

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI, A SLUMP OF 3" FOR FOOTINGS/FOUNDATIONS AND 4" FOR SLABS
2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6".
4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS.
5. HORIZONTAL FOOTING BARS SHALL HAVE 1'-0" HOOK LENGTH OR CORNER BARS WITH A 2'-1" LAP PROVIDED
6. MINIMUM LAP SPACES ON ALL REINFORCING BAR SPICES SHALL BE 40 BAR DIAMETERS TYP.
7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO FORM.

ALL REINFORCING STEEL SHALL BE NEW DEFORMED BARS FREE FROM RUST, SCALE & OIL & SHALL MEET ASTM A-615 REINFORCING FOR FOOTING SHALL BE SUPPLIED ON PRE-CAST CONCRETE PADS, TOP REINFORCING SHALL BE SUPPLIED ON TEMPORARY STRINGS OR JOEWEL FOR COLUMNS & FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS-REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING WHERE PERMITTED SHALL BE THE FOLLOWING MINIMUM, UNLESS OTHERWISE INDICATED ON THE DRAWINGS:

FTGS, WALLS, SLABS, BEAMS, SLABS:	36 DIA. OR 2'-0" MIN.
FOOTING CELL REINFORCING:	40 DIA. OR 2'-1" MIN.
TEMPERATURE REINFORCING:	20 DIA. OR 1'-0" MIN.
WELDED WIRE MESH:	8" LAP

1. HOLLOW LUD LOAM BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI ($f'_m = 1500 \text{ PSI}$)
2. MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270.
3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11".
4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT.
5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 492 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED.
6. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS.
7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM, PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.

1. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", 2018 EDITION.
2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER 2 GRADE OR BETTER SHALL BE USED REGARDLESS OF SPECIES.
3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THEM MUST BE CUTTER TO 1" DIA. SHALL HAVE 1/2" PROTECTION SHIELDS FOR ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES,TYP., U.N.O.
4. FASTENERS FOR PRESSURE PRESERVATIVE AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT-DIPPED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER.

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS.
2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MINIMUM OF 40% ALLOWABLE STRESS INCREASE) FOR A LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.
4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.
5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LOADS:
6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE PTI LATEST EDITION.
7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES . SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRACING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 2 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION:
2000 PSF SEE SOIL REPORTS AND SPECIFICATIONS FOR COMPACTION
REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET
OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL
CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION
OF FOUNDATION DESIGN. SOIL TO BE COMPACTED TO AT LEAST 95% OF
MAX. DRY DENSITY AS DETERMINED BY ASTM - D1557

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF THESE WALLS.

1. MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. EPOXY ANCHORS WITH 6" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS. SEE PLAN FOR EMBEDMENT DEPTH AT FLOOR STEPS.
2. FOR MISSED VERT. DOWELS DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON "SET", EPOXY), MIXED PER MANUFACTURER'S INSTRUCTIONS AND CURE THAT PERIOD OF TIME AFTER DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.
3. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CORN. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)
4. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 2 1/2" TITENS TO MASONRY AND (7) 10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW. IF GIRDER TRUSS CONNECTIONS ARE MISSED CONTACT ENGINEER OF RECORD FOR SUBSTITUTION.

SECTION R318 PROTECTION AGAINST TERMITES

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOLI APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT. A COMPLETE TREATMENT (SEE DEFINITION) SHALL BE PROVIDED IMMEDIATELY UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES IN ACCORDANCE WITH THE REQUIREMENTS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES".

WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE IS TO BE IMPLEMENTED. CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I.. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 PSI SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.

REF. NO. NO. DE REP.		USP		REF. NO. NO. DE REP.		USP		REF. NO. NO. DE REP.		USP		REF. NO. NO. DE REP.		USP	
105D	110C-5	CB35G66-SBS2HDG	CB35G66-HDG	RH48	HH66	L87A18	L87A18	MTM106	MTM106	MTM106	MTM106	RT42C	TT42C72	RT42C	TT42C72
105D	110C-5	CB35G66-SBS2HDG	CB35G66-HDG	RH48	HH66	L87A18	L87A18	MTM106	MTM106	MTM106	MTM106	RT42C	TT42C72	RT42C	TT42C72
1121H-HDG	LH12-HDG	CCQ4585D5.5	CCQ4585.5	LH13	KHL33	L87A24	L87A24	MTM122	MTM122	MTM122	MTM122	RTT2	TTT2	RTT2	TTT2
1121H-HDG	TH12-HDG	CCQ6086D5.5	CCQ6086	LH33HDG	KHL33-HDG	L87A30	L87A30	MTM16	MTM16	MTM16	MTM16	RTR	TTU2	RTR	TTU2
L12L	L12	CP13	CP13	L12	CP13	L87A38	L87A38	MTM20	MTM20	MTM20	MTM20	RTV	TTV2	RTV	TTV2
1121T	T112	CPB5	CPB55	LH33HDG	KHL33-HDG	L87A9	L87A9	MTM18	MTM18	MTM18	MTM18	SD0112	LH31850	SD0112	LH31850
120T	T12	CPB6	CPB66	LH33HDG	KHL33-HDG	L87A9	L87A9	MTM18	MTM18	MTM18	MTM18	SD0122	LH33850	SD0122	LH33850
120ND	N12C-5	RC14R	RC14R	LH33HDG	KHL33-HDG	L87A9	L87A9	MTM18	MTM18	MTM18	MTM18	SD0122	LH33850	SD0122	LH33850
160HDG	N15C-1	CS16	RE150	HSR416E2	HSR416E2	LSUJ2E	LSU11E-7	MTM16E	MTM16E	MTM16E	MTM16E	SD0122	LH33850	SD0122	LH33850
66L	L6	CS16-R	RE16-R	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
10T	T0T	CS16	RE16	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
8BL	L8	CS20	RE250	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A13	JA1	CS22-R	RE22-R	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A23	AJ2	CS22-R	RE22-R	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A23	AJ2	CS22-R	RE22-R	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A23	AJ2	CS22-R	RE22-R	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850
A31	TL0L	DPF72	SP77-T2	HSR6	HSR6	LTB20	NR	N105HDG	N40M-JMC	N40M-JMC	N40M-JMC	SD0122	LH33850	SD0122	LH33850

CODES:

2020 FLORIDA BUILDING CODE RESIDENTIAL
2020 FLORIDA FIRE PREVENTION CODE
2020 FLORIDA ACCESSIBILITY CODE
NEC NFPA 70 & FBCBE
ACI 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
ACI 308-19 PRECAST/PRECAST CAST-IN-PLACE CONCRETE CONSTRUCTION
ACI 530-19 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
2018 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
2018 WOOD FRAMED CONSTRUCTION MANUAL
APA PLYWOOD DESIGN SPECIFICATION
ASCE/SEI 7-16 AMERICAN SOCIETY OF CIVIL ENGINEERS

LIVE LOADS:	ROOF	20 PSF (REDUCIBLE)
	RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED	40 PSF
	BALCONIES	60 PSF
	STAIRS	40 PSF
	LIGHT PARTITIONS (DEAD LOAD), U.N.O.	20 PSF

CONCRETE STRENGTH @ 28 DAYS ALL CONCRETE UNLESS OTHERWISE INDICATED
PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY
(DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)

REINFORCING:	WELDED WIRE FABRIC SHALL CONFORM TO	ASTM A1064/A1064M
	ALL REINFORCING BARS	ASTM A615-40 40,000 PSI
	ALL STIRRUPS AND TIES	ASTM A615-40 40,000 PSI
	POLYPROPYLENE FIBERS FOR SLABS ON GRADE	MINIMUM 1.5 LBS. OF FIBER PER CUBIC YARD

CONCRETE	ASTM C90-01, STANDARD WEIGHT UNITS, fm=1500 PSI
MASONRY	MORTAR TYPE "S" 1800 PSI
UNITS:	CONCRETE GROUT 3000 PSI
	CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION

STRUCTURAL STEEL: ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.F.
SHOP AND FIELD WELDS: E70XX ELECTRODES
ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

WOOD FRAMING: BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O.

NO. 2 SOUTHERN YELLOW PINE (19% M.C.)
ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB
FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24
WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB
VERSA LAM BEAM Fb = 2900 PSI (2.0E)

WOOD ROOF TRUSSES:	DESIGN LOADS:	SHINGLE ROOF
	TOP CHORD LIVE LOAD:	20 PSF
	TOP CHORD DEAD LOAD:	10 PSF
	BOTTOM CHORD DEAD LOAD:	<u>10 PSF</u>
		40 PSF
	BOTTOM CHORD ATTIC LIVE LOAD:	10 PSF

SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

SHT NO:

COVER SHEET
FLOOR PLAN
ELEVATIONS
FOUNDATION PLAN
ELECTRICAL PLAN
TRUSS LAYOUT
DETAILS

S-1

WIND SPEED (ULTIMATE)	130 MPH
WIND SPEED (ALLOWABLE)	101 MPH
EXPOSURE CATEGORY	B
BUILDING CATEGORY	II
BUILDING TYPE	V
ENCLOSURE CLASSIFICATION	ENCLOSURE
INTERNAL PRESSURE COEFFICIENT	+/- 0.18

**THIS DRAWING AND DESIGN IS VALID
FOR 12 MONTHS AFTER THE DATE IT
IS SIGNED AND SEALED OR WHILE
CURRENT CODE IS VALID**

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESSIONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION, STOP...AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY UNDERSTAND THE REQUIREMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE.

DO NOT SCALE DRAWINGS FOR CRITICAL DIMENSIONS
INSTEAD CALL THE DESIGNER LISTED IN TITLE PAGE*



RANDOLPH WIGGINS, P.E.

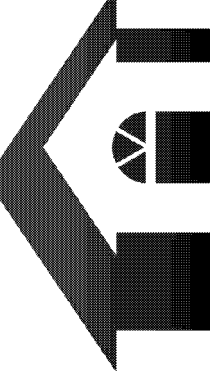


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