Columbia County Building Permit Application

Revi

For Office Use Only Application # 0602-100 Date Received 2/28/06 By GT Permit # 24297

Application Approved by - Zoning Official BLK Date 08-03-06 Plans Examiner OK JTH Date 3-23-06

Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3

Comments Accessory Use

NO - NOC

Applicants Name RONALD R & MARCIA A OLSZAK Phone 386 454-

Address P.O. BOX 2277 HIGH SPRINGS, FL 32655

Owners Name RONALD R & MARCIA A. OLSZAK Phone 386 454-

911 Address 200 SW BAY PL Fort White FL 32038

Contractors Name OWNER/BUILDER Phone ABOVE

Address ABOVE

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address RIVERA DESIGN GROUP, 1217 NW 16th A

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress

Property ID Number 30-7S-17-10058-594 Estimated Cost of Construction ~~10,000~~ 10,000

Subdivision Name SANTE FE RIVER PLANTATION Lot 4 Block ____ Unit ____

Driving Directions FROM HIGH SPRINGS TAKE 27 N TO CR 138 TURN LEFT ON CR138 TO 1ST ST. ON LEFT HEFLIN AVE. 6 1ST ST ON RT. SW BAY PL TURN RT ON BAY TO 4th LOT ON LF

Type of Construction ~~SINGLE FAM~~ STORAGE Number of Existing Dwellings on Property ____

Total Acreage 2.24 Lot Size 180.9' x 541.47' Do you need a - Culvert Permit or ~~Culvert Waiver~~ or Have an Exi

Actual Distance of Structure from Property Lines - Front 290' Side ~~165'~~ Side 39' Rear 220'

Total Building Height 22' Number of Stories 1 Heated Floor Area ~~2087~~ 720 TOTAL Roof Pitch 7

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Marcia A. Olszak

Ronald R. Olszak

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF ~~COLUMBIA~~ Alachua

Sworn to (or affirmed) and subscribed before me this 27th day of February 2006.

Personally known ____ or Produced Identification ✓

FL Driver's License

Contractor Signature

Contractors License Number

Competency Card Number

NOTARY STAMP/SEAL

Carol D. Short

Notary Signature

CAROL D. SHORT
Notary Public, State of Florida
My comm. expires Nov. 19, 2007
No. DD 269014

OLSZAK STORAGE SHED

RON & MARCIA

02-28-2006

200 S.W. BAY PLACE

PARCEL ID: 30-7S-17-10058-594

PHONE # 386-454-8450

LOT 4, OF A REPLAT OF LOTS 38,45 AND 46, OF SANTA FE RIVER PLANTATIONS, ACCORDING TO THE PLAT THERE AS RECORDED IN PLAT BOOKS, PAGE(S) 13, OF PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA

~~Hold for soil test~~

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0602-100

Ronald & Marcia Olszak Owner/Builders of lot 4 Santa Fe Plantations

On the date of March 8, 2006 application 0602-100 and plans for construction of a storage shed were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

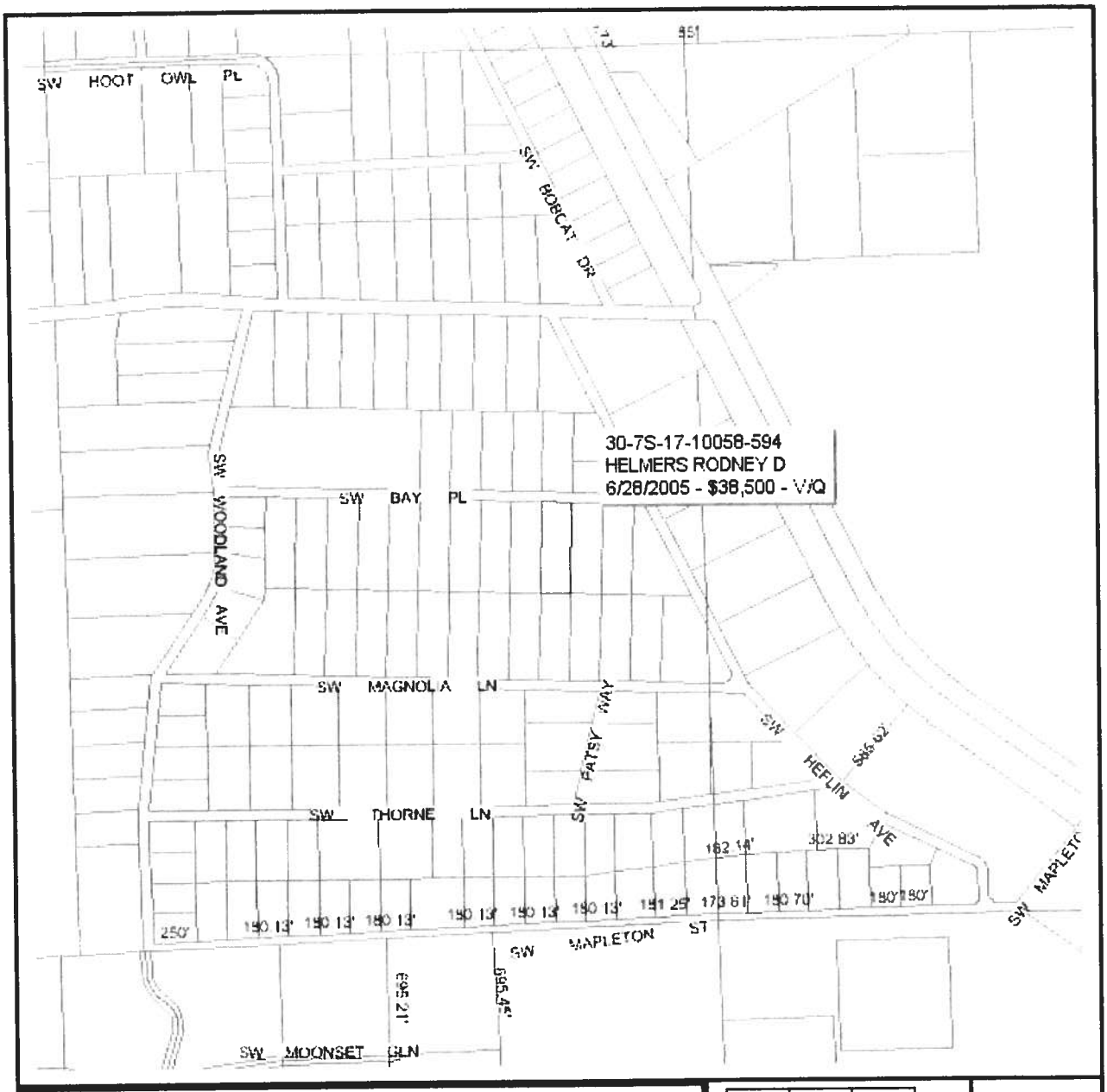
Please include application number 0602-100 when making reference to this application.

1. Please have Mr. Thomas Sputo the structural designer show on the plans the required load bearing capacities of the soils to provide adequate support for the foundations.
2. On the electrical plan show the location of the electrical sub- panel and include the total amperage rating of the electrical service panel. A panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.
3. Please submit a separate Owner Builder Disclosure Statement for the storage shed. (Form Attached)
4. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department on the storage shed.

5. On the floor plan please label the partition rooms.
6. If the storage shed will include plumbing, Please include a separate or joint copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system for the storage shed.
7. Show all the electrical receptacles, which will be required to has GFCI protection with in the storage shed or on the exterior of the storage shed.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department



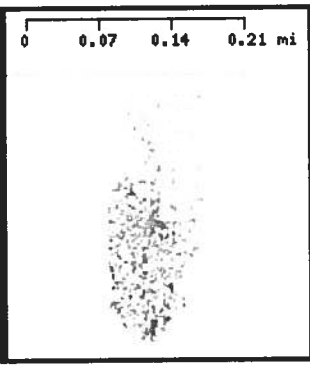
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

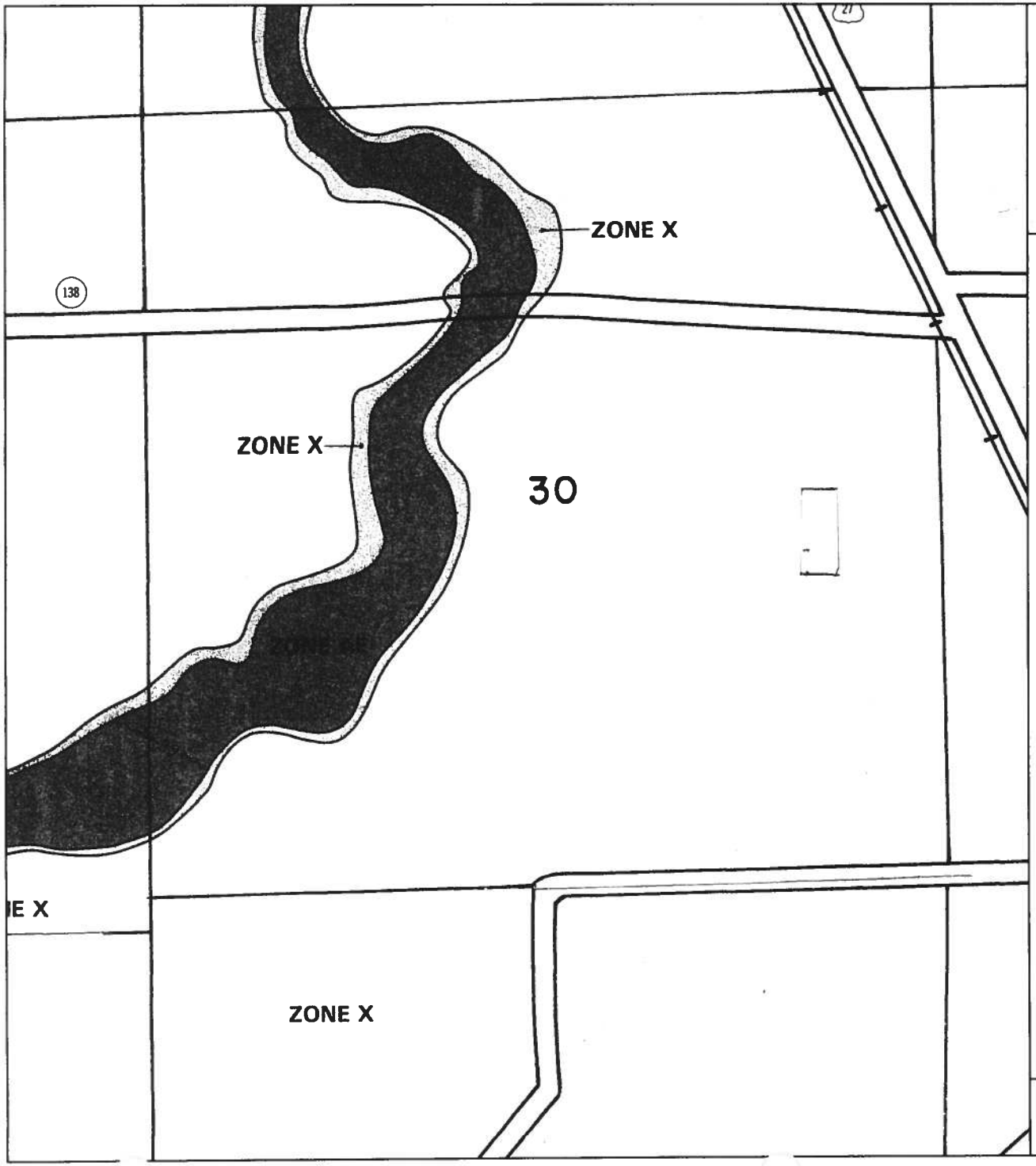
PARCEL: 30-7S-17-10058-594 - VACANT (000000)

LOT 4 SANTA FE RIVER PLANTATIONS REPLAT OF LOT 46. ORB 819-1287, 874-1287,

Name:	OLSZAK RONALD R & MARCIA A	LandVal	$33,196.00
Site:		BldgVal	$0.00
Mail:	P O BOX 2277 HIGH SPRINGS, FL 32655	ApprVal	$33,196.00
		JustVal	$33,196.00
Sales Info	11/30/2005 $81,500.00V / Q	Assd	$33,196.00
	6/28/2005 $38,500.00V / Q	Exmpt	$0.00
	5/10/2001 $100.00V / U	Taxable	$33,196.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.



APPROXIMATE SCALE IN FEET

1000 0 1000

NATIONAL FLOOD INSURANCE PROGRAM

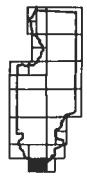
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 270 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0270 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 3/8/2006 (t scale and type A)

GEO-TECH, INC.
ENGINEERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

March 17, 2006
Project No. 062726.01G

Mr. & Mrs. Olszak
P.O. Box 2277
High Springs, FL 32655

Attention: Mr. & Mrs. Olszak

Project: Proposed Residence and Shed, Santa Fe River Plantation, Columbia County, Florida
Soil Bearing Capacity

Dear Mr & Mrs. Olszak:

As requested, Geo-Technologies, Inc. (Geo-Tech) has visited the above referenced project site. The purpose of our visit was to perform penetrometer readings in the area that the proposed residence and shed will be placed based on Hand Penetrometer (HP) results in the native soils. Six (6) auger borings with penetrometer readings were performed to four (4) feet below site grade. Based on the results of the penetrometer readings, the maximum allowable soil bearing pressures found at these locations are approximately 2, 000 pounds per square foot based

Geo-Technologies, Inc. (Geo Tech) trust this report is sufficient to meet your immediate needs. Should you have any questions concerning this report or if we may be of further assistance, please do not hesitate to contact the undersigned.

Sincerely,

Donald "Bubba" Youngblood
Branch Manager

DY/DC: kw

David A. Cappa, P.E.
Florida Registration No. 56639

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☑	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☑	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☑	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☑	☑	**Elevations including:** a) All sides
☑	☑	b) Roof pitch
☐	☑	c) Overhang dimensions and detail with attic ventilation

d) Location, size and height above roof of chimneys.
e) Location and size of skylights
f) Building height
e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned.
b) Shear walls identified.
c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 **(see attach forms).**
d) Show safety glazing of glass, where required by code.
e) Identify egress windows in bedrooms, and size.
f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, **(Please circle applicable type).**
g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
b) All posts and/or column footing including size and reinforcing
c) Any special support required by soil analysis such as piling
d) Location of any vertical steel.

Roof System:

a) Truss package including:
   1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
   2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:
   1. Rafter size, species and spacing
   2. Attachment to wall and uplift
   3. Ridge beam sized and valley framing and support details
   4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

a) Masonry wall
   1. All materials making up wall
   2. Block size and mortar type with size and spacing of reinforcement
   3. Lintel, tie-beam sizes and reinforcement
   4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
   5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
   6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
   7. Fire resistant construction (if required)
   8. Fireproofing requirements
   9. Shoe type of termite treatment (termiticide or alternative method)
   10. Slab on grade
      a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
   11. Indicate where pressure treated wood will be placed
   12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
b) Floor joist size and spacing
c) Girder size and spacing
d) Attachment of joist to girder
e) Wind load requirements where applicable

Plumbing Fixture layout

Electrical layout including:

a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
b) Ceiling fans
c) Smoke detectors
d) Service panel and sub-panel size and location(s)
e) Meter location with type of service entrance (overhead or underground)
f) Appliances and HVAC equipment
g) Arc Fault Circuits (AFCI) in bedrooms
h) Exhaust fans in bathroom

HVAC information

a) Energy Calculations (dimensions shall match plans)
b) Manual J sizing equipment or equivalent computation
c) Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

***Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water AAA PORTA SERVICE INC OR SUNSTATE $65 MONTHLY monthly $65

Location: ___ S.W. ___ Bay Place   **Project Name:** Olszak, Lot 4

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org   Santa Fe River Plantations

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Masonite International		F
2. Sliding			
3. Sectional			
4. Roll up	Clopay		
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	Alenco	Window	FL
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	James Hardie	Hard	FL 889
2. Soffits	Cameron Ashley	Sof	FL 4968.2
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane	Dupont	Tyvek	
9. Greenhouse			
10. Other	Simpson Strong Tie	Anchor Bolt 5/8 X 10	
D. ROOFING PRODUCTS			
1. Asphalt Shingles	Tamko	Shingles	FL 1956 R1
2. Underlayments	Warrior	30# Felt	FL 2346 R1
3. Roofing Fasteners	Simpson	Ties & Straps	*
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

* MSTA 21 FL 1901.36 * Shed *

* MSTAR4 FL 1901.36

14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	Strong Tie Strap-Ties	*
2. Truss plates	MiTek	MII20 MII20H & MII8H	See Truss Plg
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspectio

ntractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # **(FOR STAFF USE ONLY)**

* MSTA 21 FL 1901.56
* MSTA24 FL 1901.57
* Stud Plates SP4 / FL 474.326
* Hurricane Ties H10/FL 474.109

Columbia County Building Department
Culvert Waiver Permit / Application

Waiver No.

APPLICANT RONALD R. & MARCIA A. OLSZAK PHONE 386-454-845

ADDRESS P. O. BOX 2277 HIGH SPRINGS, FL 32655

OWNER RONALD R. & MARCIA A. OLSZAK PHONE 386-454-845

ADDRESS P.O BOX 2277 HIGH SPRINGS, FL 32655

CONTRACTOR OWNER/BUILDER PHONE ABOVE

LOCATION OF PROPERTY 200 SW BAY PL

PARCEL ID # 30-7S-17-10058-594

SUBDIVISION (Lot/Block/Phase/Unit) SANTA FE RIVER PLANTATION, LD

I HEREBY CERTIFY THAT I UNBERSTAND AND WILL FULLY COMPLY WITH THE DECISION OF COLUMBIA COUNTY PUBLIC WORKS DEPARTMENT IN CONNECTION WITH THE HEREIN PROP APPLICATION.

SIGNED: Ronald R. Olszak DATE: Feb 23, 2006

FEE: $ 50.00 A SEPARATE CHECK IS REQUIRED.
MAKE CHECKS PAYABLE TO BCC.

Public Works Department Use Only

I HEREBY CERTIFY THAT I HAVE EXAMINED THIS APPLICATION AND DETERMINE THAT THE CULVERT WAIVER IS:

_____APPROVED _____NOT APPROVED – NEEDS A CULVERT PERMIT

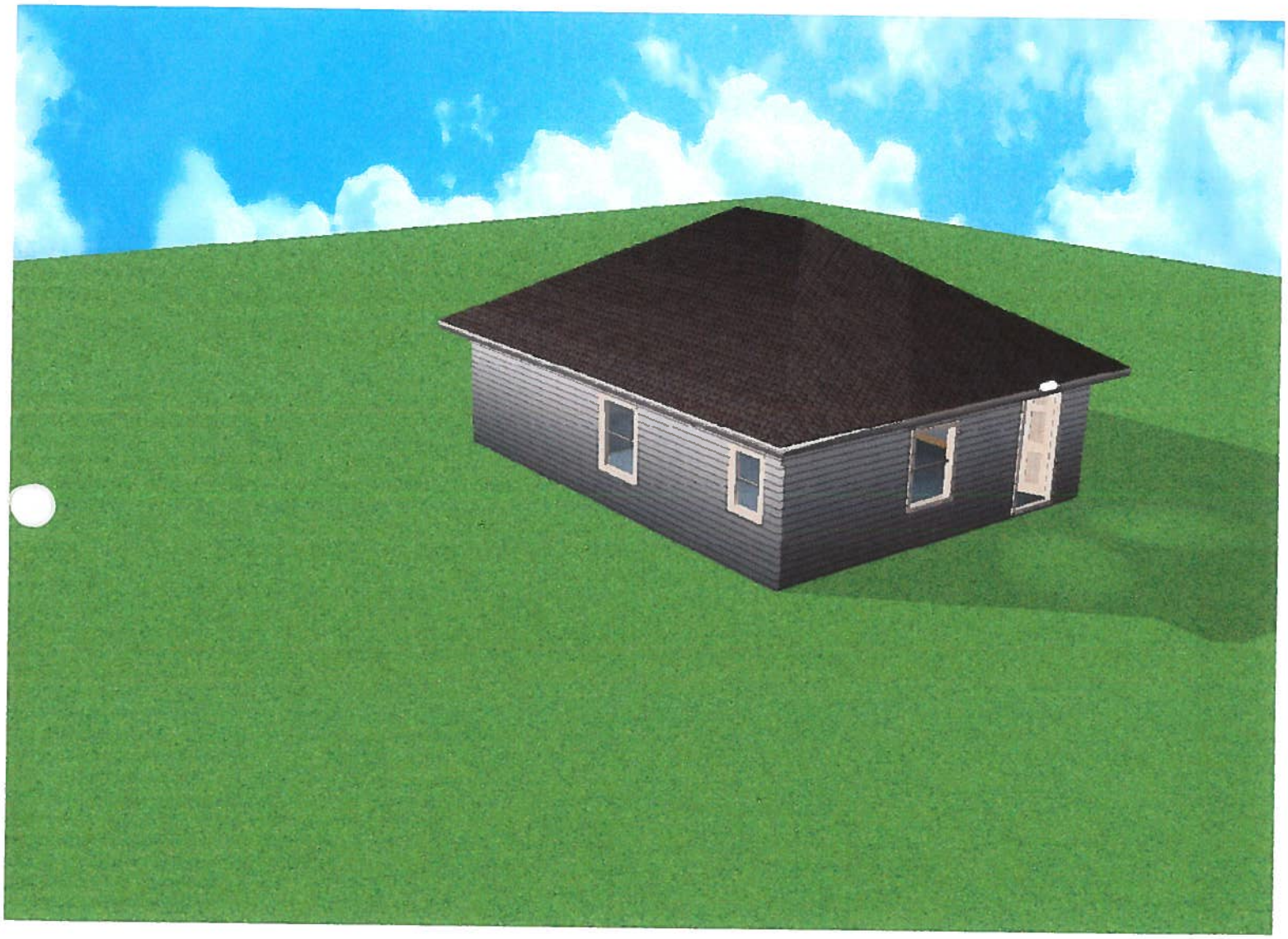
COMMENTS _____

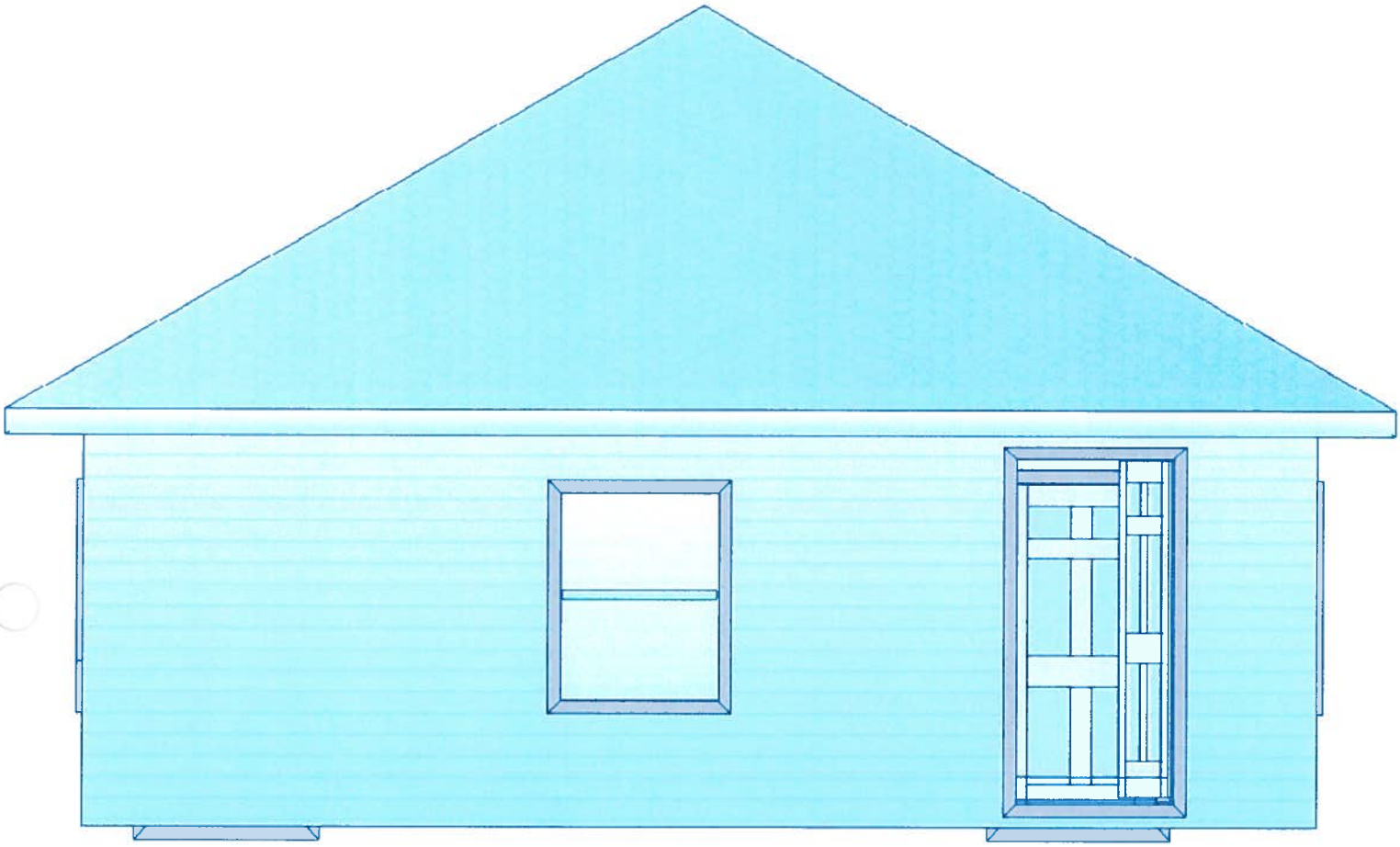
SIGNED: _____ DATE: _____

ANY QUESTIONS PLEASE CONTACT THE PUBLIC WORKS DEPARTMENT AT 386-752-5955.

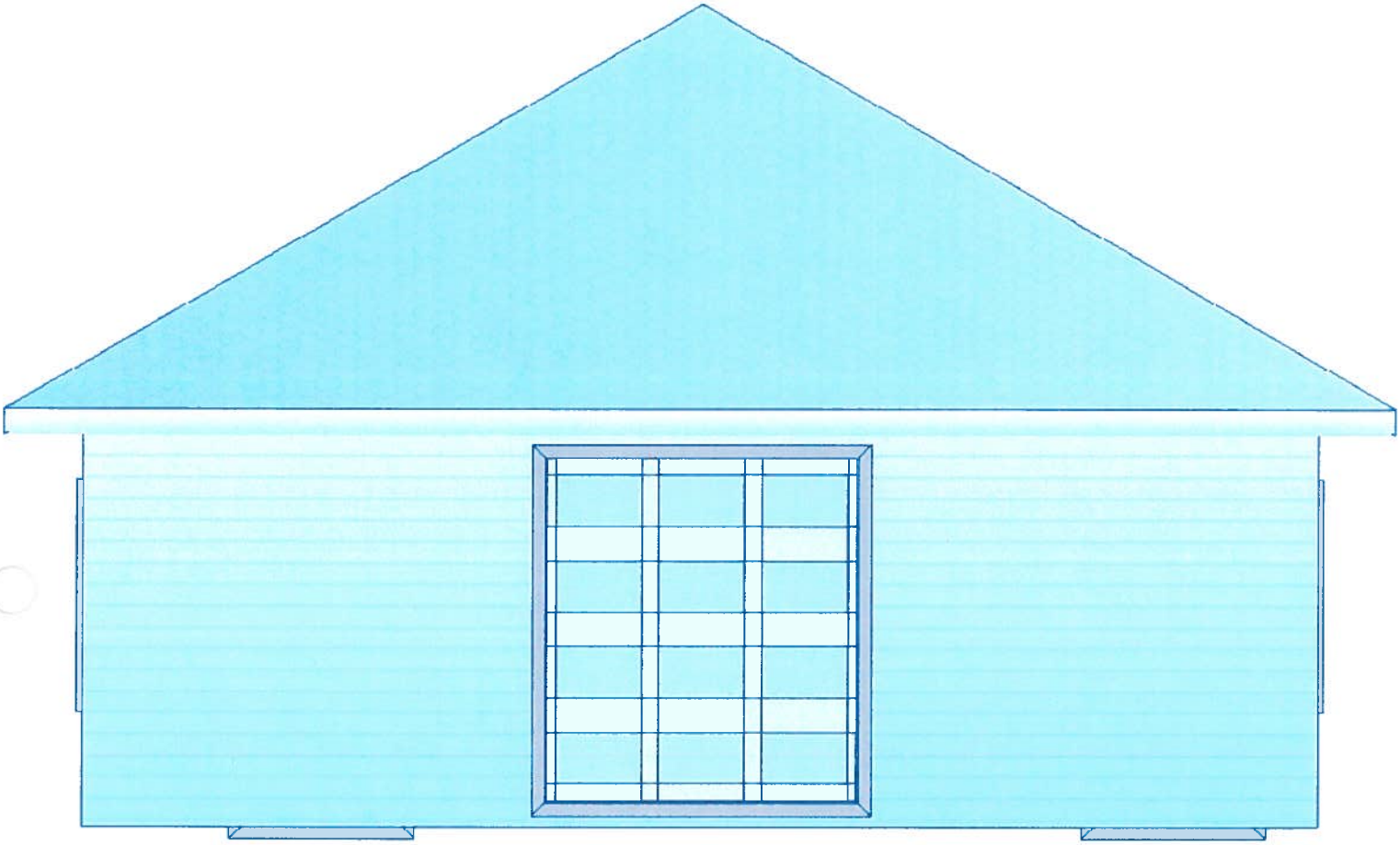
135 NE Hernando Ave., Suite B-21, Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

SEALED

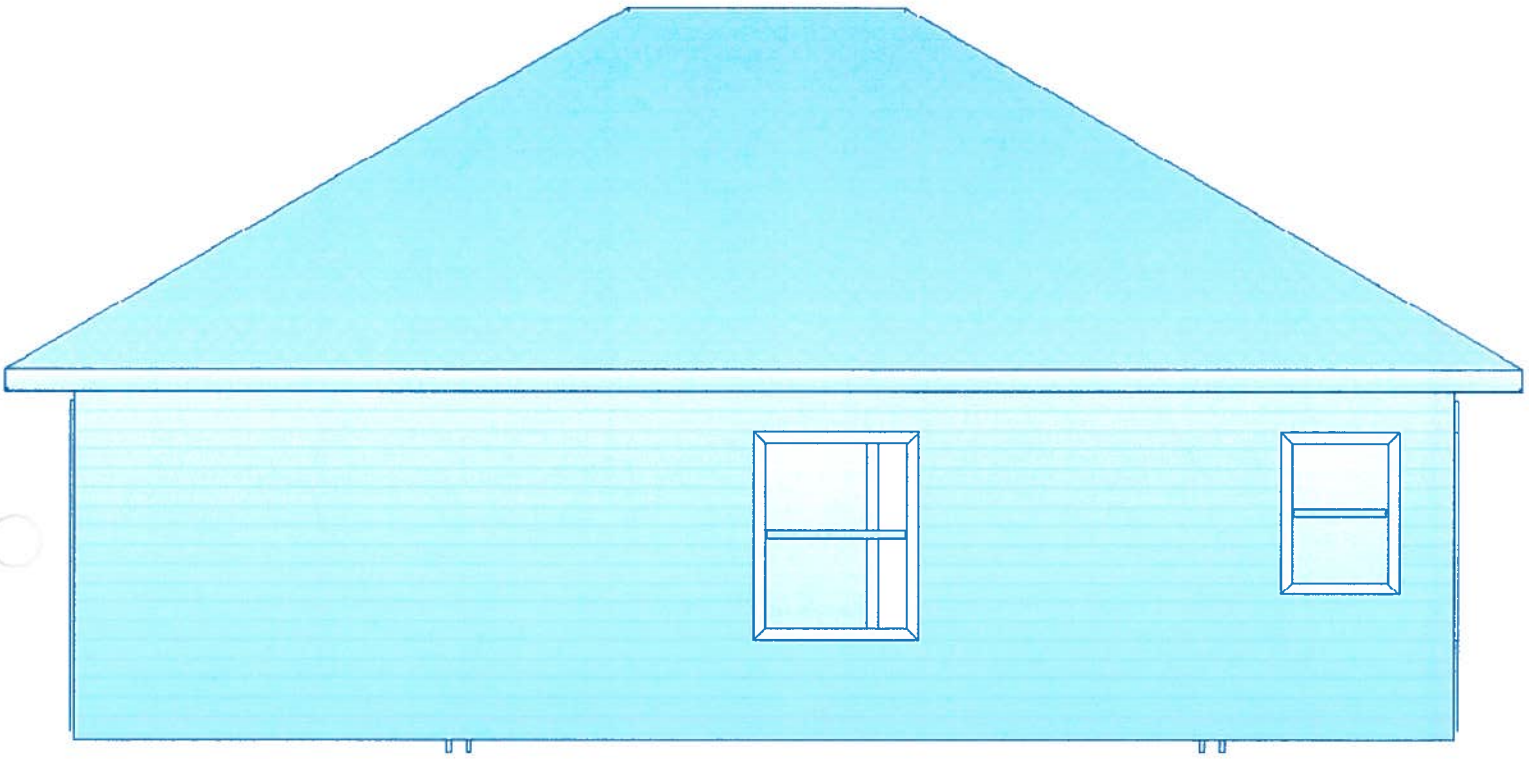




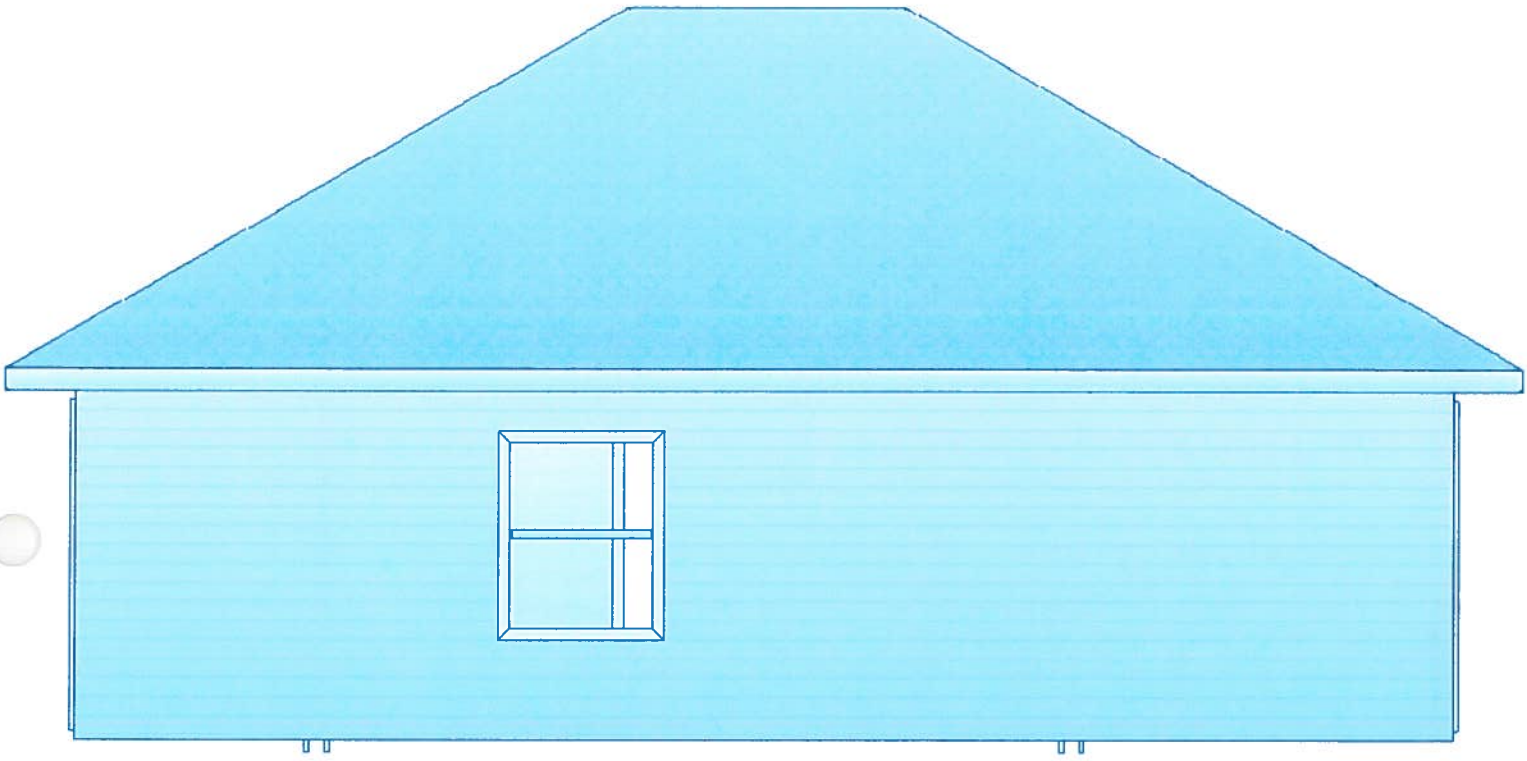
OLSZAK STORAGE SHED
FRONT ELEVATION



OLSZAK STORAGE SHED
REAR ELEVATION



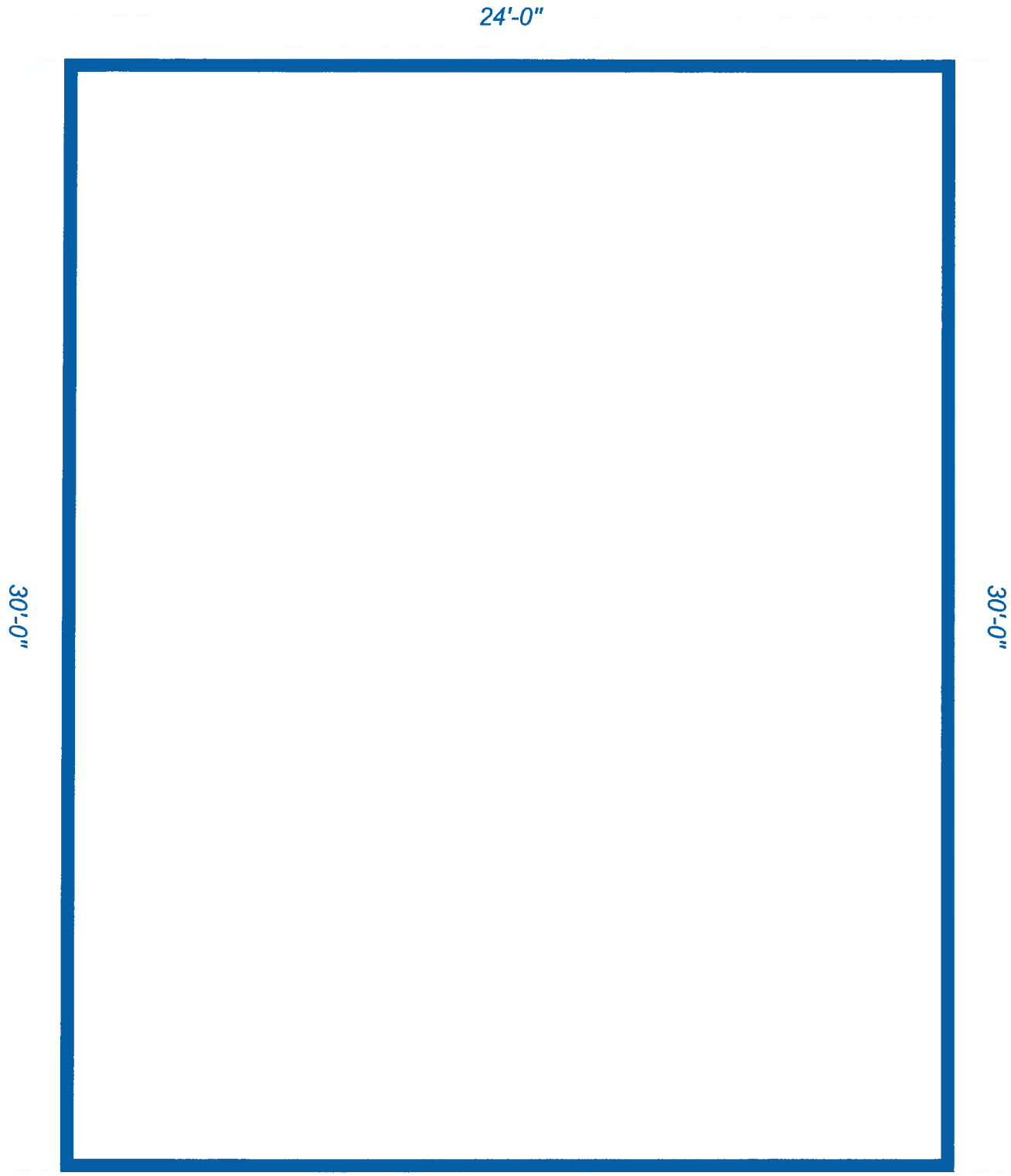
OLSZAK STORAGE SHED
LEFT ELEVATION



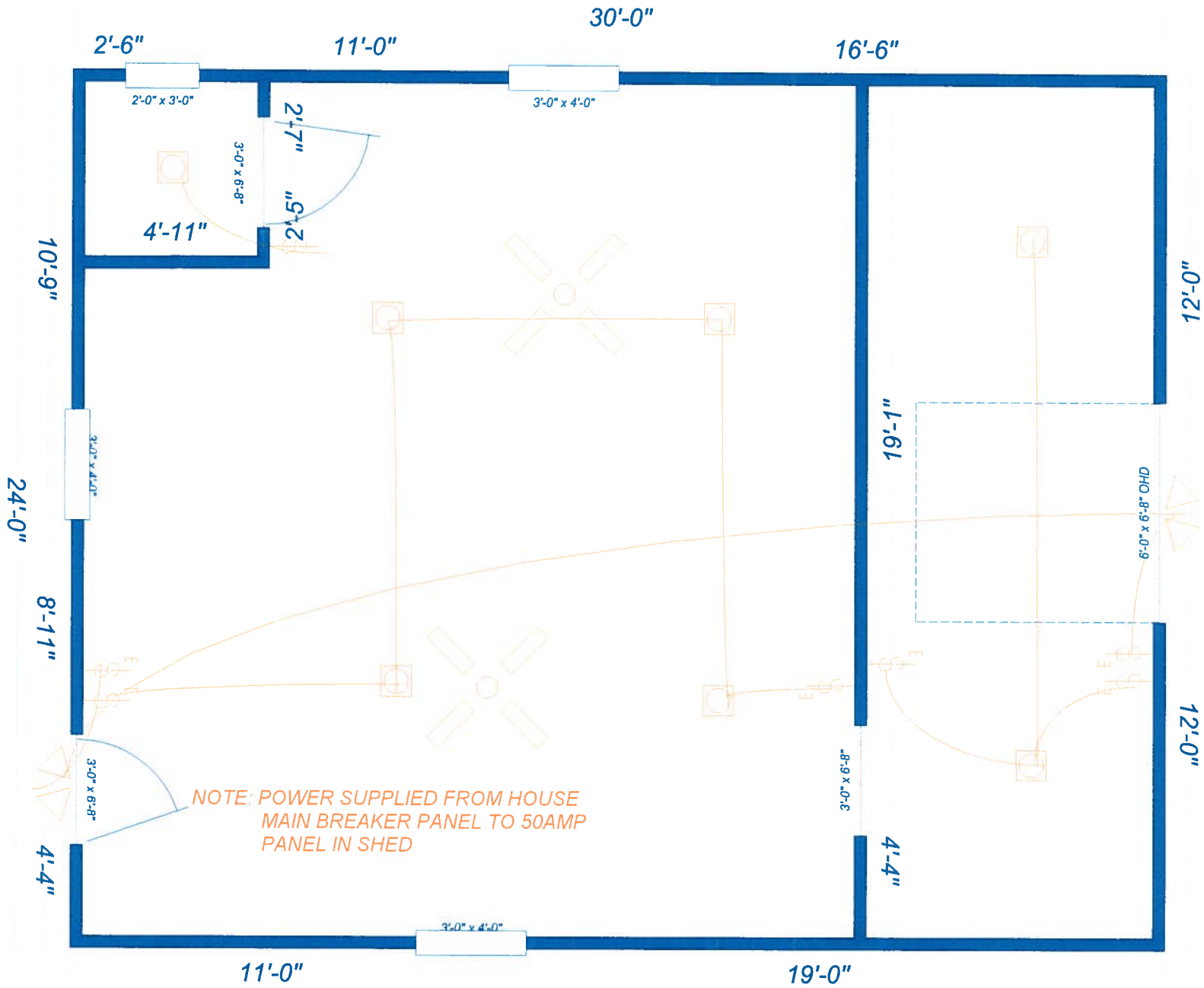
OLSZAK STORAGE SHED
RIGHT ELEVATION

OLSZAK STORAGE SHED ROOF LAYOUT

scale 1/4 inch = 1 foot



OLSZAK STORAGE SHED
scale: 1/4 in = 1 ft
foundation: Mono 4 inch slab, details attached



OLSZAK STORAGE SHED, electrical
720 SQUARE FEET
scale: 1/4 inch = 1 foot

WIND RESISTANCE ENGINEERING CALCULATIONS FOR OLSZAK RESIDENCE 200 SW Bay Pl Ft. White, FL

STORAGE SHED

DESIGNED IN ACCORDANCE WITH
REQUIREMENTS OF 2004 FLORIDA BUILDING CODE,
SECTION 1609 FOR 110 MPH WIND SPEED

6 Feb 06

THOMAS SPUTO, PH.D., P.E.
PE 39142

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1st AVENUE, GAINESVILLE, FLORIDA 32601
(352) 378-0448
CA 6855

SPUTO AND LAMMERT ENGINEERING, LLC
STRUCTURAL ENGINEERS
10 SW 1ST AVENUE, GAINESVILLE, FL 32601
PHONE: 352-378-0448 FAX: 352-373-1331
E-MAIL: sputoandlammert@mindspring.com

STORAGE SHED

Wind resistance of the referenced building has been designed using a wind speed of 110 mph as required by Section 1609, 2004 Florida Building Code.

ROOF SHEATHING: ½" Plywood or 7/16" OSB, installed without blocking. Use 8d common or 10-1/4 gage x 2" minimum length power nails at 6" o.c. at sheet edges and 12" o.c. in the sheet field. The roof acts as a structural diaphragm.

WALL SHEATHING: ½" Plywood or 7/16" OSB, installed with blocking at all horizontal sheet edges. Sheathing is installed from bottom to top plate to provide a continuous load path. Use 8d common or 10-1/4 gage x 2" minimum length power nails at 6" o.c. at vertical sheet edges, 6" o.c. at horizontal sheet edges, and 12" o.c. in the sheet field.

SHEARWALLS: See plan sheet for locations.

WALL STUDS: #2 Spruce or better 2x4 at 16" o.c.

ANCHOR BOLTS: ½" with 2" washer at maximum spacing of 48" o.c. Install one bolt within 6" of all corners, and within 6" of the ends of all windows and doors. (Anchor bolt alternate - 5/8" wedge anchor with 4" embed into concrete.) USE 2" ROUND OR SQUARE WASHERS AT EACH END OF ALL SHEARWALLS.

HURRICANE CLIPS: Sized as follows. Subject to revision by the engineer after review of engineering from truss manufacturer.

One Ply Truss (except Truss A1):	Simpson H10
Truss A1:	(2) Simpson H10
Truss to Truss:	Specified by truss manufacturer, IAW Wood Truss Council of America Standard WTCA 1-1995.

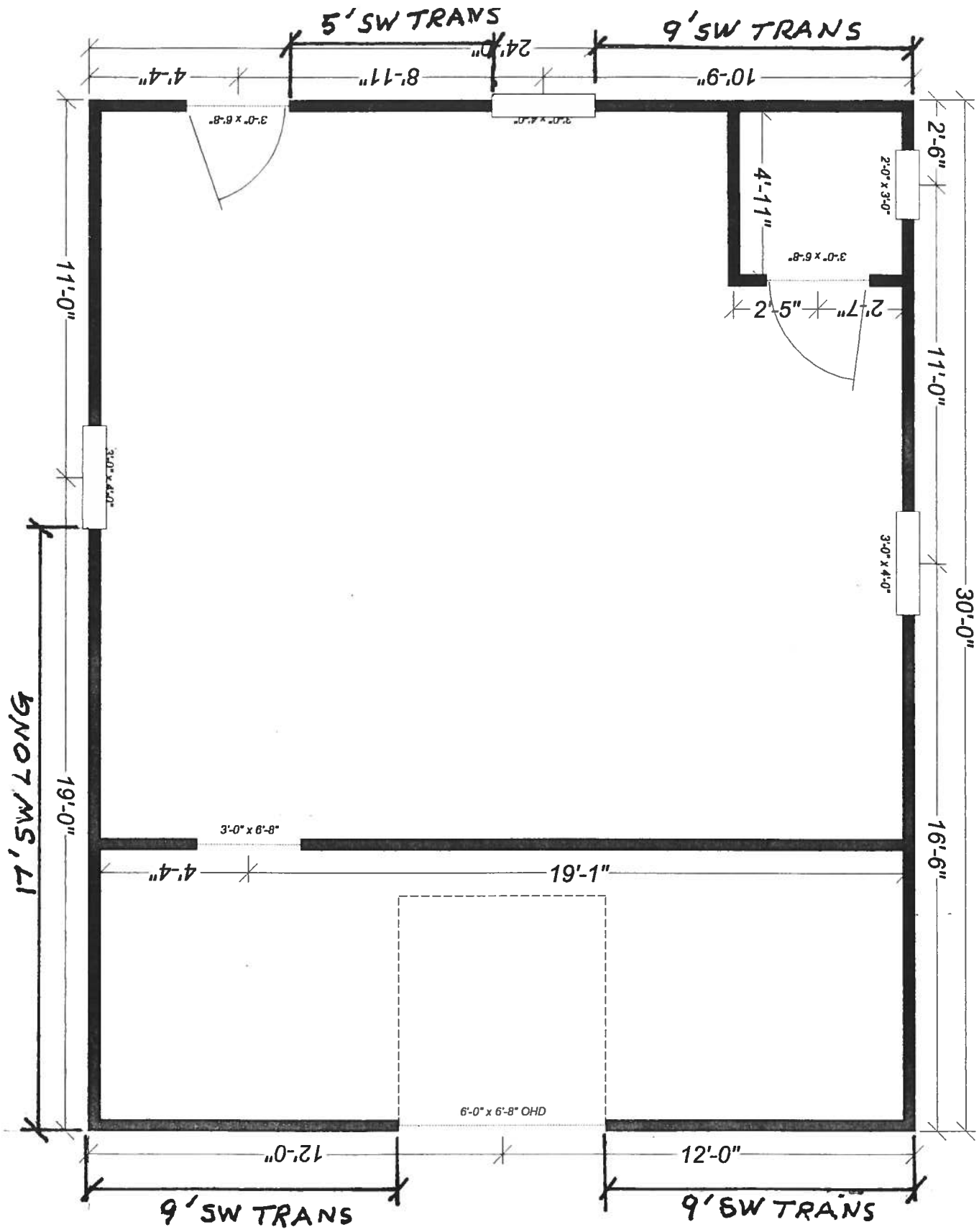
CONCRETE: All concrete shall have a 28 day compressive strength of 3000 psi.

REINFORCING STEEL: Grade 40 - #5 bars. All lap splices to be a minimum of 25 inches.

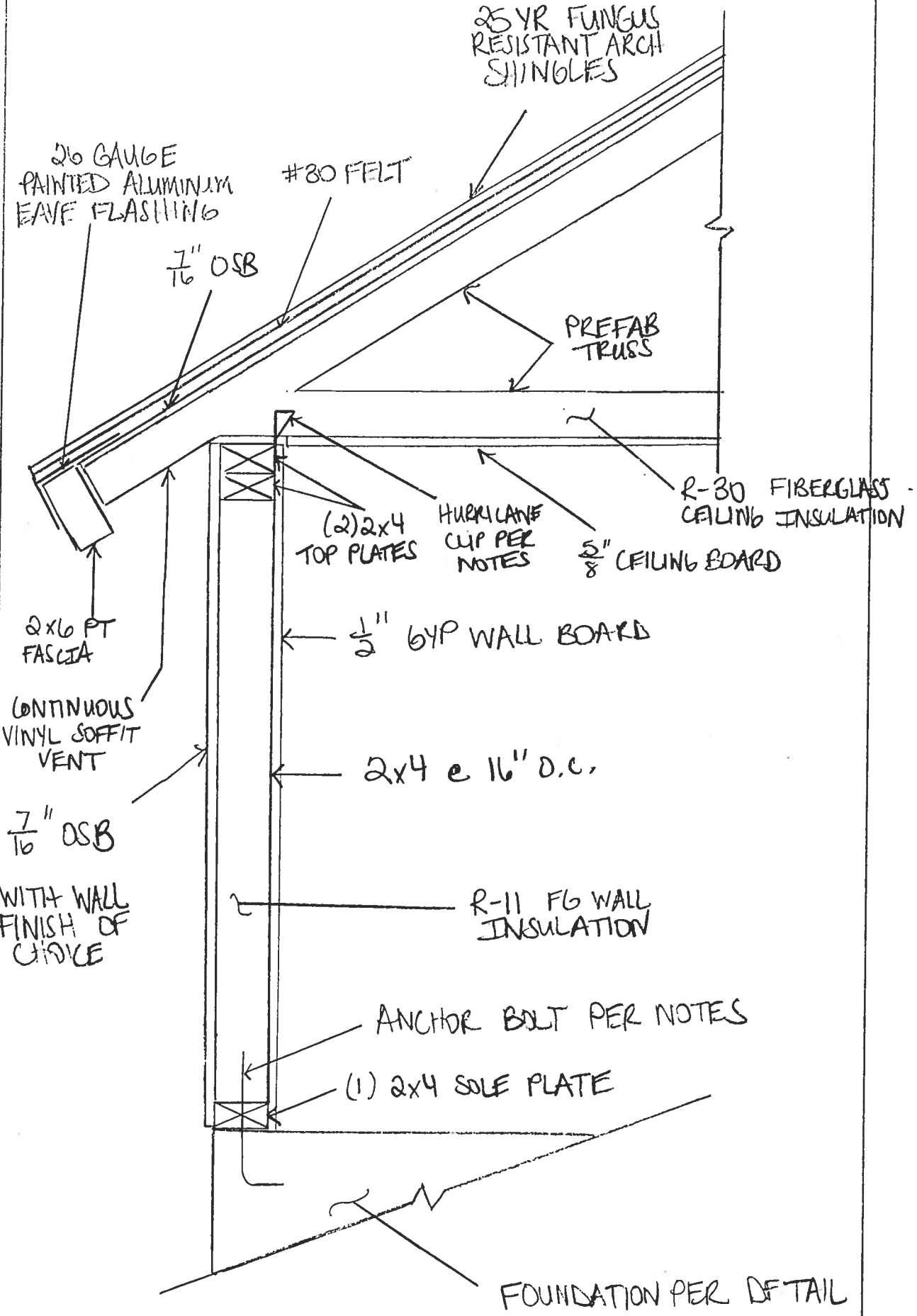
NOTE: 10-1/4 GAGE NAILS HAVE A DIAMETER OF 0.131 INCHES.

2004 Florida Building Code Section 1603.1.4 Information

Basic Wind Speed	=	110 mph
Importance Factor	=	1.00
Building Category	=	II
Wind Exposure	=	B
Internal Pressure Coefficient	=	+- 0.18
C & C Pressures	=	Zone 4 = 22.6 psf
		Zone 5 = 27.2 psf

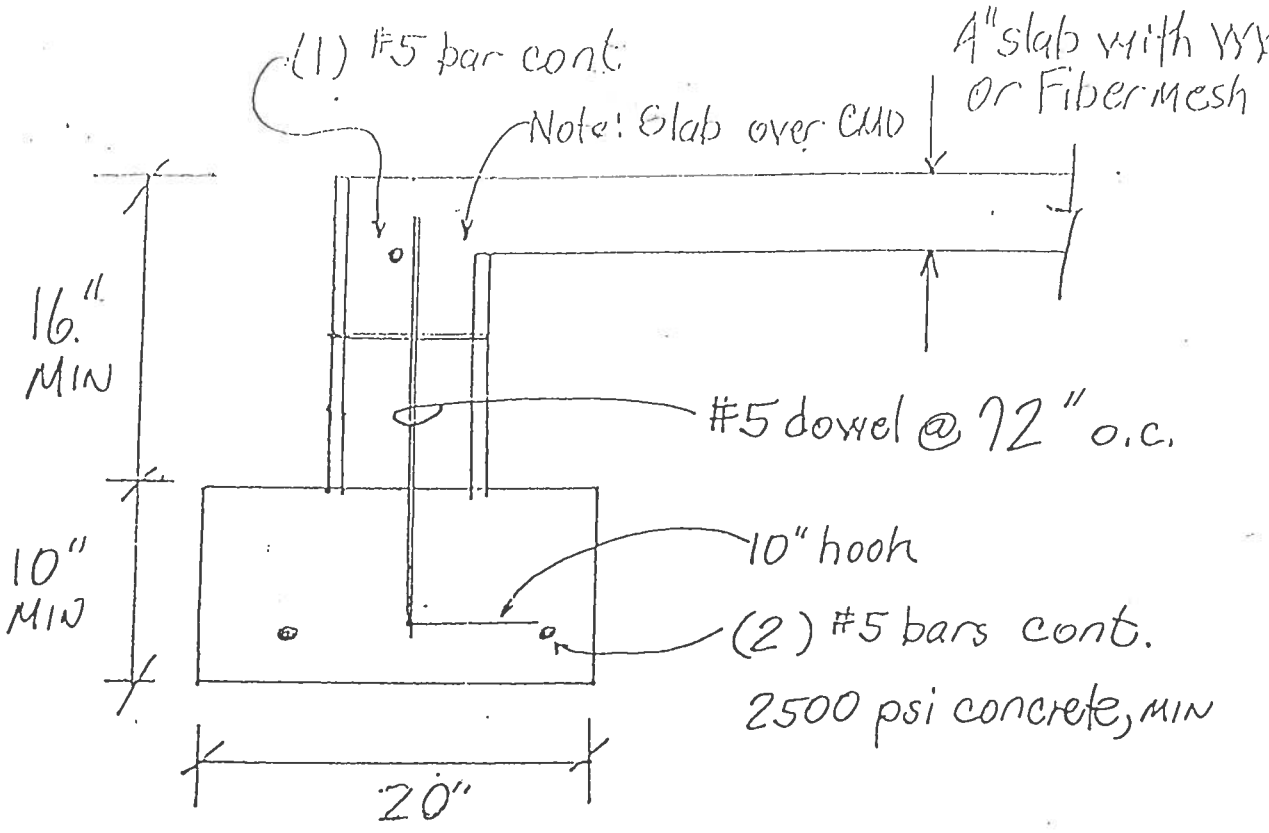

5'SW TRANS
9'SW TRANS
24'-0"
4'-4"
8'-11"
10'-9"
3'-0" x 6'-8"
3'-0" x 4'-0"
2'-0" x 3'-0"
4'-11"
2'-6"
11'-0"
2'-5"
2'-7"
3'-0" x 6'-8"
3'-0" x 4'-0"
3'-0" x 4'-0"
30'-0"
17'SW LONG
19'-0"
3'-0" x 6'-8"
4'-4"
19'-1"
16'-6"
6'-0" x 6'-8" OHD
12'-0"
12'-0"
9'SW TRANS
9'SW TRANS

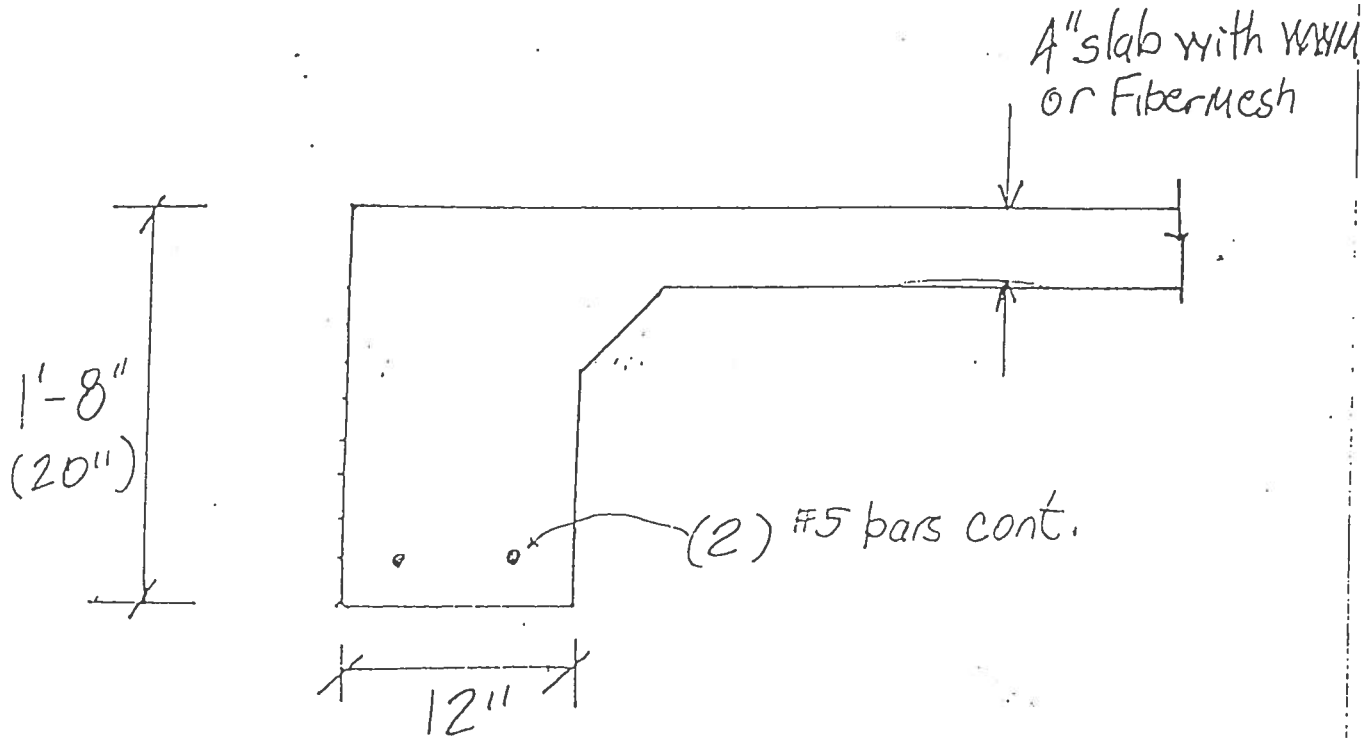
SHEAR WALLS

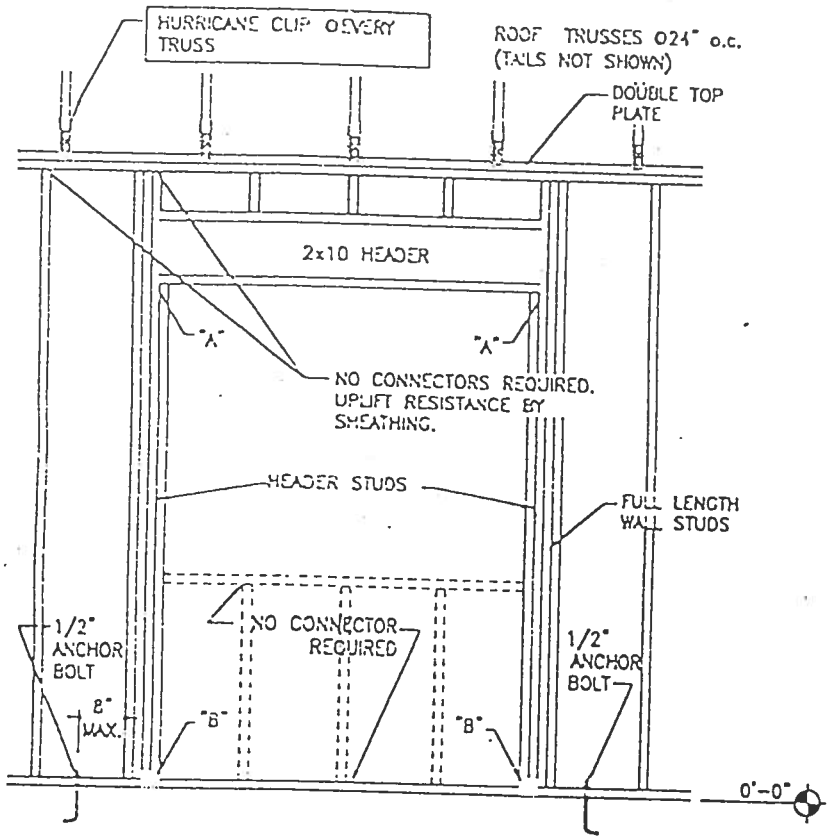

25 YR FUNGUS RESISTANT ARCH SHINGLES
26 GAUGE PAINTED ALUMINUM EAVE FLASHING
#30 FELT
7/16" OSB
PREFAB TRUSS
R-30 FIBERGLASS CEILING INSULATION
(2) 2x4 TOP PLATES
HURRICANE CLIP PER NOTES
5/8" CEILING BOARD
2x6 PT FASCIA
1/2" GYP WALL BOARD
CONTINUOUS VINYL SOFFIT VENT
2x4 @ 16" O.C.
7/16" OSB
WITH WALL FINISH OF CHOICE
R-11 FG WALL INSULATION
ANCHOR BOLT PER NOTES
(1) 2x4 SOLE PLATE
FOUNDATION PER DETAIL

All rebar - Grade 40
All concrete - 3000 psi MIN

Foundation Alternatives







OPENING FRAMING DETAIL

CONNECTOR SELECTION

SPAN	'A'	'B'	ANCHOR BOLTS
9'-0" AND UNDER	(1) LSTA24	(1) SP4 •	(1) EACH END
OVER 9'-0"	(2) LSTA24	(2) SP4 •	(1) EACH END

• USE SP6 ON 2X6 WALLS

NOTES:

1. CONNECTORS INDICATED ARE BY SIMPSON STRONG TIE CO., INC. PRE-APPROVED EQUAL MAY BE USED.

2. STANDARD WALL HEIGHT SHOWN. WALL HEIGHT MAY VARY. ADJUST HEAD AND SILL HEIGHT WITH CRIPPLES AS REQUIRED.

3. REFER TO HEADER HOLD DOWN CHART FOR NUMBER OF FULL-LENGTH AND HEADER STUDS REQUIRED FOR DIFFERENT OPENING WIDTHS.

4. SHEAR AND UPLIFT RESISTANCE PROVIDED BY SHEATHING. REFER TO STRUCTURAL ENGINEER'S NOTES ON THIS SHEET.

HEADER HOLD DOWNS

		MAXIMUM HEADER SPAN (IL)					
		3	6	9	12	15	18
		NUMBER OF HEADER STUDS SUPPORTING END OF HEADER					
		1	1	2	2	2	2
UNSUPPORTED WALL HEIGHT	STUD SPACING	NUMBER OF FULL-LENGTH STUDS AT EACH END OF HEADER					
10'-0" OR LESS	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
GREATER THAN 10'-0"	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

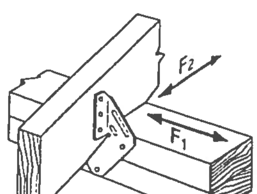
SIMPSON Strong-Tie

H Seismic & Hurricane Ties

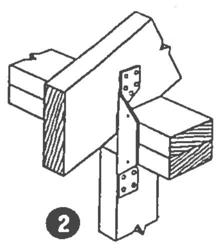
Available with additional corrosion protection. Check with factory.

Model No.	Ga	Fasteners: To Rafters/Truss	Fasteners: To Plates	Fasteners: To Studs	DF/SP Allowable Loads: Uplift (133)	DF/SP Uplift (160)	DF/SP Lateral (133/160) F_1	DF/SP Lateral (133/160) F_2	Uplift Load with 8dx1½" Nails (133 & 160)	SPF/HF Allowable Loads: Uplift (133)	SPF/HF Uplift (160)	SPF/HF Lateral (133/160) F_1	SPF/HF Lateral (133/160) F_2	Uplift Load with 8dx1½" Nails (133 & 160)	Code Ref.
H1	18	6-8dx1½	4-8d	—	490	585	485	165	455	400	400	415	140	370	
H2	18	5-8d	—	5-8d	335	335	—	—	335	230	230	—	—	230	2, 40, 82, 121, 140
H2.5	18	5-8d	5-8d	—	415	415	150	150	415	365	365	130	130	365	
H2.5A	18	5-8d	5-8d	—	600	600	110	110	480	520	535	110	110	480	[illegible], 122
H2.5T	18	5-8d	5-8d	—	545	545	135	145	425	545	545	135	145	425	
H3	18	4-8d	4-8d	—	455	455	125	160	415	320	320	105	140	290	2, 40, 82, 121, 140
H4	20	4-8d	4-8d	—	360	360	165	160	360	235	235	140	135	235	2, 40, 121, 140
H5	18	4-8d	4-8d	—	455	465	115	200	455	265	265	100	170	265	2, 40, 82, 121, 140
H5A	18	3-8d	3-8d	—	350	420	115	180	290	245	245	100	120	170	10
H6	16	—	8-8d	8-8d	915	950	650	—	—	785	820	560	—	—	5, 41, 121, 140
H7Z	16	4-8d	2-8d	8-8d	930	985	400	—	—	800	845	345	—	—	
H8	18	5-10dx1½	5-10dx1½	—	620	745	75	—	—	530	565	75	—	—	125
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	875	875	680	125	—	755	755	680	125	—	170
H10	18	8-8dx1½	8-8dx1½	—	905	990	585	525	—	780	850	505	450	—	9, 121
H10R	18	8-8dx1½	8-8dx1½	—	905	990	585	525	—	780	850	505	450	—	
H10-2	18	6-10d	6-10d	—	760	760	455	395	—	655	655	390	340	—	6, 121
H11Z	18	6-16dx2½	6-16dx2½	—	830	830	525	760	—	715	715	450	655	—	170
H14	18	[1] 12-8dx1½	13-8d	—	1350	1350	515	265	—	1050	1050	480	245	—	125
		[2] 12-8dx1½	15-8d	—	1350	1350	515	265	—	1050	1050	480	245	—	

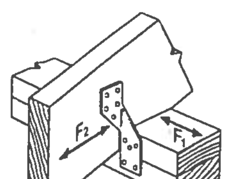
1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed, reduce where other loads govern.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate [illegible].
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. Allowable loads in the F_1 direction are not intended to replace diaphragm boundary members or prevent cross grain bending of the truss or rafter members. Additional shear transfer elements [illegible] cross grain bending [illegible]
5. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For [illegible] Continuous Load Path, connections [illegible] must be on same side of the wall.
6. Southern Pine allowable loads for H14: 1465 lbs *(133/160)*, 560 lbs *(F_1 Lateral 133/160)* and 285 lbs *(F_2 Lateral 133/160)*.
7. Refer to T-HTIEBEARING for selected hurricane ties allowable bearing enhancement loads.
8. NAILS: 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10dx1½ = 0.148" dia. x 1½" long, 8d = 0.131" dia. x 2½" long, 8dx1½ = 0.131" dia. x 1½" long. See page 16-17 for other nail sizes [illegible].



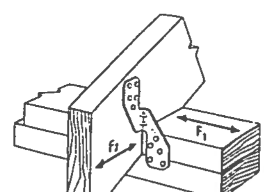
1 H1 Installation



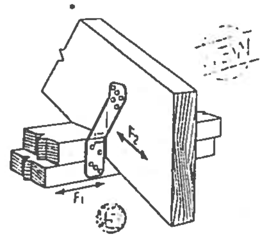
2 H2 Installation



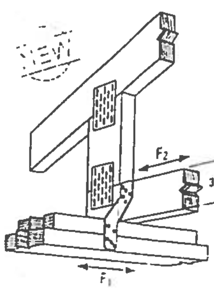
3 H2.5 Installation
(Nails into both top plates)

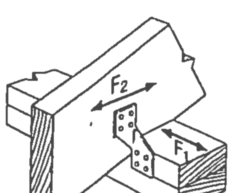


4 H2.5A Installation
(Nails into both top plates)

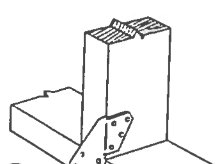


5 H2.5T Installation
(Nails into both top plates)

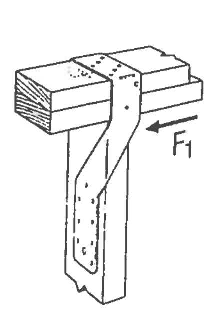




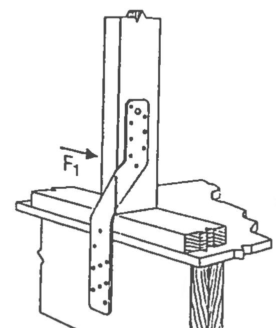
7 H3 Installation
(Nails into upper top plate)



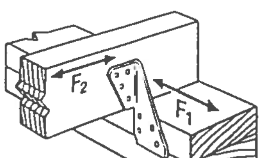
8 H4 Installation
(H2.5 similar)
(see footnote 3, page 142)



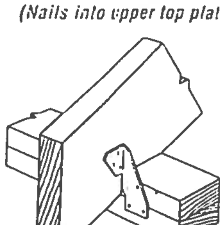
12 H6 Stud to Top Plate Installation



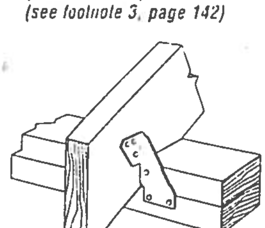
13 H6 Stud to Band Joist Installation



9 H4 Installation
(Nails into upper top plate)



10 H5 Installation
(Nails into both top plates)



11 H5A Installation
(Nails into both top plates)

Straps & Ties

HRS/ST/FHA/PS/HST/LSTA/LSTI/MST/MSTA/MSTC/MSTI Strap Ties

SIMPSON

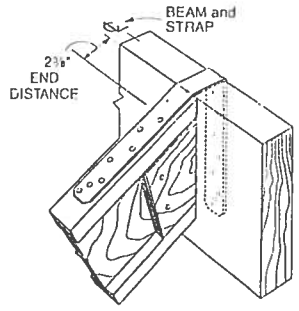
CODES: See page 12 for Code Listing Key Chart.

Available with additional corrosion protection. Check with factory.

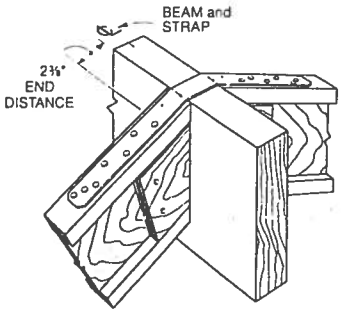
Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads (DF/SP) (133)	Allowable Tension Loads (DF/SP) (160)	Allowable Tension Loads (SPF/HF) (133)	Allowable Tension Loads (SPF/HF) (160)	Code Ref.
LSTA9	20	1¼	9	8-10d	645	775	555	665	7, 62, 90, 128
LSTA12		1¼	12	10-10d	805	970	695	830	
LSTA15		1¼	15	12-10d	970	1160	830	1000	
LSTA18		1¼	18	14-10d	1130	1235	970	1165	
LSTA21		1¼	21	16-10d	1235	1235	1110	1235	
LSTA24		1¼	24	18-10d	1235	1235	1235	1235	
ST292		2 1/16	9 5/16	12-16d	1120	1265	970	1160	3, 39, 88, 104, 121, 128
ST2122		2 1/16	12 13/16	16-16d	1505	1535	1290	1535	
ST2115		¾	16 5/16	8-16d	665	665	665	665	
ST2215		2 1/16	16 5/16	20-16d	1880	1880	1625	1880	
LSTA30	18	1¼	30	22-10d	1640	1640	1555	1640	7, 62, 90, 128
LSTA36		1¼	36	24-10d	1640	1640	1640	1640	
LSTI49		3¾	49	32-10dx1½	2580	3100	2220	2660	9, 121, 128
LSTI73		3¾	73	48-10dx1½	3870	4215	3330	3995	9, 128
MSTA9		1¼	9	8-10d	650	780	565	680	7, 62, 90, 123, 128
MSTA12		1¼	12	10-10d	815	975	705	850	
MSTA15		1¼	15	12-10d	975	1170	850	1020	
MSTA18		1¼	18	14-10d	1140	1365	990	1185	
MSTA21		1¼	21	16-10d	1300	1560	1130	1355	7, 62, 90, 128
MSTA24		1¼	24	18-10d	1465	1640	1270	1525	
MSTA30	16	1¼	30	22-10d	1835	2050	1585	1900	
MSTA36		1¼	36	26-10d	2050	2050	1870	2050	
ST6215		2 1/16	16 5/16	20-16d	1895	2095	1640	1970	3, 39, 88, 104, 121, 128
ST6224		2 1/16	23 5/16	28-16d	2540	2540	2315	2540	
ST9		1¼	9	8-16d	755	910	655	785	3, 39, 88, 104, 121, 128
ST12		1¼	11⅝	10-16d	945	1135	820	985	3, 39, 88, 121, 128
ST18		1¼	17¾	14-16d	1325	1420	1150	1380	
ST22		1¼	21⅝	18-16d	1420	1420	1420	1420	
MSTC28		3	28¼	36-16d sinkers	3000	3600	2590	3110	9, 23, 121, 128
MSTC40		3	40¼	52-16d sinkers	4335	4585	3745	4495	9, 23, 128
MSTC52		3	52¼	62-16d sinkers	4585	4585	4465	4585	
MSTC66	14	3	65¾	76-16d sinkers	5660	5660	5660	5660	
MSTC78		3	77¾	76-16d sinkers	5660	5660	5660	5660	
ST6236		2 1/16	33 13/16	40-16d	3845	3845	3465	3845	3, 39, 88, 104, 121, 128
HRS6	12	1⅜	6	6-10d	525	630	455	545	128
HRS8		1⅜	8	10-10d	875	1050	760	910	
HRS12		1⅜	12	14-10d	1225	1465	1065	1275	
FHA6		1 7/16	6⅜	8-16d	810	975	705	845	3, 39, 88, 121, 128
FHA9		1 7/16	9	8-16d	810	975	705	845	
FHA12		1 7/16	11⅝	8-16d	810	975	705	845	
FHA18		1 7/16	17¾	8-16d	810	975	705	845	
FHA24		1 7/16	23⅞	8-16d	810	975	705	845	
FHA30		1 7/16	30	8-16d	810	975	705	845	
MSTI26		2 1/16	26	26-10dx1½	2355	2830	2045	2455	
MSTI36		2 1/16	36	36-10dx1½	3265	3915	2830	3400	
MSTI48		2 1/16	48	48-10dx1½	4350	5080	3775	4530	
MSTI60		2 1/16	60	60-10dx1½	5080	5080	4720	5080	
MSTI72		2 1/16	72	64-10dx1½	5080	5080	5080	5080	3, 39, 121, 128

1. Loads include a 33% or 60% load duration increase on the fasteners for earthquake or wind loading, but DO NOT include a 33% stress increase on the steel capacity. Refer to page 13 for further explanation.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers or 10d commons may be substituted where 16d commons are specified at 0.85 of the table loads.
5. Use half of the nails in each member being connected to achieve the listed loads.
6. PS strap design loads must be determined by the for each installation. Bolts are installed both perpendicular and parallel-to-grain. Hole diameter in the part may be oversized to accommodate the HDG. Designer must determine if the oversize creates an unacceptable installation.
7. For overlap splice details, refer to T-CMST.

with no reduction in load
x 3" long

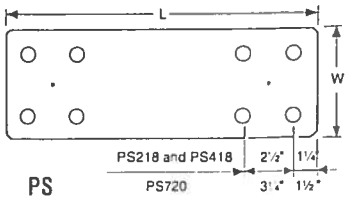


Typical LSTA Installation *(hanger not shown)*

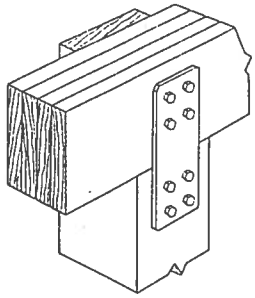


Typical LSTA Installation *(hanger not shown)*

Model No.	Material Thickness mil (ga)	Dim. W	Dim. L	Bolts Qty	Bolts Dia	Code Ref.
PS218[6]	171 mil (7 ga)	2	18	4		180
PS418[6]		4	18	4		
PS720[6]		6¾	20	8	½	



PS



Typical PS720 Installation

DSP/SSP/SP/SPH/RSP4 Stud Plate Ties

This product is preferable to similar connectors because of a) easier installation, b) higher loads, c) lower installed cost, or a combination of these features.

DSP and SSP provide flexibility in the field – can be used as a plate to stud connection AND top plate to stud connection.

The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.

MATERIAL: DSP/SSP/SPH–18 gauge, all others–20 gauge

FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 6-7.

INSTALLATION: • Use all specified fasteners; see General Notes.
- DSP/SSP–sill plate installation-fill all round holes.
- DSP/SSP–top plate installation-fill all round and triangle holes
- SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.

CODES: See page 12 for Code Listing Key Chart.

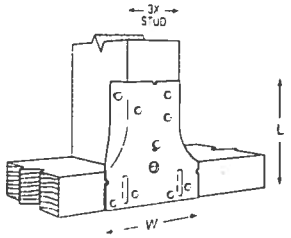
Available with additional corrosion protection. Check with factory.

Model No.	Dim. W	Dim. L	Fasteners: Studs	Fasteners: Double Top Plate	Fasteners: Single Sill Plate	Allowable Uplift Loads (133/160): Double Top Plate DF/SP/SPF	Single Sill Plate DF/SP	Single Sill Plate SPF/HF	Code Ref.
SSP	1⅜	6 1/16	4-10dx1½	3-10dx1½	—	350	—	—	62 125
				—	1-10dx1½	—	420	325	
			4-10d	3-10d	—	435	—	—	
				—	1-10d	—	455	420	
DSP	2¾	6 1/16	8-10dx1½	6-10dx1½	—	775	—	—	
				—	2-10dx1½	—	660	545	
			8-10d	6-10d	—	825	—	—	
				—	2-10d	—	825	600	

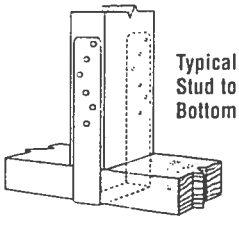
1. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed.
2. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
3. Allowable loads for DSP installed to a rim joist are 660 lbs. (DF/SP), 545 lbs. (SPF/HF).
4. NAILS: 10d = 0.148" dia. x 3" long. 10dx1½ = 0.148" dia. x 1½" long. See page 16-17 for other nail sizes and information.

Model No.	Dim. W	Dim. L	Stud	Plate Width	Fasteners: Stud[1]	Fasteners: Plate	Allowable Uplift Loads DF/SP (133)[2]	DF/SP (160)[2]	SPF (133)	SPF (160)	Code Ref.
SP1	3½	5 1/16	2x	—	6-10d	4-10d	585	585	535	535	6, 121
SP2	3½	6⅝	2x	—	6-10d	6-10d	890	1065	605	605	160
SP3	4½	6⅝	3x	—	6-10d	6-10d	890	1065	605	605	7, 121
SP4	3 9/16	7¼	2x	4x	6-10dx1½	—	735	885	630	760	160
SP5	4½	5 1/16	3x	—	6-10d	4-10d	585	585	535	535	7, 121
SP6	5 9/16	7¾	2x	6x	6-10dx1½	—	735	885	630	760	
SP8	7 5/16	8 5/16	2x	8x	6-10dx1½	—	735	885	630	760	
SPH4	3 9/16	8¾	2x	4x	10-10dx1½	—	1240	1240	1065	1065	62, 123
					12-10dx1½	—	1360	1360	1170	1170	
SPH6	5 9/16	9¼	2x	6x	10-10dx1½	—	1240	1240	1065	1065	
					12-10dx1½	—	1360	1360	1170	1170	
SPH8	7 5/16	8⅜	2x	8x	10-10dx1½	—	1240	1240	1065	1065	
					12-10dx1½	—	1360	1360	1170	1170	
RSP4(1)	2⅛	4½	2x	—	4-8dx1½	4-8dx1½	315	315	285	285	6, 30, 99, 121
RSP4(2)	2⅛	4½	2x	—	4-8dx1½	4-8dx1½	450	450	370	370	

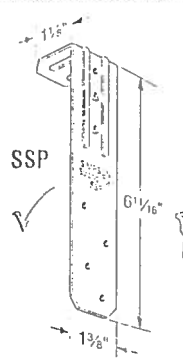
1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load *(see illustration)*
2. Allowable loads have been increased for earthquake or wind loading; no further increase allowed. Reduce where other loads govern.
3. RSP4–see installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs *(installation 1)* and 305 lbs *(installation 2)*. F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
7. For retrofit application see T-STRAP.

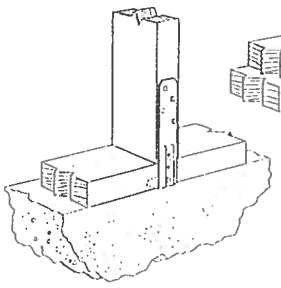


Typical SP5 Installed *(SP3 similar installed at double top plate)*

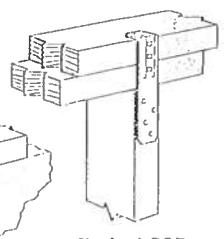


Typical SPH4 Stud to Single Bottom Plate

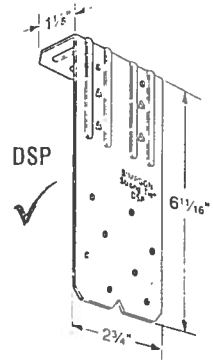


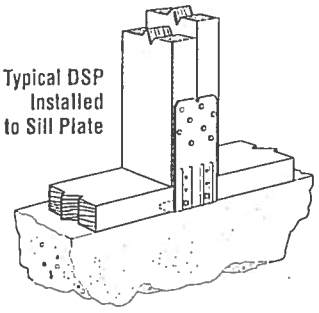


Typical SSP Installed to Sill Plate

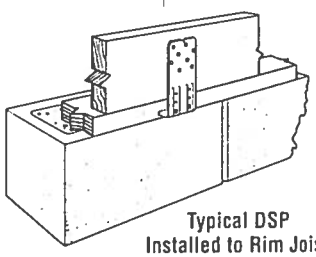


Typical SSP Installed to Top Plate

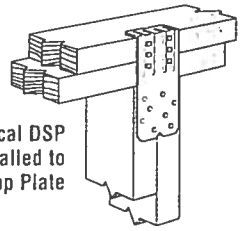




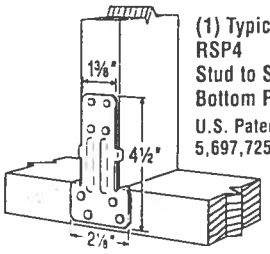
Typical DSP Installed to Sill Plate



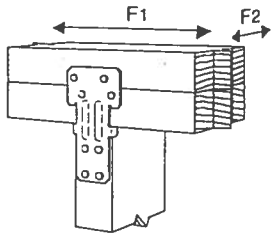
Typical DSP Installed to Rim Joist



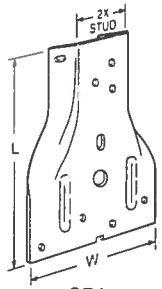
Typical DSP Installed to Top Plate



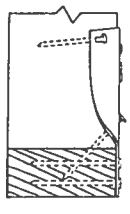
(1) Typical RSP4 Stud to Single Bottom Plate
U.S. Patent 5,697,725



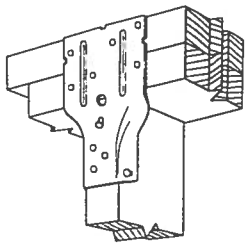
(2) Typical RSP4 Stud to Double Top Plate *(See footnote 4)*



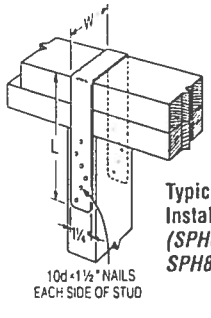
SP1



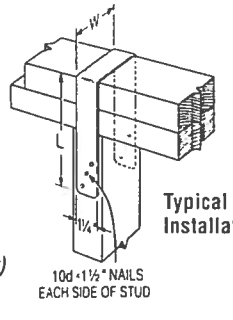
SP1 Nailing Profile



Typical SP2 Installation



Typical SPH4 Installation *(SPH6 and SPH8 similar)*



Typical SP4 Installation

WIND LOAD DESIGN PER 2004 FBC (1609.6)

SINGLE STORY BUILDING

BUILDING DIMENSIONS:

L =	30	FEET			STORAGE SHED
W =	24	FEET			
EAVE =	8.08	FEET			
PITCH =	7	/12	=	30.3 DEG.	
O'HANG =	1.5	FEET			
RIDGE =	15.08	FEET			
MEAN RF =	11.58	FEET			

WIND EXPOSURE:

VELOCITY =	110	MPH
I =	1.00	(IMPORTANCE FACTOR)
EXPOSURE =	B	
ADJUSTMENT	1.00	(PER TABLE 1609.6D)

MWFRS PRESSURE PER TABLE 1609.6A (BASE PRESSURE W/O ADJUSTMENT)
(PRESSURES IN PSF)

TRANSVERSE WIND DIRECTION

	END ZONE		INTERIOR ZONE	
HORIZONTAL LOADS				
	WALL	ROOF	WALL	ROOF
	21.6	14.8	17.2	11.8
VERTICAL LOADS				
	WINDWD	LEEWD	WINDWD	LEEWD
	8.3	-13.1	7.2	-11.3
O'HANG	-7.6		-8.7	

LONGITUDINAL WIND DIRECTION

	END ZONE		INTERIOR ZONE	
HORIZONTAL LOADS				
	WALL	ROOF	WALL	ROOF
	19.2	-10.0	12.7	-5.9
VERTICAL LOADS				
	WINDWD	LEEWD	WINDWD	LEEWD
	-23.1	-13.1	-16.0	-10.1
O'HANG	-32.3		-25.3	

CALCULATE EDGE STRIPS:

2.4	FEET	(10% OF LEAST DIM)		
3.232	FEET	(40% OF EAVE)		
			Least =	2.4 FEET
0.96	FEET	(4% OF LEAST DIM)		
3	FEET	(3 FEET)		
			Max =	3 FEET

A =	3	FEET
2A =	6	FEET

HORIZONTAL TRANSVERSE LOAD

ROOF	2730	LBS.

WALL	4596	LBS.

LONGITUDINAL TRANSVERSE LOAD

ROOF	1203	LBS.
WALL	3093	LBS.

ROOF DIAPHRAM — 7/16" OSB

TRANSVERSE — 8d COMMON OR 0.131" DIA. P-NAIL

				6"/12"	4"/12"	3"/12"	
TOTAL DRAGSTRUT LENGTH =	48	FEET					
LOAD RESISTED =	2730	ROOF		357	476	707	PINE
	2298	WALL		OK	OK	OK	
	5028	TOTAL					
	104.7	*PLF*					

LONGITUDINAL

				6"/12"	4"/12"	3"/12"	
TOTAL DRAGSTRUT LENGTH =	60	FEET					
LOAD RESISTED =	1203	ROOF		357	476	707	PINE
	1547	WALL		OK	OK	OK	
	2750	TOTAL					
	45.8	*PLF*					

SHEARWALLS — 7/16" OSB

TRANSVERSE — 8d COMMON OR 0.131" DIA. P-NAIL

				6"/12"	4"/12"	3"/12"	
TOTAL SHEARWALL LENGTH =	32	FEET					
LOAD RESISTED =	2730	ROOF		364	532	686	PINE
	2298	WALL		OK	OK	OK	
	5028	TOTAL					
	157.1	*PLF*		298	436	563	SPRUCE
				OK	OK	OK	

LONGITUDINAL

				6"/12"	4"/12"	3"/12"	
TOTAL SHEARWALL LENGTH =	17	FEET					
LOAD RESISTED =	1203	ROOF		364	532	686	PINE
	1547	WALL		OK	OK	OK	
	2750	TOTAL					
	161.8	*PLF*		298	436	563	SPRUCE
				OK	OK	OK	

WALL TENSION TIE USING SHEATHING

WALL TO WALL	O'HANG		UPLIFT LOAD		3/8" MINIMUM SHEATHING 8d COMMON NAIL SPACING SPRUCE	PINE	
24	1.5		158	PLF	7.1	8.9	INCHES

ANCHOR BOLT SPACING

WALL TO WALL	UPLIFT LOAD		1634 2" ROUND WASHER		3173 3" SQ WASHER	
24	158		124.1	IN. MAX	241.1	IN. MAX

SHEARWALL ANCHORAGE

			1634 2" ROUND WASHER	3173 3" SQ WASHER
TRANSVERSE				
CHORD FORCE =	1270	LBS		
ANCHOR BOLT =	48	INCHES O.C.		
REQUIRED FORCE =	1585	LBS	OK	OK
LONGITUDINAL				
CHORD FORCE =	1307	LBS		
REQUIRED FORCE =	1307	LBS	OK	OK

WALL STUD DESIGN

DESIGN PRESSURES: 22.6 PSF INTERIOR ZONE
27.2 PSF END ZONE

INTERIOR ZONE STUDS

STUD LENGTH FEET	SPACING INCHES	INTERIOR MOMENT IN-#		#2 SPF 2X4	#2 PINE 2X4	#2 SPF 2X6	#2 PINE 2X6	#2 PINE 3X4
			Sx	3.06	3.06	7.56	7.56	5.11
			Fb allow	2415	2760	2093	2300	2760
8	16	2893		945	945	383	383	566
				OK	OK	OK	OK	OK
0	0	0		0	0	0	0	0
				OK	OK	OK	OK	OK
0	0	0		0	0	0	0	0
				OK	OK	OK	OK	OK
0	0	0		0	0	0	0	0
				OK	OK	OK	OK	OK

END ZONE STUDS WITHIN	3 FEET OF CORNERS	END ZONE MOMENT IN-#					
8	16	3482	1138 OK	1138 OK	461 OK	461 OK	681 OK
0	0	0	0 OK	0 OK	0 OK	0 OK	0 OK
0	0	0	0 OK	0 OK	0 OK	0 OK	0 OK
0	0	0	0 OK	0 OK	0 OK	0 OK	0 OK

SHEARWALL CAPACITIES PER 2004 FBC

NAIL = 8d COMMON OR 0.131" POWER NAIL
SHEATHING = 7/16" OSB

6"/12"	4"/12"	3"/12"	
260	380	490	Per Table 2306.4.1, using 15/32" sheathing as allowed by para 2306.4.1
0.82	0.82	0.82	1.00 = Pine, 0.82 = SPF
1.4	1.4	1.4	Increase per para 2306.4.1
298	436	563	

SHEARWALL CAPACITIES PER 2001 SBC

NAIL = 8d COMMON OR 0.131" POWER NAIL
SHEATHING = 7/16" OSB

6"/12"	4"/12"	3"/12"	
260	380	490	Per Table 2310.2B, using 15/32" sheathing as allowed by para 2310.4.6
1	1	1	1.00 = Pine, 0.82 = SPF
1.4	1.4	1.4	Increase per para 2313.2.5
364	532	686	