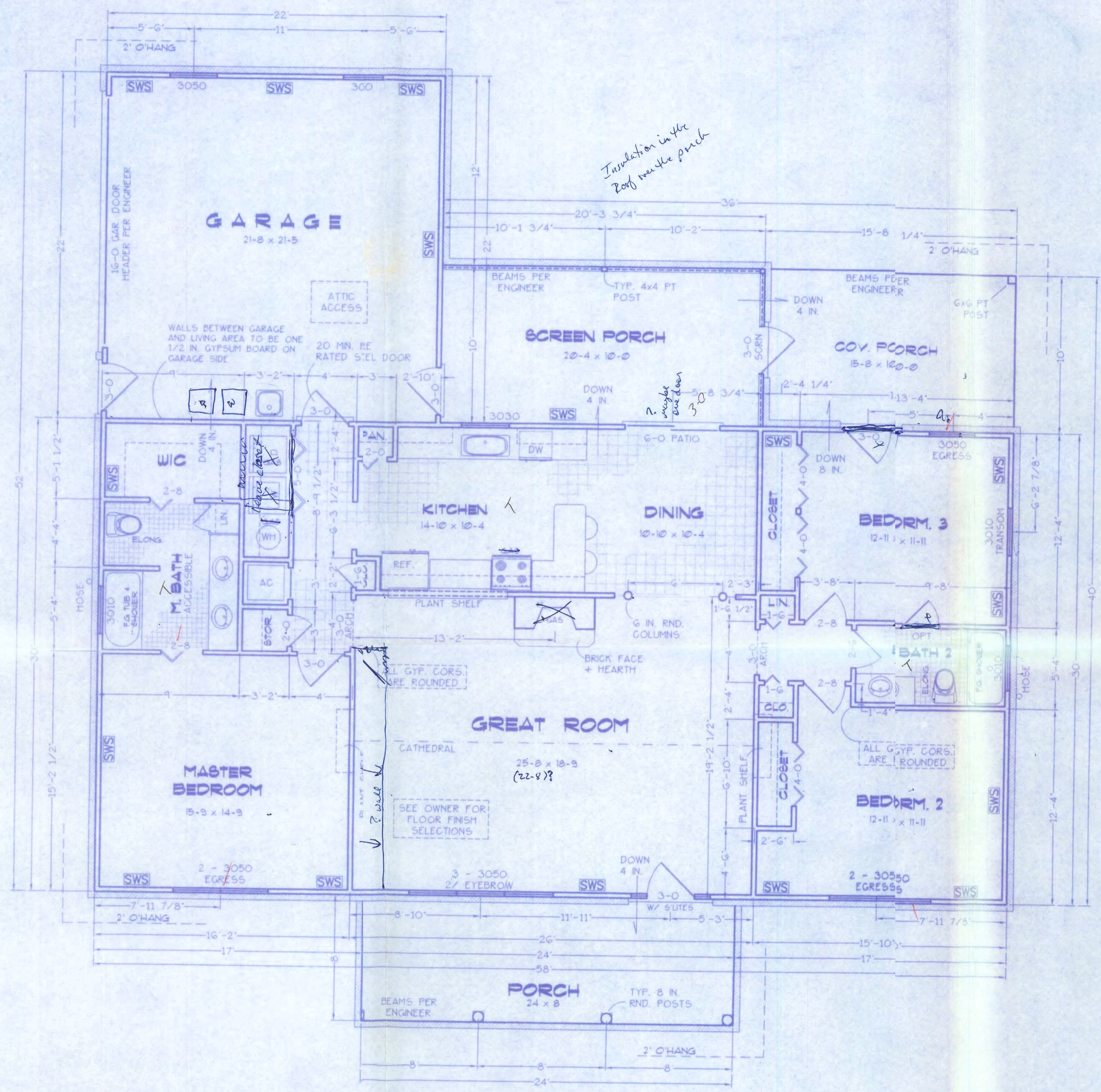


Troiano Residence

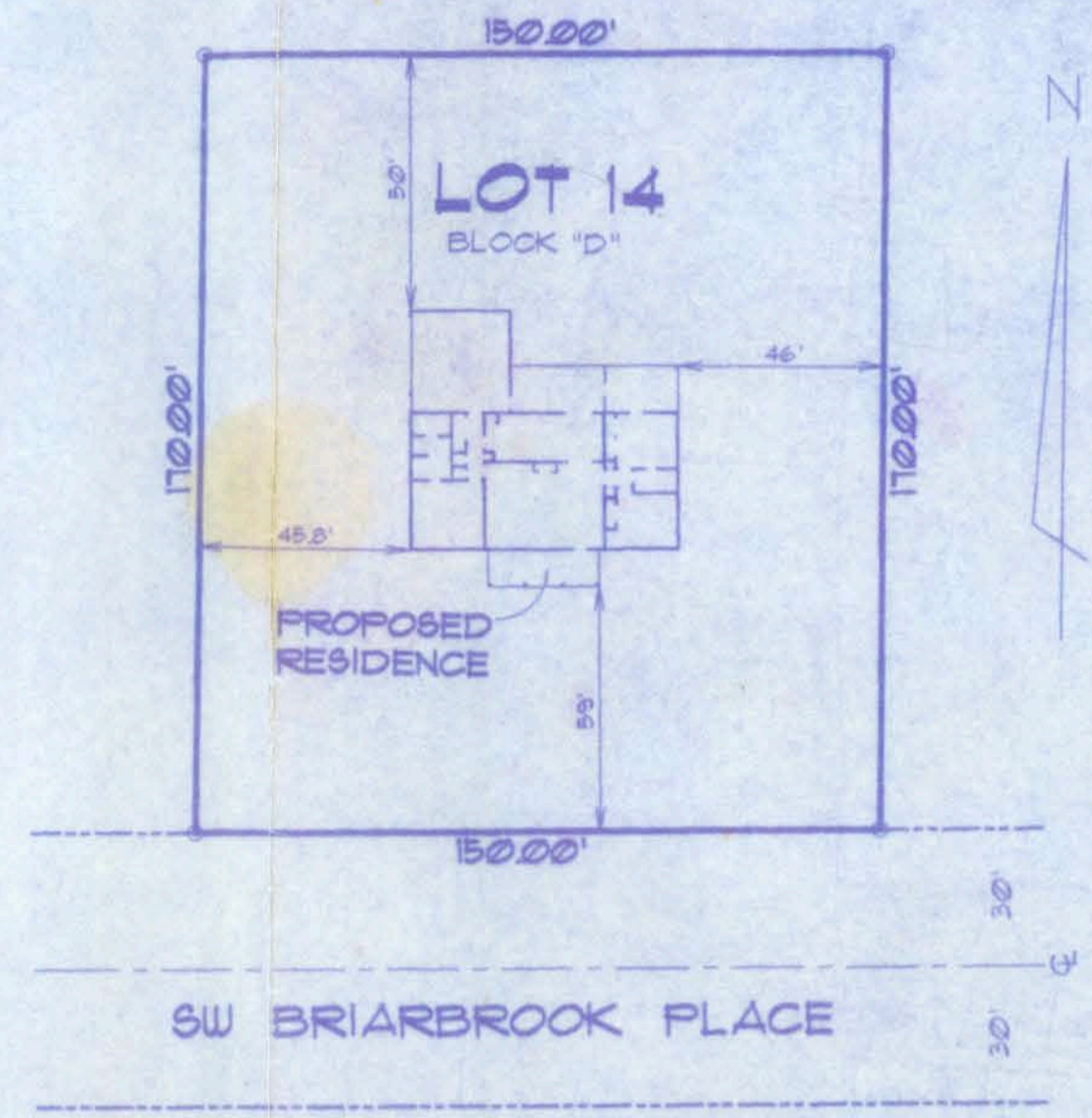


FLOOR PLAN
SCALE: 1/4 IN. = 1 FT.

DESCRIPTION:
LOT 14, BLOCK "D" OF
PICCADILLY PARK SOUTH,
A SUBDIVISION RECORDED
IN PLAT BOOK 4, PAGE 13
OF THE PUBLIC RECORDS
OF COLUMBIA CO., FLORIDA.

ADDRESS:
SW BRIARBROOK PLACE
LAKE CITY, FLORIDA

- NOTES:
- 1) BUILDING LOCATION PER OWNER OR CONTRACTOR.
 - 2) LOT DIMENSIONS TAKEN FROM SURVEY FURNISHED BY OWNER.
 - 3) BUILDER SHALL VERIFY ALL APPLICABLE SETBACKS, REGULATIONS AND DEED RESTRICTIONS.



SITE PLAN
SCALE: 1 IN. = 40 FT.

AREA SUMMARY

CONDITIONED	1740 SF
GARAGE	484 SF
FRONT PORCH	192 SF
SCRN. PORCH	203 SF
REAR COV. PORCH	151 SF
ROOF	2716 SF

SWS - Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

Index to Sheets

SHEET A-1	SITE PLAN + FLOOR PLAN
SHEET A-2	ELEVATIONS + GEN. NOTES
SHEET A-3	ELEVATIONS
SHEET A-4	FOUNDATION + SECTIONS
SHEET A-5	ELECTRICAL
SHEET S-1	WIND ENGINEERING

A-1

WINDLOAD ENGINEER: Mark Disoway, PE No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

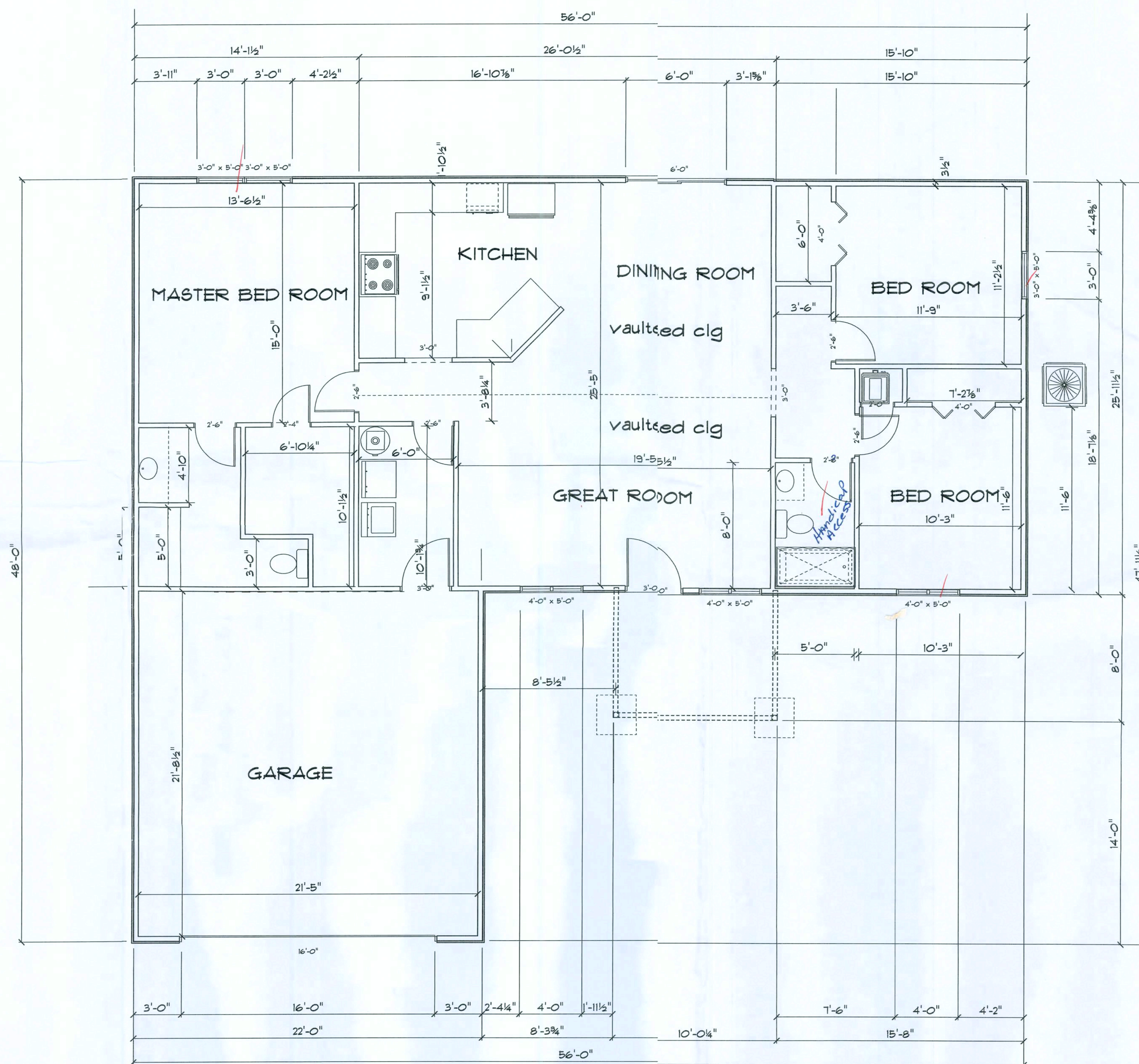
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: LOT 14, BLK D PICCADILLY PARK SOUTH SUBDIVISION
SW BRIARBROOK PLACE
LAKE CITY, FLORIDA

Job No.: 612078
11 Dec 06

FILE: OG-040	TROIANO RESIDENCE	SHEET: 1 of 5
DATE: 12-4-06		CAD FILE: OG040
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Segerwood Dr., Lake City, FL 32024 Phone: (386) 755-5881	REV:
CHECK: T A D		REV:

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

MAIN FLOOR PLAN

1456 Heated and Cooled
80 Porches
484 Garage
2020 Total

HOMETOWN HOMES

43 ADDRESS:
LOT CANNON CREEK
COLUMBIA COUNTY

DRAWN BY: JR
NORTH FLORIDA FRAMING AND TRUSSES
DRAFTING DEPT. (850) 623-3742

PRINTED DATE:
August 3, 2007

DRAWN BY: CHECKED BY:

DESIGNED BY:
Z6868

FINALS DATE:
06 / OCT / 04

PROJECT NUMBER:

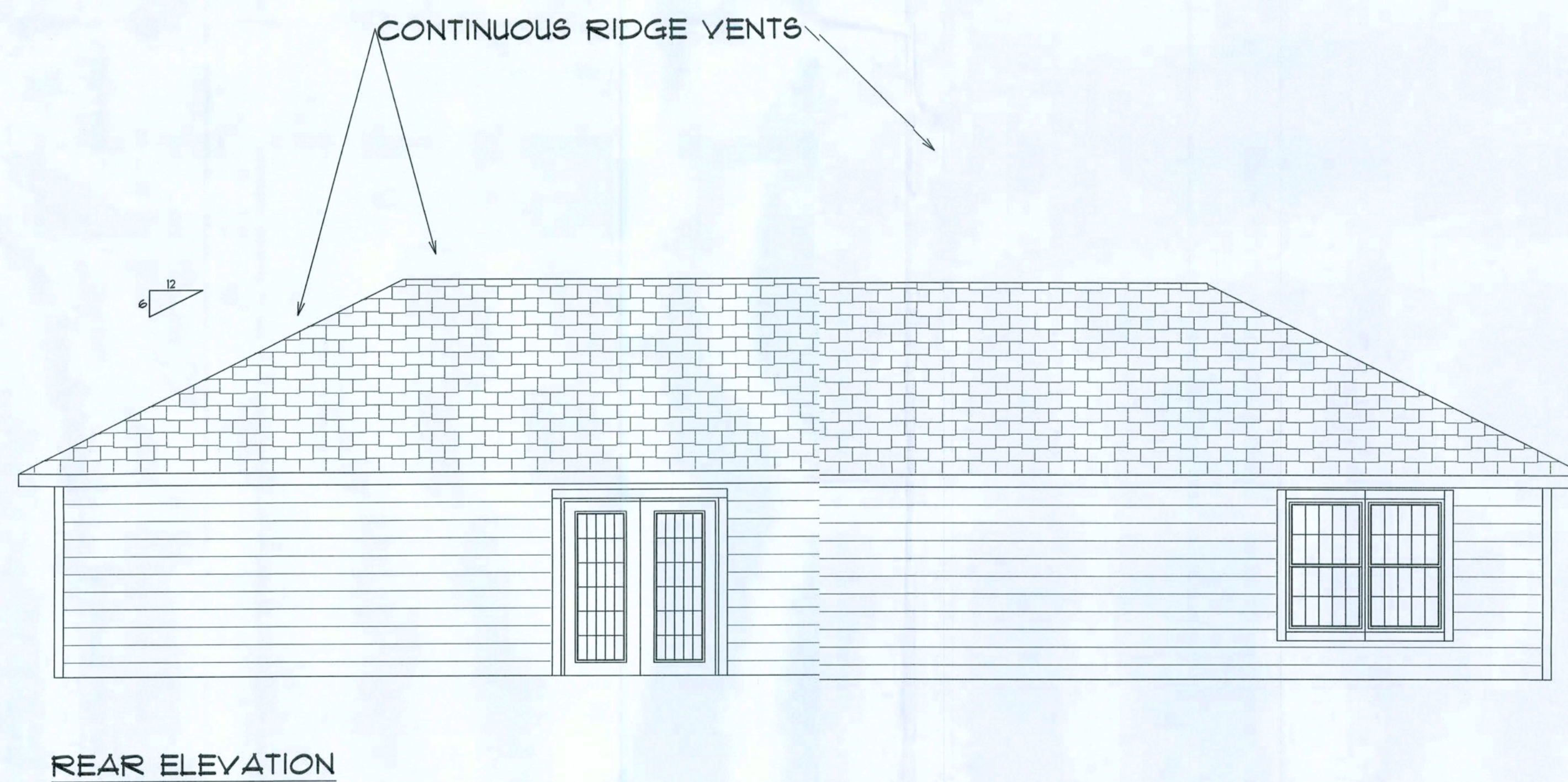
DRAWING NUMBER

A-1 FLOOR

1 OF 1 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



HOMETOWN HOMES

ADDRESS:
LOT CANNON CREEK
COLUMBIA COUNTY

DRAWN BY: JFB
NORTH FLORIDA, FRAMING AND TRUSSES
DRAFTING DEP. (386) 623-3742

PRINTED DATE:
August 21, 2007

DRAWN BY: CHECKED BY:

DESIGNED BY:

FINALS DATE
06 / OCT / 04

PROJECT NUMBER:

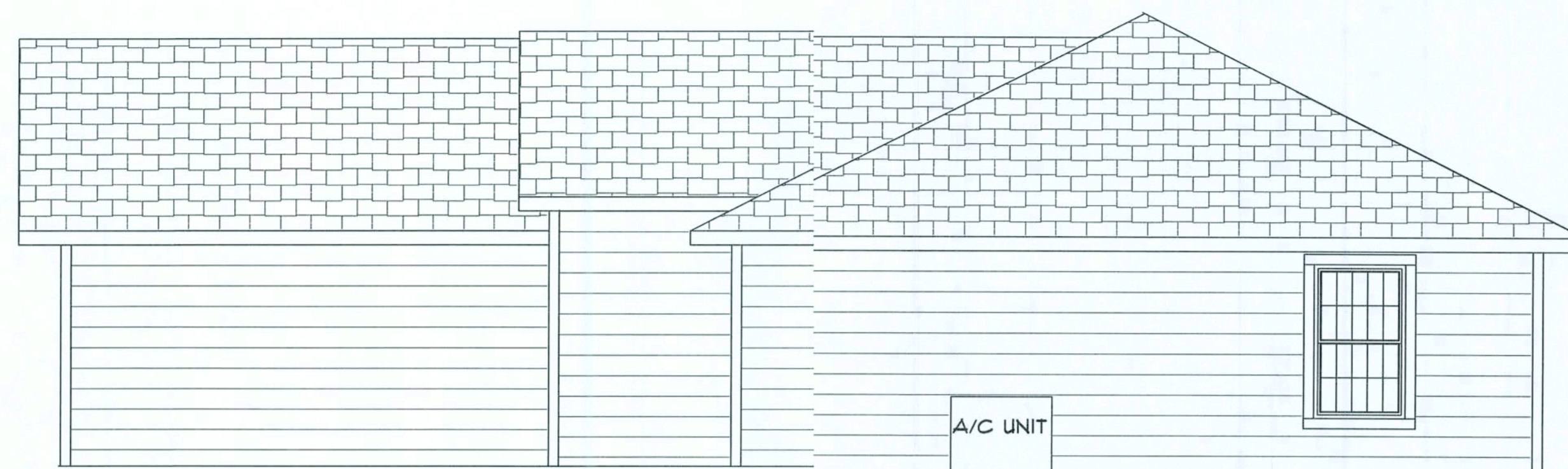
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A-2 ELEVATIONS

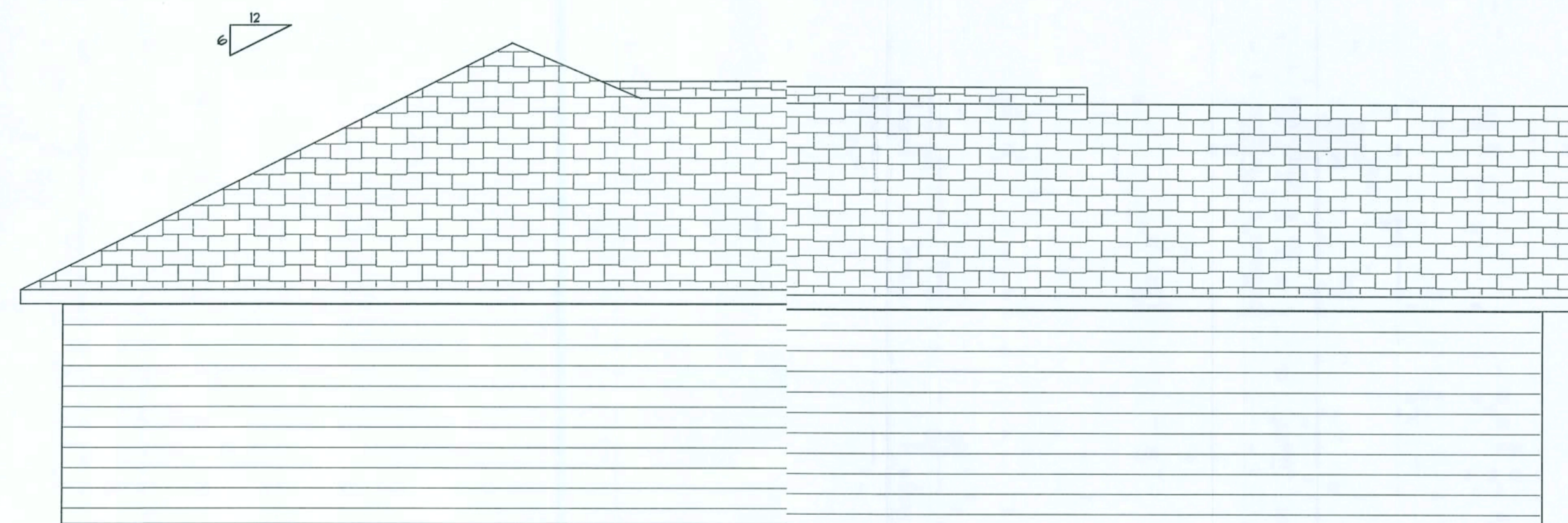
1 OF 4 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



RIGHT ELEVATION



LEFT ELEVATION

HOMETOWN HOMES

ADDRESS:
LOT CANYON CREEK
COLUMBIA COUNTY
DRAWN BY: JB
NORTH FLORIDA FRAMING AND TRUSSES
DRAFTING DEPT. 386) 623-3742

PRINTED DATE:
August 11, 2007

DRAWN BY: CHECKED BY:

DESIGNED BY:

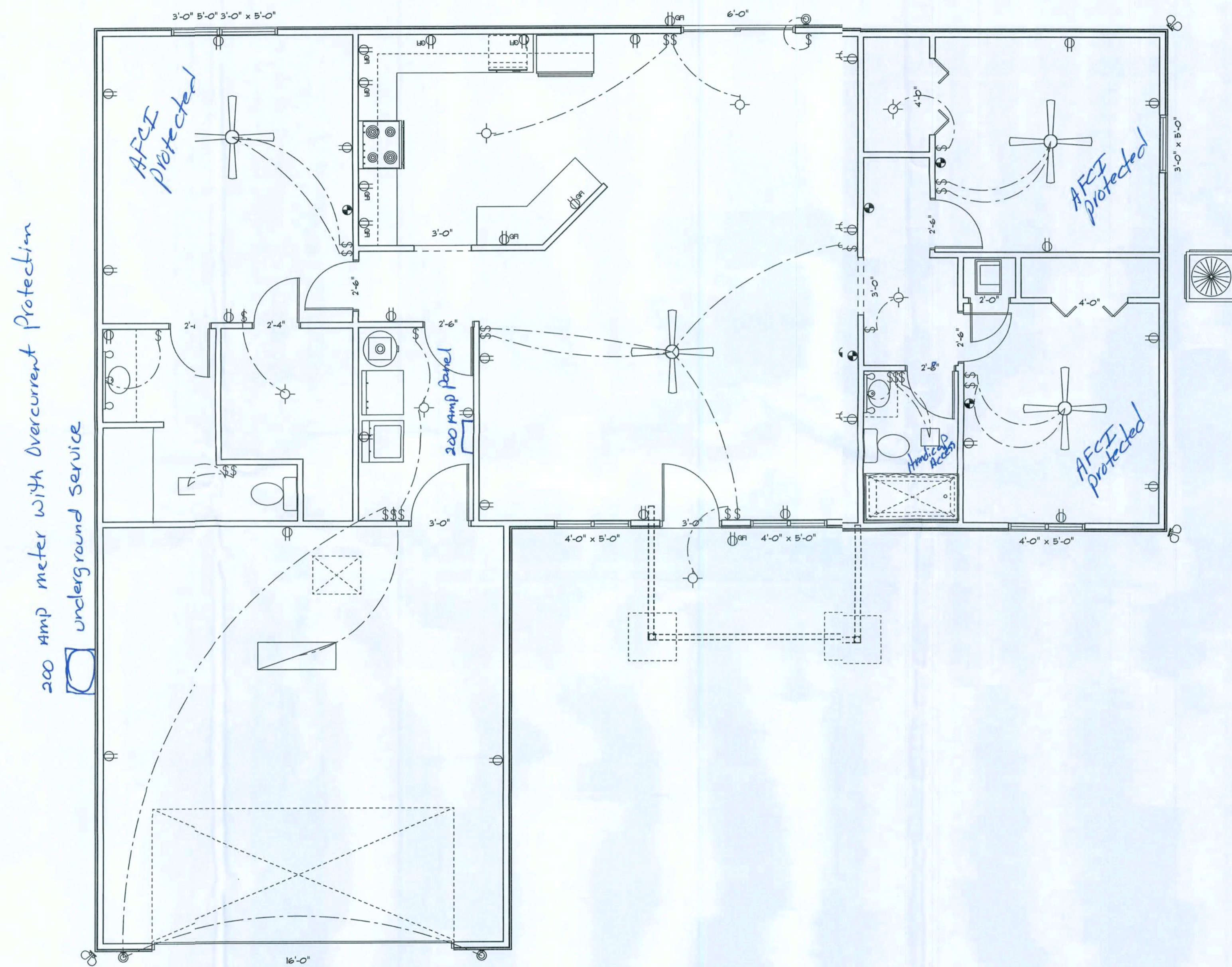
FINALES DATE:
06 / OCT / 04

PROJECT NUMBER:

DRAWING NUMBER

A-3 ELEVATIONS

1 OF 4 SHEETS



ELECTRICAL	SYMBOL
ceiling fan globe 1	
ceiling lamp large	
ceiling light vent square	
double spotlight	
track light	
wall mount 1	
light	
outlet	
outlet gfi	
smoke detector	
switch	

ELECTRICAL PLAN

HOMETOWN HOMES

ADDRESS:
LOT 1 ANNON CREEK
COLUMBIA COUNTY

DRAWN BY: JFB
NORTH FLORIDA FRAMING AND TRUSSES
DRAFTING DEPT. (386) 623-3742

PRINTED DATE:
August 21, 2007

DRAWN BY: CHECKED BY:

DESIGNED BY:

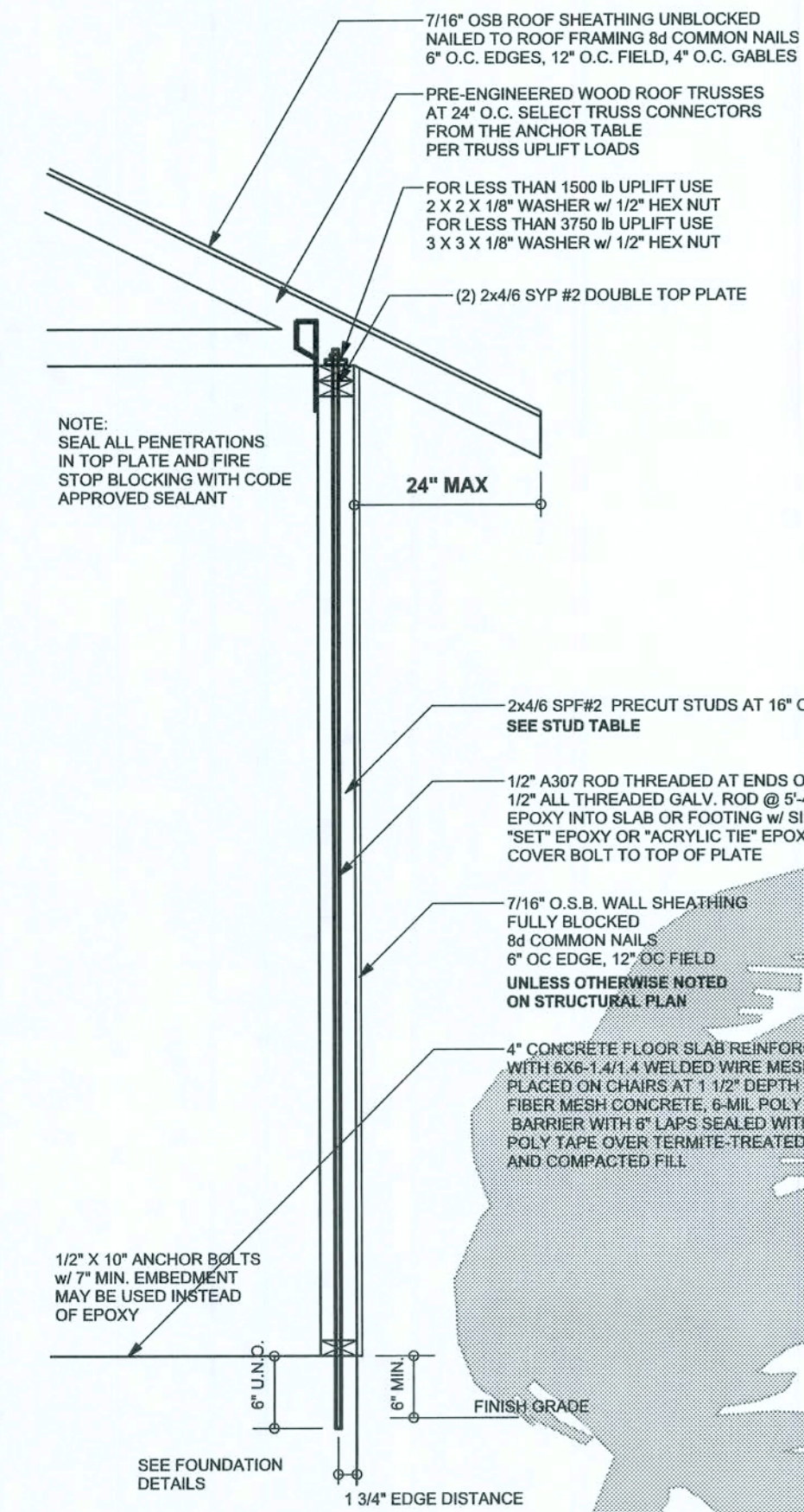
FINALS DATE:
06 / OCT / 07

PROJECT NUMBER:

DRAWING NUMBER

A-4 ELEC.

1 OF 4 SHEETS

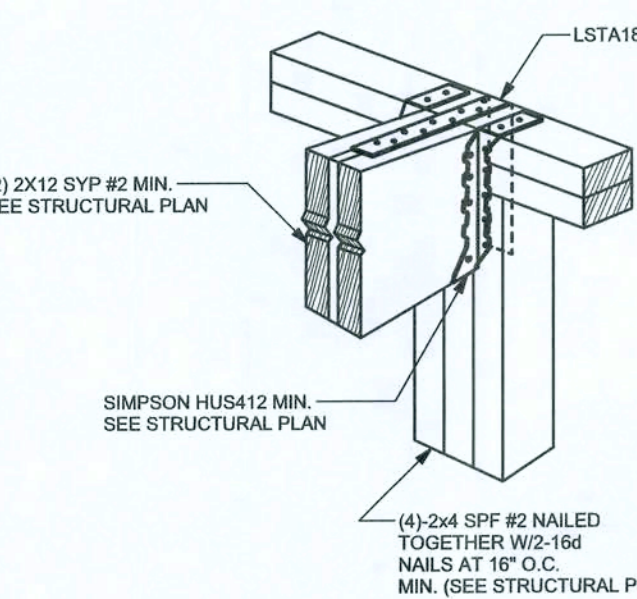


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

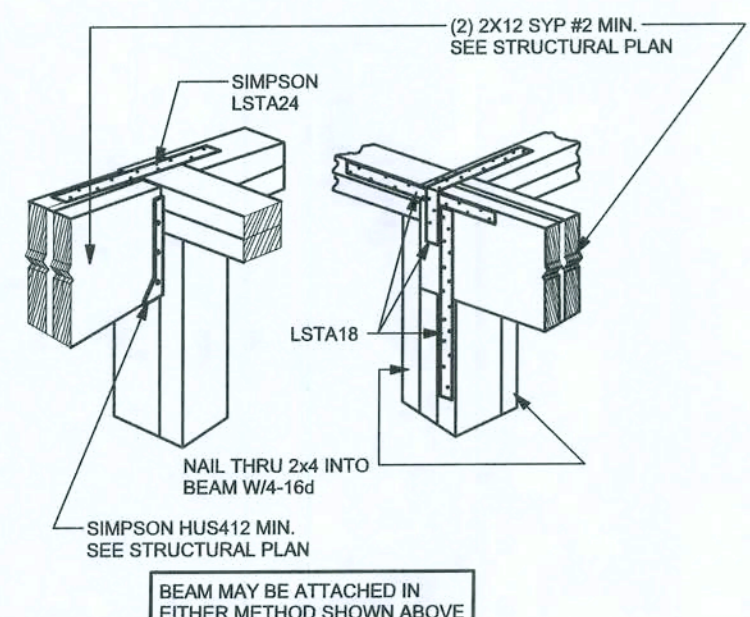
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-0" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

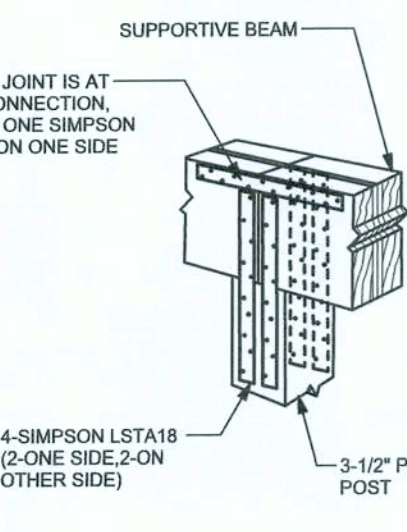


BEAM MID-WALL CONNECTION DETAIL
SCALE: N.T.S.

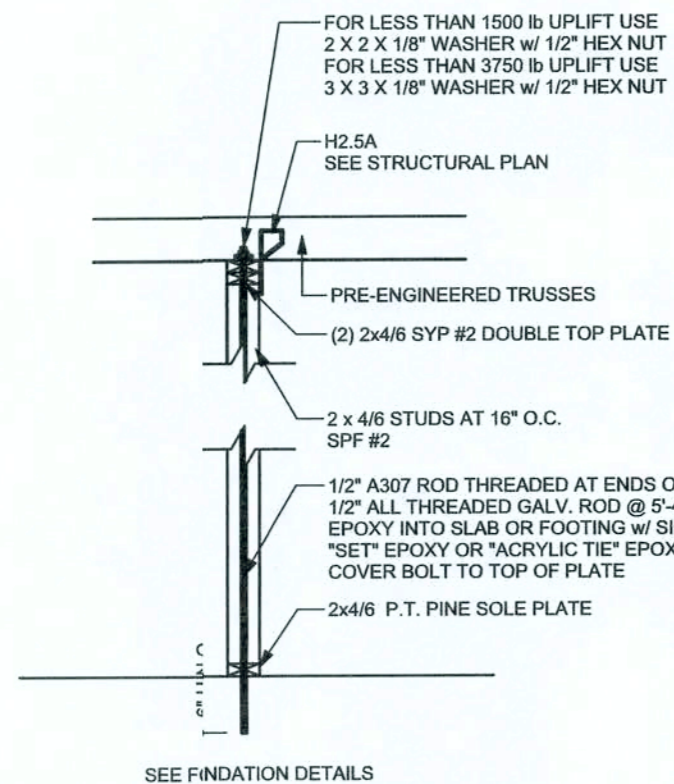


BEAM CORNER CONNECTION DETAIL
SCALE: N.T.S.

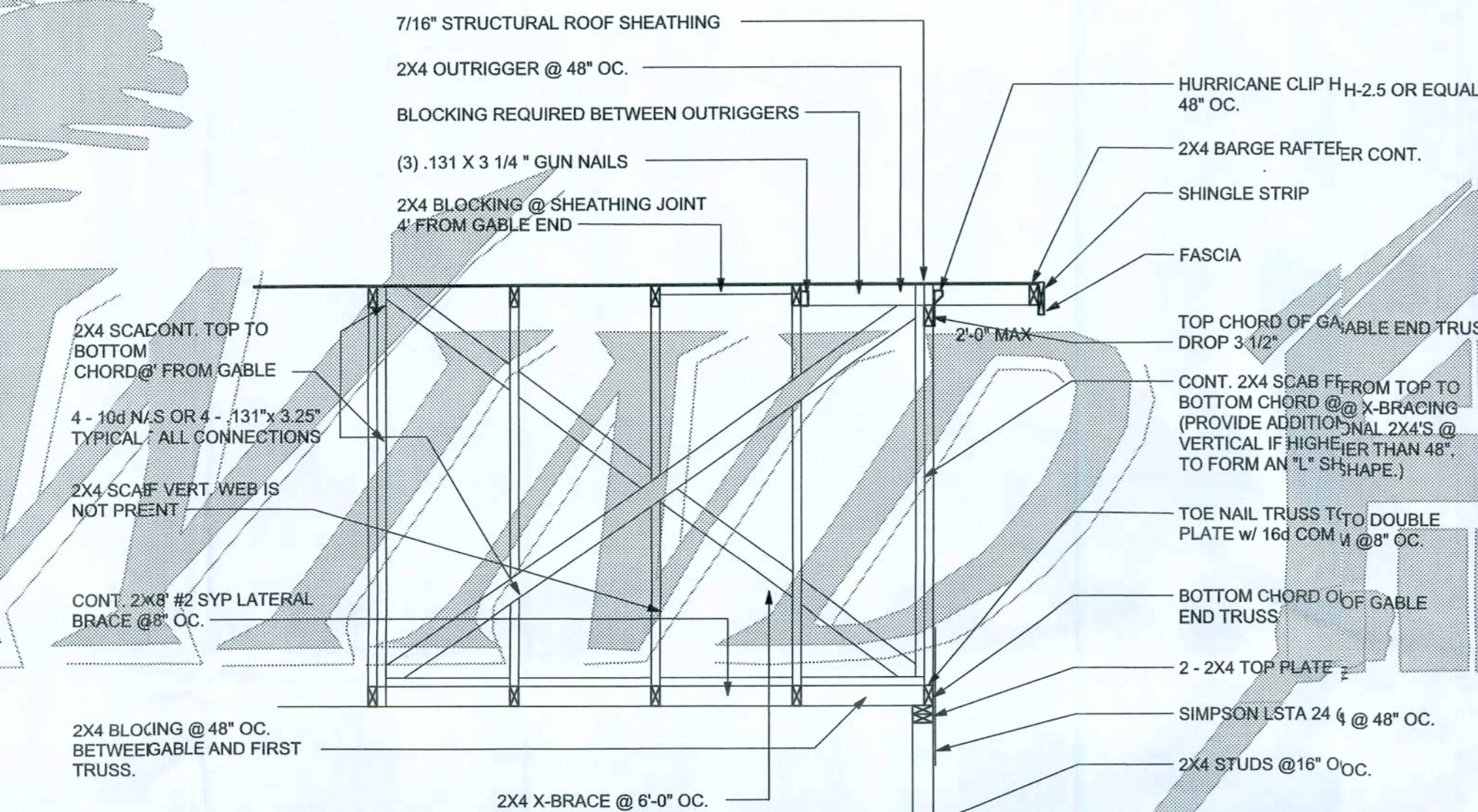
SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM
SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL
SCALE: N.T.S.

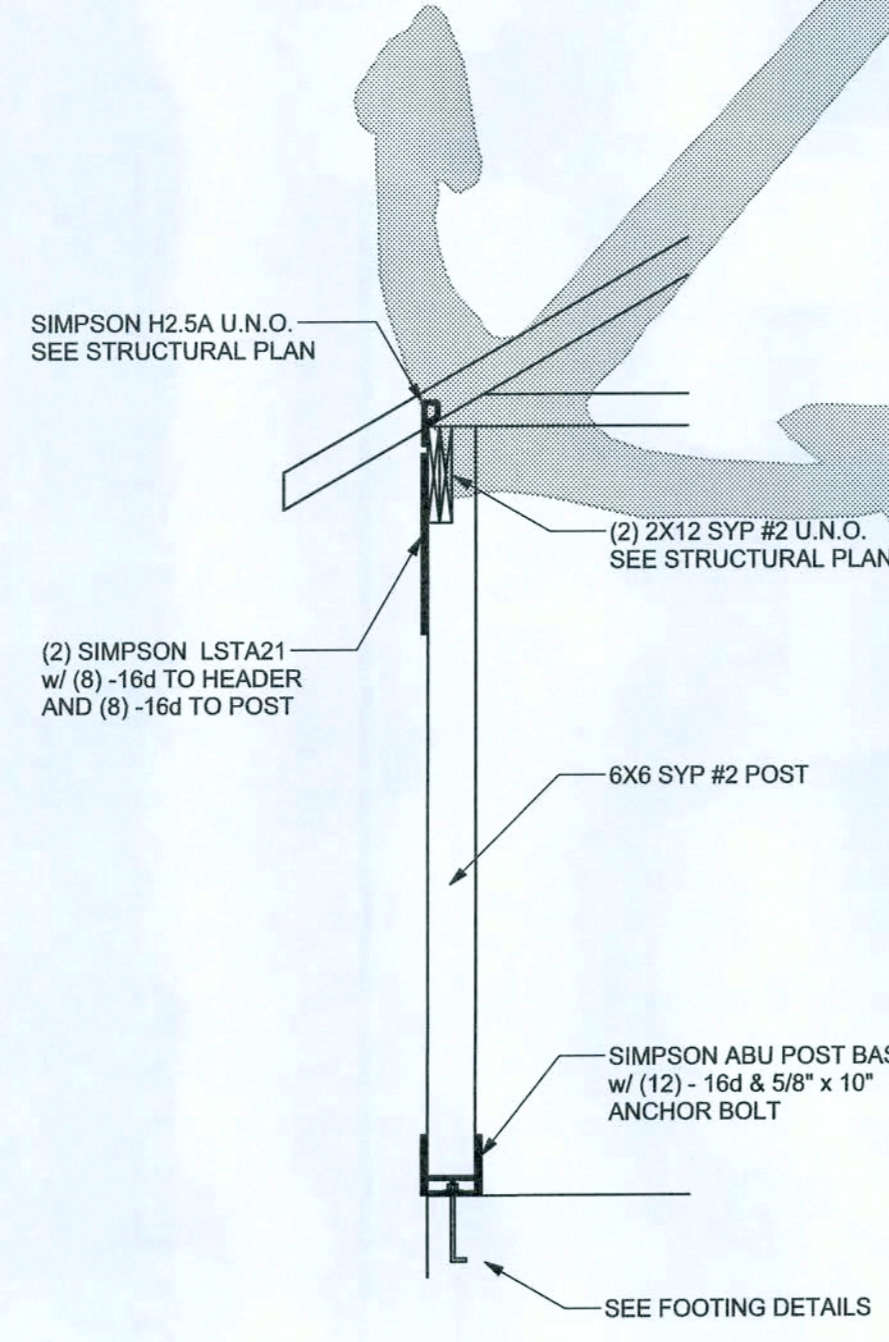


INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

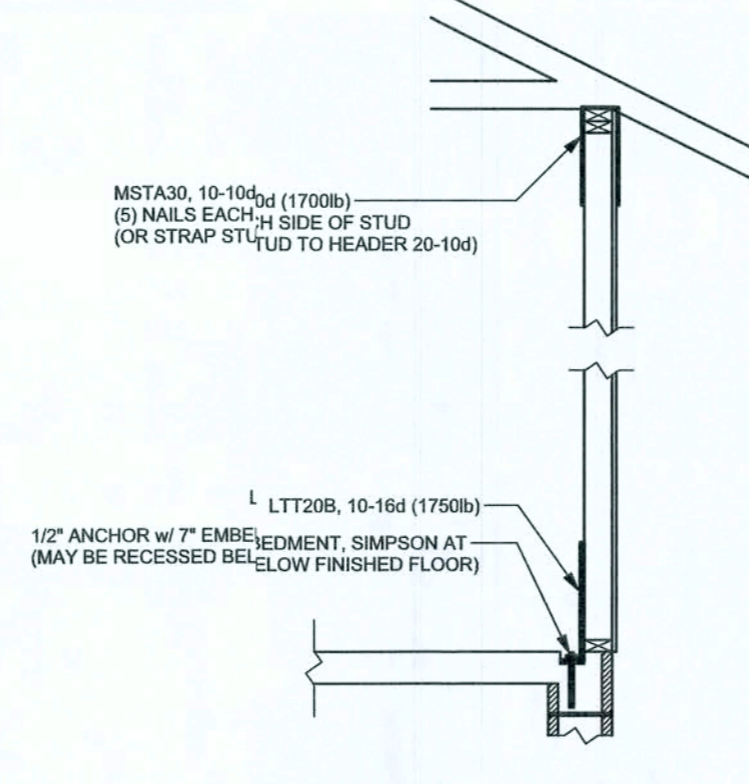


TYPICAL GABLE END (X-BRACING)

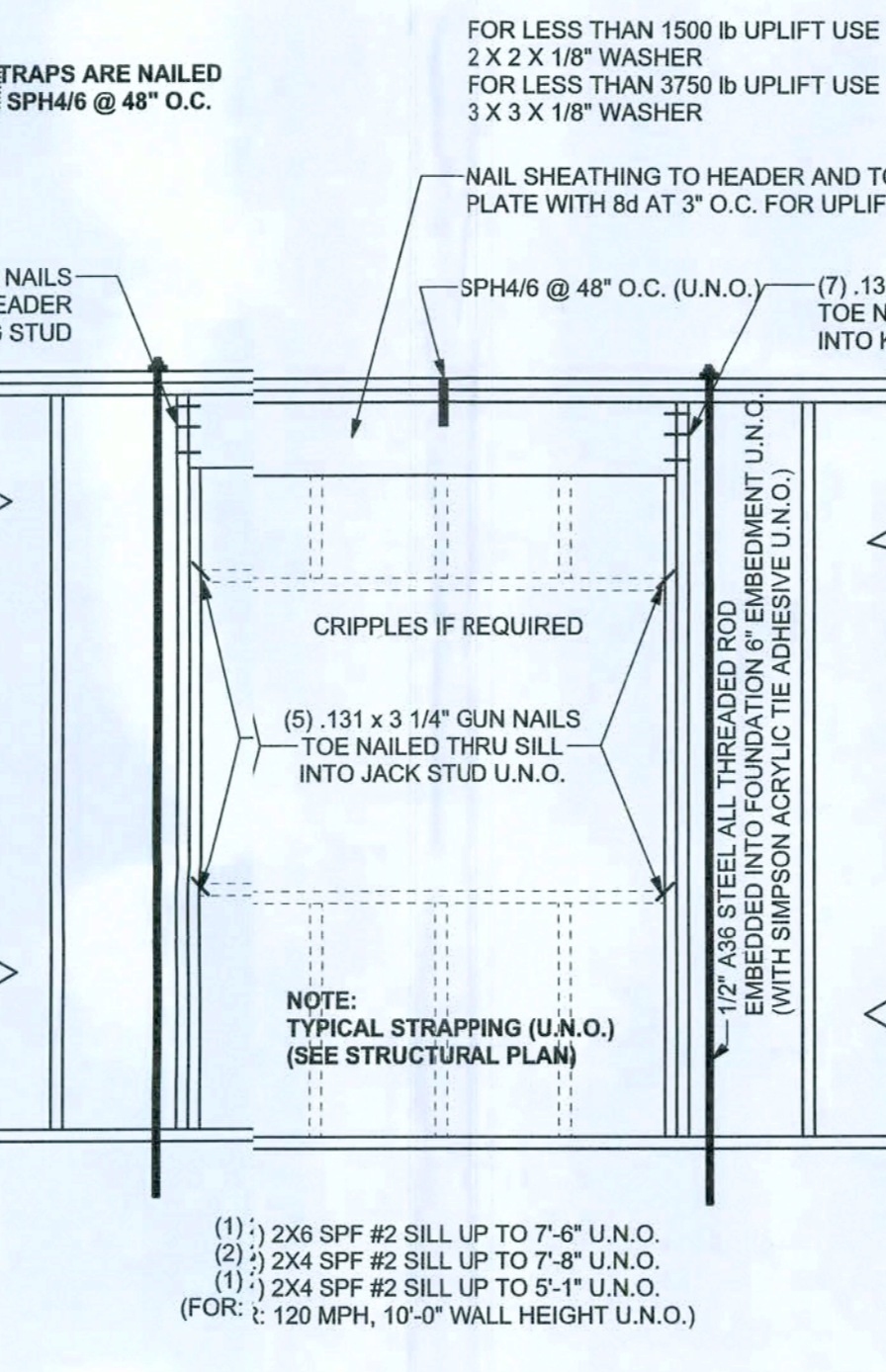
ALL MEMBERS SHALL BE SYP



TYPICAL PORCH POST DETAIL
SCALE: 1/2" = 1'-0"



**ALTERNATE WALL TIE CONNECTION WHERE
THREADED ROD CANNOT BE PLACED IN WALL**
SCALE: 1/2" = 1'-0"



TYPICAL 1 STORY HEADER STRAPPING DETAIL
SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 305	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 850	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LGT2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10580	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4		6-10d, 1 1/2"	
< 1240	< 1065	SPH4		10-10d, 1 1/2"	
< 885	< 760	SP6		6-10d, 1 1/2"	
< 1240	< 1065	SPH6		10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO STUDS
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	16-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HDS4	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1450	< 1400	/PAHD42	16-16d		
< 3335	< 3335	HPAH222	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ³ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED WITH THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDERS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 15LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET UPLIFT LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 6" W1.4 x W1.4, F_y = 60KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185 LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 48" DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2400psi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL, FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS, MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSERVED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES [TABLE R301.2(2)]

Zone	Effective Wind Area (ft ²)		
1	19.9 -21.8	18.1	-18.1
2	19.9 -42.1	18.1	-29.1
3	19.9 -42.1	18.1	-29.1
4	21.8 -23.6	18.5	-20.4
5	21.8 -29.1	18.5	-22.6

Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft ²)		
8x7 Garage Door	19.5	-21.3
16x7 Garage Door	18.5	-20.4

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	20 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
	40 PSF (DECKS)
	60 PSF (EXTERIOR BALCONIES)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SCFPA
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
PE No. 53915, 236 868, Lake City, FL
32056, 386-754419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of this plan, relating to wind engineering comply with section R301.2.1, Florida building code residential2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

21 AUG 07
SEAL

Richard Keen

Spec House
Lot 43
Cannon Creek Place S/D

ADDRESS:
Lot 43 Cannon Creek Place S/D
Columbia County, Florida

MarkDisoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
August 21, 2007

DRAWN BY
Ben Sparks

CHECKED BY:

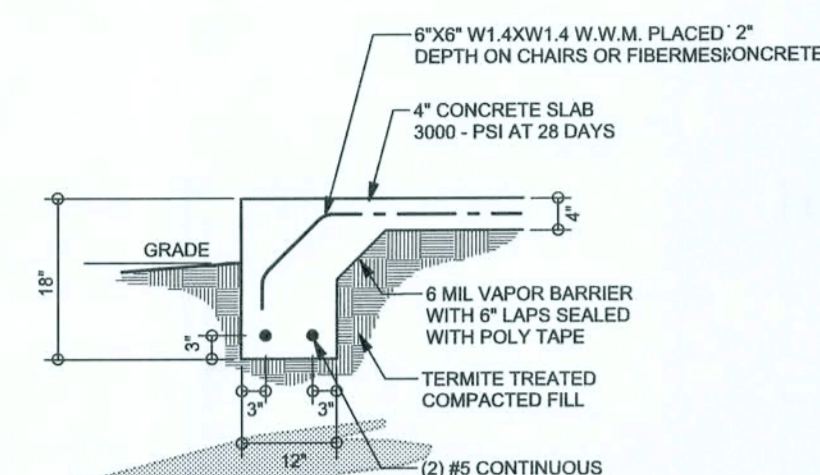
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21 / Aug / 07

JOE NUMBER:
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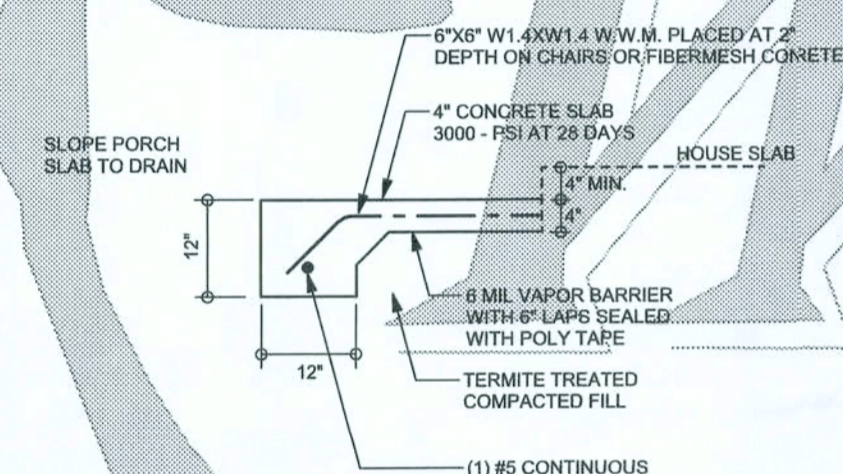
DRAWING NUMBER
S-1
OF 3 SHEETS

REVISIONS	

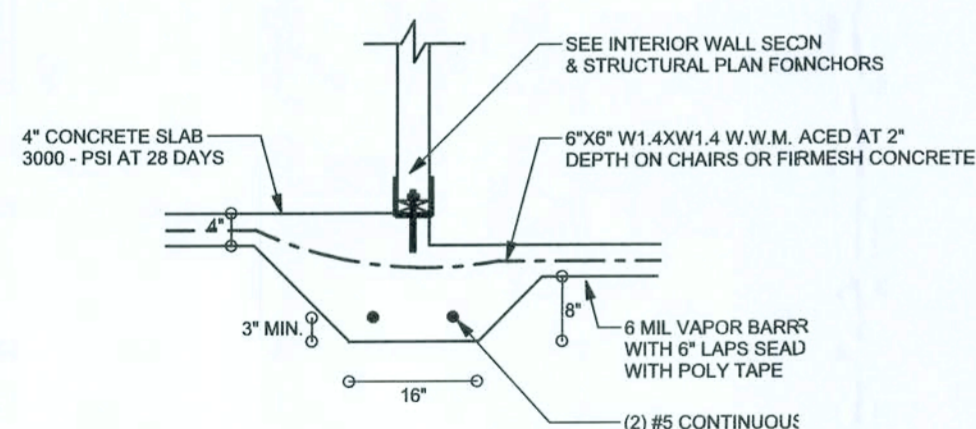
SCOTPAW
ARCHITECTURAL DESIGN SOFTWARE



F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"

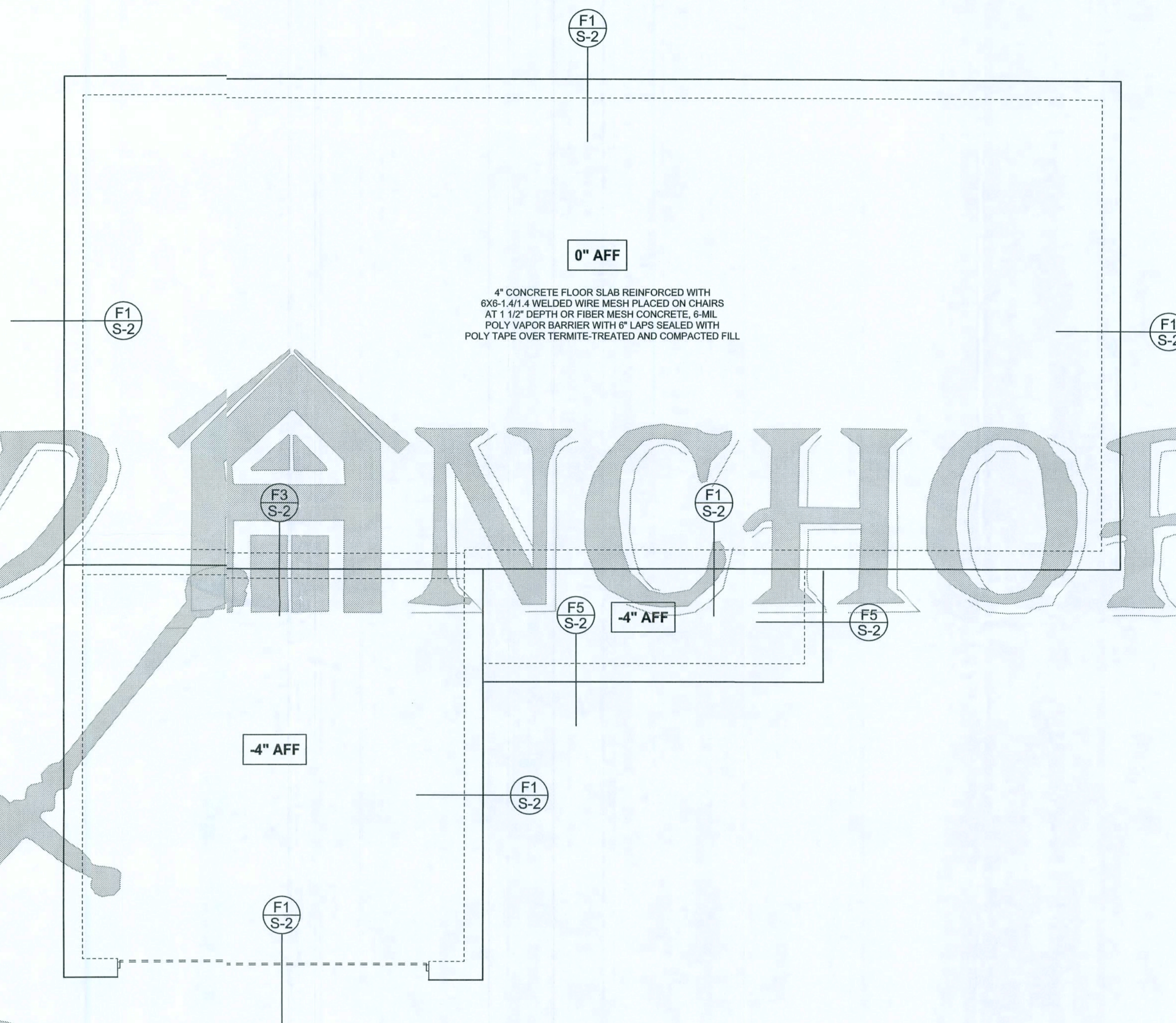


F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 33015, F.G. 868, Lake City, FL
32056, 386-754-819

DIMENSIONS:
Stated dimension supercede scaled dimensions. Rule all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 204, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 33015

Mark Disoway
2/10/07
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Richard Keen

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PRINTED DATE:
August 21, 2007

DRAWN BY: Ben Sparks
CHECKED BY:

FINALS DATE:
21 / Aug / 07

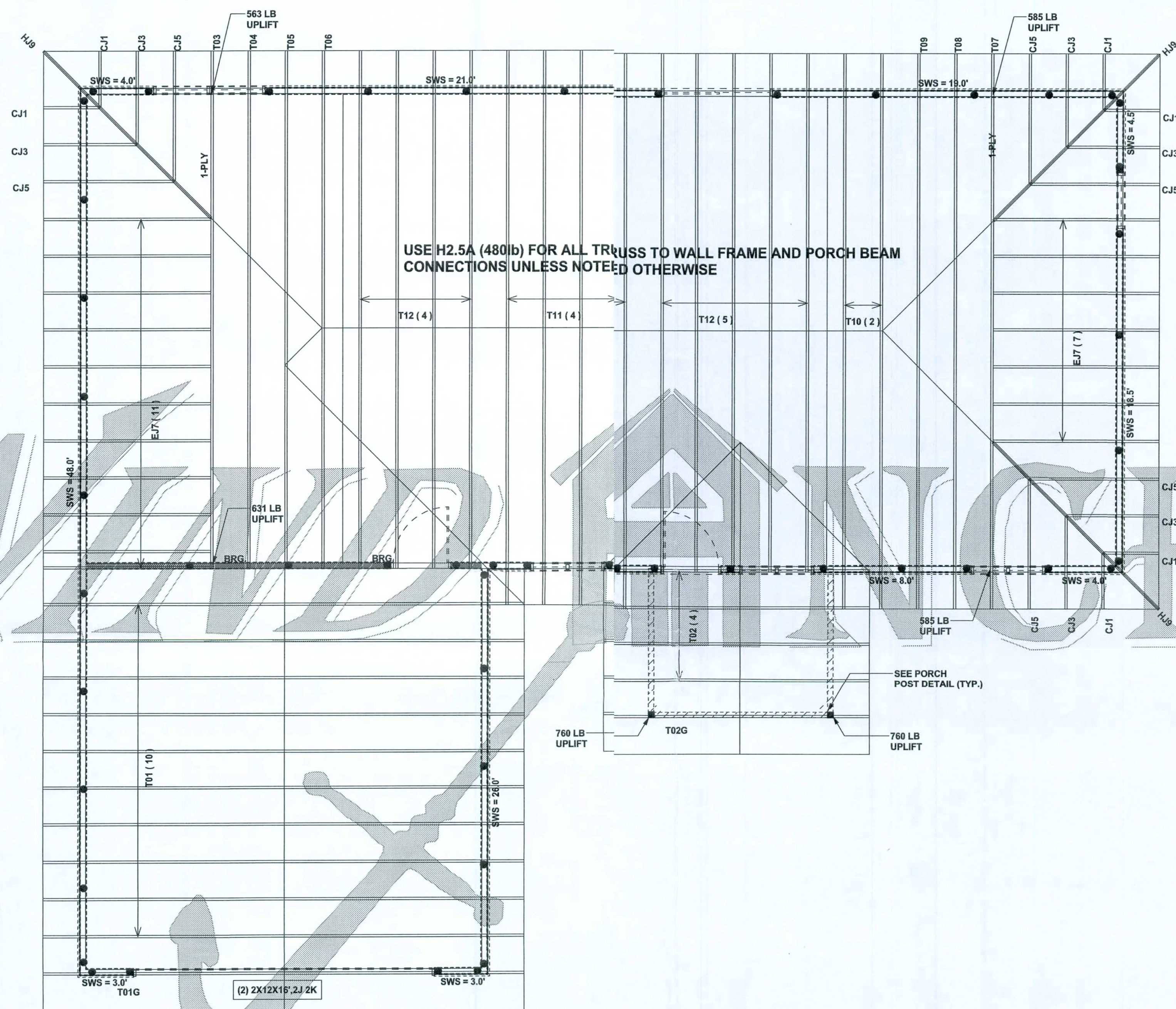
JOBNUMBER:
08152

DRAWING NUMBER
S-2

OF 3 SHEETS

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-1-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1 ST FLOOR EXTERIOR WALL
SWS = 0.0'	2 ND FLOOR EXTERIOR
IBW	1 ST FLOOR INTERIOR BEARING WALLS
IBW	2 ND FLOOR INTERIOR BEARING WALLS

THREADED ROD LEGEND

- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	34.5'	97.0'
LONGITUDINAL	15.5'	62.0'

HEADER LEGEND

- (2) 2X12X0', 1J 1K → HEADER/BEAM CALL-OUT (U.N.O.)
- ↑ NUMBER OF KING STUDS (FULL LENGTH)
- ↑ NUMBER OF JACK STUDS (UNDER HEADER)
- ↑ SPAN OF HEADER
- ↑ SIZE OF HEADER MATERIAL
- ↑ NUMBER OF PLIES IN HEADER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L249166)

WINDLOAD ENGINEER: Mark Discoway,
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32056, 386-754-5419

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CERTIFICATION: I hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with section 6301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specific location.

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P.E. 53915

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2/15/07
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OF : SHEETS