 7-8=-130/83
BOT CHORD 2-11=-635/1789, 10-11=-656/1776, 9-10=-656/1776, 8-9=-451/1156
WEBS 3-11=-232/199, 4-11=-95/530, 5-11=-229/221, 5-9=-300/200, 6-9=-121/674, 6-8=-1418/578

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.52, 5 = 0.39, 6 = 0.52, 7 = 0.29, 8 = 0.77, 9 = 0.46, 10 = 0.82 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 8 and 459 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071204

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T13	MONO HIP	1	1	Job Reference (optional)

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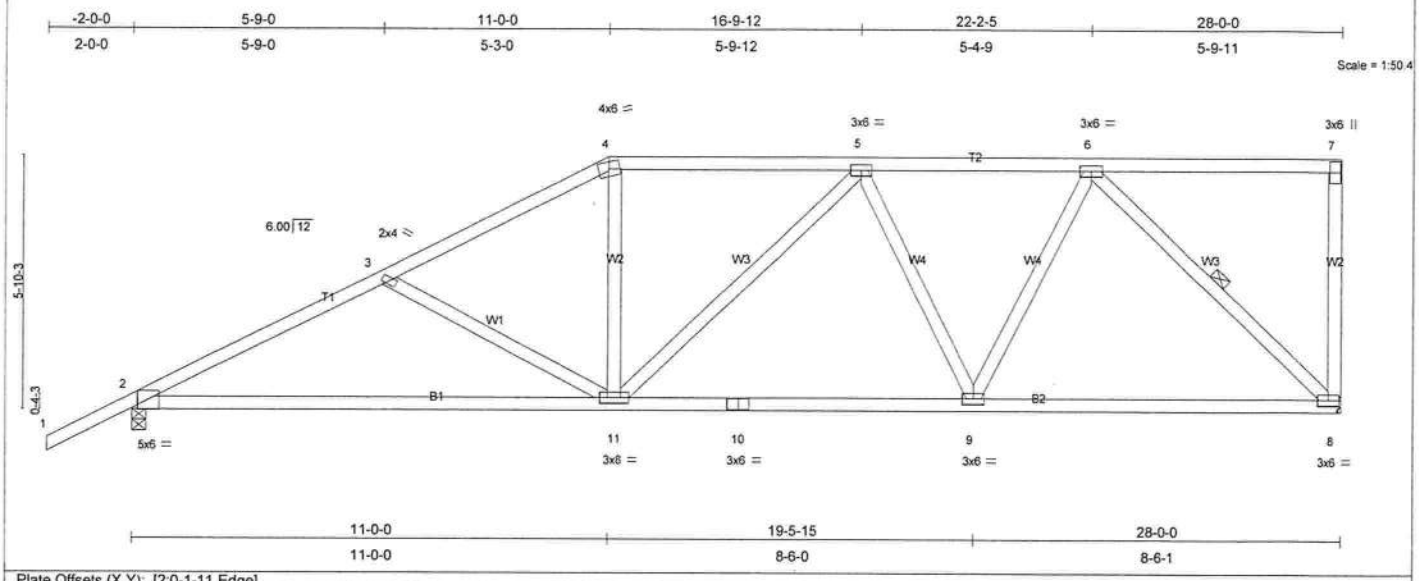

Plate Offsets (X,Y): [2:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.34	2-11	>980	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.58	2-11	>570	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-5 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=318(load case 5)
Max Uplift8=-415(load case 4), 2=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1984/572, 3-4=-1669/474, 4-5=-1443/457, 5-6=-1232/412, 6-7=-43/7, 7-8=-148/96
BOT CHORD 2-11=-679/1726, 10-11=-495/1399, 9-10=-495/1399, 8-9=-354/949
WEBS 3-11=-334/265, 4-11=-40/425, 5-11=-65/118, 5-9=-383/191, 6-9=-133/649, 6-8=-1268/486

JOINT STRESS INDEX
2 = 0.72, 3 = 0.34, 4 = 0.60, 5 = 0.46, 6 = 0.54, 7 = 0.32, 8 = 0.65, 9 = 0.54, 10 = 0.47 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 8 and 469 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071205

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T14	HIP	1	1	Job Reference (optional)

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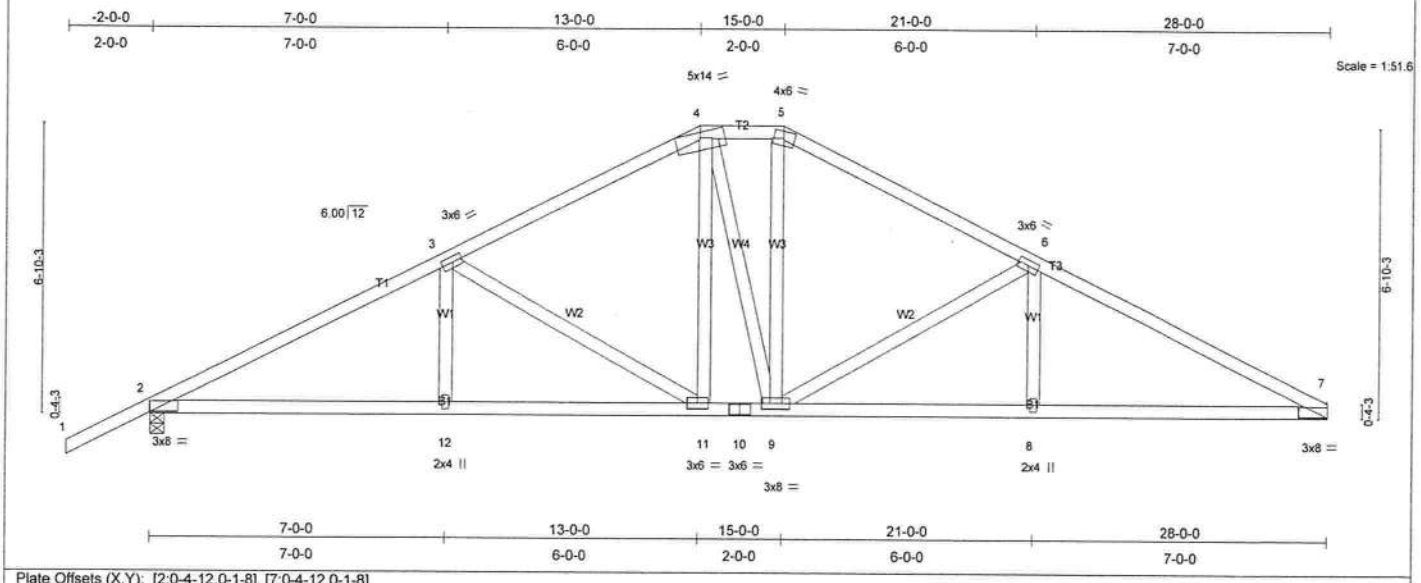

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [7:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.14	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.24	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 150 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-8 oc bracing.

REACTIONS (lb/size) 7=1162/Mechanical, 2=1288/0-4-0
Max Horz 2=154(load case 5)
Max Uplift7=-372(load case 6), 2=-501(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2066/600, 3-4=-1461/486, 4-5=-1246/490, 5-6=-1465/488, 6-7=-2108/649
BOT CHORD 2-12=-530/1765, 11-12=-530/1765, 10-11=-273/1243, 9-10=-273/1243, 8-9=-489/1812, 7-8=-489/1812
WEBS 3-12=0/224, 3-11=-618/300, 4-11=-129/411, 5-9=-146/431, 6-9=-667/350, 6-8=0/242, 4-9=-150/168

JOINT STRESS INDEX
2 = 0.78, 3 = 0.41, 4 = 0.51, 5 = 0.46, 6 = 0.41, 7 = 0.75, 8 = 0.34, 9 = 0.69, 10 = 0.50, 11 = 0.35 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 7 and 501 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071206

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T15	COMMON	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:54 2007 Page 1

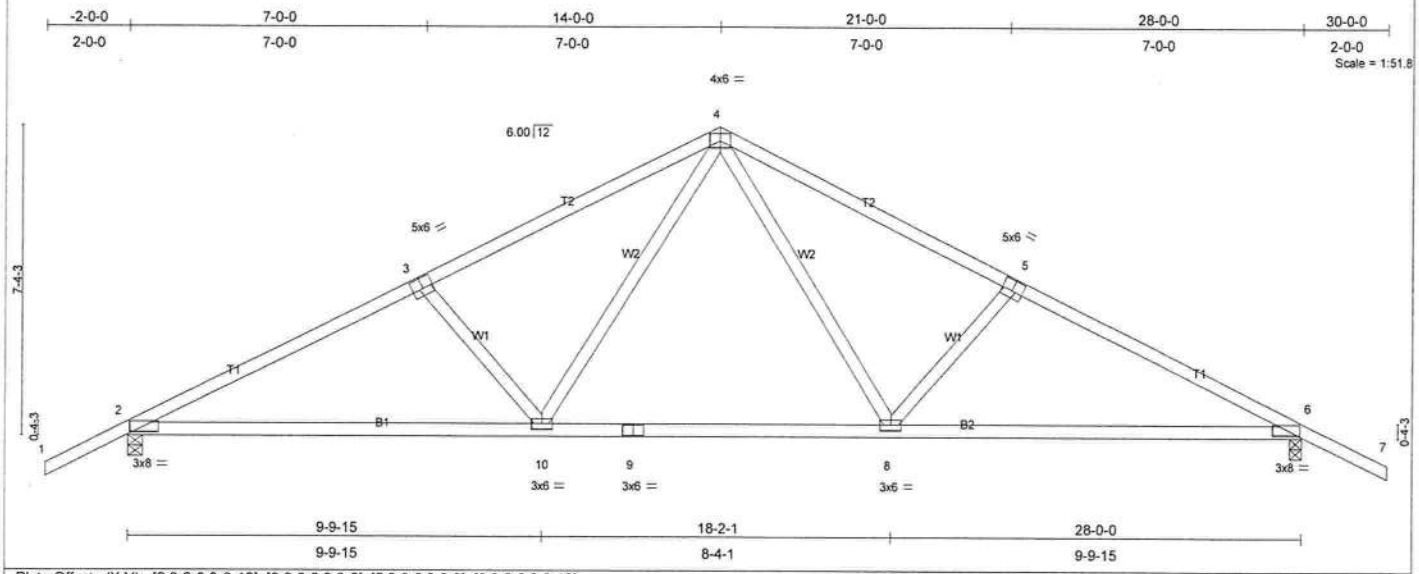

Plate Offsets (X,Y): [2:0-8-8,0-0-10], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-8-8,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.37	8-10	>887	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.60	8-10	>554	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.36	Horz(TL)	0.09	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-4-12 oc bracing.

REACTIONS (lb/size) 2=1489/0-4-0, 6=1487/0-3-8
Max Horz 2=136(load case 5)
Max Uplift2=-582(load case 5), 6=-581(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2469/811, 3-4=-2227/769, 4-5=-2229/770, 5-6=-2473/813, 6-7=0/47
BOT CHORD 2-10=-701/2128, 9-10=-353/1463, 8-9=-353/1463, 6-8=-572/2132
WEBS 3-10=-352/300, 4-10=-302/907, 4-8=-304/911, 5-8=-356/302

JOINT STRESS INDEX
2 = 0.78, 3 = 0.63, 4 = 0.75, 5 = 0.63, 6 = 0.78, 8 = 0.72, 9 = 0.80 and 10 = 0.72

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 582 lb uplift at joint 2 and 581 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MARCH 23,2007 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071207

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T17	SPECIAL	1	1	Job Reference (optional)

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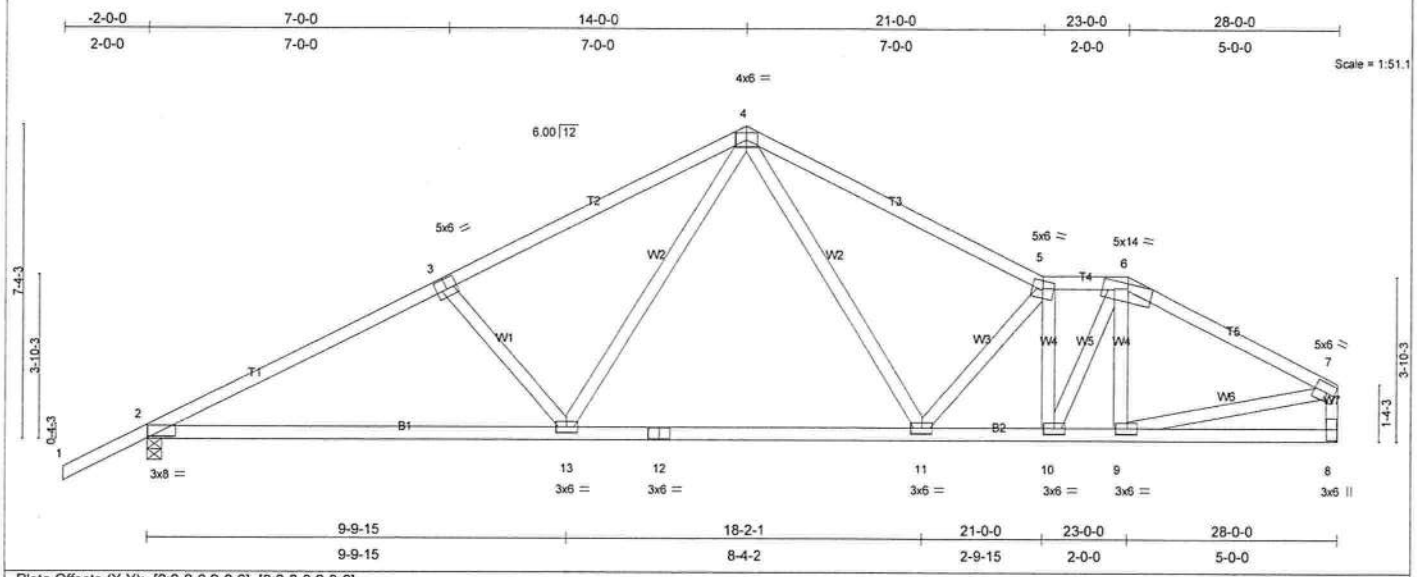

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.24	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.41	2-13	>808	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-14 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1980/628, 3-4=-1753/587, 4-5=-1713/605, 5-6=-1685/626, 6-7=-1564/508, 7-8=-1071/387
BOT CHORD 2-13=-600/1712, 12-13=-299/1156, 11-12=-299/1156, 10-11=-524/1713, 9-10=-382/1333, 8-9=-96/203
WEBS 3-13=-368/306, 4-13=-205/677, 4-11=-224/625, 5-11=-429/298, 5-10=-693/250, 6-10=-285/759, 6-9=-122/98, 7-9=-297/1163

JOINT STRESS INDEX
2 = 0.77, 3 = 0.62, 4 = 0.75, 5 = 0.77, 6 = 0.40, 7 = 0.75, 8 = 0.34, 9 = 0.64, 10 = 0.67, 11 = 0.49, 12 = 0.39 and 13 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071208

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T18	SPECIAL	1	1	Job Reference (optional)

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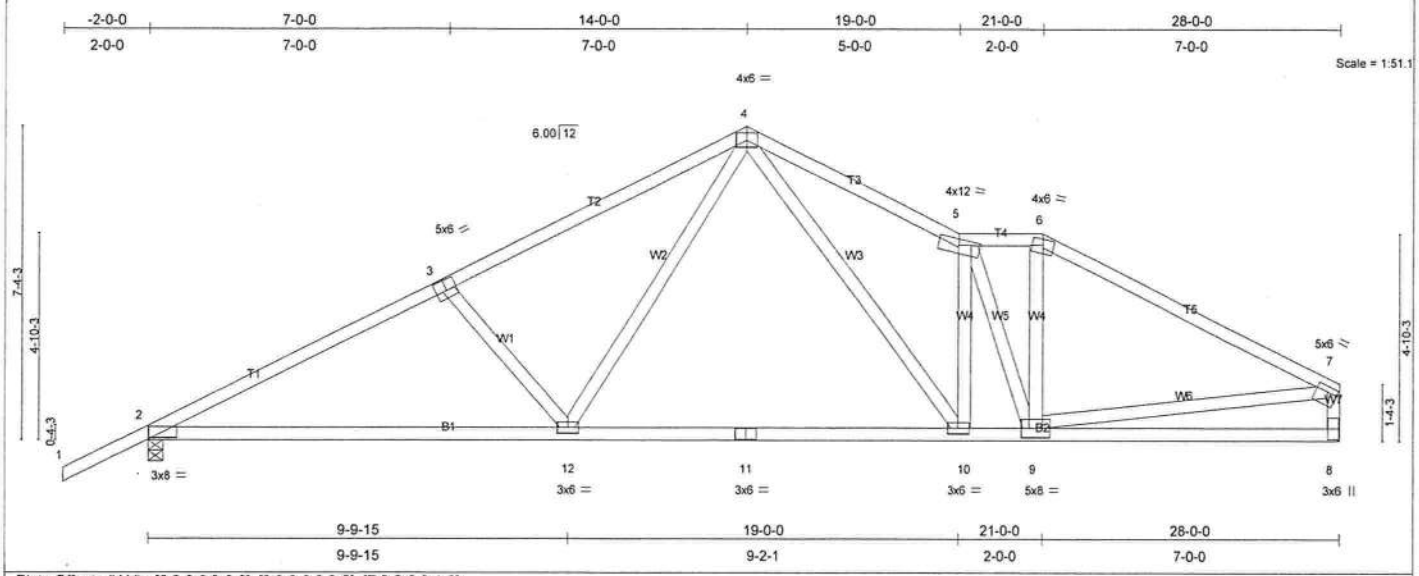

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-3-0], [7:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.23	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.39	2-12	>862	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-9 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1989/630, 3-4=-1758/585, 4-5=-1798/700, 5-6=-1375/533, 6-7=-1612/522, 7-8=-1038/394
BOT CHORD 2-12=-602/1720, 11-12=-297/1148, 10-11=-297/1148, 9-10=-401/1569, 8-9=-162/348
WEBS 3-12=-366/311, 4-12=-204/689, 4-10=-327/783, 5-10=-426/299, 5-9=-513/113, 6-9=-40/376, 7-9=-242/1027

JOINT STRESS INDEX
2 = 0.77, 3 = 0.64, 4 = 0.68, 5 = 0.50, 6 = 0.77, 7 = 0.77, 8 = 0.56, 9 = 0.47, 10 = 0.57, 11 = 0.61 and 12 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071209

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T19	SPECIAL	1	1	Job Reference (optional)

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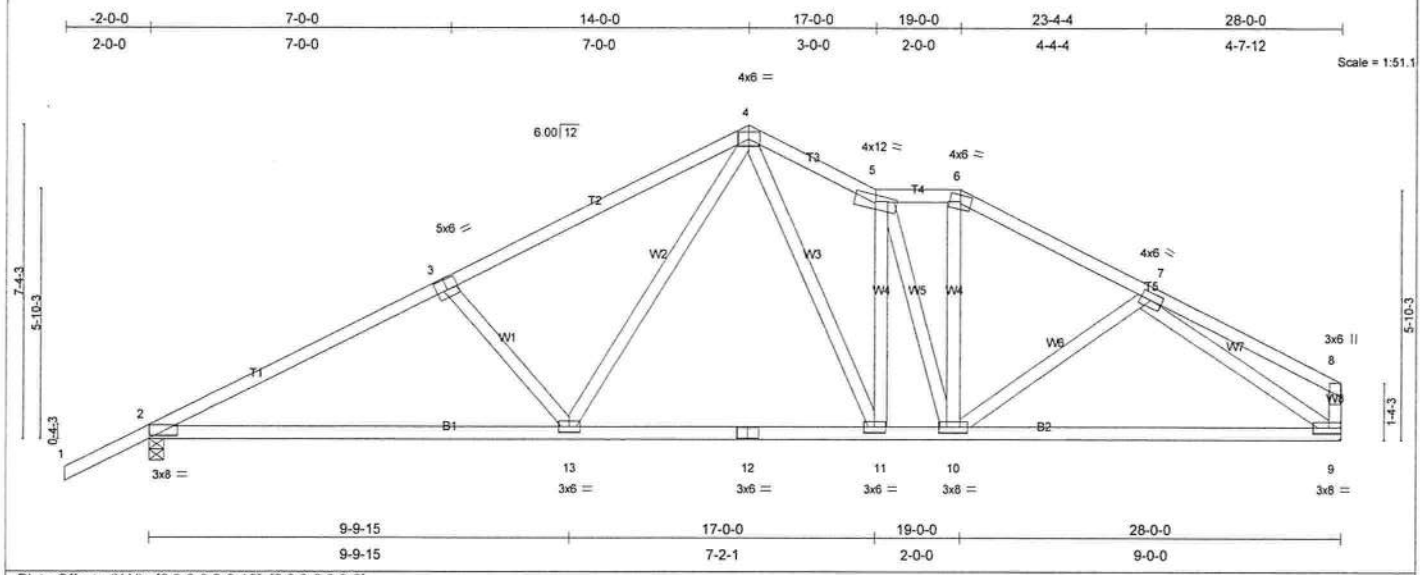

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.24	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.41	2-13	>802	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-1-8 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 9=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 9=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1983/631, 3-4=-1751/586, 4-5=-1547/610, 5-6=-1307/506, 6-7=-1502/515, 7-8=-398/104, 8-9=-283/123
BOT CHORD 2-13=-603/1716, 12-13=-296/1151, 11-12=-296/1151, 10-11=-334/1373, 9-10=-407/1275
WEBS 3-13=-365/310, 4-13=-208/663, 4-11=-261/644, 5-11=-533/244, 5-10=-208/103, 6-10=-79/392, 7-10=-23/132, 7-9=-1217/464

JOINT STRESS INDEX
2 = 0.75, 3 = 0.64, 4 = 0.68, 5 = 0.55, 6 = 0.46, 7 = 0.35, 8 = 0.41, 9 = 0.73, 10 = 0.68, 11 = 0.56, 12 = 0.52 and 13 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 9.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071210

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T20	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:50:58 2007 Page 1

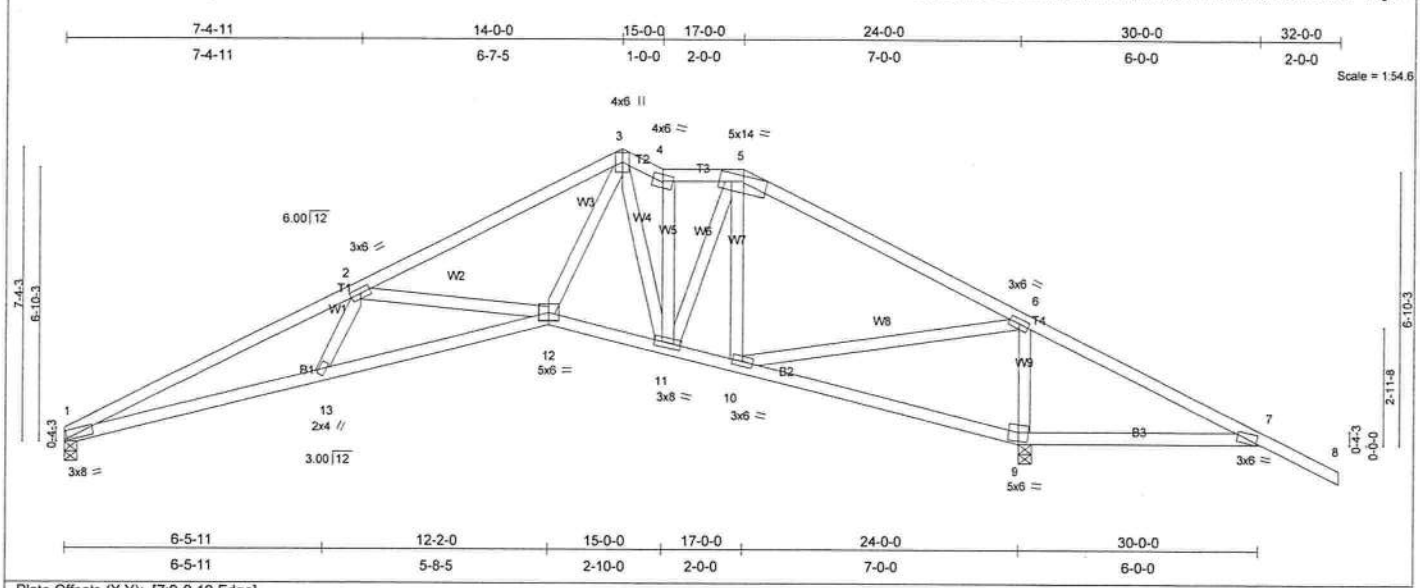

Plate Offsets (X,Y): [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.21	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.33	12-13	>863	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.19	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=906/0-4-0, 9=1708/0-4-0
Max Horz 1=-160(load case 6)
Max Uplift1=-310(load case 5), 9=-837(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2836/927, 2-3=-1785/549, 3-4=-959/333, 4-5=-939/365, 5-6=-953/306, 6-7=-775/769, 7-8=0/47
BOT CHORD 1-13=-836/2547, 12-13=-838/2423, 11-12=-176/1068, 10-11=-87/775, 9-10=-724/870, 7-9=-612/806
WEBS 2-13=0/268, 2-12=-828/515, 3-12=-316/1218, 3-11=-331/201, 4-11=-186/0, 5-11=-131/467, 5-10=-408/295, 6-10=-534/1467, 6-9=-1314/625

JOINT STRESS INDEX
1 = 0.65, 2 = 0.41, 3 = 0.61, 4 = 0.66, 5 = 0.91, 6 = 0.74, 7 = 0.83, 9 = 0.70, 10 = 0.77, 11 = 0.68, 12 = 0.88 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 1 and 837 lb uplift at joint 9.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071211

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T21	SPECIAL	6	1	Job Reference (optional)

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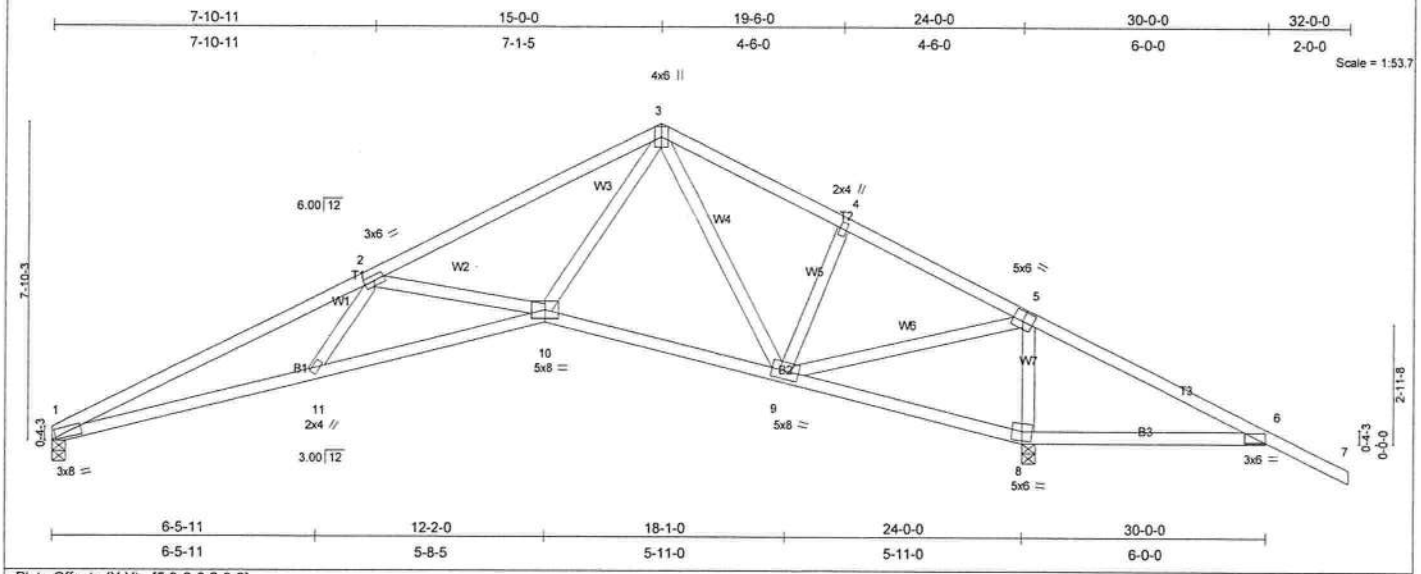

Plate Offsets (X,Y): [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.20	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.32	10-11	>894	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=906/0-4-0, 8=1708/0-4-0
Max Horz 1=-167(load case 6)
Max Uplift1=-313(load case 5), 8=-827(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2822/932, 2-3=-1780/572, 3-4=-739/314, 4-5=-835/290, 5-6=-786/803, 6-7=0/47
BOT CHORD 1-11=-844/2531, 10-11=-843/2358, 9-10=-122/861, 8-9=-718/869, 6-8=-647/821
WEBS 2-11=0/300, 2-10=-791/521, 3-10=-369/1319, 3-9=-429/236, 4-9=-201/175, 5-9=-509/1402, 5-8=-1332/601

JOINT STRESS INDEX
1 = 0.65, 2 = 0.41, 3 = 0.63, 4 = 0.34, 5 = 0.68, 6 = 0.62, 8 = 0.70, 9 = 0.69, 10 = 0.74 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 1 and 827 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071212

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:51:00 2007 Page 1

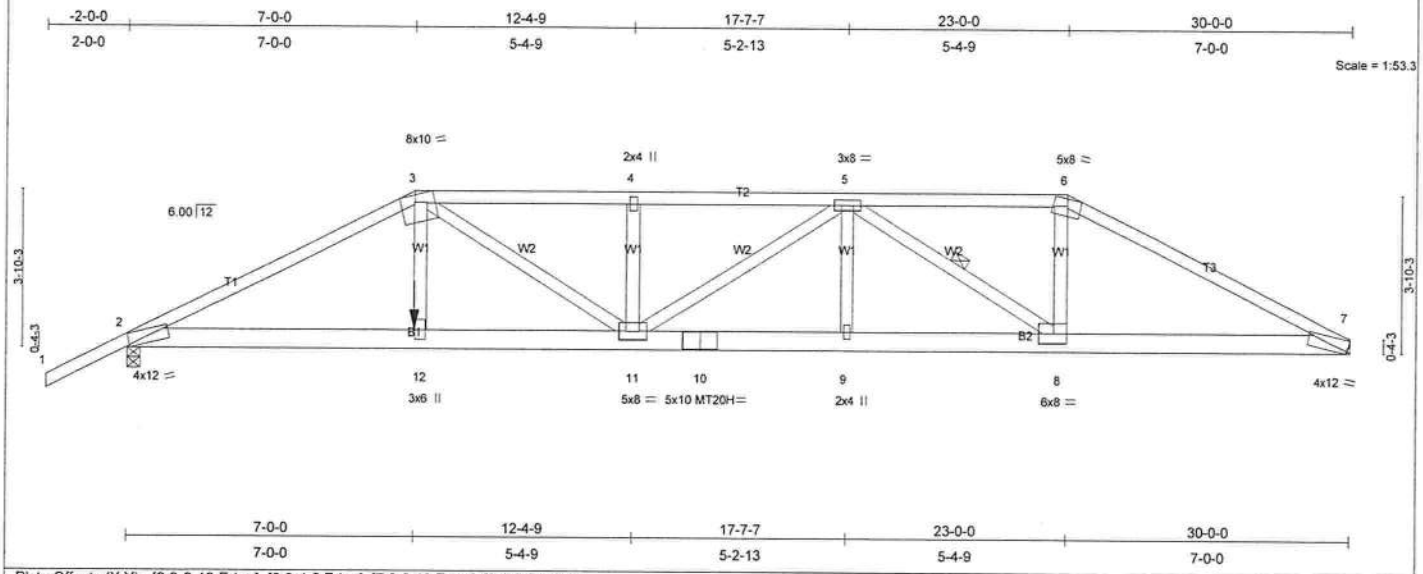

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-4-3,Edge], [7:0-0-13,Edge], [8:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.39	9-11	>920	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.62	9-11	>575	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.
WEBS 1 Row at midpt 5-8

REACTIONS (lb/size) 7=2811/Mechanical, 2=2661/0-4-0
Max Horz 2=116(load case 4)
Max Uplift7=-1064(load case 5), 2=-1128(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5124/2114, 3-4=-5968/2558, 4-5=-5968/2558, 5-6=-4572/1870, 6-7=-5082/2013
BOT CHORD 2-12=-1881/4502, 11-12=-1890/4537, 10-11=-2454/5921, 9-10=-2454/5921, 8-9=-2454/5921, 7-8=-1749/4500
WEBS 3-12=-225/838, 3-11=-823/1800, 4-11=-639/526, 5-11=-67/100, 5-9=0/259, 5-8=-1736/857, 6-8=-586/1727

JOINT STRESS INDEX
2 = 0.77, 3 = 0.95, 4 = 0.34, 5 = 0.83, 6 = 0.96, 7 = 0.75, 8 = 0.48, 9 = 0.34, 10 = 0.84, 11 = 0.83 and 12 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1064 lb uplift at joint 7 and 1128 lb uplift at joint 2.
7) Girder carries tie-in span(s): 7-0-0 from 23-0-0 to 30-0-0
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-118(F=-64), 6-7=-54, 2-12=-30, 8-12=-65(F=-35), 7-8=-135(F=-105)
Concentrated Loads (lb)
Vert: 12=-539(F)

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071213

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Mar 23 06:51:01 2007 Page 1

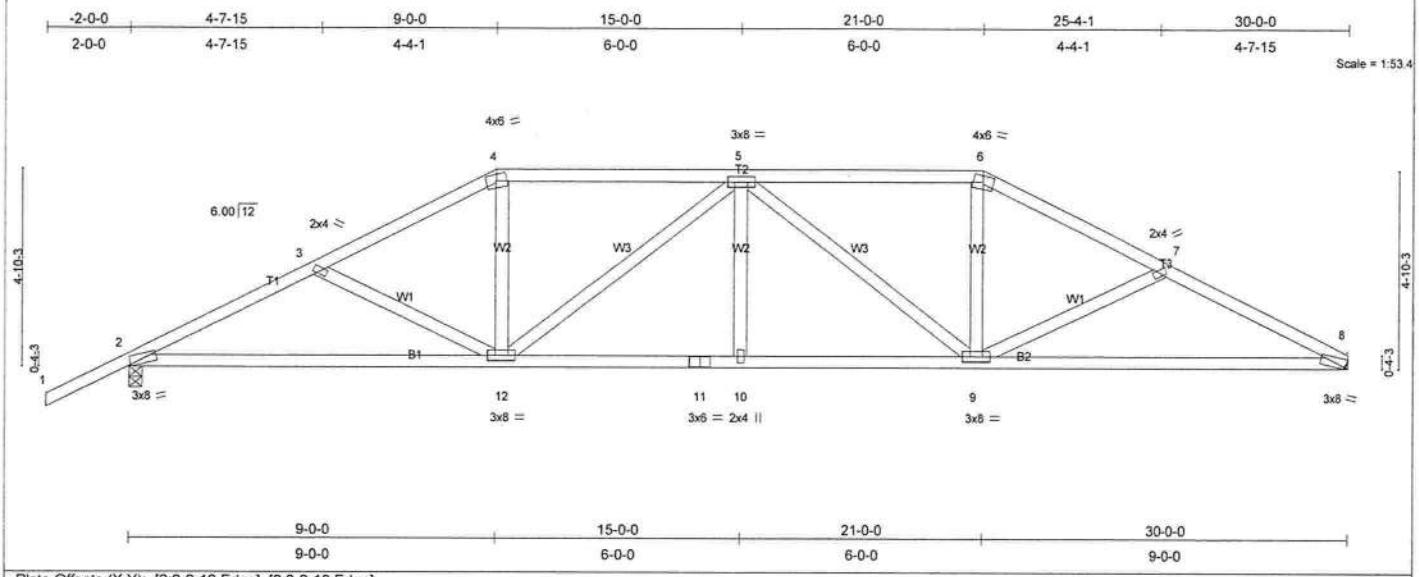

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.23	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.38	8-9	>931	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-8 oc bracing.

REACTIONS (lb/size) 8=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=126(load case 5)
Max Uplift8=-363(load case 6), 2=-491(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2233/628, 3-4=-2004/562, 4-5=-1766/545, 5-6=-1791/570, 6-7=-2034/593, 7-8=-2302/692
BOT CHORD 2-12=-548/1945, 11-12=-559/2071, 10-11=-559/2071, 9-10=-559/2071, 8-9=-557/2025
WEBS 3-12=-225/193, 4-12=-88/584, 5-12=-477/224, 5-10=0/125, 5-9=-455/213, 6-9=-101/604, 7-9=-286/243

JOINT STRESS INDEX
2 = 0.84, 3 = 0.34, 4 = 0.64, 5 = 0.57, 6 = 0.64, 7 = 0.34, 8 = 0.84, 9 = 0.57, 10 = 0.34, 11 = 0.72 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 8 and 491 lb uplift at joint 2.

LOAD CASE(S) Standard

MARCH 23,2007 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071214

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T24	HIP	1	1	Job Reference (optional)

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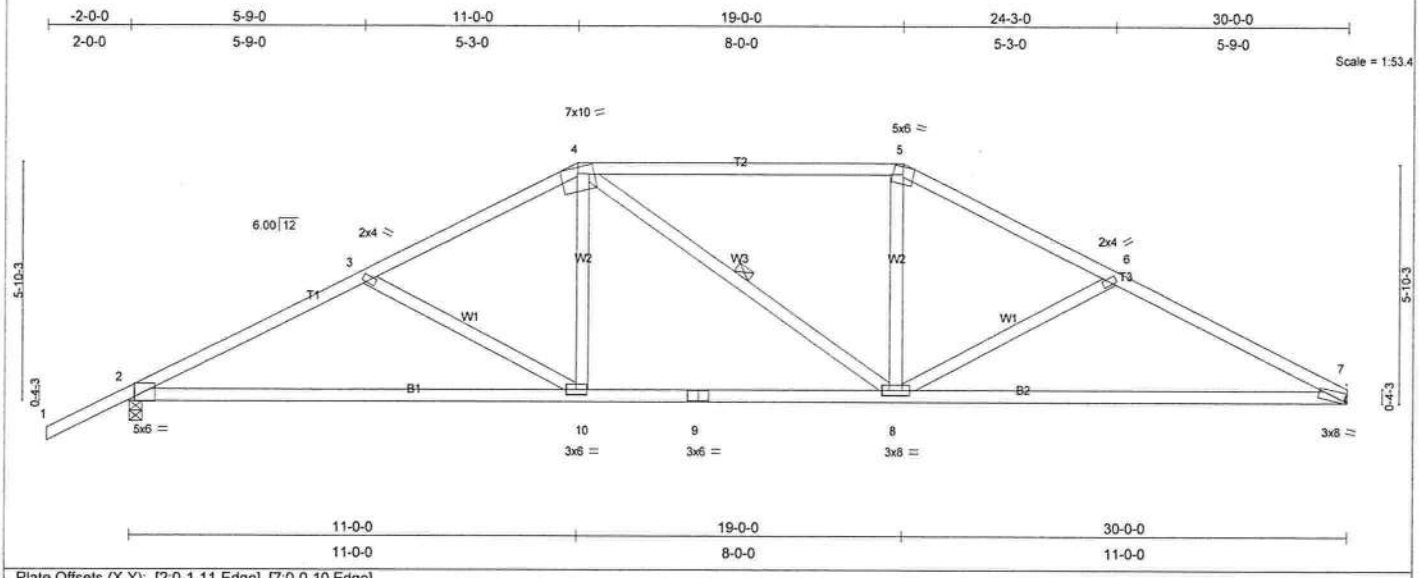

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.44	7-8	>821	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.74	7-8	>484	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 143 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-6 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS (lb/size) 7=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=140(load case 5)
Max Uplift7=-380(load case 6), 2=-508(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2158/653, 3-4=-1862/531, 4-5=-1641/548, 5-6=-1881/554, 6-7=-2207/701
BOT CHORD 2-10=-572/1880, 9-10=-352/1625, 8-9=-352/1625, 7-8=-549/1937
WEBS 3-10=-302/252, 4-10=-59/425, 4-8=-132/161, 5-8=-62/449, 6-8=-350/293

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.75, 5 = 0.71, 6 = 0.34, 7 = 0.91, 8 = 0.57, 9 = 0.60 and 10 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 7 and 508 lb uplift at joint 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0323071215

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 20 TW
L232942	T25	SPECIAL	1	1	Job Reference (optional)

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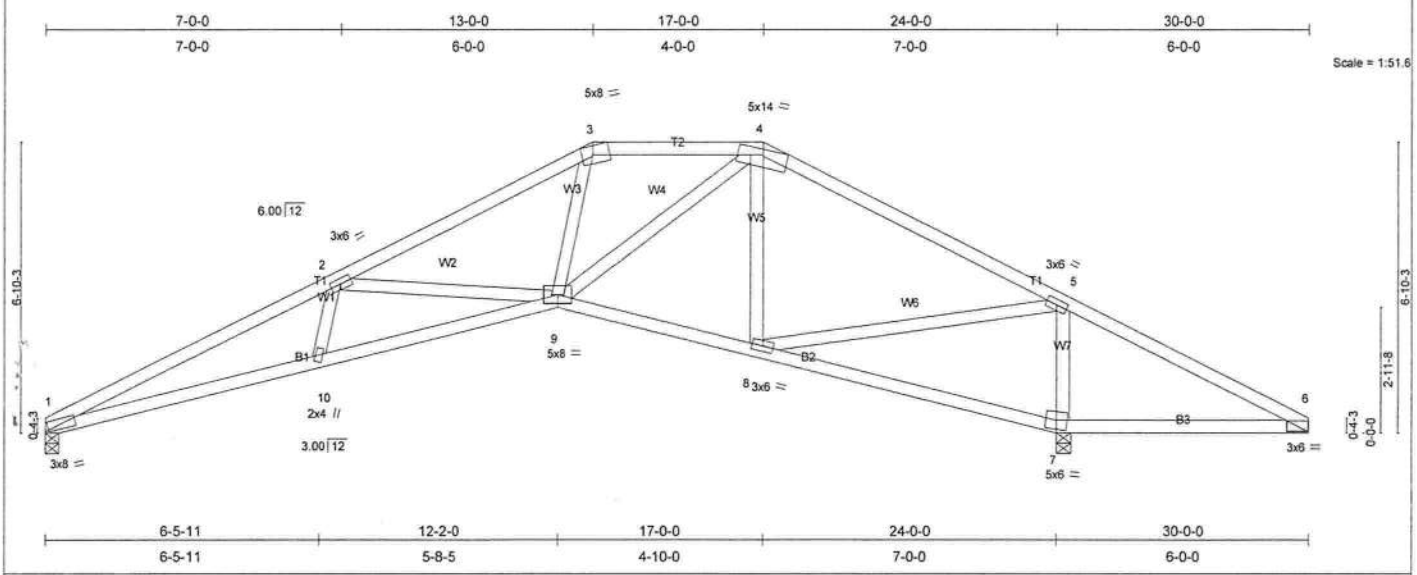

Plate Offsets (X,Y): [3:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.21	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.34	9-10	>839	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(TL)	0.20	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=938/0-4-0, 7=1568/0-4-0
Max Horz 1=96(load case 4)
Max Uplift1=-316(load case 5), 7=-666(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2977/991, 2-3=-1933/594, 3-4=-1519/557, 4-5=-1070/338, 5-6=-377/485
BOT CHORD 1-10=-918/2678, 9-10=-920/2616, 8-9=-138/905, 7-8=-454/414, 6-7=-354/367
WEBS 2-10=0/228, 2-9=-868/517, 3-9=-118/603, 4-9=-271/863, 4-8=-352/203, 5-8=-352/1303, 5-7=-1227/526

JOINT STRESS INDEX
1 = 0.68, 2 = 0.44, 3 = 0.36, 4 = 0.83, 5 = 0.66, 6 = 0.24, 7 = 0.63, 8 = 0.69, 9 = 0.82 and 10 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 316 lb uplift at joint 1 and 666 lb uplift at joint 7.

LOAD CASE(S) Standard

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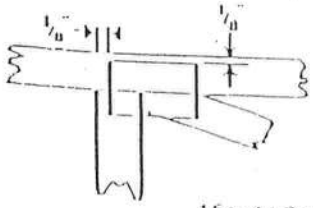
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

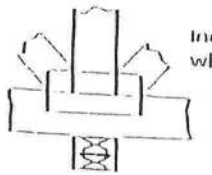
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

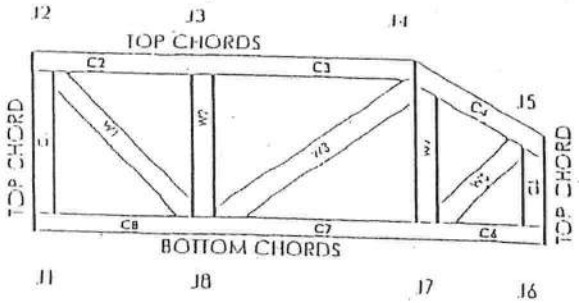
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information must be shown as per section 1603.1.4 FBC* a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☑	☐	**Elevations including:** a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys.
e) Location and size of skylights
f) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned.
b) Shear walls identified.
c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
d) Show safety glazing of glass, where required by code.
e) Identify egress windows in bedrooms, and size.
f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel.

Roof System:

a) Truss package including:
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge beam sized and valley framing and support details
   4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
   1. All materials making up wall
   2. Block size and mortar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. *All required connectors with uplift rating* and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
   6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction (if required)
   8. Fireproofing requirements
   9. Shoe type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
   11. Indicate where pressure treated wood will be placed
   12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☐ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ N/A ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☑ ☐ c)Gas System Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☑ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval: If the project is to be located within the city limits of the Town of Fort White,** prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. **(386) 497-2321**

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is **$50.00**

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit **($25.00)** must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver **($50.00)**. All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

John Norris
APPLICANT SIGNATURE

4/11/07
DATE

R-1305 01-04

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Clopay Building Products Co.
8585 Duke Blvd.
Mason, OH 45040

SCOPE: This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code. This product is approved as described herein, and has been designed to comply with the Florida Building Code including the High Velocity Hurricane Zone.

DESCRIPTION: Sectional Garage Door 16'- 2" Wide.

APPROVAL DOCUMENT: Drawing No. 101300, titled "Double Car Hurricane Pan Door", dated 02/15/95 with last revision on 01/06/04, sheets 1 and 2 of 2, prepared by Clopay Building Products Co, signed and sealed by M. W. Westerfield, P.E., bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

LIMITATION: This approval requires the manufacturer to do testing of all coils used to fabricate door panels under this Notice of Acceptance. A minimum of 2 specimens shall be cut from each coil and tensile tested according to ASTM E-8 by a Dade County approved laboratory selected and paid by the manufacturer. Every 3 months, four times a year, the manufacturer shall mail to this office: a copy of the tested reports with confirmation that the specimen were selected from coils at the manufacturer production facilities. And a notarized statement from the manufacturer that only coils with yield strength of 38000 psi or more shall be used to make door panels for Dade County under this Notice of Acceptance

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA renews NOA # 03-0829.05 and consists of this page, evidence page as well as the approval document mentioned above.

The submitted documentation was reviewed by **Candido E. Font PE.**

03/23/06

PRODUCT CONTROL ACCEPTED

NOA No 05-1212.02
Expiration Date: March 26, 2007
Approval Date: March 23, 2006

Clopay Building Products Co.

NOTICE OF ACCEPTANCE: EVIDENCE PAGE

A. DRAWINGS

1. *Drawing prepared by Clopay Building Products Co., titled "Double Car Hurricane Pan Door", Drawing No. 101300, dated 02/15/95, with last revision on 01/06/2004, sheets 1 through 2 of 2, signed and sealed by M.W. Westerfield, PE.*

B. TESTS

1. *Test report of large missile impact test per PA 201 and cyclic wind pressure test per PA 203 of " Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-408, dated 01/25/95, signed and sealed by H. M. Medina, PE.*
2. *Test report of Uniform Static Air Pressure Test Per PA 202 on " Garage Door", prepared by Hurricane Engineering & Testing, Inc., report No. HETI 95-407, dated 01/24/95, signed and sealed by H. M. Medina, PE.*
3. *Test report of Forced Entry Resistance per section 3603.2(b)5 on "Garage Door" prepared by Hurricane Engineering Testing, Inc. report No. HETI 95-407f, dated 01/25/95, signed and sealed by H. M. Medina, PE.*

C. CALCULATIONS

1. *Calculations dated 01/20/95; pages 1 and 2, prepared by M. W. Westerfield, PE, signed and sealed by M. W. Westerfield, PE.*
2. *Calculations dated 02/24/95, page 1, prepared M.W. Westerfield, PE, signed and sealed by M.W. Westerfield, PE.*

D. MATERIAL CERTIFICATIONS

1. *Test report of Tensile Test per ASTM E 8, report No. HETI 94-T59, prepared by Hurricane Engineering & Testing, Inc., dated 02/06/95, signed and sealed by H.M. Medina, PE.*
2. *Test report of Salt Spray Test per ASTM D1654 & ASTM B117, report No. 9EM-1144, prepared by Q.C. Metallurgical, Inc., dated 06/03/99, signed and sealed by K. Grate.*

E. STATEMENTS.

1. *Affidavit of yield strength compliance prepared by R. D. Shifflett employed by Clopay Building Products Co., notarized on 01/11/2001 by B. H. Schuler.*

F. QUALITY ASSURANCE.

1. *Building Code Compliance Office.*

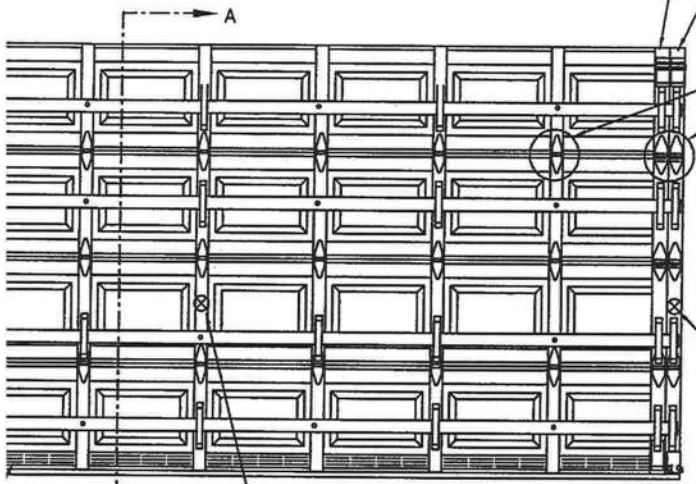
03/23/06

Candido F. Font, PE.
Senior Product Control Division
NOA No 05-1212.02
Expiration Date: March 26, 2007
Approval Date: March 23, 2006

5	8/25/2003	ADDED EXTEN
6	1/6/2004	JAMB ATTACHI

PATENTED TOG-L-LOC SYSTEM (TOP & BOT)
NE ADHESIVE (ALONG CENTER)

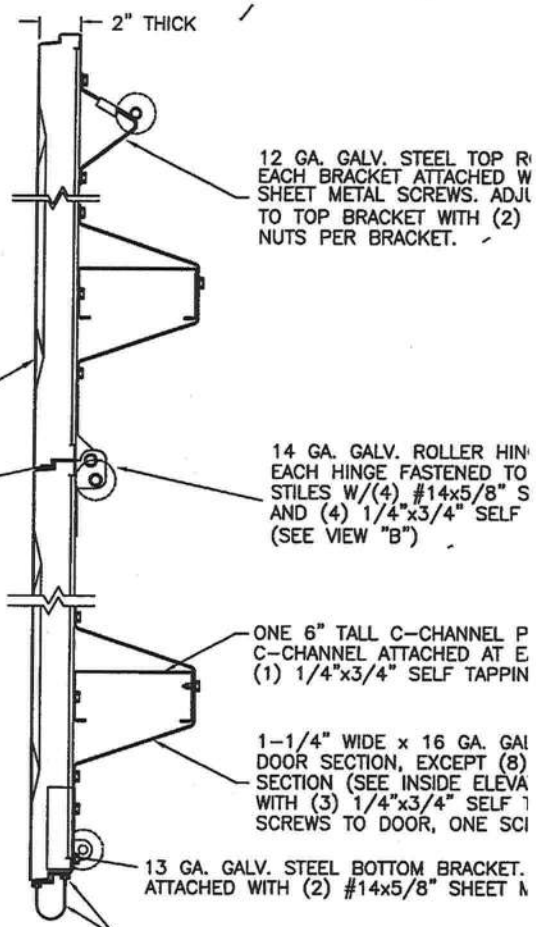
16 GA. PAINTED END STILES ATTACHED TO DOOR SKIN WITH PATENTED TOG-L-LOC SYSTEM (TOP, BOTTOM & CENTER).



LOCK POSITION (BOTH SIDES) TWO POINT LOCKING HAS BEEN TESTED PER REQUIREMENTS OF SECTION 12.1 OF TAS 202. LOCKS HAVE 5/8" MIN. ENGAGEMENT. DOOR TESTED FOR FORCED ENTRY WITH BOTH OUTSIDE KEYED LOCK AND INSIDE SLIDE BOLT LOCK OPTIONS (SEE LAYOUT OF EACH LOCK ON NEXT PAGE).

△1 △2 24 GA. DDS STEEL (MIN. YIELD STRENGTH: 38 KSI) EXTERIOR SKIN WITH G-40 GALVANIZING, BAKED-ON PRIMER AND A BAKED-ON POLYESTER PAINTED TOP COAT APPLIED TO BOTH SIDES OF STEEL SKIN. (ASTM No. A653).

SHIP LAP JOINTS.



SECTION A-A (SIDE VIEW)

HIGH

PANEL, GALV. INTER. STILES
PANEL, PAINTED/GALV. INTER. STILES

3, 84A, 93, 94
GH
ONTAL TRACK SUPPORT BY
DOOR INSTALLER (TO SUIT)
ANCE SYSTEM

L CONTINUOUS
ANGLE.

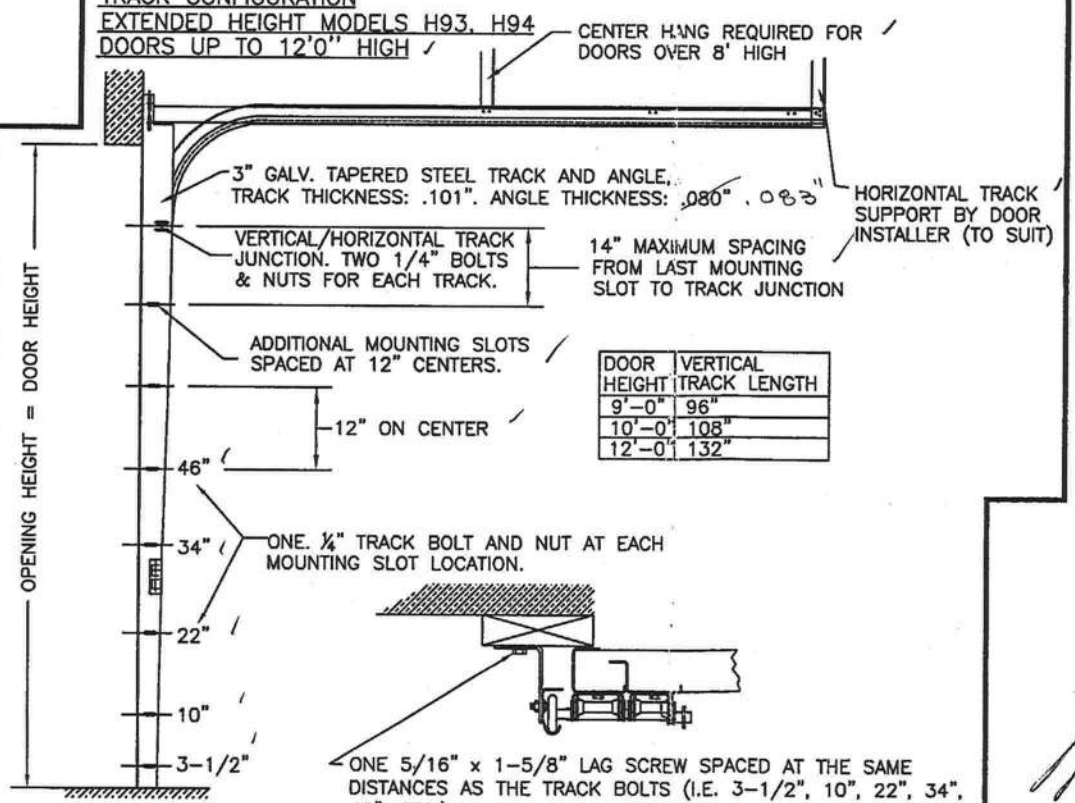
DORS ONLY)

URATION

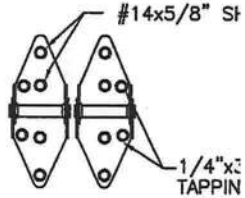
DOOR HEIGHT	"L"
6'-6"	70"
7'-0"	76"
7'-6"	82"
8'-0"	88"

ACK, .083"

TRACK CONFIGURATION
EXTENDED HEIGHT MODELS H93, H94
DOORS UP TO 12'0" HIGH



DOOR HEIGHT	VERTICAL TRACK LENGTH
9'-0"	96"
10'-0"	108"
12'-0"	132"



14 GA. END HINGES
VIEW "B"

1/16/04

DESIGN LOADS: +46.6 P.S.F. & -52.0 P.S.F. (MODELS 83, 84A, 93,
DESIGN LOADS: +46.6 P.S.F. & -51.7 P.S.F. (MODELS H93, H94)

5	8/25/03	ADDED EXTENDE
6	1/6/04	JAMB ATTACHME

JM DESIGN LOAD OF +372.8 LB & −416 LB. PER LINEAR FOOT OF JAMB.
NOT REQUIRED) COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.

BE FRAMED SOLID BY NOT LESS THAN (3) 2x6 PRESSURE TREATED GRADE
ESS GRADE NOT LESS THAN 1200 PSI NOMINAL EXTREME FIBER STRESS
'0" HIGH. STUD WALLS TO BE CONTINUOUS FROM FOOTING TO TIE BEAMS
A BUILDING CODE. (4) 2x6 PRESSURE TREATED GRADE #2 OR BETTER
LESS THAN 1200 PSI NOMINAL EXTREME FIBER STRESS IN BENDING FOR

TE

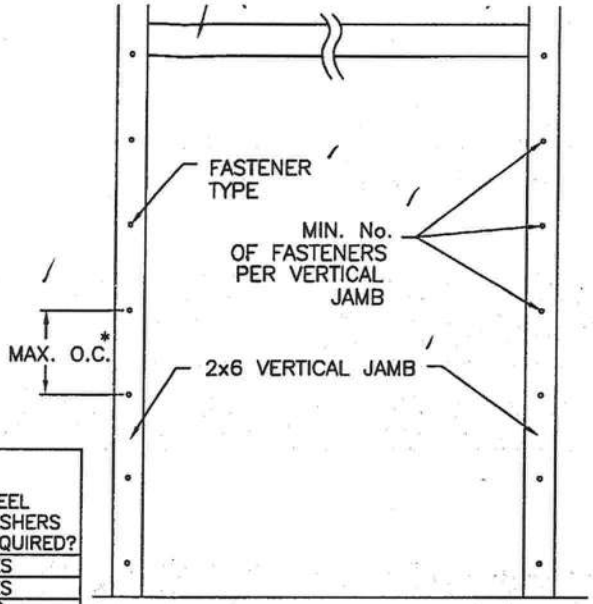
TO GROUT REINFORCED BLOCK WALL OR CONCRETE COLUMN.
WITH CONCRETE AND REINFORCED WITH #5 BAR EXTENDING
IS. ALL BARS SHALL BE CONTINUOUS FROM THE TIE BEAMS
NCRETE COLUMN. BLOCK WALLS AND CONCRETE COLUMNS TO BE
OF RECORD AND IN ACCORDANCE WITH THE FLORIDA BUILDING CODE.

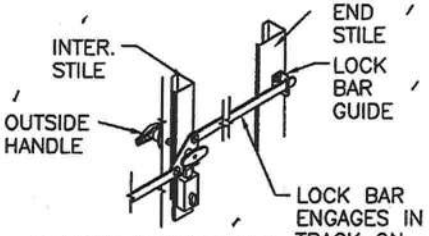
SUPPORTING STRUCTURE ATTACHMENT

ENT OF TRACK ANGLE TO 2x6 VERTICAL JAMBS OR SUPPORTING STRUCTURE)

TYPE	MAXIMUM ON-CENTER DISTANCE BETWEEN FASTENERS*	STEEL WASHERS REQUIRED?
EMBED LAG SCREW (ASTM A307, GRADE A) 1-1/2" EMBED INTO STRUCTURE	16"	YES
/4" MIN. EMBED ELCO TAPCON CONCRETE ANCHOR	10"	YES
/4" MIN. EMBED POWER-STUD EXPANSION ANCHOR (7400 SERIES)	16"	NO
/4" MIN. EMBED POWER LOK/BOLT ANCHOR BOLT (5000 SERIES)	14"	NO

NCHOR AND EDGE OF CONCRETE BLOCK: 3", EXCLUDING STUCCO THICKNESS.
O MORE THAN HALF OF THE MAXIMUM ON-CENTER DISTANCE. HIGHEST ANCHOR INSTALLED AT LEAST AS HIGH AS THE DOOR OPENING.
ENT FASTENERS.
AD HAS been USED IN THE DESIGN OF CONCRETE ANCHORS & WOOD FASTENERS.

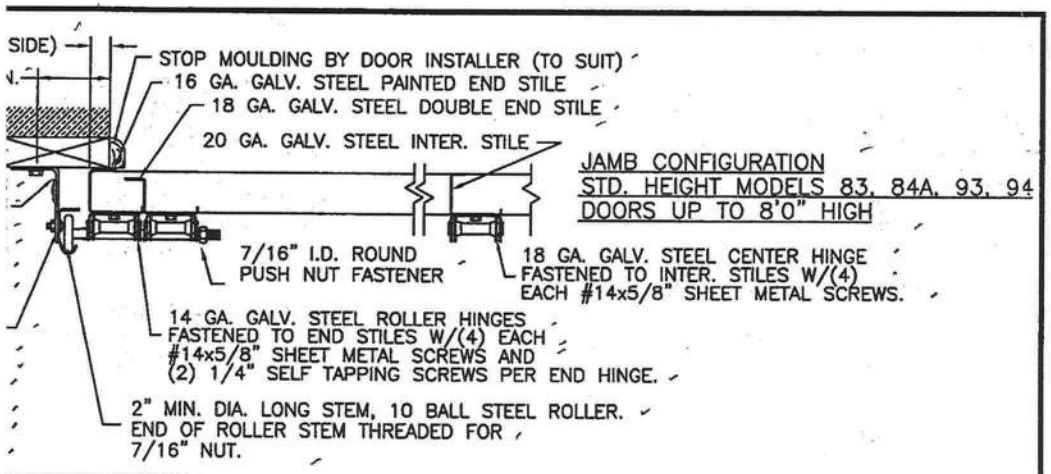




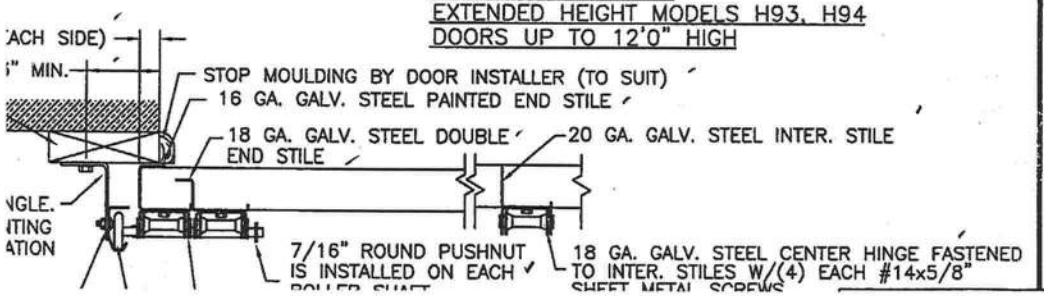
OUTSIDE KEYED LOCK
LOCK BAR LOCKING

INTER.

OUTSIDE KEYE
HANDL



JAMB CONFIGURATION
STD. HEIGHT MODELS 83, 84A, 93, 94
DOORS UP TO 8'0" HIGH



JAMB CONFIGURATION
EXTENDED HEIGHT MODELS H93, H94
DOORS UP TO 12'0" HIGH

JAMB PREPARATION NOTE
EACH CONTINUOUS ANGLE TRACK SHALL BE FASTENED TO
PINE WOOD JAMBS WITH 5/16"x1-5/8" LAG SCREWS (12
7'0" HIGH AND (13) LAG SCREWS PER SIDE UP TO 8'0"
TO 9'0" HIGH, (15) LAG SCREWS PER SIDE UP TO 10'0"
SIDE UP TO 11'0" HIGH, (17) LAG SCREWS PER SIDE U
ATTACHMENT TO THE SUPPORTING STRUCTURE OF THE PRI
SHALL BE APPROVED BY THE PROFESSIONAL OF RECORD
ACCORDANCE WITH CURRENT BUILDING CODES FOR THE LO
PREPARATION OF JAMBS BY OTHERS.

ALL MOUNTING OF TRACK, ANGLES, HORIZONTAL TRACK SU
DOOR HARDWARE TO BE INSTALLED PER CLOPAY INSTALLA
SUPPLIED WITH DOOR SYSTEM UNLESS OTHERWISE NOTED.

1/6/04

DESIGN ENGINEER
MARK W. WESTERFIELD, P.E.
FLORIDA REGISTRATION No. 48495

PRODUCT RENEWED
as complying with the Florida
Building Code
Acceptance No. 05-1212.4
Expiration Date 03/26/0
By
Miami-Dade Product Control
Division

DESIGN LOADS: +46.6 P.S.F. & −52.0 P.S.F. (MODELS 83,
DESIGN LOADS: +46.6 P.S.F. & −51.7 P.S.F. (MODEL H93,

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Tamko Roofing Products, Inc.
P.O. Box 1404
Joplin, MO 64802

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: TAMKO Heritage Declaration & Heritage XL Roof Shingles

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This consists of pages 1 through 4.
The submitted documentation was reviewed by Frank Zuloaga, RRC

PRODUCT CONTROL ACCEPTED

ROOFING ASSEMBLY APPROVAL

Category:	Roofing
Sub-Category:	07310 Composition Shingles
Materials	Dimensional
Deck Type:	Wood

1. SCOPE:

This approves **Tamko Heritage Declaration and Heritage XL** Asphalt Shingles, manufactured by **Tamko Roofing Products, Inc.** as described in this Notice of Acceptance.

2. PRODUCT DESCRIPTION

Product	Dimensions	Test Specifications	Product Description
Heritage Declaration & Heritage XL	12" x 36"	TAS 110	A heavy weight dimensional asphalt shingle.

3. EVIDENCE SUBMITTED:

Test Agency	Test Identifier	Test Name/Report	Date
PRI Asphalt Technologies, Inc.	TAS 100	TAP-066-02-01	01/09/03
		TAP-073-02-01	05/20/03
Underwriters Laboratories, Inc.	ASTM D 3462	R2919	06/12/03
Underwriters Laboratories, Inc.	TAS 107	03CA08442	06/12/03

4. LIMITATIONS

4.1 Fire classification is not part of this acceptance; refer to a current Approved Roofing Materials Directory for fire ratings of this product.

4.2 Shall not be installed on roof mean heights in excess of 33 ft.

4.3 All products listed herein shall have a quality assurance audit in accordance with the Florida Building Code and Rule 9B-72 of the Florida Administrative Code.

5. INSTALLATION

5.1 Shingles shall be installed in accordance with Roofing Application Standard RAS 115.

5.2 The manufacturer shall provide clearly written application instructions.

5.3 Exposure and course layout shall be in compliance with Detail 'A', attached.

5.4 Nailing shall be in compliance with Detail 'B', attached.

6. LABELING

5.1 Shingles shall be labeled with the Miami-Dade Logo or the wording "Miami-Dade County-Product Control Approved".

7. BUILDING PERMIT REQUIREMENTS

7.1 Application for building permit shall be accompanied by copies of the following:

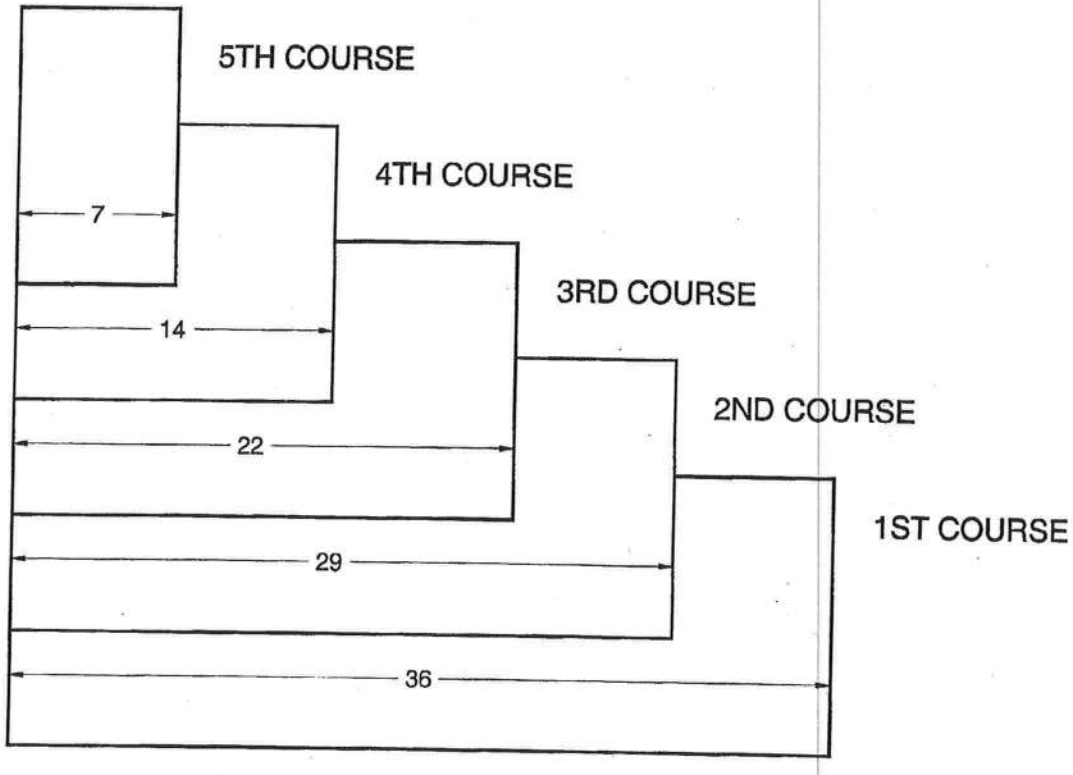
7.1.1 This Notice of Acceptance.

7.1.2 Any other documents required by the Building Official or the applicable Building Code in order to properly evaluate the installation of this system.

DETAIL A

HERITAGE DECLARATION & XL

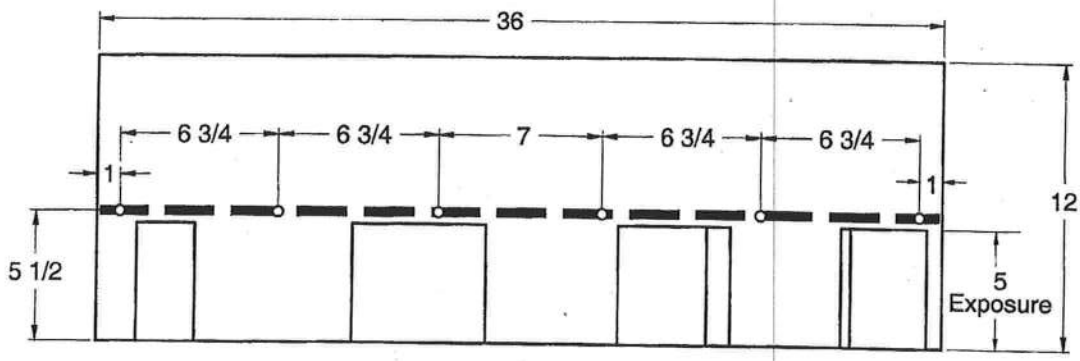
All dimensions are in inches.



DETAIL B

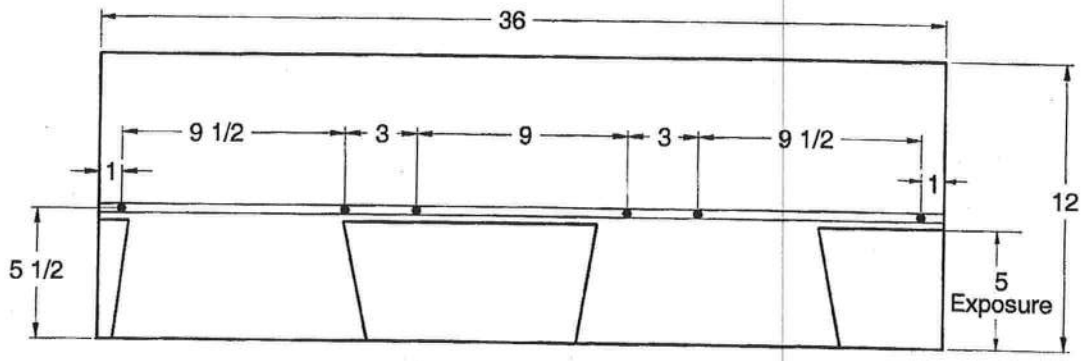
HERITAGE DECLARATION

12" x 36" LAMINATED SHINGLE
All dimensions are in inches.



HERITAGE XL

12" x 36" LAMINATED SHINGLE
All dimensions are in inches.



END OF THIS ACCEPTANCE

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation
108 Mutzfeld Road
Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by the BCCO and accepted by the Building Code and Product Review Committee (BCPRC) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The BCCO (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BCPRC reserves the right to revoke this acceptance, if it is determined by BCCO that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the South Florida Building Code, 1994 Edition for Miami-Dade County or Florida Building Code.

DESCRIPTION: Outswing Glazed Residential Steel Door w\Sidelites

APPROVAL DOCUMENT: Drawing No. S-2003, titled "Therma-Tru Wood edge Outswing", sheets 1 through 6 to 6, prepared by RW Consulting, dated 3/9/01, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA **renews NOA # 00-0207.06** and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by **Raul Rodriguez.**

THERMA-TRU®

"CONSTRUCTION" AND "PREMIUM" SERIES
INSULATED STEEL DOOR WITH WOOD FRAMES.

GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO MEET THE SOUTH FLORIDA BUILDING CODE 1994 EDITION FOR MIAMI-DADE COUNTY.
2. WOOD BUCKS BY OTHERS, MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. MIAMI-DADE APPROVED IMPACT RESISTANT SHUTTERS ARE REQUIRED.
5. DESIGNED PRESSURE RATING SEE TABLE PAGE 1.
6. SIDELITES ARE AN OPTION AND CAN BE IN A SINGLE OR DOUBLE CONFIGURATION.

RESIDENTIAL INSULATED STEEL DOOR
(Common to all frame conditions)

Door Leaf Construction:

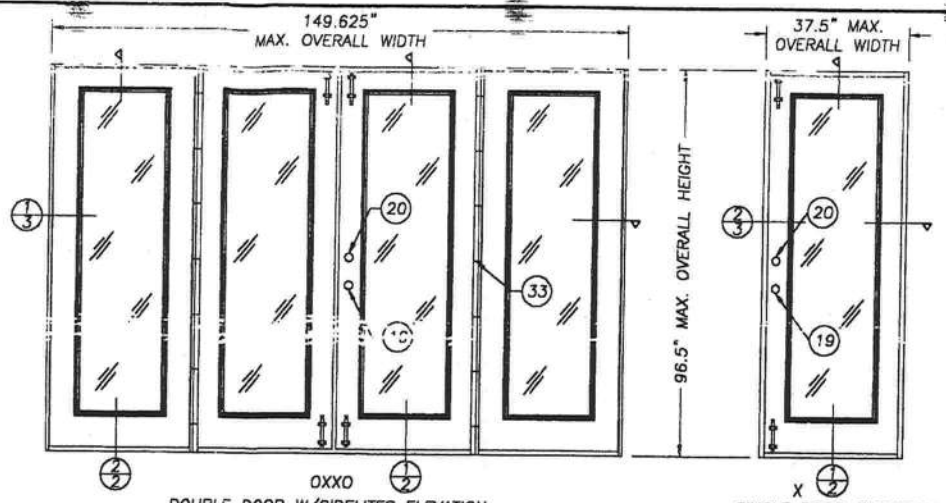
Face sheets: 25 GA.(0.018")minimum thickness, Galvanized steel A-525 commercial quality - AKDQ per ASTM 620 with yield strength Fy(min.)=47,000 psi

Core design: Polyurethane foam core, with1.9 lbs. density by BASF.

Construction: Flush or embossed type. The vertical edges of the skin, are rolled formed to provide a mechanical interlock with finger jointed pine stiles. Wood composite end rails are butt jointed to styles at corners. Panels are sandwich glazed using a two piece PVC lite frame with mitered & welded corners.

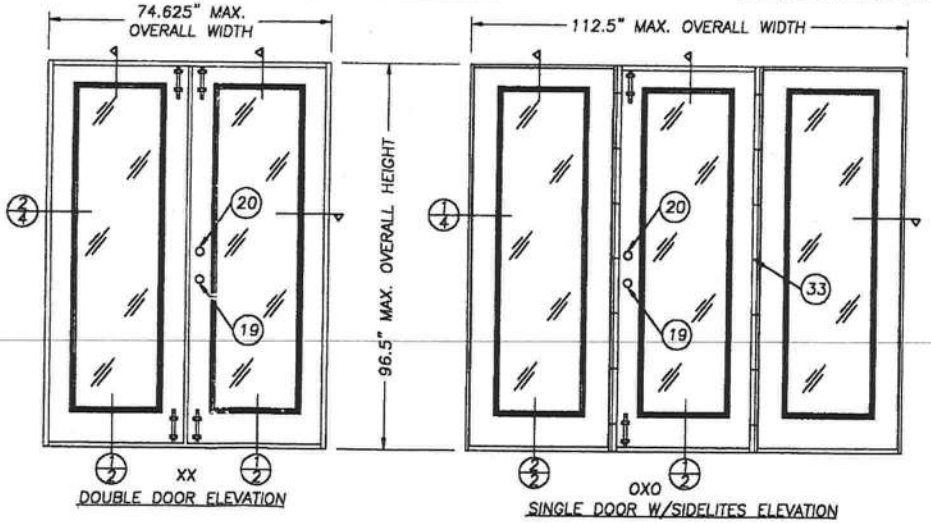
TABLE OF CONTENTS	
SHEET #	DESCRIPTION
1	COMMON (GENERAL NOTES, TYPICAL ELEVATION)
2	VERTICAL CROSS SECTIONS & BILL OF MATERIALS
3	HORIZONTAL CROSS SECTIONS & DOOR MODELS
4	HORIZONTAL CROSS SECTIONS & GLAZING DETAILS
5	ANCHORING LOCATIONS
6	ANCHORING LOCATIONS

DESIGN PRESSURE RATING	
	WHERE WATER INFILTRATION REQUIREMENT IS NEEDED
POSITIVE	+ 48.0 PSF
NEGATIVE	- 51.0 PSF



DOUBLE DOOR W/SIDELITES ELEVATION

SINGLE DOOR ELEVATION



DOUBLE DOOR ELEVATION

SINGLE DOOR W/SIDELITES ELEVATION

ALL DOOR MODELS ARE VIEWED FROM THE INTERIOR SIDE (OUTSWING DOORS)

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0418.01
Expiration Date 04/05/07
By
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE APRIL 05, 2001
BY Isaac J. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0726.06

THERMA TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

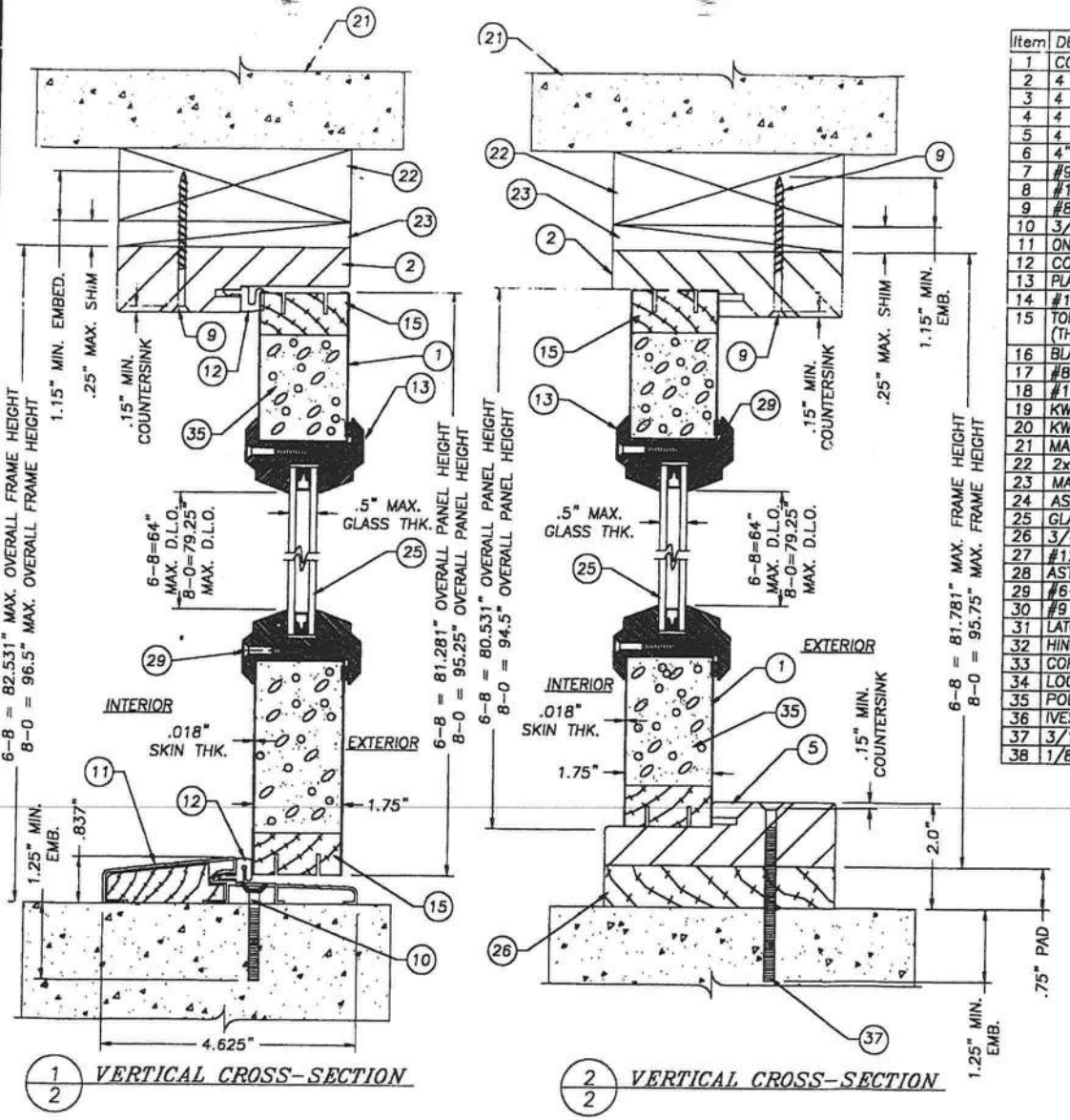
PRODUCT: THRMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES

PART OR ASSEMBLY: ELEVATIONS AND GERERAL NOTES

NO.	DATE	REVISIONS	BY
2	3/09/01	GENERAL REVISION	RW
1	4/11/00	GENERAL REVISION	TJH

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 3/3/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2003
SHEET 1 OF 6



Item	DESCRIPTION	Material
1	CONSTRUC. SERIES DOOR (25GA. .018" MIN.)	STEEL
2	4 1/2" HEADER (THERMA-TRU)	WOOD
3	4 1/2" LATCH JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
4	4 1/2" HINGE JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
5	4 1/2" BLANK JAMB (THERMA-TRU, PONDEROSA PINE)	WOOD
6	4" x 4" HINGE .097" THK. (THERMA-TRU)	STEEL
7	#9 x 3/4" lg. (Hinge to Frame)	STEEL
8	#10 WOOD SCREW X 2 1/2" LG.	STEEL
9	#8 x 2 1/2" LG. WOOD SCREW	STEEL
10	3/16" TAPCON ANCHOR (ELCO, 1.75" MIN. LG.)	STEEL
11	ONE PIECE BUMP FACE THRESHOLD (THERMA-TRU)	ALUM./WOOD
12	COMPRESSION WEATHERSTRIP (THERMA-TRU)	
13	PLASTIC LIP LITE FRAME (PVC, THERMA-TRU)	PVC
14	#10 x 1 1/4" LG. TYPE "A" FLATHEAD	STEEL
15	TOP & BOTTOM RAIL (1.75" x 1.625") (THERMA-TRU, PONDEROSA PINE)	WOOD
16	BLANK SIDE STYLE (THERMA-TRU, PONDEROSA PINE)	WOOD
17	#8 x 1/2" LG. TYPE "AB" PANHEAD	STEEL
18	#10 WOOD SCREW X 2" LG.	STEEL
19	KWIKSET 200 DL PASSAGE	
20	KWIKSET 660 DEADBOLT	
21	MASONRY WALL	
22	2x WOOD BUCK	WOOD
23	MAX. 1/4" SHIM MATERIAL	WOOD
24	ASTRAGAL (.052" WALL THK.)	WOOD/ALUM.
25	GLAZING, 1/2" INSULATED TEMPERED GLASS	GLASS
26	3/4" THK. PRESSURE TREATED SIDELITE PAD	WOOD
27	#12 x 1/2"LG. PANHEAD SHEET METAL SCREW	STEEL
28	ASTRAGAL WEATHERSRIP	VINYL
29	#6-18 x 1 3/4" PHILLIPS FLATHEAD SCREW	STEEL
30	#9 x 1" LG. PHILLIPS FLATHEAD SCREW	STEEL
31	LATCH SIDE STYLE (THERMA-TRU, PONDEROSA PINE)	WOOD
32	HINGE SIDE STYLE (THERMA-TRU, PONDEROSA PINE)	WOOD
33	CORRUGATED STAPLE FASTENER (1 1/2" X 3/4")	STEEL
34	LOCK BLOCK (4" x 11" x 1.625")	WOOD
35	POLYURETHANE FOAM (BASF, 1.9lbs. DENSITY)	FOAM
36	IVES SURFACE BOLT (.25" STEEL)	STEEL
37	3/16" TAPCON ANCHOR (ELCO, 3.25" MIN. LG.)	STEEL
38	1/8 THK. CELLULAR GLAZING TAPE (STIK-II TAPE)	—

NOTE:
SIDELITE IS DIRECT SET INTO JAMB WITH #10 x 2" PH.F.H. WOOD SCREWS AT 6" FROM EACH END AND A MAX. OF 12" O.C. ON VERTICAL LEG JAMBS ONLY.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0418.01
Expiration Date 04.05.07
By Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE APRIL 05, 2001
BY Ishaq I. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0207.06

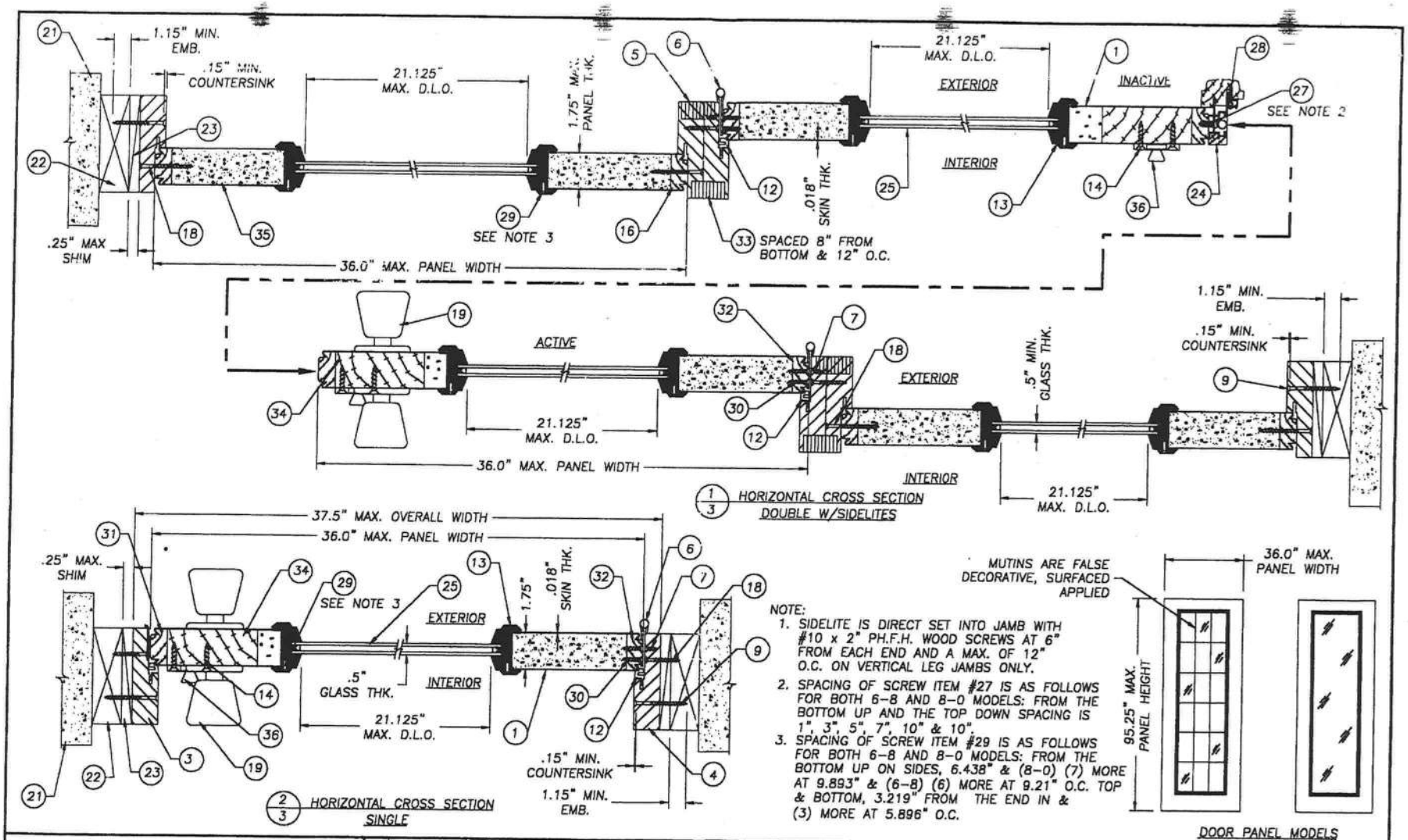
THERMA-TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

PRODUCT: THERMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES
PART OR ASSEMBLY: VERTICAL CROSS SECTIONS & BILL OF MATERIALS

NO.	DATE	REVISIONS	BY
2	3/09/01	GENERAL REVISION	RW
1	4/11/00	GENERAL REVISION	TJH

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 3/3/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2003
SHEET 2 OF 6



NOTE:

1. SIDELITE IS DIRECT SET INTO JAMB WITH #10 x 2" PH.F.H. WOOD SCREWS AT 6" FROM EACH END AND A MAX. OF 12" O.C. ON VERTICAL LEG JAMBS ONLY.
2. SPACING OF SCREW ITEM #27 IS AS FOLLOWS FOR BOTH 6-8 AND 8-0 MODELS: FROM THE BOTTOM UP AND THE TOP DOWN SPACING IS 1", 3", 5", 7", 10" & 10".
3. SPACING OF SCREW ITEM #29 IS AS FOLLOWS FOR BOTH 6-8 AND 8-0 MODELS: FROM THE BOTTOM UP ON SIDES, 6.438" & (8-0) (7) MORE AT 9.893" & (6-8) (6) MORE AT 9.21" O.C. TOP & BOTTOM, 3.219" FROM THE END IN & (3) MORE AT 5.896" O.C.

THERMA TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

NO.	DATE	REVISIONS	BY
2	3/09/01	GENERAL REVISION	RW
1	4/11/00	GENERAL REVISION	TJH

PRODUCT: THERMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES

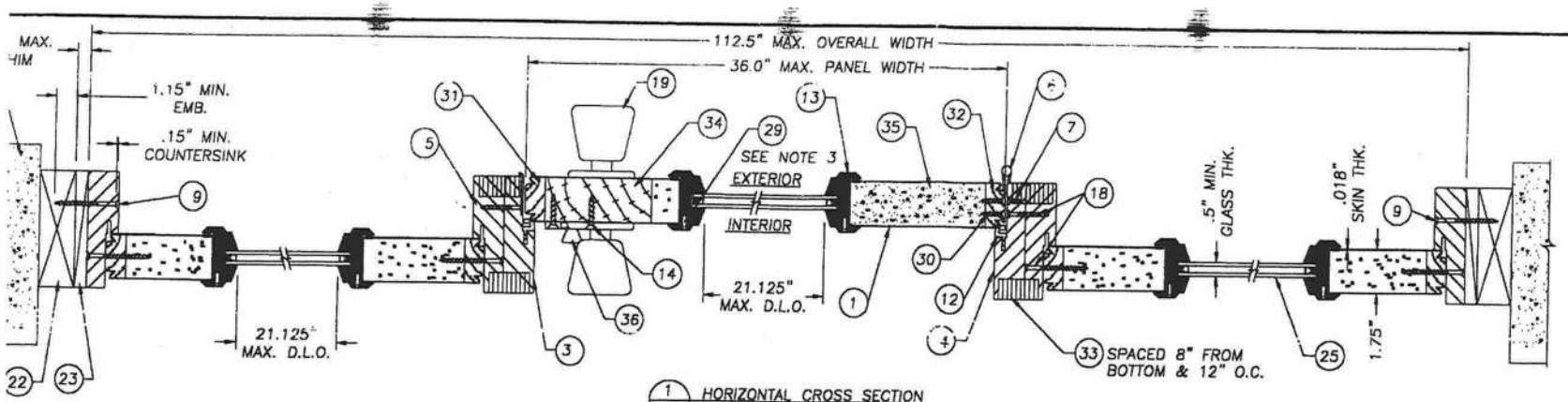
PART OR ASSEMBLY: HORIZONTAL CROSS SECTIONS & DOOR MODELS

RW BUILDING CONSULTANTS, INC. 813.684.3831

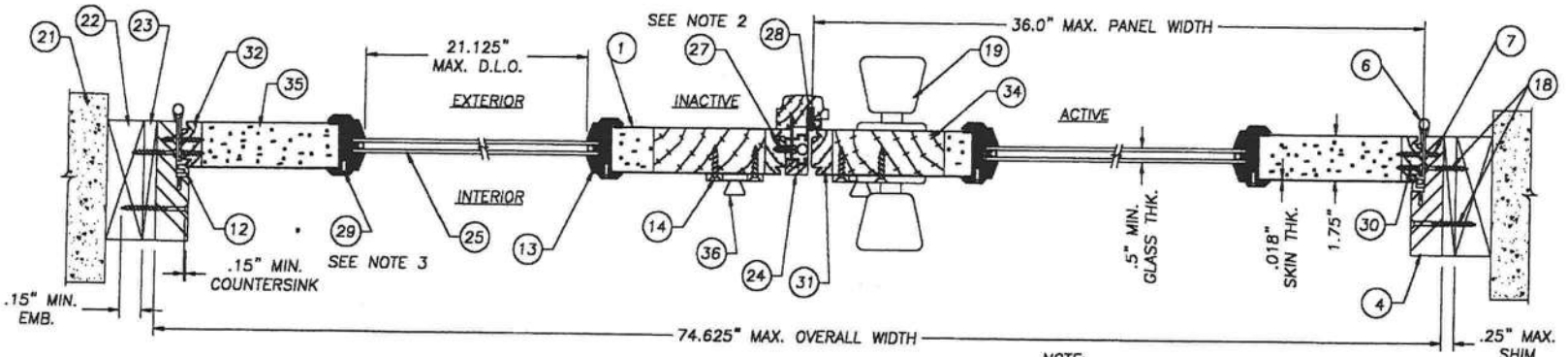
PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0415.01
Expiration Date 04/05/07
By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE APRIL 03, 2001
BY Ishaq I. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0207.06

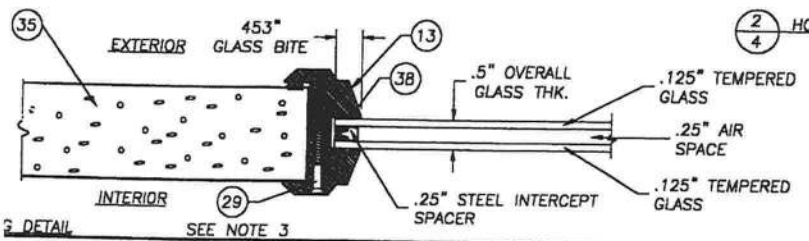
DATE:	3/3/00
SCALE:	N.T.S.
DWG. BY:	TJH
CHK. BY:	RW
DRAWING NO.:	S-2003
SHEET	3 OF 6



1/4 HORIZONTAL CROSS SECTION
SINGLE W/SIDELITES



2/4 HORIZONTAL CROSS SECTION
DOUBLE



3 DETAIL

NOTE:
1. SIDELITE IS DIRECT SET INTO JAMB WITH #10 x 2" PH.F.H. WOOD SCREWS AT 6" FROM EACH END AND A MAX. OF 12" O.C. ON VERTICAL LEG JAMBS ONLY.
2. SPACING OF SCREW ITEM #27 IS AS FOLLOWS FOR BOTH 6-8 AND 8-0 MODELS: FROM THE BOTTOM UP AND THE TOP DOWN SPACING IS 1", 3", 5", 7", 10" & 10".
3. SPACING OF SCREW ITEM #29 IS AS FOLLOWS FOR BOTH 6-8 AND 8-0 MODELS: FROM THE BOTTOM UP ON SIDES, 6.438" & (8-0) (7) MORE AT 9.893" & (6-8) (6) MORE AT 9.21" O.C. TOP & BOTTOM, 3.219" FROM THE END IN & (3) MORE AT 5.896" O.C.

THERMA TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

NO.	DATE		BY
2	3/09/01	GENERAL REVISION	RW
1	4/11/00	GENERAL REVISION	TJH

REVISIONS

PRODUCT:
THERMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES

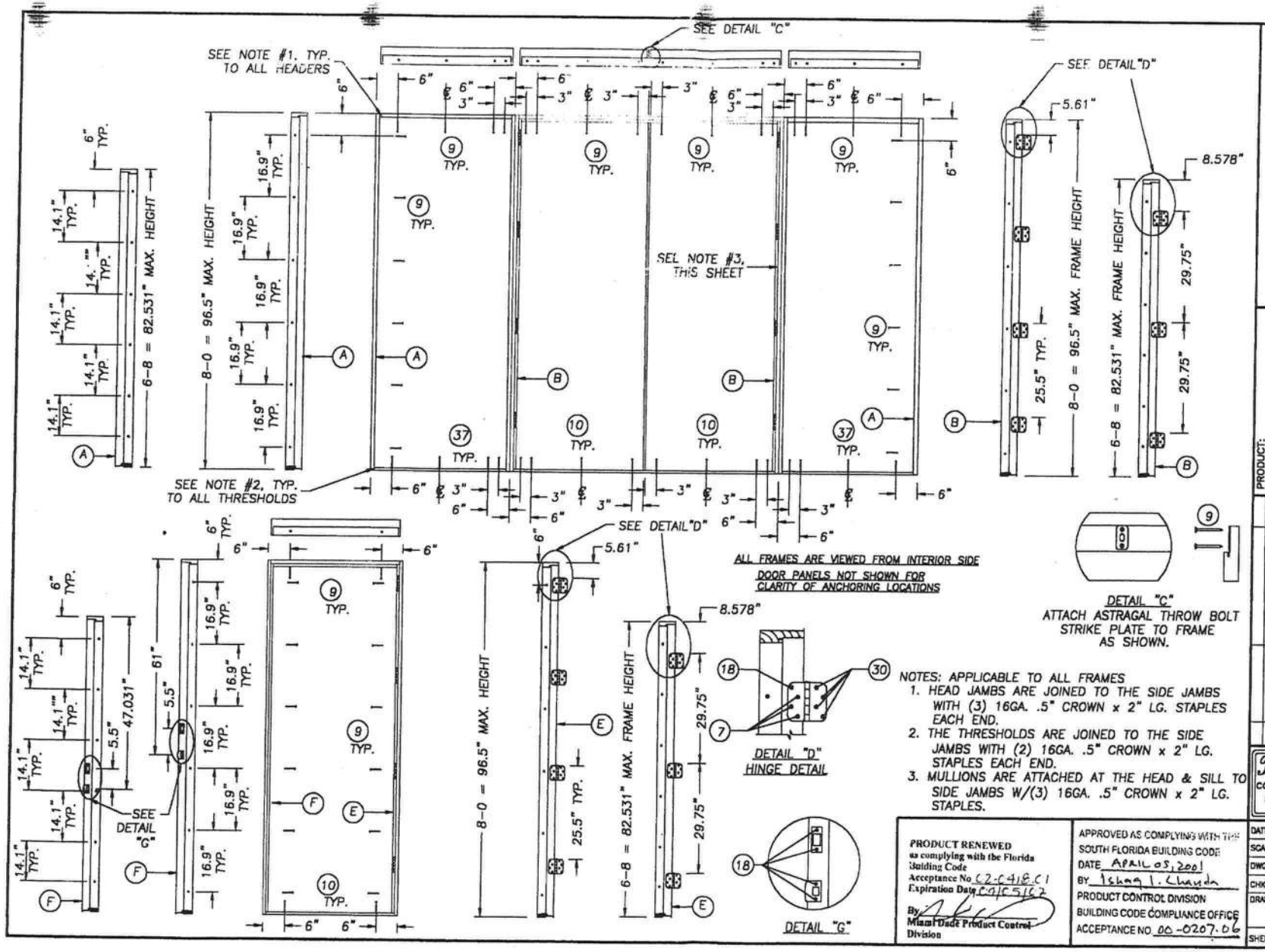
PART OR ASSEMBLY:
HORIZONTAL CROSS SECTIONS & GLAZING DETAIL

RW
BUILDING CONSULTANTS, INC.
813.684.3831

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0410.01
Expiration Date 6/05/07
By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE APRIL 05, 2001
BY Ishaq I. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO 00-0207.06

DATE: 3/3/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2003
SHEET 4 OF 6



NOTES: APPLICABLE TO ALL FRAMES

1. HEAD JAMBS ARE JOINED TO THE SIDE JAMBS WITH (3) 16GA. .5" CROWN x 2" LG. STAPLES EACH END.
2. THE THRESHOLDS ARE JOINED TO THE SIDE JAMBS WITH (2) 16GA. .5" CROWN x 2" LG. STAPLES EACH END.
3. MULLIONS ARE ATTACHED AT THE HEAD & SILL TO SIDE JAMBS W/(3) 16GA. .5" CROWN x 2" LG. STAPLES.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0418.01
Expiration Date 04/05/07
By Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE APRIL 03, 2001
BY Ishaq I. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0207.06

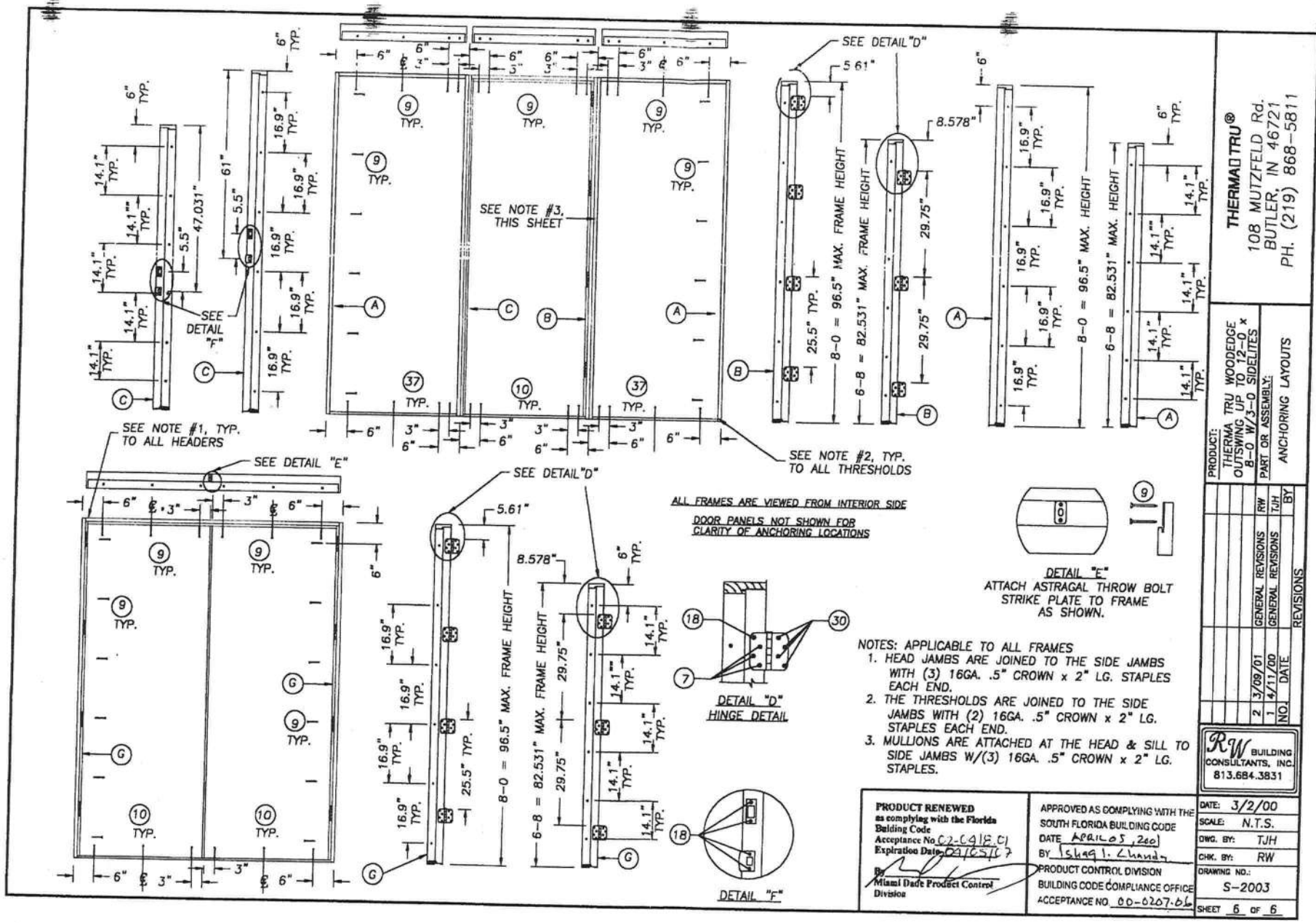
THERMA TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

PRODUCT: THERMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES
PART OR ASSEMBLY: ANCHORING LAYOUTS

NO.	DATE	REVISIONS	BY
1	4/11/00	GENERAL REVISION	TJH
2	3/09/01	GENERAL REVISION	RW

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 3/2/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2003
SHEET 5 OF 6



NOTES: APPLICABLE TO ALL FRAMES

1. HEAD JAMBS ARE JOINED TO THE SIDE JAMBS WITH (3) 16GA. .5" CROWN x 2" LG. STAPLES EACH END.
2. THE THRESHOLDS ARE JOINED TO THE SIDE JAMBS WITH (2) 16GA. .5" CROWN x 2" LG. STAPLES EACH END.
3. MULLIONS ARE ATTACHED AT THE HEAD & SILL TO SIDE JAMBS W/(3) 16GA. .5" CROWN x 2" LG. STAPLES.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 02-0416.01
Expiration Date: 09/05/07
By: Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: April 05, 2001
BY: Ishaq I. Chanda
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0207.06

THERMA TRU®
108 MUTZFELD Rd.
BUTLER, IN 46721
PH. (219) 868-5811

PRODUCT: THERMA TRU WOODEDGE OUTSWING UP TO 12-0 x 8-0 W/3-0 SIDELITES

PART OR ASSEMBLY: ANCHORING LAYOUTS

NO.	DATE	BY	REVISIONS
2	3/09/01	RW	GENERAL REVISIONS
1	4/11/00	TJH	GENERAL REVISIONS

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 3/2/00
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2003
SHEET 6 OF 6

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Mi Home Products, Inc.
650 West Market Street
Gratz, PA 17030

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.

DESCRIPTION: Series "Betterbilt D485/D3485" Aluminum Sliding Patio Door

APPROVAL DOCUMENT: Drawing No. **S-2425,** titled "Non-Impact Aluminum Sliding Patio Door Up to 6'0 x 6'8", sheets 1 through 5 of 5, prepared by R.W. Building Consultants, Inc., dated 12/18/03, signed and sealed by Lyndon F. Schmidt, P.E., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 and evidence page E-1, as well as approval document mentioned above.

The submitted documentation was reviewed by **Theodore Berman, P.E.**

2/13/2004

PRODUCT CONTROL ACCEPTED

NOA No 03-1224.01
Expiration Date: March 04, 2009
Approval Date: March 04, 2004
Page 1

Mi Home Products, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

A. DRAWINGS

1. Manufacturer's die drawings and sections.

Drawing No. **S-2425,** titled "Non-Impact Aluminum Sliding Patio Door Up to 6'0 x 6'8", sheets 1 through 5 of 5, prepared by R.W. Building Consultants, Inc., dated 12/18/03, signed and sealed by Lyndon F. Schmidt, P.E.

B. TESTS

1. Test reports on 1) Air Infiltration Test, per FBC, TAS 202-94
   2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
   3) Water Resistance Test, per FBC, TAS 202-94
   4) Forced Entry Test, per FBC 2411.3.2.1 and TAS 202-94

   along with marked-up drawings and installation diagram of an aluminum patio door, prepared by Architectural Testing, Test Report No. **ATI-03064** dated 12/17/03, signed and sealed by Steven M. Urich, P.E.

C. CALCULATIONS

1. Anchor Calculations, ASTM-E1300-98, and structural analysis, prepared by R.W. Building Consultants, Inc., dated 12/22/03, signed and sealed by Lyndon Schmidt, P.E.
2. Revised Anchor Calculations and structural analysis, prepared by R.W. Building Consultants, Inc., dated 02/10/03, signed and sealed by Lyndon Schmidt, P.E.

D. QUALITY ASSURANCE

1. Miami Dade Building Code Compliance Office (BCCO).

E. MATERIAL CERTIFICATIONS

1. None.

F. STATEMENTS

1. Statement letter of compliance and of no financial interest, dated 12/18/03, signed and sealed by Lyndon F. Schmidt, P.E.
2. Letter from MI Home Products, Inc., dated 11/08/03, stating that they have no financial interest with the laboratory that performed the testing of their products, singed by Stu White.

G. OTHER

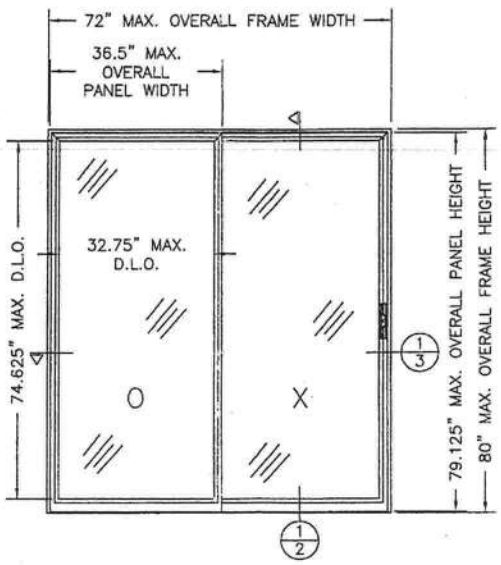
1. Letter from the consultant stating that the product is in compliance with the Florida Building Code.

Theodore Berman, P.E.
Deputy Director, Product Control Division
NOA No 03-1224.01
Expiration Date: March 04, 2009
Approval Date: March 04, 2004

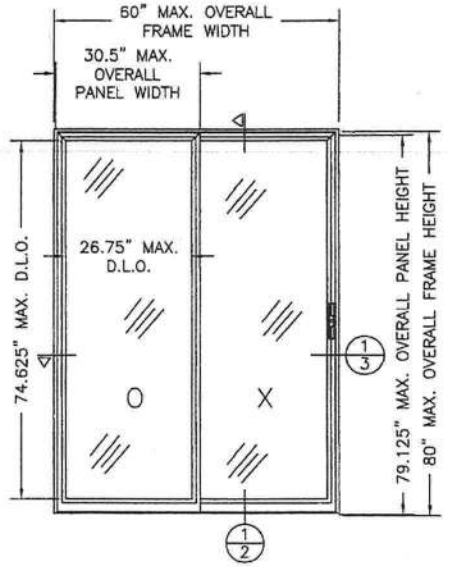
MI HOME PRODUCTS

650 WEST MARKET STREET • GRATZ, PA • 17030-0370

SERIES BETTERBILT D485/D3485
ALUMINUM SLIDING PATIO DOOR



6'0 x 6'8 PATIO DOOR



5'0 x 6'8 PATIO DOOR

ALL ELEVATIONS ARE VIEWED FROM EXTERIOR

GENERAL NOTES:
1. THIS PRODUCT IS DESIGNED TO COMPLY WITH THE HVHZ FLORIDA BUILDING CODE.
2. WOOD BUCKS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE AND TO BE REVIEWED BY BUILDING OFFICIAL.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. FOR DESIGN PRESSURE RATING SEE TABLE THIS SHEET.
5. INSTALLATION OF THIS SYSTEM IN HVHZ AREA REQUIRES THE USE OF APPROVED SHUTTER/EXTERNAL PROTECTION DEVICE COMPLYING WITH HVHZ REQUIREMENTS; INSTALLATION OF THIS SYSTEM OUTSIDE OF HVHZ SHALL MEET THE APPLICABLE CODE REQUIREMENTS FOR WINDBORNE DEBRIS PROTECTION.
6. THIS PRODUCT MEETS WATER REQUIREMENTS FOR HIGH VELOCITY HURRICANE ZONES.

TABLE OF CONTENTS

DESIGN PRESSURE RATING
+57.52 PSF -74.0 PSF

Approved as complying with the Florida Building Code
Date: 03/04/04
NOA# 03-1224.01
Miami Dade Product Control
Division
By

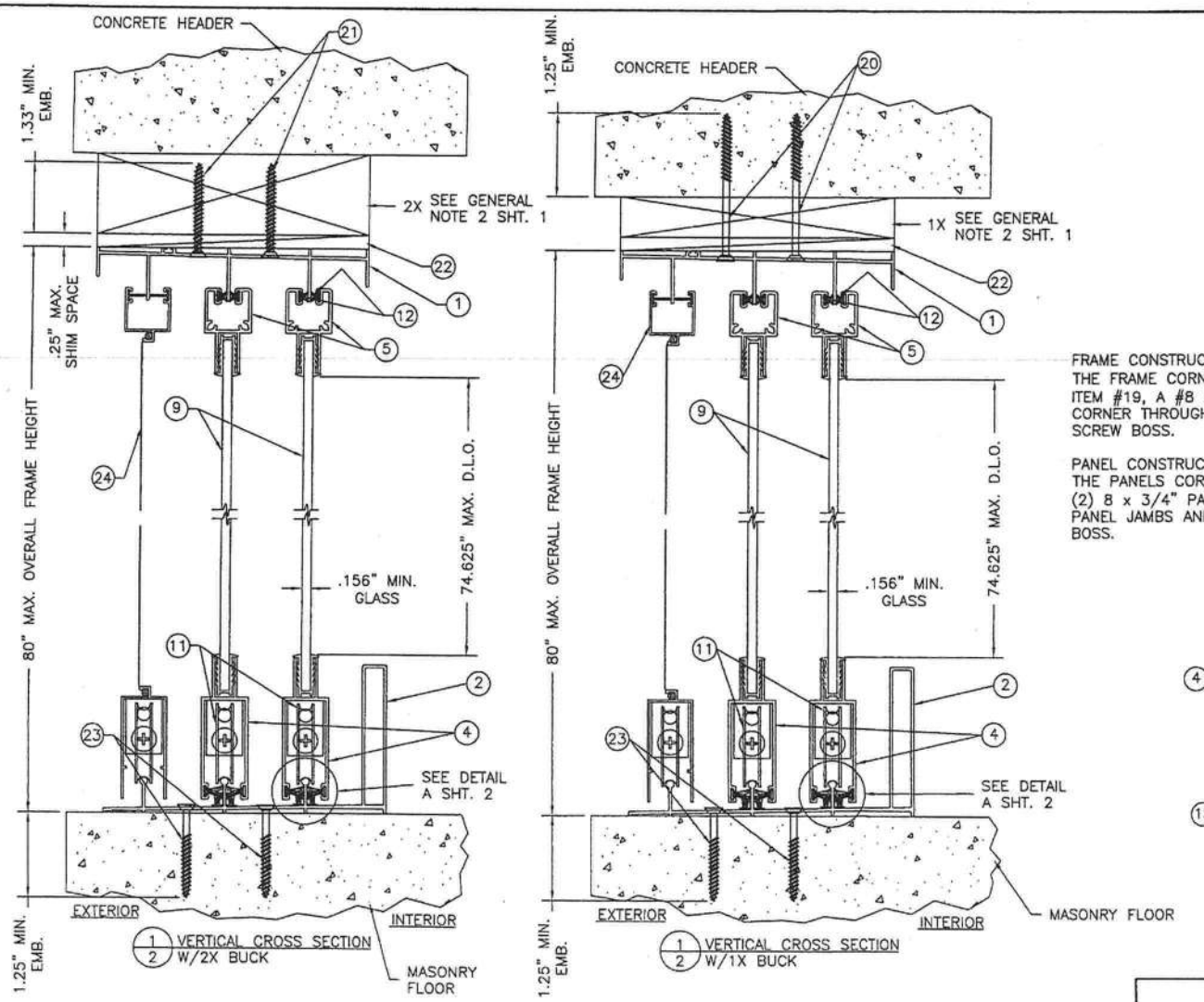
Product Approval Documents Prepared By:
RW
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
12/22/03
Lyndon F. Schmidt, P.E. NO. 43409

PRODUCT: NON-IMPACT ALUMINUM SLIDING PATIO DOOR UP TO 6'0 x 6'8

PART OR ASSEMBLY: GENERAL NOTES & TYPICAL ELEVATIONS

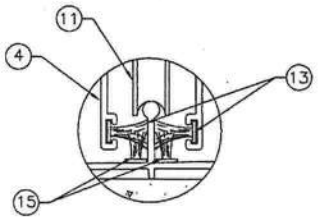
NO.	DATE	BY	REVISIONS

DATE: 12/18/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2425
SHEET 1 OF 5



FRAME CONSTRUCTION NOTE:
THE FRAME CORNERS ARE BUTTED AND SECURED WITH (2) ITEM #19, A #8 x 5/8" FLAT HEAD SCREW, AT EACH CORNER THROUGH THE HEAD AND SILL INTO THE JAMB SCREW BOSS.

PANEL CONSTRUCTION NOTE:
THE PANELS CORNERS ARE BUTTED AND SECURED WITH (2) 8 x 3/4" PAN HEAD SCREWS LOCATED THROUGH THE PANEL JAMBS AND INTO THE PANEL HEAD AND SILL SCREW BOSS.



DETAIL A WEATHERSTRIP AT PANEL BOTTOM, TYP.

Approved as complying with the Florida Building Code
Date 09/04/04
NOA# 03-1224.01
Miami-Dade Product Control Division
By

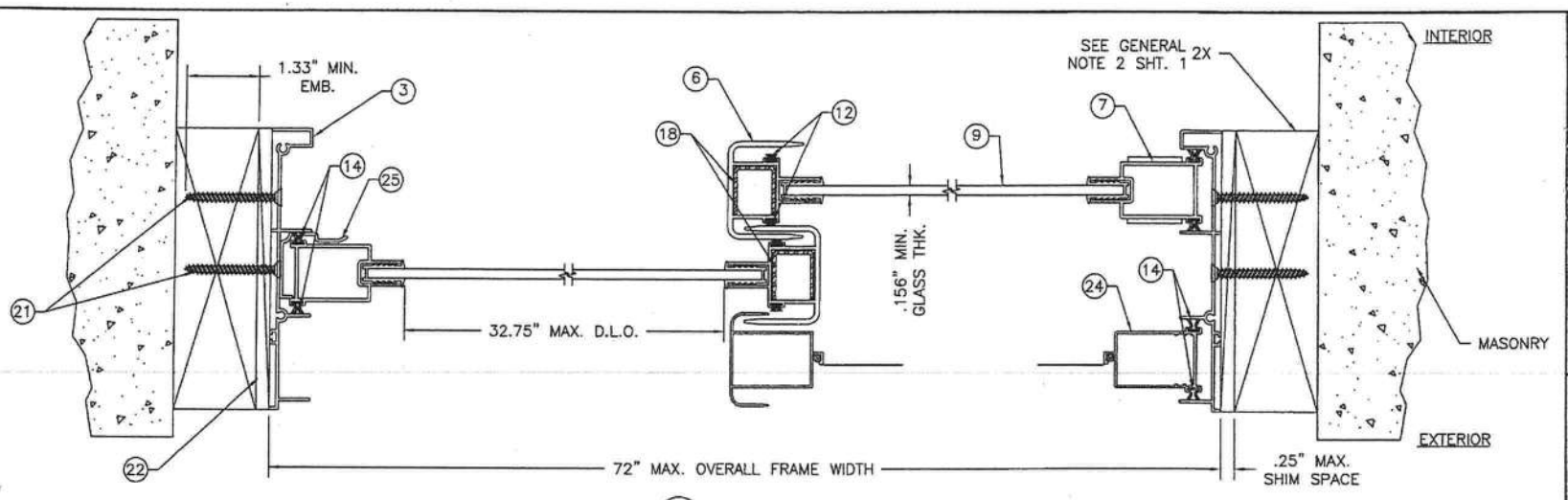
Product Approval Documents Prepared By:
RW BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
12/22/03
Lyndon F. Schmidt, P.E. NO. 43409

PRODUCT: NON-IMPACT ALUMINUM SLIDING PATIO DOOR UP TO 6'0 x 6'8

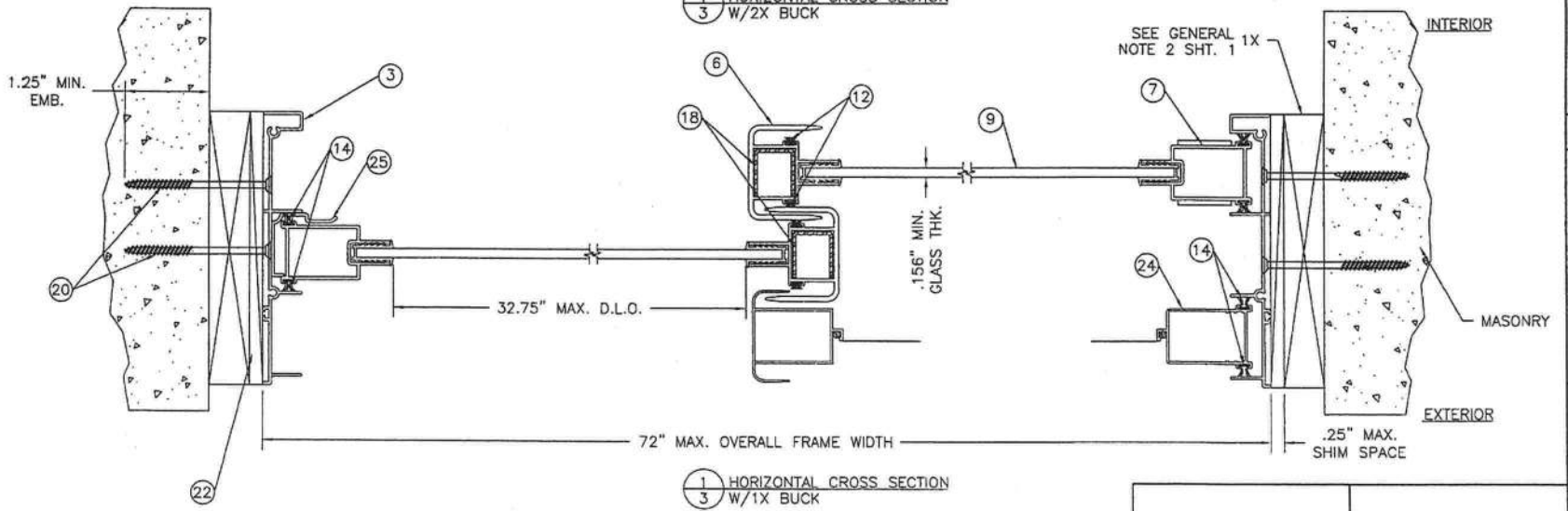
PART OR ASSEMBLY: VERTICAL CROSS SECTIONS & CONSTRUCTION NOTES

NO.	DATE	BY	REVISIONS

DATE: 12/18/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2425
SHEET 2 OF 5



1/3 HORIZONTAL CROSS SECTION W/2X BUCK



1/3 HORIZONTAL CROSS SECTION W/1X BUCK

Approved as complying with the Florida Building Code
Date 03/04/09
NOA# 03-1223.01
Miami Dade Product Control Division
By

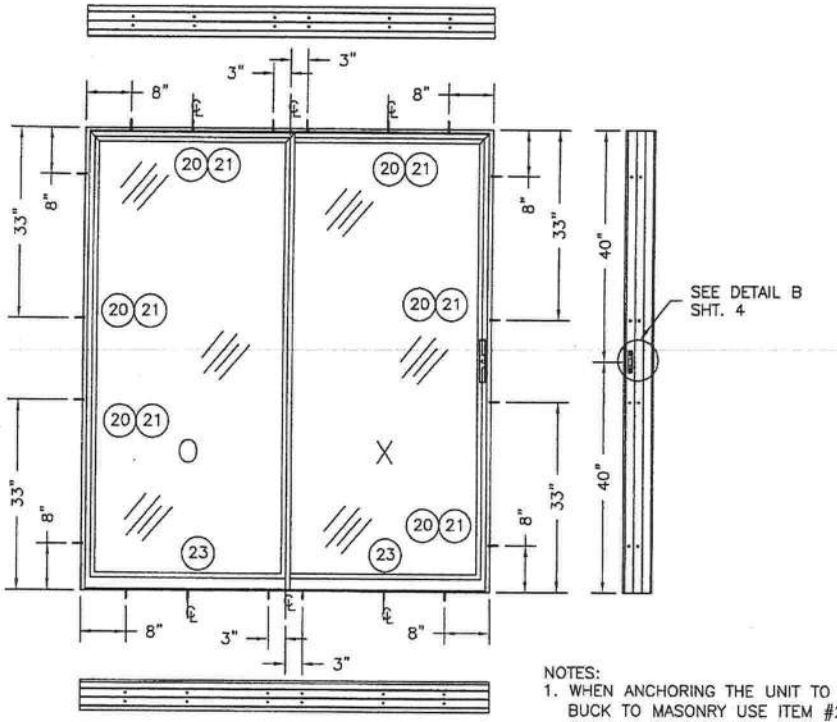
Product Approval Documents Prepared By:
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL. 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
12/22/03
Lyndon F. Schmidt, P.E. NO. 43409

PRODUCT: NON-IMPACT ALUMINUM SLIDING PATIO DOOR UP TO 6'0 x 6'8

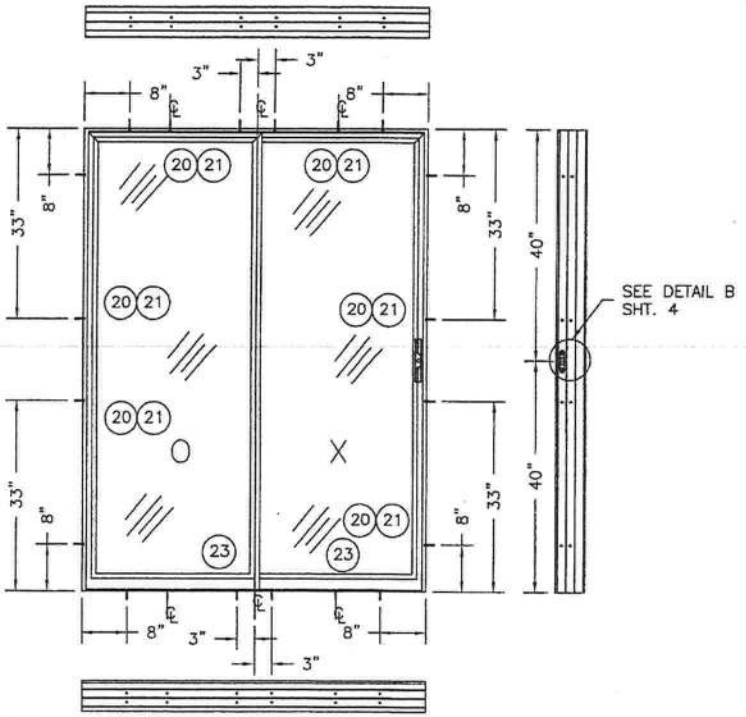
PART OR ASSEMBLY: HORIZONTAL CROSS SECTIONS

NO.	DATE	REVISIONS	BY

DATE: 12/18/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2425
SHEET 3 OF 5

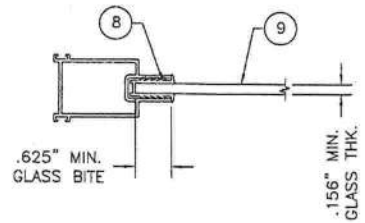


6'0 x 6'8 PATIO DOOR
SEE NOTES SHT. 4

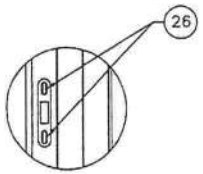


5'0 x 6'8 PATIO DOOR
SEE NOTES SHT. 4

NOTES:
1. WHEN ANCHORING THE UNIT TO A 2X BUCK TO MASONRY USE ITEM #21, A #10 x 1 3/4" PHILLIPS FLAT HEAD SHEET METAL SCREW AT THE HEAD AND SIDES.
2. WHEN ANCHORING THE UNIT THROUGH A 1X BUCK INTO MASONRY USE ITEM #20, A 3/16" x 2 3/4" TAPCON ANCHOR AT THE HEAD AND SIDES.
3. USE (2) ANCHORS PER EACH ANCHORING LOCATION SHOWN ABOVE.



5/32" TEMPERED GLASS
GLAZING DETAIL



DETAIL B
JAMB KEEPER ATTACHMENT

Approved as complying with the Florida Building Code
Date 03/04/04
NOA# 03-1224.01
Miami-Dade Product Control
Division
By

Product Approval Documents Prepared By:
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
12/22/03
Lyndon F. Schmidt, P.E. NO. 43409

PRODUCT:
NON-IMPACT ALUMINUM SLIDING PATIO DOOR UP TO 6'0 x 6'8

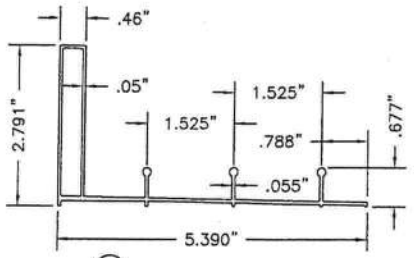
PART OR ASSEMBLY:
ANCHORING LOCATIONS & GLAZING DETAILS

NO.	DATE	BY
	REVISIONS	

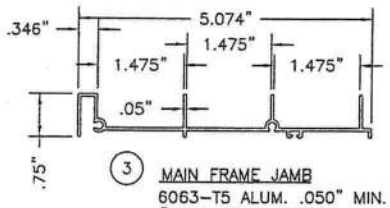
DATE: 12/18/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2425
SHEET 4 OF 5

BILL OF MATERIALS

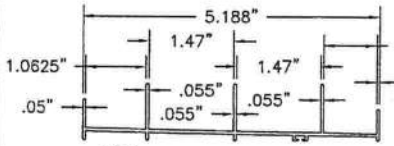
ITEM	DESCRIPTION	MATERIAL
1	EXTRUDED ALUM. MAIN FRAME HEAD #4208 BY MI METALS	ALUM.
2	EXTRUDED ALUM. MAIN FRAME SILL #4235 BY MI METALS	ALUM.
3	EXTRUDED ALUM. MAIN FRAME JAMB #4210 BY MI METALS	–
4	EXTRUDED ALUM. PANEL SILL #4200 MI METALS	ALUM.
5	EXTRUDED ALUM. PANEL HEAD #4202 BY MI METALS	ALUM.
6	EXTRUDED ALUM. PANEL INTERLOCK #4204 BY MI METALS	ALUM.
7	EXTRUDED ALUM. PANEL LOCK JAMB #4206 BY MI METALS	ALUM.
8	GLAZING CHANNEL #80-02-4202 BY MI PLASTICS	V-FLEX
9	GLAZING 5/32" TEMPERED GLASS	GLASS
10	MORTISE HANDLE SET #99-04-150 BY THRUTH HARDWARE	STEEL
11	TANDEM PANEL ROLLER #99-17-195 BY ULTRA	STEEL
12	CENTER FIN SEAL .180 x .250 PANEL HEAD & INTERLOCK BY AMESBURY	SYN. PILE
13	SIDE FIN SEAL .180 x .350 PANEL SILL BY AMESBURY	SYN. PILE
14	CENTER FIN SEAL .187 x .290 PANEL JAMB BY AMESBURY	SYN. PILE
15	NO FIN SEAL .270 x .290 SILL BY AMESBURY	SYN. PILE
16	#6 x 3/4" SQ. DR. SCREW PANEL HEAD TO JAMB	STEEL
17	1/4"-20 x 3/4" SQ. DR. SCREW FOR PANEL ASSEMBLY	STEEL
18	INTERLOCK REINFORCEMENT #99-17-525 BY MAGNOLIA METALS	STEEL
19	#8 x 5/8" FLAT HEAD SCREW FRAME CORNERS	STEEL
20	3/16" x 2 3/4" TAPCON ANCHOR	STEEL
21	#10 x 1 3/4" SHEET METAL SCREW	STEEL
22	1/4" MAX. SHIM MATERIAL	–
23	3/16" x 1 3/4" TAPCON ANCHOR	STEEL
24	ROLLING SCREEN DOOR ASSEMBLY	–
25	EXTRUDED ALUM. PANEL RETAINER #4222 BY MI METALS	ALUM.
26	#8 x 2" PHILLIPS PAN HEAD SHEET METAL SCREW	STEEL



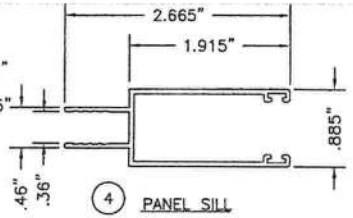
(2) MAIN FRAME SILL
6063-T5 ALUM. .050" MIN.
#4235 BY MI METALS



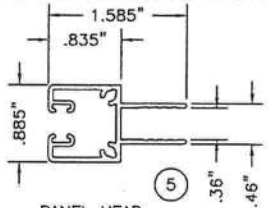
(3) MAIN FRAME JAMB
6063-T5 ALUM. .050" MIN.
#4210 BY MI METALS



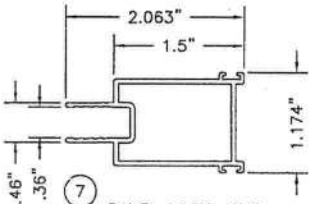
(1) MAIN FRAME HEAD
6063-T5 ALUM. .050" MIN.
#4208 BY MI METALS



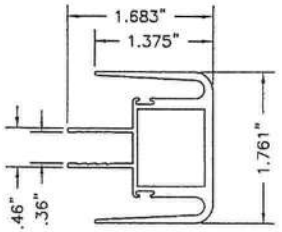
(4) PANEL SILL
6063-T5 ALUM. .055" MIN.
#4200 BY MI METALS



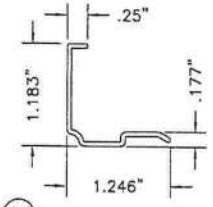
(5) PANEL HEAD
6063-T5 ALUM. .055" MIN.
#4202 BY MI METALS



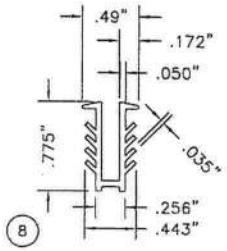
(7) PANEL LOCK JAMB
6063-T5 ALUM. .055" MIN.
#4206 BY MI METALS



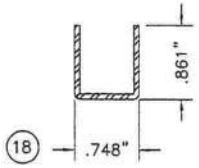
(6) PANEL INTERLOCK
6005-T5 ALUM. .055" MIN.
#4204 BY MI METALS



(25) PANEL RETAINER
6063-T5 ALUM. .055" MIN.
#4222 BY MI METALS



(8) GLAZING CHANNEL
V-FLEX
#80-02-4202
BY MI PLASTICS



(18) INTERLOCK REINFORCEMENT
.060" THK. GALVANIZED STEEL
#99-17-525 BY MAGNOLIA METALS

Approved as complying with the Florida Building Code
Date: 05/04/04
NOA# 03-1224.01
Miami-Dade Product Control
Division
By:

Product Approval Documents Prepared By:
RW
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL. 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
12/22/03
Lyndon F. Schmidt, P.E. NO. 43409

PRODUCT: NON-IMPACT ALUMINUM SLIDING PATIO DOOR UP TO 6'0 x 6'8

PART OR ASSEMBLY: BILL OF MATERIALS & UNIT COMPONENTS

NO.	DATE	REVISIONS	BY

DATE: 12/18/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2425
SHEET 5 OF 5

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

MI Home Products, Inc.
650 West Market Street
Gratz, PA 17030

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.

DESCRIPTION: Series "BetterBilt D185SH/D3185SH" Aluminum Single Hung Window

APPROVAL DOCUMENT: Drawing No. **S-2422,** titled "Non-Impact Single Hung Window Rectangle Circle Top & Oriel", sheets 1 through 5 of 5, prepared by RW Building Consultants, inc, dated 10/27/03 with revision "2", dated 02/10/04, signed and sealed by Wendell Haney, P.E., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 and evidence page E-1, as well as approval document mentioned above.

The submitted documentation was reviewed by **Theodore Berman, P.E.**

2/13/2003

PRODUCT CONTROL ACCEPTED

NOA No 03-1215.02
Expiration Date: March 04, 2009
Approval Date: March 04, 2004

MI Home Products, Inc.

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

A. DRAWINGS

1. Manufacturer's die drawings and sections.
2. Drawing No. **S-2422,** titled "Non-Impact Single Hung Window Rectangle Circle Top & Oriel", sheets 1 through 5 of 5, prepared by RW Building Consultants, inc, dated 10/27/03 with revision "2", dated 02/10/04, signed and sealed by Wendell Haney, P.E.

B. TESTS

1. Test reports on 1) Air Infiltration Test, per FBC, TAS 202-94
   2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
   3) Water Resistance Test, per FBC, TAS 202-94
   4) Forced Entry Test, per FBC 2411.3.2.1 and TAS 202-94

   along with marked-up drawings and installation diagram of an aluminum single hung window, prepared by Architectural Testing, Inc., Test Report No. **ATI 03056**, dated 11/11/03, signed by Joseph A. Reed, P.E.

C. CALCULATIONS

1. Anchor Calculations, ASTM-E1300-98, and structural analysis, prepared by R.W. Building Consultants, Inc., dated 12/11/03, signed and sealed by Lyndon F. Schmidt, P.E.
2. Revised Anchor Calculations, and structural analysis, prepared by R.W. Building Consultants, Inc., dated 02/10/04, signed and sealed by Lyndon F. Schmidt, P.E.

D. QUALITY ASSURANCE

1. Miami Dade Building Code Compliance Office (BCCO).

E. MATERIAL CERTIFICATIONS

1. None.

F. STATEMENTS

1. Statement letter of conformance and no financial interest, dated December 09, 2003, signed and sealed by Lyndon F. Schmidt, P.E.
2. Statement letter of no financial interest with the laboratory that performed the Test Report No. **ATI 03056**, dated November 08, 2003, signed by Stu White, Design Engineering Manager.

G. OTHER

1. Letter from the consultant stating that the product is in compliance with the Florida Building Code (FBC).

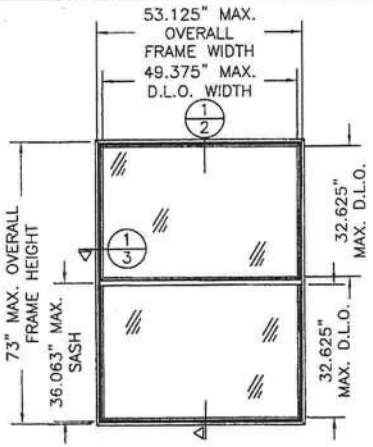
Theodore Berman, P.E.
Deputy Director, Product Control Division
NOA No 03-1215.02
Expiration Date: March 04, 2009
Approval Date: March 04, 2004

MI HOME PRODUCTS

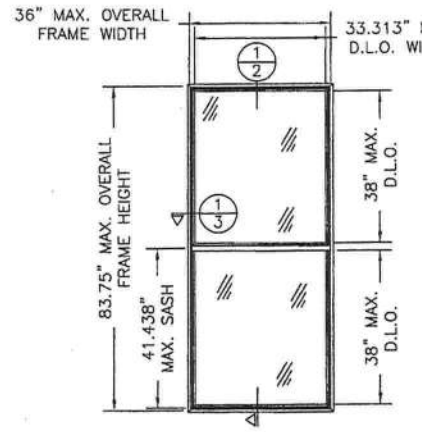
650 WEST MARKET STREET • GRATZ, PA • 17030-0370

SERIES BETTERBILT D185SH/D3185SH
ALUMINUM SINGLE HUNG WINDOW

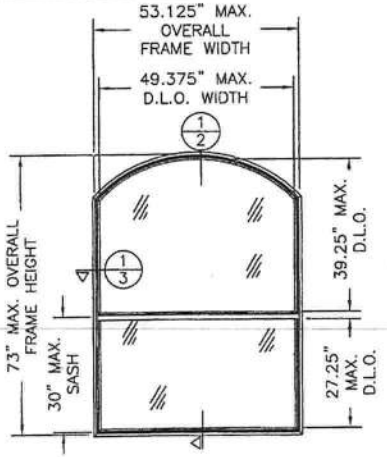
GENERAL NOTES:
1. THIS PRODUCT IS DESIGNED TO COMPLY WITH THE "HVHZ" OF THE FLORIDA BUILDING CODE.
2. WOOD BUCKS MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO STRUCTURE AND TO BE REVIEWED BY BUILDING OFFICIAL.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. FOR DESIGN PRESSURE RATING SEE TABLE THIS SHEET.
5. INSTALLATION OF THIS SYSTEM IN HVHZ AREA REQUIRES THE USE OF APPROVED SHUTTER/EXTERNAL PROTECTION DEVICE COMPLYING WITH HVHZ REQUIREMENTS; INSTALLATION OF THIS SYSTEM OUTSIDE OF HVHZ SHALL MEET THE APPLICABLE CODE REQUIREMENTS FOR WINDBORNE DEBRIS PROTECTION.
6. THIS PRODUCT MEETS WATER REQUIREMENTS FOR HIGH VELOCITY HURRICANE ZONES.



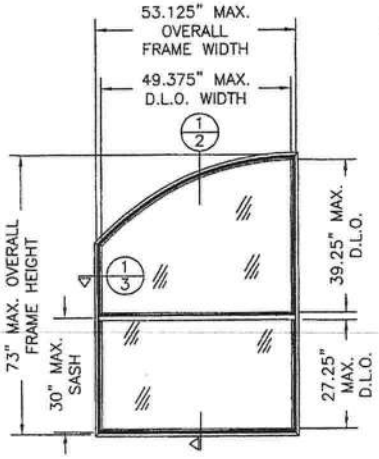
53" x 73" SINGLE HUNG WINDOW



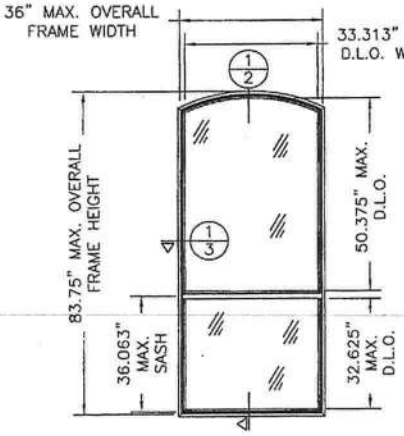
37" x 84" SINGLE HUNG WINDOW



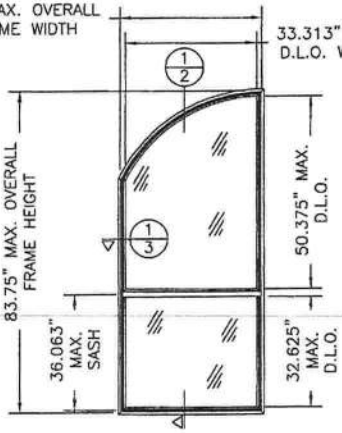
53" x 73" SINGLE HUNG WINDOW
CIRCLE TOP ORIEL



53" x 73" SINGLE HUNG WINDOW
HALF CIRCLE TOP ORIEL



37" x 84" SINGLE HUNG WINDOW
CIRCLE TOP ORIEL



37" x 84" SINGLE HUNG WINDOW
HALF CIRCLE TOP ORIEL

TABLE OF CONTENTS	
SHEET #	DESCRIPTION
1	GENERAL NOTES & TYPICAL ELEVATIONS
2	VERTICAL CROSS SECTIONS
3	HORIZONTAL CROSS SECTIONS & GLAZING DETAIL
4	ANCHORING LOCATIONS
5	COMPONENTS, BILL OF MATERIALS

DESIGN PRESSURE RATINGS (PSF)			
GLASS	MAX. SIZE	DP POS.	DP NEG.
1/8" Temp.	OA 53" x 73"	+56.7	-69.3
1/8" Temp.	OA 37" x 84"	+56.7	-69.3
3/16" Ann.	OA 53" x 73"	+42.0	-42.0
3/16" Ann.	OA 37" x 84"	+56.7	-58.0

ALL ELEVATIONS ARE VIEWED FROM EXTERIOR

Approved as complying with the Florida Building Code.
Date 03/04/04
NOA# 02-1215.02
Miami Dade Product Control Division
By

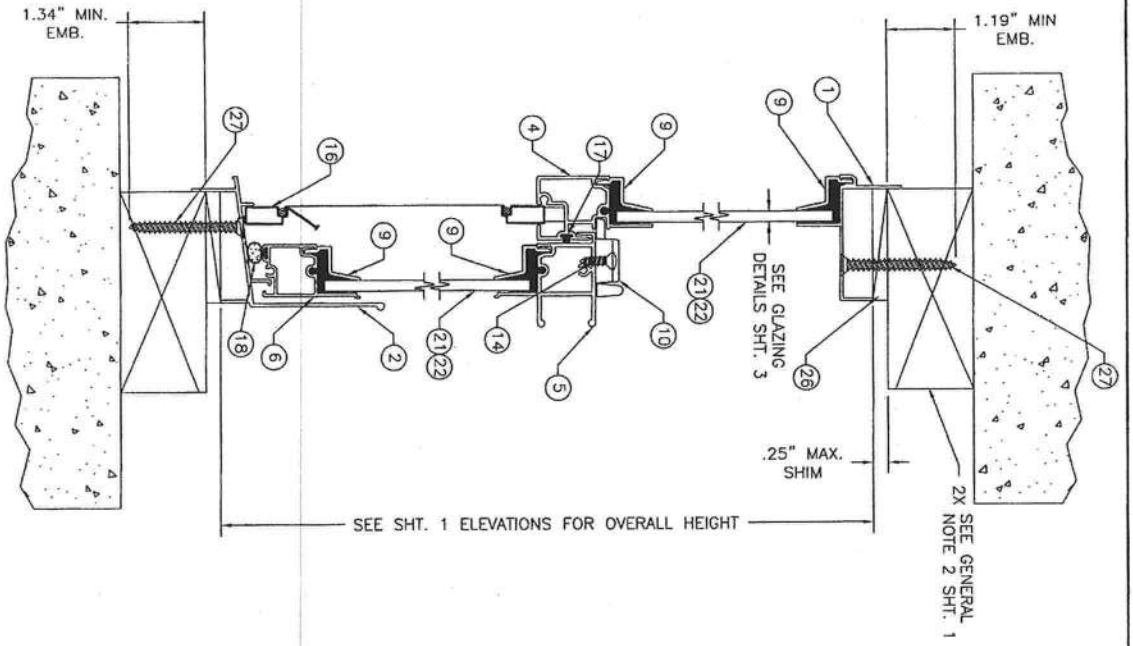
Product Approval Documents Prepared By:
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL. 33595
Phone No.: 813.689.9197
Florida Board of Professional Engineers
Certificate of Authorization No. 9813
Wendell Hicks, P.E. No. 34158
2/10/04

PRODUCT: NON-IMPACT SINGLE HUNG WINDOW RECTANGLE, CIRCLE TOP & ORIEL

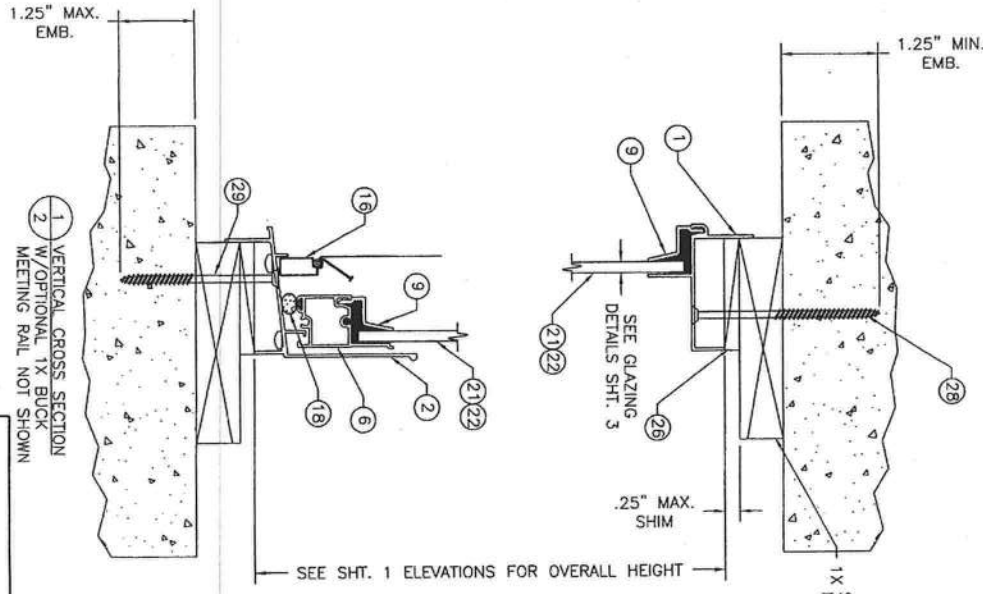
PART OR ASSEMBLY: GENERAL NOTES & TYPICAL ELEVATIONS

NO.	DATE	REVISIONS	BY
2	2/10/04	CORRECT DP TABLE	RW
1	01/04	REVISED PER DADE LETTER	WH

DATE: 10/27/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2422
SHEET 1 OF 5



1/2 VERTICAL CROSS SECTION W/2X BUCK



1/2 VERTICAL CROSS SECTION W/OPTIONAL 1X BUCK MEETING RAIL NOT SHOWN

Approved as complying with the Florida Building Code NOA 06-215.02 Miami-Dade Product Control Division By:

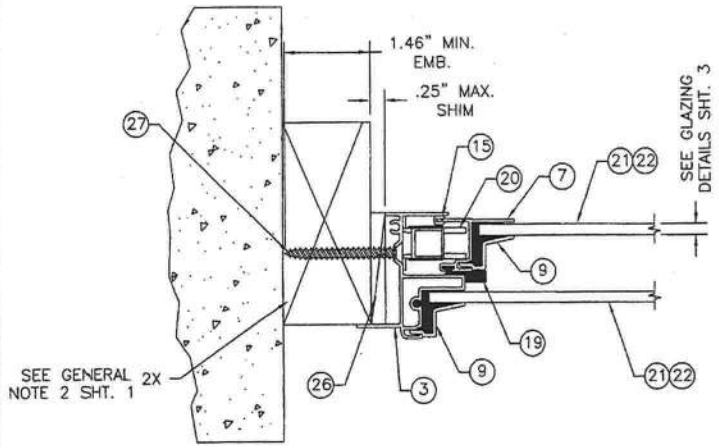
NO.	DATE	REVISIONS	BY
2	2/10/04	CORRECT DP TABLE	RW
1	01/04	REVISED PER DADE LETTER	WH

PRODUCT: NON-IMPACT SINGLE HUNG WINDOWS RECTANGLE, CIRCLE TOP & OREIAL

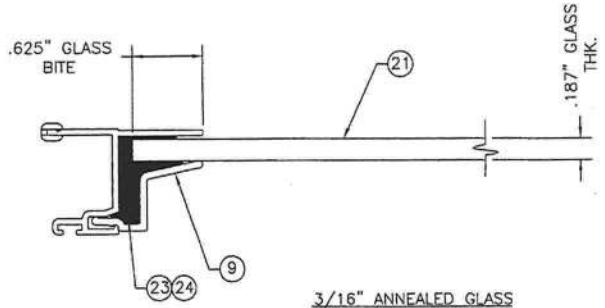
PART OR ASSEMBLY: VERTICAL CROSS SECTIONS

Product Approval Documents Prepared By:
RW BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico, FL 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813,
2/10/04
Wendell Hall, P.E. No. 54158

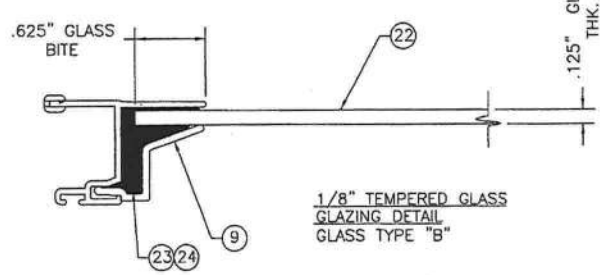
DATE: 10/27/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2422
SHEET 2 OF 5



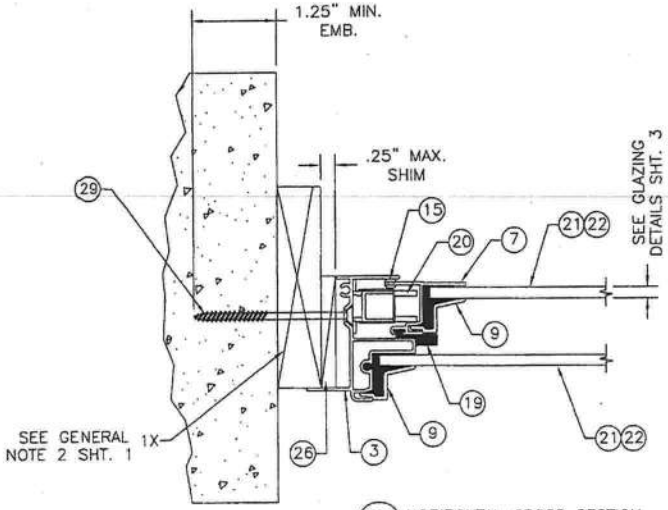
1/3 HORIZONTAL CROSS SECTION
W/2X BUCK



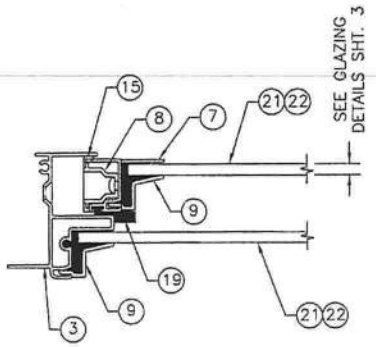
3/16" ANNEALED GLASS
GLAZING DETAIL
GLASS TYPE "A"



1/8" TEMPERED GLASS
GLAZING DETAIL
GLASS TYPE "B"



1/3 HORIZONTAL CROSS SECTION
W/1X BUCK



1/3 HORIZONTAL CROSS SECTION
SHOWING SASH CAM
MASONRY & BUCK NOT SHOWN

NOTES:
1. THE MAIN FRAME HEAD, SIDES AND SILL ARE CONNECTED TOGETHER AT EACH CORNER WITH (2) ITEM #11, A #8 x 3/4" PHILLIPS PAN HEAD SCREW. THE SCREWS RUN FROM THE HEAD DOWN INTO THE SIDES AND FROM THE SILL UP INTO THE SIDES.
2. THE FIXED MEETING RAIL IS SECURED TO THE SIDES WITH (2) EACH SIDE ITEM #12, A #8 x 1 1/4" PHILLIPS PAN HEAD SCREW.
3. THE SASH CORNERS ARE CONNECTED TOGETHER WITH (2) EACH CORNER ITEM #13, A #6 x 3/4" PHILLIPS PAN HEAD SCREW.

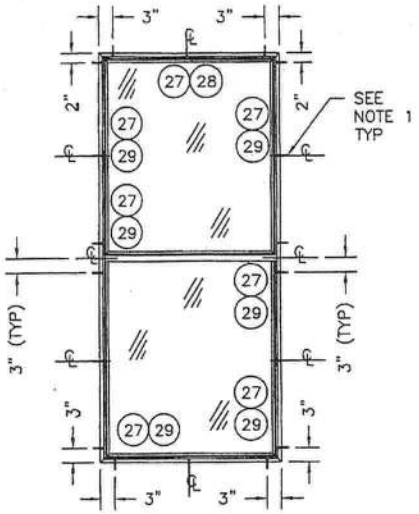
Approved as complying with the Florida Building Code
Date 03/04/04
NOA# 02-1215.02
Miami-Dade Product Control Division
By

Product Approval Documents Prepared By:
RW BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL. 33595
Phone No.: 813.659.9197
Florida Board of Professional Engineers
Certificate Of Authorization No. 9813
Wendell Hollis, P.E. No. 54158
2/10/04

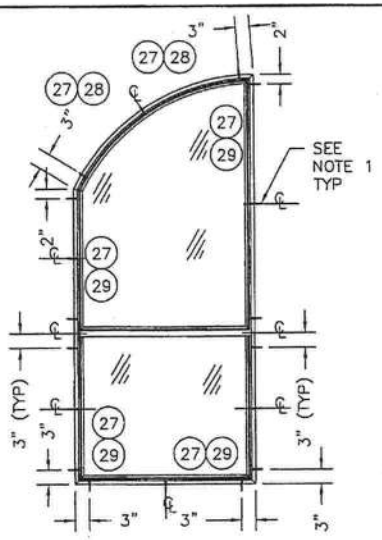
PRODUCT: NON-IMPACT SINGLE HUNG WINDOWS RECTANGLE, CIRCLE TOP & OREIAL
PART OR ASSEMBLY: HORIZONTAL CROSS SECTIONS & GLAZING DETAILS

NO	DATE	REVISIONS	BY
2	2/10/04	CORRECT DP TABLE	RW
1	01/04	REVISED PER DADE LETTER	WH

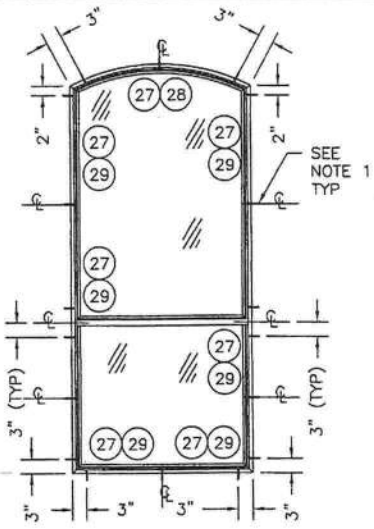
DATE: 10/27/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2422
SHEET 3 OF 5



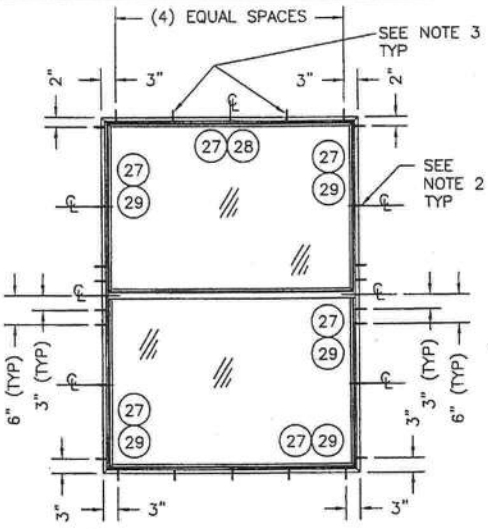
37" x 84" SINGLE HUNG WINDOW



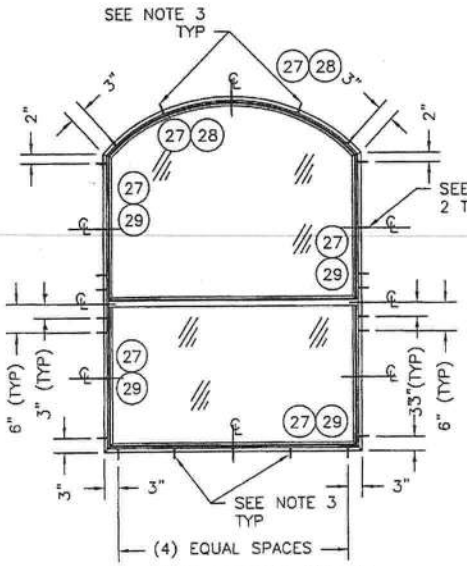
37" x 84" SINGLE HUNG WINDOW
HALF CIRCLE TOP ORIEL



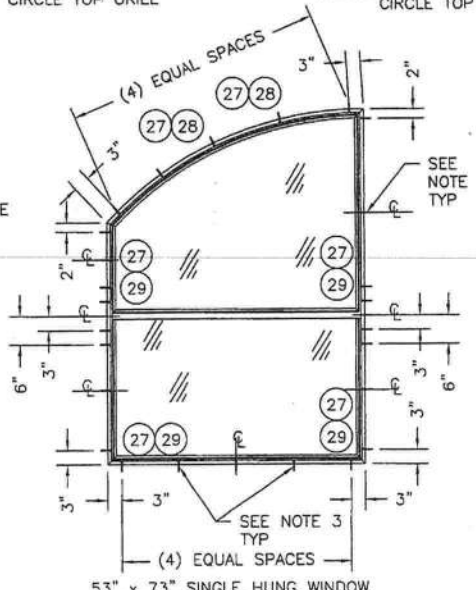
37" x 84" SINGLE HUNG WINDOW
CIRCLE TOP ORIEL



53" x 73" SINGLE HUNG WINDOW



53" x 73" SINGLE HUNG WINDOW
CIRCLE TOP ORIEL



53" x 73" SINGLE HUNG WINDOW
HALF CIRCLE TOP ORIEL

NOTES:
1. FOR UNITS SMALLER THAN 30"x60" DO NOT INSTALL ANCHOR AT CENTER LOCATION.
2. FOR UNITS SMALLER THAN 53"x60" OR SMALLER THAN 30"x66" DO NOT INSTALL ANCHOR AT CENTER LOCATION.
3. FOR UNITS SMALLER THAN 36"x66" DO NOT INSTALL ANCHORS AT EITHER SIDE OF CENTER ANCHOR AT HEAD AND SILL JAMBS.

Approved as complying with the Florida Building Code
Date 03/04/04
NOA# 02-1213.02
Miami-Dade Product Control
Division
By

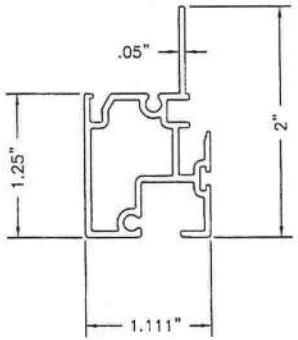
Product Approval Documents Prepared By:
RW BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico FL. 33595
Phone No.: 813.653.9197
Florida Board of Professional Engineers
Certificate of Authorization No. 9813
Wendell Hopper, P.E. No. 54158
2/10/04

PRODUCT: NON-IMPACT SINGLE HUNG WINDOW RECTANGLE, CIRCLE TOP & ORIEL
PART OR ASSEMBLY: ANCHORING LOCATIONS

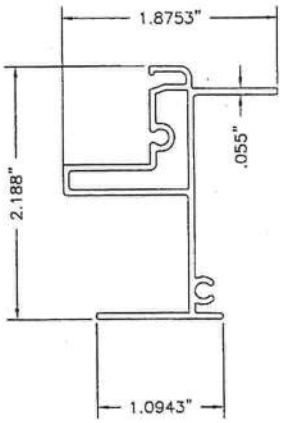
NO.	DATE	REVISIONS	BY
2	2/10/04	CORRECT DP TABLE	RW
1	01/04	REVISED PER DADE LETTER	WH

DATE: 10/27/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2422
SHEET 4 OF 5

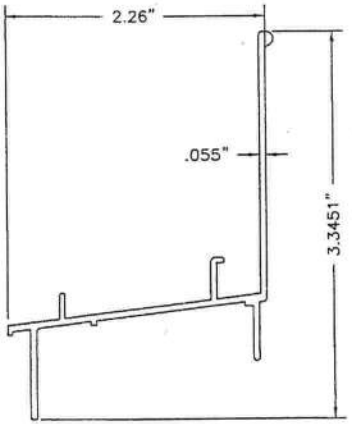
BILL OF MATERIALS		
ITEM	DESCRIPTION	MATERIAL
1	EXTRUDED ALUMINUM SINGLE HUNG 1/2" HEAD #CM-18501 BY MI METALS	ALUM.
2	EXTRUDED ALUMINUM SINGLE HUNG 1/2" SILL #CM-18502 BY MI METALS	ALUM.
3	EXTRUDED ALUMINUM SINGLE HUNG 1/2" JAMB #CM-18503 BY MI METALS	ALUM.
4	EXTRUDED ALUMINUM SINGLE HUNG FIXED MEETING RAIL #CM-18504 BY MI METALS	ALUM.
5	EXTRUDED ALUMINUM SINGLE HUNG SASH LOCK RAIL #CM-18505 BY MI METALS	ALUM.
6	EXTRUDED ALUMINUM SINGLE HUNG SASH BOTTOM RAIL #CM-18506 BY MI METALS	ALUM.
7	EXTRUDED ALUMINUM SINGLE HUNG SASH STILE #CM-18507 BY MI METALS	ALUM.
8	SASH CAM #I-185 BY BSI	-
9	GLAZING BEAD #V-185 BY MI PLASTICS	-
10	LOCK #30240-402 BY REFLECTOLITE	-
11	MAIN FRAME SCREW #8 x 3/4" PHHILLIPS PAN HEAD	STEEL
12	MEETING RAIL SCREW #8 x 1 1/4" PHILLIIPS PAN HEAD	STEEL
13	SASH SCREW #6 x 3/4" PHILLIPS PAN HEAD	STEEL
14	LOCK SCREW #8 x 5/8" PHILLIPS FLAT HEAD -PTD	STEEL
15	SASH GUIDE #80-02-8207 BY PLASTICS, AZ	-
16	WINDOW SCREEN	-
17	FIN WEATHERSTRIPPING .187" x .250" BY AMESBURY	-
18	BULB SEAL #32002 BY AMESBURY	-
19	DUST PLUG 5/8" x 7/8" x .25" BY AMESBURY	-
20	5/8" BLOCK & TACKLE 150 SERIES BY BSI	-
21	GLASS "A" SGL GLAZED 3/16" ANN. BY GUARDIAN	-
22	GLASS "B" SGL GLAZED 1/8" TEMP. BY GUARDIAN	-
23	BACKBEDDING #SM-2100 BY SCHNEE MOREHEAD	SILICONE
24	BACKBEDDING PURFECTGLAZE-H (HOTMELT)	-
25	GLASS SHIM 1/8" x 1/4" x 1" BY SECON	-
26	1/4" MAX SHIM	-
27	#12 X 2" PHILLIPS FLAT HEAD SHEET METAL SCREW	STEEL
28	3/16" x 3 1/4" ELCO TAPCON ANCHOR	STEEL
29	3/16 x 2 3/4" ELCO TAPCON ANCHOR	STEEL



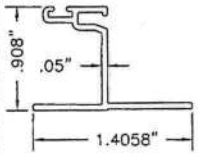
④ FIXED MEETING RAIL



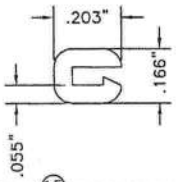
③ JAMB-FLANGE



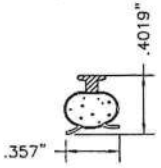
② SILL FIN



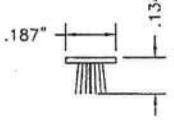
⑦ SASH STILE



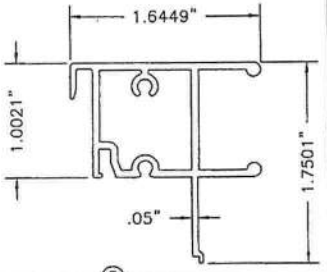
⑮ SASH GUIDE



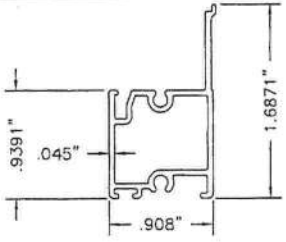
⑱ BULB SEAL



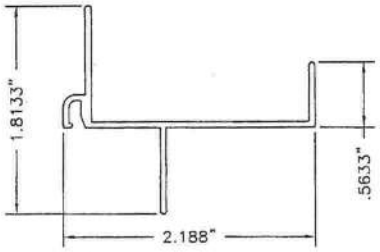
⑰ FIN WEATHERSTRIPPING



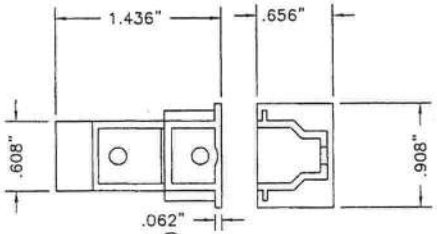
⑤ LOCK RAIL



⑥ SASH BOTTOM RAIL



① HEAD FLANGE



⑧ SASH CAM

Approved as complying with the Florida Building Code
Date: 03/09/04
NOA: 03-1215.02
Miami-Dade Product Control
Division
By:

Product Approval Documents Prepared By:
RW
BUILDING CONSULTANTS, INC.
P.O. Box 230 Valrico, FL 33595
Phone No.: 813.659.9187
Florida Board of Professional Engineers
Certificate of Authorization No. 9813,
Wendell Hones, P.E. No. 34158
2/10/04

PRODUCT: NON-IMPACT SINGLE HUNG WINDOWS RECTANGLE, CIRCLE TOP & OREIAL

PART OR ASSEMBLY: BILL OF MATERIALS & UNIT COMPONENTS

NO.	DATE	REVISIONS	BY
2	2/10/04	CORRECT OP TABLE	RW
1	01/04	REVISED PER DADE LETTER	WH

DATE: 10/27/03
SCALE: N.T.S.
DWG. BY: TJH
CHK. BY: RW
DRAWING NO.: S-2422
SHEET 5 OF 5

ESP
Energy Saving Products

Aluminum Single Hung and Picture Windows

Series 130/131/135/136

2001 Florida Product Approval #1950.1(2,3,4) / 1954.1(2,3)

2004 Florida Product Approval #5768.1

- **Test Reports**
- **Installation Detail**
- **Comparative Analysis**

Series 140/141/145/146

2001 Florida Product Approval #1950.6(7,8) / 1954.4(5,6,7)

2001 Florida Product Approval #5768.2/5769.1

- **Test Reports**
- **Installation Details**
- **Comparative Analysis**

Series 330/331/335/336

2001 Florida Product Approval #1950.11(12,13,14)

- **Test Reports**
- **Installation Detail**
- **Comparative Analysis**

Series 340/341/345/346

2001 Florida Product Approval #1950.15(16,17,18,19) / 1954.10(11,12)

2004 Florida Product Approval #5768.3

- **Test Reports**
- **Installation Detail**
- **Comparative Analysis**

Series 530/531/535/536

2001 Florida Product Approval #3210.2(3, 4)

- **Test Reports**
- **Installation Detail**
- **Comparative Analysis**

Series 540/541/545/546

Florida Product Approval #3210.2(3,4)

- **Test Reports**
- **Installation Detail**
- **Comparative Analysis**

Series 551/561/556/566 (Impact)

Florida Product Approval #1950.22

561 Florida Product Approval #5768.4

- **Test Reports**

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-6S-16-04056-120 **Building permit No.** 000025730

Use Classification SFD,UTILITY **Fire:** 11.16

Permit Holder JOHN NORRIS **Waste:**

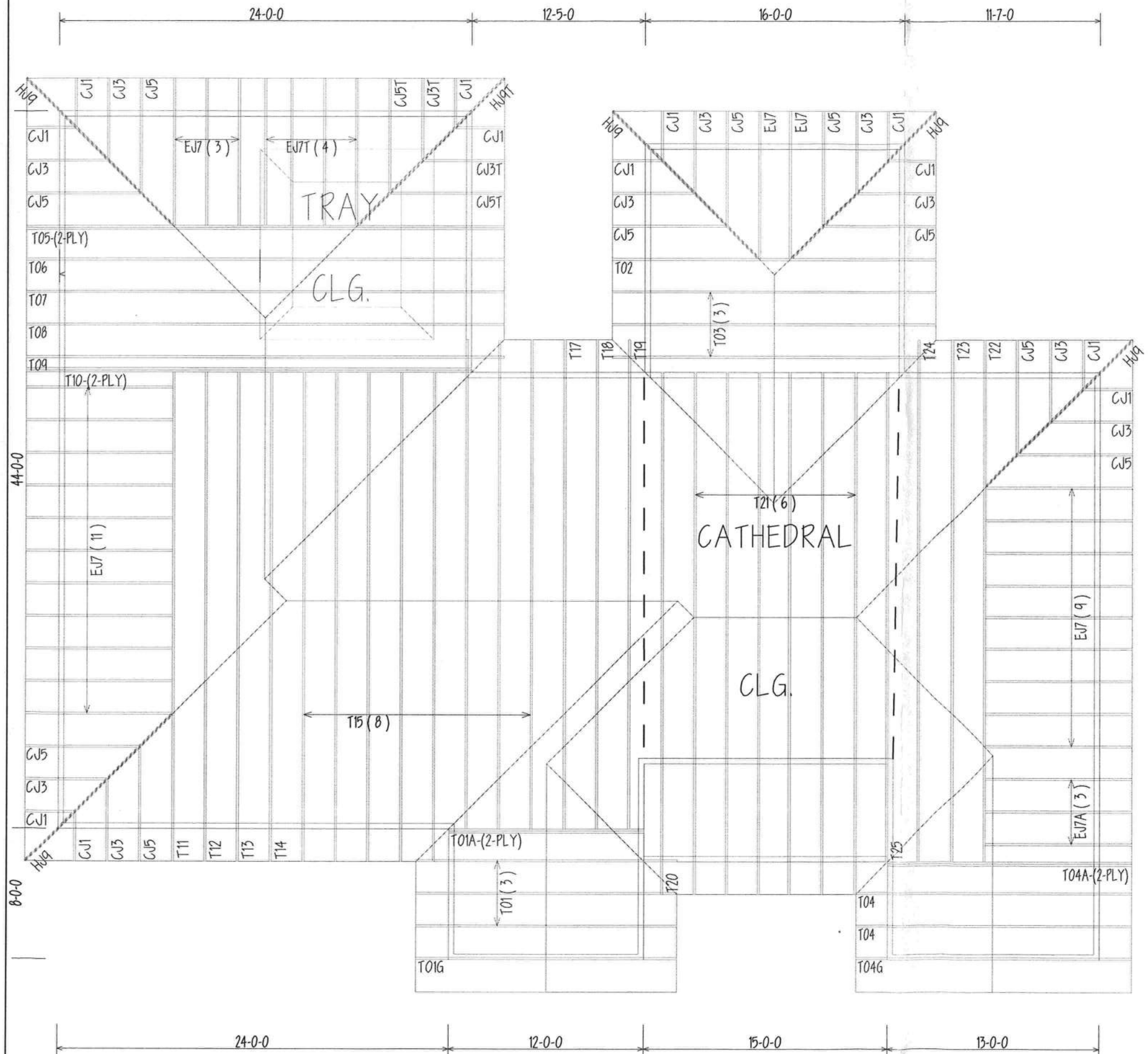
Owner of Building IMAGE DEVELOPMENT **Total:** 11.16

Location: 341 SW THISTLEWOOD LANE, FT. WHITE, FL

Date: 08/21/2007

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



BEARING HEIGHT SCHEDULE

8'-0"

6/12 PITCH
2' OH

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: JOHN NORRIS		
LEGAL ADDRESS: LOT 20 THORNWOOD		
MODEL: MADISON		REVISION:
		SCALE: NTS
DATE: 3-22-07	DRAWN BY: KLH	JOB #: L232942